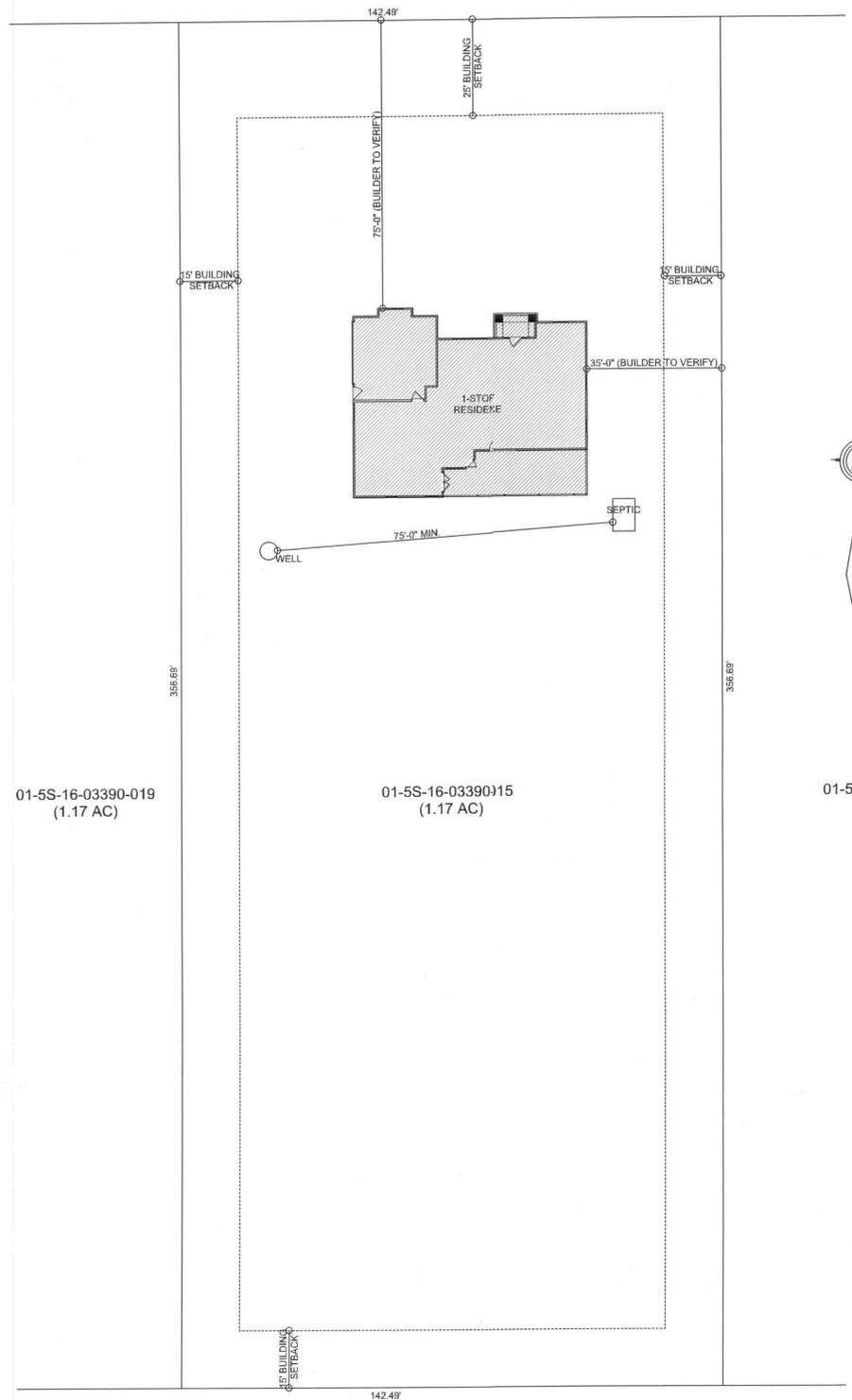


REVISIONS	



01-5S-16-03390-019  
(1.17 AC)

01-5S-16-03390-015  
(1.17 AC)

01-5S-16-03390-016  
(1.17 AC)

**SITE PLAN**  
SCALE: 1" = 20'-0"



# JAKE & ANNA ROCHE RESIDENCE

**JAKE & ANNA  
ROCHE  
RESIDENCE**

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ebeamsley@bellsouth.net

PRINTED DATE:  
May 21, 2007

DRAWN BY: Evan Beamsley      CHECKED BY:

DESIGNED BY:  
*Evan Beamsley*

FINALS DATE:  
21 / May / 07

JOB NUMBER:  
705105

DRAWING NUMBER:  
A-0

OF 4 SHEETS

REVISIONS	

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



FRONT ELEVATION  
SCALE: 1/4" = 1'-0"



LEFT ELEVATION  
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION  
SCALE: 1/4" = 1'-0"



REAR ELEVATION

**JAKE & ANNA  
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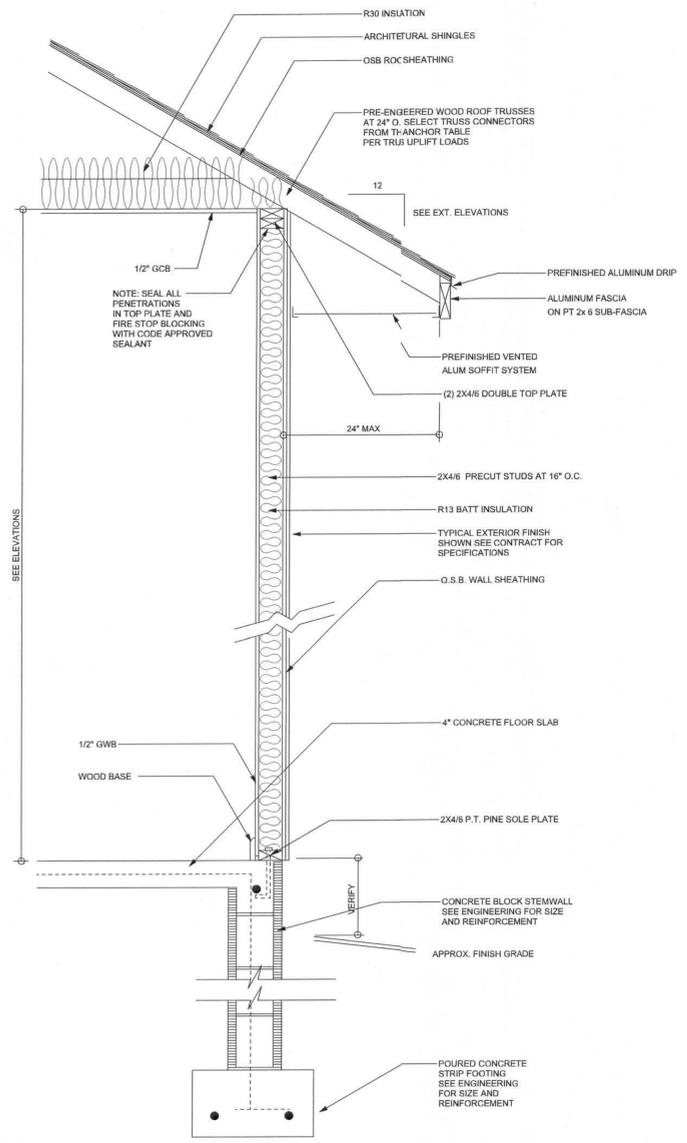
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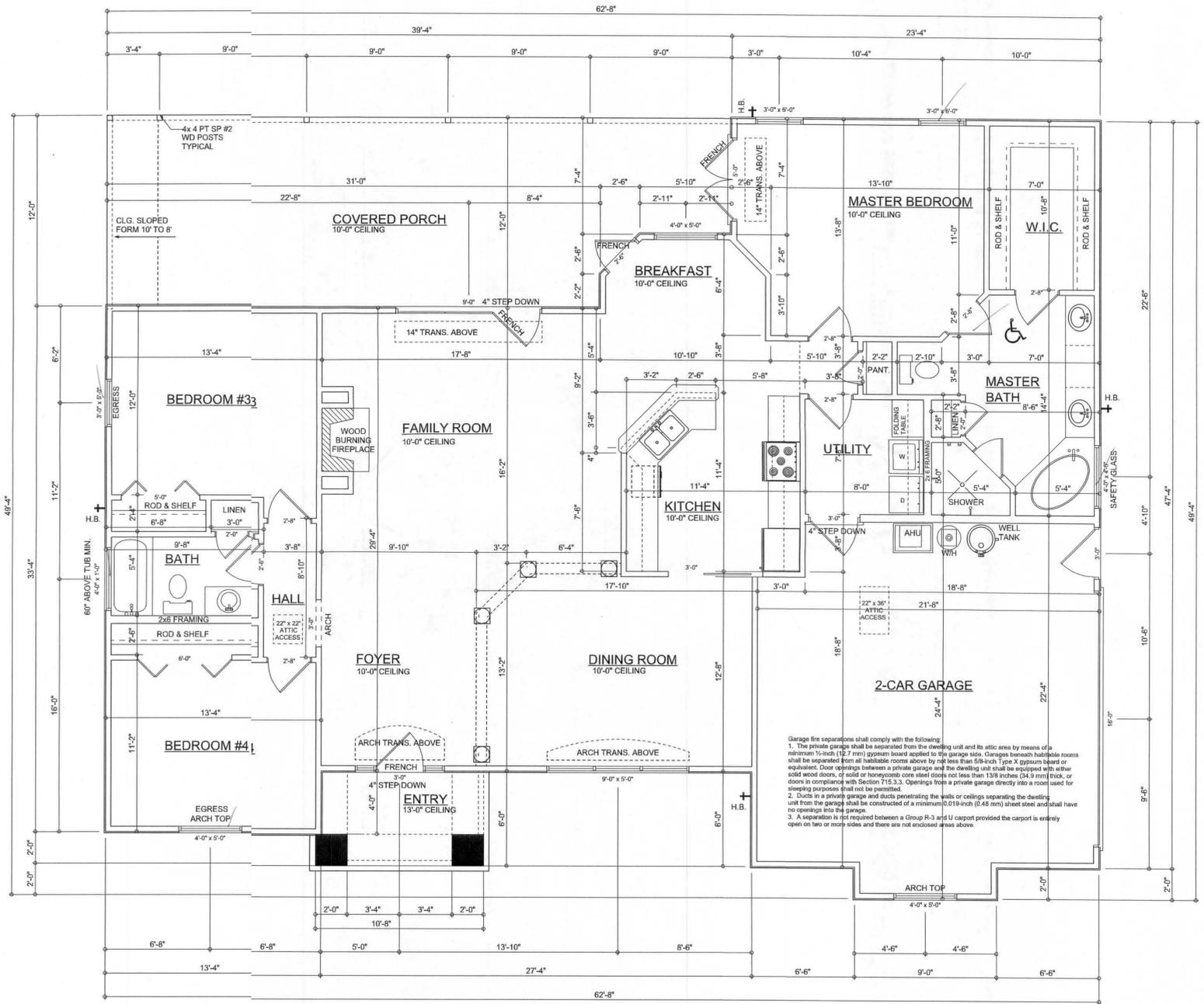
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705105

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A-1  
OF 4 SHEETS

REVISIONS	



**TYPICAL DESIGN WALL SECTION**  
NON - STRUCTURAL DATA  
SCALE: 1" = 1'-0"



**FLOOR PLAN**  
SCALE: 1/4" = 1'-0"  
NOTE: ALL CEILING HEIGHTS SHALL BE 8'-0" UNLESS OTHERWISE NOTED

**AREA SUMMARY**

LIVING AREA	1859	S.F.
GARAGE AREA	498	S.F.
COVERED PORCH AREA	436	S.F.
ENTRY PORCH AREA	63	S.F.
<b>TOTAL AREA</b>	<b>2856</b>	<b>S.F.</b>

Garage fire separations shall comply with the following:  
 1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 5/8-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 1 3/8 inches (34.9 mm) thick, or doors in compliance with Section 713.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.  
 2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.  
 3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.

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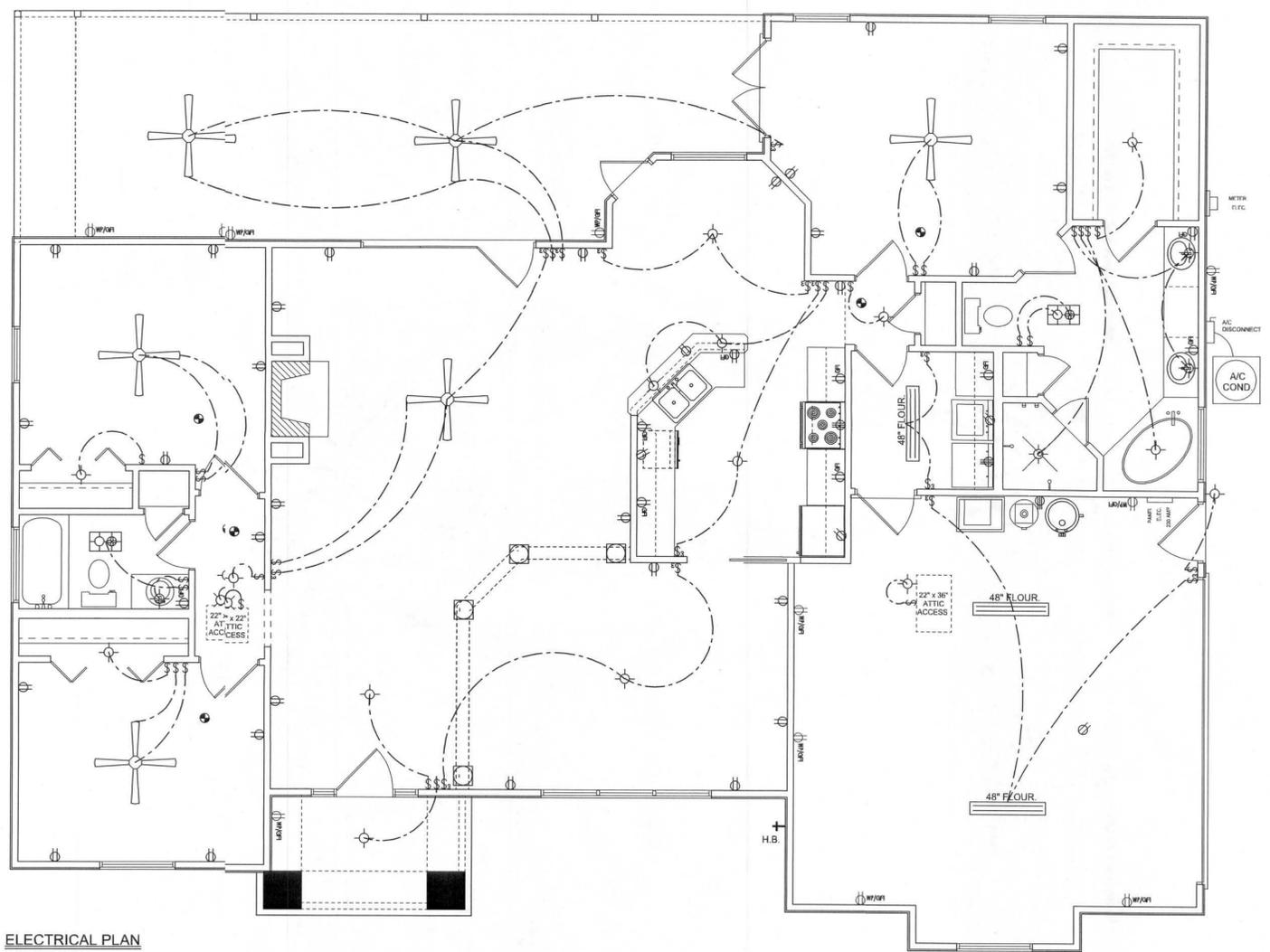
A-2  
OF 4 SHEETS

REVISIONS	



- ELECTRICAL PLAN NOTES**
- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
  - E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
  - E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
  - E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
  - E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
  - E -6 ELECTRICAL CONTRN SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
  - E -7 ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD ) TO BE DETERMINED BY POWER COMPANY.
  - E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
  - E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
  - E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER



**ELECTRICAL PLAN**  
SCALE: 1/4" = 1'-0"

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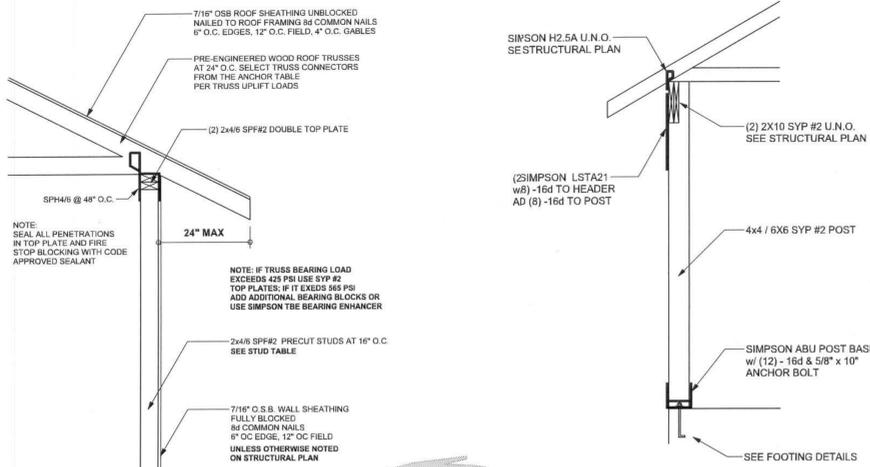
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*Evan Beamsley*

ISSUES DATE:  
2 / May / 07

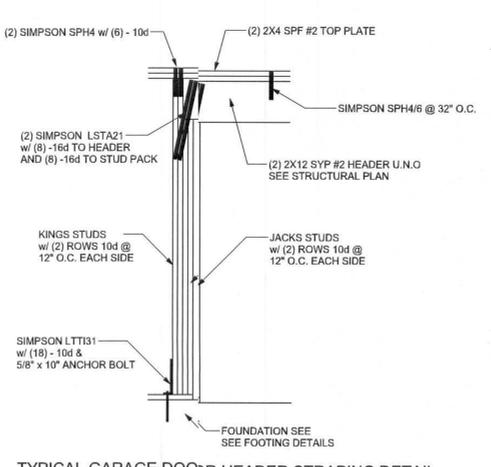
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OF 4 SHEETS

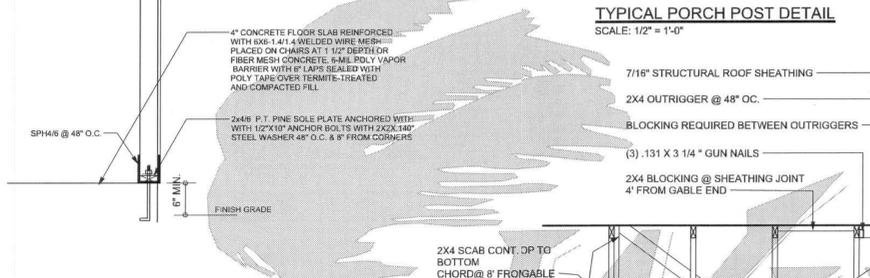
NO.	REVISIONS



**TYPICAL PORCH POST DETAIL**  
SCALE: 1/2" = 1'-0"



**TYPICAL GARAGE DOOR HEADER STRAPING DETAIL**  
SCALE: 1/2" = 1'-0"



**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

STUD SPACING	STUD HEIGHT
(1) 2x4 @ 16" OC	TO 11'-8" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-4" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-0" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13 1/2" O.C.

**GRADE & SPECIES TABLE**

Species	Fb (psi)	E (10 <sup>6</sup> psi)
2x8 SYP #2	1200	1.6
2x10 SYP #2	1050	1.6
2x12 SYP #2	975	1.6
GLB 24F-V3 SP	2400	1.8
LSL TIMBERSTRAND	1700	1.7
LVL MICROGLAM	1600	1.9
PSL PARALAM	2900	2.0

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND REACTION LOADS. FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTOR AT 16" EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH & WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT, DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENFORCE THE SLAB TO BE CUT AT A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2,400, E = 1,800,000; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ALL ROOF AREAS SHALL NOT EXCEED 15' AND TYPICAL SPACING OF CUTS TO BE 12FT, DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENFORCE THE SLAB TO BE CUT AT A GIVEN LINE.)

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED IF ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: 3/8" ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 12" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 3/8" x 8/16", WITH 5/8" BOLTS TO BE 3" x 3/8" x 9/16", WITH 3/4" BOLTS TO BE 3" x 3/8" x 9/16", WITH 7/8" BOLTS TO BE 3" x 3/8" x 9/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 620	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	13-8d	12-8d, 1 1/2"	
< 790	< 655	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1600	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2950	< 1785	LGT2	14-16d	14-16d	

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 3965	< 3330	MGT		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 800	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SP4H			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SP6H			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 1350	< 1305	LTT19		8-16d	12" AB
< 2310	< 2310	LTT31	16-10d, 1 1/2"		12" AB
< 3775	< 2670	HD2A	2-5/8" BCLTS		5/8" AB
< 4175	< 3695	HT15d	18-16d		5/8" AB
< 1400	< 1400	PAHD2	16-16d		
< 8335	< 3335	HPAH22	16-16d		
< 2200	< 2200	ABU44	12-16d		12" AB
< 2300	< 2300	ABUR6	12-16d		12" AB
< 2320	< 2320	ABUR8	18-16d		2-5/8" AB

**BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

- CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
- PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
- PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
- VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**MASONRY NOTES:**

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F <sub>m</sub> = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3-#11	ASTM 615, Grade 60, F <sub>y</sub> = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wet ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval. Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.
3.3.E.7 Movement joints	

**DESIGN DATA**

**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**  
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE  
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT < 30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 Other	-	-40.6	-40.6
3	19.9 - 25.5	18.1	-21.8
3 Other	-	-68.3	-42.4
4	21.8 - 23.8	18.5	-23.4
5	21.8 - 29.1	18.5	-22.6

Doors & Windows	Wind Case (Zone 5, 10 ft <sup>2</sup> )	21.8	-29.1
8x7 Garage Door	19.5	-22.9	
16x7 Garage Door	18.5	-21.0	

**DESIGN LOADS**

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)	
30 PSF (ATTICS WITH STORAGE)	
10 PSF (ATTICS WITHOUT STORAGE, < 3:12)	

ROOF	20 PSF (FLAT OR < 4:12)
16 PSF (4:12 TO < 12:12)	
12 PSF (12:12 AND GREATER)	
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32055, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915  
21 MAY 07  
SEAL

**Compass Builders**

**JAKE & ANNA ROCHE RESIDENCE**

ADDRESS: 01-SS-16-0390-015 COLUMBIA COUNTY FLORIDA

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PRINTED DATE: May 21, 2007

DRAWN BY: Evan Beamsley  
CHECKED BY:

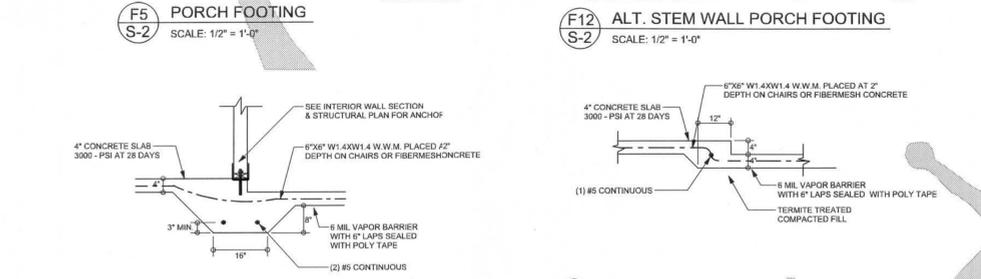
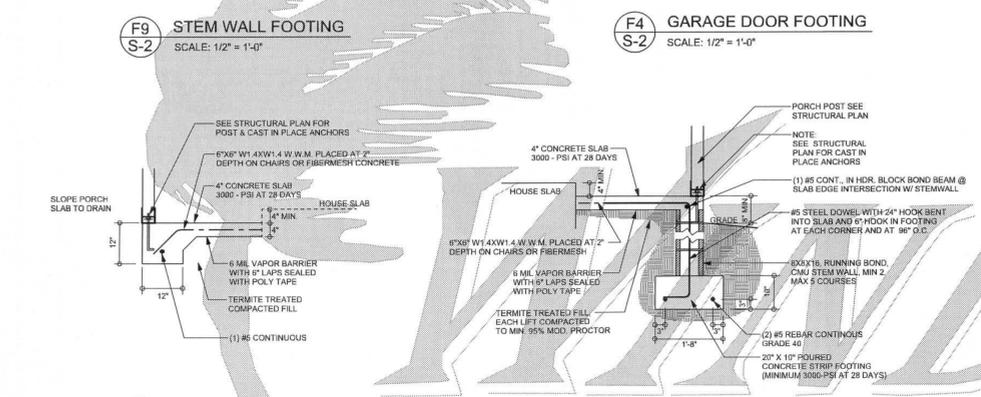
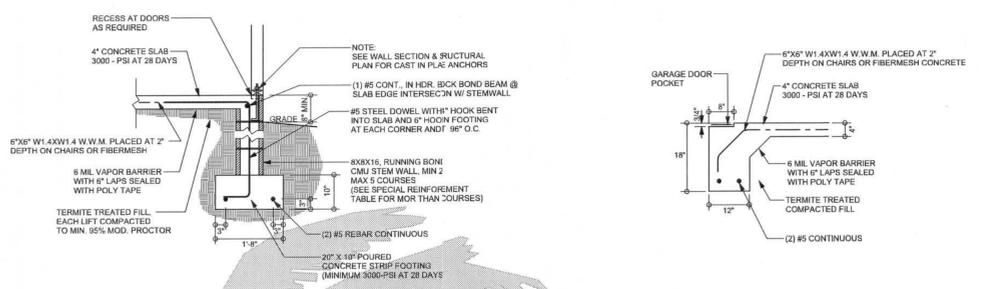
FINALS DATE: 21 / May / 07

JOB NUMBER: 705105

DRAWING NUMBER: S-1

OF 3 SHEETS

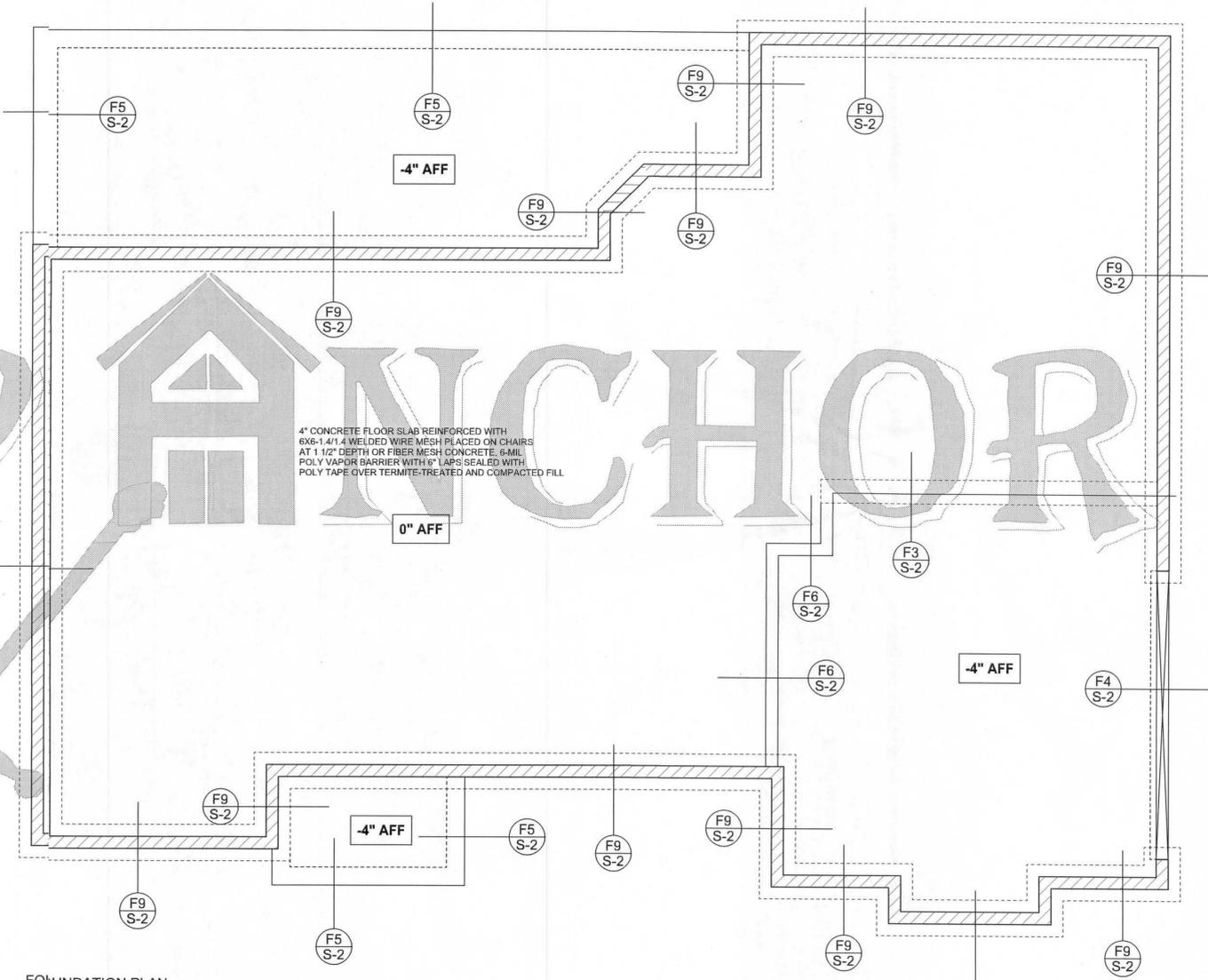
REVISIONS	



**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing a bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the interior side of the CMU wall (away from the soil pressure, within 2" of the exterior side of wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or horizontal bond beams with 16S continuous at mid height. For higher parts of the wall 12CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Discoway  
PE No. 53915, POB 866, Lake City, FL 32055, 385-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY  
P.E. 53915  
*Mark Discoway*  
21 MAY 07  
SEAL

**Compass Builders**

**JAKE & ANNA ROCHE RESIDENCE**

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Mark Discoway P.E.  
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PRINTED DATE:  
May 21, 2007

DRAWN BY: Evan Beamsley  
CHECKED BY:

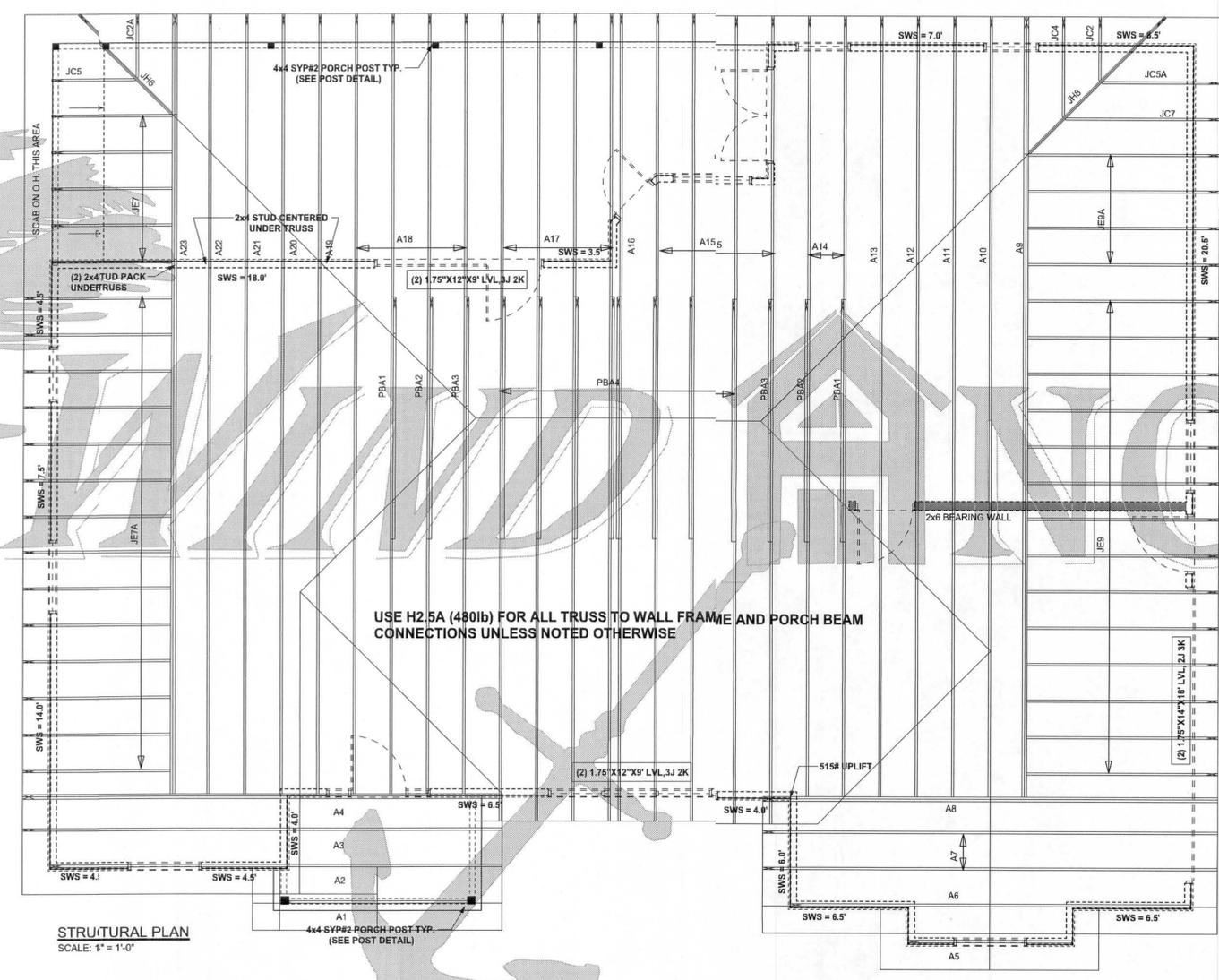
FINALS DATE:  
21 / May / 07

JOB NUMBER:  
705105

DRAWING NUMBER  
**S-2**

OF 3 SHEETS

REVISIONS

USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE

STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

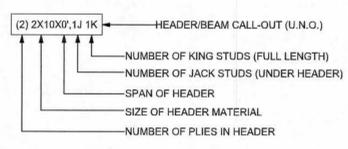
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH H&DERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.I.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTAIED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PERCS11-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WH THE SEALED TRUSS PACKAGE

WALL LEGEND

	1ST FLOOR EXTERIOR WALL
	2ND FLOOR EXTERIOR WALL
	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	51.1'	56.5'
LONGITUDINAL	25.8'	69.5'

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:** State dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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MARK DISOWAY  
P.E. 53915

*Mark Disoway*  
21 May 07  
SEAL

Compass Builders

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PRINTED DATE:  
May 21, 2007

DRAWN BY: Even Beasley      CHECKED BY:

FINALS DATE:  
21 / May / 07

JOB NUMBER:  
705105

DRAWING NUMBER  
S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER W. B. HOWLAND TRUSS JOB #4531