DATE 07/1	1/2008		ibia County I					PERIVITI
	*	This Permit Must	Be Prominently Poste				U	000027159
APPLICÁNT	JACOB K	IRSCH		P	HONE	344-4817		
ADDRESS	484	NW TURNER AVE	ENUE	LAKE CITY		· · · · · · · · · · · · · · · · · · ·	<u>FL</u>	32055
OWNER	SOUTHE	AST DEVELOPERS		P	HONE	755-2082		
ADDRESS	147	SW BUTTERCUP	DRIVE	LAKE CITY			FL	32024
CONTRACTO	OR JAC	COB KIRSCH		P	HONE	344-4817		
LOCATION C	F PROPER	TY 90W, TL	247S, TL ON CALLA	HAN, TR HOPE I	HENRY, T	R ON		
		MORNIN	IG GLORY, TR BUTT	ERCUP, 2ND LO	OT ON R.			
TYPE DEVEL	OPMENT	SFD,UTILITY	E	STIMATED COS	T OF CO	NSTRUCTIO)N 12:	5050.00
HEATED FLO	OOR AREA	1787.00	TOTAL AI	REA2501.00		HEIGHT		STORIES 1
FOUNDATIO	N CONC	C WAL	LS FRAMED	ROOF PITCH	8/12		FLOOR	SLAB
LAND USE &	ZONING	RSF-2			MAX.	HEIGHT	21	
Minimum Set	Back Requi	rments: STREET	-FRONT 25.0	0 R	EAR	15.00	SIDE	10.00
NO. EX.D.U.	0	FLOOD ZONE	X PP	DEVELOPME	NT PERM	IIT NO.		
PARCEL ID	15-4S-16-	03023-550	SUBDIVISI	ON ROLLING	MEADO	ows		
LOT <u>50</u>	BLOCK	PHASE	UNIT		TOTA	L ACRES	0.50	
000001632			CBC1253775		1			and the system dube.
Culvert Permit	No.	Culvert Waiver	Contractor's License N	umber	1	pplicant/Ow	vner/Contra	ctor
CULVERT		08-0056	BK		JH	1		Y
Driveway Con	nection	Septic Tank Number	LU & Zo:	ning checked by	Appi	roved for Issu	uance	New Resident
COMMENTS:	PLAT RE	QUIRES MFE OF 10	9 FT., ELEVATION C	ONFIRMATION	LETTER			
REQUIRED A	T SLAB,							
						Check # o	r Cash	12110
		EOP PI	UILDING & ZON	INC DEDART	MENT	ONLV		
T D	110201	FOR B		ING DEFART	INICIAL			(footer/Slab)
Temporary Pov	ver	date/app. by	Foundation	date/app. by		Monolithi		date/app. by
Under slab rou	igh in nlumb	(2020) (2	Slah			Shooth	ing/Nailing	#0F V0
Under stab rou	gn-in piumo	date/a	Slab	date/app.	hv	Sneath	ing/Naiiing	date/app. by
Framing		datera	Rough-in plumbing	WO-97000000 4.14 1/50		floor		date/app. 09
	date/ap	p. by	Rough in planning	above slab and be	ion nood			date/app. by
Electrical roug	gh-in		_ Heat & Air Duct		I	Peri. beam (L	intel)	
		date/app. by		date/app. by		cii. ocaiii (i		date/app. by
Permanent pow			C.O. Final			Culvert		
		ate/app. by		date/app. by			da	te/app. by
with the downs,	blocking, e	lectricity and plumbin		pp. by		Pool		e/app. by
Reconnection			Pump pole		Itility Pole			ларр. Оу
M/H Pole		date/app. by	da avel Trailer	te/app. by		date/app	p. by	
	ite/app. by	_	avel Italiei	date/app. by		Re-roof	date/s	app. by
BUILDING PE	RMIT FEE	\$ 630.00	CERTIFICATION F	EE\$ 12.51		SURCHAI	RGE FEE \$	12.51
MISC. FEES \$		The second secon	- G CERT. FEE \$ 50.0		\$ 0.00		ASTE FEE	7 - 49
			7 .					
FLOOD DEVE	LOPMENT	FEE \$ FLO	OOD ZONE FEE \$ 25	CULVERT	FEE\$	25.00 T	OTAL F	EE 755.02
INSPECTORS	OFFICE	- Halo	151/2/10	CLERKS	FFICE	/	N	

DEDMITT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

	• •	
For Office Use Only Application # 0801-53 Date	Received 1-11-08By LH Perm	14 1/032/77.0
The state of the s	e/8.01.08 Plans Examiner Ok MI	100.99-7.115
Development Permit N/A Zoni	no Rote Landing	0-01 0
Comments Plat Requires MFE of 109 ft. Eleva	tion Confirmation Letter Par	negory La Dec
NOC DEH Deed or PA Site Plan S	tate Road Info Parent Parcel #	Devel
	Fax 386 - 75	9 - 5047
Name Authorized Person Signing Permit Jacob	irsch Phone 386	344-4817*
Address 10 1 200 701 ner ave \$101,	Lake CITY, F1 32	05-5-
Owners Name Sugheast 1 fue on PCS	Phone 386-	755-7082
The state of the s		024
Contractors Name Jacob Kirsch - Compa		344-4817
Address 484 NW turner ave, #101,		055
Fee Simple Owner Name & Address_Southeast	Developers Group, La	
Bonding Co. Name & Address_N/H	0.00%, 20	KE (MY FZ
Architect/Engineer Name & Address Nicholas Paul	Geisler 1758 NW B	coula 116 F
Mortgage Lenders Name & Address (Olumbia Ban	k 173 NW hillshore s	t Lake City
Circle the correct power company - Fl Power & Light		FL, 3608
Property ID Number 15-45-16-63023-5-50 Subdivision Name Rolling Mediana	Suwannee Valley Elec	Progressive Energy
Subdivision Name Rolling Meadows Driving Directions from Usaa/WWW - 1880/WWW	_ Estimated Cost of Construction	18,500.
Driving Directions from Usgo/441 - go usgo we	Lot 3 BlockU	nit Phase
Sw callahan ave, tL, follow to	P 11: 10 CR-247, TL, TO	llow to
follow sw morking Glory dr +0 sw Butte	Rolling /Teadows Sub	TR,
Type of Construction S.F.D. Total Acreage Lot Size 5 Do you need a Cult Actual Distance of Structure from Property Lines Front	Number of Existing Dwellings on Pro	perty
Actual Distance of Structure from Property Lines - Front	vert Permit or Culvert Waiver or H	ave an Existing Drive
Total Building Height 7	Side Side Side	Rear 42
	Heated Floor Area 1787 Ro	of Pitch 8/123 16/13
Application is hereby made to obtain a possible d		
all laws regulating construction in this jurisdiction.	nd that all work be performed to me	et the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing info compliance with all applicable laws and regulating construction	rmation is accurate and all work will	
San and action	and Zoning.	
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE	OF COMMENCMENT MAY RESULT	IN YOU PAYING
TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU IN LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE	OF COMMENCEMENT	SULT WITH YOUR
Owner Builder or Authorized Person by Notarized Letter	Contractor Signature	
NOTARY PUBLIC-STATE OF FLORIDA	Contractors License Number	BC 1753775
COUNTY OF COLUMBIA	Competency Card NumberNOTARY STAMP/SEAL	
Sworn to (or affirmed) and subscribed being mesep 24, 2010	O TAMIFIGEAL	
his day of BONDED THRU ATLANTIC BONDING CO., INC.	M. Jechelle Fireho	0)
Personally known or Produced Identification	Notary Signature	
	Jugitature	(Revised Sept. 2006)

JU LEET MESENDE 1-18.00

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

<u>CONTRACTORS AFFIDAVIT:</u> By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's License Number CBC 1253775
Contractor's Signature (Permitee)

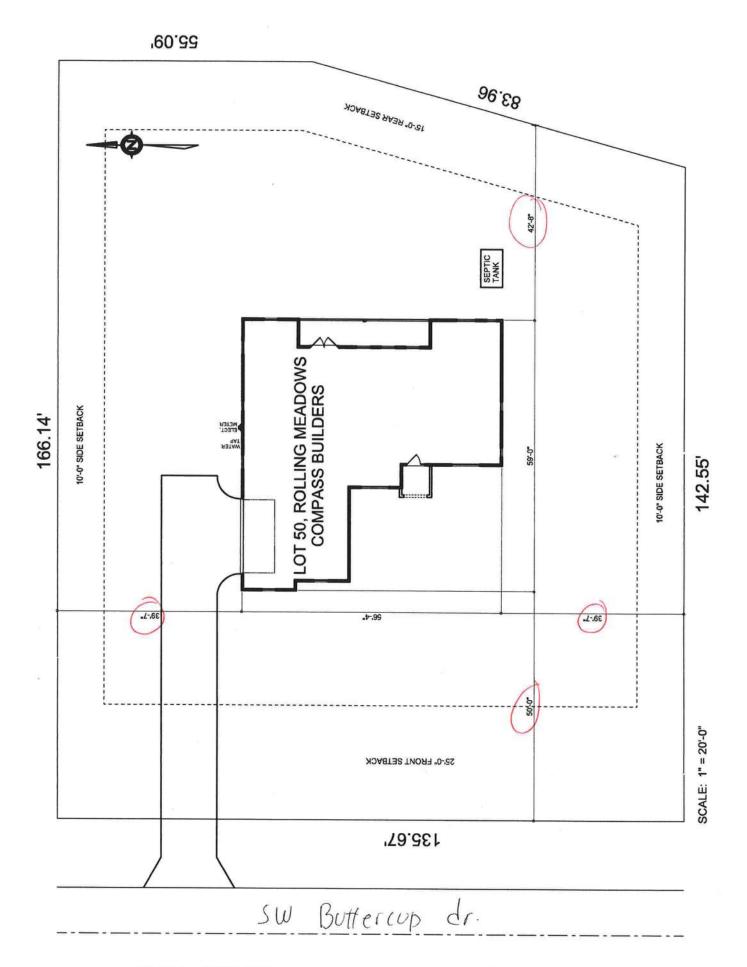
Contractor's License Number CBC 1253775
Columbia County
Competency Card Number

Affirmed under penalty of perjury to by the <u>Contractor</u> and subscribed before me this <u>31</u> day of <u>Jan</u> 20<u>08</u>.

Personally known___ or Produced Identification_____

State of Florida Notary Signature (For the Contractor)

Notary Public State of Florida Matthew Rocco My Commission DD578349 Expires 09/17/2010



15-4S-16-03023-550 LOT 50 ROLLING MEADOWS S/D. WD 1063-1963.WD 1067-2448 CWD 1104-327

Corporate Warranty Deed

This Indenture, made this December 4, 2006 A.D.

Between

Burbach Investment Group, LLC, a Florida Limited Liability Company, whose post office address is: 507 W. Duval Street, Lake City, FL 32055; Grantor and Southeast Developers Group, Inc., a Florida Corporation whose post office address is: 197 SW Waterford Court, Lake City, Florida 32025

Witnesseth, that the said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00), to it in hand paid by the said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee forever, the following described land, situate, lying and being in the County of Columbia, State of Florida, to wit:

Lots 7, 48 50, 52, and 54, ROLLING MEADOWS, according to the Plat thereof, recorded in Plat Book 8, Pages 45 and 46, of the Public Records of Columbia County, Florida.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number: R03023-507, R03023-548, R03023-550, R03023-552, and R03023-554

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2006.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

> Burbach Investment Group, LLC a Florida Limited Liability Company

Signed and Sealed in Our Presence:

MELINDA WEAVER

State of County of

Florida Columbia

The foregoing instrument was acknowledged before me this 4th day of December, 2006, by Thomas P Cady, Manager of Burbach Investment Group, LLC, a Florida Limited Liability Company, on behalf of the company. as identification.

He is personally known to me or has produced

Notary Public Notary Printed Name:

My Commission Expires::

#06-0389 Prepared by & Return to: Matt Rocco Sierra Title, LLC, 619 SW Baya Drive, Suite 102 Lake City, Florida 32025

Inst:2006028778 Date:12/06/2006 Time:12:42

Doc Stamp-Deed : 1400.00 _DC,P.DeWitt Cason,Columbia County B:1104 P:327

Notary Public State of Florida Malihew Rocco

My Commission DD578349

Expires 09/17/2010

COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787
Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

ADDRESS ASSIGNMENT DATA

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

Residential or Other Structure on Parcel Number: 15-4S-16-03023-550

Address Assignment: 147 SW BUTTERCUP DR, LAKE CITY, FL, 32024

Note: LOT 50 ROLLING MEADOWS S/D

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: Address: City, State: Owner: Climate Zone:	Compass Builders - Rae Lot: (Sub: Rolling Meadows, Plat: Lake City, FL 32025- Lof SO Spec House North	Builder: Compass Builders Permitting Office: Columbia Permit Number: 2010 Jurisdiction Number: 72000				
a. U-factor:	multi-family Single family if multi-family 1	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)				
I hereby certify that this calculation are Code. PREPARED BY DATE:	this building, as designed, is in compliance ergy Code.	DACC				

DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

BASE		AS-BU	IILT	
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area		verhang		
St. Martine - Americanic	Type/SC Orn	it Len Hgt	Area X SPM X S	OF = Points
.18 1787.0 18.59 5980.0	1.Double, Clear	W 1.5 10.		0.98 565.0
	2.Double, Clear 3.Double, Clear	W 7.5 10.	[종]	0.60 928.0
	4.Double, Clear	W 7.5 10. W 1.5 10.		0.60 1253.0 0.98 603.0
	5.Double, Clear	W 1.5 10.		0.98 226.0
	6.Double, Clear	N 1.5 8.	(F)	0.97 371.0
	7.Double, Clear	E 1.5 8.		.96 1208.0
	8.Double, Clear	E 8.5 10.	0 6.7 42.06 0	.56 155.0
9	9.Double, Clear	E 1.5 10.	0 30.0 42.06 0	.98 1234.0
	10.Double, Clear	S 1.5 8.		.92 993.0
	11.Double, Clear	S 1.5 8.	0 6.0 35.87 0	.92 198.0
	As-Built Total:		253.7	7734.0
WALL TYPES Area X BSPM = Points	Туре	R-Valu	e Area X SPM	= Points
Adjacent 164.0 0.70 114.8	1. Frame, Wood, Exterior	13.0	1230.3 1.50	1845.5
Exterior 1230.3 1.70 2091.5	2. Frame, Wood, Adjacent	13.0	164.0 0.60	98.4
Base Total: 1394.3 2206.3	As-Built Total:		1394.3	1943.9
DOOR TYPES Area X BSPM = Points	Туре		Area X SPM	= Points
Adjacent 20.0 2.40 48.0	1.Exterior Insulated		20.0 4.10	82.0
Exterior 20.0 6.10 122.0	2.Adjacent Insulated		20.0 1.60	32.0
Base Total: 40.0 170.0	As-Built Total:		40.0	. 114.0
CEILING TYPES Area X BSPM = Points	Туре	R-Value	Area X SPM X SCM	1 = Points
Under Attic 1787.0 1.73 3091.5	1. Under Attic	30.0	1950.0 1.73 X 1.00	3373.5
Base Total: 1787.0 3091.5	As-Built Total:		1950.0	3373.5
FLOOR TYPES Area X BSPM = Points	Туре	R-Valu	e Area X SPM	= Points
Slab 198.0(p) -37.0 -7326.0 Raised 0.0 0.00 0.0	Slab-On-Grade Edge Insulation	5.0	198.0(p -36.20	-7167.6
Base Total: -7326.0	As-Built Total:		198.0	-7167.6
INFILTRATION Area X BSPM = Points			Area X SPM	
1787.0 10.21 18245.3			1787.0 10.21	18245.3

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

	BASE		AS-BUILT							
Summer Ba	se Points:	22367.1	Summer As-Built Points:	24243.0						
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	9						
22367.1	0.3250	7269.3	(sys 1: Central Unit 37000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0 24243 1.00 (1.09 x 1.147 x 0.91) 0.260 0.950 24243.0 1.00 1.138 0.260 0.950	6812.6						

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

BASE	AS-BUILT	
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X WPM X WOF =	= Points
.18 1787.0 20.17 6488	1.Double, Clear W 1.5 10.0 15.0 20.73 1.01	312.0
	2.Double, Clear W 7.5 10.0 40.0 20.73 1.13	940.0
	3.Double, Clear W 7.5 10.0 54.0 20.73 1.13	1270.0
	4.Double, Clear W 1.5 10.0 16.0 20.73 1.01	333.0
	5.Double, Clear W 1.5 10.0 6.0 20.73 1.01	125.0
	6.Double, Clear N 1.5 8.0 20.0 24.58 1.00	491.0
	7.Double, Clear E 1.5 8.0 30.0 18.79 1.02 8.Double, Clear E 8.5 10.0 6.7 18.79 1.24	574.0 156.0
	9.Double, Clear E 1.5 10.0 30.0 18.79 1.01	570.0
	10.Double, Clear S 1.5 8.0 30.0 13.30 1.04	415.0
	11.Double, Clear S 1.5 8.0 6.0 13.30 1.04	83.0
	As-Built Total: 253.7	5269.0
WALL TYPES Area X BWPM = Po	rs Type R-Value Area X WPM = I	Points
Adjacent 164.0 3.60 5	4 1. Frame, Wood, Exterior 13.0 1230.3 3.40	4183.0
Exterior 1230.3 3.70 45	1 2. Frame, Wood, Adjacent 13.0 164.0 3.30	541.2
Base Total: 1394.3 51	5 As-Built Total: 1394.3	4724.2
DOOR TYPES Area X BWPM = Po	rs Type Area X WPM = F	Points
Adjacent 20.0 11.50 2	0 1.Exterior Insulated 20.0 8.40	168.0
Exterior 20.0 12.30 2	0 2.Adjacent Insulated 20.0 8.00	160.0
Base Total: 40.0 4	0 As-Built Total: 40.0	328.0
CEILING TYPES Area X BWPM = Po	s Type R-Value Area X WPM X WCM = F	Points
Under Attic 1787.0 2.05 36	3 1. Under Attic 30.0 1950.0 2.05 X 1.00	3997.5
Base Total: 1787.0 36	3 As-Built Total: 1950.0	3997.5
FLOOR TYPES Area X BWPM = Po	s Type R-Value Area X WPM = F	Points
Slab 198.0(p) 8.9 170 Raised 0.0 0.00	2 1. Slab-On-Grade Edge Insulation 5.0 198.0(p 7.60	1504.8
Base Total: 170	2 As-Built Total: 198.0	1504.8
INFILTRATION Area X BWPM = Poi		Points
1787.0 -0.59 -10	3 1787.0 -0.59 -	1054.3

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT							
Winter Base	Points:	16477.7	Winter As-Built Points:	14769.2						
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplie (System - Points) (DM x DSM x AHU)							
16477.7	0.5540	9128.7	(sys 1: Electric Heat Pump 37000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Ir 14769.2 1.000 (1.069 x 1.169 x 0.93) 0.443 0.950 14769.2 1.00 1.162 0.443 0.950	7221.4						

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

BASE				· ·	AS-BUILT								
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier		dit :	= Total
3		2635.00		7905.0	50.0 As-Built To	0.90 otal:	3		1.00	2693.56	1.	00	8080.7 8080.7

	CODE COMPLIANCE STATUS												
	BASE					AS-BUILT							
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
7269		9129		7905		24303	6813		7221		8081		22115

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.2

The higher the score, the more efficient the home.

Spec House, Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

1.	New construction or existing	New	_ 12.	Cooling systems		
2.	Single family or multi-family	Single family	_ a	. Central Unit	Cap: 37.0 kBtu/hr	_
3.	Number of units, if multi-family	1	_		SEER: 13.00	_
4.	Number of Bedrooms	3	_ b	. N/A		
5.	Is this a worst case?	No				_
6.	Conditioned floor area (ft2)	1787 ft²	_ c	. N/A		
7.	Glass type 1 and area: (Label reqd. b	by 13-104.4.5 if not default)				_
	a. U-factor:	Description Area	13.	Heating systems		
	(or Single or Double DEFAULT)		a	. Electric Heat Pump	Cap: 37.0 kBtu/hr	· ·
1	b. SHGC:			3.5	HSPF: 7.70	
	(or Clear or Tint DEFAULT)	7b. (Clear) 253.7 ft ²	_ b	. N/A		_
8.	Floor types					
	a. Slab-On-Grade Edge Insulation	R=5.0, 198.0(p) ft	c	. N/A		-
1	b. N/A					-
	c. N/A		14.	Hot water systems	w.	
9.	Wall types		a	. Electric Resistance	Cap: 50.0 gallons	Decision .
	a. Frame, Wood, Exterior	R=13.0, 1230.3 ft ²			EF: 0.90	_
1	b. Frame, Wood, Adjacent	R=13.0, 164.0 ft ²	_ b	. N/A		
	c. N/A					,,,,,,,,,
	d. N/A		— с	. Conservation credits		
	e. N/A		100 100	(HR-Heat recovery, Solar		
10.	. Ceiling types		_	DHP-Dedicated heat pump)		
	a. Under Attic	R=30.0, 1950.0 ft ²	15.	HVAC credits	PT,	
1	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,		
	c. N/A		_	HF-Whole house fan,		
11.	Ducts		_	PT-Programmable Thermostat,		
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 50.0 ft		MZ-C-Multizone cooling,		
	b. N/A		_	MZ-H-Multizone heating)		
In	ertify that this home has complie	nd with the Florida Energy	, Efficience	Code For Building		
	onstruction through the above end				OF THE STATE	
					ST 04	B.
ho	this home before final inspection	i. Otherwise, a new EPL I	Display Ca	ra will be completed	2 23	16
	sed on installed Code compliant					2
Bu	ilder Signature:		Date:		I O	×
					1.	
Ac	dress of New Home:		City/FL Z	ip:	GOD WE TRUST	1

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001632

DATE 0//1	PARCEL ID #	15-45-16-03023-550	
APPLICANT	JACOB KIRSCH	PHONE	344-4817
ADDRESS _	484 NW TURNER AVENUE	LAKE CITY	FL 32055
OWNER SO	OUTHEAST DEVELOPERS	PHONE <u>7</u>	755-2082
ADDRESS 14	47 SW BUTTERCUP DRIVE	LAKE CITY	FL 32024
CONTRACTO	R JACOB KIRSCH	PHONE	344-4817
LOCATION OF	F PROPERTY 90W, TL ON 247S, TL ON	CALLAHAN, TR ON HOPE I	HENRY, TR
MORNING GLOR	Y, TR ON BUTTERCUP CR, 2ND LOT ON RIC	GHT	
SUBDIVISION	/LOT/BLOCK/PHASE/UNIT ROLLING	G MEADOWS	50
	10		
SIGNATURE			
	INSTALLATION REQUIREMENT	'S	
X	Culvert size will be 18 inches in diamete driving surface. Both ends will be mitered thick reinforced concrete slab.	— er with a total lenght of 32	feet, leaving 24 feet of and poured with a 4 inch
	a) a majority of the current and existing b) the driveway to be served will be particular to the current and existing b. Turnouts shall be concrete or paved concrete or paved driveway, whiches current and existing paved or concrete.	ng driveway turnouts are paved or formed with conc d a minimum of 12 feet we ever is greater. The width	rete. ide or the width of the
	Culvert installation shall conform to th	e approved site plan stand	dards.
	Department of Transportation Permit in	nstallation approved stand	dards.
	Other		

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

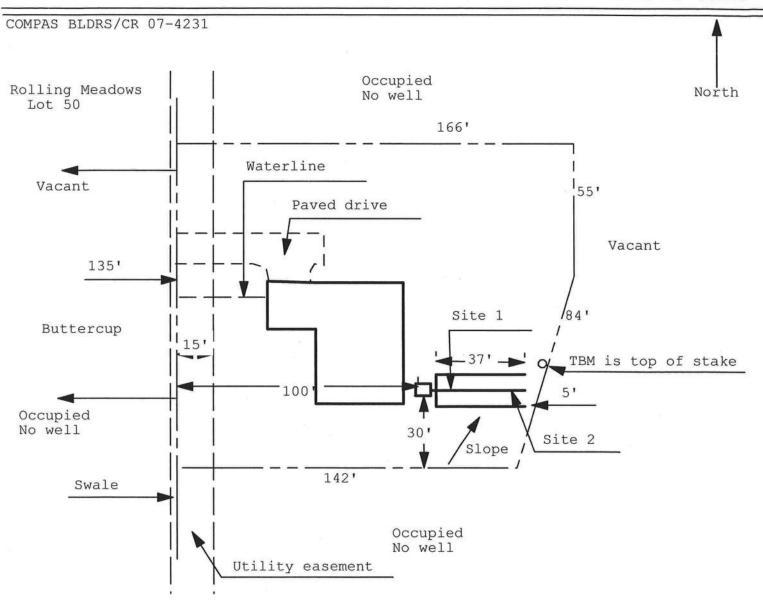
Amount Paid 25.00



08-0056

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 080 53

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



	1 inch = 40 feet
Site Plan Submitted By	Date 1/10/08
By Ma s Zanda	Columbia CPHU
Notes:	

Residential System Sizing Calculation

Summary

Spec House

Lake City, FL 32025-

Project Title:
Compass Builders - Rae

Code Only Professional Version Climate: North

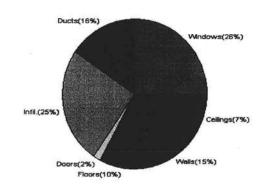
1/10/2008

				1/10/200	0
Location for weather data: Gaines	sville - Def	aults: Latitu	ude(29) Altitude(152 ft.) Temp Ran	ige(M)	
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)	5 , ,	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	31426	Btuh	Total cooling load calculation	46421	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	
Total (Electric Heat Pump)	117.7	37000	Sensible (SHR = 0.75)		27750
Heat Pump + Auxiliary(0.0kW)	117.7	37000	Latent	108.4	
The state of the s			Total (Electric Heat Pump)		37000

WINTER CALCULATIONS

Winter Heating Load (for 1787 soft)

Load component			Load	
Window total	254	sqft	8166	Btuh
Wall total	1394	sqft	4579	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1950	sqft	2298	Btuh
Floor total	198	sqft	3238	Btuh
Infiltration	191	cfm	7721	Btuh
Duct loss			4907	Btuh
Subtotal		- 4	31426	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			31426	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1787 sqft)

Load component			Load	
Window total	254	sqft	18233	Btuh
Wall total	1394	sqft	2814	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1950	sqft	3229	Btuh
Floor total			0	Btuh
Infiltration	167	cfm	3104	Btuh
Internal gain		- 1	3780	Btuh
Duct gain		- 1	6332	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			37884	Btuh
Latent gain(ducts)		- 1	1242	Btuh
Latent gain(infiltration)			6095	Btuh
Latent gain(ventilation)		1	0	Btuh
Latent gain(internal/occu	pants/othe	r)	1200	Btuh
Total latent gain	2		8537	Btuh
TOTAL HEAT GAIN			46421	Btuh

Letent Internal(3%)
Int.Oain(8%)

Ducts(16%)

Windows(39%)

Cailings(7%)

Walls(6%)

Version 8
For Florida residences only

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Rae

Professional Version Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	15.0	32.2	483 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
	2, Clear, Metal, 0.87	W	54.0	32.2	1738 Btuh
4	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btuh
5	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btuh
6	2, Clear, Metal, 0.87	N	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	E	6.7	32.2	215 Btuh
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
11	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		254(sqft)		8166 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1230	3.3	4040 Btuh
2	Frame - Wood - Adj(0.09)	13.0	164	3.3	539 Btuh
	Wall Total		1394		4579 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1950	1.2	2298 Btuh
	Ceiling Total		1950		2298Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	198.0 ft(p)	16.4	3238 Btuh
	Floor Total		198		3238 Btuh
			Envelope Su	btotal:	18798 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqf) CFM=	
	Natural	0.80	14296 1394	190.6	7721 Btuh
Ductload			(D	LM of 0.185)	4907 Btuh
All Zones		Sens	sible Subtotal Al	I Zones	31426 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued) Project Title:

Spec House

Compass Builders - Rae

Code Only Professional Version Climate: North

Lake City, FL 32025-

1/10/2008

Subtotal Sensible Ventilation Sensible Total Btuh Loss	31426 Btuh 0 Btuh 31426 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House

Project Title: Compass Builders - Rae

Code Only Professional Version

Lake City, FL 32025-

Compass Builders - Rai

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	15.0	32.2	483 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	W	54.0	32.2	1738 Btuh
4	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btuh
5	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btuh
6	2, Clear, Metal, 0.87	N	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	E	6.7	32.2	215 Btuh
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
11	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		254(sqft)		8166 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1230	3.3	4040 Btuh
2	Frame - Wood - Adj(0.09)	13.0	164	3.3	539 Btuh
	Wall Total		1394		4579 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1950	1.2	2298 Btuh
	Ceiling Total		1950		2298Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	198.0 ft(p)	16.4	3238 Btuh
	Floor Total		198	16.50.65	3238 Btuh
		z	one Envelope Su	ıbtotal:	18798 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqf	t) CFM=	
	Natural	0.80	14296 1394	190.6	7721 Btuh
Ductload	Average sealed, Supply(R6.	0-Attic), Retur	n(R6.0-Attic) (D	LM of 0.185)	4907 Btuh
Zone #1		Sens	sible Zone Subto	otal	31426 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Rae

Code Only Professional Version Climate: North

1/10/2008

	Ventilation Sensible Total Btuh Loss	0 Btuł 31426 Btuł
--	--------------------------------------	----------------------

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Rae Code Only Professional Version

Lake City, FL 32025-

Compass Builders - Ra

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

Component Loads for Whole House

	Type*		Over	hang	Win	dow Area	(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross				Unshaded		
1	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	15.0	0.0	15.0	29	80	1193	Btuh
2	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	40.0	17.3	22.7	29	80	20,000	Btuh
3	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	54.0	20.0	34.0	29	80		Btuh
4	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	16.0	0.0	16.0	29	80		Btuh
5	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	6.0	0.0	6.0	29	80	477	1000000000
6	2, Clear, 0.87, None, N, N	N	1.5ft	8ft.	20.0	0.0	20.0	29	29	579	
7	2, Clear, 0.87, None, N, N	Е	1.5ft	8ft.	30.0	0.0	30.0	29	80		Btuh
8	2, Clear, 0.87, None, N, N	E	8.5ft	10ft.	6.7	3.7	2.9	29	80	1100	Btuh
9	2, Clear, 0.87, None, N, N	E	1.5ft	10ft.	30.0	0.0	30.0	29	80		Btuh
10	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	30.0	30.0	0.0	29	34	1.000	Btuh
11	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	6.0	6.0	0.0	29	34		Btuh
	Excursion							,		2971	Btuh
	Window Total				254 (saft)				18233	
Walls	Type		R-Va	alue/U	-Value	Area(saft)		НТМ	Load	Dian
1	Frame - Wood - Ext			13.0/		123			2.1		Btuh
2	Frame - Wood - Adj			13.0/		164	70.7		1.5	0.000	Btuh
	Wall Total			10.07	0.00				1.5		
Doors	Type						4 (sqft)		LITAA	2814	Btun
						Area			HTM	Load	
1 2	Insulated - Exterior					20			9.8	196	Btuh
2	Insulated - Adjacent					20			9.8	196	Btuh
o	Door Total					4	0 (sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area(sqft)		HTM	Load	7 27
1	Vented Attic/DarkShingle			30.0		195	0.0		1.7	3229	Btuh
	Ceiling Total					195	(sqft)			3229	
Floors	Type		R-Va	lue		Siz			нтм	Load	Dian
1	Slab On Grade		00000	5.0		0.700	70.70 L		(4.2.2.2.2.2		D4 - L
	Floor Total			5.0			8 (ft(p))		0.0	0	Btuh
	1 loor Total					198.0	(sqft)			0	Btuh
						En	velope \$	Subtotal	:	24668	Btuh
nfiltration	Туре		۸	СН	Volum	o(ouff)	oll ores	(naft)	CEM-	1 4 4 4	
	SensibleNatural		A	0.70	voluin		all area	(sqit)	CFM=	Load	
Internal		,	20011	Ularian harm		14296	1394		190.6	3104	Btuh
gain			Occup			Btuh/oc		P	Appliance	Load	
ganı				6		X 230) +		2400	3780	Btuh
						Se	nsible E	nvelope	Load:	31552	Btuh
Ouct load							(DGN	1 of 0.20	01)	6332	Btuh
						Sen	sible Lo	ad All 2	Zones	37884	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Rae Code Only **Professional Version** Climate: North

1/10/2008

WHOLE HOUSE TOTALS

		,	
	Sensible Envelope Load All Zones	31552	Btul
	Sensible Duct Load	6332	Btuh
	Total Sensible Zone Loads	37884	Btu
	Sensible ventilation	0	Btul
	Blower	0	Btul
Whole House	Total sensible gain	37884	Btu
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6095	Btul
	Latent ventilation gain	0	Btul
	Latent duct gain	1242	Btul
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btul
	Latent other gain	0	Btuh
	Latent total gain	8537	Btu
	TOTAL GAIN	46421	Btul

1. Central Unit #	37000 Btuh
-------------------	------------

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default) (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House

Project Title: Compass Builders - Rae

Code Only Professional Version

Lake City, FL 32025-

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

Component Loads for Zone #1: Main

	Type*		Over	hang	Win	dow Area	a(sqft)	ŀ	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	15.0	0.0	15.0	29	80	1193	Btuh
2	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	40.0	17.3	22.7	29	80	2304	
3	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	54.0	20.0	34.0	29	80	100000000000000000000000000000000000000	Btuh
4	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	16.0	0.0	16.0	29	80	1272	 1900 (1917) (19
5	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	6.0	0.0	6.0	29	80	477	
6	2, Clear, 0.87, None,N,N	N	1.5ft	8ft.	20.0	0.0	20.0	29	29	579	Btuh
7	2, Clear, 0.87, None, N, N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btuh
8	2, Clear, 0.87, None, N, N	E	8.5ft	10ft.	6.7	3.7	2.9	29	80	342	Btuh
9	2, Clear, 0.87, None,N,N	E	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385	Btuh
10	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	30.0	30.0	0.0	29	34	869	Btuh
11	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	6.0	6.0	0.0	29	34	174	Btuh
	Window Total				254 (sqft)				15262	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	123	0.3		2.1	2566	Btuh
2	Frame - Wood - Adj			13.0/	0.09	0.000	4.0		1.5		Btuh
	Wall Total				1394 (sqft)			2814 Btu			
Doors	Туре					Area			нтм	Load	Dian
1	Insulated - Exterior						S 50 50				Divis
2	Insulated - Adjacent			20.0 20.0			9.8 9.8	196	Btuh		
	Door Total						0 (sqft)		9.6	107.70	
Ceilings	Type/Color/Surface		R-Va	عبراه		Area	The same of the sa		нтм	Load	Btuh
1	Vented Attic/DarkShingle								and a solution		
				30.0 1950.0			1.7		Btuh		
Floors	Ceiling Total		D				0 (sqft)			3229	Btuh
Control of the Contro	Туре		R-Va	alue		Siz	ze		HTM	Load	
1	Slab On Grade			5.0		19	8 (ft(p))		0.0	0	Btuh
	Floor Total					198.	0 (sqft)			0	Btuh
						Zo	ne Enve	lope Su	ubtotal:	21697	Btuh
nfiltration	Туре		Α	СН	Volum	e(cuft) v	vall area	(saft)	CFM=	Load	
	SensibleNatural		401. (1)	0.70		14296	1394		166.8	3104	Btuh
Internal		(Occup	ants		Btuh/oc	cupant	1	Appliance	Load	
gain				6		X 23		8	2400	3780	Btuh
						Se	ensible E	nvelope		28581	
Ouct load	Average sealed, Supply	(R6.0-A	Attic),	Retur	n(R6.0-	Attic)		(DGM c	of 0.201)	5736	Btuh
							Sensib	le Zone	Load	34317	3tuh

The following window Excursion will be assigned to the system loads.

Windows	July excursion for System 1	2971 Btuh
	outy execution for eyelent 1	Z9/ I Dluii
	Excursion	Subtotal: 2971 Btuh
	·	

Manual J Summer Calculations

Residential Load - Component Details (continued)
Project Title:

Spec House

Compass Builders - Rae

Code Only

Professional Version

Lake City, FL 32025-

Climate: North

1/10/2008

	Sensible Excursion Load	3567 Btuh
Duct load		596 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Rae Code Only Professional Version Climate: North

1/10/2008

WHOLE HOUSE TOTALS

		T	
	Sensible Envelope Load All Zones	31552	Btul
	Sensible Duct Load	6332	Btuh
	Total Sensible Zone Loads	37884	Btul
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	37884	Btul
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6095	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1242	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
(40	Latent other gain	0	Btuh
	Latent total gain	8537	Btul
	TOTAL GAIN	46421	Btul

ALTON DESCRIPTION OF STREET BOOK OF STREET	28 (47 H. 1966) (27 H. 1967) (27 H. 1967) (37 H. 1967) (37 H. 1967) (37 H. 1967) (37 H. 1967)	
EQUIPMENT		

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

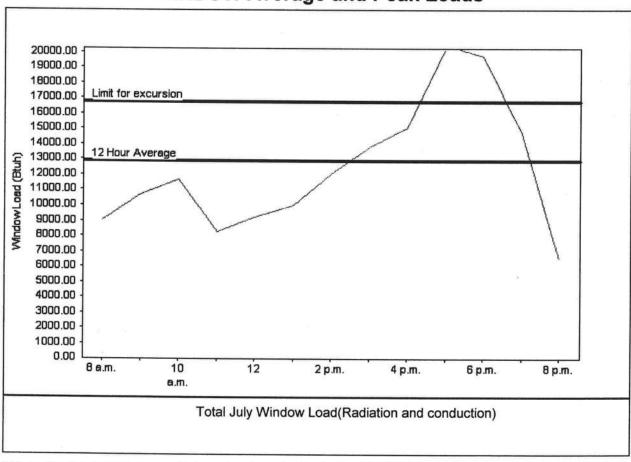
Spec House Lake City, FL 32025-

Project Title: Compass Builders - Rae Code Only Professional Version Climate: North

1/10/2008

Weather data for: Gainesville - Defaults									
Summer design temperature	92 F	:	Average window load for July	12894 Btu					
Summer setpoint	75 F	:	Peak window load for July	20440 Btu					
Summer temperature difference	17 F		Excusion limit(130% of Ave.)	16763 Btu					
Latitude	29 N	North	Window excursion (July)	3677 Btuh					

WINDOW Average and Peak Loads



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only PREPARED BY:

EnergyGauge® FLRCPB v4.5.2

DATE:

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Applicant	Plans Examine	ENTS: Two (2) complete sets of plans containing the following:
	0	All drawings must be clear, concise and drawn to scale ("Optional"
è		
OL.		Committee of the second of the
	0	Designers name and companies on document (EDC) to the
Tal.	0	or original, utilitial stat shall be attived
	u	Site Pian including:
		a) Dimensions of lot
		b) Dimensions of building set backs c) Location of all other building set
		 Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
		d) Provide a full legal description of property
Y		wind-load Engineering Summer and all at
		The modulation lines he shown as not costing 1000 to the
		VI THE HILL INCIDE IN AND INVIDENCE TO THE
		1604.5 or Table 6-1, ASCE 7 and building classification from Table 1-1, ASCE 7.
		c. Wind exposure, if more than one wind exposure is utilized, the
		approache cherostie classifications and if designed
		e. Components and Cladding The design wind
		cladding materials not specifally designed by the registered design professional.
		Elevations including:
,		a) All sides
,		b) Roof pitch
_		c) Overhang dimensions and detail with attic ventilation

0000		d) Location, size and height above roof of chimneys.
P		e) Location and size of skylights
100	0	f) Building height
D)		e) Number of stories
		Floor Plan including:
Ø.		a) Rooms labeled and dimensioned.
p	0	b) Shear walls identified.
	0	c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
Ø		d) Show safety glazing of glass, where required by code.
P		e) Identify egress windows in bedrooms, and size.
9	0	 f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
	0	g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
7		h) Must show and identify accessibility requirements (accessible bathroom)
4		Foundation Plan including:
	_	 a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
4	0	b) All posts and/or column footing including size and reinforcing
4	0	c) Any special support required by soil analysis such as piling
4		d) Location of any vertical steel.
Al"		Roof System;
4		a) Truss package including:
		 Truss layout and truss details signed and sealed by Fl. Pro. Eng. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
d	0	b) Conventional Framing Layout including:
-		Rafter size, species and spacing
		2. Attachment to wall and uplift
		Ridge beam sized and valley framing and support details
		 Roof assembly (FBC 106.1.1.2)Roofing systems, materials,
		manufacturer, fastening requirements and product evaluation with wind resistance rating)
		Wall Sections including:
		a) Masonry wall
	u	All materials making up wall
		2. Block size and mortar type with size and spacing of reinforcement
		3. Lintel, tie-beam sizes and reinforcement
		4. Gable ends with rake beams showing reinforcement or gable truss
		and wall bracing details
		All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation shall be
		designed by a Windload engineer using the engineered roof truss
		plans.
		Roof assembly shown here or on roof system detail (FBC)
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with resistance rating)
		Fire resistant construction (if required)
		8. Fireproofing requirements
		 Shoe type of termite treatment (termiticide or alternative method)
		10. Slab on grade
*		 Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
		b. Must show control joints, synthetic fiber reinforcement or
		Welded fire fabric reinforcement and supports
		 Indicate where pressure treated wood will be placed Provide insulation R value for the following:

D		b) Wood frame wall
		1. All materials making up wall
		Size and species of studs
		Sheathing size, type and nailing schedule
		4. Headers sized
		 Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with wind resistance rating)
		8. Fire resistant construction (if applicable)
		9. Fireproofing requirements
		 Show type of termite treatment (termiticide or alternative method) Slab on grade
		a. Vapor retarder (6Mil. Polyethylene with joints lapped 6
		inches and sealed
		 Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
		12. Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity
ch.		c. Crawl space (if applicable)
44		c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
		Engineer or Architect)
4/		Floor trues pockage in the time to the tim
0 0000		 a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
d	0	b) Floor joist size and spacing
þ		c) Girder size and spacing
b		d) Attachment of joist to girder
Þ		e) Wind load requirements where applicable
		Plumbing Fixture layout
		Electrical layout including:
D		a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
		b) Ceiling fans
Ψ		c) Smoke detectors
H		d) Service panel and sub-panel size and location(s)
#		e) Meter location with type of service entrance (overhead or underground)
#\	0	1) Appliances and HVAC equipment
# \		g) Arc Fault Circuits (AFCI) in bedrooms
41		h) Exhaust fans in bathroom
4		HVAC information
		a) Energy Calculations (dimensions shall match plans)
6		b) Manual J sizing equipment or equivalent computation
6)		c) Gas System Type (LP or Natural) Location and BTU demand of equipment
2		Disclosure Statement for Owner Builders
		*** Notice Of Commencement Required Before Any Inspections Will Be Done Private Potable Water
	u	TIVALE PULADIE WATER

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed coline @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Jeld-Wen	Exterior Swinning door	FL- 49X-R1
B. SLIDING		Price State	111111111111111111111111111111111111111
C. SECTIONAL/ROLL UP	ROYNOT	Rayhor	FL-4867
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI- Products	Sinale Hung Window	FL-5108
B. HORIZONTAL SLIDER		7 7	
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	James Hardie	Hardi Plant Siding	FL-889-RI
B. SOFFITS	Kaycan	Aluminum Soffit	FL- 4957
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS	 		
A. ASPHALT SHINGLES	Elk Roofing	Asphalt shingles	FL-586-RZ
B. NON-STRUCT METAL	7	Tisphan Shringtes	P2 366 K2
C. ROOFING TILES			
D. SINGLE PLY ROOF		The state of the s	
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS	SIMPSON Strong to	Truss STrups	FL- 474-RI
B. WOOD ANCHORS	1	1117/01	
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR	 		
ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE DATE



Project Information for:

L265558

Builder:

Compass Builders

Lot:

Subdivision:

Rolling Meadows

County:

Baker 45

Truss Count: Design Program: MiTek 20/20 6.3

Building Code: Truss Design Load Information:

FBC2004/TPI2002

Gravity:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 120

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for the Structural Engineering:

Jacob C. Kirsch Florida License No. CBC1253775

Address: 196 Southwest Huntsview Way Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

> Drwg. # J1925022

> J1925023

J1925024

J1925025

J1925026

J1925027

J1925028

J1925029

J1925030

J1925031

J1925032

J1925033

J1925034

J1925035

J1925036

J1925037

J1925038

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

Drwg. #

No.

28

J1925021

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T17

T18

T19

T20

T21

T22

T23

T24

T25

T26

T27

T28

T29

T30

T31

T32

T17G

Date

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

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1/10/08

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1/10/08

1	J1924994	CJ01	1/10/08	29
2	J1924995	CJ02	1/10/08	30
3	J1924996	CJ03	1/10/08	31
4	J1924997	CJ04	1/10/08	32
5	J1924998	CJ05	1/10/08	33
6	J1924999	CJ06	1/10/08	34
7	J1925000	CJ07	1/10/08	35
8	J1925001	EJ07	1/10/08	36
9	J1925002	EJ07A	1/10/08	37
10	J1925003	HJ07	1/10/08	38
11	J1925004	HJ07A	1/10/08	39
12	J1925005	T01	1/10/08	40
13	J1925006	T01G	1/10/08	41
14	J1925007	T02	1/10/08	42
15	J1925008	T03	1/10/08	43
16	J1925009	T04	1/10/08	44
17	J1925010	T05	1/10/08	45
18	J1925011	T06	1/10/08	
19	J1925012	T07	1/10/08	
20	J1925013	T08	1/10/08	
21	J1925014	T09	1/10/08	
22	J1925015	T10	1/10/08	
23	J1925016	T11	1/10/08	
24	J1925017	T12	1/10/08	
25	J1925018	T13	1/10/08	
26	J1925019	T14	1/10/08	
27	J1925020	T15	1/10/08	

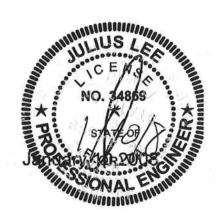
T16G

1/10/08

Truss ID

Date

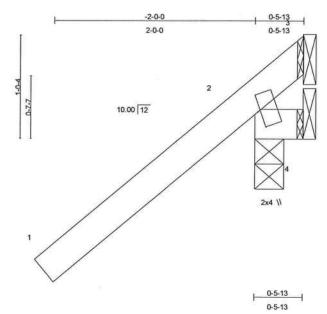
No.



Job	Truss	Truss Type	Qty	Ply	00	
L265558	CJ01	JACK	4	1		J1924994
					Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:41 2008 Page 1



Scale = 1:10.8

in	(loc)	I/defl	L/d	PLATES	GRIP
-0.00	2	>999	360	MT20	244/190
-0.00	2	>999	240	STATE STATE OF THE	

n/a

LUMBER

TCLL

TCDL

BCLL

BCDL

Plate Offsets (X,Y):

20.0

7.0

10.0

5.0

LOADING (psf)

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

0.00

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

TOP CHORD

Structural wood sheathing directly applied or

Weight: 6 lb

0-5-13 oc purlins.

n/a

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

-

REACTIONS

(lb/size) 2=404/0-3-8, 4=2/Mechanical, 3=-263/Mechanical

2-0-0

1.25

1.25

YES

CSI

TC

BC

WB

(Matrix)

0.45

0.00

0.00

Max Horz 2=114(load case 6)

[2:0-2-0,0-0-5]

Plates Increase

Rep Stress Incr

Lumber Increase

Code FBC2004/TPI2002

SPACING

Max Uplift 2=-503(load case 6), 3=-263(load case 1)

Max Grav 2=404(load case 1), 4=7(load case 2), 3=382(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/65, 2-3=-240/479

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.75

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 503 Colhi Holid at joint 3.

Truss Design Engineer Florida PE No. 34899 1109 Ceastel Bay Blvd Boynton Besch, FL 33435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
L265558	CJ01	JACK	4	1	J1924994
					Job Reference (optional)
Builders First S	Source, Jacksonville,	Florida 32244	6.300 s Feb 15 2006	MiTek Ir	ndustries, Inc. Thu Jan 10 14:19:41 2008 Page 2

LOAD CASE(S) Standard

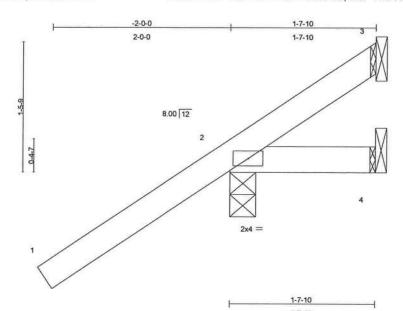
January 10,2008



Job	Truss	Truss Type	Qty	Ply	00	
L265558	CJ02	JACK	4	1		J1924995
			1		Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:42 2008 Page 1



1-7-10 LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP 20.0 TCLL Plates Increase 1.25 TC 0.37 Vert(LL) -0.00 2 >999 360 MT20 244/190 TCDL 7.0 1.25 Lumber Increase BC 0.02 Vert(TL) -0.002 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) 0.00 3 n/a n/a

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

5.0

BRACING

TOP CHORD

Structural wood sheathing directly applied or

Weight: 9 lb

1-7-10 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=240/0-3-8, 4=8/Mechanical, 3=-32/Mechanical

Max Horz 2=124(load case 6)

Code FBC2004/TPI2002

Max Uplift 2=-219(load case 6), 3=-32(load case 1)

Max Grav 2=240(load case 1), 4=24(load case 2), 3=67(load case 6)

(Matrix)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD**

1-2=0/61, 2-3=-69/54 2-4=0/0

JOINT STRESS INDEX

2 = 0.32

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 2 and 32 lb uplift at joint 3.

January 10,2008

Scale = 1:12:2

LOAD CASE(S) Standard

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

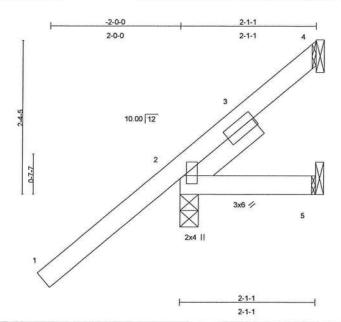
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265558	CJ03	JACK	4	1		J1924996
	20-145000	The second of th			Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:42 2008 Page 1



Scale = 1:16.7

Plate Of	fsets (X,Y	(): [2:0-1-8,0-0-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	4	n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

SLIDER

Left 2 X 4 SYP No.2 1-4-6

BRACING

TOP CHORD

Structural wood sheathing directly applied or

2-1-1 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-21/Mechanical, 2=253/0-3-8, 5=9/Mechanical

Max Horz 2=163(load case 6)

Max Uplift 4=-21(load case 1), 2=-202(load case 6)

Max Grav 4=46(load case 4), 2=253(load case 1), 5=28(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-97/41, 3-4=-48/28

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.72, 2 = 0.13 and 3 = 0.00

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb Complified is in a 202 lb uplift at joint 2.

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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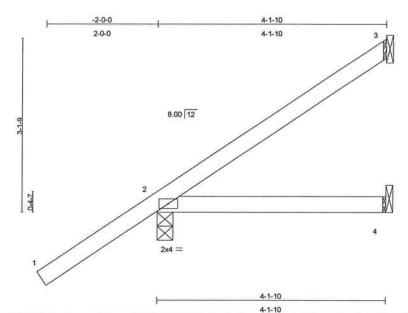
Job	Truss	Truss Type	Qty	Ply	00
L265558	CJ03	JACK	4	1	J1924996
					Job Reference (optional)
Builders First S	Source, Jacksonville,	Florida 32244 6.	300 s Feb 15 2006	MiTek Ir	ndustries, Inc. Thu Jan 10 14:19:42 2008 Page 2

LOAD CASE(S) Standard





Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:43 2008 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	20 40 4 2 Hz 2 May 2 4 2 Mg					Weight: 17 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

Scale = 1:19.6

4-1-10 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=72/Mechanical, 2=280/0-3-8, 4=20/Mechanical

Max Horz 2=186(load case 6)

Max Uplift 3=-51(load case 5), 2=-171(load case 6)

Max Grav 3=72(load case 1), 2=280(load case 1), 4=59(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-81/29

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.34

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 3 and 171 lb uplift at joint 2.

January 10,2008

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	0.0	White South Control
L265558	CJ05	JACK	4	1		J1924998
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:44 2008 Page 1

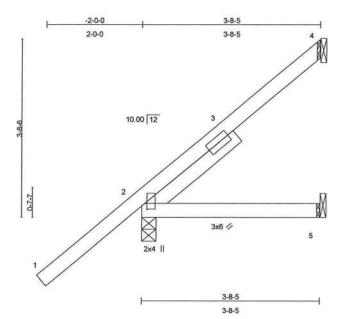


Plate Of	fsets (X, Y	(): [2:0-1-8,0-0-8]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.01	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-5	>999	240	100000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)	500000000000000000000000000000000000000					Weight: 20 lb	

LUMBE	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

SLIDER Left 2 X 4 SYP No.2 2-4-6

BRACING TOP CHORD

Structural wood sheathing directly applied or

3-8-5 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=55/Mechanical, 2=271/0-3-8, 5=17/Mechanical

Max Horz 2=214(load case 6)

Max Uplift 4=-61(load case 5), 2=-153(load case 6)

Max Grav 4=55(load case 1), 2=271(load case 1), 5=52(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-98/8, 3-4=-71/23

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.76, 2 = 0.13 and 3 = 0.00

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb Collidited joint 4 and 153 lb uplift at joint 2.

Julius Lee Trust Design Engineer Florida PE No. 34868 1 100 Castel Bay Blyd Boyston Basch, Et. 33436

January 10,2008

Scale = 1:22 6

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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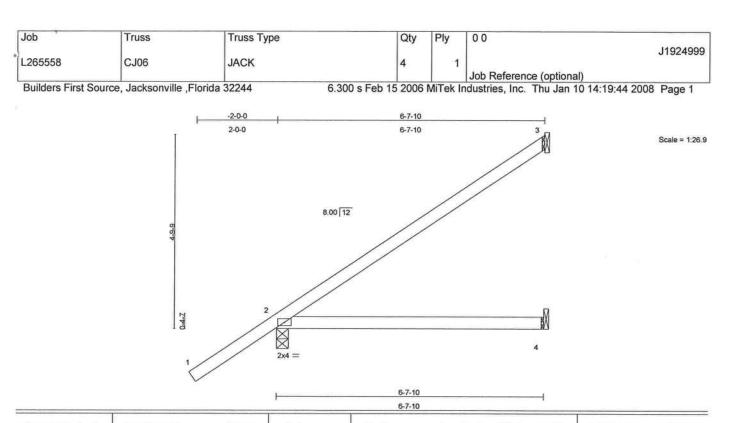


Job	Truss	Truss Type	Qty	Ply	0.0	XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
L265558	CJ05	JACK	4	1		J1924998
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:44 2008 Page 2

LOAD CASE(S) Standard





LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES** (loc) I/defl GRIP in TCLL 20.0 Plates Increase 1.25 TC 0.41 Vert(LL) -0.092-4 >861 360 MT20 244/190 7.0 TCDL 1.25 BC Lumber Increase 0.29 Vert(TL) -0.162-4 >492 240 BCLL 10.0 Rep Stress Incr WB 0.00 YES -0.00Horz(TL) 3 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 26 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=153/Mechanical, 2=347/0-3-8, 4=32/Mechanical

Max Horz 2=250(load case 6)

Max Uplift 3=-126(load case 6), 2=-162(load case 6)

Max Grav 3=153(load case 1), 2=347(load case 1), 4=96(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-151/67

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.39

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 3 and 162 lb uplift at joint 2.

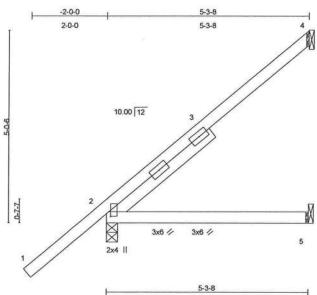
January 10,2008

LOAD CASE(S) Standard



Job *	Truss	Truss Type	Qty	Ply	00	
L265558	CJ07	JACK	2	1		J1925000
		- Constitution			Job Reference (optional)	

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						5-3-8	3					
						5-3-8	3		· K			
Plate Of	fsets (X,Y	(): [2:0-1-8,0-0-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.04	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.06	2-5	>995	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 27 lb)

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

SLIDER

Left 2 X 4 SYP No.2 3-4-15

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-3-8 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=111/Mechanical, 2=310/0-3-8, 5=25/Mechanical

Max Horz 2=265(load case 6)

Max Uplift 4=-119(load case 6), 2=-134(load case 6)

Max Grav 4=111(load case 1), 2=310(load case 1), 5=76(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/66, 2-3=-155/7, 3-4=-141/52

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.85, 2 = 0.07, 2 = 0.07 and 3 = 0.00

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 Colhi Haled shipiate and 134 lb uplift at joint 2.

January 10,2008

Scale = 1:28 4

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job *	Truss	Truss Type	Qty	Ply	00	
L265558	CJ07	JACK	2	1		J1925000
LECOCOC	0007	ONOR	-		Job Reference (optional)	

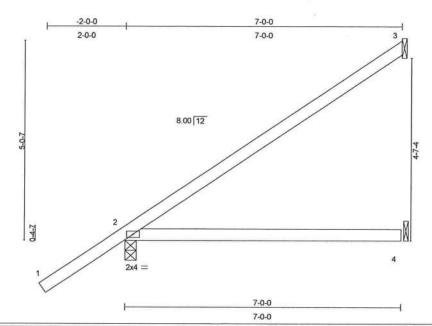
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:45 2008 Page 2

LOAD CASE(S) Standard



Job *	Truss	Truss Type	Qty	Ply	00	100000
L265558	EJ07	MONO TRUSS	7	1		J1925001
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 14:22:04 2008 Page 1



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.48	Vert(LL)	-0.11	2-4	>730	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.20	2-4	>417	240	1001114/100961	
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=164/Mechanical, 2=357/0-3-8, 4=34/Mechanical

Max Horz 2=259(load case 6)

Max Uplift 3=-136(load case 6), 2=-161(load case 6)

Max Grav 3=164(load case 1), 2=357(load case 1), 4=102(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-160/72

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.41

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 3 and 161 lb uplift at joint 2.

LOAD CASE(S) Standard

January 10,2008

Scale = 1:27.4

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265558	EJ07A	MONO TRUSS	8	1		J1925002
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:46 2008 Page 1

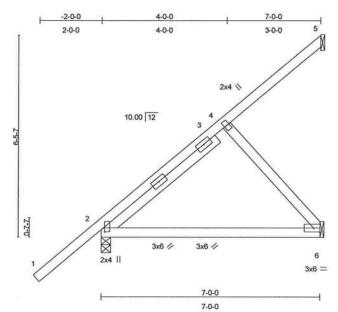


Plate Of	fsets (X,)	(): [2:0-1-8,0-0-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.12	2-6	>710	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.20	2-6	>406	240	S. S	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	110000000000000000000000000000000000000					Weight: 42 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.2 4-7-11

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=67/Mechanical, 2=359/0-3-8, 6=130/Mechanical

Max Horz 2=320(load case 6)

Max Uplift 5=-70(load case 6), 2=-123(load case 6), 6=-84(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/66, 2-3=-201/0, 3-4=-83/0, 4-5=-89/33

BOT CHORD

2-6=-152/84

WEBS

4-6=-127/230

JOINT STRESS INDEX

2 = 0.73, 2 = 0.06, 2 = 0.06, 3 = 0.00, 4 = 0.13 and 6 = 0.09

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Len Trues Design Engineer Flonda FE No. 24889 1 100 Caastal Bay Blyd Boynton Beach, FL 22435

January 10,2008

Scale = 1:34.7

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job *	Truss	Truss Type	Qty	Ply	00	
L265558	EJ07A	MONO TRUSS	8	1		J1925002
		1			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:46 2008 Page 2

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 5, 123 lb uplift at joint 2 and 84 lb uplift at joint 6.

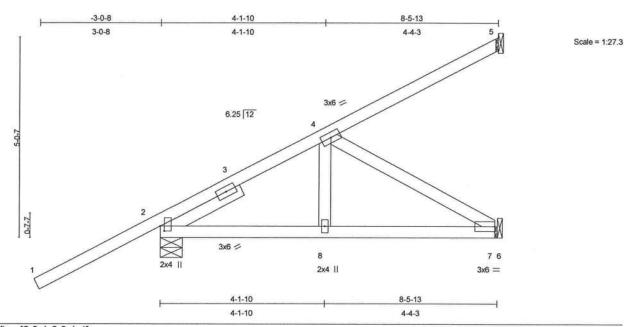
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 24869 1100 Coastal Bay Blyri 1100 Coastal Bay Blyri



Job *	Truss	Truss Type	Qty	Ply	00	
*L265558	HJ07	MONO TRUSS	2	1		J1925003
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 14:34:19 2008 Page 1



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.56	Vert(LL)	-0.01	7-8	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.03	7-8	>999	240	(WENDER ORDINARY)	
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.12	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0	1	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 46 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.2 2-3-0

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=188/Mechanical, 2=389/0-6-9, 7=164/Mechanical

Max Horz 2=259(load case 5)

Max Uplift 5=-161(load case 5), 2=-182(load case 5), 7=-33(load case 5) Max Grav 5=188(load case 1), 2=389(load case 1), 7=174(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-304/0, 3-4=-284/0, 4-5=-101/65

BOT CHORD 2-8=-93/220, 7-8=-93/220, 6-7=0/0

4-7=-254/107, 4-8=0/157 **WEBS**

JOINT STRESS INDEX

2 = 0.89, 2 = 0.14, 3 = 0.00, 4 = 0.12, 7 = 0.07 and 8 = 0.12

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 5, 182 lb uplift at joint 2 and 33 lb uplift at joint 7. E) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job ·	Truss	Truss Type	Qty	Ply	00	
L265558	HJ07	MONO TRUSS	2	1	J	11925003
	1.17001				Job Reference (optional)	

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LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54
 Trapezoidal Loads (plf)

Vert: 2=-0(F=27, B=27)-to-5=-114(F=-30, B=-30), 2=0(F=5, B=5)-to-6=-21(F=-6, B=-6)

Julius Les Trues Design Engineer Flonda PE No. 24869 1109 Coastal Bay Blvd



Job '	Truss	Truss Type	Qty	Ply	00	2017-12-0020-27-00-0
L265558	НЈ07А	MONO TRUSS	2	1		J1925004
			_		Job Reference (optional)	

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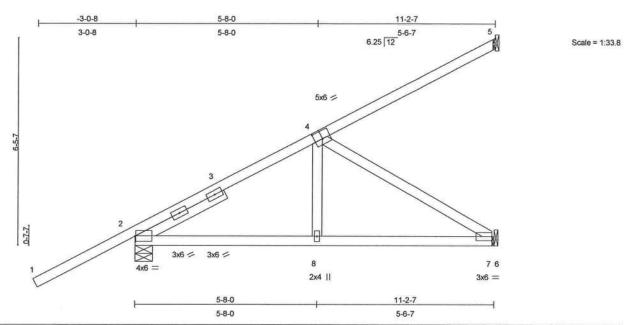


Plate Of	fsets (X,Y):	[2:0-0-4,0-2-0], [4:0-	-3-0,0-3-0]									
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.03	2-8	>999	360	MT20	244/190
TCDL	7.0	1	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.08	7-8	>999	240	Thoras-Indiana.	NAMES OF TAXABLE SAMES (MASS)
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.39	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 59 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

Left 2 X 4 SYP No.2 3-1-5 SLIDER

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0

oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=302/Mechanical, 2=521/0-6-9, 7=339/Mechanical

Max Horz 2=411(load case 5)

Max Uplift 5=-267(load case 5), 2=-177(load case 5), 7=-128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-562/0, 3-4=-477/0, 4-5=-168/102

BOT CHORD 2-8=-264/459, 7-8=-264/459, 6-7=0/0

WEBS 4-8=0/214, 4-7=-535/307

JOINT STRESS INDEX

2 = 0.77, 2 = 0.11, 2 = 0.11, 3 = 0.00, 4 = 0.44, 7 = 0.15 and 8 = 0.16

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 5, 177 lb uplift at joint 2 and 128 lb uplift at joint 7.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

January 10,2008

Continued on page 2

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This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job *	Truss	Truss Type	Qty	Ply	00	
L265558	HJ07A	MONO TRUSS	2	1		J1925004
	1.00.71	mone mose	-		Job Reference (optional)	

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LOAD CASE(S) Standard

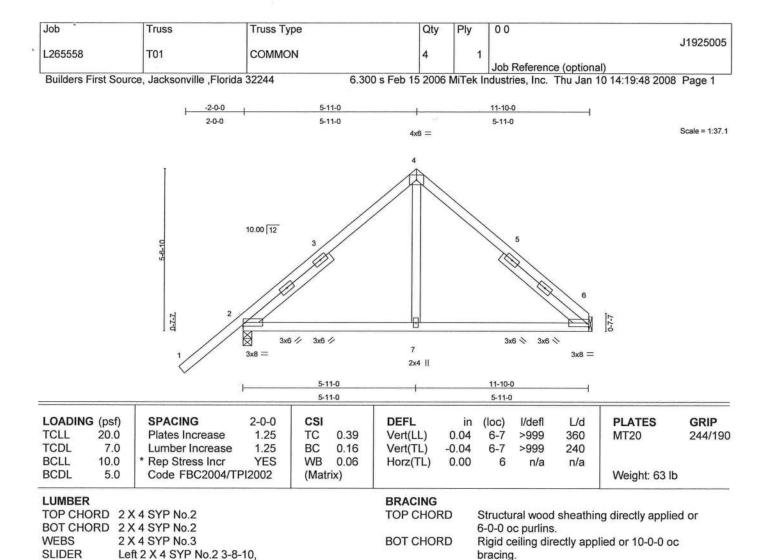
Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54
 Trapezoidal Loads (plf)

Vert: 2=-0(F=27, B=27)-to-5=-150(F=-48, B=-48), 2=0(F=5, B=5)-to-6=-28(F=-9, B=-9)

Julius Lee Truse Design Engineer Florida FE No. 34868 1 109 Ceastal Bay Blvd







REACTIONS (lb/size) 6=360/Toenail or 0-3-8 bearing, 2=505/0-3-8

Max Horz 2=205(load case 5)

Right 2 X 4 SYP No.2 3-8-10

Max Uplift 6=-99(load case 7), 2=-234(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-418/208, 3-4=-229/236, 4-5=-300/213, 5-6=-405/195

BOT CHORD 2-7=-50/230, 6-7=-50/230

WEBS 4-7=0/187

JOINT STRESS INDEX

2 = 0.68, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.66, 5 = 0.00, 6 = 0.68, 6 = 0.09, 6 = 0.09 and 7 = 0.13

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be $\,$ SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truss Cesian Engineer Florida PE No. 34869 1 100 Coastel Bay Blyd Boynton Besch, FL 03435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job '	Truss	Truss Type	Qty	Ply	00	
L265558	T01	COMMON	4	1		J1925005
		1.1.0000.853	5	8	Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 6 and 234 lb uplift at joint 2.

LOAD CASE(S) Standard





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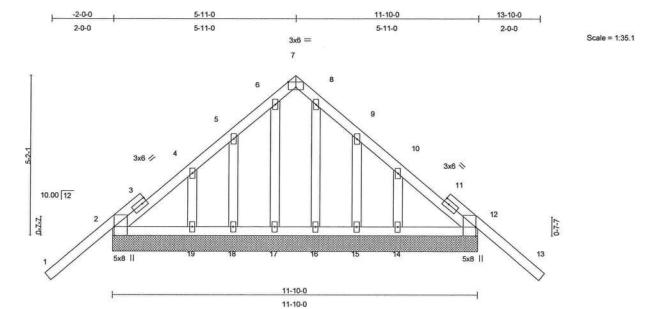


Plate Of	fsets (X,Y	'): [2:0-3-8,Edge], [7:	0-3-0,Edg	e], [12:0)-3-8,Edg	ge]						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.03	13	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.04	13	n/r	90	2000 606054	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 79 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=249/11-10-0, 12=249/11-10-0, 17=72/11-10-0, 18=93/11-10-0,

19=77/11-10-0, 16=72/11-10-0, 15=93/11-10-0, 14=77/11-10-0

Max Horz 2=-157(load case 4)

Max Uplift 2=-155(load case 6), 12=-177(load case 7), 17=-19(load case 5), 18=-109(load case 6), 19=-51(load case 5), 15=-112(load case 7),

14=-53(load case 4)

Max Grav 2=249(load case 1), 12=249(load case 1), 17=72(load case 4), 18=94(load case 10), 19=90(load case 2), 16=72(load case 1), 15=94(load case 11), 14=90(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-120/89, 3-4=-109/102, 4-5=-62/75, 5-6=-40/136, 6-7=-33/118,

7-8=-33/118, 8-9=-40/136, 9-10=-38/34, 10-11=-68/60, 11-12=-79/47, 12-13=0/65

BOT CHORD 2-19=0/231, 18-19=0/231, 17-18=0/231, 16-17=0/231, 15-16=0/231, 14-15=0/231,

12-14=0/231

6-17=-62/28, 5-18=-77/149, 4-19=-70/68, 8-16=-60/0, 9-15=-77/149, 10-14=-70/69

JOINT STRESS INDEX

WEBS

2 = 0.62, 3 = 0.00, 3 = 0.23, 4 = 0.04, 5 = 0.07, 6 = 0.03, 7 = 0.19, 8 = 0.03, 9 = 0.07, 10 = 0.04, 11 = 0.00, 11 = 0.23, 12 = 0.04, 12 = 0.04, 13 = 0.04, 14 = 0.04, 15 = 0.0.62, 14 = 0.04, 15 = 0.08, 16 = 0.02, 17 = 0.02, 18 = 0.08 and 19 = 0.04

Continued on page 2

January 10,2008

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Job	Truss	Truss Type	Qty	Ply	00
L265558	T01G	GABLE	4	1	J1925006
L203330	1016	GABLE		1	Job Reference (optional)

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2, 177 lb uplift at joint 12, 19 lb uplift at joint 17, 109 lb uplift at joint 18, 51 lb uplift at joint 19, 112 lb uplift at joint 15 and 53 lb uplift at joint 14.

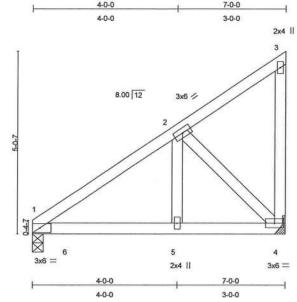
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34869 1 199 Coestel Bay Blvd



	Job	Truss	Truss Type	Qty	Ply	00	
0	L265558	T02	MONO TRUSS	1	1	J1925007	7
				100	2	Job Reference (optional)	

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Simpson HTU26

Plate Of	fsets (X,Y	'): [1:0-3-9,0-1-8]		4							4	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.04	1-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.07	1-5	>999	240	1000-000-000	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 39 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS**

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=674/0-3-8, 4=809/Mechanical

Max Horz 1=176(load case 5)

Max Uplift 1=-190(load case 5), 4=-361(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-772/179, 2-3=-56/21

BOT CHORD

1-6=-272/600, 5-6=-272/600, 4-5=-272/600

WEBS

3-4=-53/43, 2-5=-245/771, 2-4=-829/375

JOINT STRESS INDEX

1 = 0.37, 2 = 0.57, 3 = 0.02, 4 = 0.25 and 5 = 0.55

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 1 and 361 lb uplift at joint 4.

Continued on page 2

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Scale = 1:30.0

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Job ·	Truss	Truss Type	Qty	Ply	00	s services conserve a
L265558	T02	MONO TRUSS	1	1		J1925007
					Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

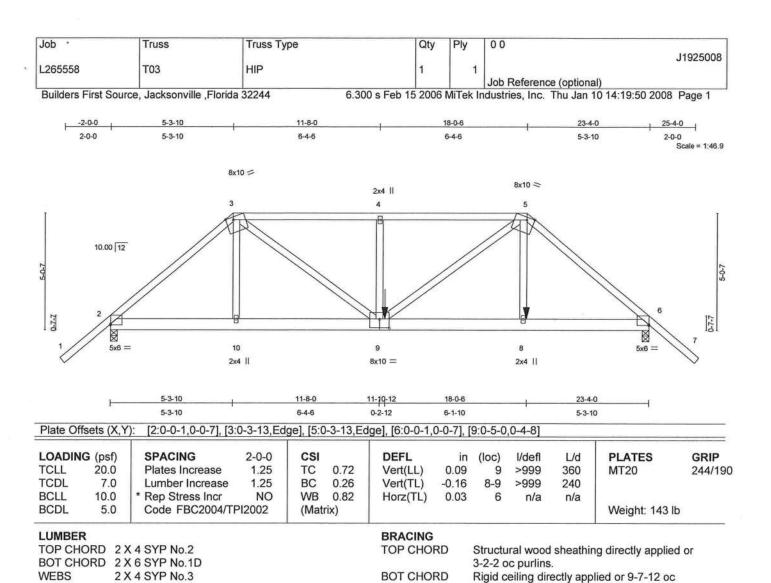
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 1-6=-10, 4-6=-190(B=-180)

Julius Les Truss Design Engineer Flonda PE No. 24869 1100 Geastal Bay Blvd





bracing.

REACTIONS (lb/size) 2=1507/0-3-8, 6=1807/0-3-8

Max Horz 2=-151(load case 3)

Max Uplift 2=-600(load case 4), 6=-757(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1921/810, 3-4=-2547/1198, 4-5=-2547/1197, 5-6=-2383/1045,

6-7=0/69

BOT CHORD 2-10=-631/1359, 9-10=-630/1361, 8-9=-711/1739, 6-8=-709/1726

WEBS 3-10=0/190, 3-9=-806/1513, 4-9=-572/407, 5-9=-563/1045, 5-8=-101/398

JOINT STRESS INDEX

2 = 0.72, 3 = 0.87, 4 = 0.33, 5 = 0.87, 6 = 0.72, 8 = 0.33, 9 = 0.35 and 10 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

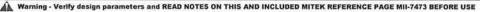
 Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

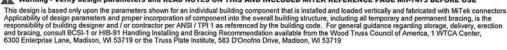
Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

EbAll hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Less Truss Design Engineer Flonda PE No. 34888 1 109 Ceastal Bay Blvd. Boynton Beach, FL 33435







Job ·	Truss	Truss Type	Qty	Ply	00	
L265558	тоз	HIP	1	1		J1925008
	75.75%				Job Reference (optional)	

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 600 lb uplift at joint 2 and 757 lb
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

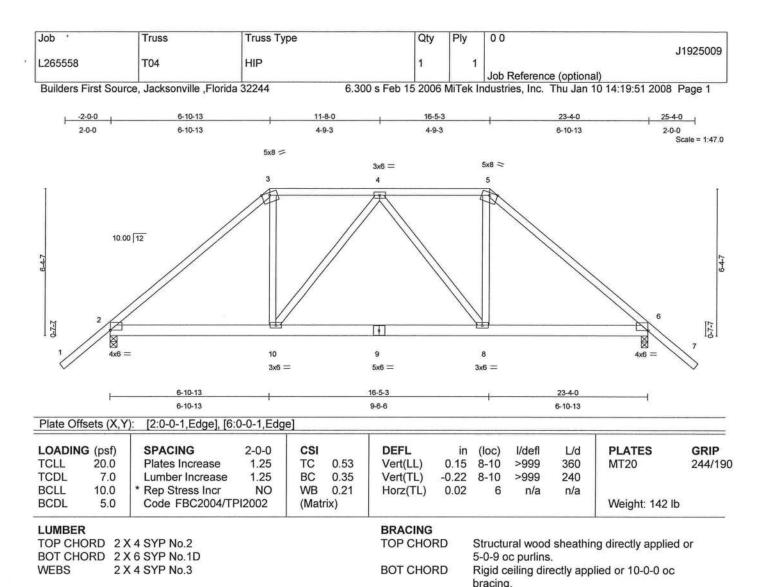
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-5=-118(F=-64), 5-7=-54, 2-9=-10, 8-9=-22(F=-12), 6-8=-10

Concentrated Loads (lb)

Vert: 9=-809(F) 8=-311(F)





REACTIONS (lb/size) 2=1135/0-3-8, 6=1135/0-3-8

Max Horz 2=194(load case 5)

Max Uplift 2=-408(load case 6), 6=-408(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1413/821, 3-4=-977/753, 4-5=-977/753, 5-6=-1413/821, 6-7=0/69

BOT CHORD 2-10=-412/963, 9-10=-419/1055, 8-9=-419/1055, 6-8=-315/963 WEBS 3-10=-238/573, 4-10=-209/211, 4-8=-209/211, 5-8=-238/573

JOINT STRESS INDEX

2 = 0.70, 3 = 0.73, 4 = 0.39, 5 = 0.73, 6 = 0.70, 8 = 0.39, 9 = 0.65 and 10 = 0.39

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Les Truss Design Engineer Flonda PE No. 34868 1109 Chastal Bay Blvd Bovnton Beach, FL 23425

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	00	
1.265558	T04	HIP	1	1	J1925	6009
			1		Job Reference (optional)	100

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NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 2 and 408 lb uplift at joint 6.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-10=-10, 8-10=-70, 6-8=-10



Job .	Truss	Truss Type	Qty	Ply	00	
L265558	T05	HIP	1	1		J1925010
					Job Reference (optional)	

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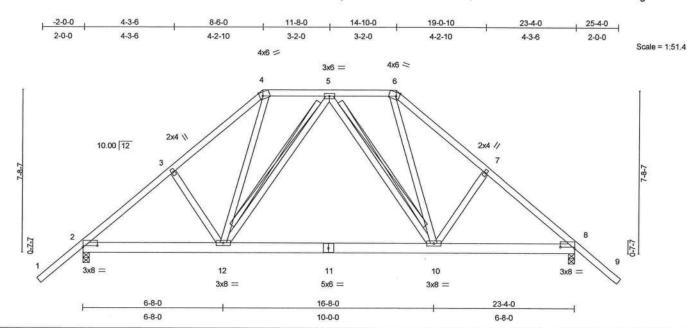


Plate Of	fsets (X,Y):	[2:0-8-1,0-0-13], [4:	0-1-13,Edg	je], [6:0-	1-13,Edge	e], [8:0-8-1,0-0	-13]					
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.21	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.30	10-12	>910	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	, ,					Weight: 164 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-11-10 oc purlins.

BOT CHORD WEBS Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 5-12,

5-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1162/0-3-8, 8=1162/0-3-8

Max Horz 2=-238(load case 4)

Max Uplift 2=-428(load case 6), 8=-428(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1485/864, 3-4=-1351/917, 4-5=-833/694, 5-6=-833/694, 6-7=-1351/917,

7-8=-1485/864, 8-9=0/69

2-12=-458/1033, 11-12=-311/868, 10-11=-311/868, 8-10=-402/1033

WEBS 3-12=-97/209, 5-12=-152/182, 5-10=-152/182, 7-10=-97/209, 4-12=-382/599,

6-10=-382/599

JOINT STRESS INDEX

BOT CHORD

2 = 0.84, 3 = 0.34, 4 = 0.57, 5 = 0.42, 6 = 0.57, 7 = 0.34, 8 = 0.84, 10 = 0.65, 11 = 0.81 and 12 = 0.65

NOTES

1) Unbalanced roof live loads have been considered for this design.

January 10,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors,
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the
responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection
and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job '	Truss	Truss Type	Qty	Ply	00	007007949 TX-005-504
L265558	T05	HIP	1	1		J1925010
	12.50				Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 428 lb uplift at joint 2 and 428 lb uplift at joint 8.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 6-9=-54, 2-12=-10, 10-12=-70, 8-10=-10

Julius Les Trues Cesign Engineer Florida PE No. 34868 1199 Ceastal Bay Blvd





6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 14:28:01 2008 Page 1

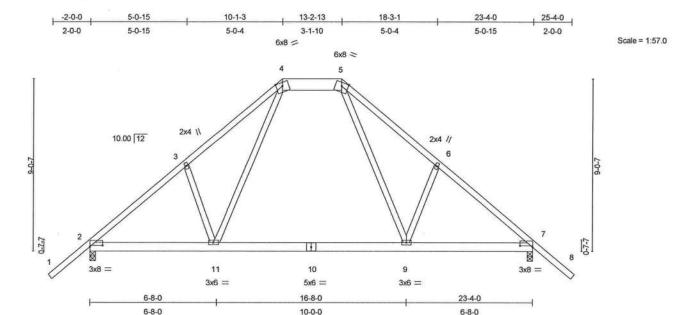


Plate Offsets (X,Y): [2:0-8-1,0-0-13], [7:0-8-1,0-0-13]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.21	9-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.31	9-11	>904	240	CONTRACTOR	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.68	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 151 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

4-5 2 X 8 SYP No.1D

BOT CHORD 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 5-0-6

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1162/0-3-8, 7=1160/0-3-8

Max Horz 2=-280(load case 4)

Max Uplift 2=-436(load case 6), 7=-435(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1488/850, 3-4=-1362/976, 4-5=-749/680, 5-6=-1353/967, 6-7=-1482/847,

7-8=0/69

BOT CHORD 2-11=-416/1034, 10-11=-227/749, 9-10=-227/749, 7-9=-377/1030 WEBS 3-11=-184/272, 4-11=-457/631, 5-9=-450/626, 6-9=-184/271

JOINT STRESS INDEX

2 = 0.78, 3 = 0.34, 4 = 0.37, 5 = 0.37, 6 = 0.34, 7 = 0.78, 9 = 0.56, 10 = 0.80 and 11 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

E) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Trues Design Engineer Florida FE No. 34888 1 100 Coastel Bay Blvd Boynton Beach, FL 33435

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job ،	Truss	Truss Type	Qty	Ply	0.0	
L265558	T06	HIP	1	1		J1925011
	1000	10,000			Job Reference (optional)	

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6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 436 lb uplift at joint 2 and 435 lb uplift at joint

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-54, 5-8=-54, 2-11=-10, 9-11=-70, 7-9=-10





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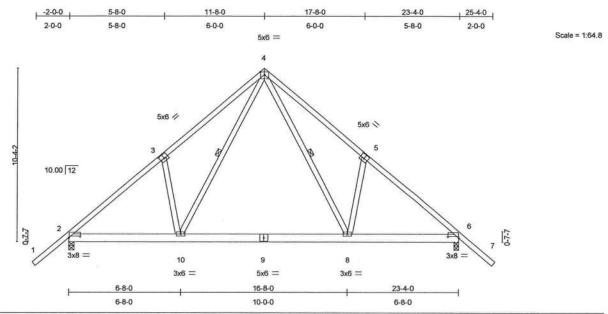


Plate Offsets (X,Y): [2:0-8-1,0-0-13], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-8-1,0-0-13]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.21	8-10	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.32	8-10	>859	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 154 lb	

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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

WFBS

2 X 4 SYP No.3

BRACING

WEBS

TOP CHORD

Structural wood sheathing directly applied or

5-0-4 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing. 1 Row at midpt

4-10, 4-8

REACTIONS (lb/size) 2=1162/0-3-8, 6=1162/0-3-8

Max Horz 2=-325(load case 4)

Max Uplift 2=-440(load case 6), 6=-440(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1488/801, 3-4=-1390/1008, 4-5=-1390/1008, 5-6=-1488/801,

BOT CHORD 2-10=-335/1034, 9-10=-121/660, 8-9=-121/660, 6-8=-335/1034

3-10=-225/360, 4-10=-558/753, 4-8=-558/753, 5-8=-225/360 **WEBS**

JOINT STRESS INDEX

2 = 0.76, 3 = 0.67, 4 = 0.71, 5 = 0.67, 6 = 0.76, 8 = 0.61, 9 = 0.79 and 10 = 0.61

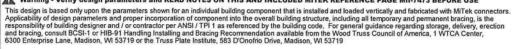
NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive deads page 2







Job	Truss	Truss Type	Qty	Ply	00	
L265558	T07	COMMON	3	1		J1925012
	1932-3053				Job Reference (optional)	

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NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 2 and 440 lb uplift at joint 6.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70, 6-8=-10

Julius Lee Truse Design Engineer Flonda PE No. 34866 1109 Ceestal Bay Blvd





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:55 2008 Page 1

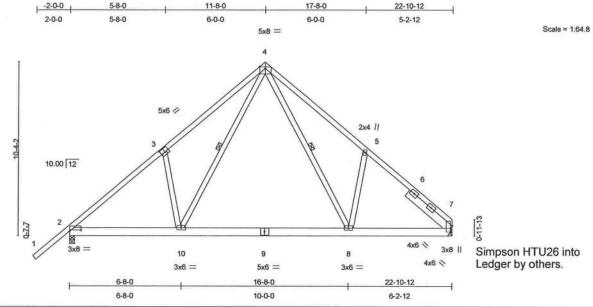


Plate Offsets (X,Y): [2:0-8-1,0-0-13], [3:0-3-0,0-3-0], [7:0-4-8,0-1-4]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.20	8-10	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.32	8-10	>852	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.02	7	n/a	n/a		
BCDL	BCDL 5.0 Code FBC2004/TI		2002	(Mat	rix)						Weight: 156 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3

SLIDER Right 2 X 6 SYP No.1D 3-4-15

REACTIONS (lb/size) 7=1030/Mechanical, 2=1155/0-3-8

Max Horz 2=362(load case 5)

Max Uplift 7=-312(load case 7), 2=-438(load case 6)

BRACING

TOP CHORD Structural wood sheathing directly applied or

4-11-8 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS 1 Row at midpt

4-10, 4-8

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1477/804, 3-4=-1377/1010, 4-5=-1341/1021, 5-6=-1359/827,

6-7=-1451/805

BOT CHORD 2-10=-460/1025, 9-10=-161/653, 8-9=-161/653, 7-8=-463/994 WEBS 3-10=-222/358, 4-10=-549/751, 4-8=-572/711, 5-8=-189/339

JOINT STRESS INDEX

2 = 0.76, 3 = 0.66, 4 = 0.57, 5 = 0.33, 6 = 0.00, 7 = 0.87, 7 = 0.26, 7 = 0.26, 8 = 0.57, 9 = 0.79 and 10 = 0.61

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp
 B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive by page 2

Truse Design Engineer Florida PE No. 34865 1109 Ccastal Bay Blyd Boynton Beach, FL 93436

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job '	Truss	Truss Type	Qty	Ply	00	
L265558	тов	COMMON	3	1		J1925013
	1.00	Comment	ľ	,	Job Reference (optional)	

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NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 7 and 438 lb uplift at joint 2.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

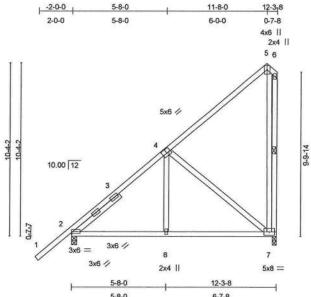
Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70, 7-8=-10

Julius Lee Truss Cosign Engineer Flonds PE No. 24869 1169 Coestal Bay Blvd



Job ·	Truss	Truss Type	Qty	Ply	00	
L265558	Т09	COMMON	5	1		J1925014
	1.00				Job Reference (optional)	

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		5x6 //				
10.00 12	. //	4		Ø	9-9-14	
2		g				
1 3x6 =	3x6 //	8 2x4		7 5x8 =		
1-	5-8-0 5-8-0		12-3-8 6-7-8	—		

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.06	7-8	>999	240	11111414141	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.31	Horz(TL)	-0.01	7	n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3 *Except*

6-7 2 X 4 SYP No.2

SLIDER

Left 2 X 4 SYP No.2 3-6-10

WEBS

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

1 Row at midpt

5-7

REACTIONS (lb/size) 2=507/0-3-8, 7=379/0-3-0

Max Horz 2=460(load case 6)

Max Uplift 2=-116(load case 6), 7=-280(load case 6) Max Grav 2=507(load case 10), 7=379(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-423/0, 3-4=-245/0, 4-5=-131/80, 5-6=-182/125, 6-7=-301/193

BOT CHORD 2-8=-306/241, 7-8=-305/241

WEBS 4-8=0/193, 5-7=-356/532, 4-7=-284/358

JOINT STRESS INDEX

2 = 0.53, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.35, 5 = 0.66, 6 = 0.51, 7 = 0.35 and 8 = 0.14

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

January 10,2008

Scale = 1:64.8

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265558	T09	COMMON	5	1		J1925014
	100.00				Job Reference (optional)	

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NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 7.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2 and 280 lb uplift at joint 7.

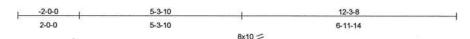
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34868 1109 Coestel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T10	MONO HIP	1	1		J1925015
	13505055		1070		Job Reference (optional)	12

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Scale = 1:35.3

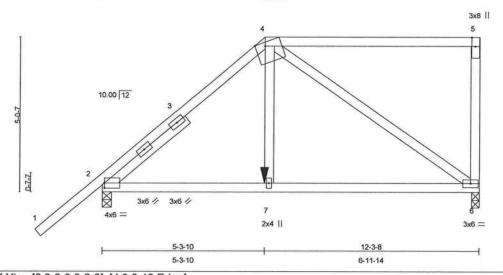


Plate Of	fsets (X, Y	(): [2:0-0-0,0-2-0], [4:	:0-3-13,Ed	ge]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.11	6-7	>999	240	110000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.01	6	n/a	n/a	1	
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 73 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3 *Except*

5-6 2 X 4 SYP No.2

SLIDER

Left 2 X 4 SYP No.2 3-5-0

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=878/0-3-0, 2=836/0-3-8

Max Horz 2=270(load case 5)

Max Uplift 6=-416(load case 4), 2=-379(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-921/307, 3-4=-756/332, 4-5=-106/58, 5-6=-411/295

BOT CHORD 2-7=-317/635, 6-7=-319/646 WEBS 4-7=-72/369, 4-6=-658/329

JOINT STRESS INDEX

2 = 0.87, 2 = 0.17, 2 = 0.17, 3 = 0.00, 4 = 0.87, 5 = 0.82, 6 = 0.74 and 7 = 0.26

NOTES

 Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

2) Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Charles in sage assumed to be SYP No.2 crushing capacity of 565.00 psi

Truss Design Engineer Florida PE No. 34888 1 109 Caestal Bay Blvd Boynton Beach, FL 33435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265558	T10	MONO HIP	1	1		J1925015
22.5.6.5					Job Reference (optional)	

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5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 416 lb uplift at joint 6 and 379 lb uplift at joint 2.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-118(B=-64), 2-7=-10, 6-7=-22(B=-12)

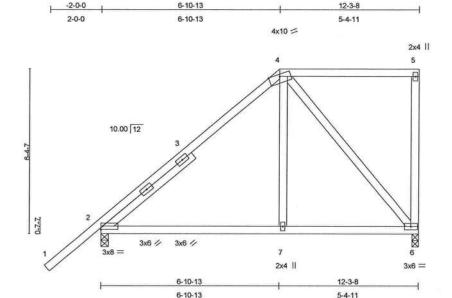
Concentrated Loads (lb)

Vert: 7=-311(B)



Job	Truss	Truss Type	Qty	Ply	00	4.0000000000000000000000000000000000000
L265558	T11	MONO HIP	1	1		J1925016
				8)	Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.04	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.07	2-7	>999	240	111111111111111111111111111111111111111	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.34	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 79 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.2 4-5-8

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=373/0-3-0, 2=516/0-3-8

Max Horz 2=320(load case 6)

Max Uplift 6=-159(load case 5), 2=-200(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-408/98, 3-4=-278/116, 4-5=-12/12, 5-6=-117/108

BOT CHORD 2-7=-225/213, 6-7=-225/215

WEBS 4-7=0/201, 4-6=-311/335

JOINT STRESS INDEX

2 = 0.64, 2 = 0.08, 2 = 0.08, 3 = 0.00, 4 = 0.77, 5 = 0.57, 6 = 0.18 and 7 = 0.14

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6. Continued on page 2

Trues Cesign Engineer Florida PE No. 34869 1109 Coastal Bay Blvd Boynton Beach, Ft. 33435

January 10,2008

Scale = 1:42.0

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Job ·	Truss	Truss Type	Qty	Ply	00	
L265558	T11	MONO HIP	1	1		J1925016
	135.5	100.2002.0.00			Job Reference (optional)	

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NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 6 and 200 lb uplift at joint 2.

LOAD CASE(S) Standard

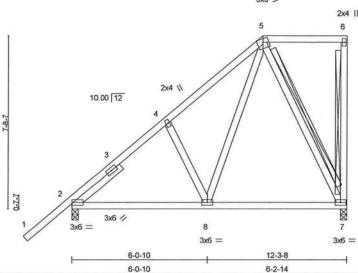


Builders First Source, Jacksonville ,Florida 32244

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Scale: 1/4"=1"



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.39	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	7-8	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.19	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0		Code FBC2004/TI	PI2002	(Mati	rix)						Weight: 88 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER Left 2 X 4 SYP No.2 2-8-1

BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 6-7, 5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=373/0-3-8, 2=516/0-3-8

Max Horz 2=371(load case 6)

Max Uplift 7=-170(load case 6), 2=-180(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-442/58, 3-4=-306/76, 4-5=-326/159, 5-6=-9/1, 6-7=-80/74

BOT CHORD 2-8=-323/260, 7-8=-157/136

WEBS 4-8=-144/248, 5-8=-184/245, 5-7=-293/352

JOINT STRESS INDEX

2 = 0.72, 2 = 0.18, 3 = 0.00, 4 = 0.14, 5 = 0.58, 6 = 0.23, 7 = 0.23 and 8 = 0.23

NOTES

 Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Flonds PE No. 24868 1109 Ceastal Bay Blvd Boynton Besch, FL 92426

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job ·	Truss	Truss Type	Qty	Ply	00	
L265558	T12	MONO HIP	1	1		J1925017
	11000				Job Reference (optional)	

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NOTES

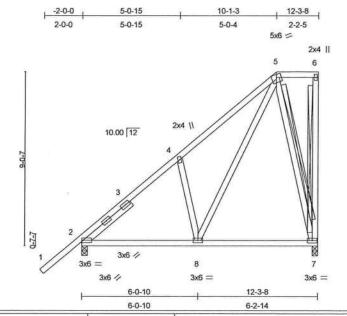
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 7 and 180 lb uplift at joint 2.

LOAD CASE(S) Standard



Job .	Truss	Truss Type	Qty	Ply	00	
L265558	T13	MONO HIP	1	1		J1925018
	2.75	60000000 5.000	15	- 2	Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	7-8	>999	240	Water Areas	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 96 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 WEBS

SLIDER Left 2 X 4 SYP No.2 3-2-5 BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.3 - 6-7, 5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=373/0-3-8, 2=516/0-3-8

Max Horz 2=422(load case 6)

Max Uplift 7=-222(load case 6), 2=-151(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-436/0, 3-4=-275/0, 4-5=-355/181, 5-6=-7/0, 6-7=-34/42

BOT CHORD 2-8=-321/252, 7-8=-103/81

WEBS 5-8=-334/295, 4-8=-212/355, 5-7=-344/454

JOINT STRESS INDEX

2 = 0.67, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.19, 5 = 0.46, 6 = 0.17, 7 = 0.36 and 8 = 0.28

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Scale = 1:56.3

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Job .	Truss	Truss Type	Qty	Ply	00	name a material a company a co
L265558	T13	MONO HIP	1	1		J1925018
	1,111,50				Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 7 and 151 lb uplift at joint

LOAD CASE(S) Standard



Job *	Truss	Truss Type	Qty	Ply	00
L265558	T14	MONO HIP	1	1	J192501
	170.505				Job Reference (optional)

11-8-6

6-7-8

5-8-0

-2-0-0

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12-3-8

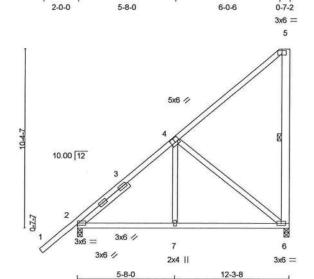


Plate Offsets (X,Y): [4:0-3-0,0-3-0], [5:0-3-0,Edge] SPACING 2-0-0 LOADING (psf) CSI DEFL **PLATES** GRIP I/defl L/d in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.39 Vert(LL) -0.036-7 >999 360 244/190 MT20 TCDL 1.25 BC 0.16 70 Lumber Increase -0.05Vert(TL) 6-7 >999 240 10.0 BCLL Rep Stress Incr YES WB 0.32 Horz(TL) -0.016 n/a n/a BCDL Code FBC2004/TPI2002 5.0 (Matrix) Weight: 93 lb

5-8-0

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

5-6 2 X 6 SYP No.1D

SLIDER Left 2 X 4 SYP No.2 3-8-5

BRACING

BOT CHORD

WEBS

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

5-6

REACTIONS

(lb/size) 6=370/0-3-8, 2=514/0-3-8

Max Horz 2=478(load case 6)

Max Uplift 6=-293(load case 6), 2=-106(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-431/0, 3-4=-254/0, 4-5=-167/84, 5-6=-159/242

BOT CHORD 2-7=-318/248, 6-7=-317/249

WFRS 4-7=0/187, 4-6=-286/367

JOINT STRESS INDEX

2 = 0.57, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.37, 5 = 0.86, 6 = 0.41 and 7 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 293 lb uplift at Comintie and 1966 buplift at joint 2.

January 10,2008

Scale = 1:62.9

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job ·	Truss	Truss Type	Qty	Ply	0 0	
L265558	T14	MONO HIP	1	1		J1925019
L203330	114	MONO THE			Job Reference (optional)	

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LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T15	MONO TRUSS	3	1		J1925020
LZOSSSS	1115	Mone moos	3		Job Reference (optional)	

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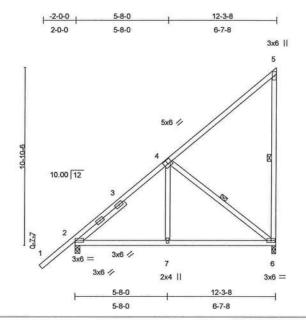


Plate Of	fsets (X,	Y): [4:0-3-0,0-3-0]		1								
LOADING (psf) SPA		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.07	6-7	>999	240	All they developed the Co.	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 86 lb	
LUMBE	R					BRACING						
TOP CH	TOP CHORD 2 X 4 SYP No.2					TOP CHO	RD	Structu	iral wood	sheathir	ng directly applie	d or
BOT CHORD 2 X 4 SYP No.2							6-0-0 c	c purlins	, except	end verticals.		
WEBS 2 X 4 SYP No.3						BOT CHORD Rigid ceiling directly applied or 10		lied or 10-0-0 oc	į.			
SLIDER Left 2 X 4 SYP No.2 3-9-8							bracino					

WEBS

1 Row at midpt

REACTIONS (lb/size) 6=373/0-3-0, 2=516/0-3-8

Max Horz 2=485(load case 6)

Max Uplift 6=-299(load case 6), 2=-102(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-438/0, 3-4=-261/0, 4-5=-173/87, 5-6=-153/235

BOT CHORD 2-7=-329/256, 6-7=-328/257

WEBS 4-7=0/199, 4-6=-305/389

JOINT STRESS INDEX

2 = 0.57, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.42, 5 = 0.46, 6 = 0.37 and 7 = 0.14

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Continuite mechanical connection (by others) of truss to bearing plate at joint(s) 6.

Truss Design Engineer Florida PE No. 34889 1109 Coestel Bay Blvd Boynton Beach, Ft. 93435

5-6, 4-6

January 10,2008

Scale = 1:66.5



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Job	Truss	Truss Type	Qty	Ply	00	
L265558	T15	MONO TRUSS	3	1		J1925020
	1.0	meno meso			Job Reference (optional)	

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NOTES

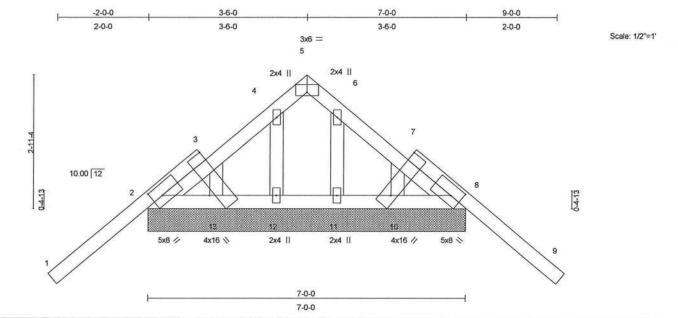
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 6 and 102 lb uplift at joint 2.

LOAD CASE(S) Standard



Job *	Truss	Truss Type	Qty	Ply	00	
L265558	T16G	GABLE	1	1		J1925021
					Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.06	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.10	9	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)						Weight: 43 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

OTHERS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=527/7-0-0, 8=527/7-0-0, 12=174/7-0-0, 13=-5/7-0-0, 11=174/7-0-0, 10=-5/7-0-0

Max Horz 2=110(load case 6)

Max Uplift 2=-323(load case 6), 8=-328(load case 7), 12=-85(load case 6), 13=-11(load case

7), 11=-75(load case 7), 10=-10(load case 4)

Max Grav 2=527(load case 1), 8=527(load case 1), 12=174(load case 1), 13=84(load case 6), 11=174(load case 1), 10=83(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-47/155, 2-3=-176/80, 3-4=-49/37, 4-5=-62/51, 5-6=-62/51, 6-7=-49/37, 7-8=-176/80,

8-9=-47/155

BOT CHORD 2-13=0/254, 12-13=0/250, 11-12=0/250, 10-11=0/250, 8-10=0/254

WEBS 4-12=-161/143, 3-13=-91/22, 6-11=-161/143, 7-10=-91/22

JOINT STRESS INDEX

2 = 0.65, 3 = 0.00, 4 = 0.09, 5 = 0.02, 6 = 0.09, 7 = 0.00, 8 = 0.65, 10 = 0.05, 11 = 0.08, 12 = 0.08 and 13 = 0.05

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

January 10,2008

Continued on page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job '	Truss	Truss Type	Qty	Ply	0 0	
L265558	T16G	GABLE	1	1		J1925021
					Job Reference (optional)	

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NOTES

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 2, 328 lb uplift at joint 8, 85 lb uplift at joint 12, 11 lb uplift at joint 13, 75 lb uplift at joint 11 and 10 lb uplift at joint 10.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-118, 5-9=-118, 2-8=-10

Julius Lee Trues Cesign Engineer Florida PE No. 24888 1109 Cessiel Bay Blyd Boymon Beach, Ft. 33436





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Scale = 1:51.5

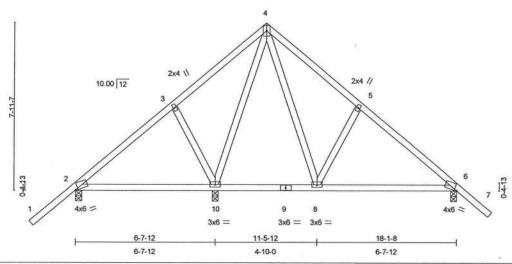


Plate Offsets	(X,Y)):	[2:0-1-6,0-0-6], [6:0-1-6,0-0-6]
---------------	-------	----	----------------------------------

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.12	2-10	>650	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.10	2-10	>744	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 105 lb	

ı	1	1	n	Л	R	E	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=339/0-3-8, 10=559/0-3-8, 6=458/0-3-8

Max Horz 2=258(load case 5)

Max Uplift 2=-257(load case 6), 10=-292(load case 6), 6=-204(load case 7)

Max Grav 2=342(load case 10), 10=559(load case 1), 6=458(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/73, 2-3=-179/109, 3-4=-64/160, 4-5=-290/260, 5-6=-407/173, 6-7=0/55

BOT CHORD

2-10=-104/203, 9-10=0/258, 8-9=0/258, 6-8=0/249

WEBS

3-10=-196/297, 4-10=-355/97, 4-8=-164/242, 5-8=-197/272

JOINT STRESS INDEX

2 = 0.82, 3 = 0.33, 4 = 0.47, 5 = 0.33, 6 = 0.82, 8 = 0.49, 9 = 0.26 and 10 = 0.49

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Les Trues Design Engineer Planda FE No. 34889 1100 Ceastal Bay Blvd Boynton Beach, FL 33435

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Coliva page 2

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	0.0	
L265558	T17	COMMON	2	1		J1925022
					Job Reference (optional)	

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NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

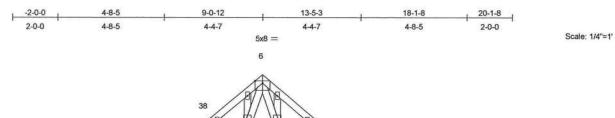
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 lb uplift at joint 2, 292 lb uplift at joint 10 and 204 lb uplift at joint 6.

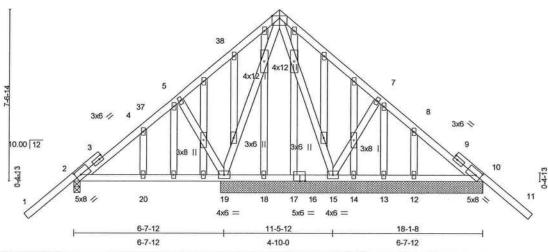
LOAD CASE(S) Standard



Job *	Truss	Truss Type	Qty	Ply	0 0	
L265558	T17G	GABLE	1	1		J1925023
		07.12-2	ľ		Job Reference (optional)	

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[2:0-2-12,0-2-12], [10:0-2-12,0-2-12], [16:0-2-12,0-0-4] Plate Offsets (X,Y): LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES GRIP** in (loc) I/defl TCLL 20.0 Plates Increase 1.25 TC 0.79 Vert(LL) 0.04 19-20 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.20 Vert(TL) 0.03 19-20 >999 240 BCLL 10.0 0.35 NO WB Rep Stress Incr Horz(TL) 0.00 10 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 166 lb

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=399/0-3-8, 19=727/11-7-8, 15=570/11-7-8, 10=590/11-7-8, 18=-6/11-7-8, 17=22/11-7-8, 14=10/11-7-8, 13=13/11-7-8, 12=155/11-7-8

Max Horz 2=236(load case 5)

Max Uplift 2=-317(load case 6), 19=-578(load case 6), 15=-325(load case 6), 10=-339(load case 7), 18=-6(load case 1), 17=-3(load case 4), 13=-10(load case 7), 12=-84(load case 6)

Max Grav 2=403(load case 10), 19=727(load case 1), 15=570(load case 1), 10=596(load case 11), 18=48(load case 4), 17=62(load case 2), 14=35(load case 2), 13=26(load case 2), 12=155(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/73, 2-3=-290/279, 3-4=-225/304, 4-37=-186/206, 5-37=-84/138, 5-38=-23/233,

BOT CHORD 2-20=-114/173, 19-20=-114/173, 18-19=-26/251, 17-18=-26/251, 16-17=-26/251,

15-16=-26/251, 14-15=-9/129, 13-14=-9/129, 12-13=-9/129, 10-12=-9/129

WEBS 5-19=-423/715, 6-19=-337/370, 6-15=-309/258, 7-15=-321/502, 4-20=-155/78,

8-12=-138/144

JOINT STRESS INDEX

2 = 0.75, 3 = 0.00, 3 = 0.53, 4 = 0.34, 5 = 0.40, 6 = 0.60, 7 = 0.40, 8 = 0.34, 9 = 0.00, 9 = 0.53, 10 = 0.75, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.35, 16 = 0.46, 17 = 0.00, 18 = 0.34, 19 = 0.35, 20 = 0.34, 21 = 0.33, 22 = 0.34, 23 = 0.16, 24 = 0.34, 25 = 0.79, 26 = 0.79,

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job '	Truss	Truss Type	Qty	Ply	00	000000000000000000000000000000000000000
L265558	T17G	GABLE	1	1		J1925023
					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2, 578 lb uplift at joint 19, 325 lb uplift at joint 15, 339 lb uplift at joint 10, 6 lb uplift at joint 18, 3 lb uplift at joint 17, 10 lb uplift at joint 13 and 84 lb uplift at joint 12.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-37=-54, 37-38=-141(F=-87), 6-38=-114(F=-60), 6-11=-114(F=-60), 2-10=-10

Julius Lee Truse Design Engineer Flonda FE No. 34869 1 100 Caestal Bay Blyd Boynton Beach, Ft. 33435



Job `	Truss	Truss Type	Qty	Ply	0.0	
L265558	T18	COMMON	1	1		J1925024
					Job Reference (optional)	

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Scale: 1/4"=1"

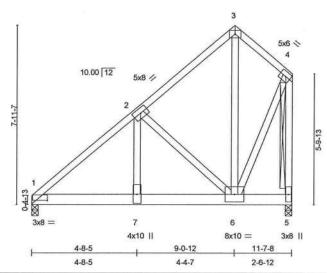


Plate Of	fsets (X,Y):	[1:0-8-6,0-1-14]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.06	1-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.11	1-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)						Weight: 91 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3 *Except*

4-5 2 X 4 SYP No.2

BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 3-7-6 oc purlins, except end verticals. Rigid ceiling directly applied or 8-5-15 oc bracing.

2 X 4 SYP No.3 - 4-5

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

REACTIONS

(lb/size) 1=2486/0-3-8, 5=2486/0-3-8

Max Horz 1=240(load case 4)

Max Uplift 1=-899(load case 5), 5=-1008(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2606/916, 2-3=-969/378, 3-4=-919/388, 4-5=-2142/878

BOT CHORD 1-7=-857/1946, 6-7=-857/1946, 5-6=-7/15

WEBS 2-7=-733/1943, 2-6=-1695/773, 3-6=-402/975, 4-6=-686/1665

JOINT STRESS INDEX

1 = 0.84, 2 = 0.69, 3 = 0.58, 4 = 0.77, 5 = 0.35, 6 = 0.47 and 7 = 0.47

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

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C) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job `	Truss	Truss Type	Qty	Ply	00	
L265558	T18	COMMON	1	1		J1925024
					Job Reference (optional)	

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NOTES

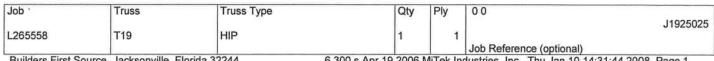
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 899 lb uplift at joint 1 and 1008 lb uplift at
- 6) Girder carries tie-in span(s): 25-5-0 from 0-0-0 to 11-7-8
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

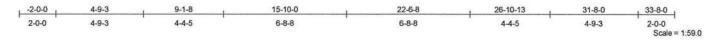
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 1-5=-385(F=-375)





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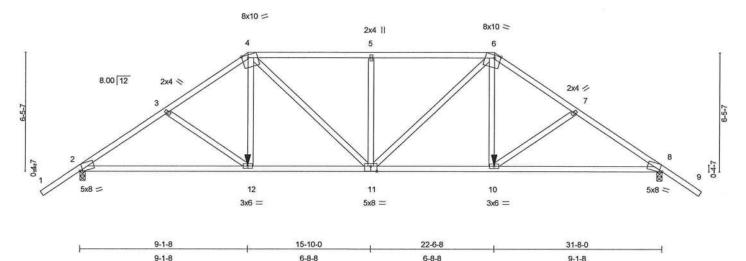


Plate Of	fsets (X,Y):	[2:0-1-13,Edge], [4:0	0-4-0,Edge], [6:0-4-	0,Edge],	[8:0-1-13,Edge	e], [11:0	-4-0,0-3	-4]			
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.30	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.42	10-11	>895	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.85	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 175 lb	

BRACING

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

LUMBER

TOP CHORD Structural wood sheathing directly applied or 3-2-2

9-1-8

BOT CHORD Rigid ceiling directly applied or 4-8-12 oc bracing.

REACTIONS (lb/size) 2=2170/0-3-8, 8=2170/0-3-8

Max Horz 2=-200(load case 3)

Max Uplift 2=-1202(load case 5), 8=-1202(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-3352/1915, 3-4=-3179/1916, 4-5=-3169/1961, 5-6=-3169/1961,

6-7=-3179/1916, 7-8=-3352/1916, 8-9=0/62

BOT CHORD 2-12=-1673/2681, 11-12=-1634/2622, 10-11=-1491/2622, 8-10=-1473/2681

WEBS 3-12=-158/98, 4-12=-544/850, 4-11=-605/820, 5-11=-507/396, 6-11=-605/820,

6-10=-544/850, 7-10=-158/98

JOINT STRESS INDEX

2 = 0.86, 3 = 0.34, 4 = 0.83, 5 = 0.34, 6 = 0.83, 7 = 0.34, 8 = 0.86, 10 = 0.56, 11 = 0.82 and 12 = 0.56

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Continued on page 2

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Job *	Truss	Truss Type	Qty	Ply	00
L265558	T19	HIP	1	1	J1925025
					Job Reference (optional)

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1202 lb uplift at joint 2 and 1202 lb uplift at joint 8.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

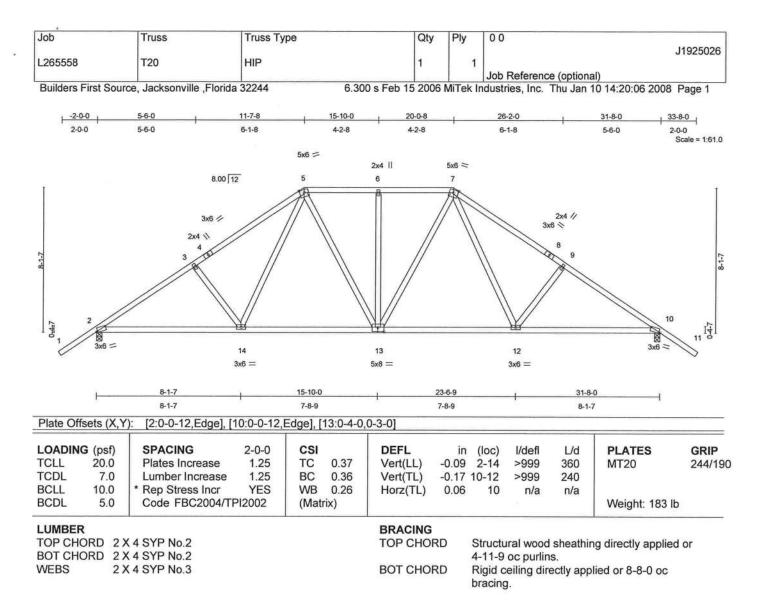
Vert: 1-4=-54, 4-6=-72(F=-18), 6-9=-54, 2-12=-10, 10-12=-69(F=-59), 8-10=-10

Concentrated Loads (lb)

Vert: 12=-535(F) 10=-535(F)

Julius Lee Truse Design Engineer Flonda PE No. 34869 1189 Coesial Bay Blvd





REACTIONS (lb/size) 2=1124/0-3-8, 10=1124/0-3-8

Max Horz 2=-256(load case 4)

Max Uplift 2=-391(load case 6), 10=-391(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1536/888, 3-4=-1364/878, 4-5=-1272/905, 5-6=-1000/790,

6-7=-1000/790, 7-8=-1272/905, 8-9=-1364/878, 9-10=-1536/888, 10-11=0/62

BOT CHORD 2-14=-515/1200, 13-14=-294/935, 12-13=-259/935, 10-12=-515/1200

WEBS 3-14=-245/297, 5-14=-193/333, 5-13=-190/259, 6-13=-197/151, 7-13=-190/259,

7-12=-193/333, 9-12=-245/297

JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.33, 5 = 0.72, 6 = 0.33, 7 = 0.72, 8 = 0.33, 9 = 0.33, 10 = 0.78, 12 = 0.46, 13 = 0.38 and 14 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

Continued on page 2



Job'	Truss	Truss Type	Qty	Ply	00	
L265558	T20	HIP	1	1		J1925026
	1.22				Job Reference (optional)	

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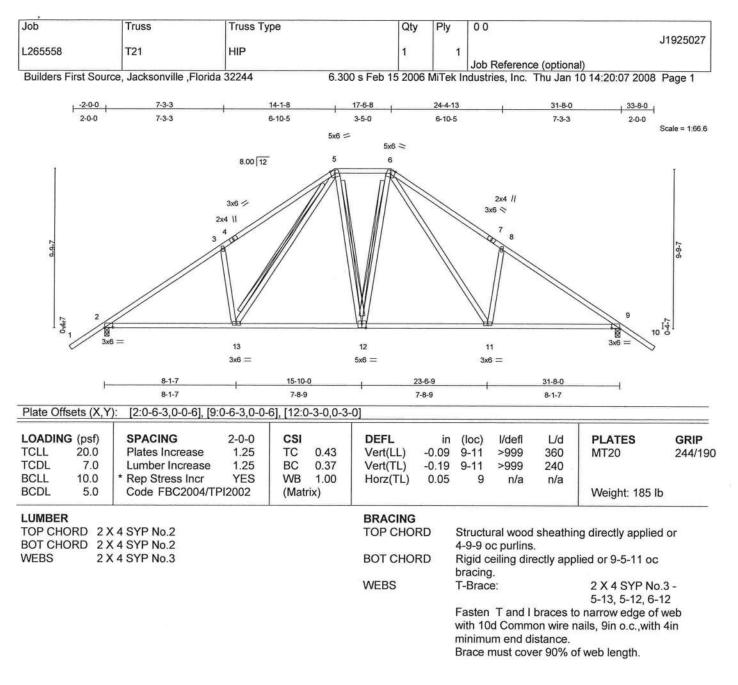
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 2 and 391 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 34888 1109 Ceastal Bay Blvd Boynton Beach, Ft. 38436





REACTIONS (lb/size) 2=1124/0-3-8, 9=1124/0-3-8

Max Horz 2=-311(load case 4)

Max Uplift 2=-405(load case 6), 9=-429(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1506/836, 3-4=-1435/1017, 4-5=-1415/1049, 5-6=-829/726,

6-7=-1413/1047, 7-8=-1433/1015, 8-9=-1505/836, 9-10=0/62

BOT CHORD 2-13=-444/1157, 12-13=-169/820, 11-12=-133/823, 9-11=-444/1157

3-13=-356/434, 5-13=-448/524, 5-12=-173/216, 6-12=-171/218, 6-11=-444/514,

8-11=-348/430

JOINT STRESS INDEX

WEBS

2 = 0.70, 3 = 0.33, 4 = 0.60, 5 = 0.67, 6 = 0.67, 7 = 0.60, 8 = 0.33, 9 = 0.70, 11 = 0.42, 12 = 0.57 and 13 = 0.42

Continued on page 2

January 10,2008



lius Lee 1988 Chesian Endineer Orda PE No. 114889 Og Caestel Bay Blvd Oscastel Bay Blvd

Job	Truss	Truss Type	Qty	Ply	00	
L265558	T21	HIP	1	1		J1925027
17,500	1850				Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 2 and 429 lb uplift at joint 9.

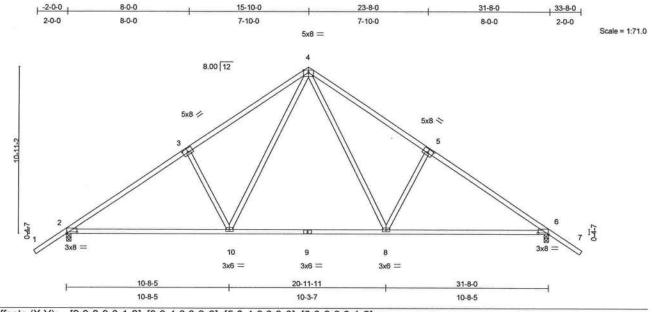
LOAD CASE(S) Standard

Julius Lee Truse Cesign Engineer Flonda PE No. 24869 1189 Cesstel Bay Blyd Boydon Best P. 19445



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T22	COMMON	2	1		J1925028
					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.25	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.49	6-8	>775	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 163 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-8-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 9-7-14 oc

bracing.

REACTIONS (lb/size) 2=1124/0-3-8, 6=1124/0-3-8

Max Horz 2=348(load case 5)

Max Uplift 2=-413(load case 6), 6=-413(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1474/821, 3-4=-1277/906, 4-5=-1277/906, 5-6=-1474/821, 6-7=0/62

BOT CHORD 2-10=-420/1127, 9-10=-93/753, 8-9=-93/753, 6-8=-420/1127

WEBS 3-10=-377/429, 4-10=-355/500, 4-8=-355/500, 5-8=-377/429

JOINT STRESS INDEX

2 = 0.73, 3 = 0.69, 4 = 0.72, 5 = 0.69, 6 = 0.73, 8 = 0.45, 9 = 0.38 and 10 = 0.45

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building oce. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T22	COMMON	2	1		J1925028
					Job Reference (optional)	

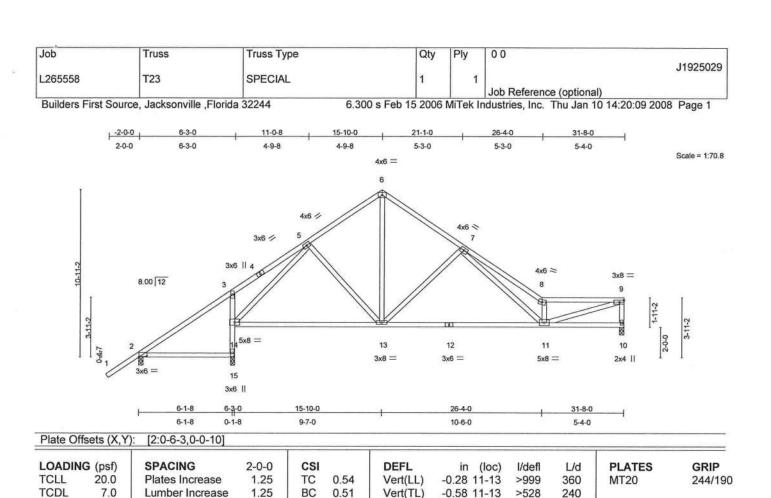
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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 413 lb uplift at joint 2 and 413 lb uplift at joint 6.

LOAD CASE(S) Standard





LUMBER

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

10.0

5.0

3-15 2 X 4 SYP No.3

Rep Stress Incr

Code FBC2004/TPI2002

WEBS

2 X 4 SYP No.3

BRACING

Horz(TL)

0.93

WB

(Matrix)

TOP CHORD

0.01

Structural wood sheathing directly applied or

Weight: 177 lb

3-7-11 oc purlins, except end verticals.

n/a

BOT CHORD Rigid ceiling directly applied or 5-3-13 oc

n/a

bracing.

10

REACTIONS

(lb/size) 10=799/0-3-8, 2=272/0-3-8, 15=1057/0-3-0

YES

Max Horz 2=375(load case 5)

Max Uplift 10=-262(load case 7), 2=-273(load case 4), 15=-472(load case 6) Max Grav 10=799(load case 1), 2=289(load case 10), 15=1057(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-274/319, 3-4=-144/260, 4-5=-123/280, 5-6=-752/568, 6-7=-766/566,

7-8=-2614/1637, 8-9=-1990/1175, 9-10=-762/478

BOT CHORD 2-15=-74/0, 14-15=-1029/653, 3-14=-298/348, 13-14=-289/551, 12-13=-554/975,

11-12=-554/975, 10-11=-97/136

WEBS 5-13=-65/146, 5-14=-926/450, 6-13=-392/480, 7-13=-558/499, 7-11=-997/1606,

8-11=-1659/1119, 9-11=-1133/1949

Julius Lee Trues Cesign Engineer Florida PE No. 34869 1100 Caastal Bay Blvd Boynton Beach, FL 33435

JOINT STRESS INDEX

2 = 0.60, 3 = 0.26, 4 = 0.15, 5 = 0.27, 6 = 0.52, 7 = 0.62, 8 = 0.80, 9 = 0.66, 10 = 0.73, 11 = 0.88, 12 = 0.59, 13 = 0.56, 14 = 0.63 and 15 = 0.44

NOTES

Unbalanced roof live loads have been considered for this design.
 Continued on page 2

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T23	SPECIAL	1	1		J1925029
					Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 262 lb uplift at joint 10, 273 lb uplift at joint 2 and 472 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda FE No. 34868 1100 Caastal Bay Blvd Boynton Besch, FL 23435



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T24	SPECIAL	1	1		J1925030
LLCOOOG	1.21	OI LOW LE			Job Reference (optional)	

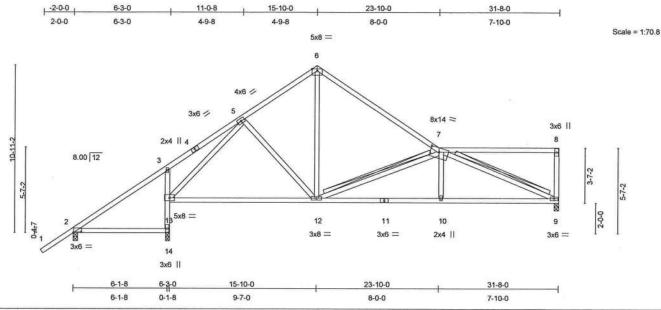


Plate Offsets (X,Y): [2:0-6-3,0-0-10], [7:0-7-0,0-3-6] LOADING (psf) **SPACING** 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 1.25 0.71 Plates Increase TC Vert(LL) -0.14 12-13 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.25 12-13 >999 240 BCLL 10.0 YES 0.77 Rep Stress Incr **WB** Horz(TL) 0.04 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 177 lb

LUMBER			
	 18.	40	
	IIIV	1	

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-14 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 5-7-0 oc

bracing. T-Brace:

WEBS

2 X 4 SYP No.3 -

7-12, 7-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 9=803/0-3-8, 2=288/0-3-8, 14=1038/0-3-0

Max Horz 2=373(load case 5)

Max Uplift 9=-266(load case 7), 2=-243(load case 4), 14=-491(load case 6)

Max Grav 9=803(load case 1), 2=296(load case 10), 14=1038(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-270/269, 3-4=-150/225, 4-5=-130/245, 5-6=-755/554, 6-7=-824/523,

7-8=-83/55, 8-9=-194/158

BOT CHORD 2-14=-74/0, 13-14=-1005/691, 3-13=-323/375, 12-13=-350/558, 11-12=-887/1396,

10-11=-887/1396, 9-10=-884/1398

WEBS 5-12=-66/134, 5-13=-853/466, 6-12=-274/436, 7-12=-878/655, 7-10=0/229,

7-9=-1431/912

Continued on page 2

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航 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T24	SPECIAL	1	1		J1925030
					Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.57, 3 = 0.57, 4 = 0.15, 5 = 0.27, 6 = 0.76, 7 = 0.40, 8 = 0.54, 9 = 0.53, 10 = 0.33, 11 = 0.53, 12 = 0.56, 13 = 0.74 and 14 = 0.48

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 9, 243 lb uplift at joint 2 and 491 lb uplift at joint 14.

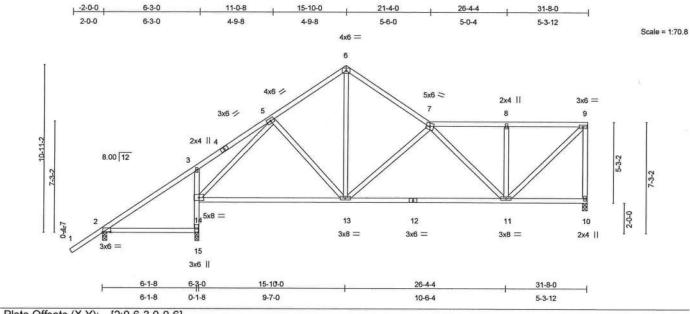
LOAD CASE(S) Standard

Julius Les Truss Design Engineer Phonda PE No. 34865 1 109 Coestel Bey Blvd



Job	Truss	Truss Type	Qty	Ply	00	West Control of the C
L265558	T25	SPECIAL	1	1		J1925031
					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.20	11-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.37	11-13	>815	240	7000 to 150	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 188 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-15 2 X 4 SYP No.3

2 X 4 SYP No.3

WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 5-9-0 oc

bracing.

REACTIONS (lb/size) 10=805/0-3-8, 2=298/0-3-8, 15=1025/0-3-0

Max Horz 2=398(load case 6)

Max Uplift 10=-272(load case 7), 2=-210(load case 4), 15=-512(load case 6)

Max Grav 10=805(load case 1), 2=302(load case 10), 15=1025(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-270/210, 3-4=-141/167, 4-5=-120/188, 5-6=-763/526, 6-7=-775/518,

7-8=-703/438, 8-9=-702/438, 9-10=-786/521

BOT CHORD 2-15=-82/0, 14-15=-990/735, 3-14=-316/373, 13-14=-412/573, 12-13=-662/996,

11-12=-662/996, 10-11=-17/19

5-13=-72/121, 6-13=-326/474, 7-13=-561/447, 5-14=-847/526, 7-11=-412/315, **WEBS**

8-11=-285/245, 9-11=-585/949

JOINT STRESS INDEX

2 = 0.66, 3 = 0.56, 4 = 0.15, 5 = 0.27, 6 = 0.59, 7 = 0.62, 8 = 0.33, 9 = 0.61, 10 = 0.37, 11 = 0.91, 12 = 0.54, 13 = 0.56, 14 = 0.560.63 and 15 = 0.51

1) Unbalanced roof live loads have been considered for this design. Continued on page 2

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0.0	
L265558	T25	SPECIAL	1	1		J1925031
					Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 10, 210 lb uplift at joint 2 and 512 lb uplift at joint 15.

LOAD CASE(S) Standard



	In	ssı	Truss Ty	ре			Qty	Ply	0 0			
.265558	T2	6	SPECIA	Ĺ			1	1				J1925032
									Job Referen	ce (optiona	1)	
Builders First	Source, Ja	cksonville ,Flo	rida 32244		6.30	0 s Feb 15	2006	MiTek In	dustries, Inc.	Thu Jan 1	0 14:20:12 2008	Page 1
	-2-0-0	6-3-0	11-0-8	-	15-10-0	18-10-0	1	25-1-4		31-8-0		
	2-0-0	6-3-0	4-9-8		4-9-8	3-0-0		6-3-4		6-6-12		
						4x6 =						Scale = 1:70.8
						6						
т.						•						
					/		4x6 <		2x4		3x6 =	
1				4x6 /	//				8		9	
11					//		A		B		厘	I

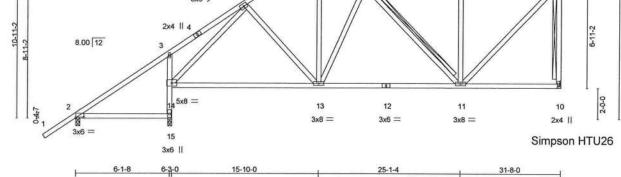


Plate Of	fsets (X,Y	(): [2:0-6-3,0-0-6]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.12	13-14	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.22	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.02	10	n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

	ΙB	

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-15 2 X 4 SYP No.3

2 X 4 SYP No.3

WEBS

BRACING

TOP CHORD

9-3-4

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

6-6-12

BOT CHORD Rigid ceiling directly applied or 5-9-1 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

9-10, 7-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 10=805/Mechanical, 2=299/0-3-8, 15=1023/0-3-0

Max Horz 2=438(load case 6)

6-1-8

0-1-8

Max Uplift 10=-331(load case 5), 2=-177(load case 4), 15=-530(load case 6) Max Grav 10=805(load case 1), 2=302(load case 10), 15=1023(load case 1)

9-7-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-323/153, 3-4=-134/109, 4-5=-114/129, 5-6=-762/495, 6-7=-733/524,

7-8=-621/414, 8-9=-620/415, 9-10=-774/549

2-15=-91/0, 14-15=-989/778, 3-14=-312/371, 13-14=-466/576, 12-13=-540/776, **BOT CHORD**

11-12=-540/776, 10-11=-14/20

WEBS 5-14=-848/588, 5-13=-74/123, 6-13=-382/553, 7-13=-498/382, 7-11=-226/183,

8-11=-381/338, 9-11=-576/863

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	No. 41 CO. 100
L265558	T26	SPECIAL	1	1		J1925032
	10252000			123	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:12 2008 Page 2

JOINT STRESS INDEX

2 = 0.64, 3 = 0.55, 4 = 0.15, 5 = 0.27, 6 = 0.37, 7 = 0.63, 8 = 0.33, 9 = 0.73, 10 = 0.47, 11 = 0.83, 12 = 0.42, 13 = 0.61, 14 = 0.69 and 15 = 0.54

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 10, 177 lb uplift at joint 2 and 530 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Plonda PE No. 24869 1 109 Coestal Bey Blvd 8 Ovnton Basch Et 24406



Job	Truss	Truss Ty	ре	Qty	Ply	0.0				14005000
L265558	T27	SPECIAL		1		1		- /KI	N	J1925033
Builders First Source	e Jacksonville Flori	ida 32244	6.30	00 s Feb 15 2006	3 MiTek			e (optional Thu Jan 1		Page 1
Danidoro i not obaro	0, 040,001,1110,1110,11	du oll i i	0.00	00 01 00 10 2000	J 1811 1 CI	· maasin	20, 1110.	rna ban r	0 14.20.10 2000	, age i
-2-0-0	6-3-0	10-9-8	15-4-0	22-0-14		26-7	7-6	31-8-0		
2-0-0	6-3-0	4-6-8	4-6-8	6-8-13		4-6	-8	5-0-10		
				4x6 =	4	x12 =				Scale = 1:68.7
044.7 10-7.22	2x 00 12 3	3x6 = 5		13	12	7	4x	6 % 8	9 10 8c.	10.7.2
	s = 11			3x8 =	3x6 = 3	x6 =			3x8 =	1
- SXI									Simpson H7	U26
	3x6	3								
H	6-1-8 6-3	100	15-4-0	22-0-14		+	31-			
	6-1-8 0-1	-8	9-1-0	6-8-13			9-7	-2		
Plate Offsets (X,Y)	[2:0-6-3,0-0-6]									
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/	YES	CSI TC 0.43 BC 0.49 WB 0.68 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.15 -0.27 0.03		I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 193	GRIP 244/190
WEBS 2 X		•		BRACING TOP CHOR BOT CHOR WEBS	:D	6-0-0 oc Rigid ce bracing. T-Brace Fasten with 10d minimum	purlins iling din T and I Comm	braces to on wire na istance.	g directly applied and verticals. Sed or 5-9-9 oc 2 X 4 SYP N 7-13 narrow edge cails, 9in o.c., with web length.	o.3 - of web

REACTIONS (lb/size) 2=304/0-3-8, 15=1018/0-3-0, 10=806/Mechanical

Max Horz 2=364(load case 5)

Max Uplift 2=-266(load case 4), 15=-495(load case 5), 10=-222(load case 7) Max Grav 2=304(load case 10), 15=1018(load case 1), 10=806(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-255/310, 3-4=-116/264, 4-5=-104/283, 5-6=-766/593, 6-7=-591/572,

7-8=-781/598, 8-9=-197/153, 9-10=-204/181

BOT CHORD 2-15=-81/4, 14-15=-982/643, 3-14=-319/354, 13-14=-329/565, 12-13=-246/602,

11-12=-246/602, 10-11=-349/611

WEBS 5-14=-819/427, 5-13=-43/125, 6-13=-51/184, 7-13=-118/108, 7-11=-63/205,

8-11=-103/154, 8-10=-751/460

Julius Leer Truss Design Engineer Florida FE No. 34869 1 100 Ceastal Bay Blyd Boynton Beach, FL 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	0 0	97025 12 12
L265558	T27	SPECIAL	1	1		J1925033
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:13 2008 Page 2

JOINT STRESS INDEX

2 = 0.66, 3 = 0.68, 4 = 0.15, 5 = 0.28, 6 = 0.72, 7 = 0.90, 8 = 0.27, 9 = 0.34, 10 = 0.70, 11 = 0.37, 12 = 0.23, 13 = 0.56, 14 = 0.73 and 15 = 0.48

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 2, 495 lb uplift at joint 15 and 222 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida FE No. 34869 1109 Ceastal Bay Blyd Boynton Beach, FL 03436



Job	Truss	3	Truss Type		Qty	Ply	00	14005004
.265558	T28		SPECIAL		1	1		J1925034
Builders First S	Source, Jack	sonville ,Flor	ida 32244	6.300 s F	eb 15 2006	MiTek Ir	Job Reference (optional) ndustries, Inc. Thu Jan 10 14:20:14 200	8 Page 1
								5. 5.55 9 .5 5.
	-2-0-0	6-3-0	12-0-8	17-10-0	19-6-14	25-5-11		
	2-0-0	6-3-0	5-9-8	5-9-8	1-8-13	5-10-13	3 6-2-5	Scale = 1:75.
					4x6 = 4x6 =			
					6 7			
1					A		Ţ	
							1	
			4x6	"//				
			5/28	_			8	
			4					
3-2		0 12	2x4		11 11	A	5x6 <> 0	\$

Plate Offsets (X,Y): [2:0-6-3,0-0-10], [9:0-3-0,0-1-8]

6-1-8

16

6-3-0

0-1-8

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.32	14-15	>948	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.57	14-15	>536	240	N. Martines	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)					1.50.00	Weight: 198 lb	

17-10-0

11-7-0

13 12

25-5-11

5-10-13

19-6-14

1-8-13

11

LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 *Except* 6-0-0 oc purlins, except end verticals. 3-16 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 5-8-9 oc **WEBS** 2 X 4 SYP No.3 *Except* bracing. 9-10 2 X 4 SYP No.2 **WEBS** T-Brace: 2 X 4 SYP No.3 -5-15, 5-14, 8-13

> Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

2x4 ||

31-8-0

6-2-5

Simpson HTU26

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=298/0-3-8, 16=1026/0-3-0, 10=805/Mechanical

Max Horz 2=419(load case 5)

Max Uplift 2=-319(load case 4), 16=-488(load case 6), 10=-238(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/62, 2-3=-329/404, 3-4=-183/332, 4-5=-158/356, 5-6=-721/573, 6-7=-524/566, TOP CHORD

7-8=-721/579, 8-9=-865/533, 9-10=-769/492

2-16=-93/3, 15-16=-990/624, 3-15=-356/392, 14-15=-333/595, 13-14=-153/524,

12-13=-333/647, 11-12=-333/647, 10-11=-83/95

5-15=-797/377, 5-14=-146/256, 6-14=-125/268, 7-13=-178/135, 8-13=-232/257,

8-11=-129/125, 9-11=-263/581

JOINT STRESS INDEX

BOT CHORD

WEBS

2 = 0.57, 3 = 0.38, 4 = 0.22, 5 = 0.26, 6 = 0.67, 7 = 0.52, 8 = 0.41, 9 = 0.82, 10 = 0.83, 11 = 0.34, 12 = 0.30, 13 = 0.36, 14 = January 10,2008 Conthued 55 page 22 and 16 = 0.48

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI/TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T28	SPECIAL	1	1		J1925034
	= 10000		100	1 2	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:14 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 3x6 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 2, 488 lb uplift at joint 16 and 238 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24805 1100 Coestel Bay Blyd Boyston Beach FL 22436



Job		Truss		Truss	- Vpe		Qt	tv	Ply	00				
000		11000		11455	ypo		1 ~	.,	1.19	00				J1925035
L265558		T29		SPECI	AL		2		1					
D 111 - 1	-:	1	10 - F1 - 1 - 1	00044		0.00	00 - 5-1-45-00	100 1	ATT 1 1			e (option		0.0
Builders I	First Sourc	e, Jackson	nville ,Florida	32244		6.30	00 s Feb 15 20)U6 I	vii i ek in	austr	es, Inc.	i nu Jan	10 14:20:15 200	8 Page 1
		-2-0-0	6-3-0	4	12-5-12		18-8-7		24-11-	3	-	31-8-0		
	F	2-0-0	6-3-0		6-2-12	1	6-2-12		6-2-12		7.	6-8-13		
								x6 =						Scale = 1:74.3
	12:10:1	2		14 15 6-3-0	12-5-12	13		12 x8 =	11 24-11-	3	10	31-8-0	6x8 = 8 9577 0077 Simpson HTU2	6
		(8)	6-1-8	0-1-8	6-2-12	to the	6-2-12	020	6-2-12			6-8-13		
Plate Offs	sets (X,Y)	: [2:0-6-	3,0-0-10], [8	3:0-3-8,E	dge]									
LOADING TCLL TCDL	G (psf) 20.0 7.0		NG Increase r Increase	2-0-0 1.25 1.25	CSI TC BC	0.35 0.46	DEFL Vert(LL) Vert(TL)		in (0.07 2 0.06 12	loc) -15 -13	I/defl >999 >999	L/d 360 240	PLATES MT20	GRIP 244/19

	R IORD 2		BRACING TOP CHO					ng directly applied	or			
BCDL	5.0	Code FBC2004/TF		(Mat		11012(11)	0.02	9	IIIa	11/a	Weight: 194 lb	
TCDL BCLL	7.0 10.0	* Rep Stress Incr	1.25 YES	BC WB	0.46 0.68	Vert(TL) Horz(TL)	-0.06 0.02	12-13	>999 n/a	240 n/a		
IOLL	20.0	r lates increase	1.25	10	0.55	VEITLE	0.07	2-13	- 333	300	101120	244/130

3-15 2 X 4 SYP No.3 2 X 4 SYP No.3 *Except* WEBS 8-9 2 X 4 SYP No.2

BOT CHORD

6-0-0 oc purins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 5-9-13 oc bracing: 14-15.

T-Brace:

WEBS

2 X 4 SYP No.3 -5-12, 7-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=306/0-3-8, 15=1015/0-3-0, 9=807/Mechanical

Max Horz 2=437(load case 5)

Max Uplift 2=-314(load case 4), 15=-507(load case 6), 9=-239(load case 7) Max Grav 2=308(load case 10), 15=1015(load case 1), 9=807(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-353/391, 3-4=-845/507, 4-5=-741/522, 5-6=-698/567, 6-7=-699/568,

7-8=-878/529, 8-9=-769/487

BOT CHORD 2-15=-75/21, 14-15=-980/638, 3-14=-947/656, 13-14=-80/48, 12-13=-303/622,

11-12=-316/650, 10-11=-316/650, 9-10=-105/115

3-13=-235/621, 5-13=-136/114, 5-12=-220/256, 6-12=-346/375, 7-12=-255/275,

7-10=-103/111, 8-10=-220/559

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T29	SPECIAL	2	1		J1925035
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:15 2008 Page 2

JOINT STRESS INDEX

2 = 0.59, 3 = 0.41, 4 = 0.38, 5 = 0.41, 6 = 0.73, 7 = 0.41, 8 = 0.49, 9 = 0.29, 10 = 0.34, 11 = 0.22, 12 = 0.56, 13 = 0.34, 14 = 0.32 and 15 = 0.47

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2, 507 lb uplift at joint 15 and 239 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34869 1100 Coesial Bay Blvd Boyston Beach Et 33435



Job	Truss		Truss Type		Qty	Ply	00		14005000
_265558	Т30		SPECIAL		1	1	Job Reference	co (optional)	J1925036
Builders First S	Source, Jacks	onville ,Flori	da 32244	6.300 s Feb 15	5 2006	MiTek Ir	dustries, Inc.	Thu Jan 10 14:20:1	6 2008 Page 1
	-2-0-0	6-3-0	12-5-12	18-8-7	-	24-11-3		31-8-0	
	2-0-0	6-3-0	6-2-12	6-2-12	4x6 =	6-2-12		6-8-13	Scale = 1:74.
					6				
			8.00	2	1				
			5			270	7		
Z Z			4						
12-10			/30					6x8 =	

6-1-8 0-1-8 6-2-12

Plate Offsets (X,Y): [2:0-6-3,0-0-10], [8:0-3-8,Edge]

15 6-3-0

12-5-12

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.07	2-15	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.06	12-13	>999	240	1377112.07572	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)	interment water					Weight: 194 lb	

18-8-7

6-2-12

13

LUMBER TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-15 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 *Except*

8-9 2 X 4 SYP No.2

BRACING

12

3x8 =

24-11-3

6-2-12

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

31-8-0

6-8-13

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

10

bracing, Except:

5-9-13 oc bracing: 14-15.

WEBS T-Brace:

5-12, 7-12

2 X 4 SYP No.3 -

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=306/0-3-8, 15=1015/0-3-0, 9=807/1-8-0

Max Horz 2=437(load case 5)

Max Uplift 2=-314(load case 4), 15=-507(load case 6), 9=-239(load case 7)

Max Grav 2=308(load case 10), 15=1015(load case 1), 9=807(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-353/391, 3-4=-845/507, 4-5=-741/522, 5-6=-698/566, 6-7=-699/568,

7-8=-878/529, 8-9=-769/487

BOT CHORD 2-15=-75/21, 14-15=-980/638, 3-14=-947/656, 13-14=-80/48, 12-13=-303/622,

11-12=-316/650, 10-11=-316/650, 9-10=-105/115

WEBS 3-13=-235/621, 5-13=-136/114, 5-12=-220/256, 6-12=-346/375, 7-12=-255/275,

7-10=-103/111, 8-10=-220/559

Julius Lee Trues Design Engineer Florida PE No. 34888 1 109 Ceasial Bay Blyd Boynton Beach, Ft. 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265558	Т30	SPECIAL	1	1		J1925036
11 1000 0000 000 000 000	1000000				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:16 2008 Page 2

JOINT STRESS INDEX

2 = 0.58, 3 = 0.41, 4 = 0.38, 5 = 0.41, 6 = 0.73, 7 = 0.41, 8 = 0.49, 9 = 0.29, 10 = 0.34, 11 = 0.22, 12 = 0.56, 13 = 0.34, 14 = 0.32 and 15 = 0.47

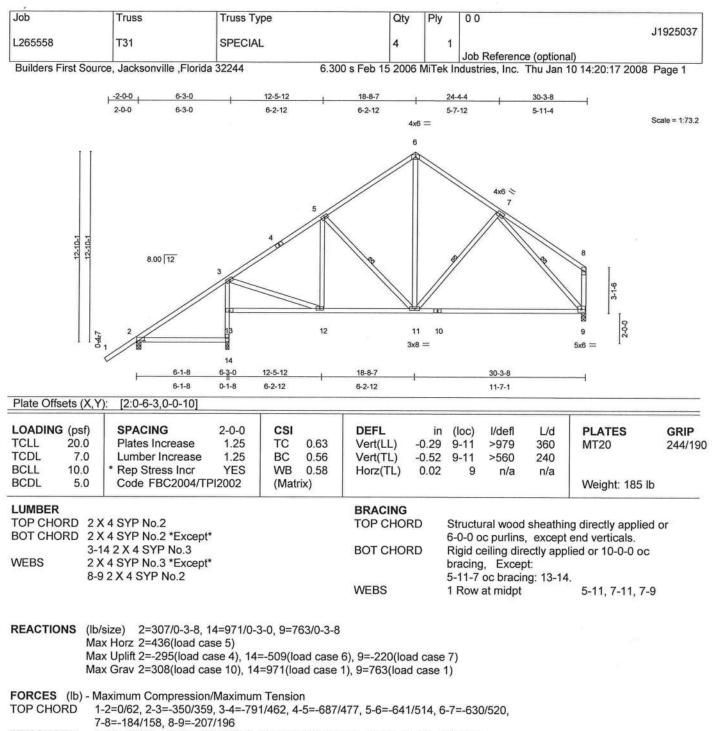
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2, 507 lb uplift at joint 15 and 239 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida FE No. 34869 1109 Coestal Bay Blvd Boynton Beach, FL 33435





BOT CHORD 2-14=-82/18, 13-14=-936/638, 3-13=-900/660, 12-13=-88/52, 11-12=-314/577,

10-11=-271/506, 9-10=-271/506

WEBS 3-12=-238/571, 5-12=-132/103, 5-11=-216/268, 6-11=-299/348, 7-11=-139/223,

7-9=-651/376

Truss Design Engineer Florida FE No. 24869 1100 Caastal Bay Blvd Boynton Beach, FL 33436

JOINT STRESS INDEX

2 = 0.59, 3 = 0.41, 4 = 0.37, 5 = 0.41, 6 = 0.68, 7 = 0.29, 8 = 0.41, 9 = 0.66, 10 = 0.19, 11 = 0.56, 12 = 0.34, 13 = 0.39 and 14 = 0.48

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T31	SPECIAL	4	1		J1925037
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:17 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are 3x6 MT20 unless otherwise indicated.

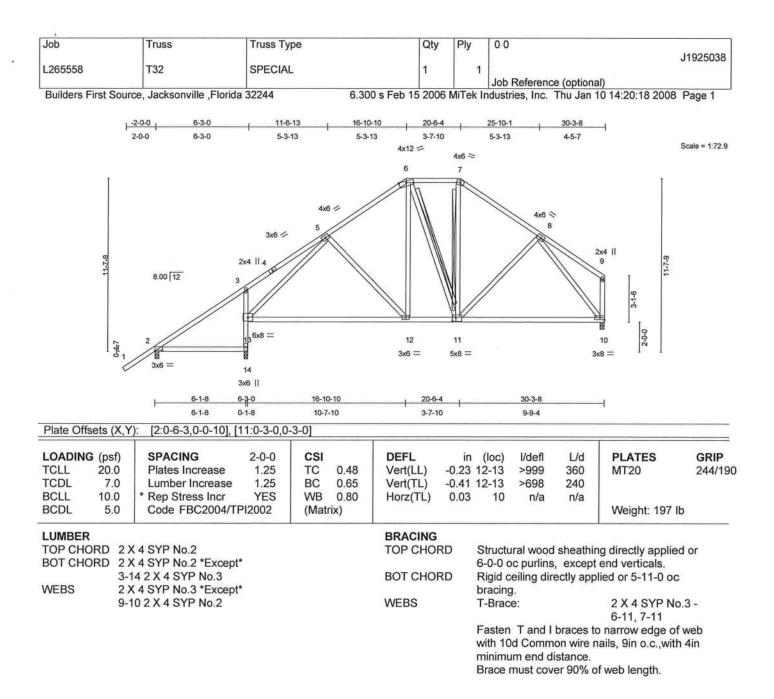
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2, 509 lb uplift at joint 14 and 220 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24869 I 100 Ceastal Bay Blvd Bovoton Beach, FL 22435





REACTIONS (lb/size) 2=302/0-3-8, 14=977/0-3-0, 10=762/0-3-8

Max Horz 2=396(load case 5)

Max Uplift 2=-272(load case 4), 14=-480(load case 6), 10=-212(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

Maximum Compression/Maximum Tension
1-2=0/62, 2-3=-286/320, 3-4=-143/254, 4-5=-121/276, 5-6=-681/534, 6-7=-487/516, TOP CHORD

BOT CHORD 2-14=-92/0, 13-14=-941/633, 3-13=-337/374, 12-13=-342/548, 11-12=-191/498,

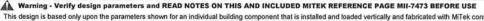
10-11=-279/475

WEBS 5-13=-749/405, 5-12=-96/219, 6-12=-116/193, 6-11=-136/147, 7-11=-82/158,

8-11=-84/143, 8-10=-714/466

JOINT STRESS INDEX

2 = 0.56, 3 = 0.54, 4 = 0.15, 5 = 0.27, 6 = 0.67, 7 = 0.51, 8 = 0.31, 9 = 0.61, 10 = 0.70, 11 = 0.70, 12 = 0.36, 13 = 0.75 and 14 January 10,2008 Continuedon page 2



Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265558	T32	SPECIAL	1	1		J1925038
220000	1,02	or Low L		***	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:18 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 2, 480 lb uplift at joint 14 and 212 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 24869 1109 Coastel Bay Blvd.

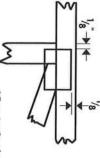


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless Dimensions are in inches. Apply plates to both sides of truss and securely seat dimensions indicate otherwise



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates

PLATE SIZE

4 × 4

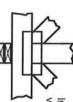
dimension is the length paralle perpendicular to slots. Second The first dimension is the width

LATERAL BRACING



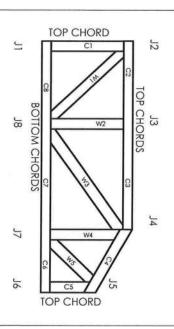
Indicates location of required continuous lateral bracing.

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

IC80

BOCA

96-31, 96-67

SBCCI

3907, 4922

9667, 9432A

WISC/DILHR 960022-W, 970036-N

561



MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

1. Provide copies of this truss design to the Failure to Follow Could Cause Property Damage or Personal Injury

N Cut members to bear tightly against each other.

building designer, erection supervisor, property owner and all other interested parties.

- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)

4

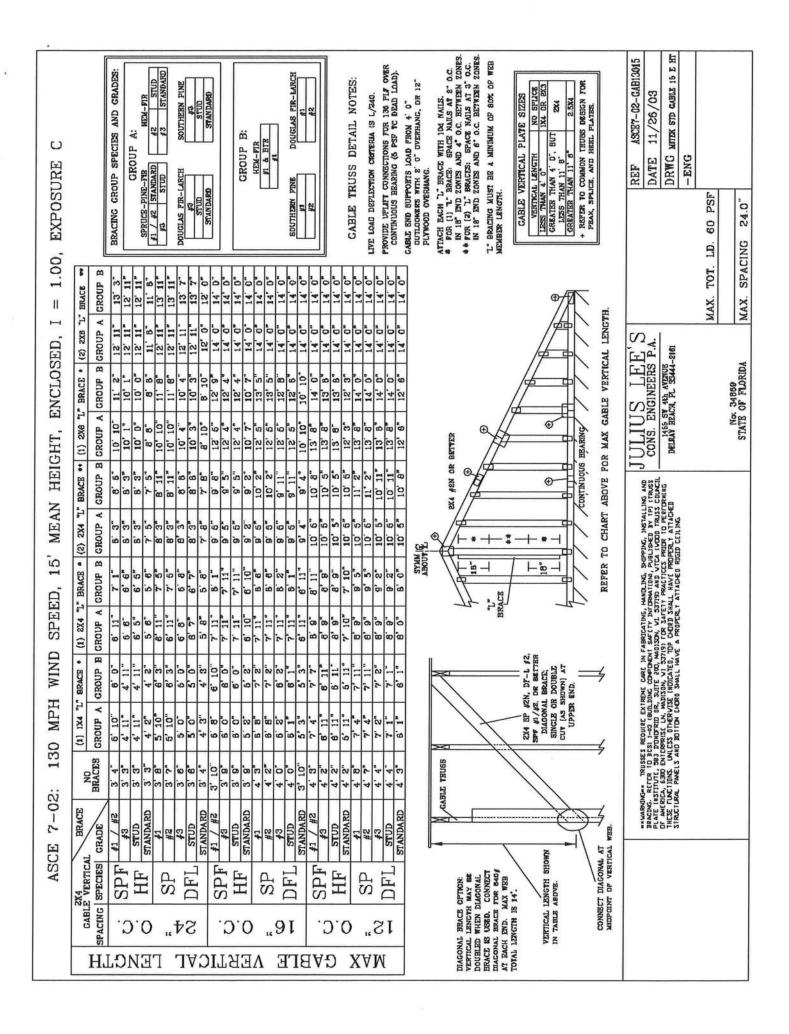
Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

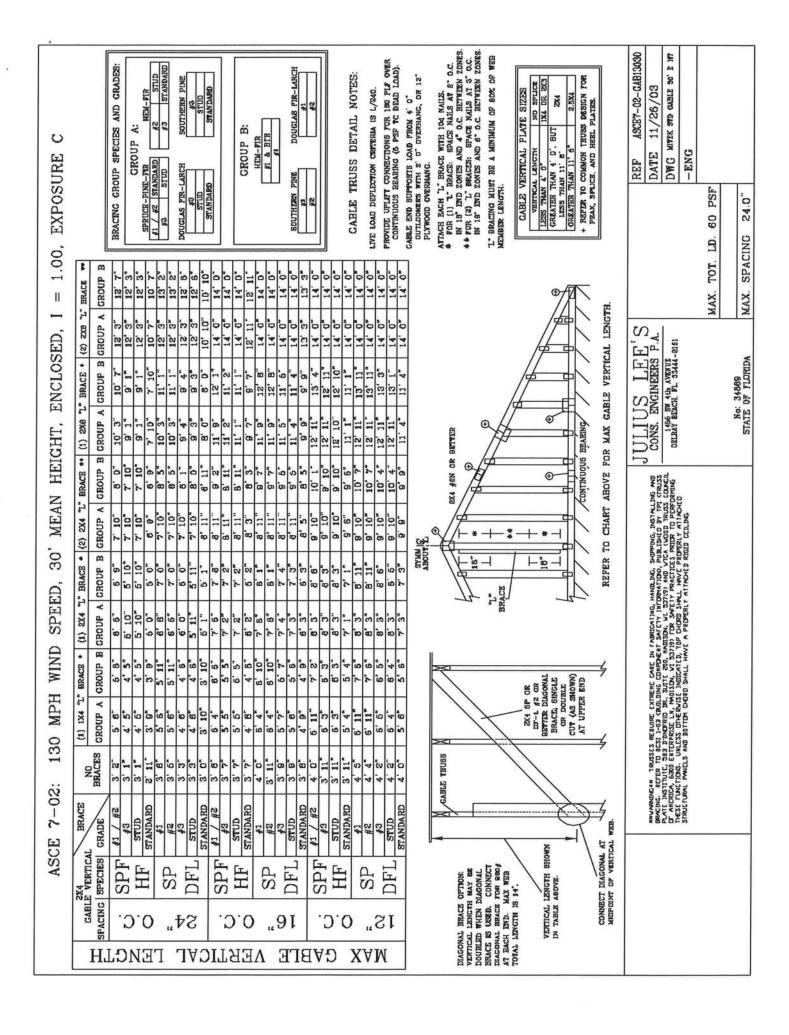
S

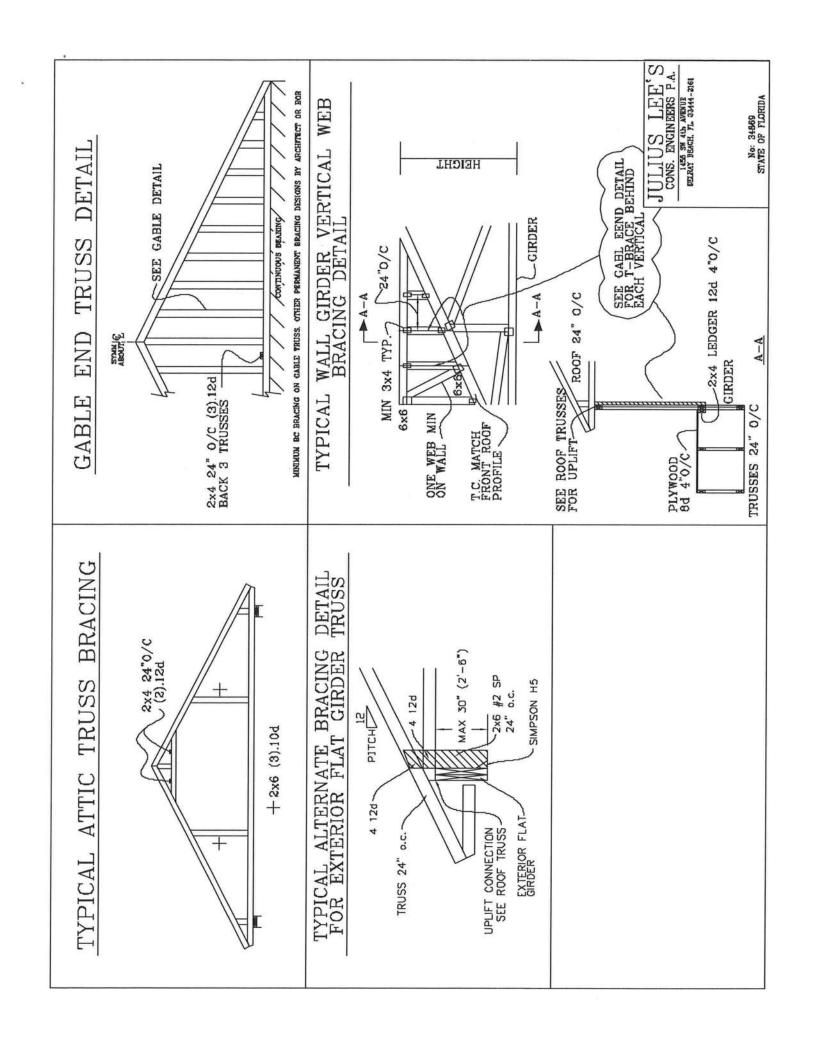
Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.

6

- 7. is the responsibility of truss fabricator. General Camber is a non-structural consideration and practice is to camber for dead load deflection.
- 8 shown indicate minimum plating requirements Plate type, size and location dimensions
- 9 Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Bottom chords require lateral bracing at 10 unless otherwise noted ft. spacing, or less, if no ceiling is installed
- Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials
- Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling erection and installation of trusses.
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ななな 2X 2X 4 CHORD TOP

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITIED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FIAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

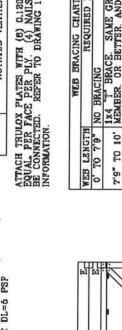
REFER TO ENGINEER'S SEALED DESIGN POR REQUIRED PURLIN SPACING.

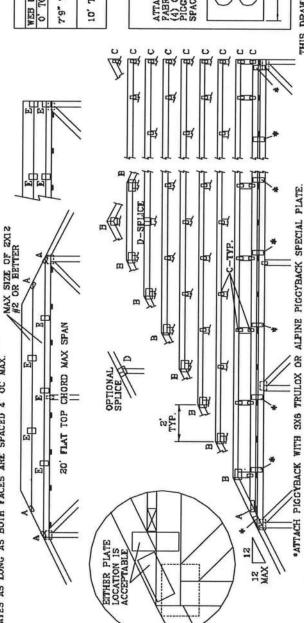
THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS: 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN BGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E.*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4. OC MAX.





EZ		SPANS	SPANS UP TO	
TYPE	30,	.45	38	.29
25 80	2X4	2.5X4	2.5X4	3X5
В	4X8	9X9	9X9	5%6
192000	1.5X3	1.5X4	1.5X4	1.5X4
238	5X4	9X9	9X2	5X6
3	4X8 01	ROTATED	4X8 OR 3X6 TRULOX AT A	4' 0C,

ATTACH TRULOX PLATES WITH (8) 0.120° X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN BACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

		WEB	WEB OC.		102
REQUIRED BRACING	NO BRACING	1x4 "T" BRACE. SAME GRADE, SPECIES AS V MEMBER. OR BETTER, AND 80% LENGTH OF V MEMBER. ATTACH WITH 8d NAILS AT 4" OC.	Zx4 "T" BRACE. SAME GRADE, SPECIES AS MEMBER. OR BETTER. AND 80% LENGTH OF MEMBER. ATTACH WITH 164 NAILS AT 4" O	* PICCYBACK SPECIAL PLATE TH TO THE PICCYBACK AT THE TIME OF ATTACH TO SUPPORTING TRUSS WITH (1.375" NAILS PER FACE PER PLY. APPLY SPECIAL PLATE TO EACH TRUSS FACE AND OR LESS.	
WEB LENGTH	o, TO 7'9"	7'9" TO 10'	10, TO 14'	ATTACH TEET PABRICATION (4) 0.120° X PICGYBACK S SPACE 4° OC	00

PIGGY THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045 PIGGYBACK STD 09/12/07 DRWGMITEK -ENG JL DATE REF 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC AT FAC. MAX LOADING 24.0" 47 PSF 1.15 DUR. SPACING ULIUS LEE'S CONS. ENGINEERS P.A. DELRAY BEACH, TL. 33444-2161 No: 34869 STATE OF PLORIDA

5

WAVARHINGW TRUSSES REQUIRE CYTROFE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BALCING, REFER TO EXST I-DD GUILLING SAFETS IN PROBABILITY, PLEISAND, PLEISAND STREAM OF A PRETICE. BAS D'ONGINED DE, SUIT C'ENCETTY PRACTICES PRINT TRUSS COUNCIL FOR A PROFICE AND MANAGENER LIN HANDLES WITH A PROFILE PRINT TO FARE DRAWS THE SE FUNCTIONS. UNLESS CHIECKLIST DIRECTED TO FACED SHALL HANCE PROPERLY ATTACHED.

8 1/4

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. WEBS 2X4 SP #3 OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
 (2) 16d BOX (0.135" X 3.5") NAILS TOE—NAILED FOR
 FBC 2004 110 MPH, ASCE 7—02 110 MPH WIND OR (3) 16d FOR
 ASCE 7—02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
 BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN

W2X4

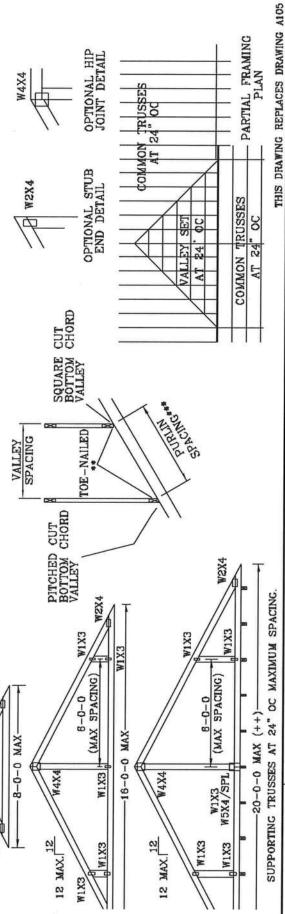
W2X4

IS MAX.

W2X4

4-0-0 MAX

CUT FROM 2X6 OR LARGER AS REQ'D



	BC II. 0 0 PSF -ENG	TRUSSES REGURE EXTREME CAME IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND TRUSSES REGURE EXTREME THE RESET BATCH RELIEVED BY THE PRESENCE OF SHIPPING. INSTALLING AND THE SECOND TRUSS COUNCIL.	ULIUS LEE'S TC LL CONS. ENGINEERS P.A. TC DL	TC LL TC DL BC DI.	20 20 20 7 15 15 5 5 5 5	20 P	PSF REF PSF DATE	REFLACES REF VA DATE 11	20 20 PSF REF VALLEY DET. 7 15 PSF DATE 11/26/03 5 5 PSF DRWG VATTRIGGET
TOT. LD. 32 40 PSF			No: 34869	DUR.FAC. 1.	.5	1.25			
	DUR.FAC. 1.25		STATE OF FLORIDA	SPACING		24"			

FAIL

103

DETAIL TOE-NAIL

TOE-NAILS TO BE DRIVEN AT AN ANCLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

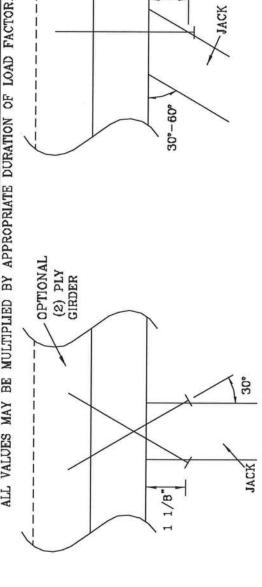
PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE, PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

SOUTHE	SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH	HEM-FIR	-FIR	SPRUCE	SPRUCE PINE FIR
PLY	2 PLIES 1 PLY	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
187#	256#	181#	234#	156#	203#	154#	188#
#962	383#	271#	351#	234#	304#	230#	#86Z
394#	1119	361#	468#	312#	406#	307#	#268
483#	#689	452#	982#	390#	207#	384#	496#



OPTIONAL (2) PLY GIRDER		CONDITION
0 3 8	1 1/8"	ALTERNATIVE CONDITION
 		→ JACK
	3060°	\

THIS DRAWING REPLACES DRAWING 784040

CONS	DELRA	
VARNING TRESSES REDUINE EXTREME CARE IN FARRICATING, HANDLING, SKIPPING, INSTALLING AND BRACING, REFER TO BEST 1-43 CRUILING CHIPPINE BY SAFETY (MTDRAATIDG), PUBLISHED BY TRY FRENCE	PLATE INSTITUTE, 303 PUNCPRID DR, SUTTE 200, MADISON, NI. 337193 AND VTCA (WIDD TRUSS COLNCIL OF ANERICA, 6300 ENTERPRISE LIA, MADISON, VI 357193 FTR SARETERS RAIDE TO PERDORMING THESE TRACTIONS. UNLESS OTHER WISE INDICATED, 170 CHORD SHALL HAVE PREPERTY ATTACHED STRUCTINAL PANELS AND BOTTON CHORD SHALL HAVE R PERPERTY ATTACHED STRUCTINAL RAIDES.	

	S'AA, SULIUS	S TC LL	PSF	REF	TOE-NAIL
S S S	CONS. ENGINEERS P.A.	TC DL	PSF	DATE	70/21/60
N C	DELRAY BEACH, FL. 33444-2161	BC DL	PSF	DRWG	CNTONAIL:103
		BC LL	PSF	-ENG	JL
		TOT. LD.	PSF		
	No: 34869	DUR. FAC. 1	00.1		
	STATE OF FLORIDA	SPACING			

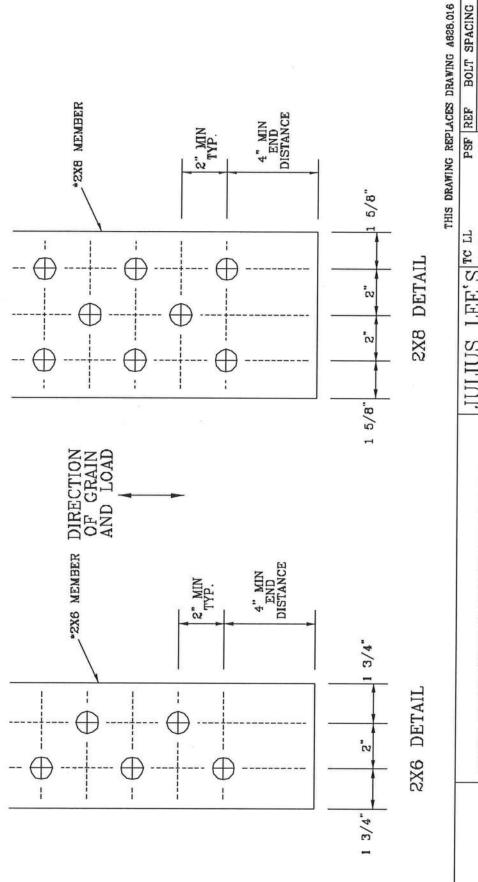
TO GRAIN. SPACING FOR LOAD APPLIED PARALLEL BOLT DIAMETER ,2,

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



	1111 11 12 1 12 12 12 13 14 15 15 15 15 15 15 15 15 15 15 15 15 15	TC LL	PSF	REF	BOLT SPA
AND	CONS. ENGINEERS P.A.	TC DL	PSF	DATE	11/26/03
NC C	DELRAY BEACH, FL. 33444-2161	BC DL	PSF	DRWG	CNBOLTSI
		BC IL	PSF	-ENG	J.F
		TOT. LD.	PSF		
		DUR. FAC.			
	STATE OF FLORIDA	SPACING			

P1103

TRULOX CONNECTION DETAIL

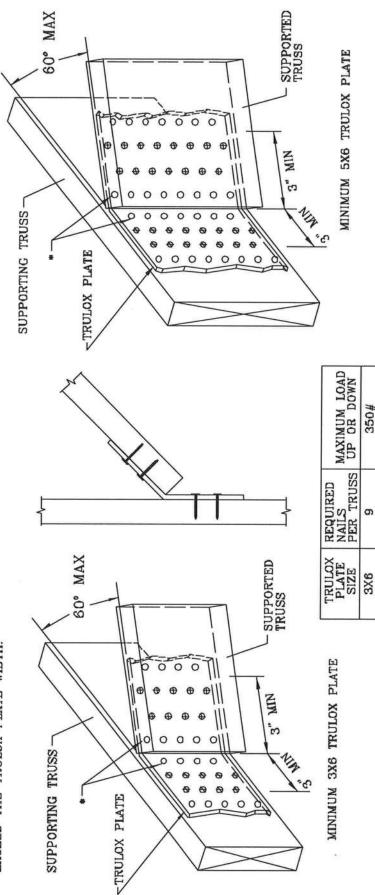
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (4).

NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



S JULIUS LEE'S CONS. ENGINEERS P.A. 1455 SW 4th AVENUE DELRAY BEACH, FL. 33444-2151

No: 34869 STATE OF FLORIDA

THIS DRAWING REPLACES DRAWINGS 1,158,989 I,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

#066

15

6X6

CNTRULOX1103

DRWG -ENG

H

11/26/03 TRULOX

DATE REF

WARNING* TRUSKS RCOURC EXTRONG CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 3633 1-60 (80LIDING COMPOUND SAFETY PROBACTION, PALLISED BY THI TRUSS PLATE INSTITUTI, BOD TOWING THE OW, MADISON, VI. 337199 AND VICA NOOD TRUSS COUNCIL OF AREINE, GATO MOTRERRISE LU, MADISON, VI. 337199 FER SAFETY PRACTICES RRIDE TO PERTORNIC THESE TAXIOLISS DIRECTION OF THE SAFETY PRACTICES RRIDE TO PERTORNIC THE STALL HAVE PROPERLY ATTACHED SHALL HAVE PROPERLY ATTACHED SHALL HAVE PROPERLY ATTACHED CELLING.

