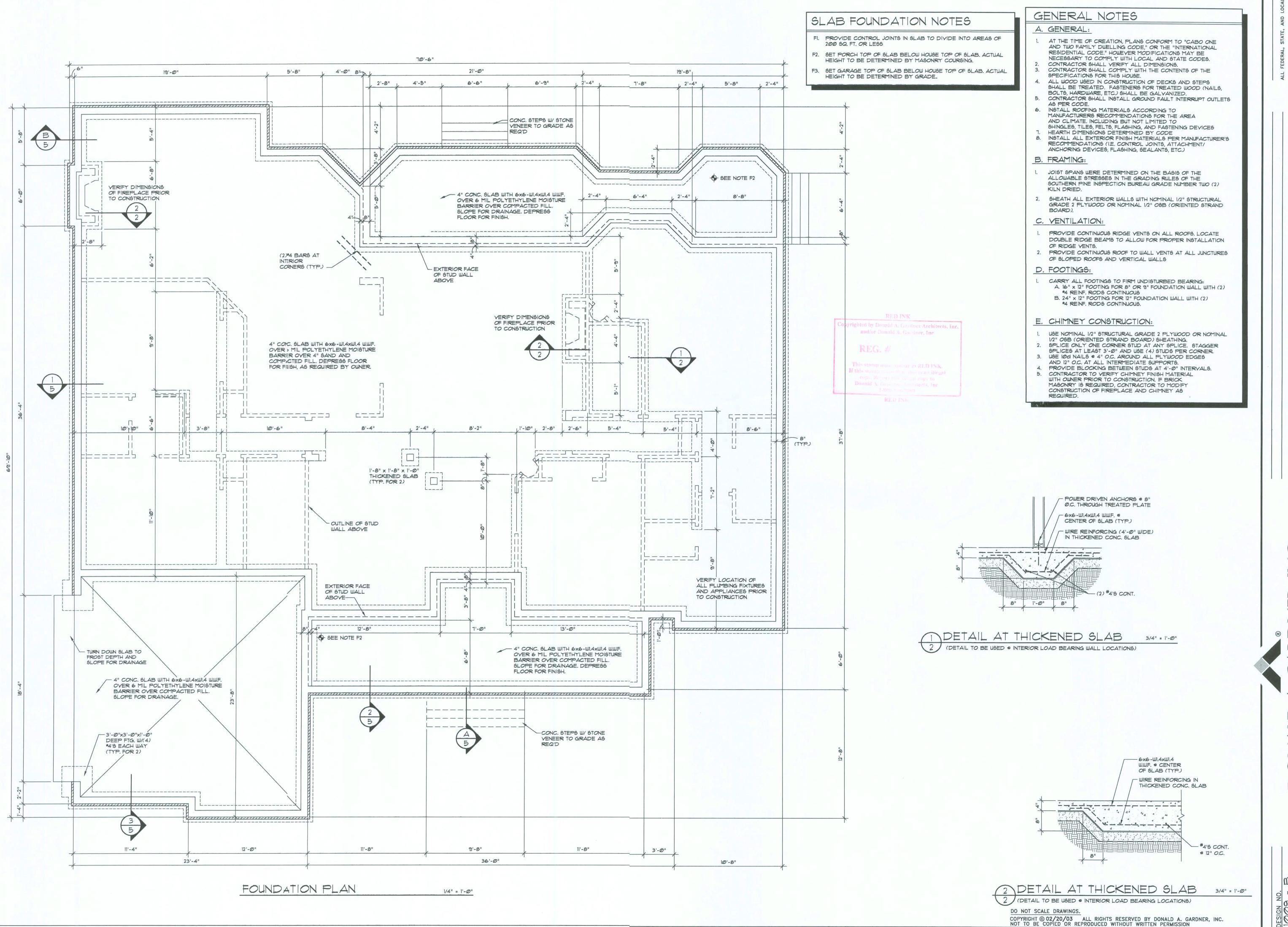


Country Lakes@ Woodborough Slake of 18,



FEDERAL, STATE, AND LOCAL
SS, ORDINANCES, REGULATIONS
SHALL BE CONSIDERED AS PART
SPECIFICATIONS FOR THIS BUILDING
SHALL TAKE PREFERENCE OVER
THING SHOWN, DESCRIBED, OR

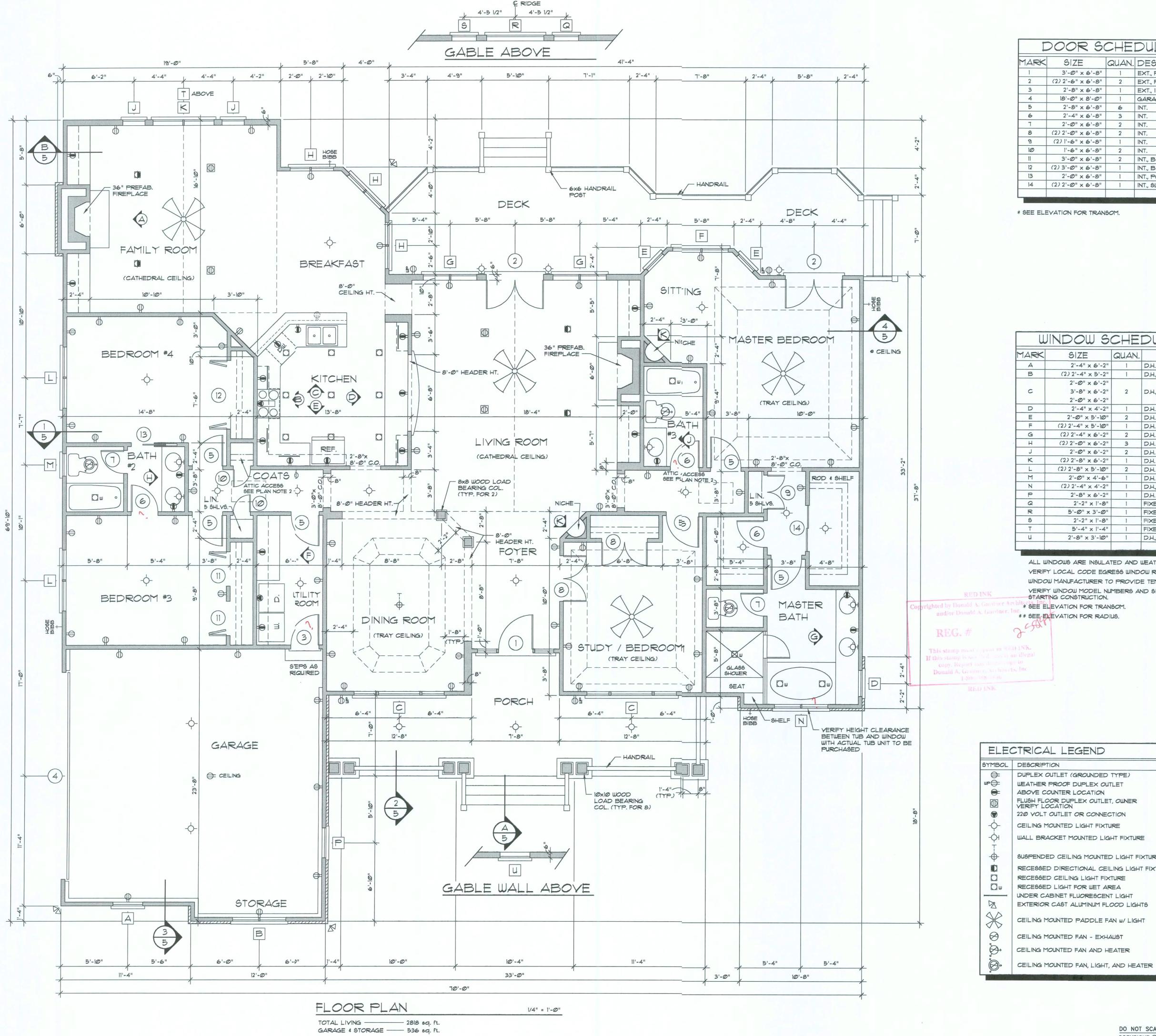
VALDA. GARDNER, INC.

1 TECTS . PLANNERS

28178 . GREENVILLE S.C. . 28618

OF DR Z

SIGN NO.



DOOR SCHEDULE SYMBOL(MARK SIZE QUAN DESCRIPTION 3'-0" x 6'-8" | 1 | EXT., FULL GLASS W/ 20" SIDELITES \$ 10" ARCH HEAD TRANSOM (2) 2'-6" x 6'-8" 2 EXT., FULL GLASS W/ 12" TRANSOM 2'-8" x 6'-8" | 1 | EXT., 1 HOUR FIRE-RATED 18'-0" x 8'-0" | 1 GARAGE 2'-8" x 6'-8" 6 INT. 2'-4" x 6'-8" 3 INT. 2'-0" x 6'-8" 2 INT. 8 (2)2'-0" x 6'-8" 2 INT. 9 (2)1'-6" x 6'-8" 1 INT. 1'-6" x 6'-8" 2 INT. 3'-0" x 6'-8" 2 INT., BIFOLD 12 (2) 3'-0" x 6'-8" | I INT., BIFOLD 13 2'-0" x 6'-8" | I | INT., POCKET DOOR 14 (2) 2'-0" x 6'-8" | I INT., SLIDING

* SEE ELEVATION FOR TRANSOM.

W	INDOW S	CHE	DULE SYMBOL
MARK	SIZE	QUAN.	DESCRIPTION
A	2'-4" x 6'-2"	1	D.H.
В	(2) 2'-4" x 5'-2"	1	D.H., W/ 10" ARCH HEAD TRANSOM *
	2'-0" x 6'-2"		
C	3'-8" x 6'-2"	2	D.H., W/ 10" ARCH HEAD TRANSOM *
	2'-0" x 6'-2"		
D	2'-4" × 4'-2"	1	D.H.
E	2'-0" x 5'-10"	2	D.H.
F	(2) 2'-4" x 5'-10"	1	D.H.
G	(2) 2'-4" x 6'-2"	2	D.H.
Н	(2) 2'-0" x 6'-2"	3	D.H.
J	2'-0" x 6'-2"	2	D.H.
K	(2) 2'-8" x 6'-2"	1	D.H.
L	(2) 2'-8" x 5'-10"	2	DH.
M	2'-0" x 4'-6"	1	D.H.
N	(2) 2'-4" × 4'-2"	1	D.H., W/ 10" ARCH HEAD TRANSOM *
P	2'-8" × 6'-2"	1	D.H.
Q	2'-2" × 1'-8"	1.	FIXED, PARTIAL ARCH HEAD LEFT **
R	5'-0" x 3'-0"	1	FIXED, ARCH HEAD **
5	2'-2" × 1'-8"	1	FIXED, PARTIAL ARCH HEAD RIGHT **
T	5'-4" x 1'-4"	1	FIXED, ARCH HEAD **
и	2'-8" x 3'-10"	1.	D.H., W/ HALF CIRCLE TRANSOM *

ALL WINDOWS ARE INSULATED AND WEATHERSTRIPPED.

VERIFY LOCAL CODE EGRESS WINDOW REQUIREMENTS PRIOR TO CONSTRUCTION. WINDOW MANUFACTURER TO PROVIDE TEMPERED GLASS WHERE REQUIRED BY CODE. VERIFY WINDOW MODEL NUMBERS AND SIZES WITH MANUFACTURER BEFORE STARTING CONSTRUCTION.

* SEE ELEVATION FOR TRANSOM.

ELEC	CTRICAL LEGEND
SYMBOL	DESCRIPTION
\Rightarrow	DUPLEX OUTLET (GROUNDED TYPE)
	WEATHER PROOF DUPLEX OUTLET
=	ABOVE COUNTER LOCATION
\bigcirc	FLUSH FLOOR DUPLEX OUTLET, OWNER VERIFY LOCATION
	220 YOLT OUTLET OR CONNECTION
- \(-	CEILING MOUNTED LIGHT FIXTURE
-фı	WALL BRACKET MOUNTED LIGHT FIXTURE
+	SUSPENDED CEILING MOUNTED LIGHT FIXTURE
	RECESSED DIRECTIONAL CEILING LIGHT FIXTURE
	RECESSED CEILING LIGHT FIXTURE
O w	RECESSED LIGHT FOR WET AREA
	UNDER CABINET FLUORESCENT LIGHT
\(\)	EXTERIOR CAST ALUMINUM FLOOD LIGHTS
\aleph	CEILING MOUNTED PADDLE FAN W/ LIGHT
\otimes	CEILING MOUNTED FAN - EXHAUST
\triangleright	CEILING MOUNTED FAN AND HEATER
R	

PLAN NOTES:

VERIFY ALL LOCAL CODES, ENERGY TYPES, AND SITE CONDITIONS PRIOR TO CONSTRUCTION.

REVIEW SELECTED MECHANICAL SYSTEMS WITH

CONSTRUCTION. VERIFY LOCAL BUILDING CODE REQUIREMENTS AND MANUFACTURER REQUIREMENTS FOR ATTIC OR GARAGE LOCATIONS.

OWNER PRIOR TO CONSTRUCTION. REVIEW SUB-CONTRACTORS LOCATIONS OF THE WATER HEATER AND HYAC UNIT(S) WITH THE OWNER PRIOR TO

H.V.A.C. EQUIPMENT IN THE ATTIC SPACE SHALL BE ACCESSIBLE BY AN OPENING LARGER THAN THE LARGEST PIECE OF EQUIPMENT (TO ALLOW REMOVAL OF THE EQUIPMENT) AND IN NO CASE LESS THAN 22"x36".

SEE STRUCTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

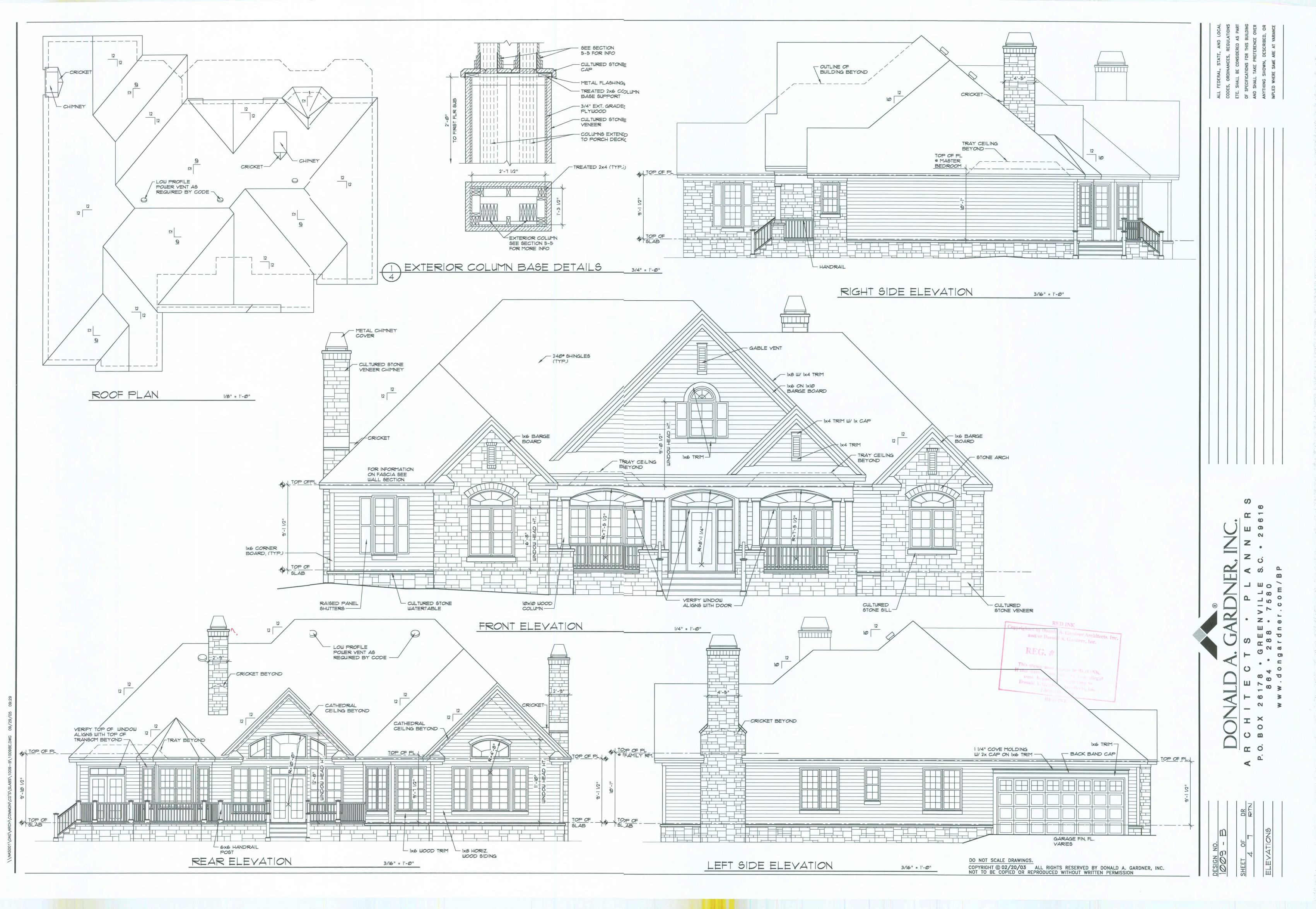
4. INSULATE AROUND ALL BATHS AND UTILITY ROOM.

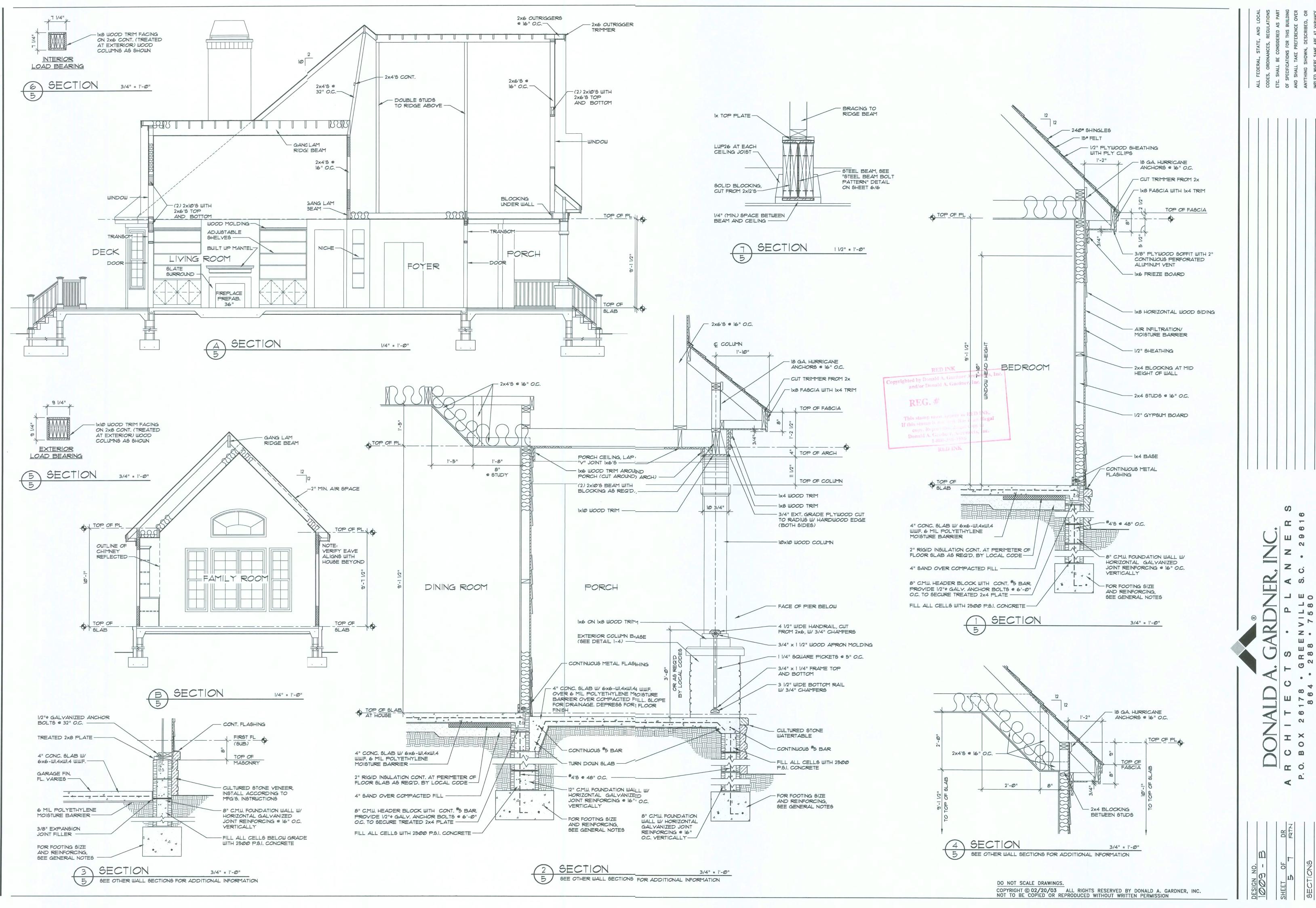
TYPICAL WALL, 2x4'S @ 16" O.C., UNLESS

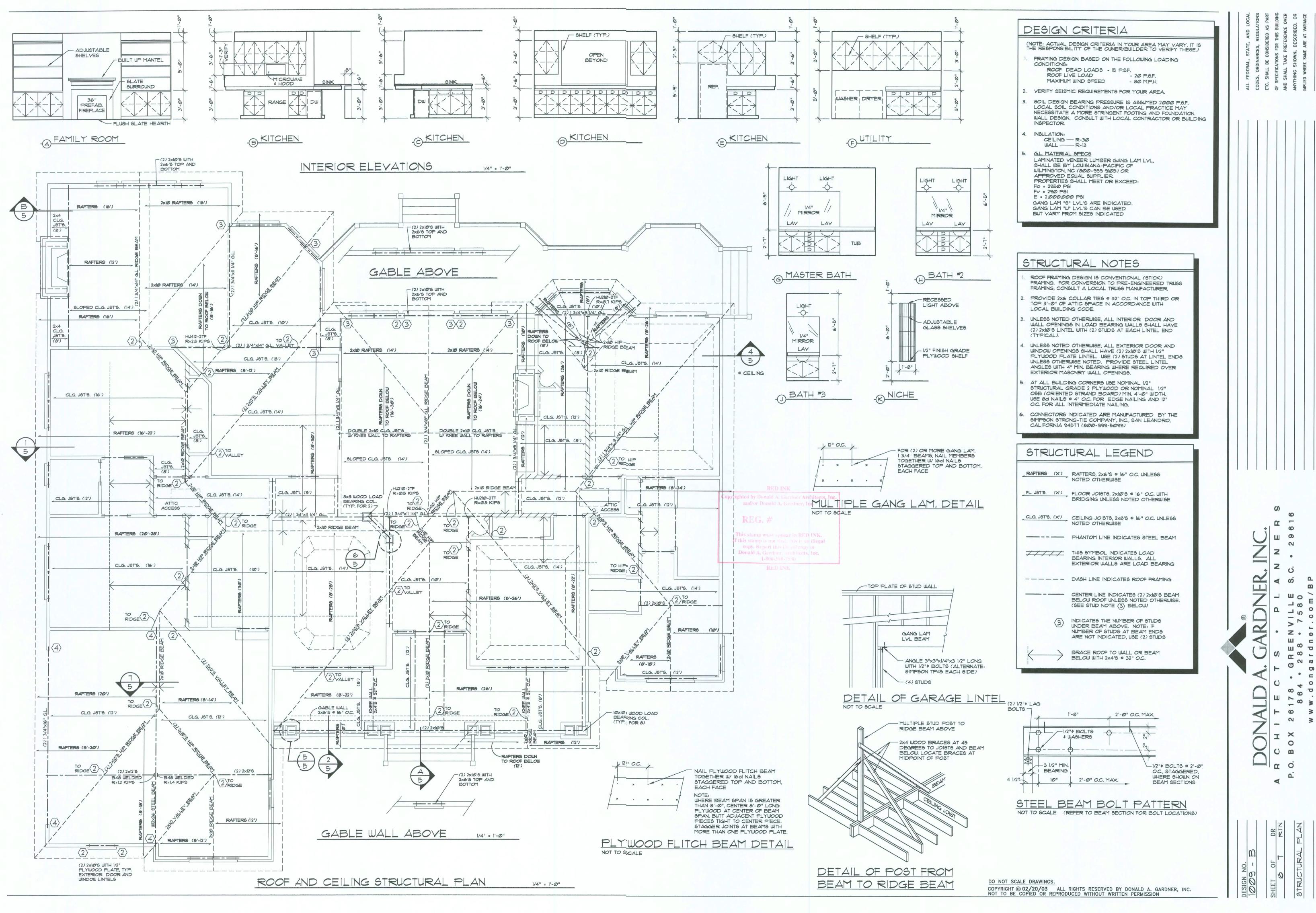
OTHERWISE DIMENSIONED. PROVIDE SMOKE DETECTORS AS REQUIRED BY CODE.

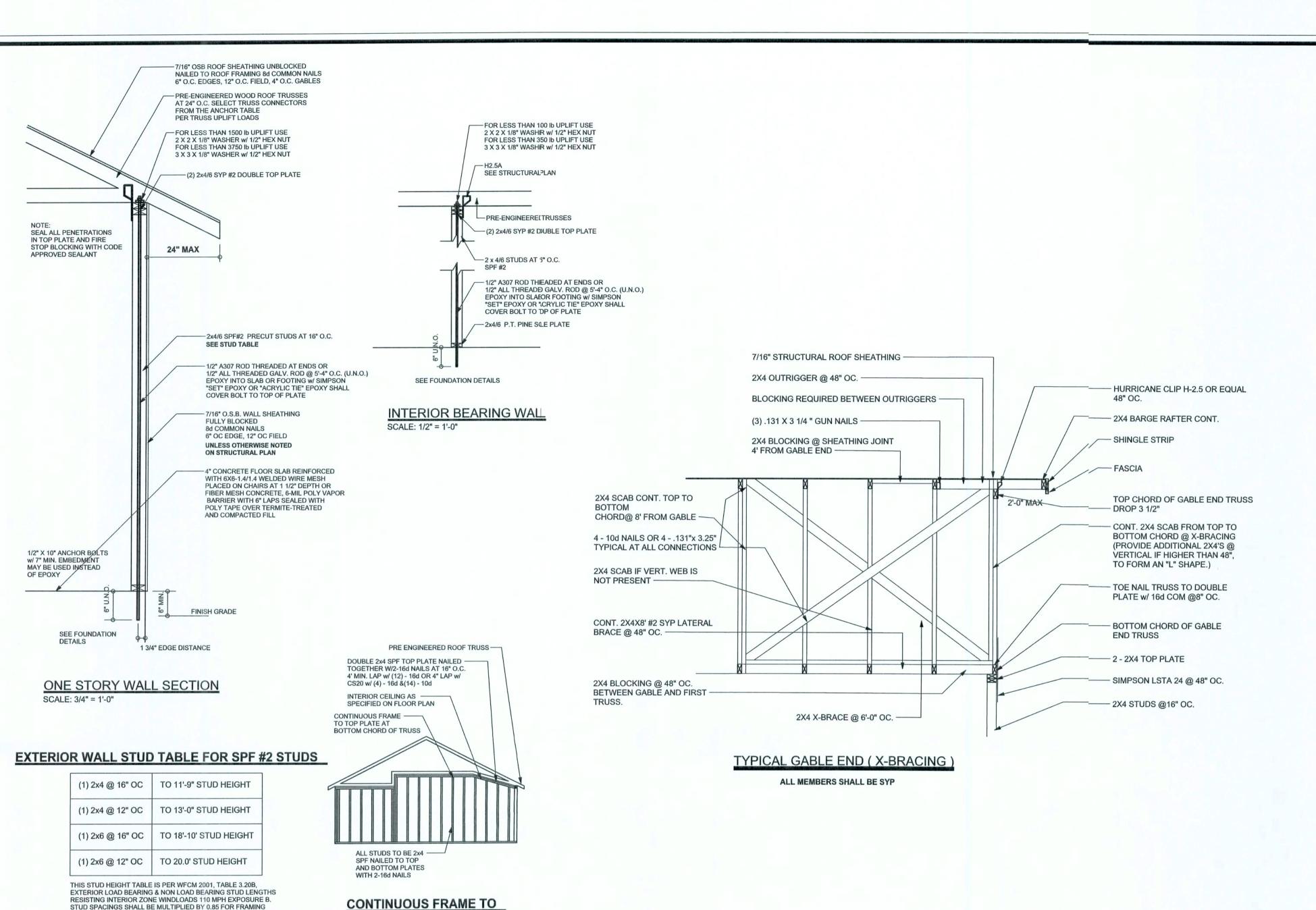
PROVIDE DOORBELLS, TRANSFORMER, AND CHIME.

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ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	1
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RC 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

WELDED WIRE REINFORCED SLAB: 6" × 6" × 0" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

FIBER CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

AHAID I							
	OADS PER FLORIDA BUILDING CODE 2004 RES						
ON UP	OSED SIMPLE DIAPHRAGM BUILDINGS WITH FI ROOF HEIGHT NOT EXCEEDING LEAST HORIZO PER HALF OF HILL OR ESCARPMENT 60FT IN E AND UNOBSTRUCTED UPWIND FOR 50x HEIG	ONTAL D	IMEN OFT I	SION	OR 60	0 FT; NOT	Γ
BUILDI	NG IS NOT IN THE HIGH VELOCITY HURRICANE	ZONE					
BUILDI	NG IS NOT IN THE WIND-BORNE DEBRIS REGIO	N					
1.) B/	ASIC WIND SPEED = 110 MPH						_
2.) W	IND EXPOSURE = B						
3.) W	IND IMPORTANCE FACTOR = 1.0		-			-	
4.) Bl	JILDING CATEGORY = II						
5.) R	OOF ANGLE = 10-45 DEGREES						
6.) MI	EAN ROOF HEIGHT = <30 FT						_
7.) IN	TERNAL PRESSURE COEFFICIENT = N/A (ENCL	OSED B	UILDI	NG)			_
	OMPONENTS AND CLADDING DESIGN WIND PR				R301.	.2(2))	
		Zone	Effec	tive W	ind Are	a (ft2)	
			_	0	r	100	
		1	19.9	-21.8	18.1	-18.1	
4	2 2	2	19.9	-25.5	18.1	-21.8	
5	1 1 2 2	2 O'hg		-40.6		-40.6	
2	2 2 2 5	3	19.9		18.1	-21.8	
	4	3 O'hg		-68.3		-42.4	
		4	21.8	-23.6	18.5	-20.4	
Ž	555	5	21.8	-29.1	18.5	-22.6	
	The state of the s	Doors	& Wind		21.8	-29.1	
6	2		5, 10	-			

DESIGN	55 22			
DESIGN			***	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

WINDLOAD ENGINEER: Mak Disosway,

PE No.53915, POB 868, Lake City, FL

32056, 386-754-5419

REVISIONS

DIMENSIONS: Stated dimensions superced scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarifcation.

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examined this plan, and that he applicable portions of the plan, relating b wind engineer comply with section R301.2.1, florida building code residential 2004, to the sest of my

CERTIFICATION: I hereby certify that I have

LIMITATION: This design is valid for one building, at specified location

Blake Construction

Spec Horse Lot 18 Country Lake in Woodbough S/D Phase [

ADDRESS: Lot 18 Country Lake in Woodbough S/D Phase I Columbia County, Florida

Mark Disosway P.E. P.O. Box 368 Lake City, Florida 32056 Phone: (386) 754 - 5419

August 25, 20)6 DRAWN BY: STRUCTURAL BY Cavid Disosway

Fax: (386) 269 - 4871

24 / Aug / 06

JOB NUMBER: 608155 DRAWING NUMBER

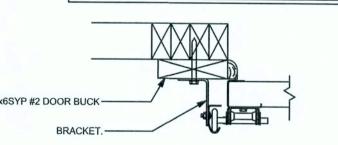
> S-1 OF 3 SHEE'S



		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

2 X 2 X 1/8" WASHER

3 X 3 X 1/8" WASHER

-- SP4/6 @ 48" O.C. (U.N.O.) /-

CRIFPPLES IF REQUIRED

(5) .1311 x 3 1/4" GUN NAILS

INTO JACK STUD U.N.O.

TOE INAILED THRU SILL-

TYPIC/AL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN)

(1) 2X6 SPFF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF= #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF= #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

FOR LESS THAN 3750 Ib UPLIFT USE

-NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT

—(7) .131 x 3 1/4" GUN NAILS

INTO KING STUD

TOE NAILED THRU HEADER

POST CONNECTION, INSTALL ONE SIMPSON LSTA18 ON ONE SIDE NAIL THRU 2x4 INT BEAM W/4-16d - SIMPSON HUS412 MIN. (2-ONE SIDE, 2-ON SEE STRUCTURAL PLAN

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

2) 2X12 SYP #2 MIN. -SÉE STRUCTURAL PLAN

> SIMPSON HUS412 MIN. SEE STRUCTURAL PLAN

SCALE: N.T.S.

LSTA24

BEAM MAY BE ATTACHED IN

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

-(4)-2x4 SPF #2 NAILED

MIN. (SEE STRUCTURAL PLAN)

SEE STRUCTURAL PLAN

NAILS AT 16" O.C.

BEAM MID-WALL CONNECTION DETAIL

BEAM CORNER CONNECTION. DETAIL SUPPORTIVE CENTER POST TO BEAM DETAL

OTHER SIDE)

SUPPORTIVE -

3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON-

OPPOSITE SIDE) EA.

NAILED WITH 14-10d

SCALE: N.T.S.

CEILING DIAPHRAGM DETAIL

- NON-SUPPORTIVE

2X4 LADDER BEAM

— SUPPORTIVE

COLUMN

SUPPORTIVE POST TO BEAM

DETAIL FOR SINGLE BEAM

SUPPORTIVE BEAM -

SEE FOOTING DETAILS

SIMPSON H2.5A U.N.O. SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21 -

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

TYPICAL PORCH POST DETAIL

-(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

ANCHOR BOLT

IF TRUSS TO WALL STRAPS ARE NAILED

TO THE HEADER THE SP4/6 @ 48" O.C.

INTO KING STUD

ARE NOT REQUIRED

(7) .131 x 3 1/4" GUN NAILS -

TOE NAILED THRU HEADER

TYPICAL 1 STORY' HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

