



**Project Information for: L252568**

Address: 555 Lauman Glen  
Ft. White, Florida  
County: Columbia  
Truss Count: 4  
Design Program: MiTek 20/20 6.3  
Building Code: FBC2004/TPI2002

**Truss Design Load Information:**

**Gravity:** **Wind:**

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

**Engineer of Record:** Unknown at time of Seal Date

Address: Unknown at time of Seal Date

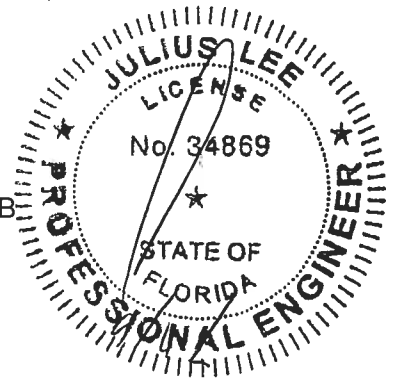
**Truss Design Engineer:** Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**Notes:**

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

September 6, 2007



No.	Drwg. #	Truss ID	Seal Date
1	J1889041	T01	9/6/07
2	J1889042	T01G	9/6/07
3	J1889043	T02	9/6/07
4	J1889044	T02G	9/6/07



**Project Information for: L252568**

... Address: 555 Lauman Glen  
... Ft. White, Florida  
County: Columbia  
Truss Count: 4  
Design Program: MiTek 20/20 6.3  
Building Code: FBC2004/TPI2002

September 6, 2007

**Truss Design Load Information:**  
**Gravity: Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B  
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

**Engineer of Record:** Unknown at time of Seal Date  
Address: Unknown at time of Seal Date

**Truss Design Engineer:** Julius Lee, PE Florida P.E. License No. 34869  
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**Notes:**

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Seal Date
1	J1889041	T01	9/6/07
2	J1889042	T01G	9/6/07
3	J1889043	T02	9/6/07
4	J1889044	T02G	9/6/07

Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T01	SCISSOR	17	1	J1889041
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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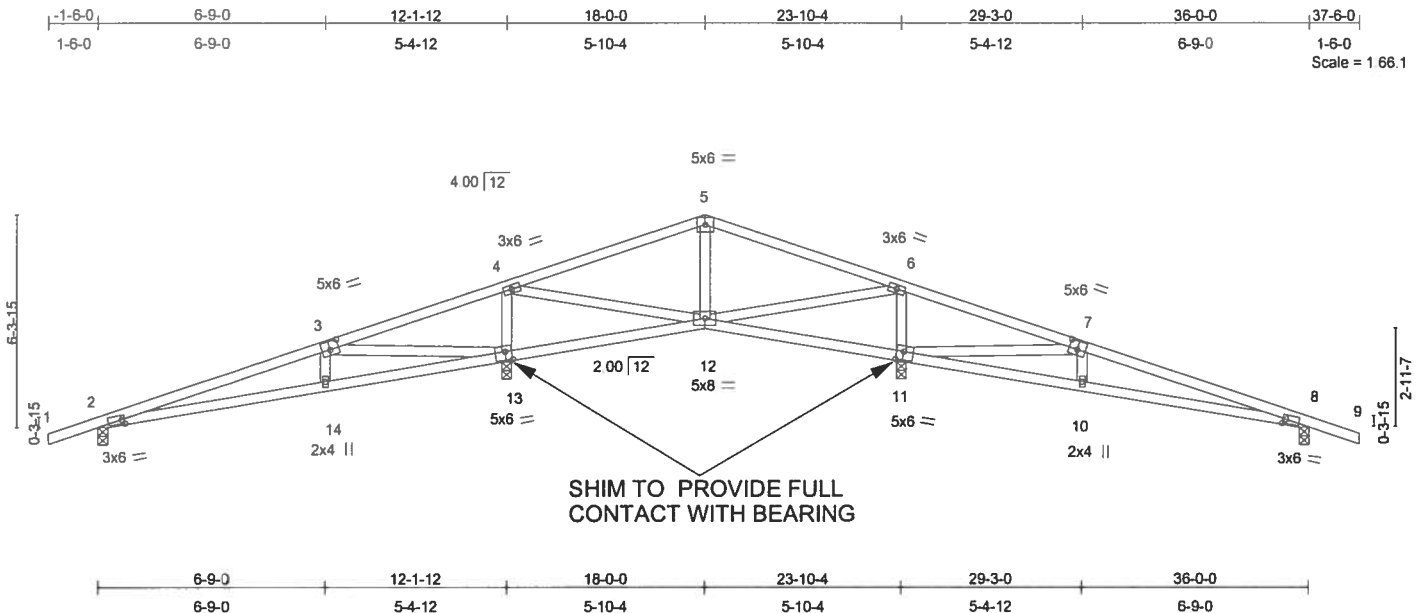


Plate Offsets (X,Y): [2:0-1-4,0-1-8], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-1-4,0-1-8], [11:0-2-8,0-3-0], [13:0-2-8,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.39	Vert(LL)	0.27	8-10	>526	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.12	8-10	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.52	Horz(TL)	-0.02	13	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 160 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

#### REACTIONS (lb/size)

13=866/0-3-8, 11=866/0-3-8, 2=365/0-3-8, 8=365/0-3-8  
Max Horz 2=88(load case 6)  
Max Uplift 13=425(load case 6), 11=403(load case 5), 2=263(load case 4),  
8=280(load case 7)  
Max Grav 13=867(load case 10), 11=867(load case 11), 2=369(load case 10),  
8=369(load case 11)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/23, 2-3=-548/1101, 3-4=-990/629, 4-5=-604/71, 5-6=-604/71, 6-7=-990/629,  
7-8=-548/1101, 8-9=0/23  
BOT CHORD 2-14=-917/476, 13-14=-782/467, 12-13=-557/1313, 11-12=-557/1313,  
10-11=-782/467, 8-10=-917/476  
WEBS 3-14=-528/192, 3-13=-997/2038, 4-13=-579/409, 4-12=-271/534, 5-12=-332/625,  
6-12=-271/534, 6-11=-579/409, 7-11=-997/2038, 7-10=-528/192

Julius Lee  
Truss Design Engineer  
Florida PE No. 34658  
1100 Crystal Bay Blvd  
Daytona Beach, FL 32126

#### JOINT STRESS INDEX

2 = 0.44, 3 = 0.61, 4 = 0.38, 5 = 0.59, 6 = 0.38, 7 = 0.61, 8 = 0.44, 10 = 0.33, 11 = 0.67, 12 = 0.35, 13 = 0.67 and 14 = 0.33

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

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#### Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T01	SCISSOR	17	1	J1889041
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 425 lb uplift at joint 13, 403 lb uplift at joint 11, 263 lb uplift at joint 2 and 280 lb uplift at joint 8.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 13, 11.

**LOAD CASE(S)** Standard

Julius Lee  
Truss Design Engineer  
Florida P.E. No. 31888  
1100 Coastal Bay Blvd  
Daytona Beach, FL 32135

September 6, 2007

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Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T01G	GABLE	1	1	J1889042
Job Reference (optional)					
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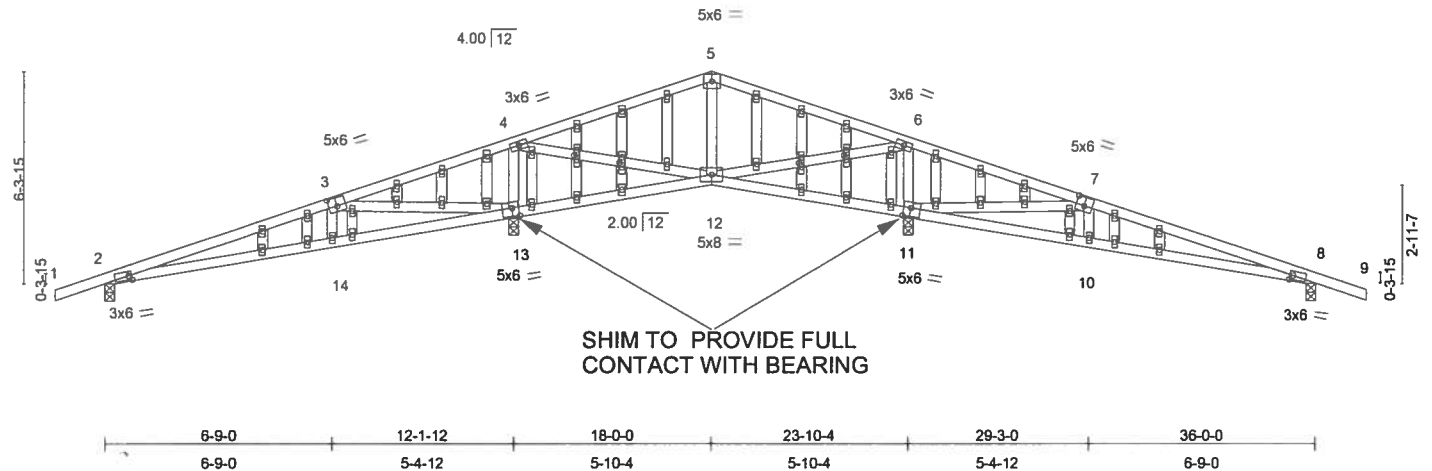
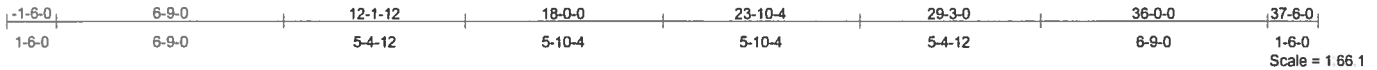


Plate Offsets (X,Y): [2:0-1-4,0-1-8], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-1-4,0-1-8], [11:0-2-8,0-3-0], [13:0-2-8,0-3-0], [17:0-1-8,0-1-0], [20:0-1-8,0-1-0], [38:0-1-8,0-1-0], [41:0-1-8,0-1-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.39	Vert(LL)	0.27	8-10	>526	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.12	8-10	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.52	Horz(TL)	-0.02	13	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 199 lb

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
 BOT CHORD 2 X 4 SYP No.2  
 WEBS 2 X 4 SYP No.3  
 OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

#### REACTIONS (lb/size) 13=866/0-3-8, 11=866/0-3-8, 2=365/0-3-8, 8=365/0-3-8

Max Horz 2=106(load case 4)  
 Max Uplift 13=-613(load case 4), 11=-585(load case 7), 2=-344(load case 6),  
 8=-370(load case 5)  
 Max Grav 13=867(load case 10), 11=867(load case 11), 2=369(load case 10),  
 8=369(load case 11)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/23, 2-3=-548/1101, 3-4=-991/629, 4-5=-604/71, 5-6=-604/71, 6-7=-991/629,  
 7-8=-548/1101, 8-9=0/23  
 BOT CHORD 2-14=-917/476, 13-14=-782/468, 12-13=-557/1313, 11-12=-557/1313,  
 10-11=-782/468, 8-10=-917/476  
 WEBS 3-14=-528/192, 3-13=-997/2038, 4-13=-579/409, 4-12=-271/534, 5-12=-332/625,  
 6-12=-271/534, 6-11=-579/409, 7-11=-997/2038, 7-10=-528/192

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Continued on page 2

September 6, 2007

**Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE**  
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T01G	GABLE	1	1	J1889042
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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#### JOINT STRESS INDEX

2 = 0.44, 3 = 0.61, 3 = 0.33, 4 = 0.38, 4 = 0.33, 5 = 0.59, 6 = 0.38, 6 = 0.33, 7 = 0.61, 7 = 0.33, 8 = 0.44, 10 = 0.33, 11 = 0.67, 11 = 0.33, 12 = 0.35, 13 = 0.67, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.45, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.45, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.33, 28 = 0.33, 29 = 0.33, 30 = 0.33, 31 = 0.33, 32 = 0.33, 33 = 0.33, 34 = 0.33, 35 = 0.33, 36 = 0.33, 37 = 0.33, 38 = 0.45, 38 = 0.33, 39 = 0.33, 40 = 0.33, 41 = 0.45, 41 = 0.33, 42 = 0.33, 43 = 0.33, 44 = 0.33, 45 = 0.33, 46 = 0.33, 47 = 0.33, 48 = 0.33, 49 = 0.33, 50 = 0.33, 51 = 0.33 and 52 = 0.33

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 13, 585 lb uplift at joint 11, 344 lb uplift at joint 2 and 370 lb uplift at joint 8.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 13, 11.
- 11) Gable truss supports 1' 0" max. rake gable overhang.

**LOAD CASE(S)** Standard

Julius Lee  
Truss Design Engineer  
Florida PE No. 34888  
1100 Coastal Bay Blvd  
Boynton Beach, FL 33435

September 6, 2007

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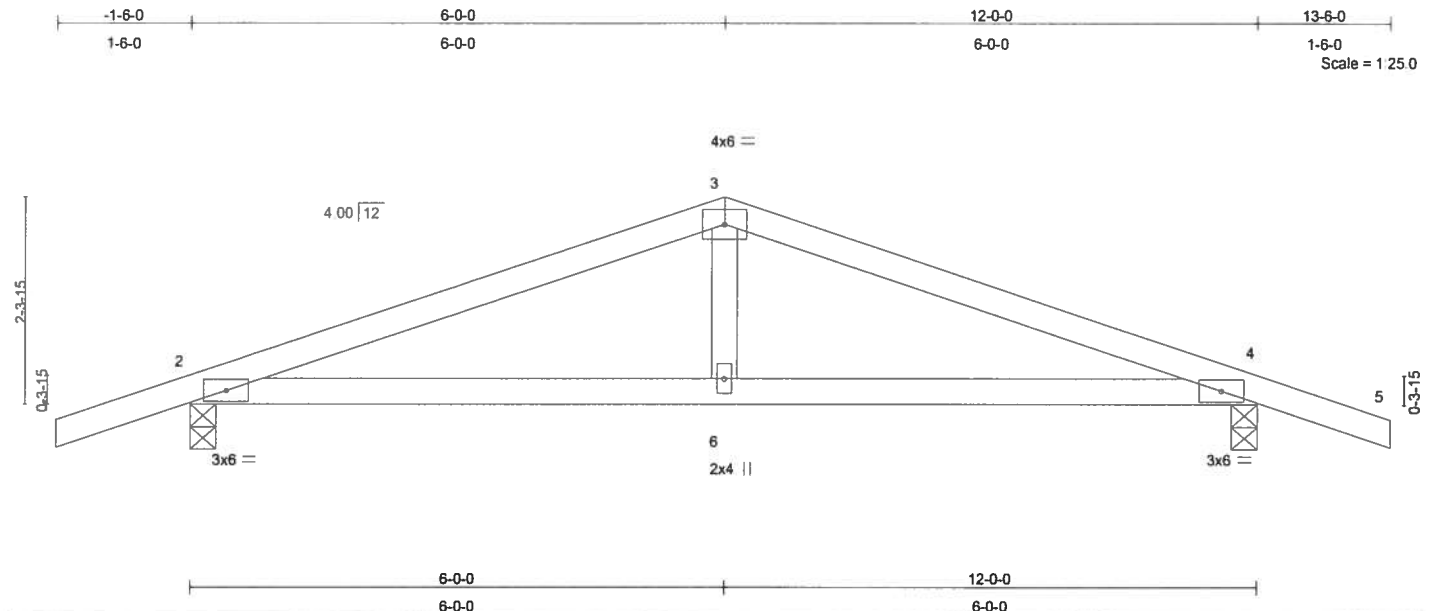
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Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T02	COMMON	17	1	J1889043
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.03	2-6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.21	Vert(TL)	-0.06	4-6	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.06	Horz(TL)	0.01	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 44 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=464/0-3-8, 4=464/0-3-8  
Max Horz 2=41(load case 6)  
Max Uplift 2=-164(load case 4), 4=-164(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/24, 2-3=-692/404, 3-4=-692/404, 4-5=0/24  
BOT CHORD 2-6=-280/609, 4-6=-280/609  
WEBS 3-6=0/200

#### JOINT STRESS INDEX

2 = 0.34, 3 = 0.70, 4 = 0.34 and 6 = 0.14

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 2 and 164 lb uplift at joint 4.

Continued on page 2

Julius Lee  
Truss Design Engineer  
Florida PE No. 31888  
1100 Crystal Bay Blvd  
Weymouth MA 01981

September 6, 2007

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Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T02	COMMON	17	1	J1889043
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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**LOAD CASE(S)** Standard

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Truss Design Engineer  
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September 6, 2007

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Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T02G	GABLE	2	1	J1889044
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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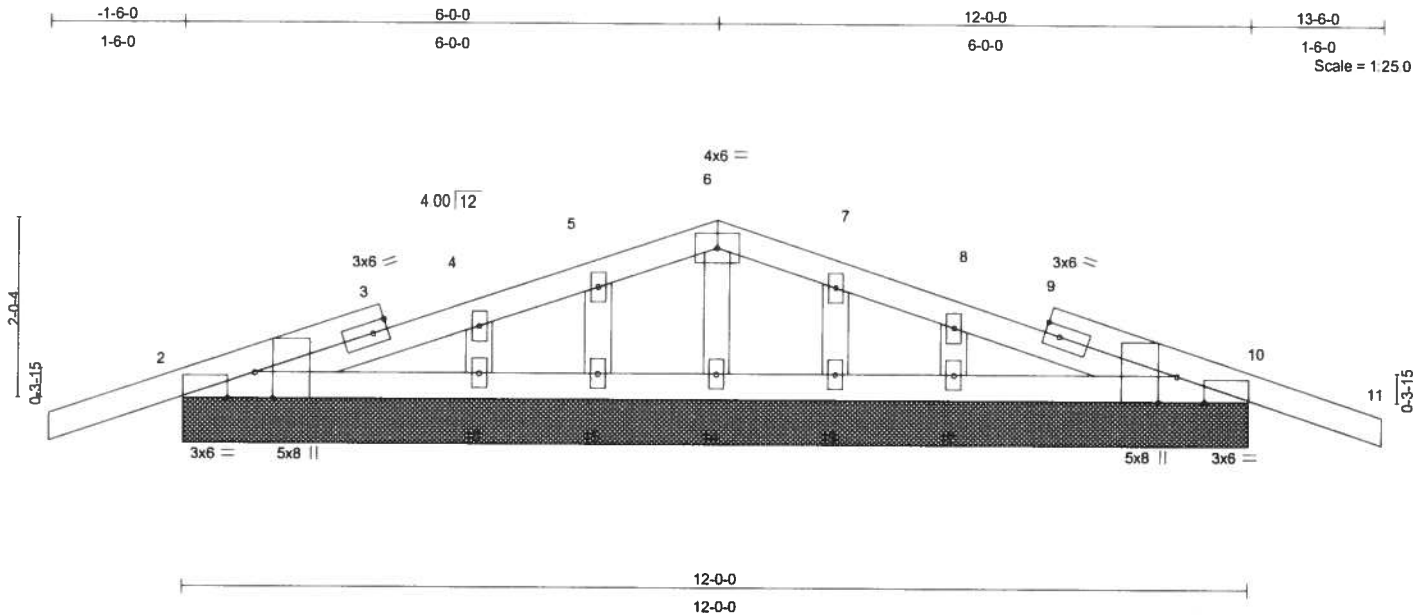


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-3-12,Edge], [10:0-3-8,Edge], [10:0-3-12,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.21	Vert(LL)	-0.01	11	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	-0.01	11	n/r	90		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.04	Horz(TL)	0.00	10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 53 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 2=303/12-0-0, 10=303/12-0-0, 14=155/12-0-0, 15=89/12-0-0, 16=243/12-0-0, 13=89/12-0-0, 12=243/12-0-0

Max Horz 2=44(load case 4)

Max Uplift 2=-206(load case 6), 10=-211(load case 7), 14=-38(load case 6), 15=-62(load case 4), 16=-106(load case 6), 13=-60(load case 5), 12=-109(load case 7)

Max Grav 2=304(load case 10), 10=304(load case 11), 14=155(load case 1), 15=92(load case 10), 16=243(load case 10), 13=92(load case 11), 12=243(load case 11)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6/39, 2-3=-15/25, 3-4=-23/56, 4-5=-14/54, 5-6=-5/74, 6-7=-5/74, 7-8=-14/54, 8-9=-5/56, 9-10=-21/15, 10-11=-6/39

BOT CHORD 2-16=-15/54, 15-16=-15/54, 14-15=-15/54, 13-14=-15/54, 12-13=-15/54, 10-12=-15/54

WEBS 6-14=-137/59, 5-15=-89/79, 4-16=-211/167, 7-13=-89/79, 8-12=-211/167

Julius Lee  
Truss Design Engineer  
Florida PE No. 31888  
1100 Coastal Bay Blvd  
Daytona Beach, FL 32118

#### JOINT STRESS INDEX

2 = 0.23, 2 = 0.00, 3 = 0.00, 3 = 0.25, 4 = 0.09, 5 = 0.04, 6 = 0.09, 7 = 0.04, 8 = 0.09, 9 = 0.00, 9 = 0.25, 10 = 0.23, 10 = 0.00, 12 = 0.09, 13 = 0.04, 14 = 0.05, 15 = 0.04 and 16 = 0.09

Continued on page 2

September 6, 2007

**Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE**

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITTEK connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	OWNER/BLDR.
L252568	T02G	GABLE	2	1	J1889044
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Sep 04 16:07:18 2007 Page 2

## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 2, 211 lb uplift at joint 10, 38 lb uplift at joint 14, 62 lb uplift at joint 15, 106 lb uplift at joint 16, 60 lb uplift at joint 13 and 109 lb uplift at joint 12.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-6=-87(F=-33), 6-11=-87(F=-33), 2-10=-10

Julius Lee  
Truss Design Engineer  
Florida PE No. 3-1888  
1100 Coastal Bay Blvd  
Gwynnton Beach, FL 33438

September 6, 2007

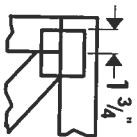
### Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

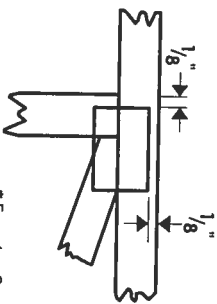


# Symbols

## PLATE LOCATION AND ORIENTATION



\*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



\*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



\*This symbol indicates the required direction of slots in connector plates.

## PLATE SIZE

4 X 4

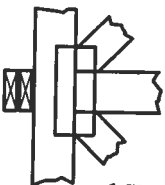
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING



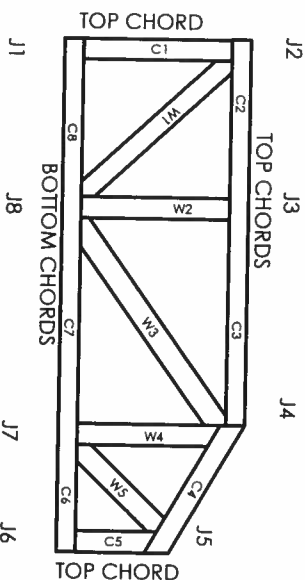
Indicates location of required continuous lateral bracing.

## BEARING



Indicates location of joints at which bearings (supports) occur.

# Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

## CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561



Mittek Engineering Reference Sheet: MIT-7473



## General Safety Notes

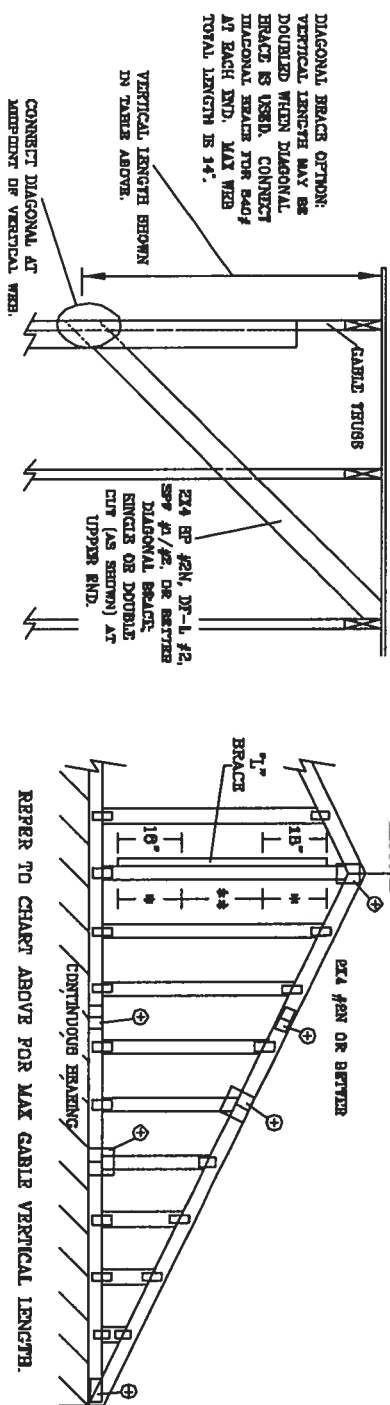
### Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ( $\pm 6"$  from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1X4 T" BRACE *		(1) 2X4 T" BRACE *		(2) 2X4 T" BRACE **		(1) 2X6 T" BRACE *		(2) 2X8 T" BRACE *	
SPACING	CABLE VERTICAL SPECIES	GRADE	BRACE	NO	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP B
24" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 8"	10' 10"	11' 2"	12' 11"	13' 3"	13' 3"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"	12' 11"	12' 11"
		STUD	8' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"	12' 11"	12' 11"
	HF	STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	9' 5"	9' 5"	11' 6"	11' 6"	11' 6"	11' 6"
		#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 11"	13' 11"	13' 11"
		#2	8' 7"	6' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 11"	13' 11"	13' 11"
16" O.C.	SPF	#1 / #2	3' 6"	5' 0"	5' 0"	6' 0"	6' 7"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"	13' 7"	13' 7"
		#3	3' 6"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	9' 8"	9' 8"	12' 11"	12' 0"	12' 0"	12' 0"
		STUD	3' 10"	6' 8"	6' 8"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STANDARD	3' 6"	5' 2"	5' 2"	6' 2"	6' 10"	8' 2"	8' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#1	4' 3"	8' 8"	8' 8"	7' 2"	7' 11"	8' 6"	8' 6"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	8' 8"	8' 8"	7' 2"	7' 11"	8' 6"	8' 6"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"



CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2X6

ATTACH EACH T" BRACE WITH 10d NAILS.  
 \* FOR (1) T" BRACE, BRACE NAILS AT 8" O.C.  
 IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.  
 \*\* FOR (2) T" BRACES, BRACE NAILS AT 3" O.C.  
 IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.  
 T" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.

LIVE LOAD DISTRIBUTION CRITERIA IS L/240.  
 PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BRACING (6 PSF VC DEAD LOAD).  
 CABLE END SUPPORTS LOAD FROM 4" O" OUTLEAKS WITH 2" O" OVERHANG, OR 12" PLYWOOD OVERHANG.

#### CABLE TRUSS DETAIL NOTES:

BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPECIES: PINE-TRE	HEM-FIR
#1 / #2	#2
STUD	STUD
GROUP B:	
SPECIES: PINE-TRE	HEM-FIR
#1 / #2	#2
STUD	STUD

DIAGONAL BRACE OPTION:  
 VERTICAL LENGTHS MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR EACH AT EACH END. MAX WEB TOTAL LENGTH IS 14'.  
 VERTICAL LENGTH SHOWN IN TABLE ABOVE.  
 CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

JULIUS LEE'S  
 CONSULTING ENGINEERS P.A.  
 1465 67th AVE. N.W.  
 MEDAN BEACH, FL 33441-2161

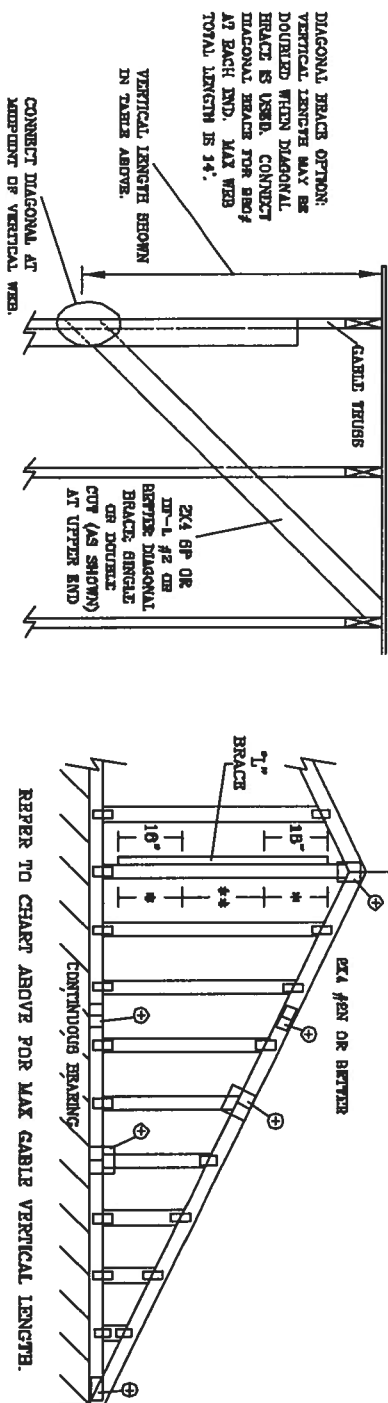
MAX. TOT. LD. 60 PSF  
 MAX. SPACING 24.0"

REF ASCE 7-02-CAB10015  
 DATE 11/26/03  
 DRWG. BY: SJD  
 ENG

No. 34866  
 STATE OF FLORIDA

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		(1) 1X4 7" BRACE		(1) 2X4 7" BRACE		(2) 2X4 7" BRACE		(1) 2X6 7" BRACE		(2) 2X8 7" BRACE	
GABLE VERTICAL SPACING	BRACE SPECIES	GRADE	NO BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 2"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"
	SPF	#3	3' 1"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"
	HF	STUD	3' 1"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"	4' 6"
	HF	STANDARD	2' 11"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"
16" O.C.	SPF	#1	3' 6"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"
	SPF	#2	3' 6"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"	5' 8"
	SPF	#3	3' 7"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"
	HF	STUD	3' 7"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"
24" O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"
	SPF	#3	3' 7"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"	5' 5"
	HF	STUD	3' 7"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"
	HF	STANDARD	3' 7"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"	5' 6"



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTHS.

BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-YR	RED-PTR	SPRUCE-PINE-YR	RED-PTR
#1 / #2	#1	#1 / #2	#1
STANDARD	STUD	STANDARD	STUD
#3	#3	#3	#3
STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD

### CABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS  $L/240$ .
- PROVIDE WEAFT CONNECTIONS FOR 120 PSF OVER CONTINUOUS BEARING (6 PSF PL DEAD LOAD).
- CABLE END SUPPORTS LOAD FROM 4' 0" OUTLINES WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH 7" BRACE WITH 104 NAILS.
- \* FOR (1) 7" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.
- \* FOR (2) 7" BRACE, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 8" O.C. BETWEEN ZONES.
- 7" BRACING MUST BE A MINIMUM OF 80X OR WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 0"	2X4
GREATER THAN 11' 0"	2X6

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPURCE, AND BEEL PLATES.

REMARKS: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES GUIDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 5100 PARKWAY DR., SUITE 100, FARMINGTON, VT, 05440 AND TRUSS BOARD FOR CONSTRUCTION AND SAFETY. ALL TRUSSES MUST BE PROPERLY ATTACHED TO THE WALLS AND CEILING. THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PLYWOOD AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**JULIUS LEE'S**  
CONS. ENGINEERS P.A.  
1465 BY 4TH AVENUE  
DEMBAY BRIDGE, FL 33444-0101

REF ASCE 7-02-CAB10030  
DATE 11/26/03  
DWG NOTE STD GABLE 30' 2' 17'

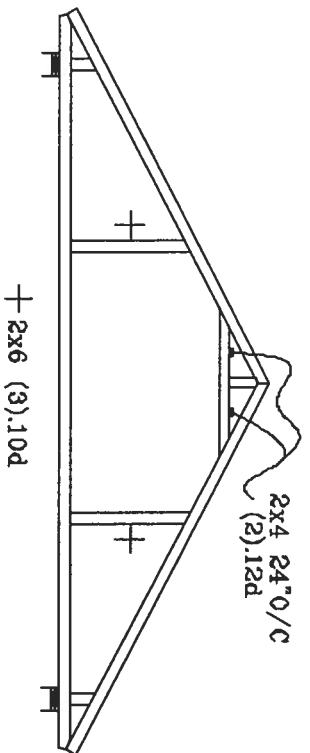
ENG

MAX. TOT. LD. 60 PSF

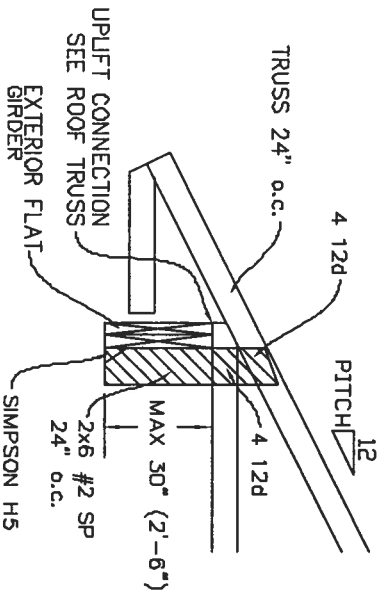
No. 34489  
STATE OF FLORIDA

MAX. SPACING 24.0"

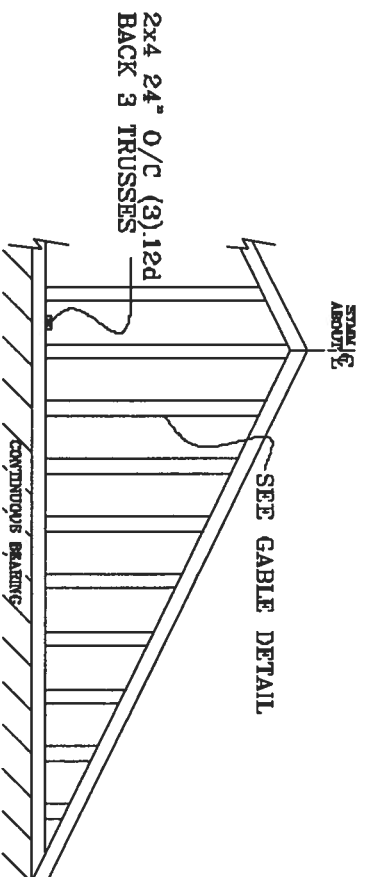
## TYPICAL ATTIC TRUSS BRACING



## TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

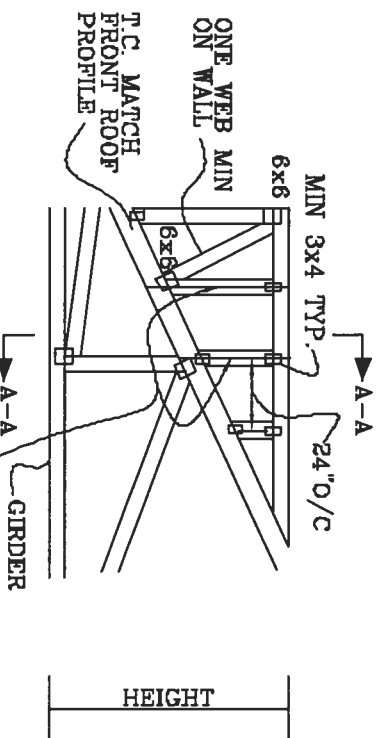


## GABLE END TRUSS DETAIL

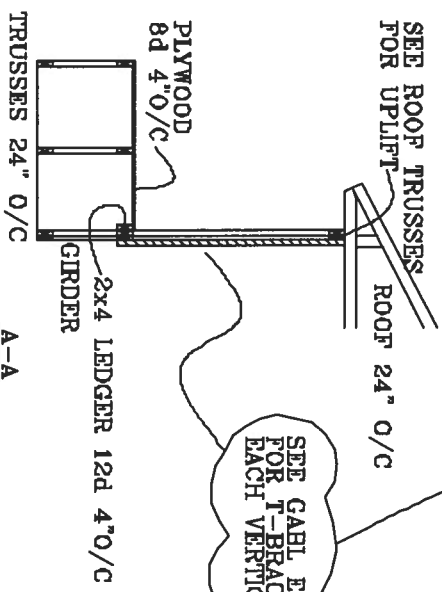


MONITOR BC BRACING ON GABLE TRUSS OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR DOB

## TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



**JULIUS LEE'S**  
CONS. ENGINEERS P.A.  
1425 SW 4TH AVENUE  
ORLANDO, FL 32804-2101

No. 34869  
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER  
BOT CHORD 2X4 #2 OR BETTER  
WEBS 2X4 #3 OR BETTER

# PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-93, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

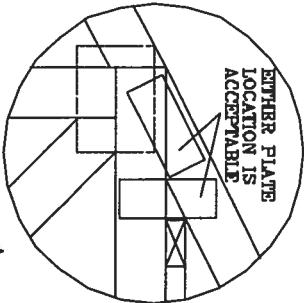
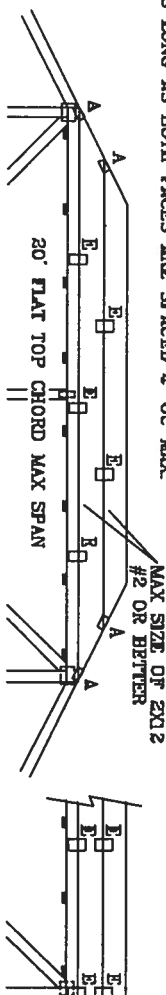
CAT 1, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SEC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

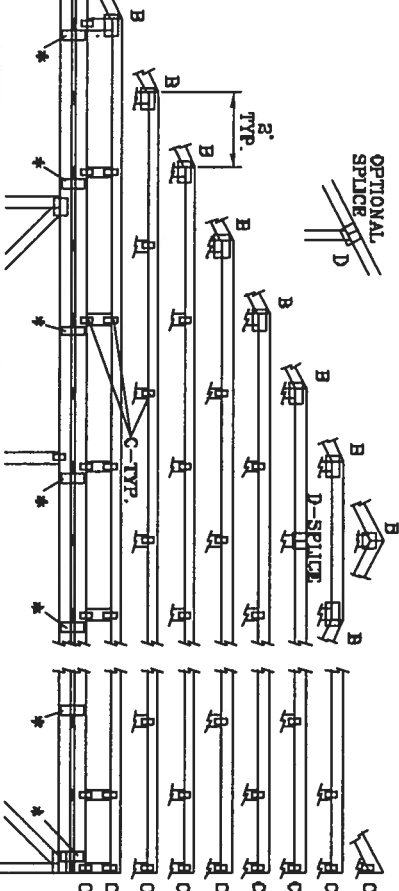
WIND TC DL=6 PSF, WIND BC DL=5 PSF

FRONT FACE (B,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



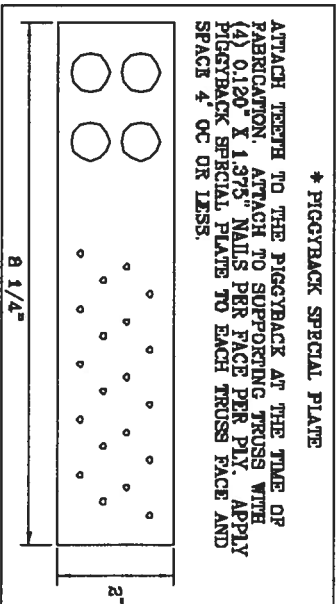
\*ATTACH PIGGYBACK WITH 5X8 TRUSS OR ALPINE PIGGYBACK SPECIAL PLATE.



JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (6) 0.120\" X 1.375\" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	WEB BRACING CHART
0' TO 7'0"	NO BRACING
7'9" TO 10'	1/4\" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER. AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4' OC.
10' TO 14'	2x4\" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER. AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



\* PIGGYBACK SPECIAL PLATE  
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120\" X 1.375\" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

PROVIDING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, DETAILING AND ERECTING. REFER TO SEALED DESIGN FOR DASHED PLATES. THIS DETAIL IS APPLICABLE FOR TRUSSES OF AMERICA, ASIDE ENTERPRISE IN HANSON, MI 3870 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED END CEILING.

**JULIUS LEE'S**  
CONS. ENGINEERS P.A.  
1409 SW 4th AVENUE  
DEER BEACH, FL 33444-2611

No. 34888  
STATE OF FLORIDA

MAX LOADING		REF PIGGYBACK	
55 PSF AT	DATE 11/26/03	DRWG/ITEK STD PIGGY	-ENG JL
1.33 DUR. FAC.			
50 PSF AT			
1.25 DUR. FAC.			
47 PSF AT			
1.15 DUR. FAC.			
SPACING 24.0"			

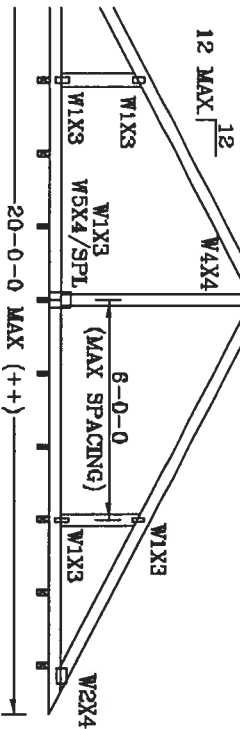
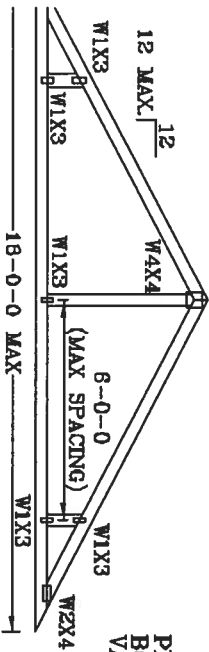
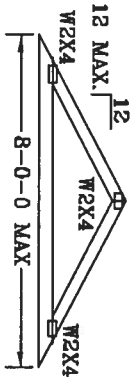
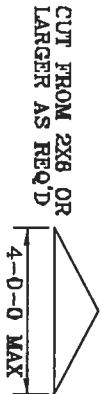
THIS DRAWING REPLACES DRAWINGS 634.018 834.017 & 847.045

# VALLEY TRUSS DETAIL

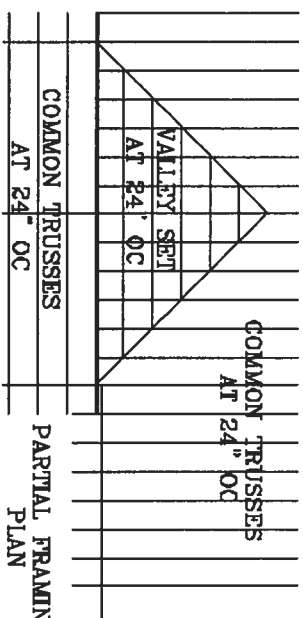
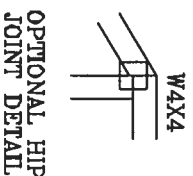
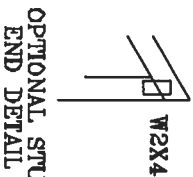
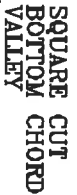
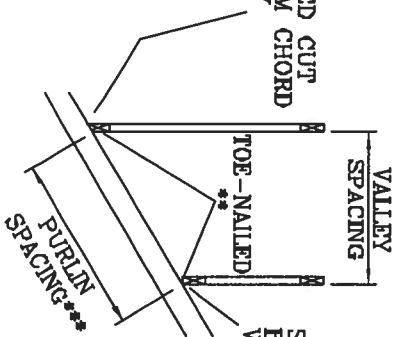
TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.  
BOT CHORD 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.  
WEBS 2X4 SP #3 OR BETTER.

\* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:  
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR  
SBC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 16d FOR  
ASCE 7-98 130 MPH WIND. 16" MEAN HEIGHT, ENCLOSED  
BUILDING, EXP. C, RESIDENTIAL, WIND TC D1=5 PSF.



**SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.**



\*\*\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "J"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.13" X 2.5") NAILS AT 8" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:  
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS  
INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

\*\*\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 120".

**BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.**

OPTIONAL STUB END DETAIL	OPTIONAL HIP JOINT DETAIL
	

### OPTIONAL HIP JOINT DETAIL

**THIS DRAWING REPLACES DRAWING A105**

JULIUS LEE'S CONS. ENGINEERS P.A. 1455 SW 4th AVENUE DELRAY BEACH, FL 33444-3201			
TC LL	20	PSF	REF VALLEY DETAIL
TC DL	7	PSF	DATE 11/26/03
BC DL	5	PSF	DRWG VALTRUSS1103
BC LL	0	PSF	-ENG JL

ON VARIOUS TYPES OF TRUSS ROOF EXPOSED TO FIRE. FABRICATING, HANDLING, SHIPPING, INSTALLING AND REPAIRING. REFER TO ACCT. 10-2, QUALIFYING DEPENDENT SAFETY METHOD (TDM), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 560 DORFMAN DR., SUITE 200, MADISON, WI 53705 AND VITA CROWN TRUSS COMPANY OF AMERICA, 6300 ENTERPRISE BL. MADISON, WI 53705 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. THESE CHANGES INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

**No. 34869**  
**STATE OF FLORIDA**

VERTICAL	1.63
SPACING	24"

[illegible]

# TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/A&PA NDS-1997 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

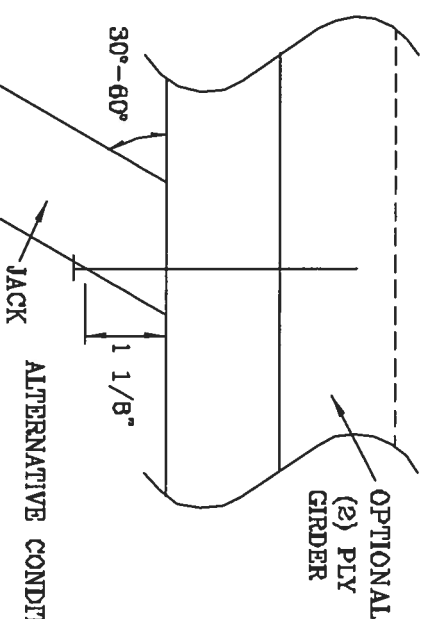
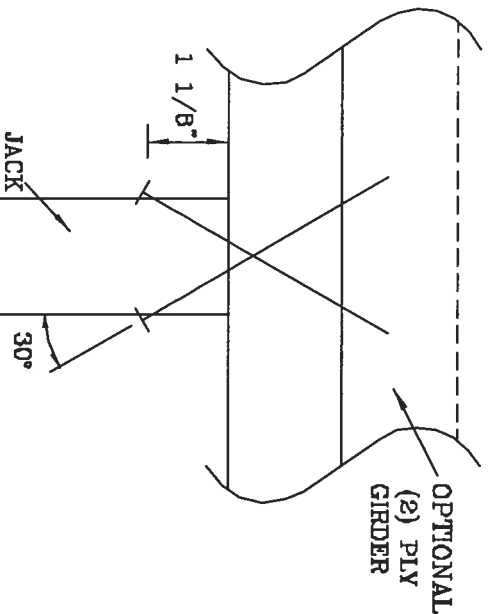
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOST 1-93 CRUISING COMPONENT SAFETY (UNRETRIEVED), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 588 PINEHURST DR., SUITE 800, NASHVILLE, TN 37219 AND APCA (WOOD TRUSS DESIGN) PUBLISHED BY THE AMERICAN WOOD PRESSES ASSOCIATION, 15077B, CRUISING TEST FACILITY, SUITE 100, CHANDLER, AZ 85226. ALL TRUSSES MUST BE PROPERLY ATTACHED TO THE STRUCTURAL PANELS AND BATTEN CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

**JULIUS LEE'S**  
CONS. ENGINEERS P.A.  
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DELRAY BEACH, FL 33444-2101

No. 34689  
STATE OF FLORIDA

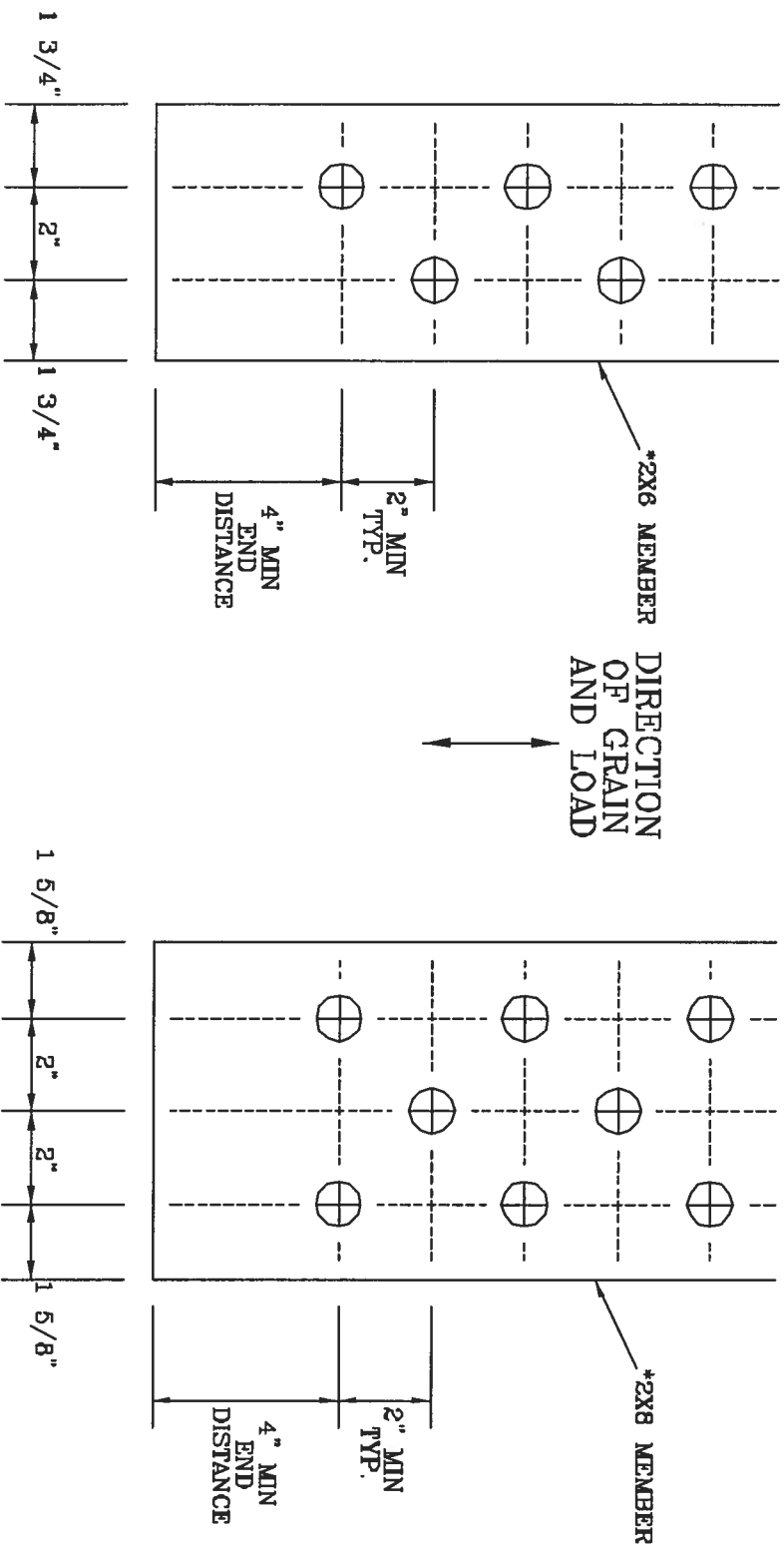
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TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

# 1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

- \* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
- BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICE BUILDING ELEMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS AND JOIST MANUFACTURERS ASSOCIATION, 1400 BY 4TH AVENUE, SUITE 200, DENVER, CO 80202. THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S  
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1400 BY 4TH AVENUE  
DENVER BRIDGE, FL 33444-2161

No: 34889  
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSR1103
BC LL	PSF	ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

# TRULOX CONNECTION DETAIL

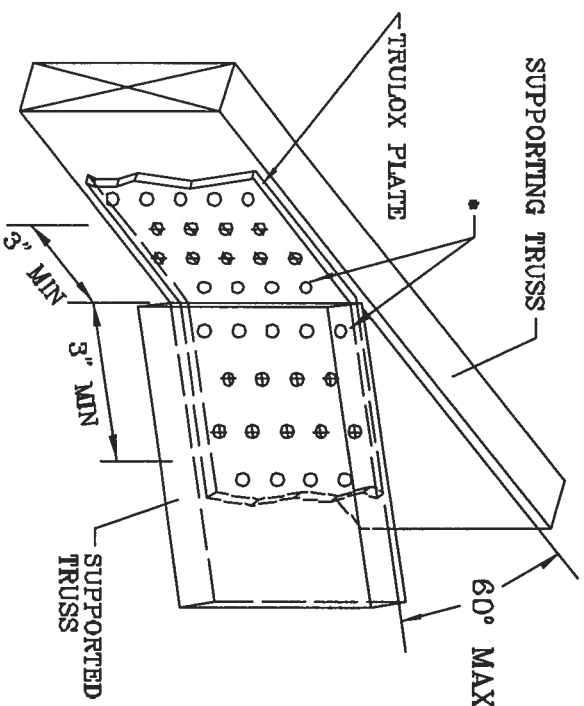
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

\* NAILS MAY BE OMITTED FROM THESE ROWS.

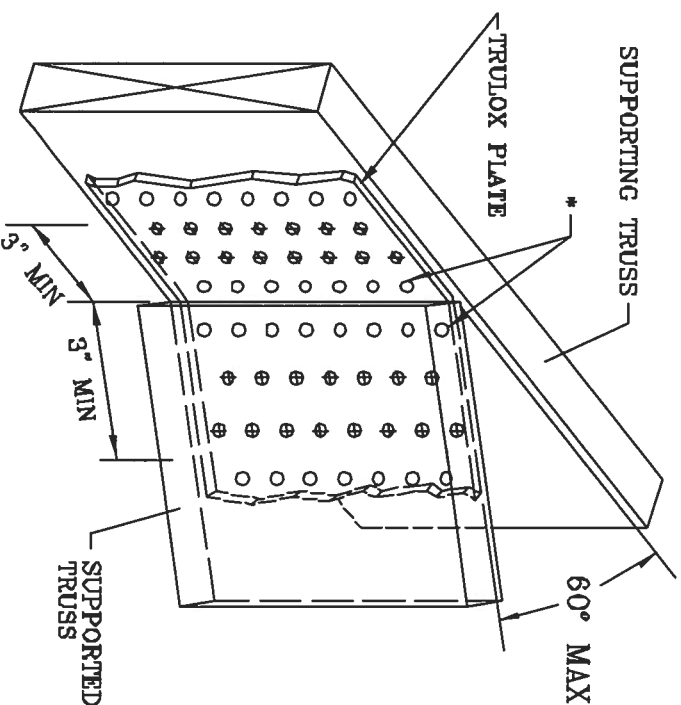
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	16	990#



THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,988/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO AISC 3-02 (BUILDING DEPARTMENT SAFETY DEPARTMENT, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 380 JONATHAN DR., SUITE 800, MIDDLEBURY, VT 05750) AND VITA (VEDO TRUSS COMPANY OF AMERICA, 6500 ENTERPRISE LN, MARIETTA, VT 05750) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**JULIUS LEE'S**  
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE  
DECATUR, GA, 30044-2018

No. 34859  
STATE OF FLORIDA

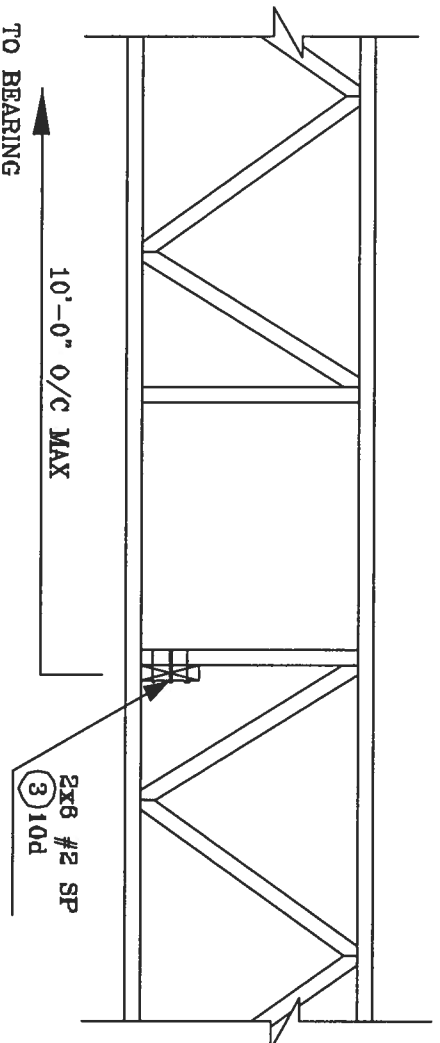
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DATE 11/26/03

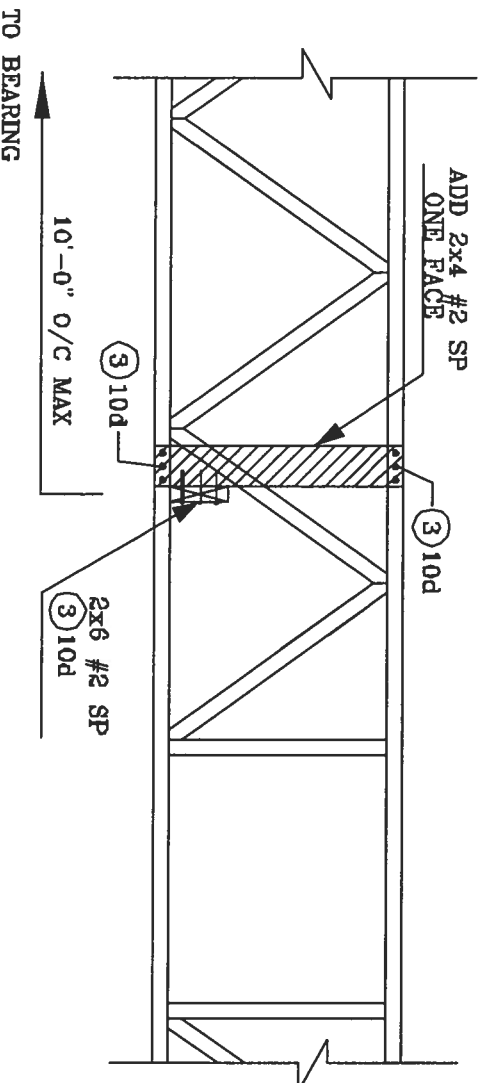
DRWG CNTRULOX1103

-ENG JL

**STRONG BACK DETAIL  
SYSTEM-42 OR FLAT TRUSS**



**ALTERNATE DETAIL FOR  
STRONG BACK WITH VERTICAL  
NOT LINING UP**



**JULIUS LEE'S**  
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No. 34860  
STATE OF FLORIDA