These truss designs rely on lumber values established by others.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3

MiTek, Inc.

314.434.1200

16023 Swingley Ridge Rd.

Chesterfield, MO 63017

RE: 0923-051 -

Site Information:

Customer Info: JERRY LERNER Project Name: FARRIS Model: .

Lot/Block: .

Subdivision: .

Address: ., .

City: .

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Design Program: MiTek 20/20 8.7

Wind Code: ASCE 7-16

Wind Speed: 130 mph

Roof Load: 27.0 psf

Floor Load: N/A psf

This package includes 2 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Truss Name Date No. Seal# T32077440 2 A02 T32077441 11/13/23

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Lee, Julius

My license renewal date for the state of Florida is February 28, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



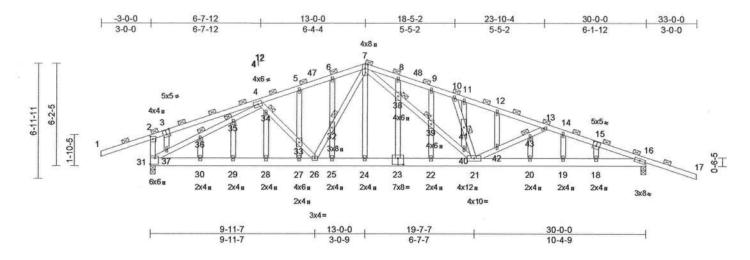
MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

November 13,2023

Job	Truss	Truss Type	Qty	Ply		and the control of th
0923-051	A01	Common Structural Gable	1	1	Job Reference (optional)	T32077440

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.72 S Oct 5 2023 Print: 8.720 S Oct 5 2023 MiTek Industries, Inc. Fri Nov 10 08:52:22 ID:ZnHHE0hFdR_Ud1ahmUqYySyTrbH-RfC?PsB70Hg3NSqPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:64.7

Plate Offsets (X, Y): [2:0-2-0,0-1-12], [3:0-2-8,0-3-4], [15:0-2-8,0-3-0], [23:0-4-0,0-4-8], [31:Edge,0-3-8]

Loading	(psf)	Spacing	4-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.94	Vert(LL)	-0.26	21-22	>999	240	MT20	244/190
TCDL	5.0	Lumber DOL	1.25	BC	0.97	Vert(CT)	-0.36	21-22	>999	180	CONTRACT MICHAEL	
BCLL	0.0*	Rep Stress Incr	NO	WB	0.62	Horz(CT)	0.07	16	n/a	n/a		
BCDL	2.0	Code	FBC2020/TPI2014	Matrix-MS							Weight 239 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 1-3,15-17:2x4 SP No.1 2x6 SP No.2 **BOT CHORD**

2x4 SP No.2 WEBS **OTHERS** 2x4 SP No.2

BRACING

TOP CHORD 2-0-0 oc purlins (2-11-4 max.), except end

verticals

(Switched from sheeted: Spacing > 2-0-0). **BOT CHORD** Rigid ceiling directly applied or 5-3-4 oc

bracing.

JOINTS 1 Brace at Jt(s): 7,

2, 32, 34, 35, 36,

38, 39, 41, 43

REACTIONS (size) 16=0-3-8, 31=0-3-8

Max Horiz 31=-207 (LC 10)

Max Uplift 16=-931 (LC 12), 31=-935 (LC 12)

Max Grav 16=1911 (LC 1), 31=1928 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

Tension

1-2=0/99, 2-4=-280/289, 4-5=-2291/1497, TOP CHORD

5-6=-2244/1533, 6-7=-2254/1600,

7-8=-3188/2190, 8-9=-3183/2126,

9-10=-3201/2093, 10-11=-3086/1983,

11-12=-3096/1949, 12-13=-3133/1891,

13-14=-3503/2148, 14-16=-3588/2145, 16-17=0/95. 2-31=-613/575

BOT CHORD

30-31=-1077/2134, 29-30=-1077/2134, 28-29=-1077/2134, 27-28=-1077/2134,

26-27=-1077/2134, 25-26=-913/1996,

24-25=-913/1996, 22-24=-913/1996, 21-22=-913/1996, 20-21=-1872/3343,

19-20=-1872/3343, 18-19=-1872/3343,

16-18=-1878/3350

WEBS

4-34=-52/262 33-34=-219/137 26-33=-188/173, 26-32=-245/467, 7-32=-239/454, 7-38=-936/1411, 38-39=-895/1351, 39-40=-900/1356,

21-40=-893/1352, 10-41=-226/156,

21-41=-347/244, 21-42=-592/421, 42-43=-496/349, 13-43=-520/373

31-37=-2409/1352, 36-37=-2365/1345,

35-36=-2326/1315, 4-35=-2489/1413,

6-32=-219/187, 25-32=-233/195,

5-33=-120/91, 27-33=-199/113,

28-34=-138/396, 29-35=-324/196

30-36=-67/109, 3-37=-208/154,

8-38=-203/179, 23-38=-112/118, 9-39=-99/84,

22-39=-107/93, 11-41=-148/117,

40-41=-16/21, 12-42=-210/165,

20-43=-68/82, 14-19=-11/108,

15-18=-182/142, 7-24=-221/318

NOTES

Unbalanced roof live loads have been considered for this design

Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=3.0psf, BCDL=1.2psf, h=15ft; B=45ft; L=30ft; eave=4ft; Cat. I; Exp B; Partially Enclosed; MWFRS (directional) and C-C Exterior(2E) -3-0-0 to 0-1-12, Interior (1) 0-1-12 to 13-0-0, Exterior (2R) 13-0-0 to 16-0-0, Interior (1) 16-0-0 to 33-0-0 zone; cantilever left and right exposed; end vertical left and right exposed: C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 (Actual dead loads used per ANSI/TPI-1)

Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1.

Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- The bottom chord dead load shown is sufficient only to cover the truss weight itself and does not allow for any additional load to be added to the bottom chord.
- Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 935 lb uplift at joint



Julius Lee PE No. 34869 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

November 13,2023



▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

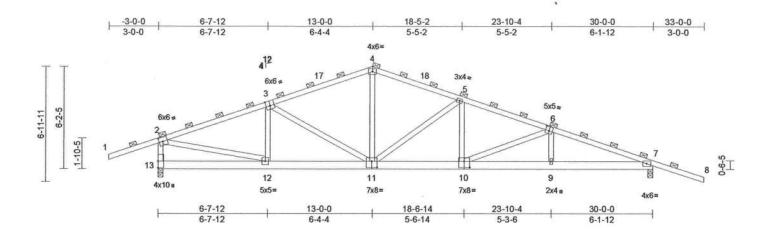


Job	Truss	Truss Type	Qty	Ply		
0923-051	A02	Common	9	1	Job Reference (optional)	T32077441

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.72 S Oct 5 2023 Print: 8.720 S Oct 5 2023 MiTek Industries, Inc. Fri Nov 10 08:52:24 ID:rb94anMK_8aKuu0VgJrCMlyTrbi-RfC?PsB70Hg3NSgPgnL8w3uITXbGKWrCDci7J4zJC?f

Page: 1



Scale = 1:64.7

Plate Offsets (X, Y): [2:0-2-11,0-3-0], [3:0-3-0,Edge], [6:0-2-8,0-3-0], [10:0-4-0,0-4-8], [11:0-4-0,0-4-8]

Loading	(psf)	Spacing	4-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.94	Vert(LL)	-0.23	9-10	>999	240	MT20	244/190
TCDL	5.0	Lumber DOL	1.25	BC	0.86	Vert(CT)	-0.31	9-10	>999	180	Court of the Court	
BCLL	0.0*	Rep Stress Incr	NO	WB	0.95	Horz(CT)	0.06	7	n/a	n/a		
BCDL	2.0	Code	FBC2020/TPI2014	Matrix-MS		7					Weight 187 lb	FT = 20%

LUMBER

2x4 SP No.1 *Except* 4-6:2x4 SP No.2 TOP CHORD

BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.2

BRACING

TOP CHORD 2-0-0 oc purlins (2-10-13 max.), except end

verticals

(Switched from sheeted: Spacing > 2-0-0). **BOT CHORD** Rigid ceiling directly applied or 5-3-14 oc

bracing.

REACTIONS 7=0-3-8, 13=0-3-8 (size)

Max Horiz 13=-207 (LC 10)

Max Uplift 7=-931 (LC 12), 13=-935 (LC 12) Max Grav 7=1911 (LC 1), 13=1928 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

Tension

2-13=-1887/1261, 1-2=0/99, 2-4=-2387/1462,

TOP CHORD

4-5=-2176/1436, 5-7=-3627/2135, 7-8=0/95

BOT CHORD

12-13=-58/226, 9-12=-1874/3354,

7-9=-1870/3350

2-12=-1260/2225, 4-11=-508/785,

5-11=-1059/707, 5-10=-165/441.

6-10=-641/438, 6-9=-93/207, 3-12=-483/337,

3-11=-441/266

NOTES

WEBS

- 1) Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=3.0psf; BCDL=1.2psf; h=15ft; B=45ft; L=30ft; eave=4ft; Cat. I; Exp B; Partially Enclosed; MWFRS (directional) and C-C Exterior(2E) -3-0-0 to 0-1-12, Interior (1) 0-1-12 to 13-0-0, Exterior (2R) 13-0-0 to 16-0-0, Interior (1) 16-0-0 to 33-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 (Actual dead loads used per ANSI/TPI-1)

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- The bottom chord dead load shown is sufficient only to cover the truss weight itself and does not allow for any additional load to be added to the bottom chord.
- Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 935 lb uplift at joint 13 and 931 lb uplift at joint 7.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

November 13,2023



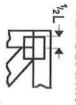
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Paraing indicated is to prevent buckling of individual truss web and/or chord membery licemporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property dramage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see "ANSI/TP1 Qualify Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



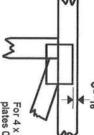
Symbols

PLATE LOCATION AND ORIENTATION



and fully embed teeth. Dimensions are in ft-in-sixteenths.

Apply plates to both sides of truss offsets are indicated. Center plate on joint unless x, y



edge of truss plates 0- 3:6" from outside For 4 x 2 orientation, locate

This symbol indicates the

connector plates. required direction of slots in

*Plate location details available in MiTek software or upon request

PLATE SIZE

4 × 4

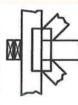
the length parallel to slots. to slots. Second dimension is width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Min size shown is for crushing only reaction section indicates joint Indicates location where bearings number/letter where bearings occur (supports) occur. Icons vary but

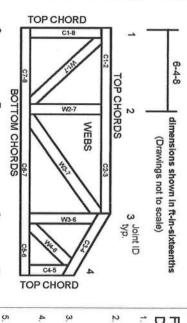
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction

DSB-22:

Guide to Good Practice for Handling, Installing, Restraining & Bracing of Meta Building Component Safety Information, Design Standard for Bracing.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1986, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other

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C

- joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1. Place plates on each face of truss at each
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- Install and load vertically unless indicated otherwise
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.