

Garage fire separations shall comply with the following:

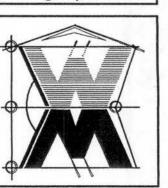
1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.

- Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
- 3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
- 4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

SOFTPIAN

ADAM'S

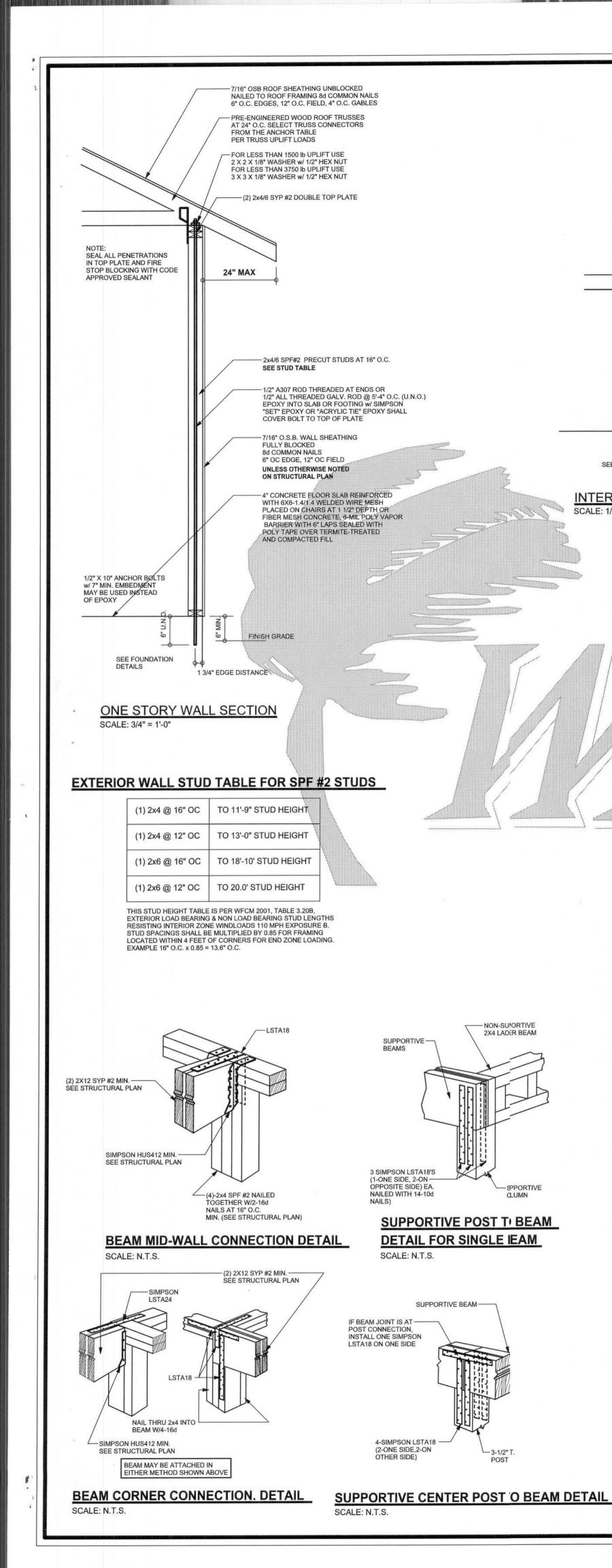
ONVILLIAM MYERS P.O. BOX 1513 LAKE CITY, FL 32056 (386) 758-8406 will@willmyers.net

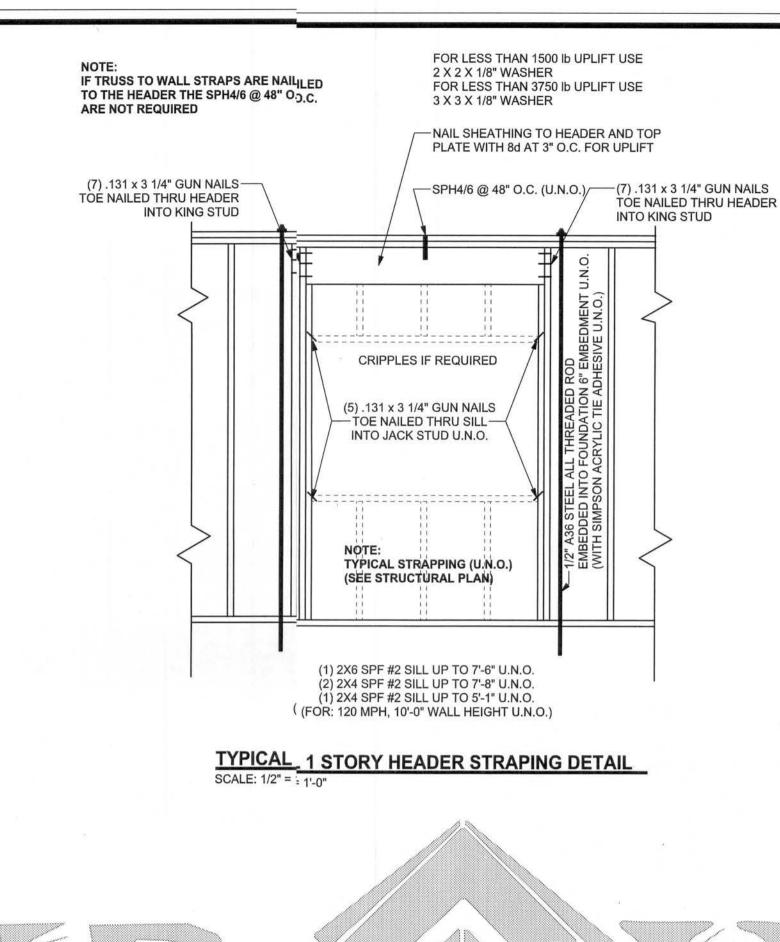


JOB NUMBER 070303

SHEET NUMBER

OF 3 SHEETS





- FOR LESS THAN 1500 Ib UPLIFT USE

2 X 2 X 1/8" WASHER w/ 1/2" HEX NU

FOR LESS THAN 3750 Ib UPLIFT USE

3 X 3 X 1/8" WASHER w/ 1/2" HEX NUT

SEE STRUCTURAL PLAN

PRE-ENGINEERED TRUSSES

-2 x 4/6 STUDS AT 16" O.C.

-(2) 2x4/6 SYP #2 DOUBLE TOP PLATE

- 1/2" A307 ROD THREADED AT ENDS OR

COVER BOLT TO TOP OF PLATE

-2x4/6 P.T. PINE SOLE PLATE

SEE FOUNDATION DETAILS

-NON-SUPORTIVE 2X4 LADER BEAM

SUPPORTIVE BEAM -

INTERIOR BEARING WALL

1/2" ALL THREADED GALV. ROD @ 5'-4" O.C. (U.N.O.) POXY INTO SLAB OR FOOTING W/ SIMPSON

> SIMPSON H2.5A U.N.O.-SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

-(2) 2X12 SYP #2 U.N.O.

-6X6 / 4X4 SYP #2 POST

w/ (12) - 16d & 5/8" x 10"

—SEE FOOTING DETAILS

4X4 / 6X6 PORCH POST DETAIL

ANCHOR BOLT

1 3/4" MIN. EDGE SPACING

SEE STRUCTURAL PLAN

'SET" EPOXY OR "ACRYLIC TIE" EPOXY SHALL

MANUFACTURER'S ENGINEERING TO PLATES TO RAFTER/TRUSS TRUSS CONNECTOR* < 420 3-8d 3-8d < 455 < 265 4-8d 4-8d < 360 < 235 4-8d 4-8d < 455 < 320 4-8d 4-8d < 415 5-8d 5-8d < 600 < 535 H2.5A 5-8d 5-8d < 950 < 820 8-8d 8-8d < 745 < 565 5-10d, 1 1/2" 5-10d, 1 1/2" < 1465 < 1050 13-8d 12-8d, 1 1/2" < 1465 15-8d < 1050 12-8d, 1 1/2" < 990 8-8d, 1 1/2" 8-8d, 1 1/2" < 760 < 655 6-10d 6-10d < 1470 < 1265 10-10d, 1 1/2" 2-10d, 1 1/2" < 1470 10-10d, 1 1/2" 2-10d, 1 1/2" < 1000 7-10d 1 1/2" 7-10d 1 1/2" < 1450 < 1245 12-10d 1 1/2" 12-10d 1 1/2" < 2900 < 2490 2 - HTS24 < 2050 < 1785 LGT2 14 -16d 14 -16d HEAVY GIRDER TIEDOWNS* TO FOUNDATION 1-5/8" THREADED ROD < 3965 < 3330 22 -10d 12" EMBEDMENT < 10980 < 6485 HGT-2 16 -10d 12" EMBEDMENT 2-5/8" THREADED ROD < 10530 < 9035 HGT-3 16 -10d 12" EMBEDMENT 2-5/8" THREADED ROD < 9250 < 9250 HGT-4 16 -10d 12" EMBEDMENT STUD STRAP CONNECTOR TO STUDS SSP DOUBLE TOP PLATE < 435 < 435 SSP SINGLE SILL PLATE 4 -10d < 825 DSP DOUBLE TOP PLATE 8 -10d < 825 < 600 DSP SINGLE SILL PLATE 2-10d 8 -10d < 885 6-10d, 1 1/2" < 1240 10-10d, 1 1/2 < 885 < 760 6-10d, 1 1/2" < 1240 < 1065 10-10d, 1 1/2" < 1235 < 1165 LSTA18 14-10d < 1235 < 1235 16-10d < 1030 < 1030 CS20 18-8d < 1705 < 1705 CS16 28-8d STUD ANCHORS* TO STUDS TO FOUNDATION < 1350 < 1305 LTT19 1/2" AB < 2310 < 2310 LTTI31 18-10d, 1 1/2' 1/2" AB < 2775 < 2570 2-5/8" BOLTS 5/8" AB < 4175 < 3695 18 - 16d 5/8" AB < 1400 < 1400 PAHD42 16-16d < 3335 ⊀ 3335 HPAHD22 16-16d < 2200 ABU44 12-16d 1/2" AB < 2300 < 2300 ABU66 12-16d

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 2320

WOOD SCREWS (MIN. 2" THREAD

PENETRATION MIN. 3/8" EDGE

-(4) #12X3/4" SS TEK SCREWS

-1/8"X3"X3"X1 1/2" ALUMINUM

-(4) #12X3/4" SS TEK SCREWS

-1/2" THD ROD, G90 OR HDG

w/ SIMPSON AT EPOXY

BRACKET 6053-T6

-1/2" WASHER, 410SS

-3"X3"X.092" ALUMINUM COLUMN, 6053-T6

-3"X3"X.092" ALUMINUM COLUMN, 6053-T6

-3"X3"X.092" ALUMINUM COLUMN, 6053-T6

-(4) #12X3/4" SS TEK SCREWS

(2) 1/4"X 2 1/2" TAPCON, 410SS

OR G95 w/ SS WASHERS 2 1/4"

MIN. EMBEDMENT (PER MIAMI

(1) #5 CONTINUOUS IN STEMWALL

DADE NOA 03-0114.03)

INTERSECTION w/ SLAB

MONOLITHIC w/ STEM WALL

-4" CONCRETE SLAB

OPTIONAL ALUMINUM PORCH POST & HEADER ANCHORS

SPACING PER NDS

-1/8"X3"X3"X1 1/2" ALUMINUM BRACKET w/ FLANGE WINGS

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO. ${f NAILS}$: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS HE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001, REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004					
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WIT MEAN ROOF HEIGHT NOT EXCEEDING LEAST HO ON UPPER HALF OF HILL OR ESCARPMENT 60FT	RIZONTAL D IN EXP. B. 3	IMEN OFT IN	SION	OR 60	FT; NC
SLOPE AND UNOBSTRUCTED UPWIND FOR 50x H	EIGHT OR 1	MILE	WHI	CHEV	ER IS LE
BUILDING IS NOT IN THE HIGH VELOCITY HURRIC	ANE ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEBRIS R	EGION				
1.) BASIC WIND SPEED = 110 MPH	0				
2.) WIND EXPOSURE = B					
3.) WIND IMPORTANCE FACTOR = 1.0					
4.) BUILDING CATEGORY = II					
5.) ROOF ANGLE = 10-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT = N/A (E	NCLOSED B	UILDI	NG)		
8.) COMPONENTS AND CLADDING DESIGN WINI	PRESSUR	ES (T	ABLE	R301.	2(2))
	Zone	Effec	tive W	ind Are	ea (ft2)
		1	0		100
	1	19.9	-21.8	18.1	-18.1
3 2	2	19.9	-25.5	18.1	-21.8
1 1 7	2 O'hg		-40.6		-40.6
2 2 2 1	3	19.9	-25.5	18.1	-21.8
4	3 O'hg		-68.3		-42.4
\3\ 4	4	21.8	-23.6	18.5	-20.4

S FRAME TO	
PHRAGM DETAIL	

6.)	MEAN ROOF HEIGHT = <30 FT					
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCL	OSED B	UILD	ING)		
8.)	COMPONENTS AND CLADDING DESIGN WIND PR	ESSURI	ES (T	ABLE	R301	.2(2)
	-2	Zone	Effective W		ind Area (ft	
			10		100	
		1	19.9	-21.8	18.1	-18.
	2 2	2	19.9	-25.5	18.1	-21.
	1 1 2	2 O'hg		-40.6		-40
5	2 2 2 1	3	19.9	-25.5	18.1	-21.
l fi	4 2 5	3 O'hg		-68.3		-42
	Y3 4	4	21.8	-23.6	18.5	-20.
	555	5	21.8	-29.1	18.5	-22.
	A STATE OF THE STA	Doors Wor (Zone	st Cas	е	21.8	-29
	5 2 3	8x7 Gar			19.5	-22
	2 5	16x7 Ga			18.5	-21.
	33 4 ZZ 25 ZZ					

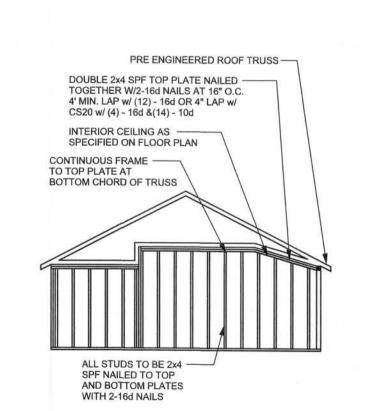
DESIGN	LOADS		
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)		
	30 PSF (SLEEPING ROOMS)		
	30 PSF (ATTICS WITH STORAGE)		
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)		
ROOF	20 PSF (FLAT OR <4:12)		
	16 PSF (4:12 TO <12:12)		
	12 PSF (12:12 AND GREATER)		

GRADE & SPECIES TABLE

18 - 16d

		Fb (psi)	E (10 ⁶ psi)	WIND LOADS PER F
2x8	SYP#2	1200	1.6	(ENCLOSED SIMPL MEAN ROOF HEIGH ON UPPER HALF O
2x10	SYP #2	1050	1.6	SLOPE AND UNOBS
2x12	SYP #2	975	1.6	BUILDING IS NOT IN
GLB	24F-V3 SP	2400	1.8	BUILDING IS NOT IN
100000	VENEZ GEROGE			1.) BASIC WIND S
LSL	TIMBERSTRAND	1700	1.7	2.) WIND EXPOSU
LVL	MICROLAM	2900	2.0	3.) WIND IMPORT.
PSL	PARALAM	2900	2.0	4.) BUILDING CAT
				5.) ROOF ANGLE
				6) MEAN BOOK H

2-5/8" AB



CONTINUOUS **CEILING DIA**

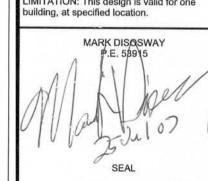
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	30 PSF (SLEEPING ROOMS)		
	30 PSF (ATTICS WITH STORAGE)		
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)		
ROOF	20 PSF (FLAT OR <4:12)		
	16 PSF (4:12 TO <12:12)		
	12 PSF (12:12 AND GREATER)		
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)		
SOIL BE	ARING CAPACITY 1000PSF		
NOT IN I	FLOOD ZONE (BUILDER TO VERIFY)		

INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.



Adam's Framing & Construction Spec House

Lot 2 Gerald Riggle S/D

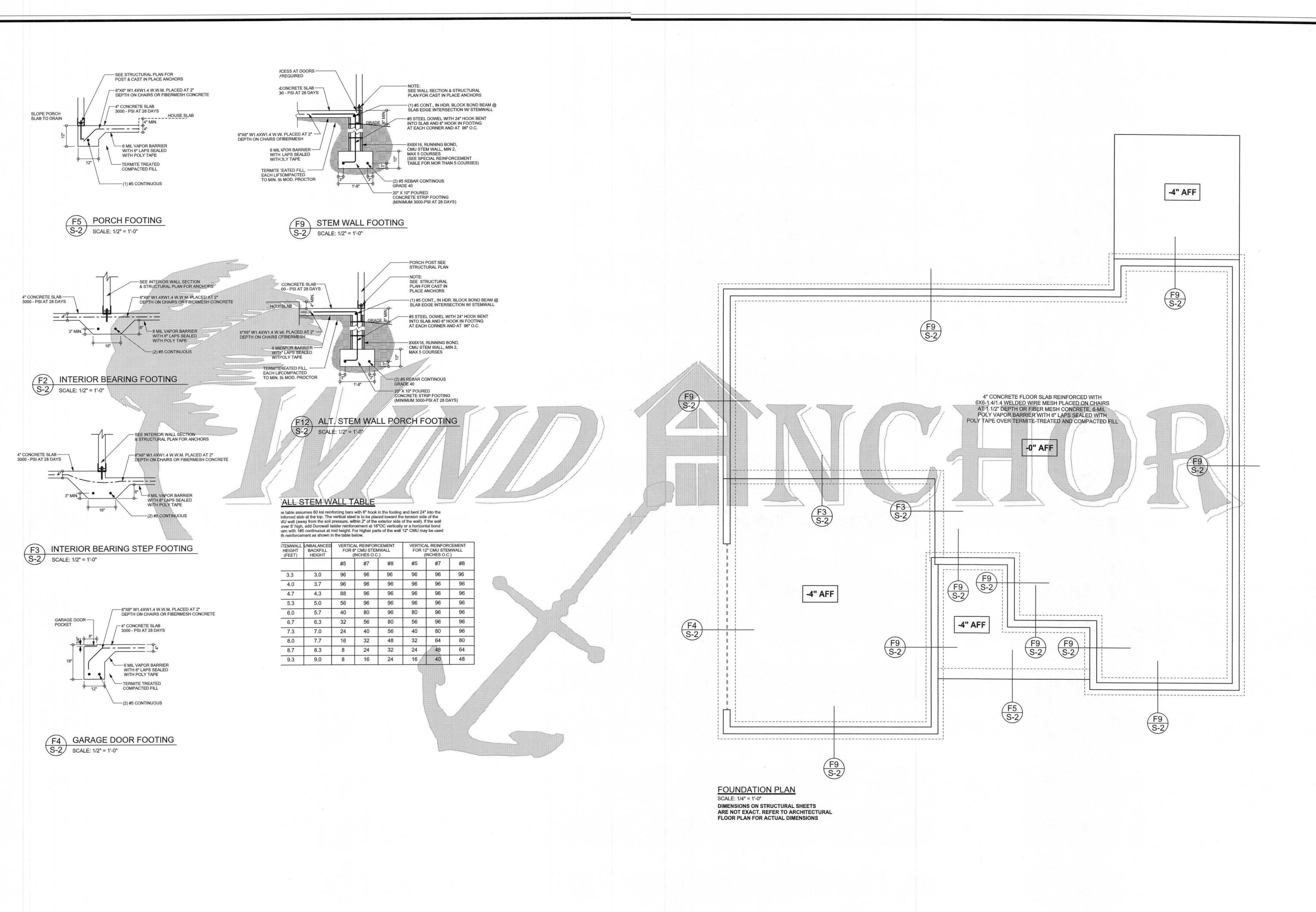
ADDRESS: Lot 2 Gerald Riggle S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

July 25, 2007 DRAWN BY: CHECKED BY: **David Disosway**

FINALS DATE: 25 / Jul / 07 JOB NUMBER: 706211 DRAWING NUMBER

OF 3 SHEETS



REVISIONS

SOFTPIAN ARCHITECTURAL DESIGN SOFTWAR

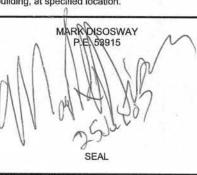
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Adam's Framing &
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Spec House Lot 2 Gerald Riggle S/D

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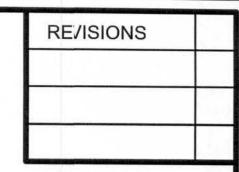
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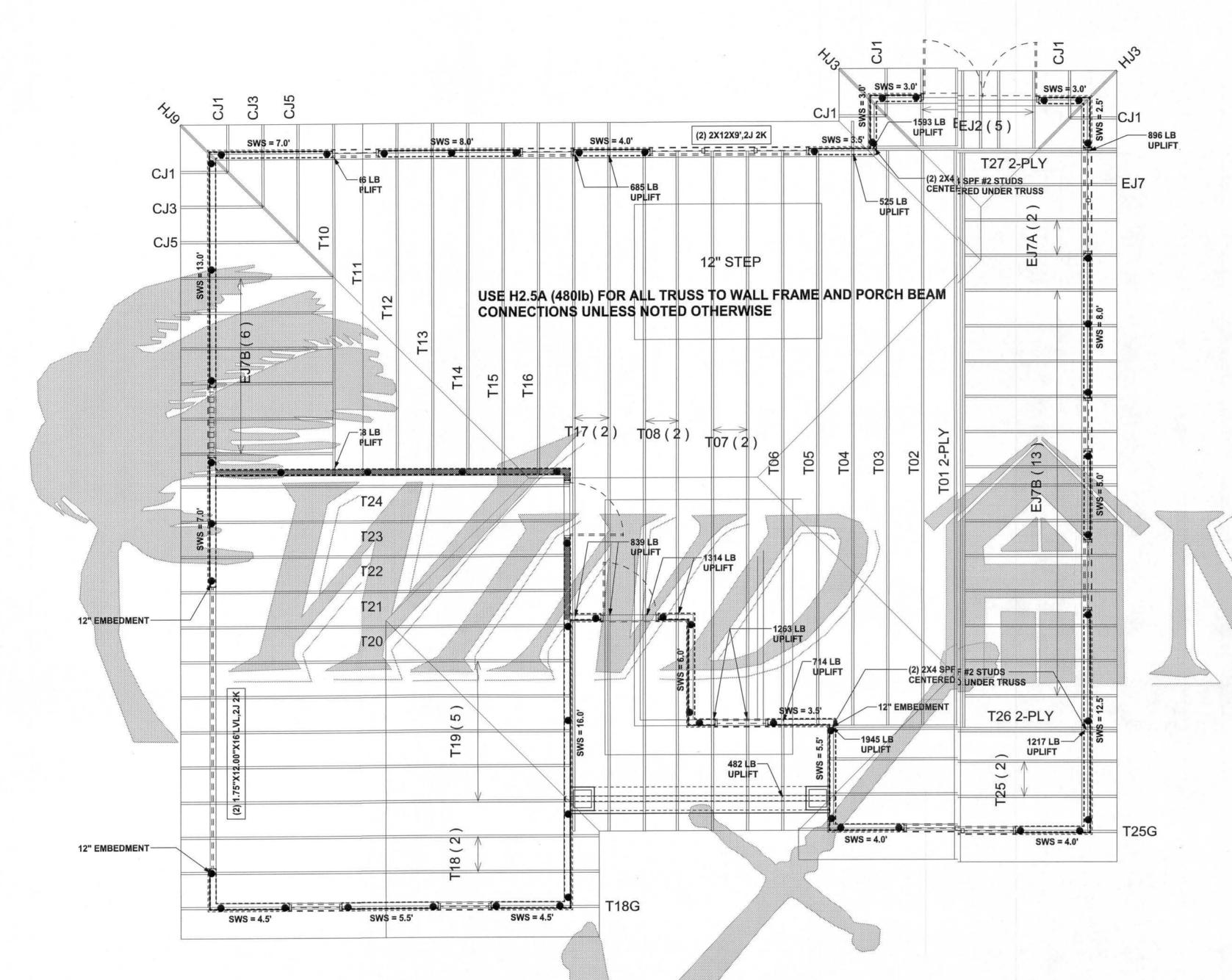
FINALS DATE: 25 / Jul / 07

> JOB NUMBER: 706211 DRAWING NUMBER

> > S-2 OF 3 SHEETS



SOFTPIAN RCHITECTURAL DESIGN SOFTWARE



SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

EACH SIDE (U.N.O.)

TRUSS PACKAGE

SN-1 ALL LOAD BEARING FRAM WALL & PORCH HEADERS SHALL BE A MINIMUM OF?) 2X12 SYP#2 (U.N.O.)

ALL LOAD BEARING FRAM WALL HEADERS

PERMANENT TRUSS BRAING IS TO BE INSTALLED AT LOCATIONS AS SHOWN (I THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO E RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE RUSS SUPPLIER, WITH THE SEALED

DIMENSIONS ON STRUCTRAL SHEETS SN-3

ARE NOT EXACT. REFER 3 ARCHITECTURAL FLOOR PLAN FOR ACTUADIMENSIONS

SN-2 SHALL HAVE (1) JACK STD & (1) KING STUD

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

- INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

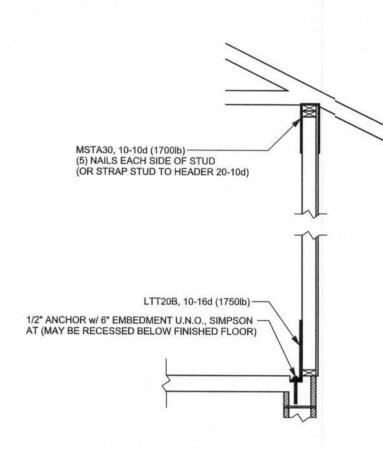
HEADER LEGEND

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) ----NUMBER OF KING STUDS (FULL LENGTH) -NUMBER OF JACK STUDS (UNDER HEADER) -SPAN OF HEADER SIZE OF HEADER MATERIAL -NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS REQUIRED ACTUAL
TRANSVERSE 36.8' 78.5'
LONGITUDINAL 32.1' 54.5'

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL SCALE: 1/2" = 1'-0"

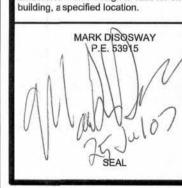
WINDLOAD ENGINEER: Mark Disosway, PE No.53t15, POB 868, Lake City, FL 32056, 38-754-5419

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Adam's Framing & Construction

Spec House Gerald Riggle S/D

ADDRESS: lot 2 Gerald Riggle S/D Clumbia County, Florida

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PRINTED DATE: July 25, 2007

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L244941

25 / Jil / 07 JOB NUMBER: 706211 DRAWING NUMBER

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