DATE MARCH 13, 2007

Columbia County Building Permit

PERMIT 000025610

This Permit Expires One Year From the Date of Issue

APPLICANT JOHN D. HARRINGTON PHONE 386-462-1509

ADDRESS 12523 NW US HIGHWAY 441 ALACHUA FL 32615

OWNER MICHAEL SWAYZE PHONE 352-494-8209

ADDRESS 152 SW ATLAS DRIVE FT. WHITE FL 32038

CONTRACTOR JOHN D. HARRINGTON PHONE 386-462-1509

LOCATION OF PROPERTY 47S, TL ON 27, TR ON CR 138, TR ON BOBCAT RD, TL ON 152ND AVE, 2ND LOT ON LEFT

TYPE DEVELOPMENT SFD ESTIMATED COST OF CONSTRUCTION 140,000

HEATED FLOOR AREA 1676 TOTAL AREA 2170 HEIGHT 14 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 30 REAR 25 SIDE 25

NO. EX.D.U. 0 FLOOD ZONE XOUT DEVELOPMENT PERMIT NO.

PARCEL ID 19-7S-17-10024-085 SUBDIVISION SASSAFRAS ACRES

LOT 85 BLOCK PHASE UNIT TOTAL ACRES 1.68

EXISTING | | CGC038861 | J S Harrington | A

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

EXISTING | 07-194 | BK | JH | | |

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: NOC ON FILE. MH MUST BE REMOVED WITHIN 45 DAYS OF CO BEING ISSUED. ONE FOOT ABOVE THE ROAD

Check # or Cash 1031

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 420.00 CERTIFICATION FEE $ 21.70 SURCHARGE FEE $ 21.70

MISC. FEES $ _____ ZONING CERT. FEE $ 50.00 FIRE FEE $ _____ WASTE FEE $ _____

FLOOD ZONE DEVELOPMENT FEE $ 25.00 CULVERT FEE $ _____ TOTAL FEE 538.40

INSPECTORS OFFICE Mike Redden CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION. IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0702-56 Date Received 2/20 By JW Permit # 25610

Application Approved by - Zoning Official BLK Date 2.20.07 Plans Examiner OKJTH Date 2-23-07

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments * Existing MH must be removed within 45 days of CO being issued

NOC EH Deed or PA Site Plan Section 23.1 N/A State Road Info Parent Parcel # N/A Development Permit

Name Authorized Person Signing Permit John D Harrington Fax 386-462-1509

Address 12523 NW U.S. Hwy 441 Alachua FL 32615 Phone 386-462-5323

Owners Name Michael Swayze Phone 352-494-8209

911 Address 152 SW Atlas Dr Ft. White FL 32038

Contractors Name John D Harrington Phone 352-316-5320

Address 12523 NW U.S Hwy 441 Alachua FL 32615

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Wayland Engineering 8200 SW 16th Pl Gainesville FL 32607

Mortgage Lenders Name & Address Country-wide Home Loans Inc. 6400 Legacy Dr. Plano TX 750

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number R10024-085 19-7S-17 Estimated Cost of Construction 140,000.

Subdivision Name Sassafras Acres Lot 85 Block ___ Unit ___ Phase ___

Driving Directions Hwy 47 to Ft. White turn (L) on Hwy 27 go to CR 138 turn (R) turn (R) on Bobcat RD - go to 152nd Ave turn (L) - 2nd [redacted] lot on left.

Type of Construction Residential - SFD

Total Acreage 1.68 Lot Size ___ Number of Existing Dwellings on Property 1*

Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive)

Actual Distance of Structure from Property Lines - Front 30' Side 45 Side 85 Rear 70'

Total Building Height 14' Number of Stories 1 Heated Floor Area 1676 TOTAL 2,170 Roof Pitch 6-12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

John D Harrington

Owner Builder or Authorized Person by Notarized Letter

John D Harrington

Contractor Signature

STATE OF FLORIDA
COUNTY OF COLUMBIA

Contractors License Number CGC038861

Competency Card Number

Sworn to (or affirmed) and subscribed before me this 20 day of Feb 2007.

Personally known ___ or Produced Identification ___

NOTARY PUBLIC STATE OF FLORIDA
Jessica R. Harrington
Commission # DD607811
Expires: NOV. 14, 2010
BONDED THRU ATLANTIC BONDING CO., INC.

NOTARY STAMP/SEAL

Jessica R. Harrington

Notary Signature

(Revised Sept. 2006)

CK# 1031

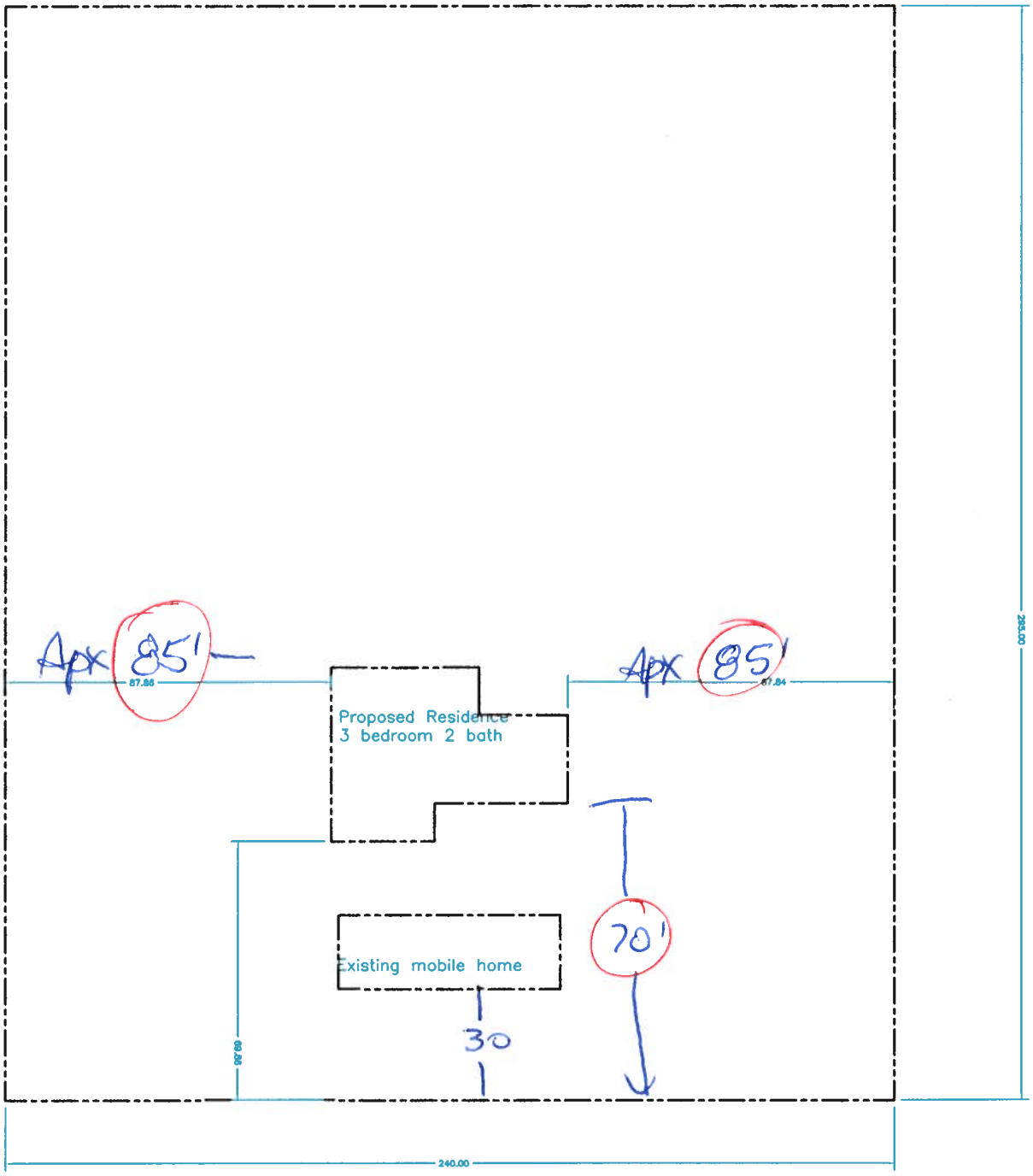
- JW - Left Message 2.27.07

@ CAM112M01 CamaUSA Appraisal System Columbia County
2/20/2007 17:02 **Legal Description Maintenance** **24176 Land 002 ***
Year T Property Sel **AG 000**
2007 R 19-7S-17-10024-085 **8521 Bldg 001 ***
152 ATLAS DR SW FT WHITE **150 Xfea 001 ***
HX SWAYZE HEATHER HETHERINGTON **32847 TOTAL B***

1	LOT 85 SASSAFRAS ACRES S/D. JOINS 10024-084. ORB 468-468,	2
3	643-753A, 813-1604, 909-2071,	4
5		6
7		8
9		10
11		12
13		14
15		16
17		18
19		20
21		22
23		24
25		26
27		28

Mnt **10/25/2000 TERRY**

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More

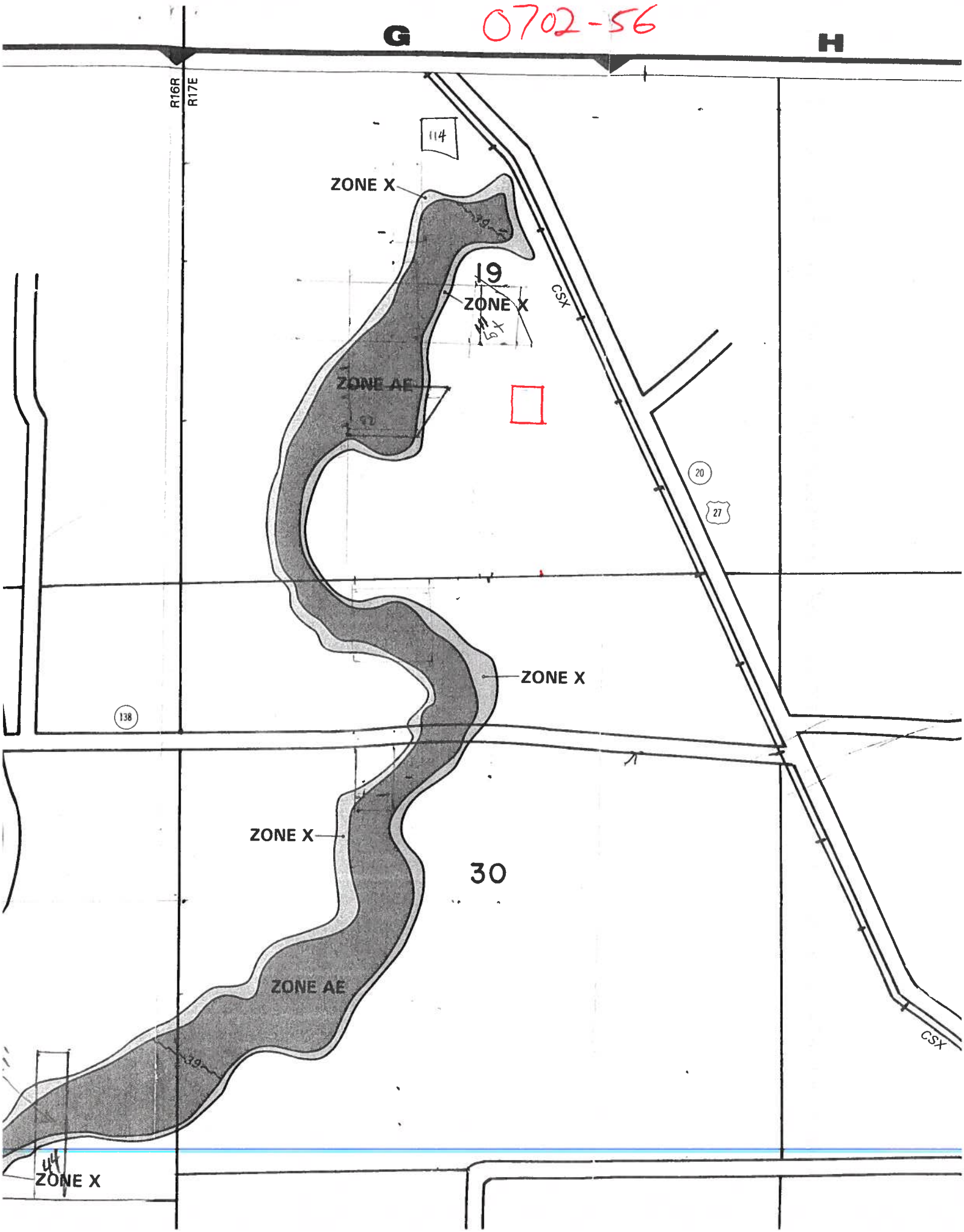


SCALE: NTS

152nd Ave. (SW Atlas Dr.)

HOMES BY HOUSE CRAFT, L.L.C.
12855 SW U.S. Hwy. 441 – Alachua, FL 32615
Phone: (386) 462-0320/FAX: (386) 462-1509

Job name:	SWAYZE
Model:	GULF
Type:	SITE PLAN
Location:	COLUMBIA CO
Drawn By:	J.D.H.
Designer:	J.D.H.
Checked By:	J.D.H.
Scale:	NTS
Date:	2/20/07
Sheet:	of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ZONE X
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ZONE AE
CSX
ZONE X

Record & Return to:
Title Resources of Gainesville
2135-C NW 40th Terrace
Gainesville, FL 32605

After Recording Return To:
COUNTRYWIDE HOME LOANS, INC.
MS SV-79 DOCUMENT PROCESSING
P.O.Box 10423
Van Nuys, CA 91410-0423

This document was prepared by:
JENNIFER WILSON
COUNTRYWIDE HOME LOANS, INC.

6400 LEGACY DR
PLANO, TX 75024

CLERK CIRCUIT COURT
COLUMBIA COUNTY, FLORIDA

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Donnie Dow Deputy Clerk
Date 02-01-2007

Inst:2007002523 Date:02/01/2007 Time:11:32
DC,P.DeWitt Cason,Columbia County B:1109 P:1530

[Space Above This Line For Recording Data]

SWAYZE
[Escrow/Closing #]

140953340
[Loan #]

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF ALACHUA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: (legal description of property, and street address if available)
SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.

Parcel ID Number: tbd

2. General description of improvement: SINGLE FAMILY DWELLING

3. Owner information:
   a. Name and address: MICHAEL AND HEATHER SWAYZE
   152 SW Atlas Drive, Fort White, FL 32038
   b. Interest in property: FEE SIMPLE
   c. Name and address of fee simple titleholder (if other than owner):

23991

1409533400000002D537

LOAN #: 140953340

4. Contractor:
   a. Name and address:
   House Craft Homes, LLC
   12523 NW US HWY 441, ALACHUA, FL 32615
   b. Phone number: (386) 462-5323
   c. Fax number (optional, if service by fax is acceptable): (386) 462-1509

5. Surety:
   a. Name and address: _____
   b. Amount of bond: $ _____
   c. Phone number: _____
   d. Fax number (optional, if service by fax is acceptable): _____

6. Lender:
   a. Name and address:
   COUNTRYWIDE HOME LOANS, INC.
   6400 Legacy Drive PTX-137 Plano, TX 75024
   b. Phone number: (800)729-4073
   c. Fax number (optional, if service by fax is acceptable): (800)219-6562

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided in section 713.13(1)(a)7., Florida Statutes:
   a. Name and address: _____
   b. Phone number: _____
   c. Fax number (optional, if service by fax is acceptable): _____

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lender's Notice as provided in Section 713.13(1)(b), Florida Statutes:
   a. Name and address: _____
   b. Phone number: _____
   c. Fax number (optional, if service by fax is acceptable): _____

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____

~~MICHAEL SWAYZE~~
Owner's Address: HEATHER SWAYZE
152 SW Atlas Drive
Fort White, FL 32038

Prepared by: JENNIFER WILSON

COUNTRYWIDE HOME LOANS, INC.

Branch #: 0000484
6400 LEGACY DR
PLANO, TX 75024
Phone: (800)556-6086
Br Fax No.: (888)898-1547

DATE: 01/29/2007
CASE #:
DOC ID #: 140953340
BORROWER: MICHAEL SWAYZE
PROPERTY ADDRESS: 152 SW ATLAS DRIVE
FORT WHITE, FL 32038

LEGAL DESCRIPTION EXHIBIT A

Lot 85, SASSAFRAS ACRES, a Subdivision as per plat recorded in Plat Book 4, Page(s) 8 and 8A of the Public Records of Columbia County, Florida.

Parcel Identification Number: R10024-085

MS HJ

Inst:2007002523 Date:02/01/2007 Time:11:32
_____DC,P.DeWitt Cason,Columbia County B:1109 P:1533

FHA/VA/CONV
Legal Description Exhibit A
2C404-XX (04/03)(d)

* 2 3 9 9 1 *

* 1 4 0 9 5 3 3 4 0 0 0 0 0 0 2 D 5 3 7 *

Inst:2007002523 Date:02/01/2007 Time:11:32
_____DC,P.DeWitt Cason,Columbia County B:1109 P:1532

LOAN #: 140953340

_____(Seal)
MICHAEL SWAYZE - Borrower
152 SW ATLAS DRIVE
FORT WHITE, FL 32038

_____(Seal)
HEATHER SWAYZE - Borrower
152 SW ATLAS DRIVE
FORT WHITE, FL 32038

_____(Seal)
- Borrower

_____(Seal)
- Borrower

STATE OF FLORIDA, ALACHUA **County ss:**

The foregoing instrument was acknowledged before me this January 29, 2007 by
MICHAEL SWAYZE AND HEATHER SWAYZE

who is personally known to me or who has produced DRIVER'S LICENSE(S)
as identification.

KATHERINE E. PREVATT
MY COMMISSION EXPIRES
July 7, 2008
#DD322111
Bonded Thru
Troy Fain-Insurance 800-385-7019
NOTARY PUBLIC, STATE OF FLORIDA

_____
Notary Public

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**SWAYZE RESIDENCE**	Builder:	**Homes by House Craft**
Address:	**152 SW Atlas Dr**	Permitting Office:	Columbia
City, State:	**Ft. White, Fl 32038-**	Permit Number:	25610
Owner:	**Michael & Heather Swayze**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 35.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1676 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAULT) 7a.	(Dble Default) 92.0 ft²	a. Electric Heat Pump	Cap: 35.0 kBtu/hr
b. SHGC:			HSPF: 7.70
(or Clear or Tint DEFAULT) 7b.	(Clear) 92.0 ft²	b. N/A	
8. Floor types			
a. Stem Wall	R=0.0, 1676.0ft²	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 40.0 gallons
a. Concrete, Int Insul, Exterior	R=6.0, 1195.0 ft²		EF: 0.92
b. Frame, Wood, Adjacent	R=6.0, 258.0 ft²	b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar	
10. Ceiling types		DHP-Dedicated heat pump)	
a. Under Attic	R=30.0, 1676.0 ft²	15. HVAC credits	CF,
b. Under Attic	R=30.0, 96.0 ft²	(CF-Ceiling fan, CV-Cross ventilation,	
c. N/A		HF-Whole house fan,	
11. Ducts		PT-Programmable Thermostat,	
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 115.0 ft	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	

Glass/Floor Area: 0.08

Total as-built points: 24373
Total base points: 26277

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: JD Haught

DATE: 2-19-07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: JD Haught

DATE: 2-19-07

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.4

The higher the score, the more efficient the home.

Michael & Heather Swayze, 152 SW Atlas Dr, Ft. White, Fl, 32038-

1.	New construction or existing	New		12. Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 35.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	3		b. N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1676 ft²		c. N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description	Area	13. Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default)	92.0 ft²	a. Electric Heat Pump	Cap: 35.0 kBtu/hr
	b. SHGC:				HSPF: 7.70
	(or Clear or Tint DEFAULT) 7b.	(Clear)	92.0 ft²	b. N/A	
8.	Floor types				
	a. Stem Wall	R=0.0, 1676.0ft²		c. N/A	
	b. N/A				
	c. N/A			14. Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 40.0 gallons
	a. Concrete, Int Insul, Exterior	R=6.0, 1195.0 ft²			EF: 0.92
	b. Frame, Wood, Adjacent	R=6.0, 258.0 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10.	Ceiling types			15. HVAC credits	CF,
	a. Under Attic	R=30.0, 1676.0 ft²		(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
	b. Under Attic	R=30.0, 96.0 ft²			
	c. N/A				
11.	Ducts				
	a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 115.0 ft			
	b. N/A				

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: JO Harrington Date: 2-14-07

Address of New Home: 152 SW Atlas Dr City/FL Zip: Ft White, FL

NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **152 SW Atlas Dr, Ft. White, Fl, 32038-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	NA
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	✓
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **152 SW Atlas Dr, Ft. White, Fl, 32038-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	40.0	0.92	3		1.00	2635.00	1.00	7905.0
				As-Built Total:							**7905.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
9399	**8973**	**7905**	**26277**	**5605**	**10863**	**7905**	**24373**

PASS

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **152 SW Atlas Dr, Ft. White, Fl, 32038-** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**14302.1**	**Winter As-Built Points:**					**19628.4**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 35000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
			19628.4	1.000	(1.069 x 1.169 x 1.00)	0.443	1.000	10862.8
14302.1	**0.6274**	**8973.2**	**19628.4**	**1.00**	**1.250**	**0.443**	**1.000**	**10862.8**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **152 SW Atlas Dr, Ft. White, Fl, 32038-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1676.0	12.74	3843.4	Double, Clear	E	2.0	5.7	34.0	18.79	1.07	681.6	
				Double, Clear	E	2.0	5.7	25.0	18.79	1.07	501.2	
				Double, Clear	W	2.0	5.7	50.0	20.73	1.05	1085.6	
				Double, Clear	N	2.0	5.7	25.0	24.58	1.01	617.7	
				As-Built Total:				**134.0**			**2886.1**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	258.0	3.60	928.8	Concrete, Int Insul, Exterior			6.0	1195.0	5.15		6154.2	
Exterior	1195.0	3.70	4421.5	Frame, Wood, Adjacent			6.0	258.0	5.26		1356.3	
Base Total:	**1453.0**		**5350.3**	**As-Built Total:**				**1453.0**			**7510.6**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	22.0	11.50	253.0	Exterior Insulated				22.0	8.40		184.8	
Exterior	65.0	12.30	799.5	Exterior Insulated				43.0	8.40		361.2	
				Adjacent Insulated				22.0	8.00		176.0	
Base Total:	**87.0**		**1052.5**	**As-Built Total:**				**87.0**			**722.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1676.0	2.05	3435.8	Under Attic			30.0	1676.0	2.05 X	1.00	3435.8	
				Under Attic			30.0	96.0	2.05 X	1.00	196.8	
Base Total:	**1676.0**		**3435.8**	**As-Built Total:**				**1772.0**			**3632.6**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	0.0(p)	0.0	0.0	Stem Wall			0.0	1676.0	3.50		5866.0	
Raised	1676.0	0.96	1609.0									
Base Total:			**1609.0**	**As-Built Total:**				**1676.0**			**5866.0**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1676.0	-0.59	-988.8					1676.0	-0.59		-988.8	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **152 SW Atlas Dr, Ft. White, Fl, 32038-** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 22031.3			**Summer As-Built Points:**					**17975.8**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 35000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
			17976	1.00	(1.09 x 1.147 x 1.00)	0.263	0.950	5605.3
22031.3	**0.4266**	**9398.5**	**17975.8**	**1.00**	**1.250**	**0.263**	**0.950**	**5605.3**

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **152 SW Atlas Dr, Ft. White, Fl, 32038-** PERMIT #:

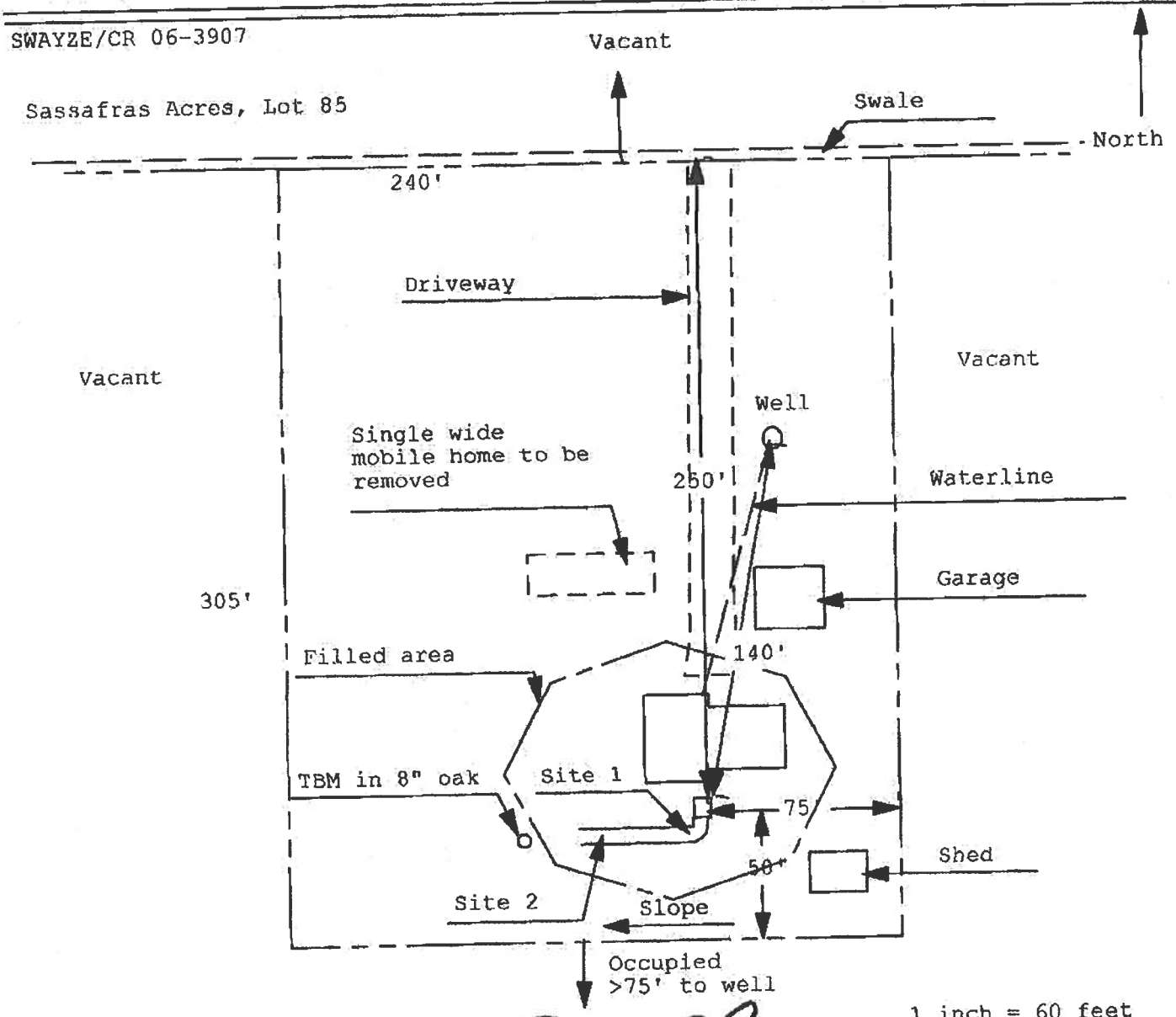
BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1676.0	20.04	6045.7	Double, Clear	E	2.0	5.7	34.0	42.06	0.83	1190.5	
				Double, Clear	E	2.0	5.7	25.0	42.06	0.83	875.4	
				Double, Clear	W	2.0	5.7	50.0	38.52	0.83	1606.5	
				Double, Clear	N	2.0	5.7	25.0	19.20	0.89	427.7	
				As-Built Total:				**134.0**			**4100.1**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	258.0	0.70	180.6	Concrete, Int Insul, Exterior			6.0	1195.0	0.85		1015.8	
Exterior	1195.0	1.70	2031.5	Frame, Wood, Adjacent			6.0	258.0	1.00		258.0	
Base Total:	**1453.0**		**2212.1**	**As-Built Total:**				**1453.0**			**1273.8**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	22.0	2.40	52.8	Exterior Insulated				22.0	4.10		90.2	
Exterior	65.0	6.10	396.5	Exterior Insulated				43.0	4.10		176.3	
				Adjacent Insulated				22.0	1.60		35.2	
Base Total:	**87.0**		**449.3**	**As-Built Total:**				**87.0**			**301.7**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1676.0	1.73	2899.5	Under Attic			30.0	1676.0	1.73 X 1.00		2899.5	
				Under Attic			30.0	96.0	1.73 X 1.00		166.1	
Base Total:	**1676.0**		**2899.5**	**As-Built Total:**				**1772.0**			**3065.6**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	0.0(p)	0.0	0.0	Stem Wall			0.0	1676.0	-4.70		-7877.2	
Raised	1676.0	-3.99	-6687.2									
Base Total:			**-6687.2**	**As-Built Total:**				**1676.0**			**-7877.2**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1676.0	10.21	17112.0					1676.0	10.21		17112.0	

07-194

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 12-SG-112450

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul Lloyd Date 3/5/07

Plan Approved X Not Approved ____ Date ____

By Sallie Maddox EHS 3-12-07 CPHU

Columbia CHD

Notes: ____

STRUCTURAL CALCULATIONS

FOR

SWAYZE RESIDENCE

LOCATION

w s e

WAYLAND

STRUCTURAL ENGINEERING

8200 SW 16th Place Gainesville, FL 32607

Phone/Fax 352-331-0727

FL COA #8236

Project Number
07026

February 12, 2007

Prepared For:
HOMES BY HOUSECRAFT, LLC
12523 NW US Highway 441
Alachua, FL 32615

TABLE OF CONTENTS

GREGORY S. WAYLAND, PE
FL PE #54396

2/12/07

FILE COPY

WAYLAND STRUCTURAL ENGINEERING			Date:	2/12/2007
Gregory S. Wayland, PE	FL PE #54396	FL COA #8236	By:	GSW
8200 SW 16th Place Gainesville, FL 32607	Ph/Fax 352-331-0727		Page:	1
Project Name: SWAYZE RESIDENCE		For:	Homes by Housecraft, LLC	
WSE Project Number: 07026			12523 NW US Hwy 441	
Project Location:			Alachua, Florida 32615	

STRUCTURAL SPECIFICATION

A. GENERAL

1. This STRUCTURAL SPECIFICATION shall be considered part of the contract documents for this project and shall be attached to the drawings prepared by: **HOMES BY HOUSECRAFT, LLC** Date:
2. Roof truss layout, uplift loads and gravity loads relied upon for design of supporting walls, lintels, headers, footings, etc. prepared by: **BUILDERS FIRST SOURCE** Date: 2/6/2007
3. Information and materials specified in this STRUCTURAL SPECIFICATION shall take precedence over that shown on the drawings.
4. Signing and sealing this document and/or the construction drawings by Wayland Structural Engineering certifies only the structural systems for this building, and is not a certification of the site plan, architectural, electrical, mechanical, plumbing or other systems that may be shown on the same drawing. WSE is not responsible for changes made to this document by others without written consent.
5. It is assumed that this building site is not located within a 100 year floodplain and is not designed for hydrostatic or moving water loads.

B. GOVERNING CODE

FLORIDA BUILDING CODE, 2006 SUPPLEMENT

C. DESIGN LOADS

1. Dead Loads		(Section 1606)
Roof Top Chord	10	psf
Roof Bottom Chord	10	psf
Floor	10	psf
2. Live Loads		(Section 1607)
Floor Live Load	40	psf
Balconies	60	psf
Attics w/o storage	10	psf
Attics w storage	20	psf
3. Roof Live Loads		(Section 1607.11.2)
12:12 pitch	12	psf
10:12 pitch	14	psf
8:12 pitch	16	psf
6:12 pitch	18	psf
Flat to 4:12 pitch	20	psf

4. Wind Loads (Section 1609)

Enclosure Classification	Enclosed	
Basic Wind Speed (3 sec. gust)	110	mph
Wind Importance Factor, Iw	1.0	
Exposure Category	B	
Internal Pressure Coefficients:	+0.18, -0.18	

Design Wind Pressures for Doors and Windows:

Opening Area (sf)	Inward Pressure (psf)	Outward Pressure (psf)
0-10	21.8	-29.1
11-20	20.8	-27.2
21-50	19.5	-24.6
51-100	18.5	-22.6

D. EARTHWORK

1. **General:**
   a. **OWNER/CONTRACTOR CAUTION:** A geotechnical or soil investigation has not been performed for this site. It is recommended that the Owner or Contractor employ the services of a geotechnical engineer to perform soil borings and provide recommendations for preparation of the soils specific to this building site, and confirm the soil type assumed in this specification. WSE has no knowledge of the on-site soils and therefore accepts no responsibility for their bearing capacity or performance.
   b. Bearing soil is therefore presumed to be sandy soil with no organics, peat, clay, expansive clays, or boulders.
   c. It is assumed that seasonal high groundwater table is well below footing bearing elevation.
   d. The allowable soil bearing pressure is assumed to be 2,000 pounds per square foot.
   e. If the Contractor or Building Inspector encounters organics, clays, silts, boulders or high grounwater levels during foundation excavation, engineer of record and/or geotechnical engineer shall be contacted and/or employed to assess conditions first hand and give direction for additional corrective work or modifications to the design that may need to be performed.
2. **Site Preparation:**
   a. Strip all trees, grasses, topsoil and other organics from building footprint. Use root rake or similar equipment to remove roots.
   b. Proofcompact existing grade with loaded dump truck or compactor to densify existing soils and identify soft or loose soils.
   c. If soft soils are encountered during proofcompaction, overcut unsuitable material and replace with well graded sand. (See 1e. above)
3. **Excavation:**
   a. Excavations are to be performed in accordance with current OSHA standards. Contractor is responsible for excavation safety.
   b. Compact all excavation bottoms to firm unyielding condition. See B.6.c. for compaction requirement.
4. **Footing Bearing:**
   a. All foundations are to bear on undisturbed sandy soil soil or compacted fill as described herein.
   b. Bottom of footings are to extend at least 12 inches below grade.
5. **Ground/Surface Water Control:**
   a. Excavation and backfill operations are to be maintained in a dry condition.
   b. Slope or crown building subgrades to promote run-off and prevent ponding.
   c. Surface and infiltrating water are to be removed by grading and pumping from sumps if required.
6. **Backfill and Compaction:**
   a. Use only clean, well graded sand with no more than 10% passing #200 sieve for fill and backfill within building footprint.
   b. Mechanically compact all backfill within building footprint in maximum 12" loose lifts to firm unyielding consistency.
   c. Suggest compact to 95% of maximum dry density per Modified Proctor Test, ASTM D-1557.
7. **Pest Control:**
   a. Treat all slab subgrades for termites in accordance with the Florida Building Code and local ordinances.
8. **Exterior Grading:**
   a. Exterior grade is to be kept at least 6 inches below wood siding and/or foam insulation.
   b. Slope exterior grade away from building to promote drainage.

STRUCTURAL SPECIFICATION (Continued)

E. CONCRETE

1. **General:** Comply with Florida Building Code, 2006 Supplement, Chapter 19, and ACI 301-99 Specifications for Structural Concrete.
2. **Concrete:**
   - a. Cement: ASTM C150, Type I Portland cement
   - b. Aggregate: ASTM C33, maximum aggregate size = 1 inch
   - c. Water/cement ratio: 0.50 maximum
   - d. Slump: 4 inches +/- 1 inch.
   - e. Air entraining: ASTM C 260, concrete is to be air entrained for mild exposure, 3 - 6%.

COMPRESSIVE STRENGTH, (psi) min. at 28 days	
Member	Strength
Footings, slabs-on-grade	2,500

3. **Reinforcing:** ASTM A615, Grade 40.

LAPS, BENDS, HOOKS			
Bar Size	Lap Length	Bend Diameter	Hook Length
#3	15"	2 1/4"	6"
#4	20"	3"	8"
#5	25"	3 3/4"	10"
#6	30"	4 1/2"	12"

BAR COVER	
Condition	Minimum Cover
Cast against and exposed to earth	3"
Exposed to earth or weather	1 1/2"
Not exposed to weather or earth	
Slabs, walls, joists	3/4"
Beams, columns (stirrups, ties)	1 1/2"

4. **Footings:**

BEARING WALL FOOTINGS			
Type	Width	Depth	Reinforcing
Stem wall	20"	10"	(3) #5

Corner bars: Provide 90 degree bend at all footing corners.

5. **Slabs-On-Grade:**
   - a. Thickness: 4 in.
   - b. Vapor retarder: 6 mil polyethylene, lap edges 6 inches.
   - c. Reinforcing: Welded Wire Reinforcing (WWR): ASTM A185, 6x6-W1.4xW1.4 (6x6-10/10) sheets, lap edges minimum 10 inches, support on chairs @ 3'-0" o.c. each way. WWR need not be installed on chairs if used in conjunction with fiber reinforcement. (Optional) Fibrous Reinforcing: ASTM C 1116, Fibermesh "Stealth" or "Inforce e3" polypropylene fibers by SI Concrete Systems or equivalent. Add to concrete mix at rate of 1.5 lb/cy.
   - d. Protection: Cure all slabs for 7 days using sprayed-on curing compound or continuous water sprinkling.
   - e. Slab joints: As concrete slabs cure and dry out, they will shrink causing cracks to form in surface of slab. Slab reinforcement is placed in slab to help limit width of cracks that do form. All slabs left exposed should be saw-cut in roughly 10'-0" squares.

F. MASONRY

1. **General:** Comply with the Florida Building Code, 2006 Supplement, Chapter 21 and ACI 530.1-02 Specifications for Masonry Structures.
2. **Masonry:** ASTM C90, Type 1, two core, normal weight units, 1,900 psi net area compressive strength.
3. **Mortar:** ASTM C270, Type M or S.
4. **Grout:** ASTM C476, fine or coarse grout, minimum 3,000 psi compressive strength at 28 days, 8-9 inch slump.
5. **Joint Reinf.:** (Optional) ASTM A951, truss type, hot-dip galvanized per ASTM A153, class B, 9 gauge wires spaced 16" o.c. vertically.
6. **Reinforcing:** ASTM A615, Grade 40. Provide clean-out at base of wall for pours over 5 feet high, lap bars 48 bar diameters.
   - a. Vertical: Provide #5 bars @ 7'-0" o.c. and at all corners and ends of walls. Provide one vertical #5 bar in first cell at all window and door jambs. Provide (2) #5 at garage door jambs. Provide (4) #5 vertical bars in all masonry columns.
   - b. Horizontal: Provide one #5 bar continuous in bond beam at top of wall.
   - c. Hooks: Provide standard 90 degree hook into footing at bottom and into bond beam at top of wall.
   - d. Corners: Provide 90 degree bend corner bars at all wall corners and intersections.
   - e. Lintels: Provide precast/pre-reinforced U-shaped concrete lintels over all openings sized for span and loading.

STRUCTURAL SPECIFICATION (Continued)

G. WOOD FRAMING

1. General: Comply with the Florida Building Code, 2006 Supplement, Chapter 23.

2. Trusses:
a. Wayland Structural Engineering is not responsible for design and detailing or installation of engineered wood roof trusses.
b. Truss engineering drawings to be signed and sealed by Professional Engineer registered in State of Florida.
c. Truss manufacturer to Engineer trusses to support dead, live and wind loads per Florida Building Code, 2004 or ASCE 7-02.
d. Engineer trusses to comply with ANSI/TPI 1 "National Design Standard for Metal Plate Connected Wood Truss Construction.
e. Comply with TPI HIB "Commentary and Recommendations for Handling, Installing and Bracing of Metal Plate Connected Wood Trusses."
f. Comply with TPI DSB "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses."
g. Truss spacing = 2'-0" o.c. maximium.

3. Fascia Board: No. 2, Spruce-Pine-Fir (SPF).

4. Sheathing:
a. Roof Sheathing: 15/32" thick, Oriented Strand Board (OSB), Sheathing Grade, Exposure 1.
Fasten with 8d common nails @ 6" o.c at panel edges, 12" o.c. along intermediate supports.
Lay panels perpendicular to supports, stagger joints one-half panel length. Provide "H" panel clips between panel supports.
Nail panel edges to fascia board.
b. Wall Sheathing: 15/32" thick, Oriented Strand Board (OSB), Sheathing Grade, Exposure 1.
Fasten with 8d nails @ 6" o.c at panel edges, 12" o.c. along intermediate supports.
Install panels vertically. Nail top edge to top plate. Provide solid blocking at all panel edges.

5. Fasteners:
a. Nails: Comply with Florida Building Code, 2006 Supplement, Table 2304.9.1, "Fastening Schedule."
b. Epoxy: Simpson "SET" or Hilti "HIT HY150" Epoxy Adhesive. Follow manufacturer's installation instructions exactly.
c. Bolts: ASTM A307, hot-dip galvanized, see plan for size and quantity.
d. Uplift Anchors & Ties: Simpson Strong-Tie,
e. Corrosion Protection: All fasteners exposed to weather or in contact with preservative treated wood shall be hot-dip galvanized to G185. For Simpson connectors, provide "Z-Max" coating.

H. WINDOWS, DOORS, SKYLIGHTS

1. Design: Wayland Structural Engineering is not responsible for the design, construction, or attachment of windows, doors or skylights. The building envelope is designed assuming a fully enclosed condition, therefore windows, doors and skylights must be designed to support the same wind pressures that walls and roofs are designed for.

2. Certification: Window, door and skylight manufacturer shall submit certification indicating that window or door units can adequately support design wind pressures for the specified wind zone as shown in section C.4. above.

3. Fastenings: Window, door and skylight manufacturer is to provide fastening information for attachment to supporting construction.

WAYLAND STRUCTURAL ENGINEERING
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Date: 2/12/2007
By: GSW
Page: 4

Project Name: **SWAYZE RESIDENCE**
WSE Project Number: 07026
Project Location:

For: Homes by Housecraft
12523 NW US Highway 441
Alachua, Florida 32615

A. UPLIFT CHECKS

1. BOND BEAM CHECK (upward bending)

Vertical bar spacing	s =	7.00 ft	
Gross uniform uplift load	ug =	-237 plf	(worst case from truss engineering)
Bond beam weight	wd =	42 plf	(one course high x 8 inches wide)
Calculated net uniform uplift load	un =	-195 plf	

		Calc'd	Supplied	
Maximum net shear (kips)	U =	**0.68**	**2.16**	**OK**
Maximum net moment (kip-in)	M =	**14.3**	**25.5**	**OK**

*** USE ONE COURSE HIGH x 8 INCH WIDE MASONRY BOND BEAM WITH (1) #5 CONTINUOUS TOP**

2. VERTICAL BAR CHECK (upward tension)

Allowable reinforcing tension Fs = 20,000 psi
Stress increase for wind Cw = 1.33

		* Calc'd Uplift (kips)	Vertical Reinforcing Quantity	Vertical Reinforcing Size (#)	**Supplied Uplift (kips)	
For typical common trusses,		1.365	1	5	8.161	OK
For girder trusses,	T01	0.921	1	5	8.161	OK
	T13	1.063	1	5	8.161	OK
	T16	2.647	1	5	8.161	OK
	T19	2.008	1	5	8.161	OK

* uplift values taken from truss engineering.
** includes stress increase for wind.

USE (1) #5 VERTICAL BAR @ 7'-0" O.C. MAX, (2) #5 AT GARAGE JAMB.

3A. WALL + FOOTING + SOIL WEIGHT CHECK (uplift at common trusses)

				Resisting Weight Supplied
Wall height	hw =	9.33 ft		
Wall thickness	tw =	8 in		
Wall unit weight	ww =	52 psf		
Bond beam height	hbb =	8 in	Bond beam	58
Bond beam unit weight	wbb =	130 psf	Wall	450
Footing thickness	tf =	10 in	Footing	208
Footing width	bf =	20 in	Soil (inside)	67
Footing depth below slab	df1 =	26 in	Soil (outside)	33
Footing depth below grade	df2 =	18 in	**Wr =**	**817** plf
Soil unit weight	ws =	100 psf		

Safety Factor Against Uplift	SF =	1.00	
Gross uniform uplift load	ug =	237 plf	
Required Resisting Weight, Wr = SF*ug		**237 plf**	**OK**

USE MINIMUM 8" THICK MASONRY WALL WITH 10"X20" FOOTING WITH (3) #5 BARS CONTINUOUS.

3B. WALL + FOOTING + SOIL WEIGHT CHECK (uplift at girder truss bearing points and columns)

Girder Truss or Column	Downward Load (lb)	Uplift Load (lb)	Adjacent Uplift Load (plf)	*Required Uplift Load (lb)	**Resisting Weight (lb)	Rqd Footing ***Weight (lb)	Rqd Concrete Volume (cf)	Footing Thickness (in)	Min. Square Footing (ft)
T01	2,066	921	216	2,433	6,533	-4100	-26	10	**0.0**
T13	2,445	1,063	237	2,722	6,533	-3811	-24	10	**0.0**
T16 left	5,946	2,264	245	3,979	6,533	-2554	-16	10	**0.0**
T16 right	7,194	2,647	245	4,362	6,533	-2171	-13	10	**0.0**
T17	1,862	844	205	2,279	6,533	-4254	-27	10	**0.0**
T19 left	4,190	1,693	205	3,128	6,533	-3405	-21	10	**0.0**
T19 right	5,450	2,008	205	3,443	6,533	-3090	-19	10	**0.0**

** Resisting weight equals weight of wall, footing, soil for 4 feet each side of load point.
*** Required footing weight equals weight required in addition to 10" x 20" footing.

B. LINTELS

1. TYPICAL LINTELS (with uniform load only)

	Unit Load (psf)	Trib. Width (ft)	Uniform Load (kips/ft)	Load Factor	Factored Uniform Load (kips/ft)	
Roof Dead Load	15	16.00	0.240	1.40	0.336	
Wall Dead Load	87	1.33	0.116	1.40	0.162	
Roof Live Load	16	16.00	0.256	1.70	0.435	
Roof Attic Load	10	16.00	0.160	1.70	0.272	
		w =	**0.772**	wu =	**1.205**	
*Uplift Load			0.253	1.60	0.405	(*from truss engineering)

Lintel Span	L =	**4.67**	**6.33**					ft
Unfactored Reaction	R =	1.80	2.44					kips
Unfactored Net Uplift Reaction	Unet =	0.43	0.58					kips
Factored Uplift Moment	Munet =	0.50	0.92					kip-ft
Factored Shear	Vu =	2.81	3.81					kips
Factored Design Shear	Vud =	**1.41**	**2.41**					kips
Factored Moment	Mu =	**3.29**	**6.04**					kip-ft
	Select Lintel	**TYPE A FILLED/ W 1 COURSE MASONRY**	**TYPE B FILLED/ W 1 COURSE MASONRY**					

4. 16'-0" GARAGE LINTEL (with no girder truss bearing)

	Unit Load (psf)	Trib. Width (ft)	Uniform Load (kips/ft)	Load Factor	Factored Uniform Load (kips/ft)	
Roof Dead Load	15	5.00	0.075	1.40	0.105	
Wall Dead Load	87	1.33	0.116	1.40	0.162	
Roof Live Load	16	5.00	0.080	1.70	0.136	
Roof Attic Load	10	5.00	0.050	1.70	0.085	
		w =	**0.321**	wu =	**0.488**	
*Uplift Load			0.102	1.60	0.163	(*from truss engineering)

Lintel Span	L =	**16.00**	ft
Unfactored Reaction	R =	2.57	kips
Unfactored Net Uplift Reaction	Unet =	0.26	kips
Factored Uplift Moment	Munet =	1.04	kip-ft
Factored Shear	Vu =	3.90	kips
Factored Design Shear	Vud =	**3.33**	kips
Factored Moment	Mu =	**15.62**	kip-ft

***USE TYPE E FILLED W/ ONE COURSE MASONRY**

WAYLAND STRUCTURAL ENGINEERING
Gregory S. Wayland, PE FL PE #54396 FL COA #8236
8200 SW 16th Place Gainesville, FL 32607 Ph/Fax 352-331-0727

Date:	2/12/2007
By:	GSW
Page:	6

Project Name:	**SWAYZE RESIDENCE**	For:	Homes by Housecraft
WSE Project Number:	07026		12523 NW US Highway 441
Project Location:	0		Alachua, Florida 32615

C. HORIZONTAL FORCES ON WALLS & TRUSSES

1. TYPICAL WALL

Wall height	9.33	ft	
Wind pressure	22.6	psf	(Zone 5)
Uniform lateral load	105	plf	(Top & Bottom of Wall)
Lateral force on Truss	**211**	lb/truss	(Based on 2 ft. spacing, perpendicular to wall)

WAYLAND STRUCTURAL ENGINEERING
Gregory S. Wayland, PE — FL PE #54396 — FL COA #8236
8200 SW 16th Place Gainesville, FL 32607 — Ph/Fax 352-331-0727

Date: 2/12/2007 | By: GSW | Page: 7

Project Name: **SWAYZE RESIDENCE**
WSE Project Number: 07026
Project Location: 0

For: Homes by Housecraft
12523 NW US Highway 441
Alachua, Florida 32615

D. LATERAL ANALYSIS

1. Building Data

Building Length	L =	64.00	ft
Building Width	B =	46.50	ft
Eave Height	he =	8.00	ft
Peak ht above eave	hp =	7.00	ft
Roof Slope		6	/12

2. Edge Zone

a = 0.10*B	4.65	ft
a = 0.40*h	3.20	ft
a =	3.20	ft
a = 0.04*B	1.86	ft
a = 3.00	3.00	ft
a =	**3.20**	**ft**

3. End Zone

z = 2*a = 6.40 **ft**

4. LONGITUDINAL DIRECTION

Exposure Category	B	
Adjustment Coefficient	1.00	

MWFRS Wind Pressures:

Wall Interior Zone	12.7	psf
Wall End Zone	19.2	psf
Roof Interior Zone	-5.9	psf
Roof End Zone	-10.0	psf

Wall Shear Force:

Interior	1.71	kips
End	0.98	kips
Total	2.70	kips

Roof Shear Force:

Interior	-1.01	kips
End	-0.20	kips
Total	-1.22	kips
Use	0.00	kips

Total Shear Force:

V = 2.70 **kips**

Roof Diaphragm Check:

Diaphragm shear	v =	21	plf
Allowable shear	v =	240	plf
	check	OK	

Roof Truss Lateral Load: (perpendicular to truss)

Load per truss — v = 42 lb/truss

Shear Wall Check:

Shear wall length	d =	12.67	ft
Shear wall height	h =	9.33	ft
Shear wall effective thickness	be =	2.50	in
Masonry strength	f'm =	1500	psi
Actual Shear	V =	1.35	kips
Overturning moment	M =	12.57	kip-ft
Actual shear stress	**fv =**	3.5	**psi**
	check	OK	

Allowable shear stress

M/V*d	0.74	
M/V*d >= 1.0	NO	
Allowable shear stress Fv1 =	42.13	psi
Fv2 =	46.86	psi
Fv =	**42.13**	**psi**

5. TRANSVERSE DIRECTION

MWFRS Wind Pressures:

Wall Interior Zone	17.7	psf
Wall End Zone	26.6	psf
Roof Interior Zone	-3.9	psf
Roof End Zone	-7.0	psf

Wall Shear Force:

Interior	3.62	kips
End	1.36	kips
Total	4.99	kips

Roof Shear Force:

Interior	-1.02	kips
End	-0.14	kips
Total	-1.16	kips
Use	0.00	kips

Total Shear Force:

V = 4.99 **kips**

Roof Diaphragm Check:

Diaphragm shear	v =	54	plf
Allowable shear	v =	240	plf
	check	OK	

Roof Truss Lateral Load: (perpendicular to truss)

Load per truss — v = 107 lb/truss

USE 15/32" OSB SHEATHING GRADE W/ 8d NAILS @ 6" O.C. EDGE, 12" O.C. FIELD

Shear Wall Check:

Shear wall length	d =	7.33	ft
Shear wall height	h =	9.33	ft
Shear wall effective thickness	be =	2.50	in
Masonry strength	f'm =	1500	psi
Actual Shear	V =	2.49	kips
Overturning moment	M =	23.26	kip-ft
Actual shear stress	**fv =**	11.3	**psi**
	check	OK	

Allowable shear stress

M/V*d	1.27	
M/V*d >= 1.0	YES	
Allowable shear stress Fv1 =	38.73	psi
Fv2 =	35.00	psi
Fv =	**35.00**	**psi**

USE 8" CMU W/ TYPE S MORTAR, FACE SHELL BEDDING, GROUT ONLY AT REINFORCED CELLS.

WAYLAND STRUCTURAL ENGINEERING			Date: 2/12/2007
Gregory S. Wayland, PE	FL PE #54396	FL COA #8236	By: GSW
8200 SW 16th Place Gainesville, FL 32607	Ph/Fax 352-331-0727		Page: 8
Project Name: **SWAYZE RESIDENCE**		For:	
WSE Project Number: **07026**			
Project Location:			

Truss Engineering: BUILDIERS FIRST SOURCE, BUNNELL, FL Date: 2/6/2007

Notes:

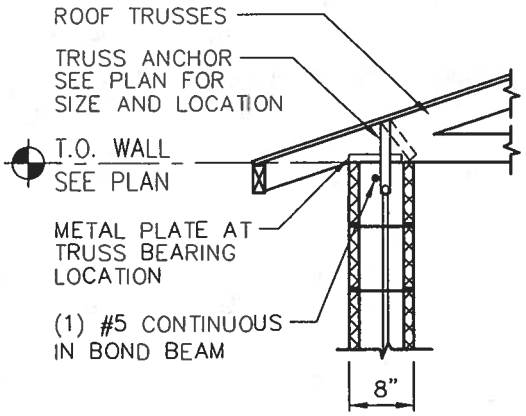
1. ALL ANCHOR ARE BY SIMPSON STRONG-TIE. ALTERNATE MANUFACTURERS ARE ALLOWED PROVIDED THE UPLIFT, LATERAL AND DOWNWARD LOADS SPECIFIED BELOW ARE EQUALED OR EXCEEDED.
2. TRUSS TO TRUSS CONNECTIONS ARE TO BE SPECIFIED AND SUPPLIED BY TRUSS MANUFACTURER.
3. USE "META16" FOR ALL TRUSSES BEARING ON MASONRY UNLESS OTHERWISE NOTED BELOW (U = 1450 lb).

ROOF TRUSS ANCHOR SCHEDULE

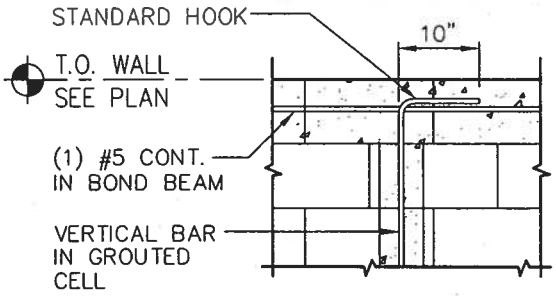
Truss	Bearing Point #1		Bearing Point #2		Bearing Point #3		Bearing Point #4	
Designation	Uplift (lb)	Anchor	Uplift (lb)	Anchor	Uplift (lb)	Anchor	Uplift (lb)	Anchor
T16	2264	MGT	2647	MGT				
T19	1633	HETA16	2008	MGT				

ANCHOR ALLOWABLE LOAD TABLE

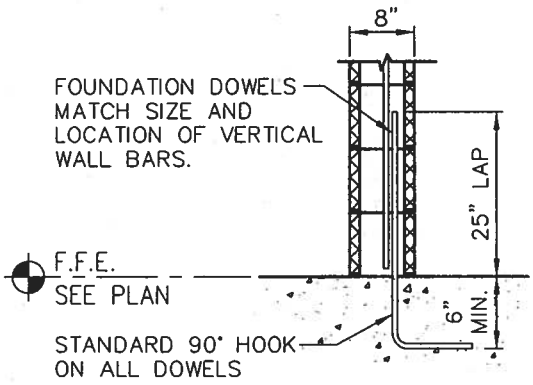
Simpson	Upward Loads		Lateral Loads		Downward
Anchor	(133)	(160)	F1	F2	Loads
META16	1450	1450	335	635	
HETA16	1805	1810	335	730	
MGT	3965	3965			



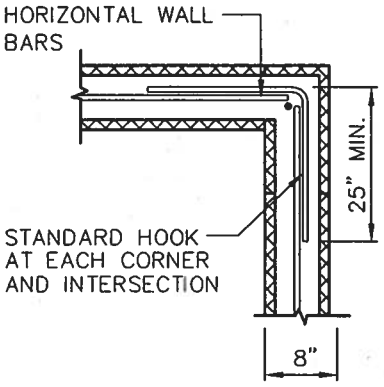
1 EXTERIOR TRUSS BEARING
1/2" = 1'-0"



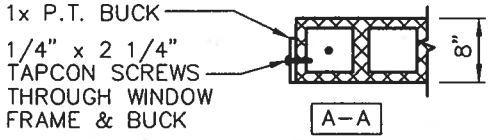
2 HOOK TO BOND BEAM
1/2" = 1'-0"

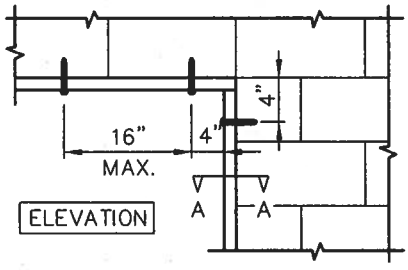


3 WALL BASE
1/2" = 1'-0"

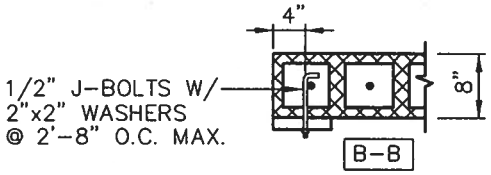


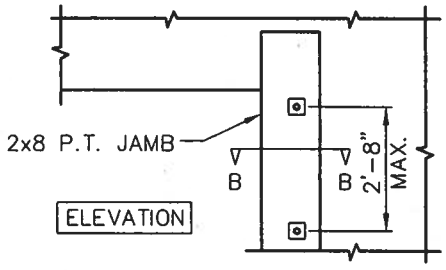
4 WALL CORNER REINFORCING
1/2" = 1'-0"





5 DOOR/WINDOW BUCK
1/2" = 1'-0"





6 GARAGE DOOR BUCK
1/2" = 1'-0"

wse
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8200 SW 16th Place
Gainesville, FL 32607
Phone (352) 331-0727
Fax (352) 331-0727

DWG. NAME:	TYPICAL CMU WALL DETAILS
SCALE:	VARIES
PROJECT NAME:	SWAYZE RESIDENCE
PROJECT NO:	07026
DRAWN BY:	GSW
DATE:	2/12/2007
DWG NO.	CMU1

LINTEL DESIGNATION

F = FILLED WITH GROUT/ U = UNFILLED

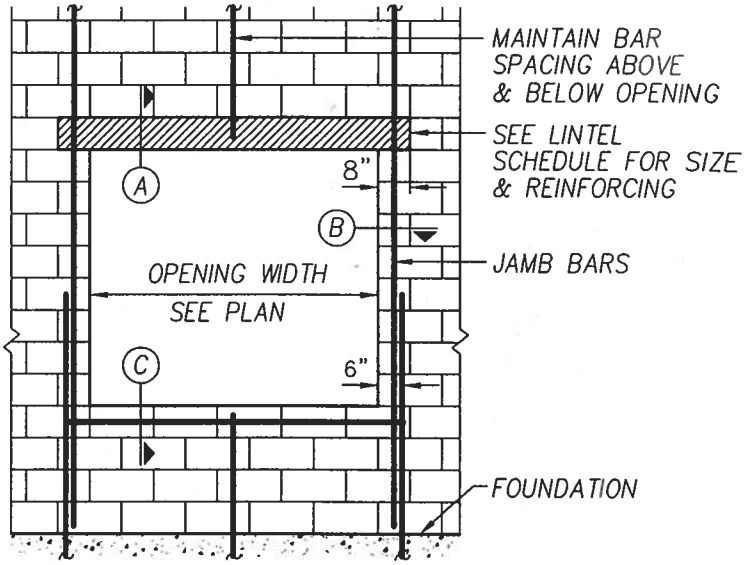
QUANTITY OF #5 FIELD ADDED REBAR AT BOTTOM OF LINTEL CAVITY

8F16–1B/1T

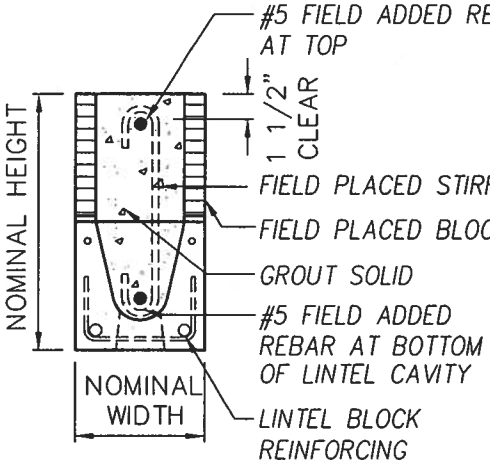
NOMINAL WIDTH

NOMINAL HEIGHT

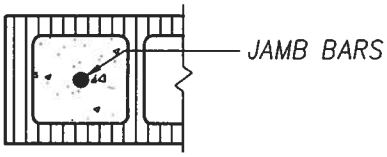
QUANTITY OF #5 FIELD ADDED REBAR AT TOP



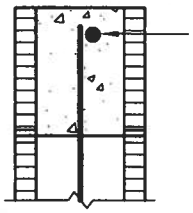
OPENING ELEVATION



HEAD SECTION A



JAMB SECTION B



SILL SECTION C

LINTEL SCHEDULE

OPENING WIDTH	LINTEL DESIGNATION	STIRRUPS	JAMB BARS	LOADS (PLF)	
				GRAVITY	UPLIFT
8" WIDE PRECAST U-LINTEL					
UP TO 5'-2"	8F16-1B/1T	NONE	(1) #5	4360	2093
5'-3" THRU 6'-10"	8F16-1B/1T	NONE	(1) #5	2661	1634
6'-11" THRU 9'-10"	8F16-1B/1T	NONE	(1) #5	1533	914
9'-11" THRU 13'-4"	8F16-1B/1T	NONE	(1) #5	1002	559
8" WIDE PRECAST RECESSED U-LINTEL					
UP TO 5'-2"	8RF6-1B/1T	NONE	(1) #5	4242	1900
5'-3" THRU 9'-0"	8RF14-1B/1T	NONE	(1) #5	1568	931
8" WIDE PRESTRESSED U-LINTEL					
13'-5" THRU 18'-8"	8F16-1B/1T	#3@8"	(2) #5	750	347
OR	8F20-1B/1T	NONE	(2) #5	1037	446
18'-9" THRU 23'-4"	8F24-1B/1T	#3@8"	(2) #5	884	416
OR	8F28-1B/1T	NONE	(2) #5	1092	491

1. PRECAST U-BLOCK BY CAST-CRETE OR EQUAL.
2. BEAR ALL LINTELS MINIMUM 8 INCHES EACH END.
3. FILL LINTELS WITH 3,000 PSI GROUT.

WSE
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Fax (352) 331-0727

DWG. NAME: TYPICAL CMU WALL DETAILS
SCALE: VARIES
PROJECT NAME: SWAYZE RESIDENCE
PROJECT NO: 07026
DRAWN BY: GSW
DATE: 2/12/2007
DWG NO. CMU2

yaw

TRUSS LAYOUT

Truss Number	Number of Straps	Type of strap	Number of Buckets	
CJ1	1	Meta 20	∅	
CJ3	1	"	∅	
CJ5	1	"	∅	
EJ7	1	"	∅	
HJ9	1	"	∅	
T01	2	"	∅	
T02	2	"	∅	
T03	2	"	∅	
T04	2	"	∅	
T05	2	MGT	∅	
T06	2	Meta 20	∅	
T07	2	"	∅	
T08	1	"	1	
T09	1	"	1	
T10	2	"	∅	
T11	2	"	1	HTV 28
T12	1	"	1	
T15	1	"	1	
T16	1	"	1	
T17	1	"	1	HETA-20
T18	3	"	∅	
T19	2	"	1	HHETA-20
All Buckets	By Truss Co.			
All straps	Simpson			

Project Information for:	L225956		
Builder:	HOMES BY HOUSECRAFT	Date:	2/7/2007
Lot:	N/A	Start Number:	**1235**
Subdivision:	152 SW ATLAS DR	SEI Ref:	L225956
County or City:	ALACHUA COUNTY		
Truss Page Count:	24		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

HARRINGTON,JOHN D. CGC038861
Address: 24113 NW OLD BELLAMY ROAD
HIGH SPRINGS, FL. 32643 Designer: **65**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0207071235	2/7/2007				
2	CJ3	0207071236	2/7/2007				
3	CJ5	0207071237	2/7/2007				
4	EJ7	0207071238	2/7/2007				
5	HJ9	0207071239	2/7/2007				
6	T01	0207071240	2/7/2007				
7	T02	0207071241	2/7/2007				
8	T03	0207071242	2/7/2007				
9	T04	0207071243	2/7/2007				
10	T05	0207071244	2/7/2007				
11	T06	0207071245	2/7/2007				
12	T07	0207071246	2/7/2007				
13	T08	0207071247	2/7/2007				
14	T09	0207071248	2/7/2007				
15	T10	0207071249	2/7/2007				
16	T11	0207071250	2/7/2007				
17	T12	0207071251	2/7/2007				
18	T13	0207071252	2/7/2007				
19	T14	0207071253	2/7/2007				
20	T15	0207071254	2/7/2007				
21	T16	0207071255	2/7/2007				
22	T17	0207071256	2/7/2007				
23	T18	0207071257	2/7/2007				
24	T19	0207071258	2/7/2007				

FEB 07 2007

Structural Engineering and Inspections, Inc. 16105 N. Florida Ave. Ste B Lutz, FL 33549 Thomas Miller. PE 56877. EB 9196

LATERAL TOE-NAIL DETAIL

ST-TOENAIL

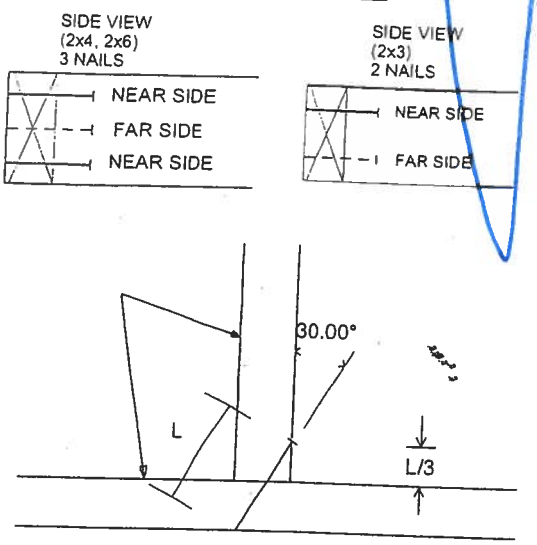
NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

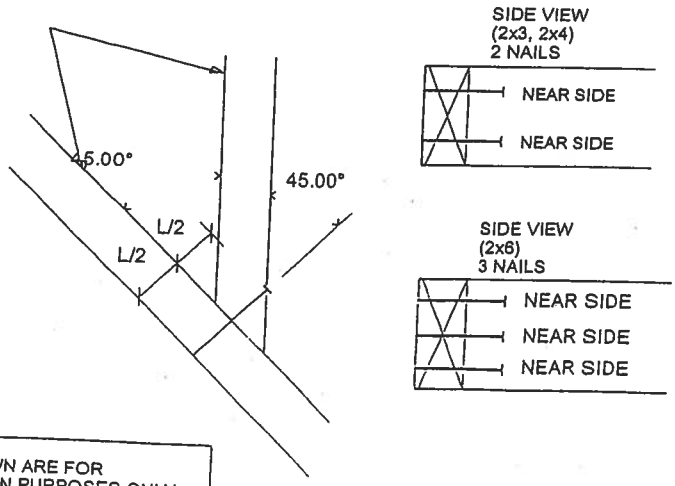
	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

SQUARE CUT



45 DEGREE ANGLE BEVEL CUT



VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.

FEB 07 2007

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Licensee Details

Licensee Information

Name:	**HARRINGTON, JOHN D (Primary Name)**
	HOMES BY HOUSE CRAFT, L.L.C. (DBA Name)
Main Address:	**24113 NW OLD BELLAMY RD HIGH SPRINGS Florida 32643**
County:	**ALACHUA**
License Mailing:	
LicenseLocation:	**24113 NW OLD BELLAMY RD HIGH SPRINGS FL 32643**
County:	**ALACHUA**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC038861**
Status:	**Current,Active**
Licensure Date:	**12/05/1986**
Expires:	**08/31/2008**

Special Qualifications	Qualification Effective
Bldg Code Core Course Credit	
Qualified Business License Required	02/20/2004

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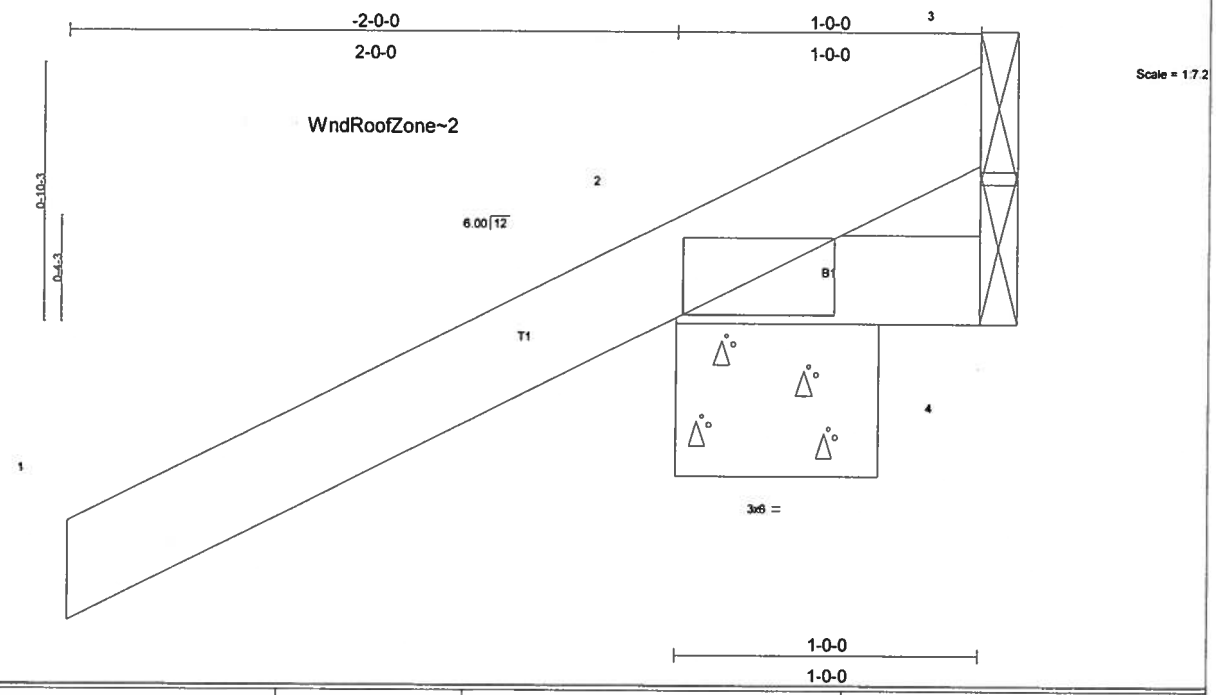
FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071235

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	CJ1	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:12 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=275/0-8-0, 4=14/Mechanical, 3=-100/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 3=-100(load case 1)
Max Grav 2=275(load case 1), 4=14(load case 1), 3=138(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-75/85
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2 and 100 lb uplift at joint 3.

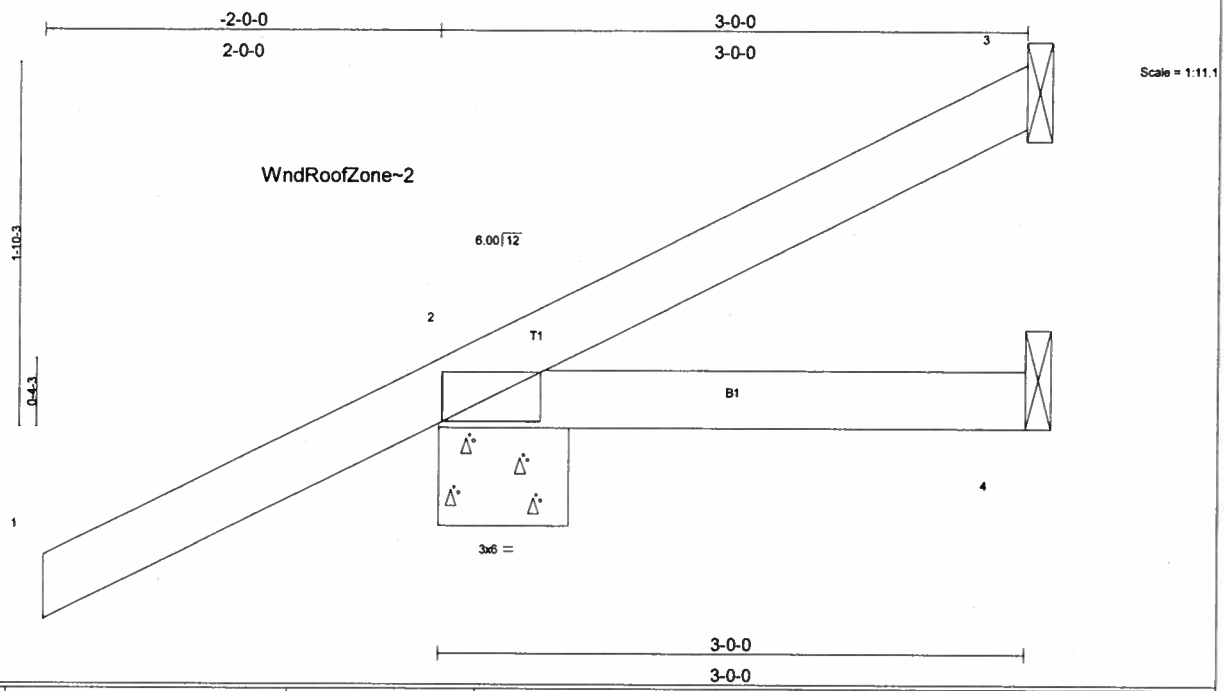
LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071236

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	CJ3	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:15 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=14/Mechanical, 2=292/0-8-0, 4=39/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-22(load case 6), 2=-222(load case 5)
Max Grav 3=18(load case 3), 2=292(load case 1), 4=39(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-63/8
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 3 and 222 lb uplift at joint 2.

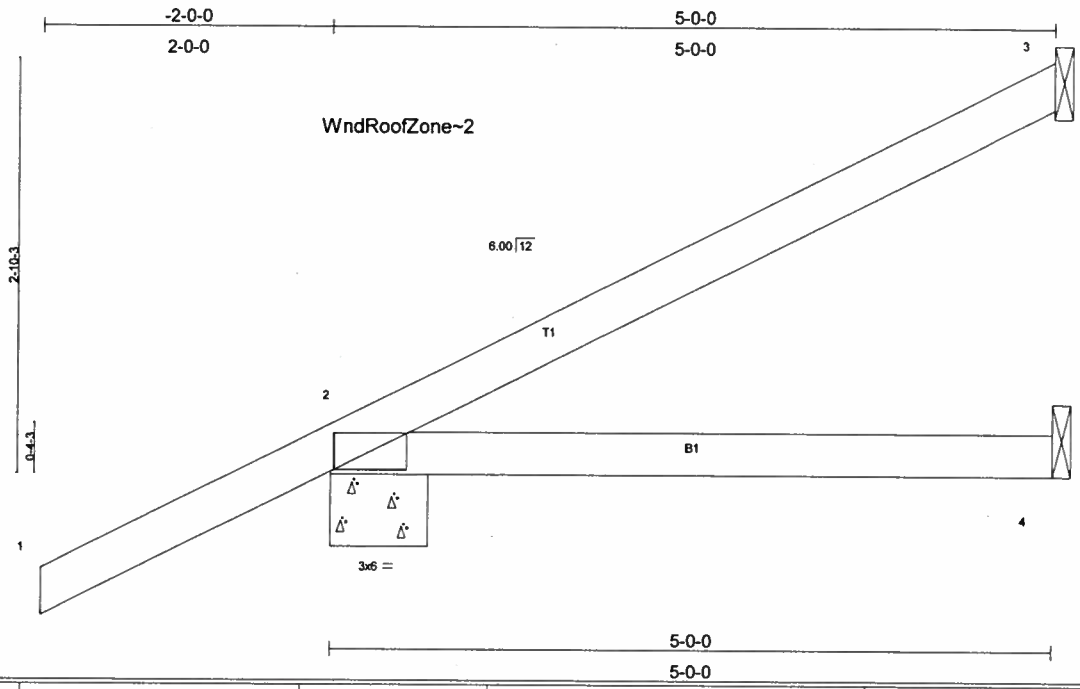
LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071237

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	CJ5	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:19 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.04	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=92/Mechanical, 2=351/0-8-0, 4=69/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-79(load case 5), 2=-212(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-84/32
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 3 and 212 lb uplift at joint 2.

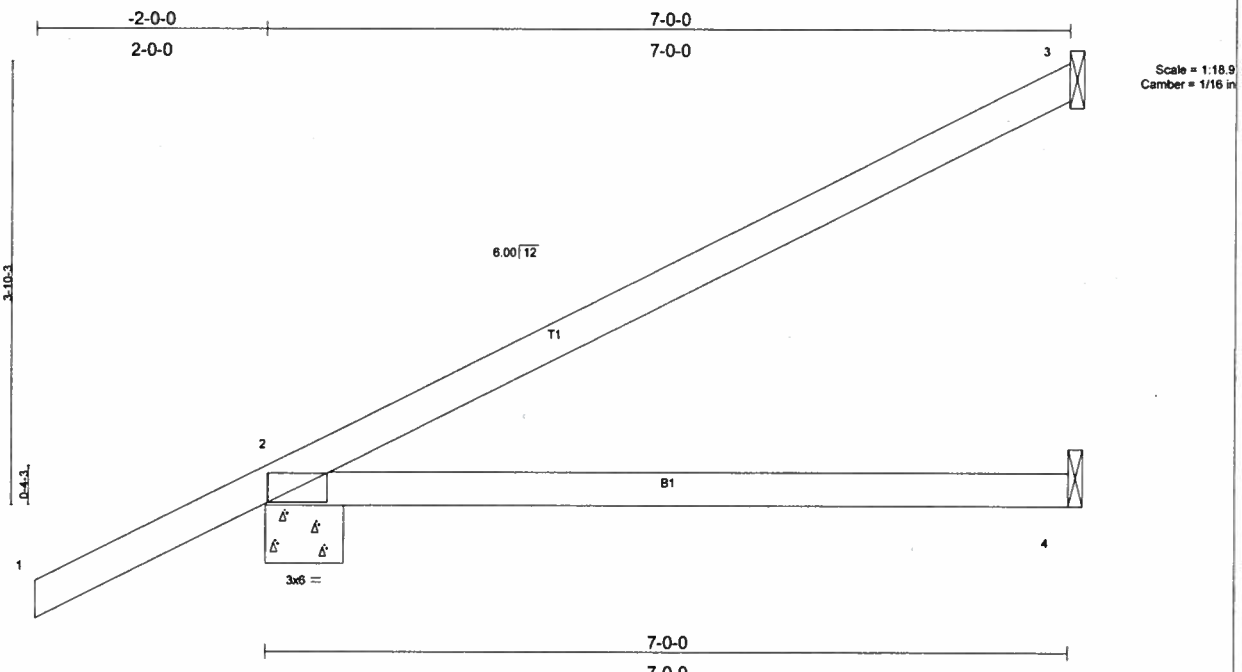
LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071238

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	EJ7	JACK	30	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:22 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.10	2-4	>765	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.17	2-4	>455	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=156/Mechanical, 2=426/0-8-0, 4=99/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-130(load case 5), 2=-221(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-94/56
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 3 and 221 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071239

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	HJ9	MONO TRUSS	6	1	Job Reference (optional)

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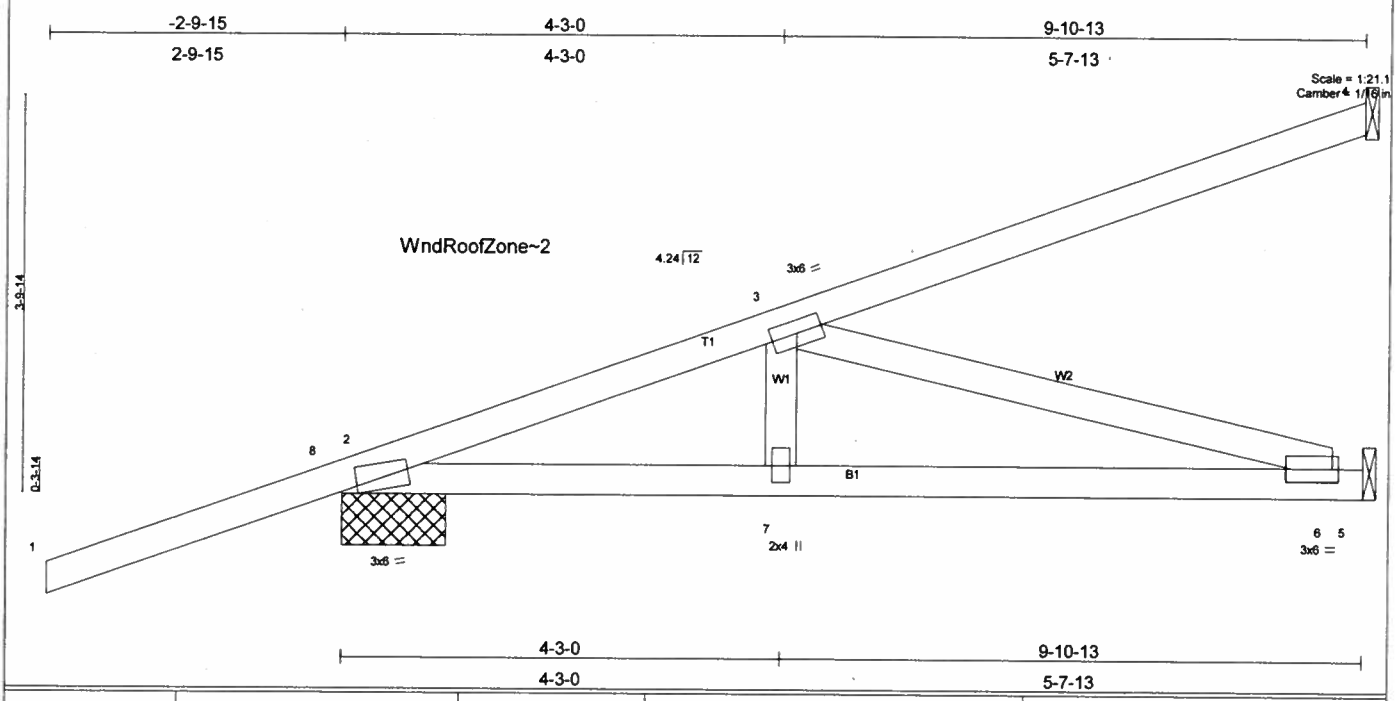

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.17	6-7	>681	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.45	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 2=533/1-0-1, 5=373/Mechanical
Max Horz 2=265(load case 2)
Max Uplift4=-231(load case 2), 2=-274(load case 2), 5=-59(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-8=0/45, 2-8=0/45, 2-3=-873/105, 3-4=-105/66
BOT CHORD 2-7=-294/805, 6-7=-294/805, 5-6=0/0
WEBS 3-7=0/180, 3-6=-837/306

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 274 lb uplift at joint 2 and 59 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-8=-54
Trapezoidal Loads (plf)
Vert: 8=0(F=27, B=27)-to-4=-134(F=-40, B=-40), 2=-3(F=14, B=14)-to-5=-74(F=-22, B=-22)

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071240

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T01	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:30 2007 Page 1

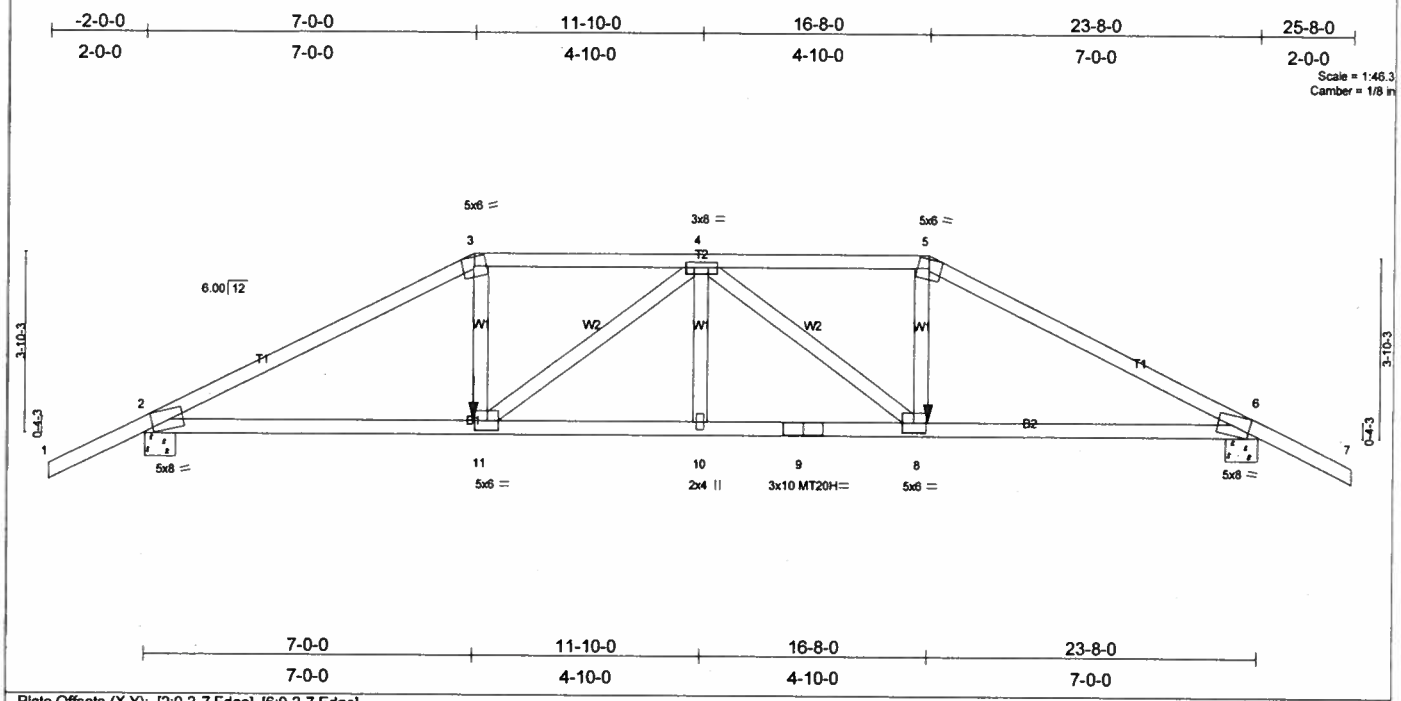

Plate Offsets (X,Y): [2:0-2-7,Edge], [6:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.21	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.33	10-11	>835	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.13	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-1 oc bracing.

REACTIONS (lb/size) 2=2066/0-8-0, 6=2066/0-8-0
Max Horz 2=-87(load case 5)
Max Uplift2=-921(load case 4), 6=-921(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-3665/1483, 3-4=-3217/1368, 4-5=-3217/1368, 5-6=-3665/1483, 6-7=0/48
BOT CHORD 2-11=-1271/3167, 10-11=-1534/3745, 9-10=-1534/3745, 8-9=-1534/3745, 6-8=-1230/3167
WEBS 3-11=-451/1259, 4-11=-765/398, 4-10=0/284, 4-8=-765/398, 5-8=-451/1259

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 921 lb uplift at joint 2 and 921 lb uplift at joint 6.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 16-8-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-113(F=-59), 5-7=-54, 2-11=-30, 8-11=-63(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 8=-539(F)

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

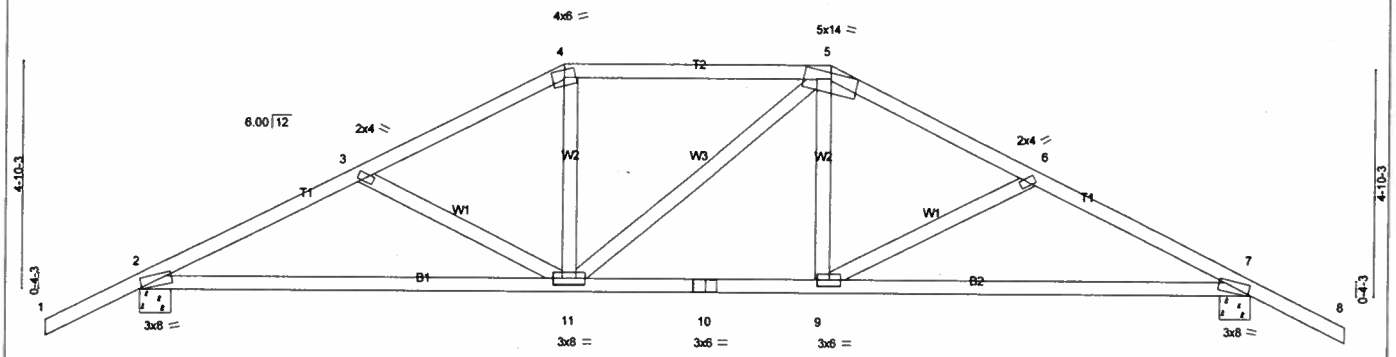
Dwg.#0207071241

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:33 2007 Page 1

-2-0-0 | 4-9-4 | 9-0-0 | 14-8-0 | 18-10-12 | 23-8-0 | 25-8-0
2-0-0 | 4-9-4 | 4-2-12 | 5-8-0 | 4-2-12 | 4-9-4 | 2-0-0

Scale = 1:46.3
Camber = 1/8 in



9-0-0 | 14-8-0 | 23-8-0
9-0-0 | 5-8-0 | 9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.16	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.27	7-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 118 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1092/0-8-0, 7=1092/0-8-0
Max Horz 2=-101(load case 6)
Max Uplift2=-432(load case 5), 7=-432(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-1568/463, 3-4=-1352/377, 4-5=-1175/383, 5-6=-1352/377, 6-7=-1568/463, 7-8=0/48
BOT CHORD 2-11=-371/1346, 10-11=-182/1174, 9-10=-182/1174, 7-9=-286/1346
WEBS 3-11=-207/179, 4-11=-39/303, 5-11=-103/105, 5-9=-31/303, 6-9=-207/179

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 2 and 432 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071242

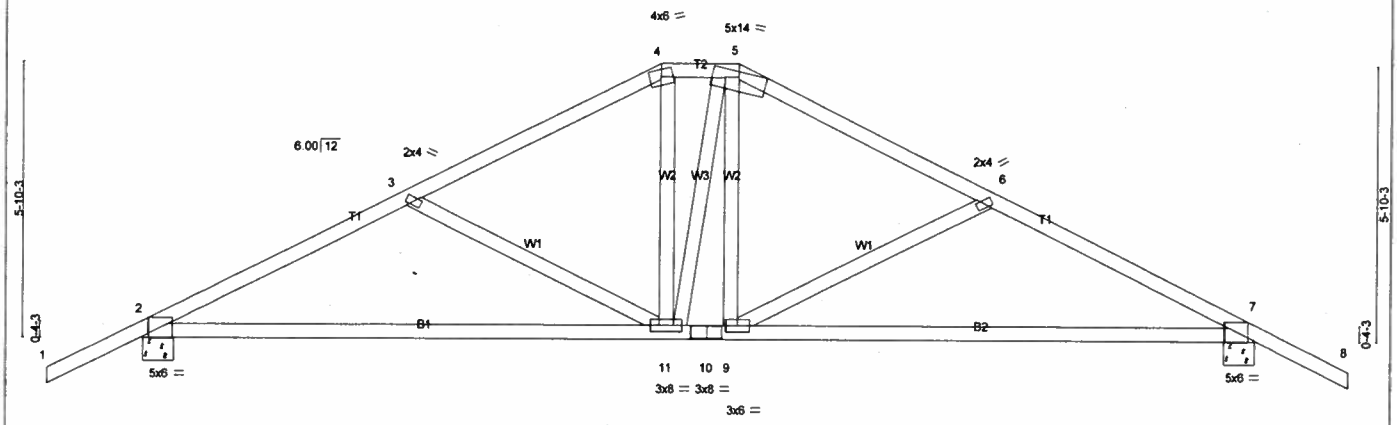
Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:37 2007 Page 1

-2-0-0 | 5-9-4 | 11-0-0 | 12-8-0 | 17-10-12 | 23-8-0 | 25-8-0

2-0-0 | 5-9-4 | 5-2-12 | 1-8-0 | 5-2-12 | 5-9-4 | 2-0-0

Scale = 1:46.3
Camber = 3/16 in



11-0-0 | 12-8-0 | 23-8-0

11-0-0 | 1-8-0 | 11-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.28	7-9	>970	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.49	7-9	>561	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 123 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-10-1 oc bracing.

REACTIONS (lb/size) 2=1092/0-8-0, 7=1092/0-8-0
Max Horz 2=-115(load case 6)
Max Uplift2=-445(load case 5), 7=-445(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-1532/490, 3-4=-1221/368, 4-5=-1036/377, 5-6=-1218/367, 6-7=-1532/491, 7-8=0/48
BOT CHORD 2-11=-402/1319, 10-11=-160/1031, 9-10=-160/1031, 7-9=-310/1319
WEBS 3-11=-335/256, 4-11=-101/375, 5-11=-129/156, 5-9=-63/336, 6-9=-340/258

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 445 lb uplift at joint 2 and 445 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071243

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T04	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:40 2007 Page 1

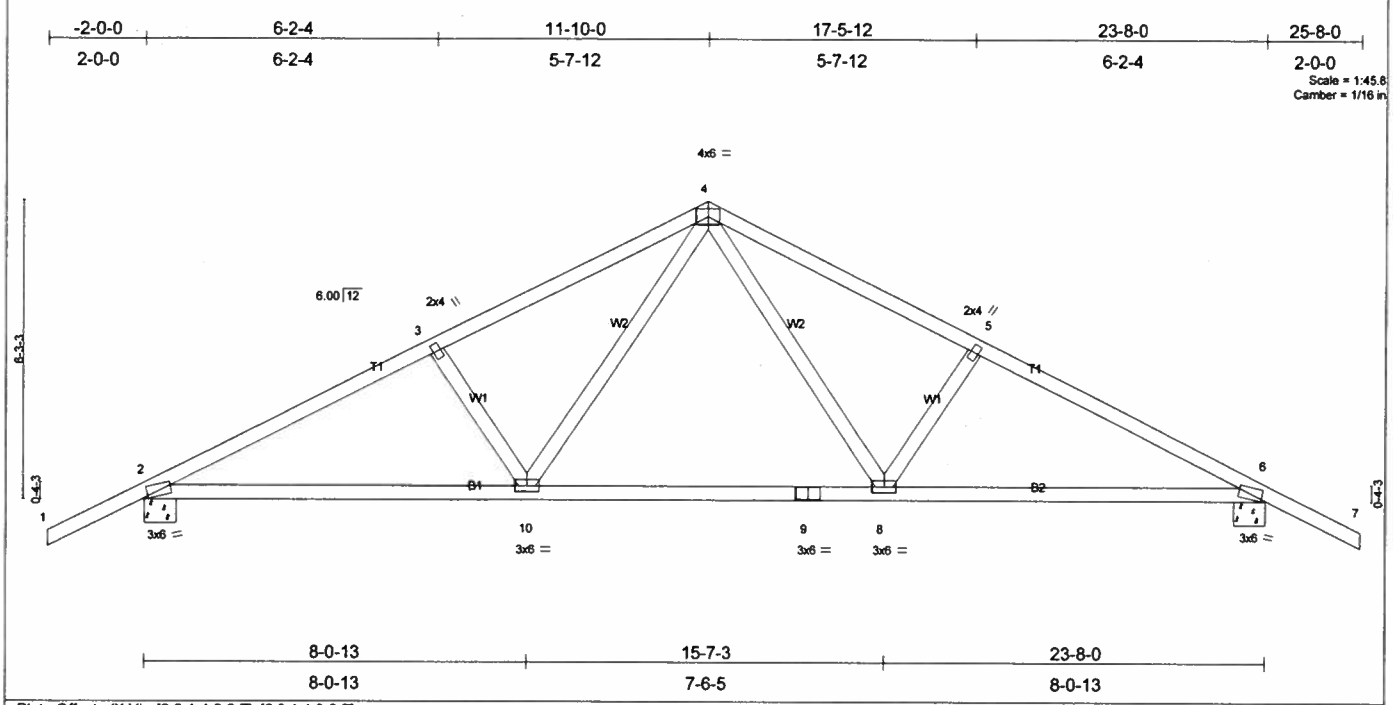


Plate Offsets (X,Y): [2:0-1-1,0-0-7], [6:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.11	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.18	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-9 oc bracing.

REACTIONS (lb/size) 2=1092/0-8-0, 6=1092/0-8-0
Max Horz 2=121(load case 5)
Max Uplift2=-450(load case 5), 6=-450(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-1588/479, 3-4=-1419/479, 4-5=-1419/479, 5-6=-1588/479, 6-7=0/48
BOT CHORD 2-10=-396/1341, 9-10=-176/922, 8-9=-176/922, 6-8=-303/1341
WEBS 3-10=-272/237, 4-10=-174/552, 4-8=-174/552, 5-8=-272/237

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 450 lb uplift at joint 6.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

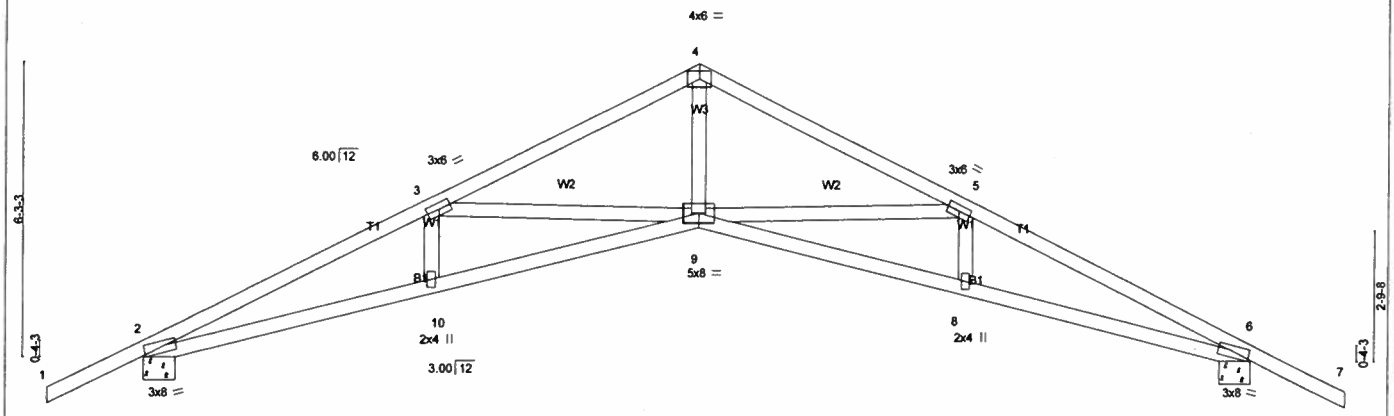
Dwg.#0207071244

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T05	SPECIAL	11	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:44 2007 Page 1

-2-0-0 | 6-1-11 | 11-10-0 | 17-6-5 | 23-8-0 | 25-8-0
2-0-0 | 6-1-11 | 5-8-5 | 5-8-5 | 6-1-11 | 2-0-0

Scale = 1:46.3
Camber = 3/16 in



6-1-11 | 11-10-0 | 17-6-5 | 23-8-0
6-1-11 | 5-8-5 | 5-8-5 | 6-1-11

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.25	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.40	8-9	>692	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.29	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-13 oc bracing.

REACTIONS (lb/size) 2=1092/0-8-0, 6=1092/0-8-0
Max Horz 2=119(load case 5)
Max Uplift2=-450(load case 5), 6=-450(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-2827/805, 3-4=-2066/543, 4-5=-2066/562, 5-6=-2827/695, 6-7=0/45
BOT CHORD 2-10=-703/2507, 9-10=-707/2509, 8-9=-542/2509, 6-8=-541/2507
WEBS 3-10=0/170, 3-9=-691/356, 4-9=-305/1463, 5-9=-691/369, 5-8=0/170

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 450 lb uplift at joint 6.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071245

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:47 2007 Page 1

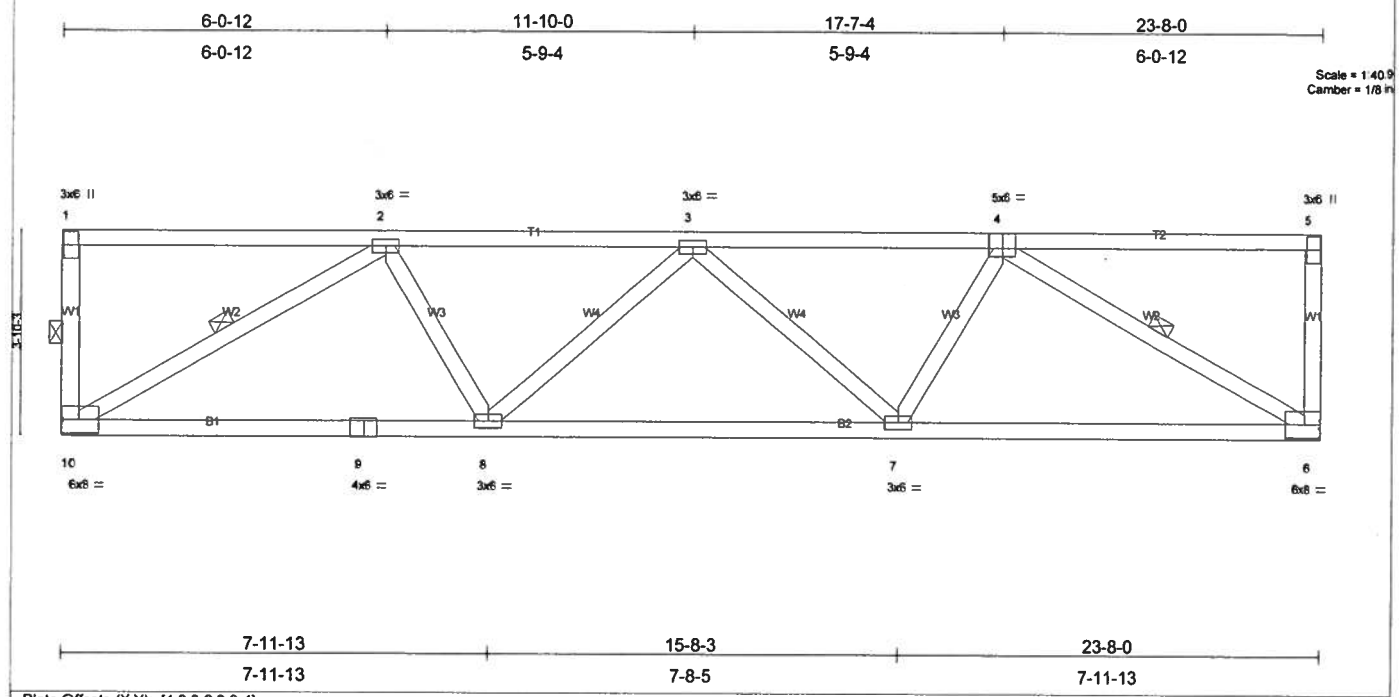


Plate Offsets (X,Y): [4:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	1.00	Vert(LL)	-0.22	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.36	7-8	>774	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.76	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 126 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-9 oc bracing.
WEBS 1 Row at midpt 1-10, 2-10, 4-6

REACTIONS (lb/size) 10=2045/Mechanical, 6=2045/Mechanical
Max Uplift10=-918(load case 2), 6=-918(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-10=-325/249, 1-2=-151/44, 2-3=-3053/1300, 3-4=-3051/1301, 4-5=-150/45, 5-6=-324/249
BOT CHORD 9-10=-1221/2615, 8-9=-1221/2615, 7-8=-1575/3398, 6-7=-1222/2613
WEBS 2-10=-2874/1374, 2-8=-166/921, 3-8=-470/374, 3-7=-472/374, 4-7=-166/920, 4-6=-2874/1373

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 918 lb uplift at joint 10 and 918 lb uplift at joint 6.
5) Girder carries hip end with 0-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-112(F=-58), 6-10=-63(F=-33)

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0207071246

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:51 2007 Page 1

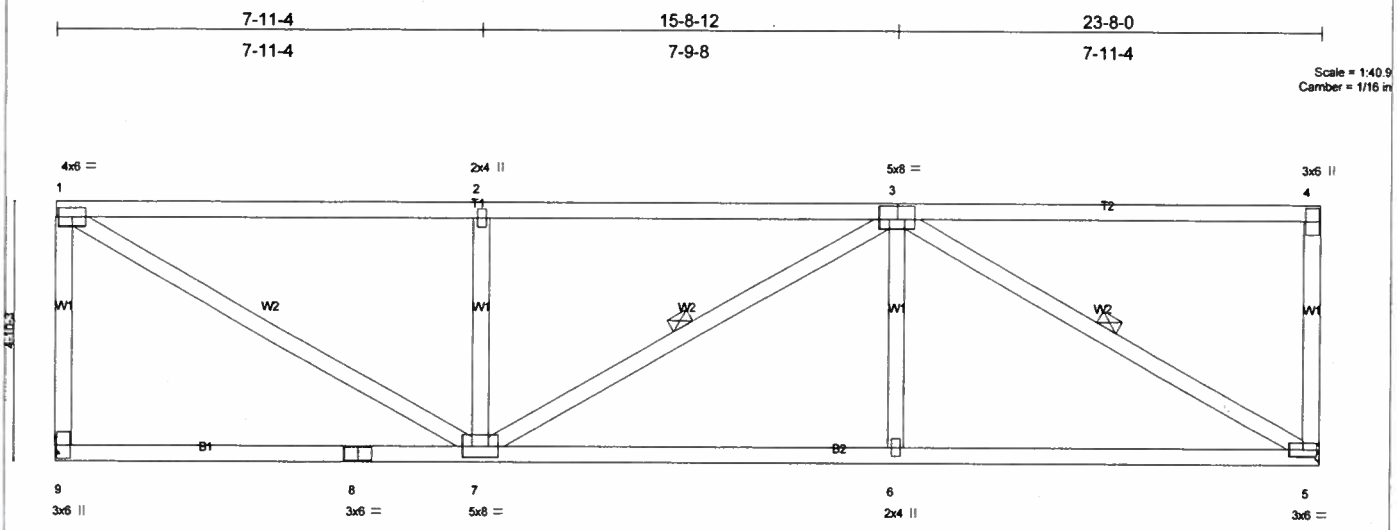


Plate Offsets (X,Y): [3:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.18	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 134 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-13 oc bracing.
WEBS 1 Row at midpt 3-7, 3-5

REACTIONS (lb/size) 9=982/Mechanical, 5=982/Mechanical
Max Uplift9=-371(load case 3), 5=-371(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-9=-866/387, 1-2=-1213/458, 2-3=-1213/458, 3-4=-73/27, 4-5=-199/135
BOT CHORD 8-9=-28/73, 7-8=-28/73, 6-7=-458/1213, 5-6=-458/1213
WEBS 1-7=-498/1320, 2-7=-431/305, 3-7=-1/1, 3-6=0/235, 3-5=-1321/499

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 9 and 371 lb uplift at joint 5.

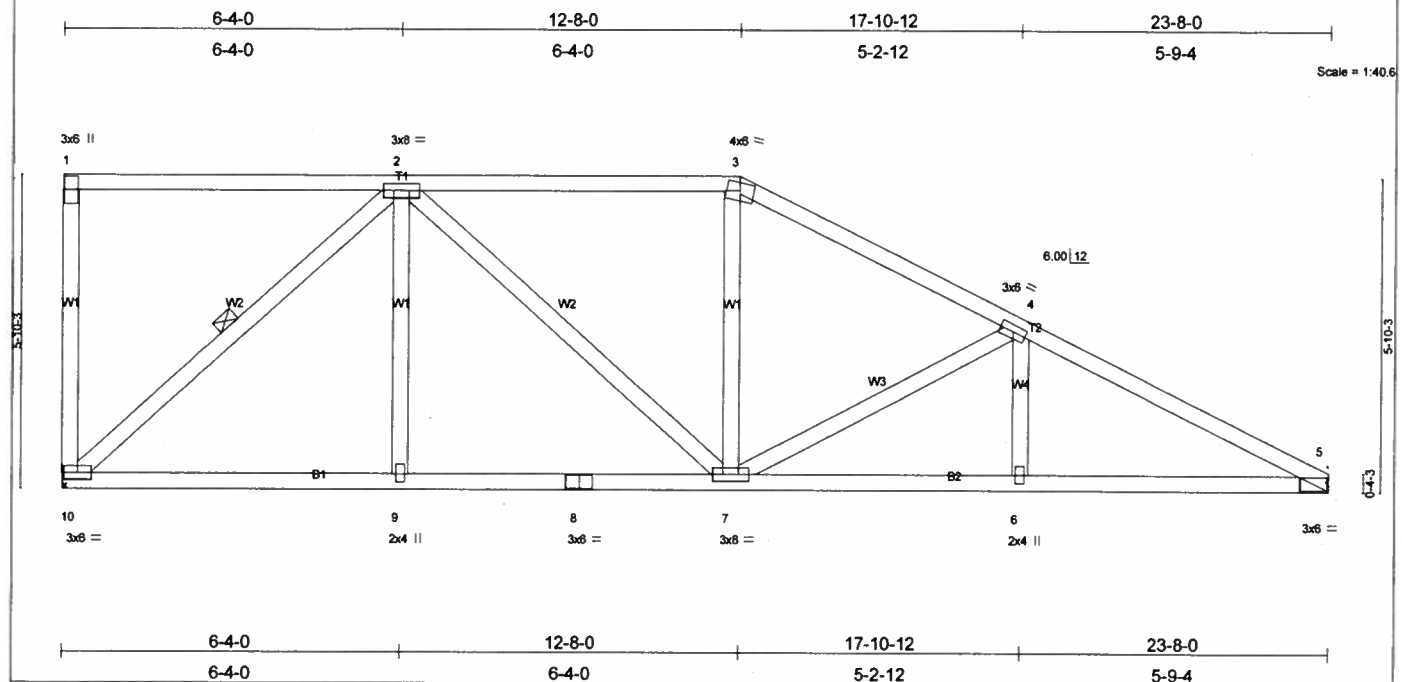
LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071247

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:54 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.08	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.13	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-11-10 oc bracing.
WEBS 1 Row at midpt 2-10

REACTIONS (lb/size) 10=985/Mechanical, 5=985/Mechanical
Max Horz 10=-256(load case 6)
Max Uplift10=-345(load case 3), 5=-295(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-10=-152/102, 1-2=-32/12, 2-3=-1074/376, 3-4=-1258/368, 4-5=-1784/506
BOT CHORD 9-10=-169/861, 8-9=-169/861, 7-8=-169/861, 6-7=-378/1532, 5-6=-378/1532
WEBS 2-10=-1113/383, 2-9=0/192, 2-7=-164/286, 3-7=0/230, 4-7=-528/293, 4-6=0/190

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 10 and 295 lb uplift at joint 5.

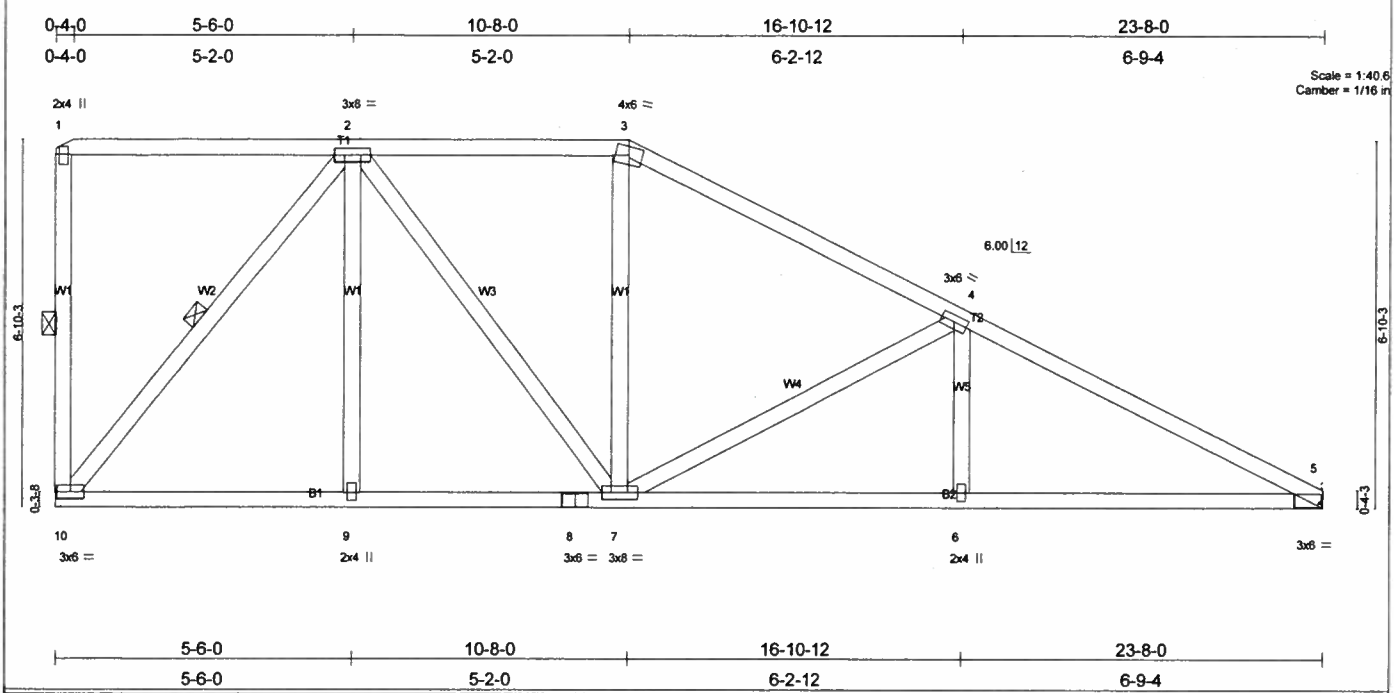
LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071248

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:17:58 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.11	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.18	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 139 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-10, 1-10

REACTIONS (lb/size) 5=985/Mechanical, 10=985/Mechanical
Max Horz 10=-302(load case 6)
Max Uplift5=-297(load case 6), 10=-334(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-20/7, 2-3=-899/340, 3-4=-1084/320, 4-5=-1736/490, 1-10=-134/89
BOT CHORD 9-10=-74/651, 8-9=-74/651, 7-8=-74/651, 6-7=-352/1483, 5-6=-352/1483
WEBS 2-10=-997/331, 2-9=0/155, 2-7=-212/409, 3-7=0/153, 4-7=-667/355, 4-6=0/235

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 5 and 334 lb uplift at joint 10.

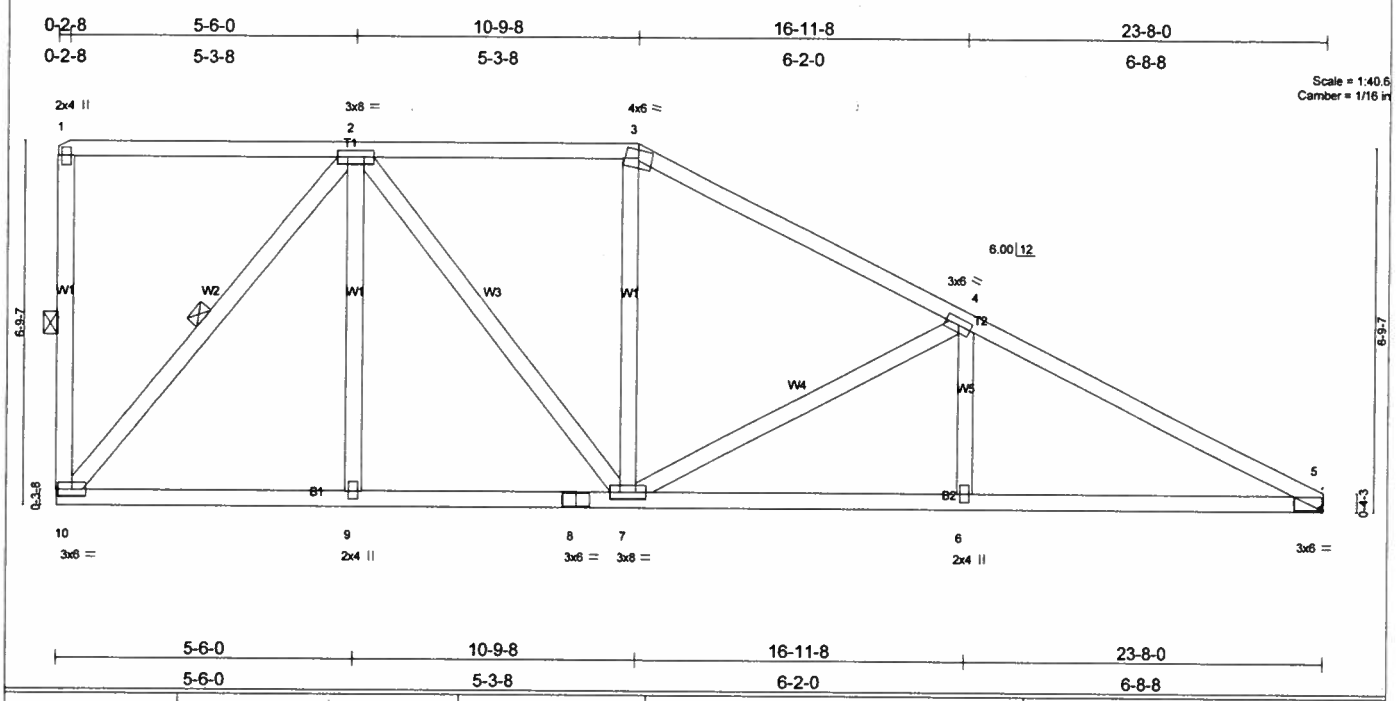
LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071249

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:18:01 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.11	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.18	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 139 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-10, 1-10

REACTIONS (lb/size) 5=985/Mechanical, 10=985/Mechanical
Max Horz 10=-299(load case 6)
Max Uplift5=-297(load case 6), 10=-335(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-20/7, 2-3=-910/343, 3-4=-1095/323, 4-5=-1739/491, 1-10=-133/89
BOT CHORD 9-10=-78/658, 8-9=-78/658, 7-8=-78/658, 6-7=-354/1486, 5-6=-354/1486
WEBS 2-10=-1002/333, 2-9=0/158, 2-7=-210/406, 3-7=0/156, 4-7=-658/351, 4-6=0/232

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 5 and 335 lb uplift at joint 10.

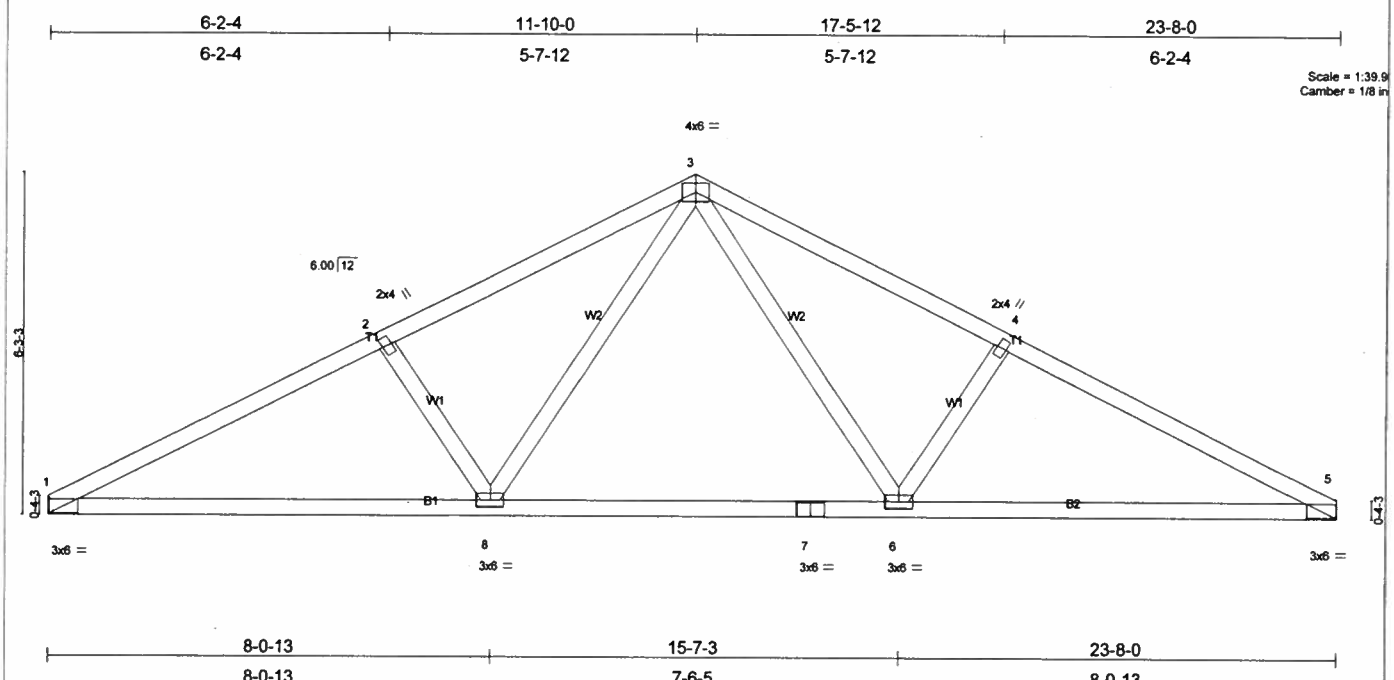
LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071250

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T11	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:18:04 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.16	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.27	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-14 oc bracing.

REACTIONS (lb/size) 1=989/Mechanical, 5=989/Mechanical
Max Horz 1=-87(load case 3)
Max Uplift1=-322(load case 5), 5=-322(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1709/559, 2-3=-1546/554, 3-4=-1546/554, 4-5=-1709/559
BOT CHORD 1-8=-505/1482, 7-8=-233/991, 6-7=-233/991, 5-6=-421/1482
WEBS 2-8=-325/276, 3-8=-231/626, 3-6=-231/626, 4-6=-325/276

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 322 lb uplift at joint 1 and 322 lb uplift at joint 5.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071251

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T12	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:18:08 2007 Page 1

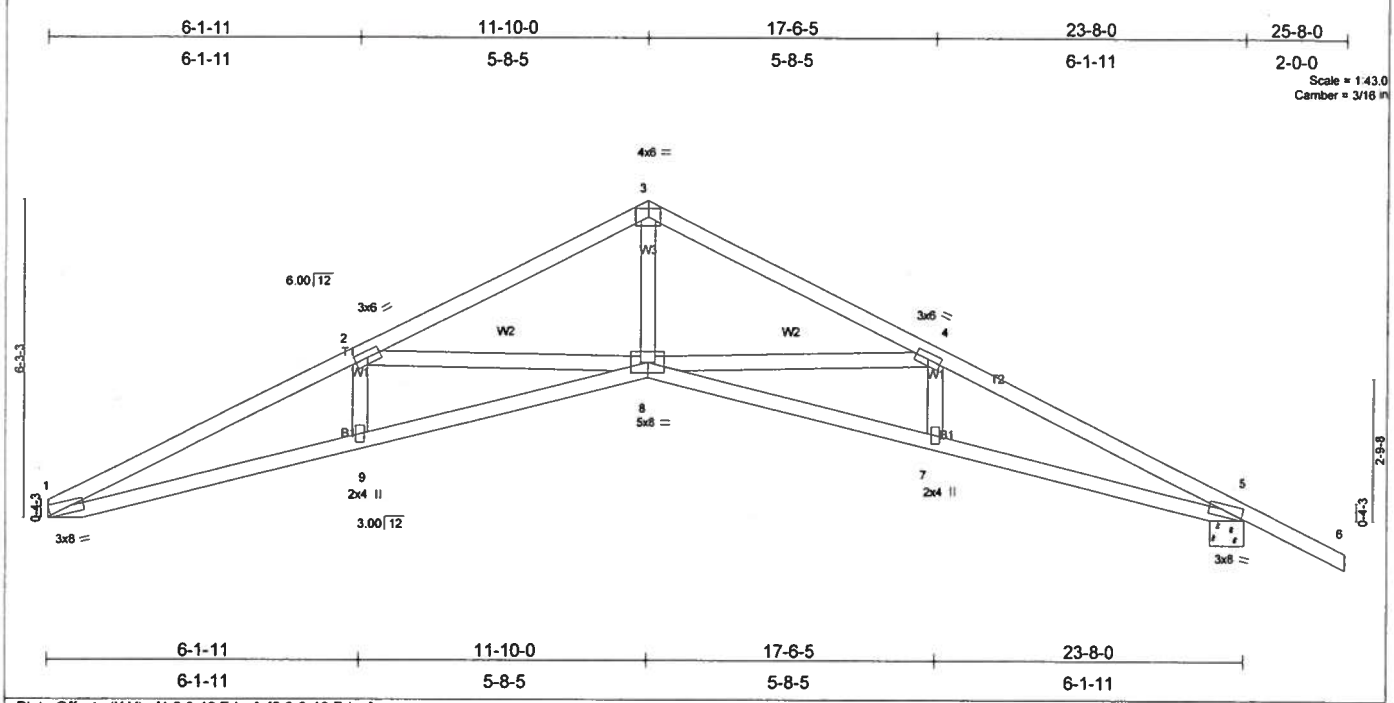


Plate Offsets (X,Y): [1:0-0-10,Edge], [5:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.27	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.43	7-8	>653	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.31	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 104 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-13 oc bracing.

REACTIONS (lb/size) 1=971/Mechanical, 5=1110/0-8-0
Max Horz 1=-139(load case 6)
Max Uplift1=-316(load case 5), 5=-455(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3037/953, 2-3=-2136/589, 3-4=-2134/605, 4-5=-2895/736, 5-6=0/45
BOT CHORD 1-9=-856/2722, 8-9=-853/2717, 7-8=-583/2570, 5-7=-582/2568
WEBS 2-9=0/192, 2-8=-827/461, 3-8=-351/1531, 4-8=-690/369, 4-7=0/172

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 316 lb uplift at joint 1 and 455 lb uplift at joint 5.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

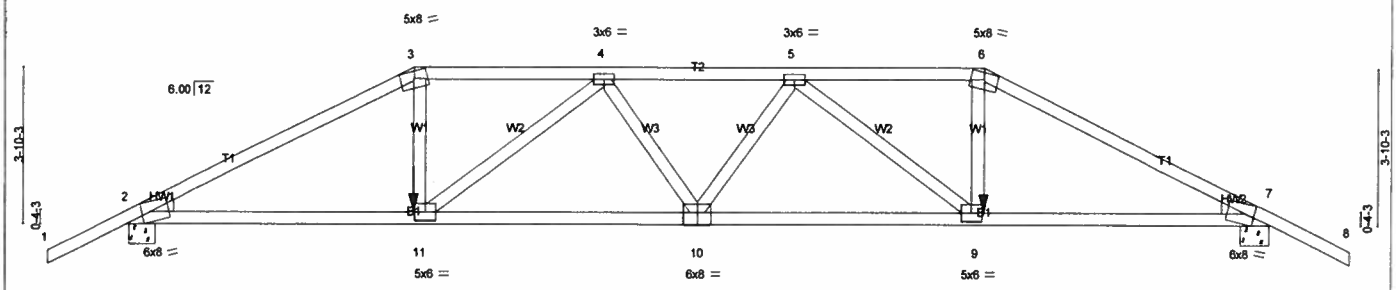
Dwg.#0207071252

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T13	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:18:12 2007 Page 1

-2-0-0 | 7-0-0 | 11-8-0 | 16-4-0 | 21-0-0 | 28-0-0 | 30-0-0
2-0-0 | 7-0-0 | 4-8-0 | 4-8-0 | 4-8-0 | 7-0-0 | 2-0-0

Scale = 1:53.6
Camber = 1/4 in



7-0-0 | 14-0-0 | 21-0-0 | 28-0-0
7-0-0 | 7-0-0 | 7-0-0 | 7-0-0

Plate Offsets (X,Y): [2:0-4-9,Edge], [7:0-4-9,Edge], [10:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.35	10-11	>929	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.57	10-11	>573	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.76	Horz(TL)	0.17	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-8-0 oc bracing.

REACTIONS (lb/size) 2=2445/0-8-0, 7=2445/0-8-0
Max Horz 2=-87(load case 5)
Max Uplift2=-1063(load case 4), 7=-1063(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-4485/1847, 3-4=-3956/1697, 4-5=-5043/2127, 5-6=-3956/1697, 6-7=-4485/1848, 7-8=0/48
BOT CHORD 2-11=-1593/3889, 10-11=-2084/4895, 9-10=-2067/4895, 7-9=-1552/3889
WEBS 3-11=-619/1653, 4-11=-1286/677, 4-10=0/299, 5-10=0/299, 5-9=-1286/677, 6-9=-619/1653

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1063 lb uplift at joint 2 and 1063 lb uplift at joint 7.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 21-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-113(F=-59), 6-8=-54, 2-11=-30, 9-11=-63(F=-33), 7-9=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 9=-539(F)

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0207071253

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:18:15 2007 Page 1

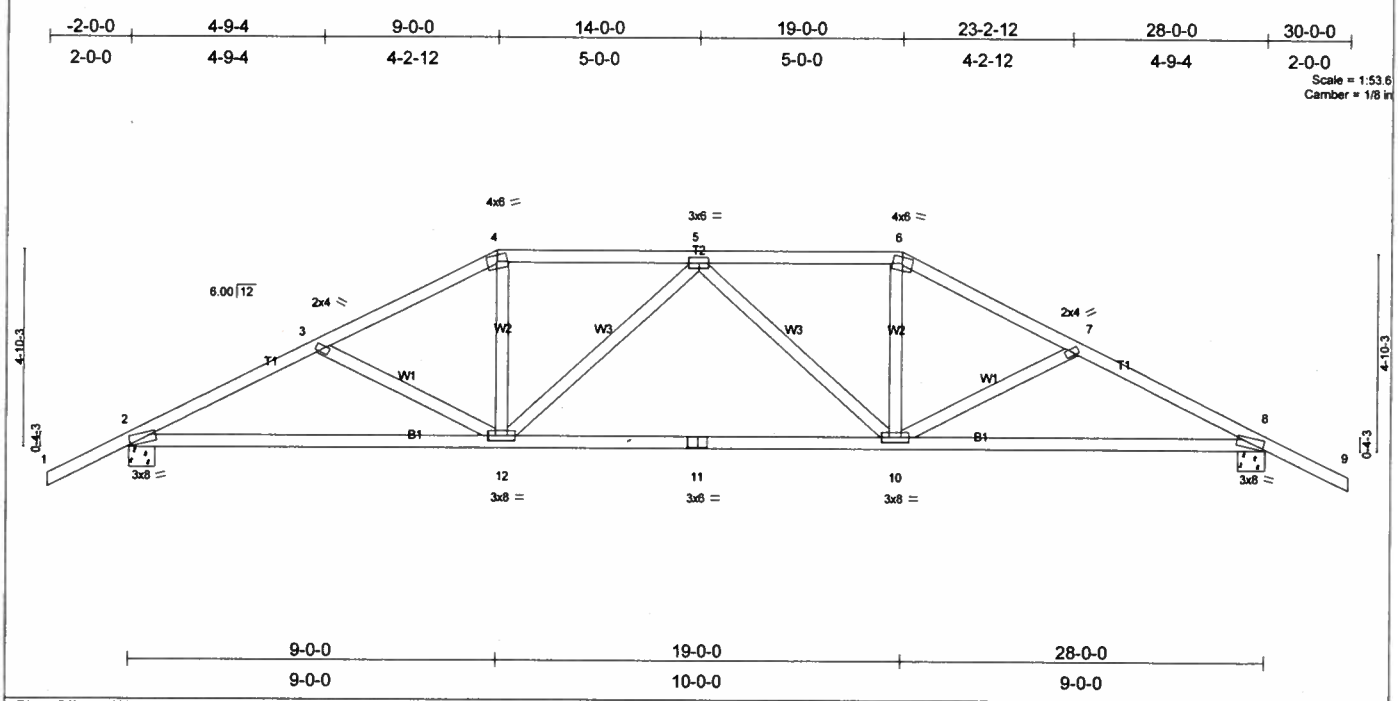


Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.22	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.36	10-12	>913	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.22	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 139 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-3-13 oc bracing.

REACTIONS (lb/size) 2=1274/0-8-0, 8=1274/0-8-0
Max Horz 2=101(load case 5)
Max Uplift2=-474(load case 5), 8=-474(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-1960/548, 3-4=-1758/467, 4-5=-1543/461, 5-6=-1543/461, 6-7=-1758/467, 7-8=-1960/549, 8-9=0/48
BOT CHORD 2-12=-447/1686, 11-12=-407/1684, 10-11=-407/1684, 8-10=-352/1686
WEBS 3-12=-186/178, 4-12=-71/504, 5-12=-282/187, 5-10=-282/187, 6-10=-70/504, 7-10=-186/179

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 8.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

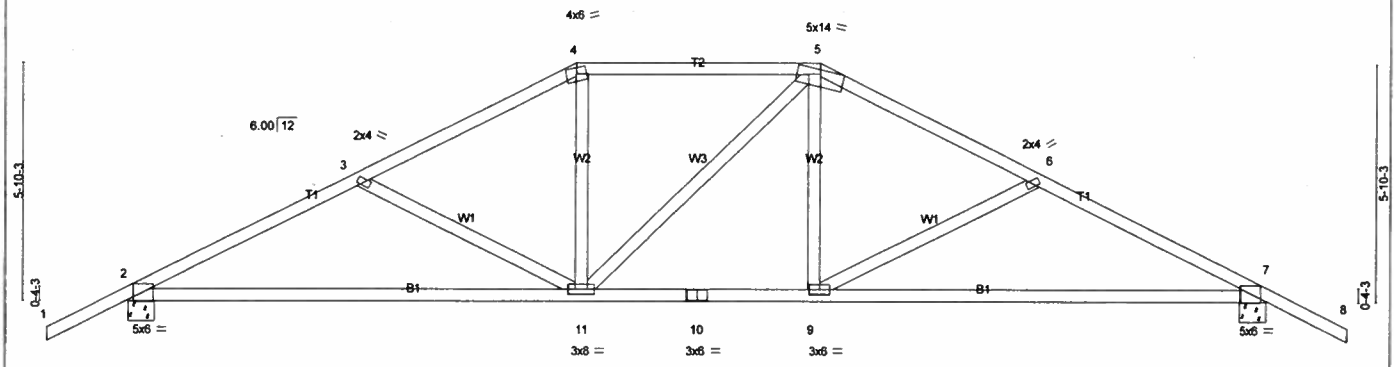
Dwg.#0207071254

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:18:19 2007 Page 1

-2-0-0	5-9-4	11-0-0	17-0-0	22-2-12	28-0-0	30-0-0
2-0-0	5-9-4	5-2-12	6-0-0	5-2-12	5-9-4	2-0-0

Scale = 1:53.6
Camber = 1/4 in



11-0-0	17-0-0	28-0-0
11-0-0	6-0-0	11-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.35	7-9	>949	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.60	7-9	>550	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 139 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-11-9 oc bracing.

REACTIONS (lb/size) 2=1274/0-8-0, 7=1274/0-8-0
Max Horz 2=115(load case 5)
Max Uplift2=-490(load case 5), 7=-490(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-1901/585, 3-4=-1605/461, 4-5=-1386/467, 5-6=-1605/461, 6-7=-1901/586, 7-8=0/48
BOT CHORD 2-11=-485/1644, 10-11=-227/1385, 9-10=-227/1385, 7-9=-370/1644
WEBS 3-11=-304/249, 4-11=-63/384, 5-11=-128/130, 5-9=-59/385, 6-9=-305/249

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 490 lb uplift at joint 2 and 490 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

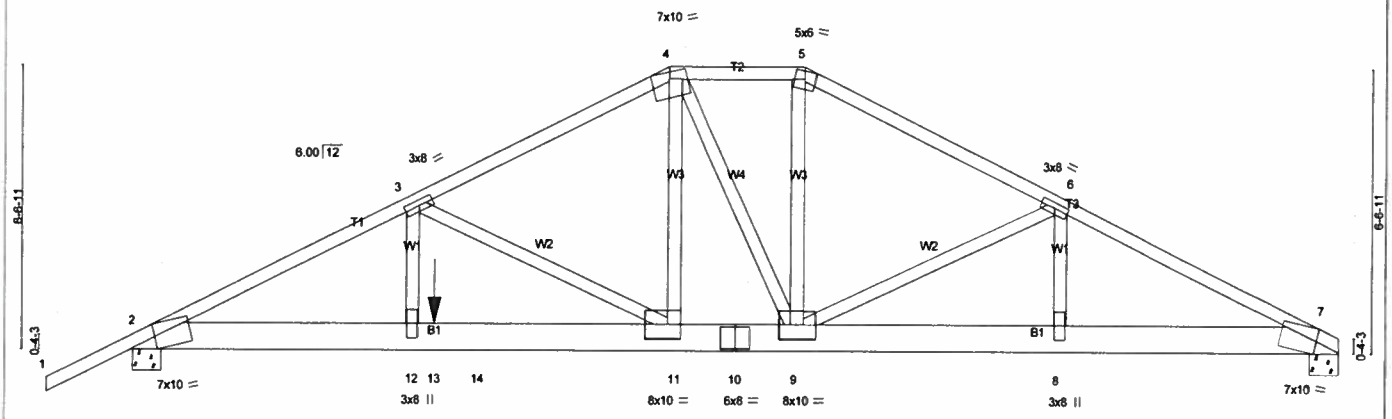
Dwg.#0207071255

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T16	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 06 16:18:22 2007 Page 1

-2-0-0	6-5-12	12-5-0	15-7-0	21-6-4	28-0-0
2-0-0	6-5-12	5-11-4	3-2-0	5-11-4	6-5-12

Scale = 1:50.4
Camber = 3/16 in



6-5-12	7-0-0	12-5-0	15-7-0	21-6-4	28-0-0
6-5-12	0-6-4	5-5-0	3-2-0	5-11-4	6-5-12

Plate Offsets (X,Y): [2:0-6-11,Edge], [7:0-6-11,Edge], [11:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.28	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.45	11-12	>726	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.65	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 378 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=7194/0-8-0, 2=5946/0-8-0
Max Horz 2=158(load case 4)
Max Uplift7=-2647(load case 5), 2=-2264(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-12343/4461, 3-4=-9147/3362, 4-5=-8202/3098, 5-6=-9143/3367, 6-7=-12679/4643
BOT CHORD 2-12=-3978/10932, 12-13=-3978/10932, 13-14=-3978/10932, 11-14=-3978/10932, 10-11=-2897/8206, 9-10=-2897/8206, 8-9=-4061/11257, 7-8=-4061/11257
WEBS 3-12=-887/2636, 3-11=-3174/1253, 4-11=-1408/3836, 4-9=-166/129, 5-9=-1400/3809, 6-9=-3544/1434, 6-8=-1013/2937

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2647 lb uplift at joint 7 and 2264 lb uplift at joint 2.
7) Girder carries tie-in span(s): 23-8-0 from 8-0-0 to 28-0-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2045 lb down and 772 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-7=-54, 2-14=-30, 7-14=-471(F=-441)
Concentrated Loads (lb)
Vert: 13=-2045(F)

FEBRUARY 07,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

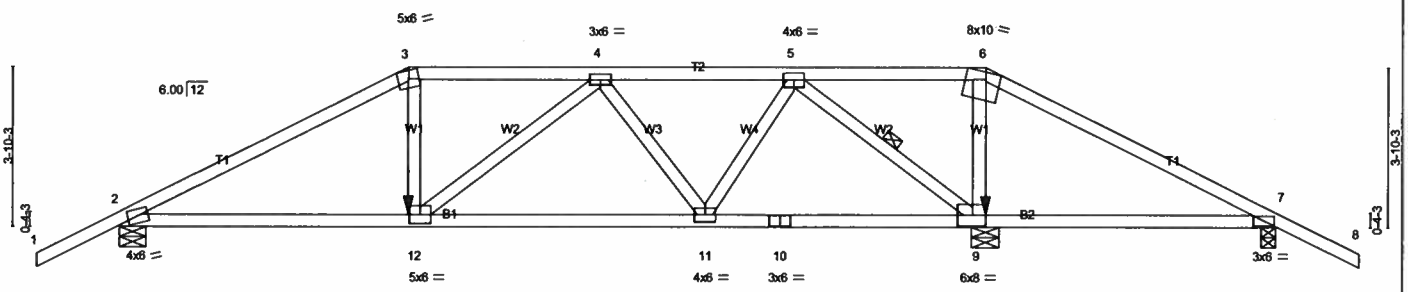
Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 14 10:44:56 2007 Page 1

-2-0-0	7-0-0	11-8-0	16-4-0	21-0-0	28-0-0	30-0-0
2-0-0	7-0-0	4-8-0	4-8-0	4-8-0	7-0-0	2-0-0

Scale = 1:53.6
Camber = 1/8 in

*** Uplift in gravity load case.**



7-0-0	14-2-3	21-0-0	28-0-0
7-0-0	7-2-3	6-9-13	7-0-0

Plate Offsets (X,Y): [2:0-3-1,0-0-11], [6:0-4-3,Edge], [9:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.18	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.29	11-12	>843	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 131 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

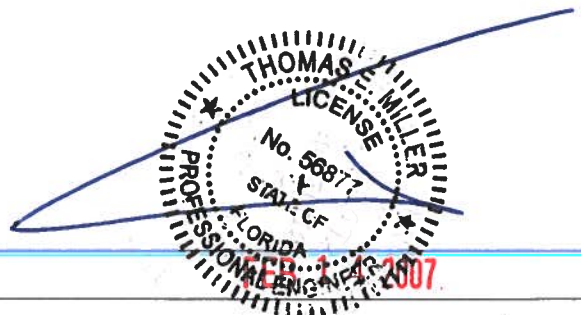
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-5-7 oc bracing.
WEBS 1 Row at midpt 5-9

REACTIONS (lb/size) 2=1555/0-8-0, 9=3567/0-8-0, 7=-226/0-4-0
Max Horz 2=-87(load case 5)
Max Uplift2=-718(load case 4), 9=-1566(load case 2), 7=-457(load case 8)
Max Grav 2=1558(load case 8), 9=3567(load case 1), 7=120(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-2605/1071, 3-4=-2260/997, 4-5=-1594/696, 5-6=-468/1180, 6-7=-552/1356, 7-8=0/47
BOT CHORD 2-12=-908/2230, 11-12=-918/2064, 10-11=-422/923, 9-10=-422/923, 7-9=-1142/529
WEBS 3-12=-254/790, 4-12=-81/251, 4-11=-827/524, 5-11=-374/1305, 5-9=-2675/1234, 6-9=-1063/529

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 718 lb uplift at joint 2, 1566 lb uplift at joint 9 and 457 lb uplift at joint 7.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 21-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-113(F=-59), 6-8=-54, 2-12=-30, 9-12=-63(F=-33), 7-9=-30
Concentrated Loads (lb)
Vert: 12=-539(F) 9=-539(F)



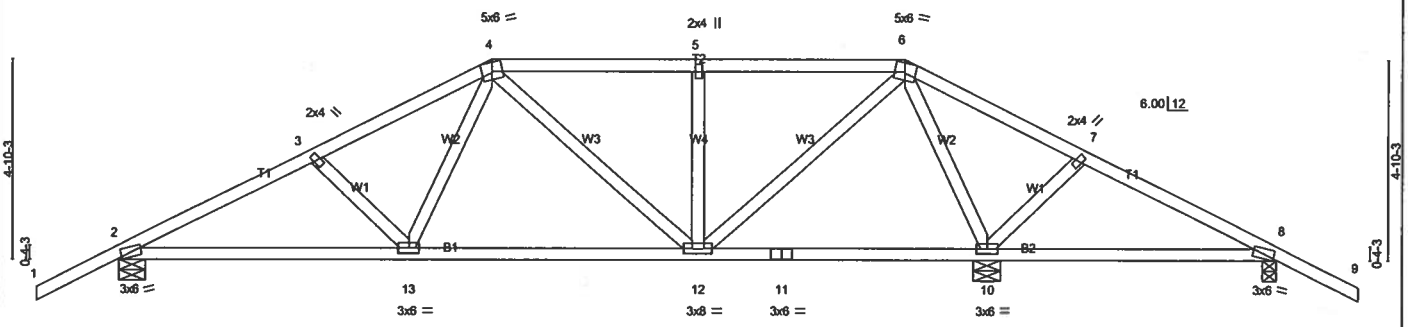
The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer.

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Feb 14 10:44:59 2007 Page 1

-2-0-0	4-9-4	9-0-0	14-0-0	19-0-0	23-2-12	28-0-0	30-0-0
2-0-0	4-9-4	4-2-12	5-0-0	5-0-0	4-2-12	4-9-4	2-0-0

Scale = 1:53.6



7-0-0	14-0-0	21-0-0	28-0-0
7-0-0	7-0-0	7-0-0	7-0-0

Plate Offsets (X,Y): [2:0-1-1,0-0-7], [8:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.10	8-10	>814	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	0.09	8-10	>939	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

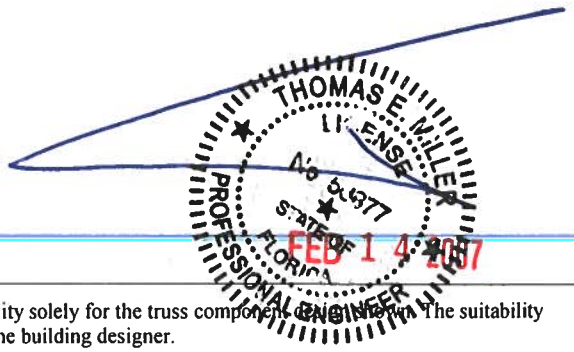
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 8-10.

REACTIONS (lb/size) 2=928/0-8-0, 10=1422/0-8-0, 8=203/0-4-0
Max Horz 2=101(load case 5)
Max Uplift2=-391(load case 5), 10=-427(load case 6), 8=-296(load case 6)
Max Grav 2=931(load case 9), 10=1422(load case 1), 8=239(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 6-7=-78/491, 7-8=-83/350, 8-9=0/47, 4-5=-712/276, 5-6=-712/276, 1-2=0/48, 2-3=-1293/367, 3-4=-1132/334
BOT CHORD 2-13=-290/1086, 12-13=-205/822, 11-12=0/129, 10-11=0/129, 8-10=-290/112
WEBS 7-10=-242/214, 6-10=-1125/332, 6-12=-251/875, 5-12=-284/202, 4-12=-155/105, 4-13=-74/360, 3-13=-169/167

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 2, 427 lb uplift at joint 10 and 296 lb uplift at joint 8.

LOAD CASE(S) Standard



FEB 14 2007

The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component shown. The suitability and use of this component for any particular building design is the responsibility of the building designer.

Job	Truss	Truss Type	Qty	Ply	HOMES BY HOUSE CRAFT
L225956	T19	HIP	1	2	Job Reference (optional)

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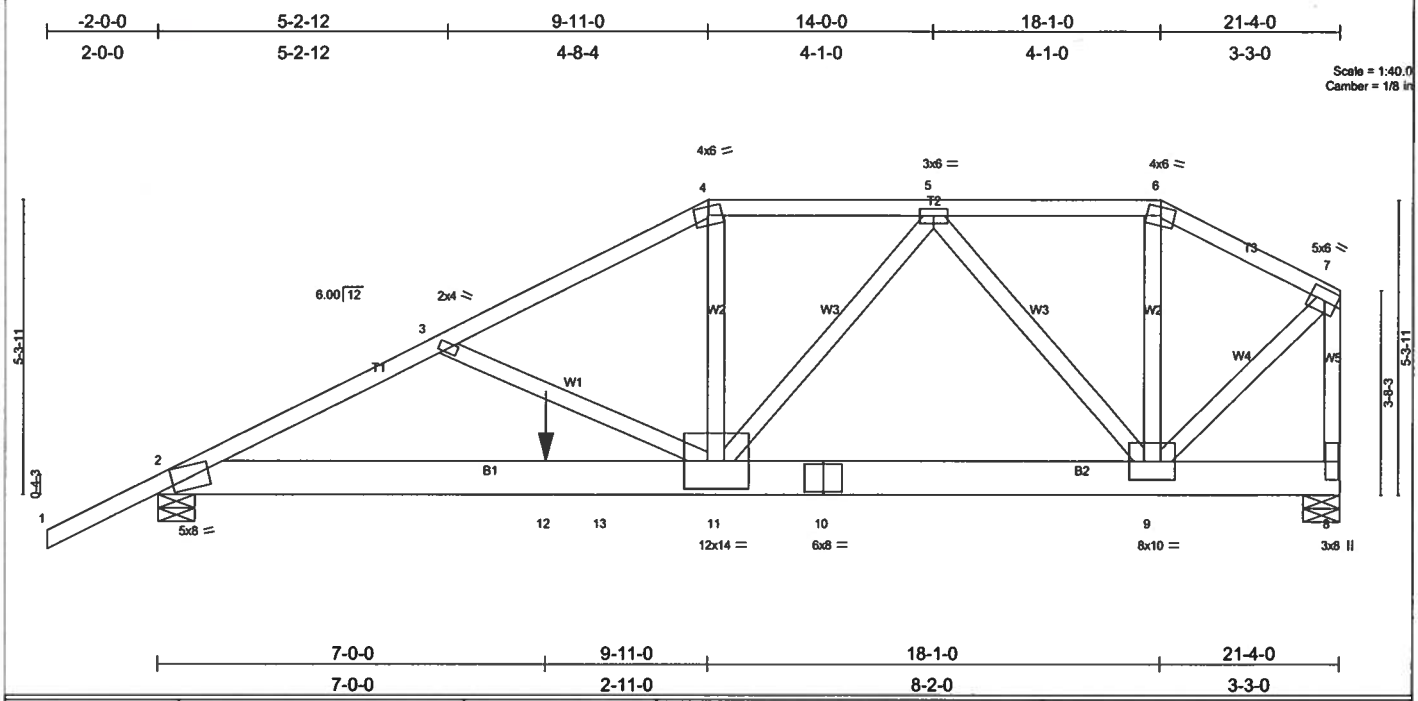


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.19	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.31	2-11	>820	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.75	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 304 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

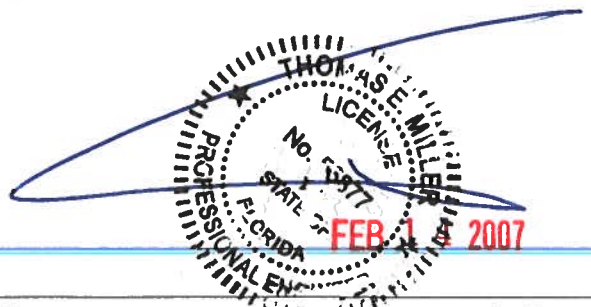
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=4239/0-8-0, 8=5499/0-8-0
Max Horz 2=246(load case 4)
Max Uplift2=-1632(load case 4), 8=-1974(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-7653/2798, 3-4=-7373/2656, 4-5=-6626/2452, 5-6=-3377/1261, 6-7=-3769/1366, 7-8=-5076/1829
BOT CHORD 2-12=-2607/6795, 12-13=-2607/6795, 11-13=-2607/6795, 10-11=-1867/5080, 9-10=-1867/5080, 8-9=-27/57
WEBS 3-11=-289/263, 4-11=-1052/3034, 5-11=-902/2465, 5-9=-2722/1032, 6-9=-523/1525, 7-9=-1686/4659

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1632 lb uplift at joint 2 and 1974 lb uplift at joint 8.
7) Girder carries tie-in span(s): 21-4-0 from 0-0-8 to 21-4-0; 23-8-0 from 8-0-0 to 21-4-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2045 lb down and 772 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 6-7=-54, 2-13=-30, 8-13=-471(F=-441)
Concentrated Loads (lb)
Vert: 12=-2045(F)

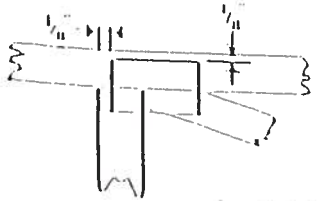


The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer.

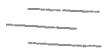
Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

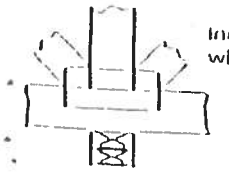
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

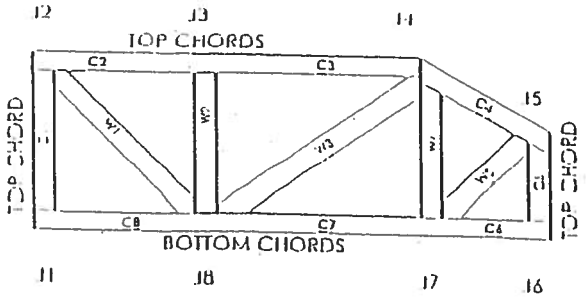
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-11
FIER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Swayze
HVAC Load Calculations

for

House Craft Homes
12523 N.W U.S HWY 441
Alachua Fl 32615

Elite Software

RHVAC RESIDENTIAL HVAC LOADS

Prepared By:
Chuck Fischer
North Central Florida Air Conditioning
P.O Box 700
High Springs Fl 32655-0700
386-454-4767
Saturday, February 03, 2007

Project Report

General Project Information

Project Filename:	C:\Documents and Settings\Heat\My Documents\Projects\AutoLoad MJ8.rhv
Project Title:	Swayze
Designed By:	Chuck Fischer
Project Date:	February 3 2007
Client Name:	House Craft Homes
Client Address:	12523 N.W U.S HWY 441
Client City:	Alachua Fl 32615
Client Phone:	386-462-5323
Client Fax:	386-462-1509
Client Comment:	
Company Name:	North Central Florida Air Conditioning
Company Representative:	Chuck Fischer
Company Address:	P.O Box 700
Company City:	High Springs Fl 32655-0700
Company Phone:	386-454-4767
Company Fax:	386-454-4854
Company Comment:	Bedroom 2,3 & 4 R/A are 10x10x8 Master bedroom R/A is 12x12x9 Main R/A is 20x20x18

Design Data

Reference City:		Gainesville, Florida
Daily Temperature Range:		Medium
Latitude:	29	Degrees
Elevation:	152	ft.
Altitude Factor:	0.995	
Elevation Sensible Adj. Factor:	1.000	
Elevation Total Adj. Factor:	1.000	
Elevation Heating Adj. Factor:	1.000	
Elevation Heating Adj. Factor:	1.000	

	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum	Indoor Dry Bulb	Grains Difference
Winter:	31	0	0	68	0
Summer:	93	77	50	75	50

Check Figures

Total Building Supply CFM:	1,130	CFM Per Square ft.:	0.674
Square ft. of Room Area:	1,676	Square ft. Per Ton:	626
Volume (ft³) of Cond. Space:	15,387	Air Turnover Rate (per hour):	4.4

Building Loads

Total Heating Required With Outside Air:	37,375 Btuh	37.375 MBH	
Total Sensible Gain:	24,725 Btuh	85 %	
Total Latent Gain:	4,393 Btuh	15 %	
Total Cooling Required With Outside Air:	29,118 Btuh	2.43 Tons (Based On Sensible + Latent)	
		2.68 Tons (Based On 77% Sensible Capacity)	

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

Miscellaneous Report

System 1 Main Floor Input Data	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum	Indoor Dry Bulb	Grains Difference
Winter:	31	0	50	68	30.84
Summer:	93	77	50	75	50.06

Duct Sizing Inputs

	Main Trunk		Runouts	
Calculate:	Yes		Yes	
Use Schedule:	No		No	
Roughness Factor:	0.00300		0.01000	
Pressure Drop:	0.1000	in.wg./100 ft.	0.1000	in.wg./100 ft.
Minimum Velocity:	650	ft./min	450	ft./min
Maximum Velocity:	900	ft./min	750	ft./min
Minimum Height:	0	in.	0	in.
Maximum Height:	0	in.	0	in.

Outside Air Data

	Winter		Summer	
Infiltration:	0.900	AC/hr	0.400	AC/hr
Volume of Conditioned Space:	X 15387	Cu.ft.	X 15387	Cu.ft.
	13,848	Cu.ft./hr	6,155	Cu.ft./hr
	X 0.0167		X 0.0167	
Total Building Infiltration:	231	CFM	103	CFM
Total Building Ventilation:	0	CFM	0	CFM

---System 1---

Infiltration & Ventilation Sensible Gain Multiplier:	19.69	= (1.10 X 0.995 X 18.00 Summer Temp. Difference)
Infiltration & Ventilation Latent Gain Multiplier:	33.85	= (0.68 X 0.995 X 50.06 Grains Difference)
Infiltration & Ventilation Sensible Loss Multiplier:	40.48	= (1.10 X 0.995 X 37.00 Winter Temp. Difference)

Load Preview Report

Scope	Area	Sens Gain	Lat Gain	Net Gain	Sens Loss	Win CFM	Sum CFM	Sys CFM	Duct Size
Building: 2.43 Net Tons, 2.68 Recommended Tons, 626 ft.²/Ton, 37.38 MBH Heating									
Building	1,676	24,725	4,393	29,118	37,375	488	1,130	1,130	
System 1: 2.43 Net Tons, 2.68 Recommended Tons, 626 ft.²/Ton, 37.38 MBH Heating									
System 1	1,676	24,725	4,393	29,118	37,375	488	1,130	1,130	15x15
Zone 1	1,676	24,725	4,393	29,118	37,375	488	1,130	1,130	
1-Master Bedroom	216	3,415	904	4,319	5,080	66	156	156	1-7
2-Master Bath	83	1,219	275	1,494	2,756	36	56	56	1-4
3-Master W.I.C	45	551	79	630	809	11	25	25	1-3
4-Kitchen	171	3,679	516	4,195	2,687	35	168	168	1-8
5-Dining Room	189	2,251	115	2,366	2,039	27	103	103	1-6
6-Living Room	255	3,038	499	3,537	5,315	69	139	139	1-7
7-Foyer	70	1,966	399	2,365	3,337	44	90	90	1-6
8-Laundry Room	45	702	88	790	802	10	32	32	1-3
9-Bedroom 3	159	3,022	647	3,669	4,433	58	138	138	1-7
10-Bath 2	52	766	88	854	930	12	35	35	1-3
11-Bedroom 2	364	4,069	783	4,852	9,096	119	186	186	1-8
12-Hall	27	44	0	44	91	1	2	2	1-1

Total Building Summary Loads

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 100% coverage	44	1,058	0	990	990
10B-f: Glazing-French door, double pane clear glass, insulated fiberglass frame, ground reflectance = 0.32	40.8	725	0	685	685
1D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	75	1,804	0	1,308	1,308
11P: Door-Polyurethane Core	19	204	0	160	160
13A-5ocs: Wall-Block, board insulation only, R-5 board insulation, open core, siding finish	1356.2	6,272	0	3,104	3,104
12B-5sw: Wall-Frame, R-11 insulation in 2 x 4 stud cavity, R-5 board insulation, siding finish, wood studs	302.6	761	0	419	419
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1675.4	1,984	0	2,306	2,306
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	206	8,994	0	0	0
Subtotals for structure:		21,802	0	8,972	8,972
People:	4		920	1,200	2,120
Equipment:			0	1,200	1,200
Lighting:	2115			7,212	7,212
Ductwork:		6,230	0	4,120	4,120
Infiltration: Winter CFM: 231, Summer CFM: 103		9,343	3,473	2,021	5,494
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
Total Building Load Totals:		37,375	4,393	24,725	29,118

Check Figures

Total Building Supply CFM:	1,130	CFM Per Square ft.:	0.674
Square ft. of Room Area:	1,676	Square ft. Per Ton:	626
Volume (ft³) of Cond. Space:	15,387	Air Turnover Rate (per hour):	4.4

Building Loads

Total Heating Required With Outside Air:	37,375 Btuh	37.375 MBH	
Total Sensible Gain:	24,725 Btuh	85 %	
Total Latent Gain:	4,393 Btuh	15 %	
Total Cooling Required With Outside Air:	29,118 Btuh	2.43	Tons (Based On Sensible + Latent)
		2.68	Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 1 Main Floor Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 100% coverage	44	1,058	0	990	990
10B-f: Glazing-French door, double pane clear glass, insulated fiberglass frame, ground reflectance = 0.32	40.8	725	0	685	685
1D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	75	1,804	0	1,308	1,308
11P: Door-Polyurethane Core	19	204	0	160	160
13A-5ocs: Wall-Block, board insulation only, R-5 board insulation, open core, siding finish	1356.2	6,272	0	3,104	3,104
12B-5sw: Wall-Frame, R-11 insulation in 2 x 4 stud cavity, R-5 board insulation, siding finish, wood studs	302.6	761	0	419	419
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1675.4	1,984	0	2,306	2,306
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	206	8,994	0	0	0
Subtotals for structure:		21,802	0	8,972	8,972
People:	4		920	1,200	2,120
Equipment:			0	1,200	1,200
Lighting:	2115			7,212	7,212
Ductwork:		6,230	0	4,120	4,120
Infiltration: Winter CFM: 231, Summer CFM: 103		9,343	3,473	2,021	5,494
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
System 1 Main Floor Load Totals:		37,375	4,393	24,725	29,118

Check Figures

Supply CFM:	1,130	CFM Per Square ft.:	0.674
Square ft. of Room Area:	1,676	Square ft. Per Ton:	626
Volume (ft³) of Cond. Space:	15,387	Air Turnover Rate (per hour):	4.4

System Loads

Total Heating Required With Outside Air:	37,375 Btuh	37.375 MBH	
Total Sensible Gain:	24,725 Btuh	85 %	
Total Latent Gain:	4,393 Btuh	15 %	
Total Cooling Required With Outside Air:	29,118 Btuh	2.43 Tons (Based On Sensible + Latent)	
		2.68 Tons (Based On 77% Sensible Capacity)	

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 1, Zone 1 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
1D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 100% coverage	44	1,058	0	990	990
10B-f: Glazing-French door, double pane clear glass, insulated fiberglass frame, ground reflectance = 0.32	40.8	725	0	685	685
1D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 50% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.35 and 50% coverage	75	1,804	0	1,308	1,308
11P: Door-Polyurethane Core	19	204	0	160	160
13A-5ocs: Wall-Block, board insulation only, R-5 board insulation, open core, siding finish	1356.2	6,272	0	3,104	3,104
12B-5sw: Wall-Frame, R-11 insulation in 2 x 4 stud cavity, R-5 board insulation, siding finish, wood studs	302.6	761	0	419	419
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1675.4	1,984	0	2,306	2,306
22A-pm: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy dry or light wet soil	206	8,994	0	0	0
Subtotals for structure:		21,802	0	8,972	8,972
People:	4		920	1,200	2,120
Equipment:			0	1,200	1,200
Lighting:	2115			7,212	7,212
Ductwork:		6,230	0	4,120	4,120
Infiltration: Winter CFM: 231, Summer CFM: 103		9,343	3,473	2,021	5,494
System 1, Zone 1 Load Totals:		37,375	4,393	24,725	29,118

Check Figures

Supply CFM:	1,130	CFM Per Square ft.:	0.674
Square ft. of Room Area:	1,676	Square ft. Per Ton:	626
Volume (ft³) of Cond. Space:	15,387	Air Turnover Rate (per hour):	4.4

Zone Loads

Total Heating Required:	37,375 Btuh	37.375 MBH	
Total Sensible Gain:	24,725 Btuh	85 %	
Total Latent Gain:	4,393 Btuh	15 %	
Total Cooling Required:	29,118 Btuh	2.43	Tons (Based On Sensible + Latent)
		2.68	Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 1 Room Load Summary

No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Air Sys CFM
---Zone 1---										
1	Master Bedroom	216	5,080	66	1-7	584	3,415	904	156	156
2	Master Bath	83	2,756	36	1-4	639	1,219	275	56	56
3	Master W.I.C	45	809	11	1-3	513	551	79	25	25
4	Kitchen	171	2,687	35	1-8	482	3,679	516	168	168
5	Dining Room	189	2,039	27	1-6	524	2,251	115	103	103
6	Living Room	255	5,315	69	1-7	520	3,038	499	139	139
7	Foyer	70	3,337	44	1-6	458	1,966	399	90	90
8	Laundry Room	45	802	10	1-3	654	702	88	32	32
9	Bedroom 3	159	4,433	58	1-7	517	3,022	647	138	138
10	Bath 2	52	930	12	1-3	713	766	88	35	35
11	Bedroom 2	364	9,096	119	1-8	533	4,069	783	186	186
12	Hall	27	91	1	1-1	369	44	0	2	2
	System 1 total	1,676	37,375	488			24,725	4,393	1,130	1,130

System 1 Main Trunk Size: 15x15 in.
Velocity: 809 ft./min
Loss per 100 ft.: 0.080 in.wg

Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Total Btuh
Net Required:	2.43	85% / 15%	24,725	4,393	29,118
Recommended:	2.68	77% / 23%	24,725	7,385	32,111
Actual:	2.92	76% / 24%	26,500	8,500	35,000

Equipment Data

	Heating System	Cooling System
Type:	Air Cooled Condensor	Air Cooled Condensor
Model:	GSH130361+ARPF364216+HKR-10	GSH130361+ARPF364216
Brand:	Goodman	Goodman
Efficiency:	7.7	Seer 13
Sound:		
Capacity:	32.000	35.000
Sensible Capacity:	n/a	26,500 Btuh
Latent Capacity:	n/a	8,500 Btuh

Permit # _____
User ID _____

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)	X
1. EXTERIOR DOORS				
A. SWINGING	Masonite	Entry Door	FL. 4940.4	
B. SLIDING	HR Danvid	502 SGD	Fl 6396.5	
C. SECTIONAL/ROLL UP	Overhead Door	Garage door	FL 674	
D. OTHER				
2. WINDOWS				
A. SINGLE/DOUBLE HUNG	Kinco	M50 SH Alum. Window	Fl. 123	
B. HORIZONTAL SLIDER				
C. CASEMENT				
D. FIXED	Kinco	M50 PC Window	Fl. 125	
E. MULLION	HR	340	FL 5872	
F. SKYLIGHTS				
G. OTHER / GLASS BLOCK	Hy-Lite	Glass Block window	FL 1956.3	
3. PANEL WALL				
A. SIDING				
B. SOFFITS	Kaycan	Aluminum soffits	FL 1146.5	
C. STOREFRONTS				
D. GLASS BLOCK				
F. OTHER				
4. ROOFING PRODUCTS				
A. ASPHALT SHINGLES	Tamko	Heritage 38-R	FL. 7154	
B. NON-STRUCT METAL				
C. ROOFING TILES				
D. SINGLE PLY ROOF				
E. OTHER				
5. STRUCT COMPONENTS				
A. WOOD CONNECTORS				
B. WOOD ANCHORS	Simpson	Truss anchors	1901.17 1901.45	
C. TRUSS PLATES			1901.25 1901.21	
D. INSULATION FORMS				
E. LINTELS	Cenemt Precast	Concrete lintels	FL. 4569	
F. TRUSSES	Thomas E. Miller	engineer	PE 56877	
6. NEW EXTERIOR				
ENVELOPE PRODUCTS				
A.				

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite: 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

J D Harrington
APPLICANT SIGNATURE

2/26/07
DATE

ATTACHMENT
INFORMATION

Jax Apex Technology, Inc.
4745 Sutton Park Court, Suite 402
Jacksonville, FL 32224

All products listed in this report are currently approved for state use under the provisions of Florida Product Approval Rule 9B-72 and/or 61G15-36. Reference product approval number FL1901. All substantiating data submitted for the original application has been reviewed for compliance with the 2004 Florida Building and *Residential Codes.*

Evaluation reports are the opinion of the engineer who prepared the report, based on the findings, and in no way constitute or imply approval by a local building authority. The engineer, in review of the data submitted, finds that, in his opinion, the product, material, system, or method of construction specifically identified in this report conforms with or is a suitable alternate to that specified in the Florida Building Code, SUBJECT TO THE LIMITATIONS IN THIS REPORT

Jeffrey P. Arneson, P.E., a licensed Florida professional engineer and employee of Jax Apex Technology, Inc. (Apex Technology) has reviewed the data submitted for compliance with the Florida Building Code. Neither Jeffrey P. Arneson, nor Apex Technology, are responsible for any errors or omissions to any documents, calculations, drawings, specifications, tests, or summaries prepared and submitted by the design professional or preparer of record who are listed in the Substantiating Data section of this report.

REPORT NO: SIM200401-R2

EXPIRES: October 1st, 2008

CATEGORY: Metal Connectors

SUBMITTED BY:
SIMPSON STRONG-TIE COMPANY, INC.
4120 DUBLIN BLVD., SUITE 400
DUBLIN, CA 94568

1. PRODUCT NAME

Strap Ties
LSTA9, LSTA12, LSTA15, LSTA18, LSTA21, LSTA24, LSTA30, LSTA36, MSTA9, MSTA12, MSTA15, MSTA18, MSTA21, MSTA24, MSTA30, MSTA36, MSTC28, MSTC40, MSTC52, MSTC66, MSTC78, MST27, MST37, MST48, MST60, MST72, LSTI49, LSTI73, MSTI26, MSTI36, MSTI48, MSTI60, MSTI72, RPS18, RPS22, RPS28, ST2115, ST292, ST2122, ST2215, ST6215, ST6224, ST6236, ST9, ST12, ST18, ST22, FHA6, FHA9, FHA12, FHA18, FHA24, FHA30.

Coiled Strap Ties
CMST12, CMST14, CMSTC16, CS16, CS18, CS20, CS22

Wood to Masonry Strap Ties
MSTAM24, MSTAM36, MSTCM40

Pre-bent Strap Ties
MSTC48B3, MSTC66B3

Heavy Straps
HRS6, HRS8, HRS12

Embedded Truss Anchors

META12, META14, META16, META18, META20, META22, META24, META40, HETA12, HETA16, HETA20, HETA24, HETA40, HETAL12, HETAL16, HETAL20, HHETA12, HHETA16, HHETA20, HHETA24, HHETA40

2. SCOPE OF EVALUATION

Load Evaluation as a Structural Component using the requirements of the Florida Building and Residential Codes

3.11 MSTCB3 Pre-bent Strap Tie. The MSTC48B3 and MSTC66B3 Pre-bent Strap Ties are designed to transfer a heavy tension load from framing on an upper story wall to a beam or header on the story below. For example, this could be from shearwall overturning or a large girder truss uplift load. They are installed with 10d common nails, with a minimum of four nails in the bottom of the beam or header. Allowable loads are shown in Table 8. The straps are manufactured from 14 ga. steel meeting ASTM A-653 SS Grade 50, Class 1. They are coated with a G90 galvanized finish.

3.12 META, HETA, HETAL, HHETA Embedded Truss Anchors. Embedded Truss Anchors are used to anchor a wood member (usually a truss) to a masonry or concrete wall. Embedded truss anchors fasten to a single-ply wood truss with 10d×1½ nails or to a multiple-ply truss with 16d common nails. They are embedded in the masonry or concrete wall to a depth indicated on the side of the anchor (4" for META, HETA, and HETAL, and $5^{1}/_{16}$" for HETAL). The strap portion of the anchor is 1⅛" wide. Allowable loads are shown in Table 9 for single installations and Table 10 for double installations. The anchors are manufactured from steel meeting ASTM A-653 SS Grade 50, Class 1, with the exception of the truss seat of the HETAL which is manufactured from steel meeting ASTM A-653 SS Grade 33. Steel thickness is as specified in Table 9. The Embedded Truss Anchors are coated with a G90 galvanized finish.

4. MATERIALS

4.1 Steel. Steel specifications for each product listed in this evaluation report shall be as indicated in the previous section. In addition to the standard G90 finish, some products are available with a G185 finish, indicated as Z-Max. Allowable loads published in this report will apply to G185 products as well as G90 products.

4.2 Wood. Wood members to which these connectors are fastened shall be solid sawn lumber, glued-laminated lumber, or structural composite lumber having dimensions consistent with the connector dimensions shown in Tables1 through 4. Unless otherwise noted, lumber shall be Southern Pine or Douglas Fir-Larch having a minimum specific gravity of 0.50. Where indicated by SPF, lumber shall be Spruce-Pine-Fir having a minimum specific gravity of 0.42.

4.3 Nails and Bolts. Unless noted otherwise, nails shall be common nails. Nails shall comply with ASTM F 1667 and shall have the minimum bending yield strengths F_{yb}:

Nail Pennyweight	Nail Shank Diameter (inch)	F_{yb} (psi)
10d Common	0.148	90,000
16d Sinker	0.148	90,000
16d Common	0.162	90,000

Fasteners for galvanized connectors in pressure-preservative treated wood shall be hot-dipped zinc coated galvanized steel, except where otherwise permitted by the treatment manufacturer. Fasteners for stainless steel connectors shall be stainless steel.

4.4 Concrete/Masonry. Concrete and Masonry design specifications shall be the stricter of the specifications by the engineer of record, the Florida Building Code minimum standards, or the following:

Material	Specification	Minimum Compressive Strength
Concrete, f'c	-	2500 psi
Masonry, f'm	ASTM E447	1500 psi
Masonry Unit	ASTM C90	1900 psi
Mortar	ASTM C270 Type S	1800 psi (or by proportions)
Grout	ASTM C476	2000 psi (or by proportions)

5. INSTALLATION

Installation shall be in accordance with this report and the most recent edition of the Simpson Strong-Tie *Wood Construction Connectors* catalog. Information in this report supersedes any conflicting information between information provided in this report and the catalogue, the information in this report supersedes the catalogue.

6. SUBSTANTIATING DATA

Test data submitted by Testing Engineers Inc. and Product Testing, Inc., and signed and sealed calculations performed by Jeremy Gilstrap, P.E. in accordance with the 2004 Florida Building and Residential Codes.

7. FINDINGS

Upon review of the data submitted by Simpson Strong-Tie, it is my opinion that the connectors as described in this report conform with or are a suitable alternative to the standards and sections in the 2004 Florida Building and Residential Code editions listed in section 10 of this report. Connectors shall be installed in accordance with this report. Maximum allowable loads shall not exceed the allowable loads listed in this report.

8. LIMITATIONS

1. Maximum allowable loads shall not exceed the allowable loads listed in this report. Allowable loads listed in this report are based on allowable stress design. The loads in this report are not applicable to Load and Resistance Factor Design.
2. Capacity of wood members is not covered by this report. Capacity of wood members must be checked by the building designer.
3. Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
   (Design Uplift/Allowable Uplift) + (Design Lateral Parallel to Plate/Allowable Lateral Parallel to Plate) + (Design Lateral Perp. to Plate/Allowable Lateral Perp. to Plate) < 1.0

9. ALLOWABLE LOADS

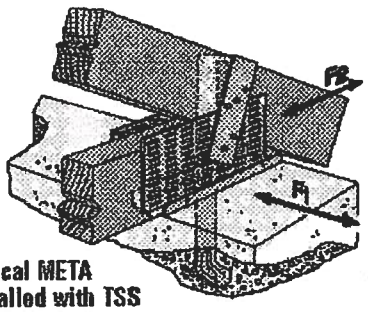
The tables that follow provide the allowable loads for the aforementioned products.

TABLE 9 ALLOWABLE LOADS

Model No.	Ga	H	Fasteners and Uplift								Lateral Loads	
			160 Load Duration Increase				133 Load Duration Increase				133/160	
			1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss		1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss		F_1 (parallel to wall)	F_2 (perpen. to wall)
			Fasteners	Load	Fasteners	Load	Fasteners	Load	Fasteners	Load		
META12	18	8	7-10d×1½	1450	6-16d	1450	7-10d×1½	1240	7-16d	1450	280	725
META14		10	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META16		12	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META18		14	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META20		16	6-10d×1½	1270	5-16d	1245	8-10d×1½	1415	6-16d	1250	280	725
			7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META22		18	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META24		20	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
META40		36	7-10d×1½	1450	6-16d	1450	9-10d×1½	1450	7-16d	1450	280	725
HETA12	16	8	7-10d×1½	1520	7-16d	1780	7-10d×1½	1265	7-16d	1475	280	725
HETA16		12	9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HETA20		16	8-10d×1½	1735	7-16d	1780	9-10d×1½	1630	8-16d	1690	280	725
			9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HETA24		20	9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HETA40		36	9-10d×1½	1810	8-16d	1810	10-10d×1½	1810	9-16d	1810	280	725
HHETA12	14	8	7-10d×1½	1565	7-16d	1820	7-10d×1½	1305	7-16d	1520	435	815
HHETA16		12	10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HHETA20		16	9-10d×1½	2010	8-16d	2080	11-10d×1½	2050	10-16d	2170	435	815
			10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HHETA24		20	10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HHETA40		36	10-10d×1½	2235	9-16d	2235	12-10d×1½	2235	11-16d	2235	435	815
HETAL12	16	7	10-10d×1½	1085	10-16d	1270	10-10d×1½	905	10-16d	1055	415	1100
HETAL16		11	14-10d×1½	1810	13-16d	1810	15-10d×1½	1810	14-16d	1810	415	1100
HETAL20		15	14-10d×1½	1810	13-16d	1810	15-10d×1½	1810	14-16d	1810	415	1100

Notes:

1. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.
2. Five nails must be installed into the truss seat of the HETAL
3. Parallel-to-plate load towards face of HETAL is 1975 lbs.
4. Except for HETAL straps, lateral loads are based on a minimum installation of 12 nails and the strap wrapped over the heel
5. Minimum f'c is 2,000psi
6. It is acceptable to use a reduced number of fasteners in a product provided that there is a reduction in load capacity. The load per nail can be approximated by dividing the allowable load by the number of fasteners. This concept applies to all member sizes. There should be a minimum of 4 nails installed in the strap.

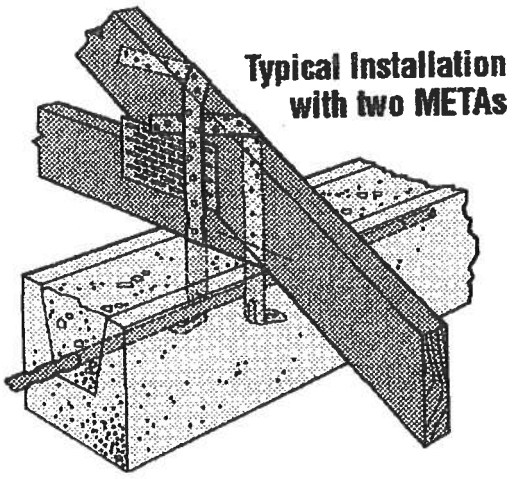


Typical META Installed with TSS

TABLE 10 ALLOWABLE LOADS FOR DOUBLE EMBEDDED TRUSS ANCHORS										
Double Embedded Anchor Installation Into Grouted CMU Bond Beam										
Model No.	Uplift - 160 Load Duration Increase				Uplift - 133 Load Duration Increase				Lateral Loads 133/160	
	1 Ply Southern Pine Truss		2 or 3 Ply Southern Pine Truss		1 Ply Southern Pine Truss		2 or 3 Ply Southern Pine Truss		F_1 (parallel to wall)	F_2 (perpen. to wall)
	Fasteners	Load	Fasteners	Load	Fasteners	Load	Fasteners	Load		
META	10-10d×1½	1985	14-16d	1900	12-10d×1½	1985	14-16d	1900	1210	1160
HETA	10-10d×1½	2035	12-16d	2500	12-10d×1½	2035	14-16d	2500	1225	1520
HHETA	10-10d×1½	2035	12-16d	2500	12-10d×1½	2035	14-16d	2500	1225	1520

Notes:

1. Minimum f'c is 2,500psi.
2. Install with spoons facing outward and spaced no more than 1/8" wider than the truss width.
3. Install half of the required number of fasteners in each strap.
4. For uplift loads for poured concrete tie beam applications with 2 or 3 ply trusses, increase the META load by 35%, the HETA load by 8%, and the HHETA load by 34%. Listed lateral loads apply to concrete applications.
5. Lateral loads apply only to anchors spaced a minimum of 3" apart.



Typical Installation with two METAs

10. CODE REFERENCES:

Florida Building Code 2004 Edition

Section 104.11	Alternate Materials and Methods
Chapter 1714.2	Load Test Procedure Specified
Chapter 21	Masonry
Chapter 22	Steel
Chapter 23	Wood

Florida Residential Code 2004 Edition

R101.2.1	Scope
R4407	HVHZ Masonry
R4408	HVHZ Steel
R4409	HVHZ Wood

11. IDENTIFICATION:

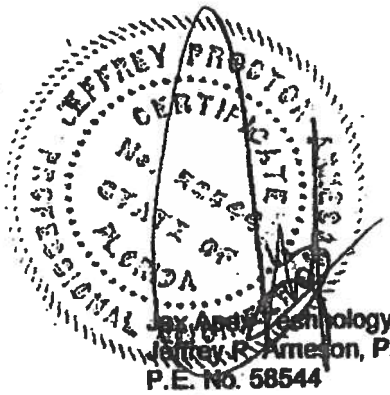
Each connector covered by this report shall be stamped with the manufacturer's name and/or trademark and the product name.

12. PERIOD OF ISSUANCE:

The content of this report expires on October 1st, 2008. For information on this report, contact Apex Technology. (904) 821-5200

13. CERTIFICATION OF INDEPENDENCE:

Jeffrey P. Ameson, the Florida engineer who prepared this report, and Apex Technology have no financial interest in the manufacturing, sales, or distribution of the products included in this report. Jeffrey P. Ameson and Apex Technology comply with all criteria as stated in Florida Administrative Code Chapter 9B-72.110.



Jax Apex Technology, Inc.
Jeffrey R. Ameson, P.E.
P.E. No. 58544
August 5, 2005

SIMPSON Strong-Tie

META/HETA/HHETA/HETAL/TSS

EMBEDDED TRUSS ANCHORS AND TRUSS SEAT SNAP-IN

The embedded truss anchor series provides an engineered method to properly attach roof trusses to concrete and masonry walls. The products are designed with staggered nail patterns for greater uplift resistance. New to this year's catalog is information regarding the use of two anchors on single- and multi-ply trusses.

The TSS, a companion product of the META, provides a moisture barrier between the concrete and truss. The preassembled unit is riveted with no height adjustment.

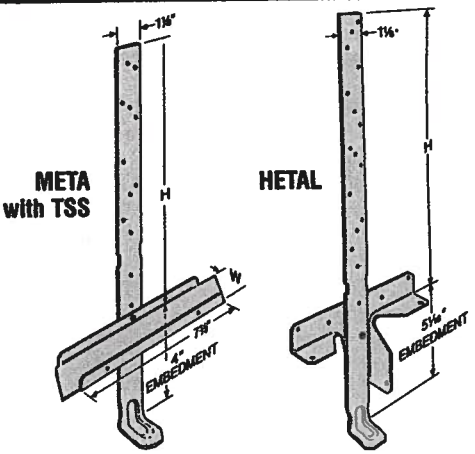
MATERIAL: HHETA–14 gauge; HETA–16 ga; HETAL strap 16 gauge, truss seat 18 gauge; META–18 gauge; TSS–22 gauge.

FINISH: Galvanized. Some products available in Z-MAX; see Corrosion Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- The META, HETA and HHETA are embedded 4" into a concrete beam or grouted block wall; HETAL is embedded 5 1/16".
- Do not drive nails through the truss plate on the opposite side of the truss, which could force the plate off the truss.
- The TSS moisture barrier may be preattached to the truss using 6d commons.

CODES: See page 10 for Code Listing Key Chart.

Model No.	W
TSS2	1¾
TSS2-2	3⅛
TSS4	3⅝

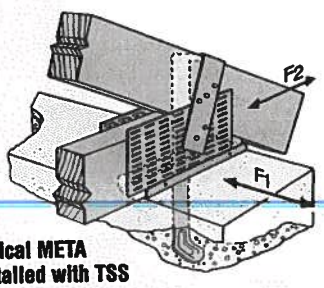


Model No.	H	Fasteners and Uplift								Lateral Loads (133 & 160)				Code Ref.
		133 Load Duration Increase				160 Load Duration Increase				DF/SP		SPF/HF		
		1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss		1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss						
		Fasteners	Load	Fasteners	Load	Fasteners	Load	Fasteners	Load	F_1	F_2	F_1	F_2	
META12	8	7-10dx1½	1240	7-16d	1450	7-10dx1½	1450	6-16d	1450	335	635	270	545	160
META14	10	9-10dx1½	1450	7-16d	1450	7-10dx1½	1450	6-16d	1450	335	635	270	545	
META16	12	9-10dx1½	1450	7-16d	1450	7-10dx1½	1450	6-16d	1450	335	635	270	545	
META18	14	9-10dx1½	1450	7-16d	1450	7-10dx1½	1450	6-16d	1450	335	635	270	545	
META20	16	8-10dx1½	1415	6-16d	1250	6-10dx1½	1270	5-16d	1245	335	635	270	545	
		9-10dx1½	1450	7-16d	1450	7-10dx1½	1450	6-16d	1450	335	635	270	545	
META22	18	9-10dx1½	1450	7-16d	1450	7-10dx1½	1450	6-16d	1450	335	635	270	545	
META24	20	9-10dx1½	1450	7-16d	1450	7-10dx1½	1450	6-16d	1450	335	635	270	545	
META40	36	9-10dx1½	1450	7-16d	1450	7-10dx1½	1450	6-16d	1450	—	—	—	—	
HETA12	8	7-10dx1½	1265	7-16d	1475	7-10dx1½	1520	7-16d	1780	335	730	270	625	8, 62
HETA16	12	10-10dx1½	1810	9-16d	1810	9-10dx1½	1810	8-16d	1810	335	730	270	625	
HETA20	16	9-10dx1½	1630	8-16d	1690	8-10dx1½	1735	7-16d	1780	335	730	270	625	
		10-10dx1½	1810	9-16d	1810	9-10dx1½	1810	8-16d	1810	335	730	270	625	
HETA24	20	10-10dx1½	1810	9-16d	1810	9-10dx1½	1810	8-16d	1810	335	730	270	625	170
HETA40	36	10-10dx1½	1810	9-16d	1810	9-10dx1½	1810	8-16d	1810	—	—	—	—	
HHETA12	8	7-10dx1½	1305	7-16d	1520	7-10dx1½	1565	7-16d	1820	335	730	270	625	160
HHETA16	12	12-10dx1½	2235	11-16d	2235	10-10dx1½	2235	9-16d	2235	335	730	270	625	
HHETA20	16	11-10dx1½	2050	10-16d	2170	9-10dx1½	2010	8-16d	2080	335	730	270	625	
		12-10dx1½	2235	11-16d	2235	10-10dx1½	2235	9-16d	2235	335	730	270	625	
HHETA24	20	12-10dx1½	2235	11-16d	2235	10-10dx1½	2235	9-16d	2235	335	730	270	625	
HHETA40	36	12-10dx1½	2235	11-16d	2235	10-10dx1½	2235	9-16d	2235	—	—	—	—	
HETAL12	7	10-10dx1½	905	10-16d	1055	10-10dx1½	1085	10-16d	1270	415	1100	355	945	8, 62
HETAL16	11	15-10dx1½	1810	14-16d	1810	14-10dx1½	1810	13-16d	1810	415	1100	355	945	
HETAL20	15	15-10dx1½	1810	14-16d	1810	14-10dx1½	1810	13-16d	1810	415	1100	355	945	

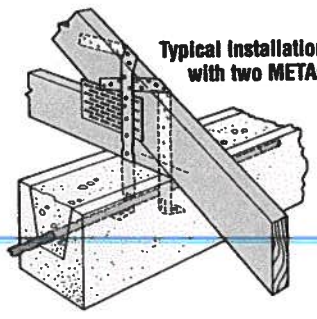
1. Loads include a 33% or 60% load duration increase on the fasteners for seismic or wind loading, but do not include a 33% stress increase on the steel capacity. Refer to page 12 for further explanation.
2. Five nails must be installed into the truss seat of the HETAL.
3. Parallel-to-plate load towards face of HETAL is 1975 lbs.
4. Lateral loads are based on a minimum installation of 12 nails and the strap wrapped over the heel.
5. Minimum f'c is 2,000psi.
6. It is acceptable to use a reduced number of fasteners in a product provided that there is a reduction in load capacity. The load per nail can be approximated by dividing the allowable load by the number of fasteners. This concept applies to all member sizes. There should be a minimum of 4 nails installed in the strap.

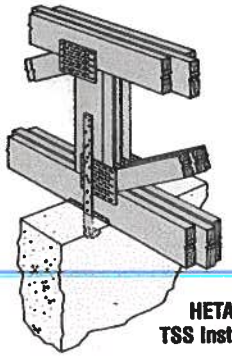
Model No.	Double Embedded Anchor Installation Into Grouted CMU Bond Beam								Lateral Loads (133 & 160)				Code Ref.
	133 Load Duration Increase				160 Load Duration Increase				DF/SP		SPF/HF		
	1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss		1 Ply So. Pine Truss		2 or 3 Ply So. Pine Truss						
	Fasteners	Load	Fasteners	Load	Fasteners	Load	Fasteners	Load	F_1	F_2	F_1	F_2	
META	12-10dx1½	1985	14-16d	1900	10-10dx1½	1985	14-16d	1900	1210	1160	1040	1000	160
HETA	12-10dx1½	2035	14-16d	2500	10-10dx1½	2035	12-16d	2500	1225	1520	1055	1305	

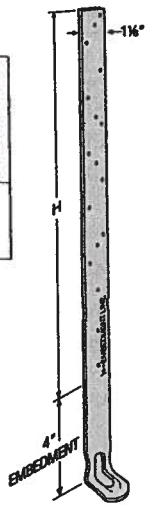
1. For concrete tie beam applications for 2 or 3 ply trusses, increase the META load 35% and the HETA load 8%.
2. Divide total number of fasteners equally between both straps.
3. Minimum f'c is 2,500 psi.
4. See instruction to the Designer page 9 for loads in multiple directions.
5. Lateral loads are based on a minimum installation of 12 nails and the strap wrapped over the heel.



Typical META Installed with TSS


Typical Installation with two METAs


Typical HETA20 with TSS Installation



HETA20 (HHETA similar)

/MGT/HGT HEAVY GIRDER TIEDOWNS

SIMPSON Strong-Tie

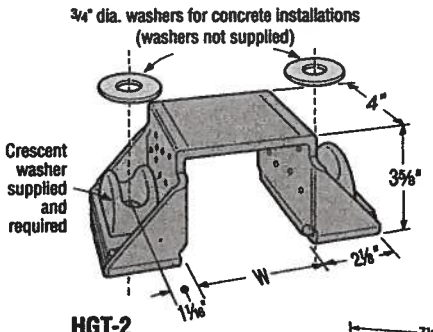
...GT provide lighter load alternatives for ...he HGT-4 is sized for 4-2x widths. This series ...uplift resistance for wood frame and concrete block ...n. The HGT can be installed on trusses and beams ...chord slopes from 3:12 to 8:12. Available in 2-ply, ...and 4-ply widths.

MATERIAL: LGT—14 ga; MGT—12 ga; HGT—7 ga.

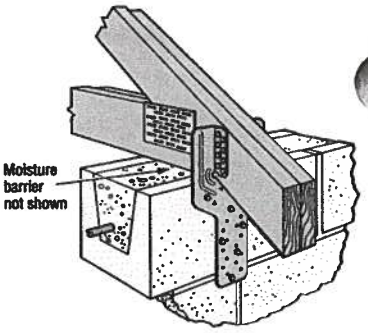
FINISH: HGT—Simpson gray paint;
LGT, MGT— galvanized

INSTALLATION:
- When the HGT-3 is used with a 2-ply girder or beam, shimming is required. Fasten to act as one unit.
- Attach to grouted concrete block with a minimum one #5 rebar horizontal in the top lintel block.
- Minimum f'_c = 2500 psi maximum aggregate ⅜".

CODES: See page 10 for Code Listing Key Chart.



HGT-2
(HGT-3 and HGT-4 similar)

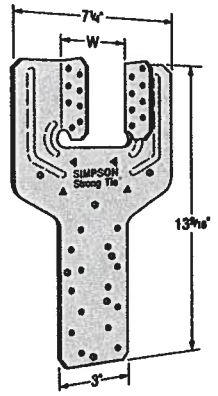


Typical LGT2 Installation into Masonry

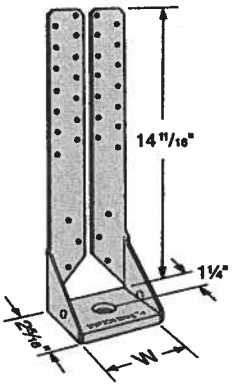
Model No.	W	O.C. Dim Between Anchors	Fasteners: Anchor Dia. Concrete	Fasteners: Girder	Avg Ult	DFL/SP Allowable Loads (133/160)	SPF Allowable Loads (133/160)	Code Ref.
MGT	3¾	—	1-⅝	22-10d	13005	3965	3330	160
HGT-2	3⁵⁄₁₆	5¾	2-¾	16-10d	35400	10980	6485	6,38, 62
HGT-3	4¹⁵⁄₁₆	7⅜	2-¾	16-10d	35580	10530	9035	
HGT-4	6⁹⁄₁₆	9	2-¾	16-10d	28805	9250	9250	

Masonry Application

Model No.	W	O.C. Dim Between Anchors	Fasteners: CMU	Fasteners: Concrete	Fasteners: Girder	Avg Ult	DFL/SP Allowable Loads (133/160)	SPF Allowable Loads (133/160)	Code Ref.
LGT2	3⅛	—	7-¼x2¼ Titen	7-¼x1¾ Titen	16-16d Sinker	6533	2150	1850	160

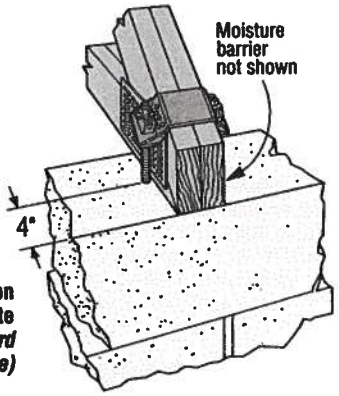


LGT2

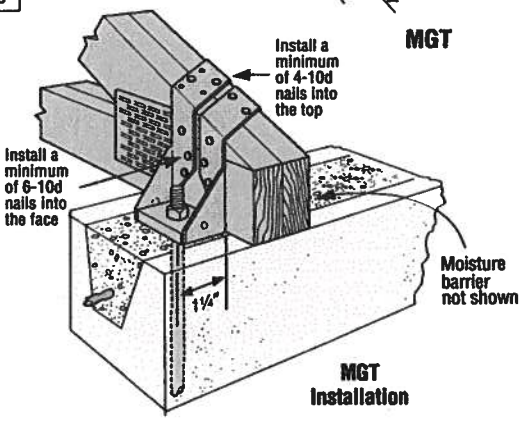


MGT

1. Attached members must be designed to resist applied loads.
2. To achieve the loads listed, anchorage into a concrete block bond beam shall be designed by the building designer.
3. To achieve the loads listed for the HGT, anchorage into a 8" wide concrete tie-beam can be made using Simpson SET epoxy with a ¾" diameter anchor and a minimum embedment depth of 12".
4. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.



Typical HGT-2 Installation into Concrete
(¾" diameter standard washers required for concrete)



MGT Installation

MTSM/HTSM TWIST STRAPS

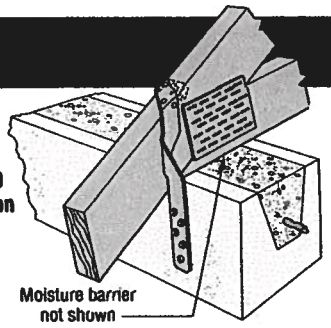
The MTSM and HTSM offer high strength truss to masonry connections.

MATERIAL: MTSM–16 gauge; HTSM–14 gauge

FINISH: Galvanized. Some products available in stainless steel and Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- Attach to grouted concrete block with a minimum one #5 rebar horizontal.
- Minimum f'_c = 2500 psi maximum aggregate ⅜".

CODES: See page 10 for Code Listing Key Chart.



Typical MTSM20 Installation

Model No.	L	Fasteners[2]: Truss	Fasteners[2]: CMU	Fasteners[2]: Concrete	Doug-Fir-Larch/So Pine Allowable Uplift Loads[1]: 10d (133/160)	Doug-Fir-Larch/So Pine Allowable Uplift Loads[1]: 10dx1½" (133)	Doug-Fir-Larch/So Pine Allowable Uplift Loads[1]: 10dx1½" (160)	Spruce-Pine-Fir Allowable Uplift Loads[1]: 10d (133/160)	Spruce-Pine-Fir Allowable Uplift Loads[1]: 10dx1½" (133)	Spruce-Pine-Fir Allowable Uplift Loads[1]: 10dx1½" (160)	Code Ref.
MTSM16	16	7-10d	4-¼x2¼ Titen	4-¼x1¾ Titen	860	840	860	750	730	750	160
MTSM20	20	7-10d	4-¼x2¼ Titen	4-¼x1¾ Titen	860	840	860	750	730	750	
~~HTSM16~~	~~16~~	~~8-10d~~	~~4-¼x2¼ Titen~~	~~4-¼x1¾ Titen~~	~~1175~~	~~1045~~	~~1175~~	~~1020~~	~~905~~	~~1020~~	
HTSM20	20	10-10d	4-¼x2¼ Titen	4-¼x1¾ Titen	1175	1045	1175	1020	1020	1020	

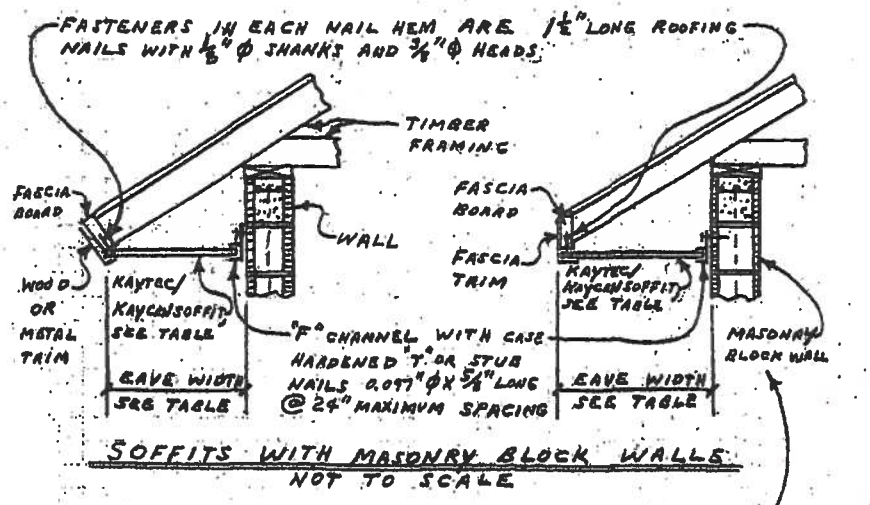
MTSM20

MTSM16

1. Loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed; reduced where other loads govern.
2. Twist straps do not have to be wrapped over the truss to achieve the allowable load.
3. Minimum edge distance for Titens is 1½".

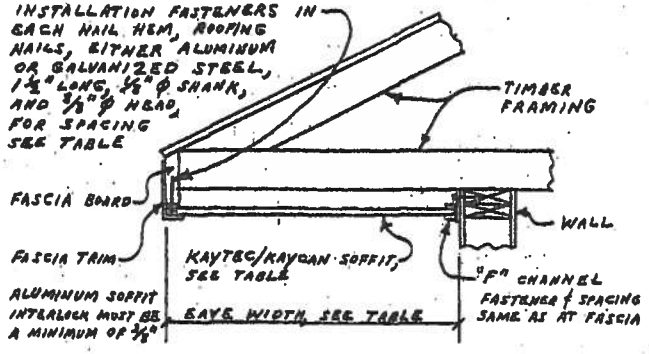
ALUM FASICA & SOFFIT

ALLOWABLE DESIGN WIND PRESSURES IN PSF					
KAYTEC/KAYCAN SOFFIT TYPE	EAVE WIDTH IN INCHES				
	12"	15"	18"	21"	24"
SP-600 (ALUMINUM) 16" VENTED CATALOG # 0605	+/- 72.6	+/- 58.1	+/- 48.4	+/- 41.5	+/- 36.3
SP-600 (ALUMINUM) 16" SOLID CATALOG # 0606	+/- 72.2	+/- 57.7	+/- 48.1	+/- 41.2	+/- 36.1
VENTED PANEL 12" (ALUMINUM) CATALOG # 9500	+/- 96.6	+/- 77.2	+/- 64.4	+/- 55.2	+/- 48.3
SOLID PANEL 12" (ALUMINUM) CATALOG # 9510	+/- 96.0	+/- 76.8	+/- 64.0	+/- 54.9	+/- 48.0
FULL O-VENT 7" (12") VINYL PRODUCT CODE: 0622	+/- 47.5	+/- 36.5	+/- 29.7	+/- 25.2	+/- 21.6
ECO SOLID, CENTER VENT & FULL VENT 7" (12") VINYL PROD. CODES 0623, 0623, 0631	+/- 45.3	+/- 34.9	+/- 28.4	+/- 23.9	+/- 20.6
SOLID, CENTER VENT & FULL VENT 7" (12") VINYL PROD. CODES 0623, 0623, 0631	+/- 45.3	+/- 34.9	+/- 28.4	+/- 23.9	+/- 20.6
SOLID & VENTED D5 (10") VINYL PROD. CODES 0626, 0635	+/- 54.4	+/- 41.8	+/- 34.0	+/- 28.6	+/- 24.7
BEADED, SOLID AND VENTED 7 & 67 (8") VINYL PROD. CODES 0632, 0635	+/- 68.0	+/- 52.3	+/- 42.5	+/- 35.8	+/- 30.9



SOFFITS WITH MASONRY BLOCK WALLS
NOT TO SCALE

ALTERNATE WALL DETAIL
NOT TO SCALE



SOFFIT WITH WOOD FRAMED WALL
NOT TO SCALE

GENERAL NOTES

1. DESIGN PRESSURES LISTED IN TABLE ARE BASED ON A RATIONAL ANALYSES FOR ALUMINUM SOFFITS DONE IN HR ENGINEERING PROJECT 04020002, AND A COMPARATIVE ANALYSES FOR VINYL SOFFITS DONE IN 04050001 THAT ARE IN CONFORMANCE WITH FLORIDA BUILDING CODE 2004 SECTION 1609 "WIND LOADS".

2. FLORIDA BUILDING CODE 2004 DOES NOT DIRECTLY MANDATE WIND LOAD DESIGN PRESSURES ON SOFFITS.

HR Engineering, Inc.	DATE: 14 JULY 2005	PROJECT NO. 05070005 SHEET 1 OF 1
CLIENT: KAYTEC INC.	BY: A. REEVES	PROJECT NAME: KAYTEC/KAYCAN SOFFITS

Allen M. Reeves
14 July 2005

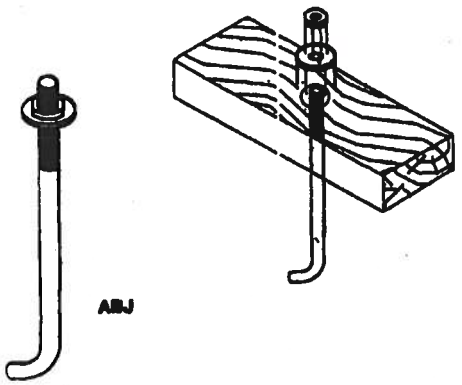
SEMCO™
METAL CONNECTORS
Technical Support (800) SE SPECS - (800) 737-7327

ANCHOR BOLT WITH NUT & 1" WASHER

Design Features:
- Bolt only with code minimum embedment with 3000 PSI concrete will resist 4,800 lbs, see washer capacity below.

Materials: Black and galvanized steel

Footnote: Other sizes available on request. All references to bolts or MB's are structural quality through bolts equal to or better than ASTM Standard A307.



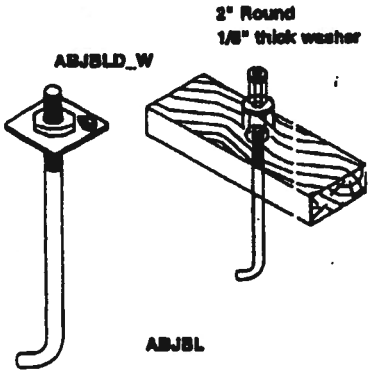
SIZE	PRODUCT CODE	DESCRIPTION	PER CTN
1/2 X8	ABJBL8C	Black	50
1/2 X9	ABJBL9C	Black	50
1/2 X10	ABJBL10C	Black	50
1/2 X12	ABJBL12C	Black	50
1/2 X14	ABJBL14C	Black	50
1/2 X15	ABJBL15C	Black	50
1/2 X18	ABJBL18C	Black	50
1/2 X8	ABJGA8	Galv.	50
1/2 X9	ABJGA9	Galv.	50
5/8 X10	ABJBL10D	Black	50
5/8 X12	ABJBL12D	Black	50

ANCHOR BOLT (WITH NUT & 2"X2"X1/8" WASHER)

Design Features:
- 6" minimum embedment with 3000 PSI concrete will resist 1,635 lbs.

Materials: Black steel

Footnote: *Supplied with a 2" round washer 1/8" thick. Wind uplift loads are based on the shear capacity of No. 2 Southern Pine. Compression perpendicular to grain 565 (psi).



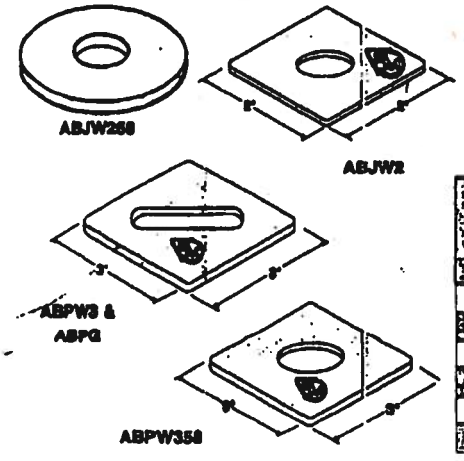
SIZE	PRODUCT CODE	DESCRIPTION	PER CTN
1/2x8	ABJBL8W	Black	50
1/2x9	ABJBL9W	Black	50
1/2x10	ABJBL10W	Black	50
1/2x12	ABJBL12W	Black	50
5/8x12	ABJBL12DW*	Black	50

ANCHOR BOLT WASHER/PLATE

Design Features:
- The washer/plate adds increased resistance to wind uplift for bottom sill plate anchor bolt.

Materials: 10 Gauge Galvanized & 1/8 & 1/4 Black steel

Footnote: *Also available in 50# ctn approximately 345 pcs. Wind uplift loads are based on the shear capacity of No. 2 Southern Pine. Compression perpendicular to grain 565 (psi). +Round hole in washer.



SIZE (INCHES)	GAUGE	PRODUCT CODE	ANCHOR BOLT (DIAMETER)	ALLOWABLE LOADS: WIND/EARTHQUAKE UPLIFT 160%	UPLIFT 100%	PER CTN
2x2	1/8	ABJW2*	1/2"	1635	1635	50
2x2	1/4	ABJW2SQ*	5/8"	1470	1470	50
3x3	10	ABPG12	1/2"	3675	3675	40
3x3	10	ABPG58	5/8"	3675	3675	40
3x3	1/4	ABPW3	1/2"	4800	4800	40
3x3	1/4	ABPW3SQ	5/8"	4800	4800	40

WINDOWS, DOORS, AND MULL BARS INFORMATION

NOTICE OF PRODUCT CERTIFICATION

CERTIFICATION NO: NI006592
DATE: 06/16/2006
CERTIFICATION PROGRAM: Structural
COMPANY: Atrium
CODE: A-447-1

The "Notice of Product Certification" is valid only when Administrator's Seal is applied to the upper left hand portion of this form and a certification label is applied to the product. This certification seal represents product conformity to the applicable specification and that all certification criteria has been satisfied.

The product described below is approved for listing in the Directory of Certified Products at www.NAMICertification.com . Please review, and advise NAMI immediately if data, as shown, requires corrections.

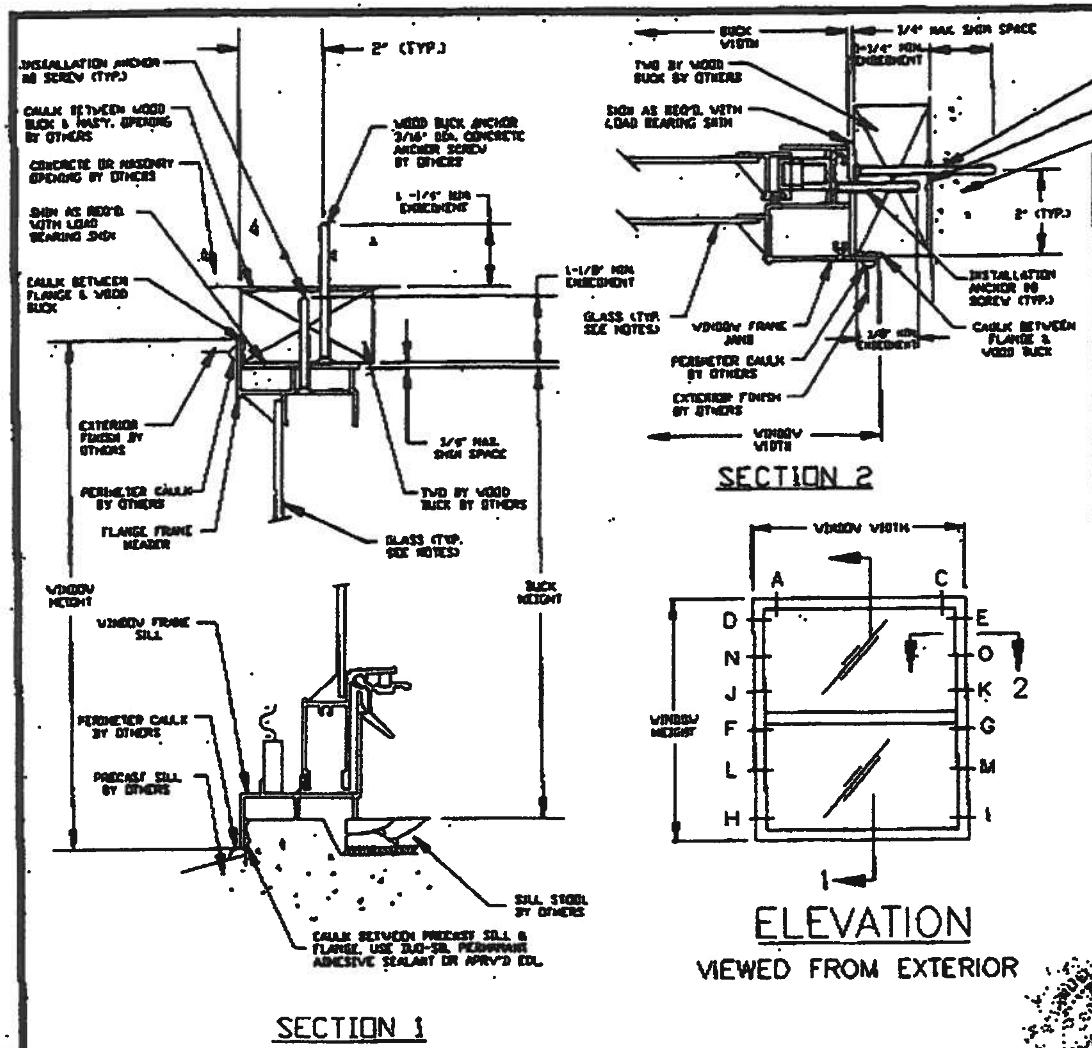
COMPANY NAME AND ADDRESS	PRODUCT DESCRIPTION
Atrium Windows & Doors-Florida 3600 Port Jacksonville Parkway Jacksonville, FL 32226	"Mark 40/50 Premium" Aluminum Single Hung Standard Flange Frame Window Configuration: OX Glazing: O-3/16"Annealed Glass/X-5/32" Annealed Glass STP PSF Frame: W-4'5" Sash: W-4'2" Pos+60.0 H-8'1" H-3'3" Neg-67.5

SPECIFICATION	PRODUCT RATING
AAMA/NWWDA 101/I.S.2-97/ AAMA 1302.5-76 Glass Complies to ASTM E1300-02	H-LC35 FER-Passed

Product Tested By: Certified Testing Laboratories
Report No: CTLA-1049W (Structural/FER)
Expiration Date: March 31, 2007

Administrator's Signature: _____

NATIONAL ACCREDITATION AND MANAGEMENT INSTITUTE, INC.
11870 Merchants Walk Suite 202
Newport News, VA 23606
TEL: (757) 594-8658
FAX: (757) 594-8659



ANCHOR LOCATION CHART					
CALL SIZE	BUCK SIZE	LOCATIONS IN HEAD		LOCATIONS IN JAMB	
		UP TO 45PSF	46PSF TO 70PSF	UP TO 45PSF	46PSF TO 70PSF
12	18 X 25	A,C	A,C	[illegible]	[illegible]
13	18 X 37 3/8	A,C	A,C	[illegible]	[illegible]
14	18 X 49 5/8	A,C	A,C	[illegible]	[illegible]
15	18 X 62	A,C	A,C	[illegible]	[illegible]
16	18 X 71	A,C	A,C	[illegible]	[illegible]
17	18 X 83	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
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[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
23	[illegible]	A,C	A,C	[illegible]	[illegible]
24	[illegible]	A,C	A,C	[illegible]	[illegible]
25	[illegible]	A,C	A,C	[illegible]	[illegible]
26	[illegible]	A,C	A,C	[illegible]	[illegible]
27	[illegible]	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
[illegible]	[illegible]	A,C	A,C	[illegible]	[illegible]
33	[illegible]	A,C	A,C	[illegible]	[illegible]
34	[illegible]	A,C	A,C	[illegible]	[illegible]
35	52 X 62	A,C	A,C	[illegible]	[illegible]
36	52 X 71	A,C	A,C	[illegible]	[illegible]
37	52 X 83	A,C	A,C	[illegible]	[illegible]
38	52 X 95	A,C	A,C	[illegible]	[illegible]

NOTES:

1) SHIM AS REQ'D. AT EACH INSTALLATION ANCHOR WITH LOAD BEARING SHIM. MAX. ALLOWABLE SHIM STACK TO BE 1/4". USE SHIMS WHERE SPACE GREATER THAN 1/16" IS PRESENT.
2) WINDOW FRAME MATERIAL: ALUMINUM ALLOY 6063.
3) #8 INSTALLATION ANCHOR SCREW MUST BE OF SUFFICIENT LTH. TO ACHIEVE MIN. EMBEDMENT OF 1-1/8" INTO WOOD BUCK. (APPROX. LENGTH 1 3/8")
4) CAULK BEHIND WINDOW FLANGE WITH DUO-SIL ADHESIVE CAULK OR APPROVED EQUAL. APPLICATION OF DUO-SIL ADHESIVE CAULK SEALANT MUST COMPLY WITH SEALANT MANUFACTURER'S RECOMMENDATIONS.
5) USE LATEX CAULK OR EQUIVALENT FOR PERIMETER SEAL AROUND EXTERIOR OF WINDOW FLANGE BY OTHERS.
6) IF EXACT WINDOW SIZE IS NOT LISTED IN ANCHOR CHART, USE ANCHOR QUANTITY LISTED WITH NEXT LARGER SIZE, FOR THE APPROPRIATE DESIGN PRESSURE REQ'D.
7) GLASS THICKNESS MAY VARY PER THE REQUIREMENTS OF ASTM E1300 GLASS CHARTS. SINGLE GLAZING IS SHOWN, INSULATING GLASS ALSO QUALIFIED.
8) LETTER DESIGNATIONS ON THE ANCHOR CHART INDICATE WHERE ANCHORS ARE TO BE INSTALLED WHEN USING THE EXTERIOR ELEVATION AS A KEY.
9) NOT ALL ANCHORS SHOWN ON ELEVATION ARE APPLIED BY FACTORY IN ALL SIZES. ALL FACTORY APPLIED HOLES NOT DESIGNATED FOR ANCHORS SHOULD BE PUNCHED 6" FROM CORNERS & 24" O.C. MAX. ALL FACTORY APPLIED HOLES NOT DESIGNATED FOR ANCHORS SHOULD BE FILLED WITH #8 SCREWS OF SUFFICIENT LENGTH TO PROVIDE MIN. 1-1/8" EMBEDMENT INTO WOOD BUCK.
10) CONCRETE SCREW ANCHORS FOR THE ATTACHMENT OF WOOD BUCKS ARE TO BE 3/16" DIA. & OF SUFFICIENT LENGTH TO ACHIEVE MIN. EMBEDMENT OF 1-1/4" INTO CONCRETE OR MASONRY. SPACE CONCRETE SCREW ANCHORS 6" FROM END & 24" O.C. MAX.

KINCO, LTD. 3245 OLD KINGS ROAD JACKSONVILLE, FLORIDA 32254			
TITLE: INSTALLATION DETAIL W/TWO BY WOOD BUCK FLG. SINGLE HUNG WINDOW H40/50 & H40/50 HP			
ENGINEER: MANUEL MARTINEZ	DRN: BB	DATE: 11/16/01	
DISCIPLINE: CIVIL	SCALE: N.T.S.	DWG. NO.	
FL. REG. NO. 47182	REV. LETTER	SHEET 1 OF 1	

REV.	DESCRIPTION	DATE

PREPARED BY: PRODUCT & APPLICATION ENGINEERING, INC. [illegible]

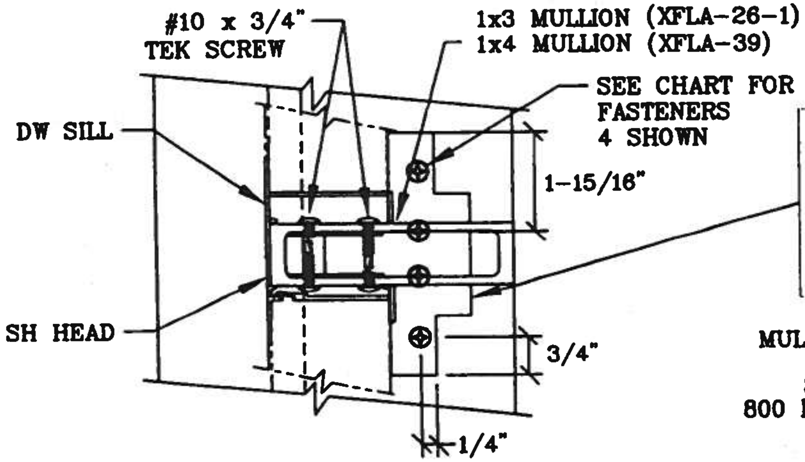
NOTES:

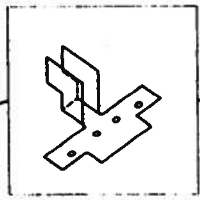
1) ALL ALUMINUM EXTRUSIONS ARE ALLOY 6063 T6, OR 6063 T5.
2) WHEN THERE IS ONE TAPCON (1/4ø" X 1-1/2") ON EACH ANGLE LEG, THE TAPCON SHALL BE PLACED ON MULLION CLIP CENTERLINE.
3) CONCRETE COMPRESSIVE STRENGTH = 3,000 PSI AT 28 DAYS.

CHARLES A. HAGEN, P.E.
FL. REG. ENG. # [illegible]
DATE: 3/21/02

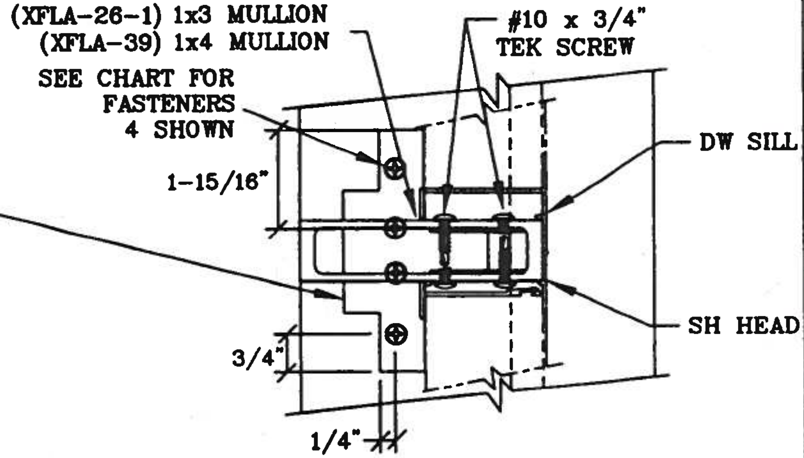
HORIZONTAL MULLION SCHEDULE

SINGLE UNIT WINDOW WIDTH INCH	SINGLE UNIT WINDOW HEIGHT INCH	TYPE OF MULLION DESIGN PRESSURE 35 PSF	TYPE OF CLIP FLA-45	NUMBER AND TYPE OF FASTENERS
19-1/8"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	63"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	76-3/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
26-1/2"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	63"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	76-3/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
37"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	63"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	76-3/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
53-1/8"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(2) 1/4ø" x 1-1/2" TAPCONS
	63"	1.0 x 3.0	OK	(2) 1/4ø" x 1-1/2" TAPCONS
	76-3/4"	1.0 x 3.0	OK	(2) 1/4ø" x 1-1/2" TAPCONS





FLA-45
MULLION ANCHOR CLIP
16 GA. GALV.
SHEET METAL
800 LB. MAX CAPACITY



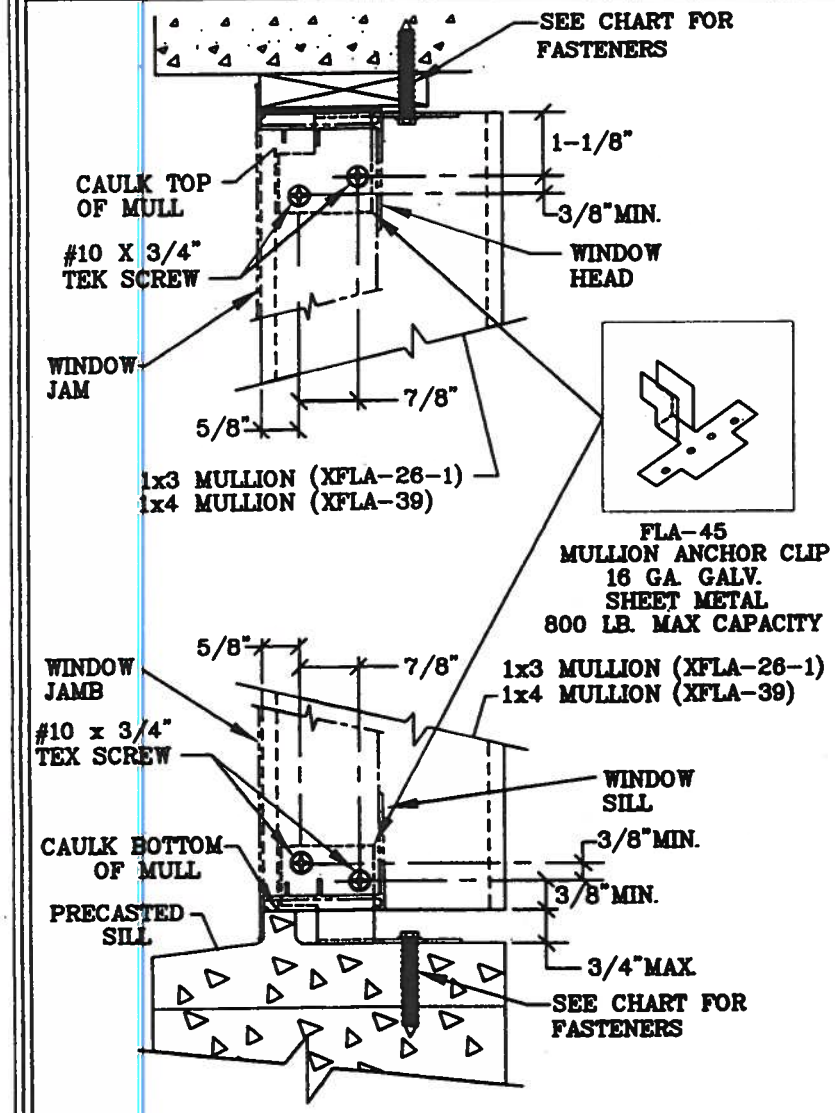
HORIZONTAL MULLION
FLA-45 ANCHOR CLIP
INSTALLATION DETAIL AND
FASTENER SCHEDULE

SERIES: MULLION
ALUMINUM SINGLE HUNG

NORANDEX
4606 30th STREET WEST
BRADENTON, FL. 34207
PHONE: (941) 756-1691

NO.	DATE	REVISIONS DESCRIPTION

3/11/02
SCALE: N.T.S.
DWG. BY: RAE
CHK. BY: RLK
DWG. NO.: FBC-030



VERTICAL MULLION SCHEDULE

SINGLE UNIT		TYPE OF MULLION	TYPE OF CLIP	NUMBER AND TYPE OF FASTENERS
WINDOW WIDTH INCH	WINDOW HEIGHT INCH	DESIGN PRESSURE 35 PSF	FLA-45	
19-1/8"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	63"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	76-3/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
26-1/2"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
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37"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	63"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	76-3/4"	1.0 x 4.0	OK	(4) 1/4ø" x 1-1/2" TAPCONS
53-1/8"	26"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	38-1/4"	1.0 x 3.0	OK	(4) 3/16ø" x 1-1/2" TAPCONS
	50-5/8"	1.0 x 3.0	OK	(2) 1/4ø" x 1-1/2" TAPCONS
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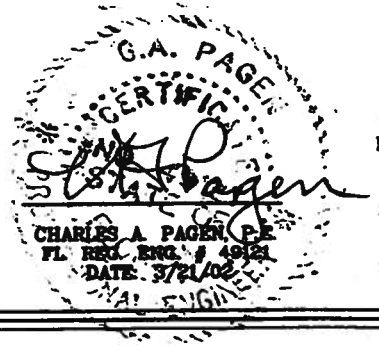


NOTES:
1) ALL ALUMINUM EXTRUSIONS ARE ALLOY 6063 T6, OR 6063 T5.
2) WHEN THERE IS ONE TAPCON (1/4ø" X 1-1/2") ON EACH ANGLE LEG, THE TAPCON SHALL BE PLACED ON MULLION CLIP CENTERLINE.
3) CONCRETE COMPRESSIVE STRENGTH = 3,000 PSI AT 28 DAYS.

VERTICAL MULLION
FLA-45 ANCHOR CLIP
INSTALLATION DETAIL AND
FASTENER SCHEDULE

SERIES: MULLION
ALUMINUM SINGLE HUNG

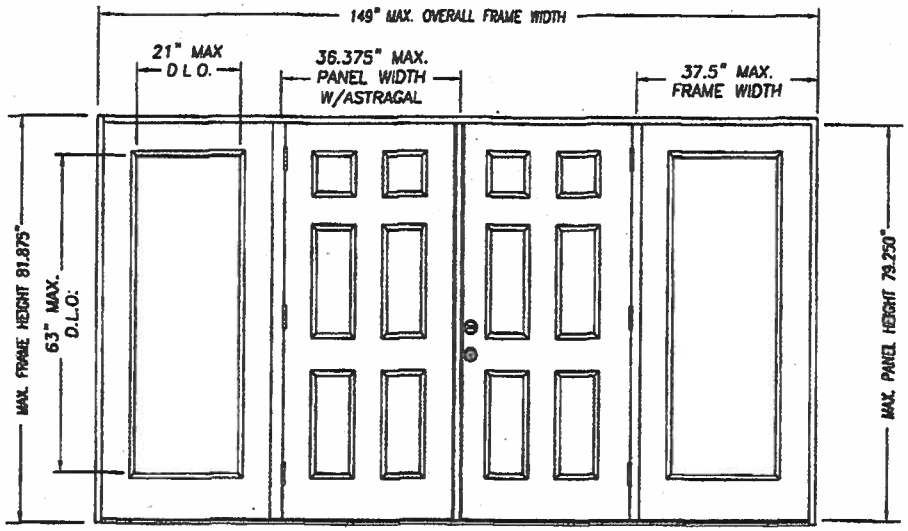
NORANDEX
4501 60th STREET WEST
BRADENTON, FL. 34207
PHONE: (941) 755-1691

NO.	DATE	REVISIONS DESCRIPTION

4/11/02
SCALE: N.T.S.
DWG. BY: RLE
CHK. BY: RLE
DWG. NO.: 725-038

Masonite

SIDE-HINGED METAL-EDGE STEEL DOOR UNIT
6'-8" DOUBLE DOOR WITH / WITHOUT SIDELITES



DOUBLE INSWING UNIT W/SIDELITES

Addendum to NOA#
Certification No.: NT006115
Reviewed By:
Date Reviewed: 8/12/05

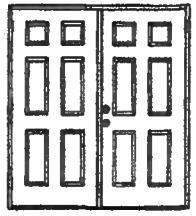
GENERAL NOTES

1. EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLORIDA BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED.
2. HURRICANE PROTECTIVE SYSTEM (SHUTTERS) IS NOT REQUIRED ON OPAQUE PANELS, BUT IS REQUIRED ON GLAZED SIDELITES
3. POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 60 PER ASTM E84.
4. PLASTICS TESTING OF LITE FRAME MATERIAL:

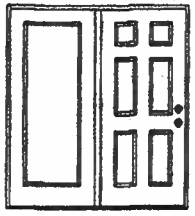
TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	680 °F > 650 °F
RATE OF BURNING	ASTM D635	1.10 IN/MIN
SMOKE DENSITY	ASTM D2843	69.6%
TENSILE STRENGTH*	ASTM D638	-7.48% DIFF

* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1

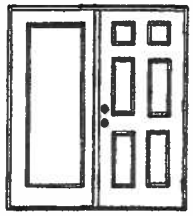
SINGLE DOOR UNIT



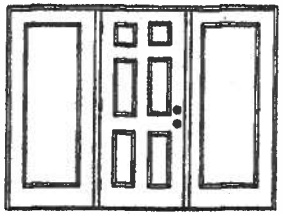
DOUBLE DOOR UNIT



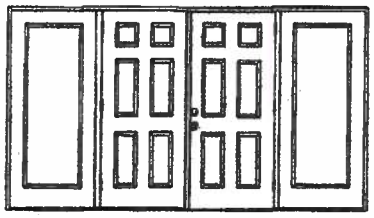
SINGLE DOOR UNIT WITH SIDELITE



SINGLE DOOR UNIT WITH SIDELITE



SINGLE DOOR UNIT W/SIDELITES



DOUBLE DOOR UNIT W/SIDELITES

TABLE OF CONTENTS

CONFIG	MAX WIDTH	DESIGN PRESSURE RATING INSWING	DESIGN PRESSURE RATING OUTSWING	WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE INSWING	WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE OUTSWING
X	37.5"	+76.0 / -76.0	+76.0 / -76.0	+19.0 / -19.0	+55.0 / -55.0
XX	74"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0
OX or XO	75"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0
OXO	112.5"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0
OXXO	149"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0

MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

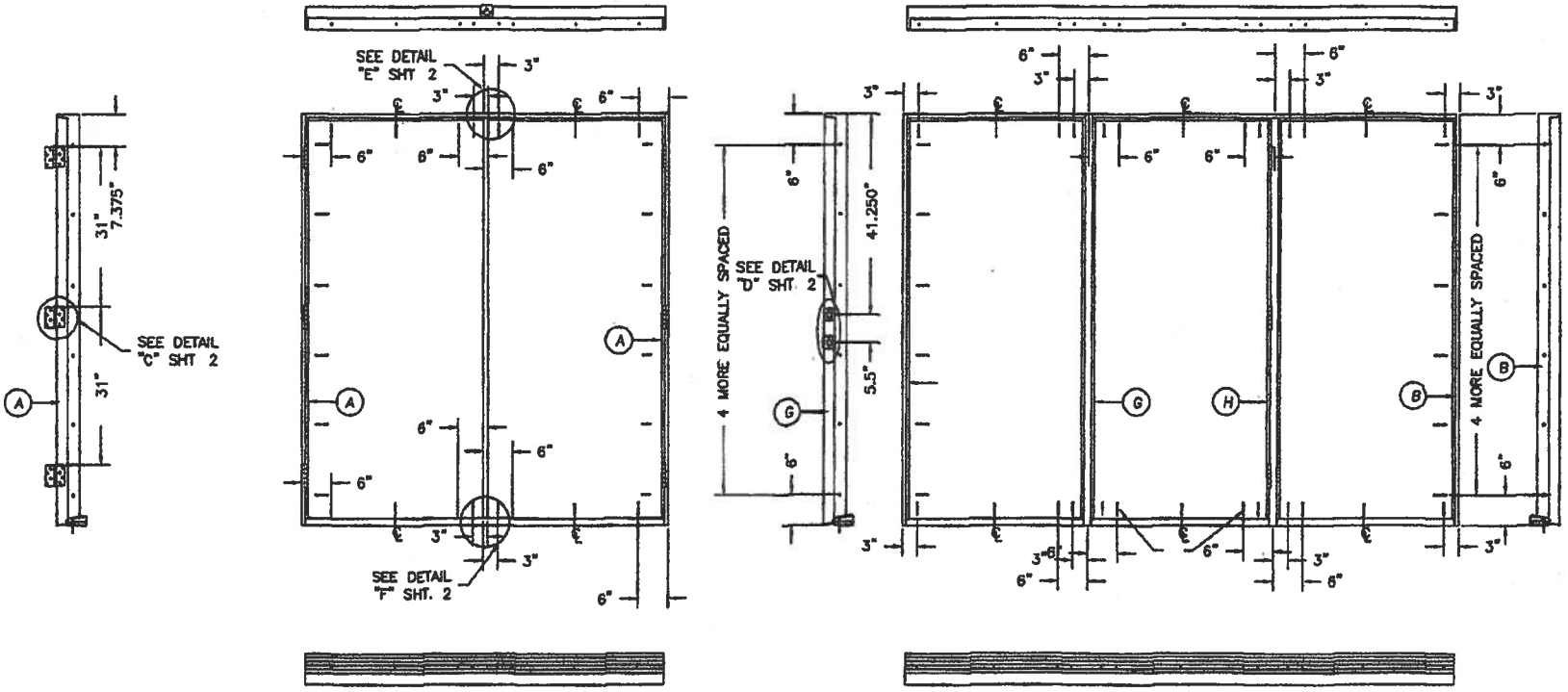
PRODUCT: "EXTERIOR DOOR PRODUCT" DOUBLE 6'8" OPAQUE METAL-EDGE STEEL DOOR

PART OR ASSEMBLY: TYPICAL ELEVATIONS & GENERAL NOTES

REVISIONS

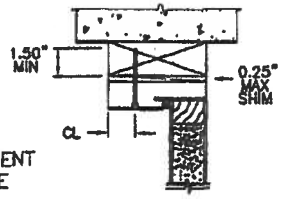
NO.	DATE	BY

DATE: 7/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0132-05
SHEET 1 OF 3



ATTACHMENT DETAIL

1. ANCHOR ANALYSIS FOR LOADING CONDITIONS PREPARED, SIGNED AND SEALED BY HAROLD E. RUPP, PE (FLORIDA #15935) WITH THE LOWEST (LEAST) FASTENER RATING FROM THE DIFFERENT FASTENERS BEING CONSIDERED FOR USE. JAMB, HEAD, AND THRESHOLD FASTENERS ANALYZED FOR THIS UNIT INCLUDE #10 WOOD SCREWS OR 3/16" TAPCONS. A PHYSICAL SHIM MUST BE PLACED IN SHIM SPACE AT EACH ANCHOR LOCATION.
2. THE WOOD SCREW SINGLE SHEAR DESIGN VALUES COME FROM ANSI/AF&PA NDA FOR SOUTHERN PINE LUMBER AND ACHEIVEMENT OF 1-1/2" MINIMUM EMBEDMENT. THE TAPCON MUST ACHIEVE MINIMUM EMBEDMENT OF 1-1/4".
3. WOOD BUCKS BY OTHERS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE.
4. MINIMUM DESIGN VALUE STRENGTH OF ANCHORS 171 LBS.



TYPICAL ANCHOR INSTALLATION

HARDWARE SCHEDULE

1.	KWIKSET OR SCHLEGE ANSI/BHMA GRADE 3 OR BETTER CYLINDRICAL AND DEADLOCK HARDWARE TO BE INSTALLED AT 5-1/2" CENTERLINE.
2.	4" X 4" FULL MORTISE BUTT HINGES

Addendum to NAMI
Certification No.: NI006115
Reviewed By:
Date Reviewed: 8/12/05

MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-0" METAL-EDGE STEEL DRAPE DOUBLE DOOR UNIT
PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS

NO.	DATE	BY
	REVISIONS	

DATE: 7/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0132-05
SHEET 3 OF 3

GARAGE DOORS INFORMATION

TECHNICAL DATA SHEET #155o

GARAGE DOOR WIND LOAD GUIDE
BASED ON THE 2001 FLORIDA BUILDING CODE (ASCE 7-98) EXPOSURE B

Mean Roof Height	Door Size	90 MPH	100 MPH	110 MPH	120 MPH	130 MPH	140 MPH	150 MPH
Less than 30 Feet	Single 9'x7'	-14.5	-17.9	-21.6	-25.8	-30.2	-35.1	-40.2
	Double 16'x7'	-13.7	-16.9	-20.4	-24.3	-28.5	-33.1	-38.0

Design pressures above are in Pounds per Square Foot (PSF)

Testing, if required by local authority, may be performed to ASTM E-330, or preferably DASMA 108.
Impact and cyclic wind pressure testing on glazed doors may be performed to ASTM E-1886, or preferably DASMA 115.

Test conditions:

1. Garage doors shall be tested to both negative and positive pressures. Doors shall be installed simulating normal conditions (i.e., top roller in track radius, other rollers in tracks, all hinges in place, reinforcing hardware in place)
2. Total test duration for each test direction shall be as follows:
   A. Total of 3600/V seconds, at design pressure; where V is fastest-mile design wind speed.
   B. Pressure equal to 1.5 times the design pressure shall be included for 10 seconds during each test.

The door successfully passes the test if it remains safely operable through the full travel up and down, and recovers at least 75% of its maximum deflection. Standard engineering principles may be used to interpolate or extrapolate test results to door sizes not specifically tested. Doors shall include a manufacturer's label certifying compliance to specific load.

This guide is provided for reference purposes only. In all cases the local building authority is the sole and final determiner of the structural and safety requirements, and suitability of the garage door.

Notes:

- Basic Wind Speeds above are three second peak-gust values
- Negative pressures assume door has 2 feet of width in building's end zone.
- Garage doors evaluated as attached to enclosed buildings with a Use Factor of 1.0
- Doors larger than 100 square feet should use the 16 x 7 loads. Doors less than 100 square feet may be interpolated.
- Garage doors evaluated as Components and Cladding
- Installation details vary. Consult manufacturer's instructions.

For more information, contact DASMA, 1300 Sumner Avenue, Cleveland OH 44115-2851
Phone (216) 241-7333 E-mail: dasma@dasma.com Fax (216) 241-0105 URL: www.dasma.com

REVISIONS			
[illegible]	[illegible]	[illegible]	[illegible]
-	RFP PER EN [illegible]	[illegible]	[illegible]
A	REV PER CN [illegible]	[illegible]	[illegible]
B	REV PER EN [illegible]	[illegible]	[illegible]

▷ NOTES

1. ALL THE LOAD FROM THE DOOR IS TRANSFERRED TO THE TRACK AND THEN FROM THE TRACK TO THE VERTICAL JAMBS (SPF STUD GRADE OR BETTER). NO LOAD FROM THE DOOR IS TRANSFERRED TO THE HORIZONTAL (TOP) JAMB WITH STANDARD WINDLOAD SYSTEM. WITH VERTICAL WINDLOAD POST, PART OF THE FORCE IS TRANSFERRED TO THE HORIZONTAL GARAGE DOOR HEADER.

2. EACH VERTICAL JAMB SEES A MAXIMUM DESIGN LOAD OF +2980 LB & -2980 LB AND A MAXIMUM TEST LOAD OF +4480 LB & -4480 LB. THE HORIZONTAL GARAGE DOOR HEADER SEES A MAX TEST LOAD OF 1200# FOR SINGLE POST, AND MAX COMBINED LOAD OF 2970# FOR MULTIPLE POSTS.

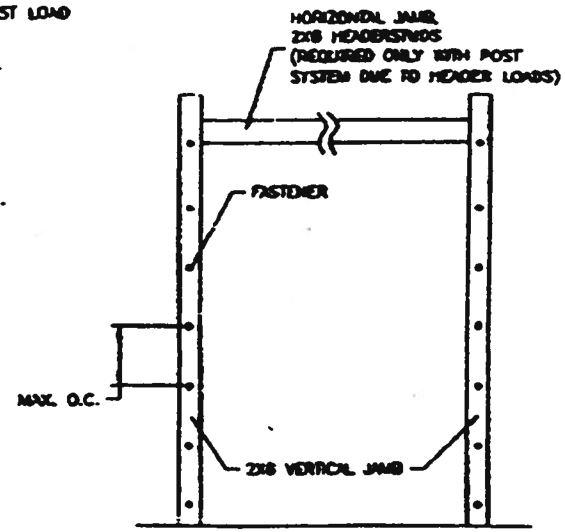
3. ALL JAMB FASTENERS MAY BE (BUT NOT REQUIRED) COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.

WOOD FRAME BUILDINGS

STUD WALLS OF DOOR OPENING SHALL BE FRAMED SOLID BY NOT LESS THAN 2 FULL LENGTH STUDS AND 2 HEADERSTUDS USING SPF STUD GRADE OR BETTER WOOD. STUD WALLS TO BE CONTINUOUS FROM FOOTING TO TIE BEAMS AND IN ACCORDANCE WITH SFBC SECTION 2905.1. INSTALLATION IN ACCORDANCE WITH DWG 409783 IS AN ACCEPTABLE ALTERNATIVE.

BLOCK WALL OR CONCRETE

2X4 MIN. WOOD JAMB SHALL BE ANCHORED TO GROUT REINFORCED BLOCK WALL OR CONCRETE COLUMN. BLOCK WALL CELLS SHALL BE FILLED WITH CONCRETE AND REINFORCED WITH #5 BAR EXTENDING INTO THE FOOTING AND INTO THE BEAMS. (STRENGTH IS ASSUMED TO BE 2500 PSI). ALL BARS SHALL BE CONTINUOUS FROM THE TIE BEAMS TO FOOTING PER BLOCK WALL OR CONCRETE COLUMN. BLOCK WALLS AND CONCRETE COLUMNS TO BE DESIGNED BY BUILDING PROFESSIONAL OF RECORD AND IN ACCORDANCE WITH SFBC SECTION 2704.2.



2X6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

(NOT TO BE USED FOR ATTACHMENT OF TRACK BRACKETS TO 2X8 VERTICAL JAMBS OR SUPPORTING STRUCTURE)

BUILDING TYPE	FASTENER TYPE	MIN. NO. OF FASTENERS PER VERTICAL JAMB		MAXIMUM ON CENTER DISTANCE BETWEEN FASTENERS	STEEL WASHERS REQUIRED?
		7' HIGH	8' HIGH		
WOOD FRAME (SPF)	5/16" X 3" LAG SCREW (ASTM A307, GRADE A), 1-5/8" MIN. EMBED.	7	8	16"	YES
C-90 BLOCK (2,500 PSI GROUT)	1/4" x 4" ITW TAPCON CONCRETE ANCHOR, 1-3/4" MIN. EMBED.*	7	8	16"	YES
C-90 BLOCK (2,500 PSI GROUT)	3/8" X 4" RAWL LOK/BOLT ANCHOR BOLT, 1-5/8" MIN. EMBED.*	8	7	16"	NO
CONCRETE COLUMN (2,500 PSI)	3/8" X 4" RAWL LOK/BOLT ANCHOR BOLT, 1-5/8" MIN. EMBED.*	6	7	16"	NO

* - TAPCONS/ANCHOR BOLTS CAN BE INSTALLED DIRECTLY THROUGH TRACK BRACKETS/ANGLE IN LIEU OF 5/16" X 1-5/8" LAG SCREWS. RAWL LOK/BOLT SHALL BE TORQUED AS SPECIFIED BY THE RAWL DRILLING AND ANCHORING SYSTEMS DESIGN MANUAL.

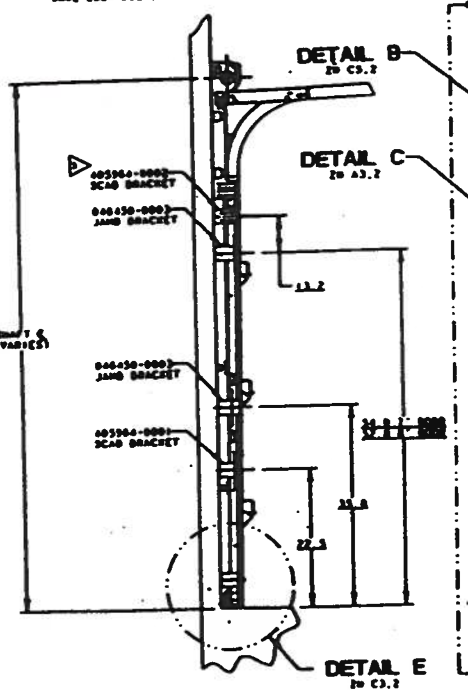
APPROVED

DRAWN BY: G. FINEMAN	8/12/98	DRAWING TITLE: RESIDENTIAL JAMB DETAIL	
MATERIAL: N/A	CHECKED BY: PE VESSELS	8/19/98	DRAWING NUMBER: C-409431
N/A	N/A	APPROVED BY: DAVID FAW	8/19/98	SCALE: FULL	SHEET 1 of 1

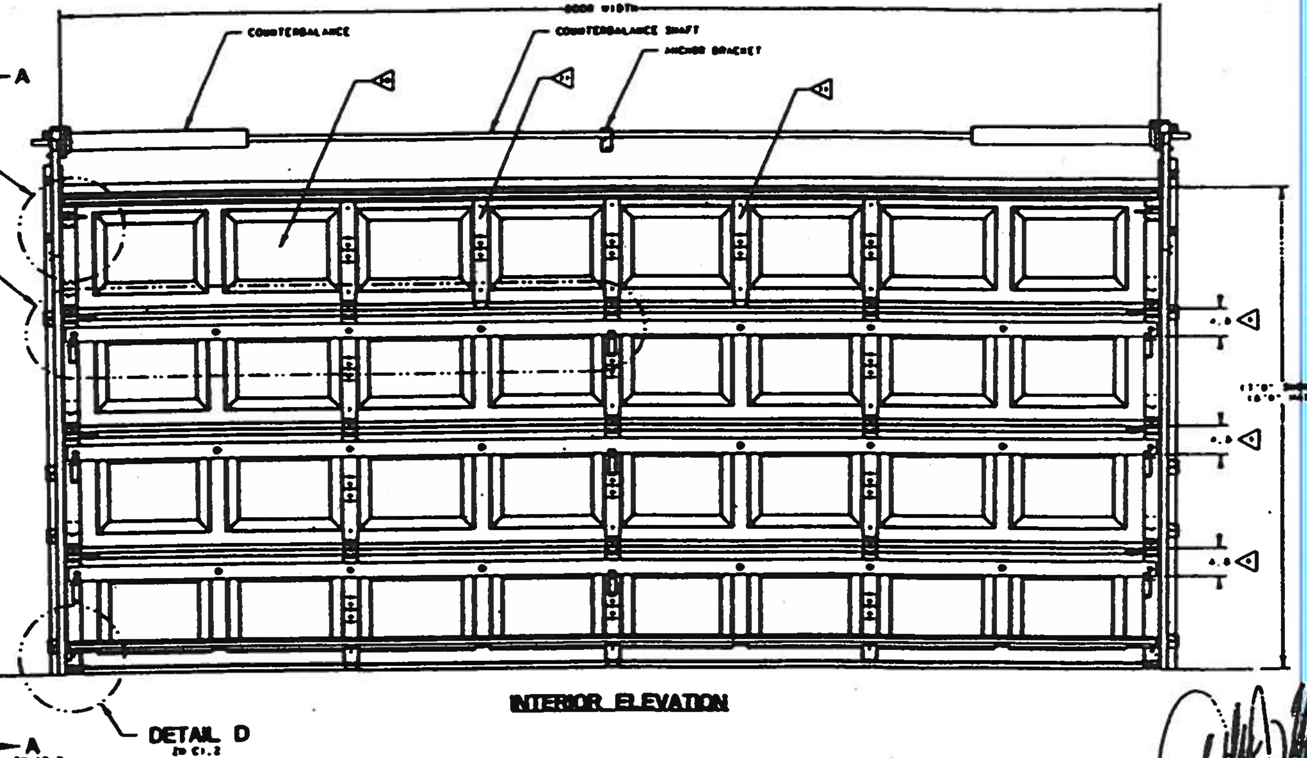
NOTES:
1. TESTED IN ACCORDANCE WITH DASMA 108.
2. FOUR SECTION 7' TALL DOOR SHOWN. 8' DOORS HAVE 3 SECTIONS. SECTION HEIGHTS OF 20.00, 19.00 AND 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
3. EMBOSSMENT PATTERN OF 14.5 X 20.4 SHOWN. ALTERNATE PATTERNS OF 12.5 X 43.4 AND 12.5 X 20.4 MAY BE USED.
4. ADVANTAGE/REVOLUTION COUNTERBALANCE SHOWN. TORSION AND EXTENSION COUNTERBALANCE SYSTEMS AVAILABLE. 403964-0002 REQUIRED AT TRACK INTERFACE ON TORSION AND EXTENSION SYSTEMS.
5. USE THIS BRACKET 403964-0002 ON 8' HIGH DOORS ONLY.
6. LOCATE 409317 CHANNELS AS SHOWN. VERTICAL PLACEMENT CAN VARY PLUS/MINUS 3 INCHES.
7. ON THE MIDDLE CENTER STILE THERE IS NO TOOL CLEARANCE HOLE FOR THE UPPER 003911-0001 SCREW. INSTALL FROM BELOW.
8. USE 003911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.
9. JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783. JAMB/SCAB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: 003370-0001 SCREW, LAG 5/16 X 1-3/4 - WOOD JAMB. BAR LOCK BOLT, 5/16 X 1-1/2 EMBEDMENT - CONCRETE BLOCK
10. WINDOW INSTALLATION ALLOWED. WINDOWS MUST BE INSTALLED IN TOP SECTION.
11. EXTRA CENTER STILES ARE NOT REQUIRED ON DOORS WITHOUT A WINDOW SECTION.
12. USE 18 GAUGE CHANNEL

REVISIONS				
[illegible]	-	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]

OHD/GENIE DOOR EQUIVALENCY CHART		
OHD DOOR SERIES No.	GENIE DOOR SERIES No.	PAN STEEL THICKNESS
SERIES 100	SERIES GD0300	26 GA
SERIES 180	SERIES GD0300	26 GA
SERIES 201	SERIES GD0200	25 GA
SERIES 200	SERIES GD0300	25 GA



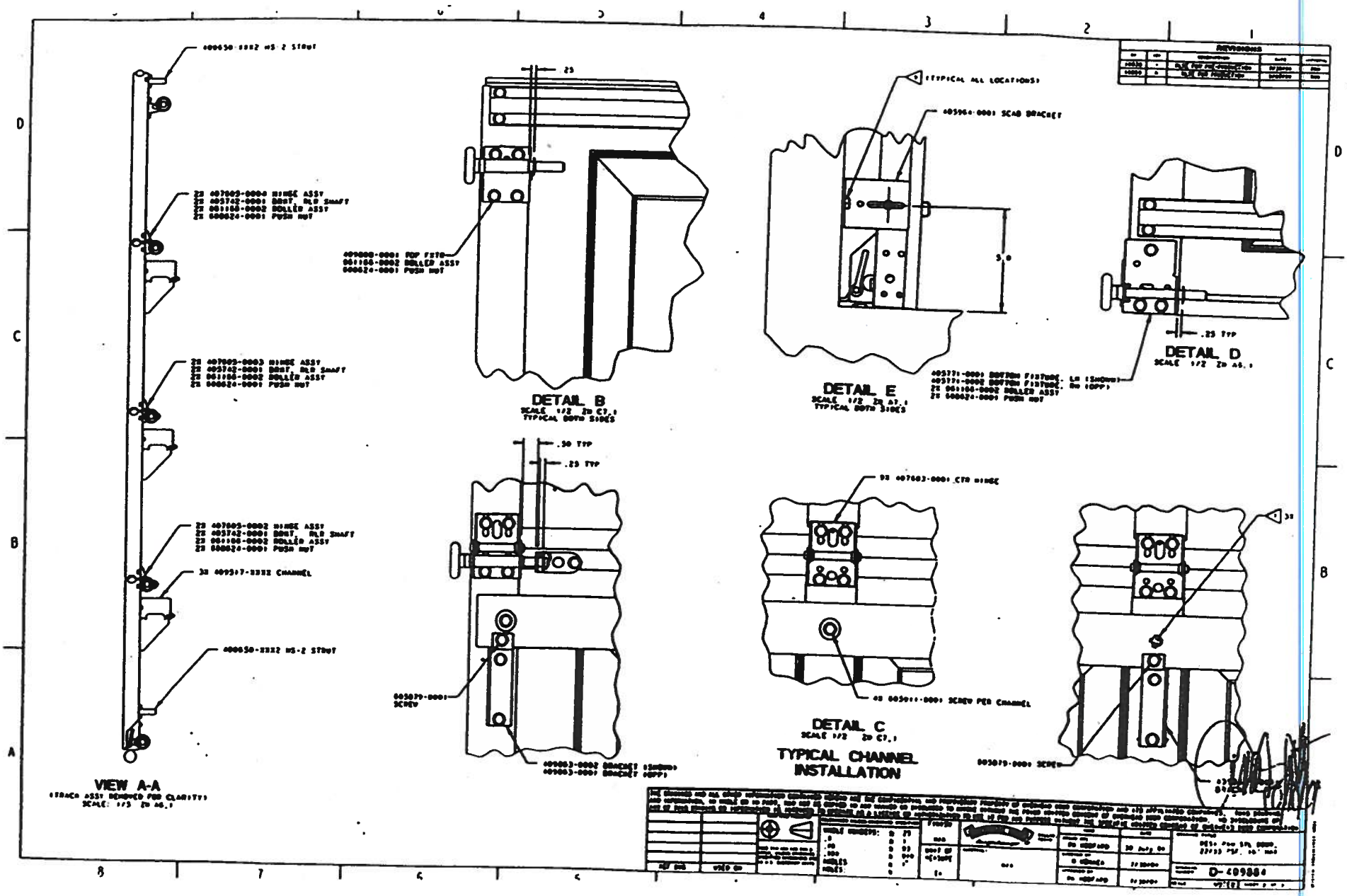
STD TRACK DETAIL FOR 16' WIDE DOOR



INTERIOR ELEVATION

DOOR WIDTH	CENTER STILE	END STILE	[illegible]	STRUTS/SECT	ROLLER	VERT TRK GA	JAMB LOAD
16'	[illegible]	SINGLE	YES	[illegible]	[illegible]	[illegible]	[illegible]

D-40988C


VIEW A-A
DETAIL B
DETAIL C
TYPICAL CHANNEL INSTALLATION
DETAIL D
DETAIL E
D-409884

ROOFING INFORMATION

TAMKO BUILDING PRODUCTS

Application Instructions for

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS

LAMINATED ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

TAMKO does not recommend re-roofing over existing roof.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents. FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

3. FASTENERS

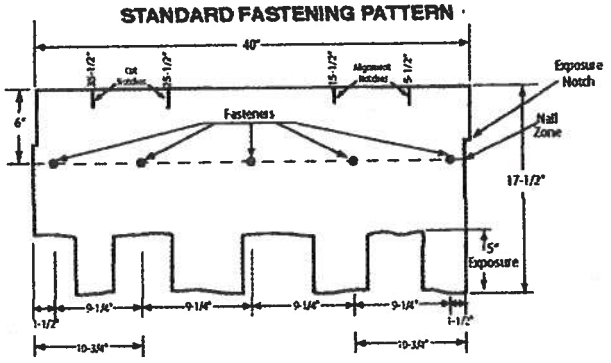
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, this will result in the termination of TAMKO's liabilities under the limited warranty. TAMKO will not be responsible for damage to shingles caused by winds in excess of the applicable miles per hour as stated in the limited warranty. See limited warranty for details.

FASTENING PATTERNS: Fasteners must be placed 6 in. from the top edge of the shingle located horizontally as follows:

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1-1/2 in. back from each end, one 10-3/4 in. back from each end and one 20 in. from one end of the shingle for a total of 5 fasteners. (See standard fastening pattern illustrated below).

STANDARD FASTENING PATTERN



2) Mansard or Steep Slope Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) Use standard nailing instructions with four additional nails placed 6 in. from the butt edge of the shingle making certain nails are covered by the next (successive) course of shingles.

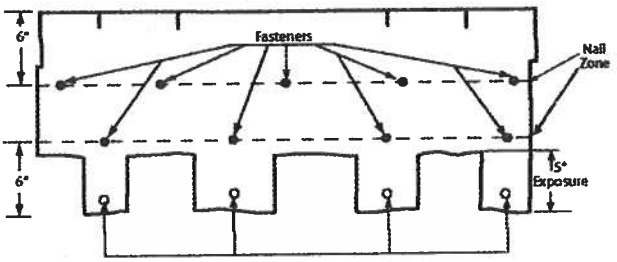
(Continued)

(CONTINUED from Pg. 1)

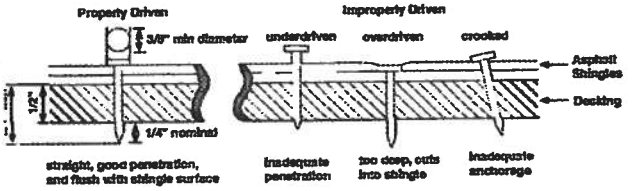
• HERITAGE® VINTAGE™ AR – Phillipsburg, KS
LAMINATED ASPHALT SHINGLES

Each shingle tab must be sealed underneath with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a $.25 piece and applied to shingles with a 5 in. exposure, use 9 fasteners per shingle.

MANSARD FASTENING PATTERN



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12 gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in. into the roof deck. Where the deck is less than 3/4 in. thick, the nails should be long enough to penetrate completely through plywood decking and extend at least 1/8 in. through the roof deck. Drive nail head flush with the shingle surface.



4. UNDERLAYMENT

UNDERLAYMENT: An underlayment consisting of asphalt saturated felt must be applied over the entire deck before the installation of TAMKO shingles. Failure to add underlayment can cause premature failure of the shingles and leaks which are not covered by TAMKO's limited warranty. Apply the felt when the deck is dry. On roof decks 4 in. per foot and greater apply the felt parallel to the eaves lapping each course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left exposed, the underlayment felt may be adversely affected by moisture and weathering. Laying of the underlayment and the shingle application must be done together.

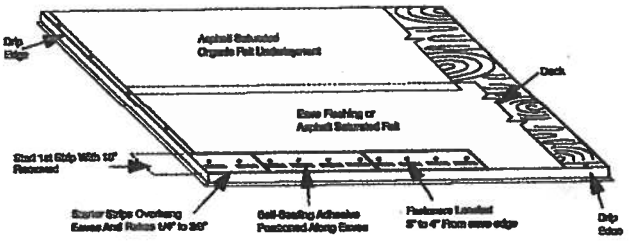
Products which are acceptable for use as underlayment are:

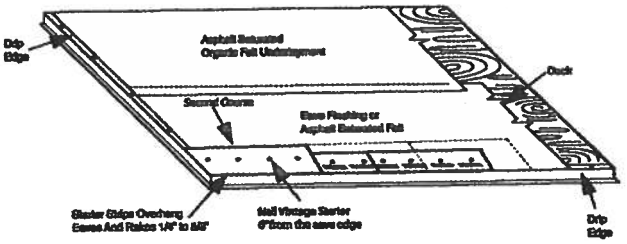
- TAMKO No. 15 Asphalt Saturated Organic Felt
- A non-perforated asphalt saturated organic felt which meets ASTM: D226, Type I or ASTM D4869, Type I
- Any TAMKO non-perforated asphalt saturated organic felt
- TAMKO TW Metal and Tile Underlayment, TW Underlayment and Moisture Guard Plus® (additional ventilation maybe required. Contact TAMKO's technical services department for more information)

In areas where ice builds up along the eaves or a back-up of water from frozen or clogged gutters is a potential problem, TAMKO's Moisture Guard Plus® waterproofing underlayment (or any specialty eaves flashing product) may be applied to eaves, rakes, ridges, valleys, around chimneys, skylights or dormers to help prevent water damage. Contact TAMKO's Technical Services Department for more information.
TAMKO does not recommend the use of any substitute products as shingle underlayment.

5. APPLICATION INSTRUCTIONS

STARTER COURSE: Two starter course layers must be applied prior to application of Heritage Vintage AR Shingles.
The first starter course may consist of TAMKO Shingle Starter, three tab self-sealing type shingles or a 9 inch wide strip of mineral surface roll roofing. If three tab self-sealing shingles are used, remove the exposed tab portion and install with the factory applied adhesive adjacent to the eaves. If using three tab self-sealing shingles or shingle starter, remove 18 in. from first shingle to offset the end joints of the Vintage Starter. Attach the first starter course with approved fasteners along a line parallel to and 3 in. to 4 in. above the eave edge. The starter course should overhang both the eave and rake edge 1/4 in. to 3/8 in. Over the first starter course, install Heritage Vintage Starter AR and begin at the left rake edge with a full size shingle and continue across the roof nailing the Heritage Vintage Starter AR along a line parallel to and 6 in. from the eave edge.





Note: Do not allow Vintage Starter AR joints to be visible between shingle tabs. Cutting of the starter may be required.

HERITAGE VINTAGE STARTER AR
12 1/2" x 36" 20 PIECES PER BUNDLE
60 LINEAL FT. PER BUNDLE

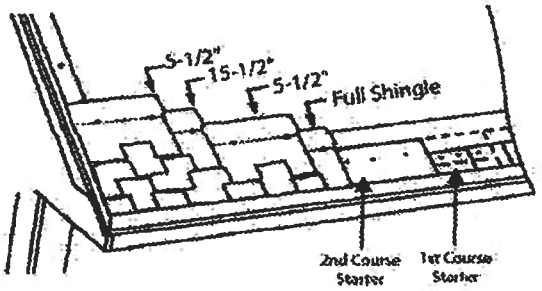
(Continued)

(CONTINUED from Pg. 2)

TAMKO BUILDING PRODUCTS

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS
LAMINATED ASPHALT SHINGLES

SHINGLE APPLICATION: Start the first course at the left rake edge with a full size shingle and overhang the rake edge 1/4 in. to 3/8 in.. To begin the second course, align the right side of the shingle with the 5-1/2 in. alignment notch on the first course shingle making sure to align the exposure notch. (See shingle illustration on next page) Cut the appropriate amount from the rake edge so the overhang is 1/4" to 3/8". For the third course, align the shingle with the 15-1/2 in. alignment notch at the top of the second course shingle, again being sure to align the exposure notch. Cut the appropriate amount from the rake edge. To begin the fourth course, align the shingle with the 5-1/2 in. alignment notch from the third course shingle while aligning the exposure notch. Cut the appropriate amount from the rake edge. Continue up the rake in as many rows as necessary using the same formula as outlined above. Cut pieces may be used to complete courses at the right side. As you work across the roof, install full size shingles taking care to align the exposure notches. Shingle joints should be no closer than 4 in.



8. LOW SLOPE APPLICATION
On pitches 2 in. per foot to 4 in. per foot cover the deck with two layers of underlayment. Begin by applying the underlayment in a 19 in. wide strip along the eaves and overhanging the drip edge by 1/4 to 3/4 in. Place a full 36 in. wide sheet over the 19 in. wide starter piece, completely overlapping it. All succeeding courses will be positioned to overlap the preceding course by 19 in. If winter temperatures average 25°F or less, thoroughly cement the laps of the entire underlayment to each other with plastic cement from eaves and rakes to a point of a least 24 in. inside the interior wall line of the building. As an alternative, TAMKO's Moisture Guard Plus self-adhering waterproofing underlayment may be used in lieu of the cemented felts.

7. VALLEY APPLICATION
TAMKO recommends an open valley construction with Heritage Vintage AR shingles.

To begin, center a sheet of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment in the valley.

After the underlayment has been secured, install the recommended corrosion resistant metal (26 gauge galvanized metal or an equivalent) in the valley. Secure the valley metal to the roof deck. Overlaps should be 12" and cemented.

Following valley metal application; a 9" to 12" wide strip of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment should be applied along the edges of the metal valley flashing (max. 6" onto metal valley flashing) and on top of the valley underlayment. The valley will be completed with shingle application.

SHINGLE APPLICATION INSTRUCTIONS (OPEN VALLEY)

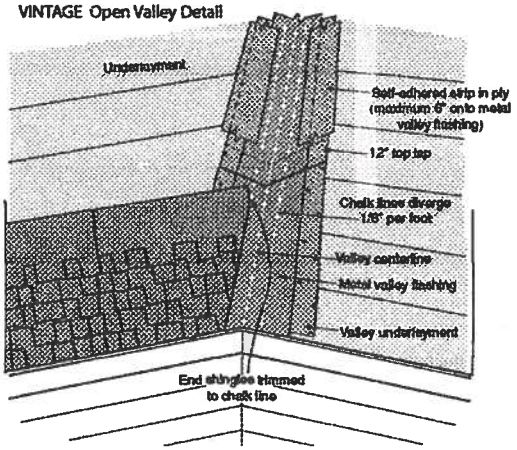
- Snap two chalk lines, one on each side of the valley centerline over the full length of the valley flashing. Locate the upper ends of the chalk lines 3" to either side of the valley centerline.
- The lower end should diverge from each other by 1/8" per foot. Thus, for an 8' long valley, the chalk lines should be 7" either side of the centerline at the eaves and for a 16' valley 8".

As shingles are applied toward the valley, trim the last shingle in each course to fit on the chalk line. Never use a shingle trimmed to less than 12" in length to finish a course running into a valley. If necessary, trim the adjacent shingle in the course to allow a longer portion to be used.

- Clip 1" from the upper corner of each shingle on a 45° angle to direct water into the valley and prevent it from penetrating between the courses.

- Form a tight seal by cementing the shingle to the valley lining with a 3" width of asphalt plastic cement (conforming to ASTM D 4586).

VINTAGE Open Valley Detail



• **CAUTION:**
Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.

(Continued)

(CONTINUED from Pg. 3)

TAMKO BUILDING PRODUCTS

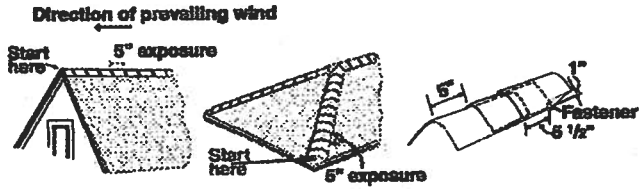
• HERITAGE® VINTAGE™ AR – Phillipsburg, KS
LAMINATED ASPHALT SHINGLES

8. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener on each side, 5-1/2 in. back from the exposed end and 1 in. up from the edge. TAMKO recommends the use of TAMKO Heritage Vintage Hip & Ridge shingle products.

Fasteners should be 1/4 in. longer than the ones used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLE IN COLD WEATHER.



THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

LINTEL
INFORMATION

Cement Precast Products, Inc.

INSTALLATION INSTRUCTIONS

Version 1.0

February 28, 2005

Cement Precast Products, Inc.

PRECAST LINTELS
(6", 8" AND 12" WIDE)

In order for proper installation of precast and prestressed lintels, DANSCO Engineering, LLC, has prepared this installation instructions to be used in conjunction with quality control methods of the contractor and good construction practices.

Preparation

1. All reinforcements shall be cleaned by removing mud, oil, or other materials that will adversely affect or reduce bond at the time mortar or grout is placed. Reinforcement with rust, mill scale, or a combination of both will be accepted as being satisfactory without cleaning or brushing provided the dimensions and weights, including heights of deformations, of a cleaned sample are not less than required by the ASTM specification covering this reinforcement in this Specification.

2. Prior to placing masonry, remove laitance, loose aggregate, and anything else that would prevent mortar from bonding to the lintel.

3. Debris – Construct grout spaces free of mortar dropping, debris, loose aggregates, and any material deleterious to masonry grout.

Lintel / masonry erection

1. Placing lintel – Length of bearing of lintels on their support shall be a minimum of 4 inches for filled lintels and 6 ½ inches for unfilled lintels in the direction of span. Provide a temporary support for lintels that are greater than 14' – 0"; the temporary support shall not be removed until 2 days after the grout placement.

2. Placing mortar and units (for composite lintels only)
   - Bed and head joints – Unless otherwise required, construct 3/8 inches thick bed and head joints. Construct joints that also conform to the following:
   a) Unless otherwise required, tool joint with a round jointer when the mortar is thumbprint hard.
   b) Remove masonry protrusions extending ½ inches or more into cells or cavities to be grouted.

   - Place hollow units so:
   a) Face shells of bed joints are fully mortared.
   b) Head joints are mortared, a minimum distance from each face equal to the face shell thickness of the unit.

c) Vertical cells to be grouted are aligned and unobstructed openings for grout are provided in accordance with the Project Drawings.

- Place clean units while the mortar is soft and plastic. Remove and relay in fresh mortar any unit disturbed to the extend that initial bond is broken after initial positioning.

Reinforcement installation

a) Support and fasten reinforcement together to prevent displacement beyond the tolerances allowed by construction loads or by placement of grout or mortar.
b) Completely embed reinforcing bars in grout in accordance with ACI530-02 Article 3.5.
c) Maintain clear distance between reinforcing bars and any face of masonry unit or formed surface, but not less than ¼ inches for fine grout or ½ inches for coarse grout.
d) Splice only where indicated on the Project Drawings, unless otherwise acceptable.
e) Unless accepted by the Architect/ Engineer, do not bend reinforcement after it is embeded in grout or mortar.
f) Place joint reinforcement so that longitudinal wires are embeded in mortar with a minimum cover of ½ inches when not exposed to weather or earth and 5/8 inches when exposed to weather or earth.

Grout placement

a) Placing time – Place grout within 1½ hours from introducing water in the mixture and prior to initial set.
b) Confinement – Confine grout to the areas indicated on the Project Drawings. Use material to confine grout that permits bond between masonry units and mortar.
c) Grout pour height – Do not exceed the maximum grout pour height given in the ACI530-02 Table 7.
d) Grout lift height – Place grout in lifts not exceeding 5 feet.
e) Consolidation – Consolidate grout at the time of placement.
   - Consolidate grout pours 12 inches or less in height by mechanical vibration or by puddling.
   - Consolidate pours exceeding 12 inches in height by mechanical vibration and reconsolidate by mechanical vibration after initial water loss and settlement has occurred.

Field quality control

a) Verify masonry unit strength, f'm in accordance with the ACI530-02 Article 1.6.
b) Sample and test grout as required by the ACI530-02 Articles 1.4B and 1.6.

DE
DANSCO ENGINEERING, LLC

P.O. Box 3400
Apollo Beach, FL 33572

Telephone (813) 645-0166
Facsimile (813) 645-9698

E-mail: dengine1@danscoengineering.com
CA25948

Cement Precast Products, Inc.

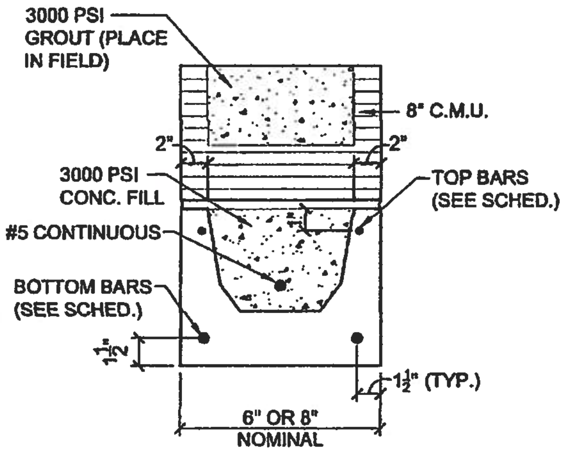
Precast concrete lintels 6", 8" and 12" wide have been reviewed by our office for compliance with the following codes:

- Florida Building Code 2004 Residential, sections R402.2 and R606.
- Florida Building Code 2004 Building, sections 1901.2 and 2107.

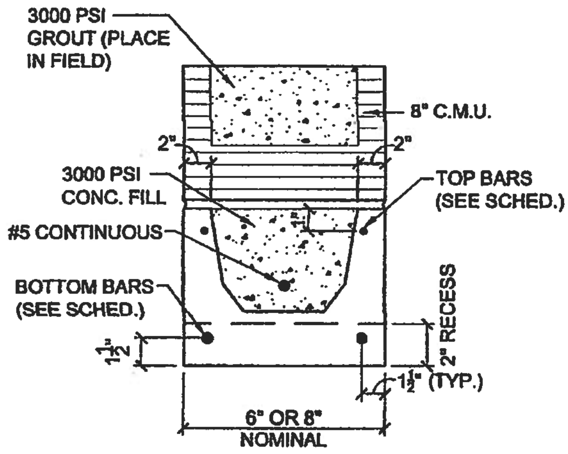
Our review is limited to the precast concrete lintels together with verification that they are accurate and appropriate for use according to the requirements of the above-referenced codes. Only Cement Precast Products, Inc. lintels may be used for the work depicted herein.

G:\Clerical Files\Documents\Forms\Cement Precast\Submittal\Proposal statement, C (2004 codes).doc

cp
Cement Precast Products, Inc.
2033 N.E. 27th Avenue Gainesville, Fl 32609 - (352) 372-0953 - Fax: (352) 378-4611
www.cementprecast.com



TYPICAL LINTEL SECTION
(6" OR 8"x16" LINTEL IS SHOWN)



TYPICAL RECESS LINTEL SECTION
(6" OR 8"x16" LINTEL IS SHOWN)

ENGINEERING SPECIFICATIONS:

1.) SAFE LOADS ARE TOTAL SUPERIMPOSED ALLOWABLE LOADS.
2.) DESIGNER MAY EVALUATE CONCENTRATED LOADS FROM THE SAFE LOAD TABLES BY CALCULATING MAX. RESISTING SHEAR AND MOMENT FOR THE LISTED LINTELS.
3.) SAFE LOADS LISTED ON ALL TABLES ARE IN UNITS OF POUND PER LINEAR FOOT.

GENERAL NOTES:

1.) CODES:
- 1.1 FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTIONS R402.2, & R606.
- 1.2 FLORIDA BUILDING CODE 2004 BUILDING, SECTIONS 1901.2 & 2107.
- 1.3 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-02).
- 1.4 AMERICAN SOCIETY OF CIVIL ENGINEERS MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES(ASCE 7-98).

2.) CONCRETE:
- 2.1 CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
  - 2.1.1 CONCRETE FILL (PLACE IN FIELD) - 3000 PSI
  - 2.1.2 PRECAST W/ STANDARD REINFORCEMENT - 5000 PSI
  - 2.1.3 GROUT PER ASTM C476 - 3000 PSI W/ MAX. 3/8" AGGERATE, 8" TO 11" SLUMP.
- 2.2 REINFORCING BARS:
  - 2.2.1 STEEL IN LINTEL AND KNOCKOUT BLOCK (PLACED IN FIELD) ASTM A615 (GRADE 40).
- 2.3 DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315.
- 2.4 CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.

3) MASONRY:
- 3.1 DESIGN AND CONSTRUCTION SHALL CONFORM TO THE SPECIFICATION OF THE NATIONAL CONCRETE MASONRY ASSOCIATION AND ACI 530-02.
- 3.2 MINIMUM MASONRY UNIT STRENGTH: F'M 1500 PSI.
- 3.3 MORTAR SHALL BE TYPE S.

4) STRUCTURAL:
- 4.1 SAFE LOAD VALUES ARE BASED ON LINTELS HAVING A BEARING OF 4".
- 4.2 FOR LINTELS THAT ARE GREATER THAN 14'-0" CLEAR SPAN THEY SHALL BE PROVIDED A TEMPORARY SUPPORT, AND THE TEMPORARY SUPPORT SHALL NOT BE REMOVED UNTIL 2 DAYS AFTER GROUT PLACEMENT.

THUY H. HUYNH
LICENSE
No. 60122
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
APR 0 6 2005

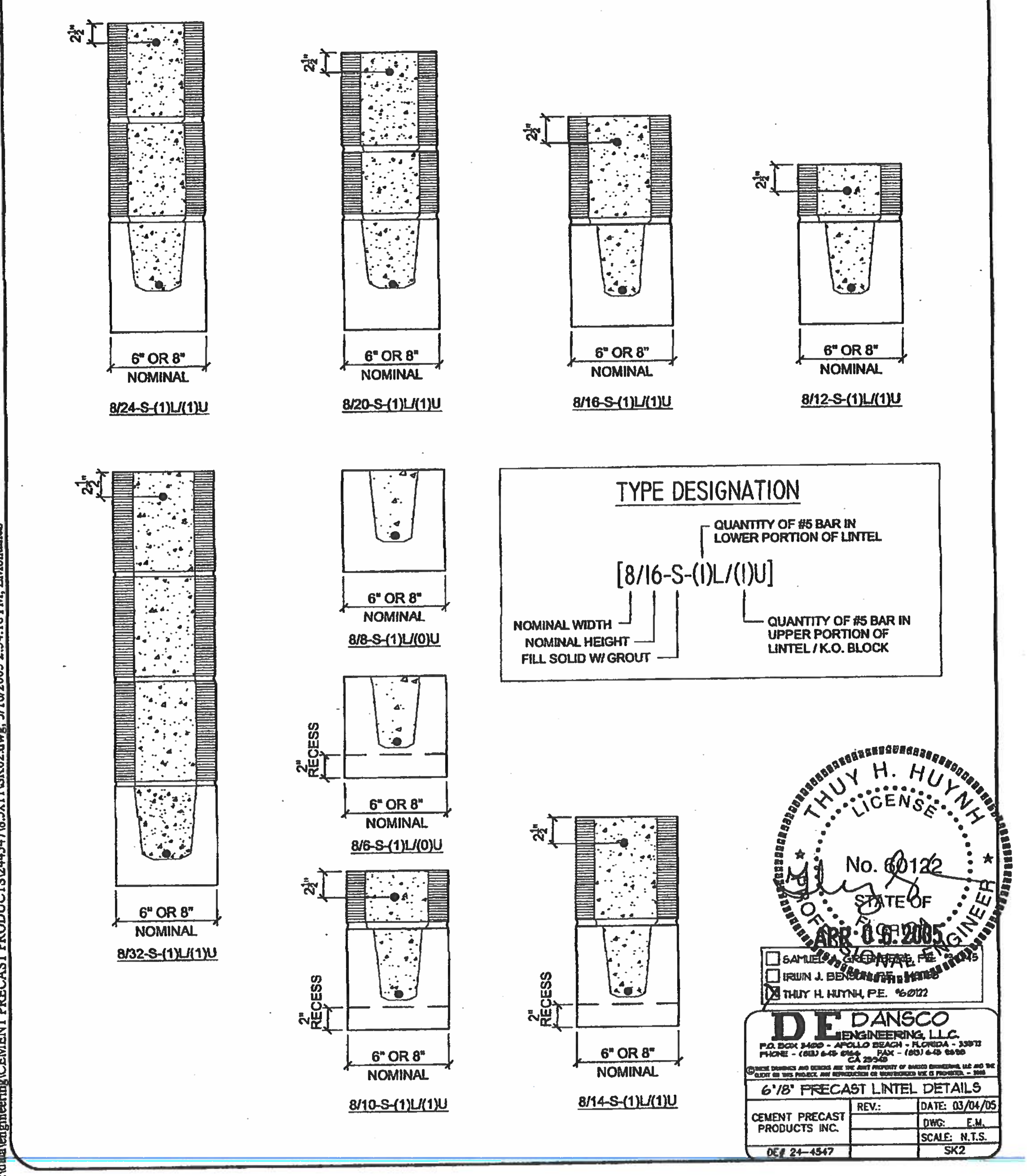
☐ SAMUEL A. GREENBERG, P.E. #34245
☐ IRWIN J. BENSON, P.E. #49758
☒ THUY H. HUYNH, P.E. #60122

DE DANSCO ENGINEERING, L.L.C.
P.O. BOX 3400 - APOLLO BEACH - FLORIDA - 33572
PHONE - (813) 645 6194 FAX - (813) 645 5659
CA 29949

GENERAL NOTES & DETAILS		
CEMENT PRECAST PRODUCTS INC.	REV.:	DATE: 03/04/05
		DWG: E.M.
		SCALE: N.T.S.
DE# 24-4547		SK1

G:\data\engineering\CEMENT PRECAST PRODUCTS\244547\8.5x11\SK01.dwg, 5/31/2005 9:09:12 AM, KJoyner

cp
Cement Precast Products, Inc.
2033 N.E. 27th Avenue Gainesville, Fl 32609 - (352) 372-0953 - Fax: (352) 378-4611
www. cementprecast.com



G:\data\engineering\CEMENT PRECAST PRODUCTS\244547\8.5x11\SK02.dwg, 3/16/2005 2:54:18 PM, EMontanez

cp

Cement Precast Products, Inc.

2033 N.E. 27th Avenue Gainesville, Fl 32609 • (352) 372-0953 • Fax: (352) 378-4611
www. cementprecast.com

LINTEL		8" LINTEL SAFE GRAVITY LOADS (PLF)							
TOTAL LENGTH	CLEAR SPAN	TOP REINF.	BOTTOM REINF.	8/8-S-(0)L 8/8-S-(1)L	8/12-S-(0)L 8/12-S-(1)L	8/16-S-(0)L 8/16-S-(1)L	8/20-S-(0)L 8/20-S-(1)L	8/24-S-(0)L 8/24-S-(1)L	8/32-S-(0)L 8/32-S-(1)L
3'-0"	1'-8"	None	(2) #3	3859	8159	10000	10000	10000	10000
				3859	10000	10000	10000	10000	10000
3'-6"	2'-2"	None	(2) # 3	2861	5757	8114	10000	10000	10000
				2861	7219	10000	10000	10000	10000
4'-0"	2'-8"	None	(2) # 3	2270	4279	6034	7791	9550	10000
				2270	5183	10000	10000	10000	10000
4'-6"	3'-2"	None	(2) # 3	1879	3301	4658	6015	7375	10000
				1879	4037	7450	10000	10000	10000
4'-8"	3'-4"	None	(2) # 3	1776	3048	4301	5556	6811	9324
				1776	3739	6748	10000	10000	10000
5'-4"	4'-0"	None	(2) # 3	1340	2275	3213	4153	5093	5744
				1457	2948	4895	8224	10000	10000
5'-10"	4'-6"	None	(2) # 3	1101	1875	2644	3418	4193	6975
				1282	2529	4055	5408	9705	10000
6'-4"	5'-0"	(2) # 2	(2) # 4	1178	2258	3513	5083	6245	8570
				1178	2258	3513	5323	8288	10000
6'-6"	5'-2"	(2) # 2	(2) # 4	1166	2167	3347	4808	5907	8107
				1166	2167	3347	5015	7658	10000
6'-8"	5'-4"	(2) # 2	(2) # 4	1098	2084	3196	4554	5595	7680
				1098	2084	3196	4741	7121	10000
7'-6"	6'-2"	(2) # 2	(2) # 4	938	1745	2604	3539	4349	5972
				938	1745	2604	3719	5281	10000
7'-8"	6'-4"	(2) # 3	(2) # 4	912	1690	2511	3376	4150	5699
				912	1690	2511	3585	5020	9974
8'-0"	6'-8"	(2) # 3	(2) # 4	862	1589	2342	3082	3789	5204
				862	1589	2342	3291	4567	9115
8'-8"	7'-4"	(2) # 3	(2) # 4	778	1407	2001	2598	3193	4387
				778	1418	2069	2858	3889	7207
9'-4"	8'-0"	(2) # 3	(2) # 5	718	1292	1856	2527	3461	5859
				718	1292	1856	2527	3368	6944
10'-4"	9'-0"	(2) # 3	(2) # 5	632	1125	1597	2141	2800	4669
				632	1125	1597	2141	2898	4669
11'-4"	10'-0"	(2) # 3	(2) # 5	563	995	1399	1854	2391	3837
				563	995	1399	1854	2391	3837
12'-0"	10'-8"	(2) # 3	(2) # 5	524	923	1291	1701	2178	3427
				524	923	1291	1701	2178	3427
12'-8"	11'-4"	(2) # 3	(2) # 5	490	861	1198	1570	1998	3066
				490	861	1198	1570	1998	3098
13'-4"	12'-0"	(2) # 3	(2) # 5	460	806	1117	1457	1932	2748
				460	806	1117	1457	1932	2817
14'-0"	12'-8"	(2) # 3	(2) # 5	434	757	1046	1359	1712	2475
				434	757	1046	1359	1712	2686
14'-8"	13'-4"	(2) # 4	(2) # 6	412	716	985	1275	1600	2391
				412	716	985	1275	1600	2391
15'-8"	14'-4"	(2) # 4	(2) # 6	377	658	903	1163	1452	2142
				380	658	903	1169	1452	2142
17'-4"	16'-0"	(2) # 4	(2) # 6	263	579	790	1012	1255	1822
				290	579	790	1012	1255	1822
19'-4"	18'-0"	(2) # 4	(2) # 6	157	505	686	874	1076	1540
				173	505	686	874	1076	1540
20'-0"	18'-8"	(2) # 5	(2) # 6	132	484	656	835	1027	1463
				140	484	656	835	1027	1463
21'-4"	20'-0"	(2) # 5	(2) # 6	95	372	604	766	939	1329
				105	440	604	766	939	1329
22'-0"	20'-8"	(2) # 5	(2) # 6	80	323	580	735	901	1271
				89	382	580	735	901	1271
24'-0"	22'-8"	(2) # 5	(2) # 6	N.R.	214	493	656	800	1120
				N.R.	268	519	656	800	1120

N.R. = NOT RECOMMENDED

THUY H. HUYNH
LICENSE
No. 60122
STATE OF FLORIDA
PROFESSIONAL ENGINEER
APR 0 6 2005

☐ SAMUEL A. GREENBERG, P.E. #34245
☐ IRWIN J. BENSON, P.E. #49158
☒ THUY H. HUYNH, P.E. #60122

DE DANSCO ENGINEERING, L.L.C.
P.O. BOX 3400 • APOLLO BEACH • FLORIDA • 33572

8" LINTEL SAFE GRAVITY LOADS

CEMENT PRECAST PRODUCTS INC.	REV.:	DATE: 03/04/05
		DWG: E.M.
		SCALE: N.T.S.
DE# 24-4547		SK7

cp Cement Precast Products, Inc.

2033 N.E. 27th Avenue Gainesville, Fl 32609 • (352) 372-0953 • Fax: (352) 378-4611
www.cementprecast.com

LINTEL				8" LINTEL SAFE UPLIFT LOADS (PLF)						LATERAL LOADS (PLF)	
TOTAL LENGTH	CLEAR SPAN	TOP REINF.	BOTTOM REINF.	8/8-S-(1)U 8/8-S-(2)U	8/12-S-(1)U 8/12-S-(2)U	8/16-S-(1)U 8/16-S-(2)U	8/20-S-(1)U 8/20-S-(2)U	8/24-S-(1)U 8/24-S-(2)U	8/32-S-(1)U 8/32-S-(2)U	8/8-P	8/8-S
3'-0"	1'-8"	None	(2) #3	3369 3369	9999 10000	10000 10000	10000 10000	10000 10000	10000 10000	811	2089
3'-6"	2'-2"	None	(2) # 3	2539 2539	6508 10000	10000 10000	10000 10000	10000 10000	10000 10000	575	1481
4'-0"	2'-8"	None	(2) # 3	2038 2038	4763 4763	7739 9605	10000 10000	10000 10000	10000 10000	429	1105
4'-6"	3'-2"	None	(2) # 3	1701 1701	3756 3756	5993 6316	7894 10000	9796 10000	10000 10000	332	855
4'-8"	3'-4"	None	(2) # 3	1613 1613	3508 3508	5541 6288	7298 10000	9057 10000	10000 10000	307	791
5'-4"	4'-0"	None	(2) # 3	1334 1334	2401 2401	4162 4636	5481 7289	6803 10000	9448 10000	230	594
5'-10"	4'-6"	None	(2) # 3	1182 1182	2350 2777	3439 3874	4530 6101	5622 10000	7808 10000	190	491
6'-4"	5'-0"	(2) # 2	(2) # 4	1110 1110	1975 2175	2890 3325	3906 5038	4724 7760	6561 10000	333	894
6'-6"	5'-2"	(2) # 2	(2) # 4	1083 1083	1869 2035	2736 3175	3603 4759	4472 7213	6211 10000	316	847
6'-8"	5'-4"	(2) # 2	(2) # 4	1048 1048	1772 1950	2594 3036	3416 4511	4240 6739	5888 10000	299	803
7'-6"	6'-2"	(2) # 2	(2) # 4	802 802	1384 1665	2025 2489	2663 3579	3311 5069	4598 9101	233	627
7'-8"	6'-4"	(2) # 3	(2) # 4	916 958	1322 1695	1934 2414	2548 3496	3162 4890	4392 8692	273	742
8'-0"	6'-8"	(2) # 3	(2) # 4	823 823	1209 1514	1770 2259	2331 3164	2893 4613	4018 7958	249	679
8'-8"	7'-4"	(2) # 3	(2) # 4	746 746	1023 1359	1498 2002	1973 2775	2449 3764	3410 6731	205	575
9'-4"	8'-0"	(2) # 3	(2) # 5	721 721	877 1235	1294 1798	1694 2460	2099 3281	2916 5752	175	653
10'-4"	9'-0"	(2) # 3	(2) # 5	639 689	711 1083	1040 1559	1370 2101	1700 2751	2362 4670	125	529
11'-4"	10'-0"	(2) # 3	(2) # 5	573 573	587 985	859 1376	1132 1834	1405 2369	1952 3790	92	437
12'-0"	10'-8"	(2) # 3	(2) # 5	538 537	522 900	764 1276	1006 1891	1249 2168	1735 3404	N.R	389
12'-8"	11'-4"	(2) # 3	(2) # 5	479 505	467 843	684 1190	900 1569	1118 1999	1552 3015	N.R	348
13'-4"	12'-0"	(2) # 3	(2) # 5	431 476	420 798	615 1115	810 1462	1006 1864	1397 2768	N.R	313
14'-0"	12'-8"	(2) # 3	(2) # 5	388 451	380 741	557 1048	733 1378	910 1724	1264 2602	N.R	283
14'-8"	13'-4"	(2) # 4	(2) # 6	432 443	346 673	506 989	667 1288	827 1619	1149 2275	N.R	447
15'-8"	14'-4"	(2) # 4	(2) # 6	353 400	302 588	442 867	582 1146	723 1427	1004 1988	N.R	357
17'-4"	16'-0"	(2) # 4	(2) # 6	258 291	246 479	360 705	474 939	588 1160	817 1617	N.R	251
19'-4"	18'-0"	(2) # 4	(2) # 6	169 189	196 383	288 564	379 746	471 929	654 1294	N.R	169
20'-0"	18'-8"	(2) # 5	(2) # 6	181 195	183 357	269 597	354 597	439 867	610 1208	N.R	168
21'-4"	20'-0"	(2) # 5	(2) # 6	141 152	161 290	235 462	310 611	385 760	535 1060	N.R	131
22'-0"	20'-8"	(2) # 5	(2) # 6	126 185	151 260	221 434	291 574	362 714	503 995	N.R	116
24'-0"	22'-8"	(2) # 5	(2) # 6	101 108	134 212	196 385	259 509	321 634	446 883	N.R	80

N.R. = NOT RECOMMENDED

THUY H. HUYNH
LICENSE
No. 60122
STATE OF FLORIDA
PROFESSIONAL ENGINEER
APR 0 6 2005

☐ SAMUEL A. GREENBERG, P.E. #34245
☐ IRWIN J. BENSON, P.E. #49758
☒ THUY H. HUYNH, P.E. #60122

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PHONE - (813) 645 8166 FAX - (813) 645 5659
CA 25349

8" LINTEL SAFE UPLIFT LOADS		
CEMENT PRECAST PRODUCTS INC.	REV.:	DATE: 03/04/05
		DWG: E.M.
		SCALE: N.T.S.
DE# 24-4547		SK8

cp
Cement Precast Products, Inc.
2033 N.E. 27th Avenue Gainesville, Fl 32609 • (352) 372-0953 • Fax: (352) 378-4611
www.cementprecast.com

LINTEL				8" RECESS LINTEL SAFE GRAVITY LOADS (PLF)		
TOTAL LENGTH	CLEAR SPAN	TOP REINF.	BOTTOM REINF.	8/6-S-(0)L 8/6-S-(1)L	8/10-S-(0)L 8/10-S-(1)L	8/14-S-(0)L 8/14-S-(1)L
3'-6"	2'-2"	None	(2) #3	1530 1530	4400 4400	6931 10000
4'-0"	2'-8"	None	(2) # 3	1250 1250	3351 3351	5155 6754
4'-4"	3'-0"	None	(2) # 3	1113 1113	2851 2890	4322 5483
4'-6"	3'-2"	None	(2) # 3	1005 1005	2624 2703	3979 5010
4'-8"	3'-4"	None	(2) # 3	1003 1003	2422 2538	3674 4612
5'-8"	4'-4"	None	(2) # 3	764 771	1584 1856	2406 3115
6'-8"	5'-4"	(2) # 2	(2) # 4	652 652	1491 1491	2401 2401
7'-8"	6'-4"	(2) # 3	(2) # 4	546 546	1225 1225	1919 1919

THUY H. HUYNH
LICENSE
No. 60122
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

APR 0 6 2005

☐ SAMUEL A. GREENBERG, P.E. #245
☐ IRWIN J. BENSON, P.E. #158
☒ THUY H. HUYNH, P.E. #60122

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CA 25346

8" RECESS LINTEL SAFE GRAVITY LOADS		
CEMENT PRECAST PRODUCTS INC.	REV.:	DATE: 03/04/05
		DWG: E.M.
		SCALE: N.T.S.
DE# 24-4547		SK9

cp
Cement Precast Products, Inc.
2033 N.E. 27th Avenue Gainesville, Fl 32609 • (352) 372-0953 • Fax (352) 378-4611
www. cementprecast.com

LINTEL				8" RECESS LINTEL SAFE UPLIFT LOADS (PLF)		
TOTAL LENGTH	CLEAR SPAN	TOP REINF.	BOTTOM REINF.	8/6-S-(1)U 8/6-S-(2)U	8/10-S-(1)U 8/10-S-(2)U	8/14-S-(1)U 8/14-S-(2)U
3'-6"	2'-2"	None	(2) #3	2571 2571	3966 3966	8716 9261
4'-0"	2'-8"	None	(2) # 3	2063 2063	3063 3063	6237 6237
4'-4"	3'-0"	None	(2) # 3	1823 1823	2660 2660	5122 5122
4'-6"	3'-2"	None	(2) # 3	1723 1723	2496 2496	4701 4701
4'-8"	3'-4"	None	(2) # 3	1633 1633	2350 2350	4345 4345
5'-8"	4'-4"	None	(2) # 3	1244 1224	1742 1742	2986 2986
6'-8"	5'-4"	(2) # 2	(2) # 4	1005 1085	1358 1348	2179 2275
7'-8"	6'-4"	(2) # 3	(2) # 4	708 842	1013 1148	1625 1887

THUY H. HUYNH
LICENSE
No. 60122
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
APR 0 8 2005

☐ SAMUEL A. GREENBERG, P.E. 34245
☐ IRWIN SIMPSON, P.E. 34868
☑ THUY H. HUYNH, P.E. 60122

DE DANSCO ENGINEERING, L.L.C.
P.O. BOX 3400 - APOLLO BEACH - FLORIDA - 33572
PHONE - (813) 645 6966 FAX - (813) 645 5658
CA 25949

8" RECESS LINTEL SAFE UPLIFT LOADS

CEMENT PRECAST PRODUCTS INC.	REV.:	DATE: 03/04/05
		DWG: E.M.
		SCALE: N.T.S.
DE# 24-4547		SK10

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UNIVERSAL
ENGINEERING SCIENCES
Consultants In: Geotechnical Engineering • Environmental Sciences • Construction Materials Testing

4475 S.W. 35th Terrace • Gainesville, Florida 32608 • (352) 372-3392

REPORT ON IN-PLACE DENSITY TESTS

25610

CLIENT: Homes by House Craft

PROJECT: Swayze Residence
152 S.W Atlas Dr. Fort White

AREA TESTED: Fill V prop

COURSE: F/G

DEPTH OF TEST: 0-1'

TYPE OF TEST: ASTM D-2922

DATE TESTED: 3-13-07

NOTE: The below tests ~~DO~~ /~~DO NOT~~ meet the minimum 95 % compaction requirements of maximum density.

REMARKS:

LOCATION OF TESTS	DRY DEN.	MAX. DEN.	% MAX. DEN.	% MOIST.	OPT. MOIST.
		107.1			10.8
Approx cntr & S. prop ext					
foundation	102.0		95.2	6.8	
Approx 25' N from the S.W corner					
of found.	101.8		95.0	6.6	
Approx 20' N from the S.E corner					
of found.	101.9		95.1	6.4	

TECH. J.I

4404

TURNER PEST CONTROL, LLC
480 EDGEWOOD AVE S
JACKSONVILLE, FL 32205
PHONE (904) 355-5300 FAX (904) 353-1488

25610

CERTIFICATE OF COMPLIANCE FOR TERMITE PROTECTION (AS REQUIRED BY FLORIDA BUILDING CODE SECTION 1816.7)

PROPERTY ADDRESS:	152 SW ATLAS DRIVE
PROPERTY NAME:	MICHAEL SWAYZE
CITY, STATE:	FORT WHITE, FLORIDA
PERMIT:	000025610
BUILDER:	HOMES BY HOUSECRAFT
DATE OF APPLICATION:	JUNE 21, 2007

METHOD OF TERMITE PREVENTION TREATMENT: FINAL
(SOIL BARRIER, WOOD TREATMENT, BAIT SYSTEM, OTHER)

THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES BY TURNER PEST CONTROL LLC.

TREATMENT PERFORMED BY TURNER PEST CONTROL LLC IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES.

AUTHORIZED SIGNATURE
CERTIFIED OPERATOR

JUNE 21, 2007
DATE

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 19-7S-17-10024-085 **Building permit No.** 000025610

Use Classification SFD, UTILITY **Fire:** 0.00

Permit Holder JOHN D. HARRINGTON **Waste:**

Owner of Building MICHAEL SWAYZE **Total:** 0.00

Location: 152 SW ATLAS DRIVE, FT. WHITE, FL

Date: 06/19/2007

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Turner Pest Control
What's Bugging You?

Treasure/Space Coast, FL (772) 621 7905
Tampa, FL (813) 681 6381
St.Marys, GA (912) 576 1300

Main Office
480 S. Edgewood Ave.
Jacksonville, FL 32205-3775
Phone: (904) 355.5300
Fax: (904) 353.1488
Toll Free: (800) 225.5305
www.turnerpest.com

PRE-TREAT LABEL

BUILDER Time By HandCraft PERMIT NUMBER 800025

LOT NO _____ BLOCK _____ SECTION _____ SUBDIVISION Fort White, FL

ADDRESS 152 SW Atlas Dr

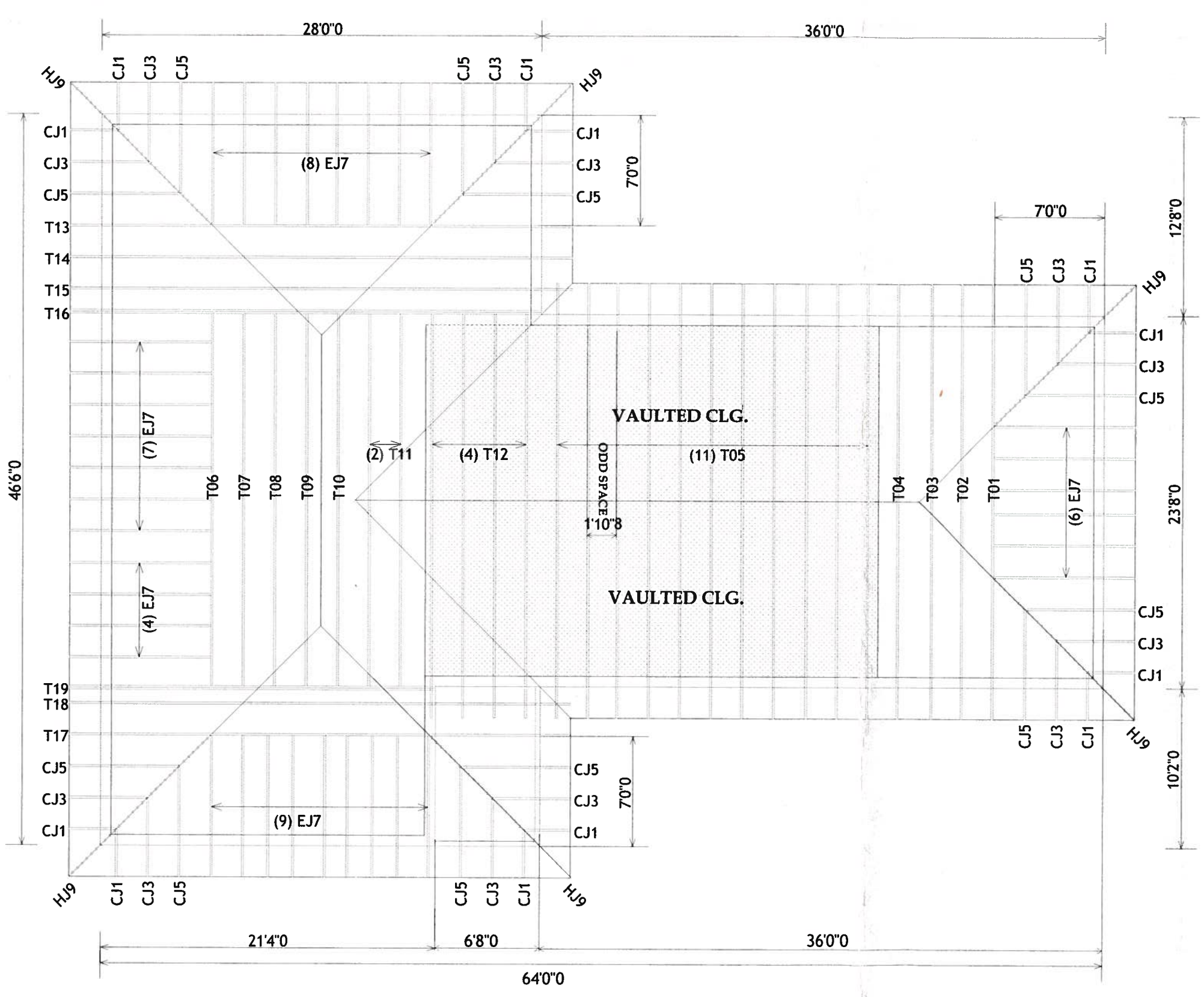
TREATMENT AREA	DATE / TIME TREATED	CHEMICAL USED	%	GALLONS USED	EMPLOYEE #
PRE-TREAT SLAB / PORCH / ENTRY	3/21/22	Prowse Pro	.06	180	26
WOOD TREATMENT					
INT STRUCT / EXT BAND					
WASTE ARMS / DRIVE / WALKS					
FINAL PERIMETER GRADE					

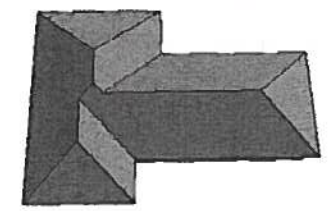
CIRCLE ONE: COMMERCIAL (RESIDENTIAL) TYPE OF SLAB: (DIRT FILL) MONOLITHIC

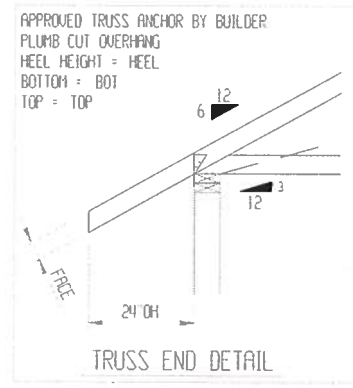
	SQUARE FOOTAGE	LINEAR FOOTAGE	%	GALLONS USED
FIRST FLOOR / LIVING AREA	2170	182	.06	180
GARAGE				
PATIO / PORCH / REAR				
FRONT ENTRY				
TOTAL TREATED	2170	182	.06	180

TECH PK TS

Annual renewal due one (1) year from treatment date



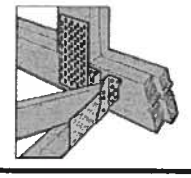




TRUSS END DETAIL

HANGER SCHEDULE

TRUSS HANGER INFORMATION Check TRUSS ENGINEERING For gravity and uplift values if the value exceeds the capacity of a hanger.
(18)HTU26



BEARING HEIGHT SCHEDULE

8'-0"

OVERHANG
2'-0"

ROOF PITCH(S)
6/12

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL VIDS FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER:
HOMES BY HOUSE CRAFT

LEGAL ADDRESS:
ALACHUA, FL

MODEL:	REVISION:
GULF	SCALE: NTS
DATE: 02/06/07	DRAWN BY: A MONDRAGON
	JOB #: L225956