

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2033215 - HARRISON RES.

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: John Norris Const. Project Name: Harrison Res. Model: Custom

Subdivision: N/A

Lot/Block: N/A Address: 809 NW Milo Terrace, N/A

City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special

Loading Conditions):

Design Code: FBC2017/TPI2014

Wind Code: ASCE 7-10

Design Program: MiTek 20/20 8.2

Wind Speed: 130 mph Floor Load: N/A psf

Roof Load: 37.0 psf

This package includes 18 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet

conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

| No. | Seal# | Truss Name | Date |
|----------------------------|------------------------|--------------|--------------------|
| 1 | T17713745 | CJ01 | 7/29/19 |
| 2 3 4 5 6 7 | T17713746 T17713747 | CJ02 CJ03 | 7/29/19 7/29/19 |
| 4 | T17713748 | EJ01 | 7/29/19 |
| 5 | T17713749 T17713750 | HJ01 T01 | 7/29/19 7/29/19 |
| 7 | T17713751 | T02 | 7/29/19 |
| 8 | T17713752 T17713753 | T03 T04 | 7/29/19 7/29/19 |
| 10 | T17713754 | T05 T05G | 7/29/19 7/29/19 |
| 11 12 | T17713755 T17713756 | T06 | 7/29/19 |
| 13 | T17713757 T17713758 | T08 T08G | 7/29/19 7/29/19 |
| 14 15 | T17713759 | T09 | 7/29/19 |
| 16 | T17713760 T17713761 | T10 T12 | 7/29/19 7/29/19 |
| 17 18 | T17713762 | †12G | 7/29/19 |



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 29,2019

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Job Truss Truss Type Qty HARRISON RES Ply T17713745 2033215 CJ01 JACK-OPEN Job Reference (optional) 8.240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 08 2019 Page 1 Builders FirstSource. Jacksonville, FL - 32244 ID Na4?LzVoi51_vU6na?cx?BzOClt-i1pyO7dp8n53az_B2ejIXLkYCuAld?Bzn4Q3Y6ytHTP -1-6-0 3x4 / Scale ≈ 1.9.0 6.00 12 0-10-0 0-8-11 3x6 | 1-0-0 [2:0-1-12,0-0-6], [3:0-1-4,0-1-8] Plate Offsets (X,Y)--LOADING (psf) SPACING-2-0-0 CSL DEFI in (loc) I/defl I /d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.17 Vert(LL) 0.00 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.02 Vert(CT) 0.00 >999 180 **BCLL** 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) -0.003 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MP Weight 8 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins. 2x4 SP No.2 BOT CHORD **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. Left 2x4 SP No.3 1-2-0 SLIDER

REACTIONS. (lb/size) 3=-8/Mechanical, 2=179/0-3-8, 4=-16/Mechanical

Max Horz 2=48(LC 12) Max Uplift 3=-8(LC 1), 2=-94(LC 12), 4=-16(LC 1)

Max Grav 3=6(LC 16), 2=179(LC 1), 4=16(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp.C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Refer to girder(s) for truss to truss connections
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 3, 94 lb uplift at joint 2 and 16 lb uplift at joint 4.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

July 29,2019

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI⊦7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property manage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type HARRISON RES Qty Ply T17713746 2033215 **CJ02** JACK-OPEN Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8 240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06:53 09 2019 Page 1 ID Na4?LzVoi51_vU6na?cx?BzOClt-ADNKbTeRv5DwC6ZNcLEX4YGjyIVeMSR6?k9c5ZytHTO 3-0-0 1-6-0 3-0-0 Scale = 1 14 0 6.00 12 3x4 = 2-2-1-10-0 0-8-11 3x6 3-0-0 Plate Offsets (X,Y)-[2:0-1-12,0-0-6] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) I/defi L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.17 Vert(LL) 0.01 5-8 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.11 Vert(CT) -0.015-8 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) -0.002 n/a n/a BCDL 10:0 Code FBC2017/TPI2014 Matrix-MP Weight 15 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.2

SLIDER Left 2x4 SP No.3 1-11-8

(lb/size) 4=60/Mechanical, 2=210/0-3-8, 5=28/Mechanical REACTIONS.

Max Horz 2=103(LC 12)

Max Uplift 4=-62(LC 12), 2=-85(LC 12), 5=-27(LC 9) Max Grav 4=60(LC 1), 2=210(LC 1), 5=48(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 4, 85 lb uplift at joint 2 and 27 lb uplift at joint 5.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 29,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design in the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the ablocation, storage, delivery, erection and bracing of trusses and truss systems, see
ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandra, VA 22314



Job Truss Truss Type Qty HARRISON RES T17713747 2033215 CJ03 JACK-OPEN 1 Job Reference (optional) Builders FirstSource Jacksonville FL - 32244 8,240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 09 2019 Page 1 ID_Na4?LzVoi51_vU6na?cx?BzOClt-ADNKbTeRv5DwC6ZNcLEX4YGfyIRXMSR6?k9c5ZytHTO 5-0-0 1-6-0 Scale = 1 18 9 6.00 12 3x4 / 3 2-10-0 3-2-D-B-11 5-0-0 [2:0-1-12.0-0-6] Plate Offsets (X,Y)-LOADING (psf) SPACING-2-0-0 CSL DEFI l/defl **PLATES** GRIP (loc) L/d TCLL 20.0 Plate Grip DOL 1.25 TC 0.43 Vert(LL) 0.09 5-8 >677 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.37 Vert(CT) 0.08 5-8 >765 180 BCLL 0.0 YES Rep Stress Incr WB 0.00 Horz(CT) -0.022 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-AS Weight 21 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied BOT CHORD Rigid ceiling directly applied.

BOT CHORD 2x4 SP No.2

Left 2x4 SP No.3 1-11-8 SLIDER

REACTIONS. (lb/size) 4=115/Mechanical, 2=276/0-3-8, 5=55/Mechanical

Max Horz 2=151(LC 12)

Max Uplift 4=-110(LC 12), 2=-102(LC 12), 5=-46(LC 9) Max Grav 4=115(LC 1), 2=276(LC 1), 5=85(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp.C; Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 4, 102 lb uplift at joint 2 and 46 lb uplift at joint 5.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Job Truss Truss Type Qtv HARRISON RES Ply T17713748 2033215 EJ01 JACK-PARTIAL Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8 240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 10 2019 Page 1 ID Na4?LzVoi51_vU6na?cx?BzOCIt-eQxiopf3gOLnqG8aA3Imdmpjlihs5vhGEOvAd?ytHTN Scale = 1.24.3 6.00 12 3-10-0 3x4 / D-8-11 5 3x6 7-0-0 Plate Offsets (X,Y)-[2:0-1-12,0-0-6] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) I/defl I /d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.87 Vert(LL) 0.33 5-8 >254 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 вс 0.75 0.29 Vert(CT) 5-8 >290 180 **BCLL** 0.0 Rep Stress Incr YE\$ **WB** 0.00 Horz(CT) -0.064 n/a n/a Code FBC2017/TPI2014 BCDL 10.0 Matrix-AS Weight: 28 lb FT = 20%LUMBER-BRACING-TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied TOP CHORD BOT CHORD 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied.

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS.

(lb/size) 4=169/Mechanical, 2=346/0-3-8, 5=79/Mechanical Max Horz 2=138(LC 12)

Max Uplift 4=-101(LC 12), 2=-104(LC 9), 5=-61(LC 9)

Max Grav 4=169(LC 1), 2=346(LC 1), 5=121(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-371/412

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 4, 104 lb uplift at joint 2 and 61 lb uplift at joint 5.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 29,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters and in individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly increporate this design into the overall building designer must verify the applicability of design parameters and properly increporate this design into the overall building designer must verify the applicability of design parameters and properly microporate this design into the overall building designer must verify the applicability of design parameters and properly microporate this design into the overall building designer must verify for a design parameter and director of the overall building of middle truss we had director of the overall building designer must verify a trusse of the overall building designer must be overall building designer. Building designer must be overall building designer must be overall building designer must be overall building designer. Building designer must be overall building designer must be overall building designer. Building designer must be overall building designer must be overall building designer. Building designer must be overall building designer must building designer must be overall building designer. Building designer must building designer must building designer must building designer. Building designer must building designer. Building designer must build



Job Qty HARRISON RES Truss Truss Type Ply T17713749 2033215 HJ01 DIAGONAL HIP GIRDER Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244 8,240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 11 2019 Page 1 ID.Na4?LzVoi51_vU6na?cx?BzOClt-6cV509fiRiTeRQjmjmH?9zM_y55cqlIPT2ej9RytHTM 9-10-13 5-1-14 4.24 12 3x4 = - 25 -0-8-6 15 16 17 8 2x4 || 3x8 3x4 5-1-14 4-8-15 Plate Offsets (X,Y)-[2:0-3-8,Edge] LOADING (psf) SPACING-**PLATES** GRIP 2-0-0 CSI. DEFL L/d in (loc) I/defl TCLL 20.0 Plate Grip DOL 1.25 TC 0.46 244/190 Vert(LL) 0.08 7-8 >999 240 MT20 TCDL 7.0 Lumber DOL вс 0.46 -0.08 1.25 Vert(CT) 7-8 >999 180 **BCLL** 0.0 WB 0.30 Horz(CT) Rep Stress Incr NO -0.01 5 n/a n/a

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 5=135/Mechanical, 2=498/0-4-15, 6=309/Mechanical

Max Horz 2=218(LC 4)

Max Uplift 5=-123(LC 4), 2=-466(LC 4), 6=-304(LC 4)

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

TOP CHORD 2-4=-590/478

BOT CHORD 2-8=-545/530, 7-8=-545/530 WEBS 4-8=-145/258, 4-7=-592/609

NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

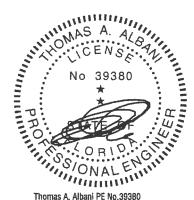
Matrix-MS

- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 123 lb uplift at joint 5, 466 lb uplift at joint 2 and 304 lb uplift at joint 6.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 71 lb down and 84 lb up at 1-5-12, 71 lb down and 84 lb up at 1-5-12, 71 lb down and 84 lb up at 1-5-12, 30 lb down and 53 lb up at 4-3-11, 30 lb down and 53 lb up at 4-3-11, and 54 lb down and 109 lb up at 7-1-10, and 54 lb down and 109 lb up at 7-1-10 on top chord, and 48 lb down and 45 lb up at 1-5-12, 48 lb down and 45 lb up at 1-5-12, 19 lb down and 35 lb up at 4-3-11, and 40 lb down and 61 lb up at 7-1-10, and 40 lb down and 61 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

 Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 6-9=-20



Weight 47 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 7-7-3 oc bracing.

FT = 20%

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 29,2019

Continued on page 2



| ŀ | dop | Truss | Truss Type | Qty | Ply | HARRISON RES |
|---|---------|-------|---------------------|-----|-----|--------------------------|
| | 2033215 | HJ01 | DIAGONAL HIP GIRDER | 2 | , | T17713749 |
| | | | - ON TEXT ON TEXT | | | Job Reference (optional) |

Builders FirstSource,

Jacksonville, FL - 32244

8 240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 12 2019 Page 2 ID:Na4?LzVoi51_vU6na?cx?BzOCII-ao3TDVgKB0bV3alyHToEiBu9hVRrZIYZhiOGiuyHTL

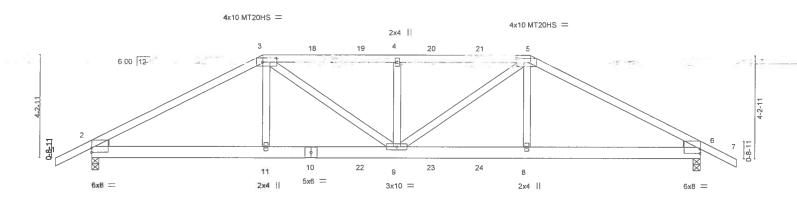
LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 3=40(F=20, B=20) 13=-0(F=-0, B=-0) 14=-77(F=-39, B=-39) 16=-7(F=-4, B=-4) 17=-54(F=-27, B=-27)



| Job | Truss | Truss Type | | Qty | Ply | HARRISON RES | |
|-----------------------|---------------------------|------------|------|----------|-------------|---|---------------------|
| 2033215 | T01 | HIP GIRDER | | 1 | 1 | | T17713750 |
| | | | | | | Job Reference (optional) | |
| Builders FirstSource, | Jacksonville, FL - 32244, | | | Į. | 3.240 s Jui | n 8 2019 MiTek Industries, Inc. Mon Jul 29 06 | 5 53 13 2019 Page 1 |
| | | | ID N | a4?LzVoi | 51_vU6na | ?cx?BzOClt-2?drRqhyyJjMhkt9rBJTEORK6vr | BI8niwM7qEKytHTK |
| -1-6-0 | 7-0-0 | 12-6-0 | | 18-0- | 0 | 25-0-0 | 26-6-0 |
| 1-6-0 | 7-0-0 | 5-6-0 | , | 5-6-0 |) | 7-0-0 | 1-6-0 |

Scale = 1 45 6



| | - | 7-0-0 7-0-0 | | 12-6 5-6 | | | 18-0-0 5-6-0 | | | | 25-0-0 7-0-0 | —— |
|-----------|------------|-----------------------------|------------------|-------------|------|----------|-----------------|-------|--------|-----|-----------------|----------|
| Plate Off | sets (X,Y) | [2:0-0-0,0-2-2], [3:0-7-0,0 | -2-0], [5:0-7-0, | | | | 3-0-0 | | | | 7-0-0 | |
| LOADING | G (psf) | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plate Grip DOL | 1.25 | тс | 0.48 | Vert(LL) | 0.22 | 8-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.45 | Vert(CT) | -0.24 | 8-9 | >999 | 180 | MT20HS | 187/143 |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.55 | Horz(CT) | 0.06 | 6 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/T | PI2014 | Matrix- | -MS | | | | | | Weight: 139 lb | FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP M 31

BOT CHORD 2x6 SP M 26 WEBS 2x4 SP No.3

WEBS WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=1954/0-3-8, 6=1954/0-3-8

Max Horz 2=-58(LC 6)

Max Uplift 2=-1270(LC 5), 6=-1270(LC 4)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3353/2325, 3-4=-3741/2634, 4-5=-3741/2634, 5-6=-3352/2325

BOT CHORD 2-11=-2041/2905, 9-11=-2054/2919, 8-9=-2014/2918, 6-8=-2001/2905

WEBS

3-11=-355/516, 3-9=-697/1078, 4-9=-769/447, 5-9=-698/1079, 5-8=-355/516

NOTES- (11)

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1270 lb uplift at joint 2 and 1270 lb uplift at joint 6
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 220 lb down and 240 lb up at 7-0-0, 115 lb down and 108 lb up at 9-0-12, 115 lb down and 108 lb up at 11-0-12, 115 lb down and 108 lb up at 12-6-0, 115 lb down and 108 lb up at 13-11-4, and 115 lb down and 108 lb up at 15-11-4, and 220 lb down and 240 lb up at 18-0-0 on top chord, and 340 lb down and 420 lb up at 7-0-0, 81 lb down and 81 lb up at 9-0-12, 81 lb down and 81 lb up at 11-0-12, 81 lb down and 81 lb up at 12-6-0, 81 lb down and 81 lb up at 13-11-4, and 81 lb up at 15-11-4, and 340 lb down and 420 lb up at 17-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-8-11 oc purlins.

Rigid ceiling directly applied or 6-8-6 oc bracing

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 29,2019

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE U.S.E.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see _____ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information ______available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | HARRISON RES |
|---------------------|-----------------------|------------|-----|-----|--------------------------|
| 2033215 | T01 | HIP GIRDER | 1 | 1 | T17713750 |
| Puildom FiretCaures | Independing El. 22244 | | | | Job Reference (optional) |

Builders FirstSource,

Jacksonville, FL - 32244

8 240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 13 2019 Page 2 ID Na4?LzVoi51_vU6na?cx?BzOClt-2?drRqhyyJjMhkt9rBJTEORK6vnBl8niwM7qEKytHTK

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert 1-3=54, 3-5=54, 5-7=54, 12-15=20

THE STATE OF THE S

Concentrated Loads (lb)

Vert 3=-173(F) 10=-59(F) 11=-340(F) 9=-59(F) 4=-115(F) 8=-340(F) 18=-115(F) 19=-115(F) 20=-115(F) 21=-115(F) 22=-59(F) 23=-59(F) 24=-59(F)

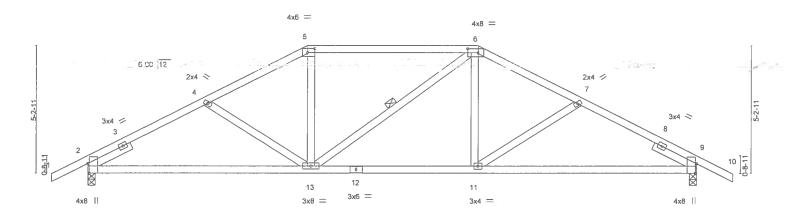
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



| Job | Truss | Truss Type | Qty | Ply | HARRISON RES | | |
|-----------------------|---------------------------|------------|--------------|-------------|----------------------|----------------------------------|---------------|
| | | | | | ** | | T17713751 |
| 2033215 | T02 | HIP | 1 | 1 | | | |
| | | J | | <u> </u> | Jab Reference (aptic | nal) | |
| Builders FirstSource, | Jacksonville, FL - 32244, | | | 3 240 s Jur | n 8 2019 MiTek Indus | stries, Inc. Mon Jul 29 06:53 14 | 1 2019 Page 1 |
| | | | ID Na4?LzVoi | 51_vU6na7 | cx?BzOCIt-WBBDeA | iajdrCluSLPuqinc_V3J3g1gAs | 90tNmmytHTJ |
| 1-6-0 | 4-10-14 | 9-0-0 | 16-0-0 | | 20-1-2 | 25-0-0 | 26-6-0 |
| 1-6-0 | 4-10-14 | 4-1-2 | 7-0-0 | | 4-1-2 | 4-10-14 | 1-6-0 |

Scale = 1:45.6



| | - | 9-0-0 | | | | 7-0-0 | | + | | | 25-0-0 | |
|-------------|------------|-------------------------------------|-----------------|-----------------|------------|----------|---------|-------|--------|-----|-----------|---------------|
| Plate Offse | ets (X,Y)- | 9-0-0 [2:0-4-12,Edge], [5:0-3-8, | 0-2-0], [6:0-5- | 4,0-2-0], [9.0- | 4-12,Edge] | | | | | | 9-0-0 | |
| LOADING | (psf) | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
| CLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.47 | Vert(LL) | 0.24 1 | 1-20 | >999 | 240 | MT20 | 244/190 |
| CDL | 7.0 | Lumber DOL | 1.25 | BC | 0.69 | Vert(CT) | -0.24 1 | 1-20 | >999 | 180 | | |
| BCLL | 0.0 | Rep Stress Incr | YES | WB | 0.23 | Horz(CT) | 0.05 | 9 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TF | PI2014 | Matrix | -AS | | | | | | Weight 12 | 9 lb FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1006/0-3-8, 9=1006/0-3-8

Max Horz 2=-71(LC 10)

Max Uplift 2=-432(LC 9), 9=-432(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1429/1726, 4-5=-1268/1626, 5-6=-1116/1523, 6-7=-1268/1626, 7-9=-1429/1725

BOT CHORD 2-13=-1409/1229, 11-13=-1245/1116, 9-11=-1421/1229

WEBS 5-13=-475/331, 6-11=-477/331

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II, Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- will fit between the bottom chord and any other members.
 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 2 and 432 lb uplift at joint 9.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

July 29,2019

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

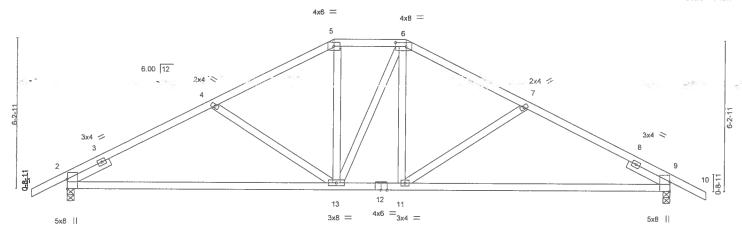
ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information.



6904 Parke East Blvd. Tampa, FL 36610

| | Job | Truss | Truss Type | | Qty | Ply | HARRISON RES. | |
|---|--------------------------|--------------------------|------------|--------|-----|-------------|--|----------------------|
| | 2033215 | тоз | HIP | | 1 | 1 | | T17713752 |
| Į | | | | | | | Job Reference (optional) | |
| | Builders FirstSource, Ja | acksonville, FL - 32244, | | | 8 | 1.240 s Jur | 8 2019 MiTek Industries, Inc. Mon Jul 29 | 06 53 15 2019 Page 1 |
| | | | | ID Na | | | x?BzOClt-?NkbsWiCUxz3w10XycLxKpWgd | |
| | -1-6-0 | 6-1-2 | 11-0-0 | 14-0-0 | | 18-10-14 | 25-0-0 | 26-6-0 |
| | 1-6-0 | 6-1-2 | 4-10-14 | 3-0-0 | | 4-10-14 | 6-1-2 | 1-6-0 |

Scale = 1 46 1



| | - | | 1-0-0 1-0-0 | | 14-0-0 3-0-0 | | | 25-0-0 | | | |
|---------------------------------|-------------------------------|---|------------------------------|---------------------------------------|---|---|-------------------------------|--------------------------|----------------|-----------------|--|
| Plate Offse | ts (X.Y)- | [2:0-4-12,Edge], [6:0-5-4, | | 2,Edge] | 3-0-0 | | | 11-0-0 | | | |
| LOADING TCLL TCDL BCLL | (psf) 20.0 7.0 0.0 * | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr | 2-0-0 1.25 1.25 YES | CSI. TC 0.48 BC 0.96 WB 0.35 | DEFL. Vert(LL) Vert(CT) Horz(CT) | in (loc) 0.52 11-20 -0.53 11-20 0.04 9 | l/defl >578 >563 n/a | L/d 240 180 n/a | PLATES MT20 | GRIP 244/190 | |
| BCDL | 10.0 | Code FBC2017/TI | PI2014 | Matrix-AS | , , | | | | Weight, 133 lb | FT = 20% | |

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8 SLIDER

REACTIONS. (lb/size) 2=1006/0-3-8, 9=1006/0-3-8

Max Horz 2=-85(LC 10)

Max Uplift 2=-400(LC 9), 9=-400(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4-1408/1683, 4-5-1158/1509, 5-6-983/1410, 6-7-1157/1508, 7-9-1408/1683 TOP CHORD

BOT CHORD 2-13=-1379/1215, 11-13=-1077/982, 9-11=-1387/1215

WEBS 4-13=-299/384, 5-13=-517/329, 6-11=-520/329, 7-11=-300/386

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind; ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone, porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 2 and 400 lb uplift at joint 9.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

July 29,2019

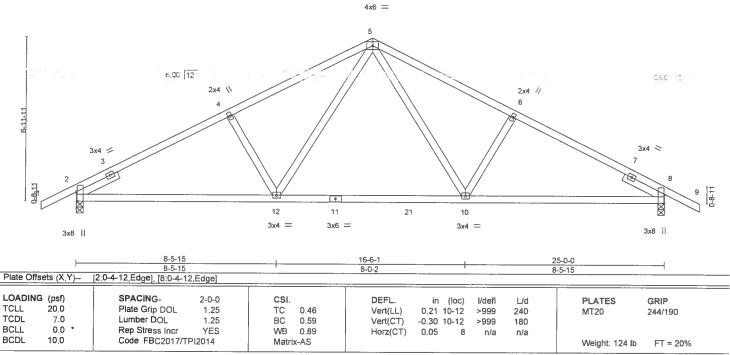
A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI⊩7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing as always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



| Job | Truss | Truss Type | Qty | Ply | HARRISON RES | | |
|-----------------------|--------------------------|------------|-------------|-----------------|--------------------------|--------------------|-------------------|
| 2033215 | T04 | COMMON | 6 | 1 | | | T17713753 |
| | | | | | Job Reference (optional) | | |
| Builders FirstSource, | Jacksonville, FL - 32244 | | | 3 240 s Jur | 8 2019 MiTek Industries | Inc. Mon Jul 29 06 | 53 16 2019 Page 1 |
| | | | ID Na4?LzVo | i51_vU6na | ?cx?BzOClt-Tal_3sjqFE5w | vYBbkWJsAs13ri6m | ijVQL8cJMUrfytHTH |
| -1-6-0 | 6-5-15 | 12-6-0 | 18 | -6-1 | 1 | 25-0-0 | 26-6-0 |
| 1-6-0 | 6-5-15 | 6-0-1 | ' 6 | -0-1 | , | 6-5-15 | 1-6-0 |

Scale = 1:47.1



BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1006/0-3-8, 8=1006/0-3-8

Max Horz 2=95(LC 11)

Max Uplift 2=374(LC 9), 8=374(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1433/1758, 4-5=-1300/1763, 5-6=-1300/1763, 6-8=-1433/1758 **BOT CHORD** 2-12=-1422/1228, 10-12=-888/868, 8-10=-1430/1228

WEBS

5-10=-757/474, 6-10=-286/306, 5-12=-757/474, 4-12=-286/306

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C, Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 374 lb uplift at joint 2 and 374 lb uplift at joint 8.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

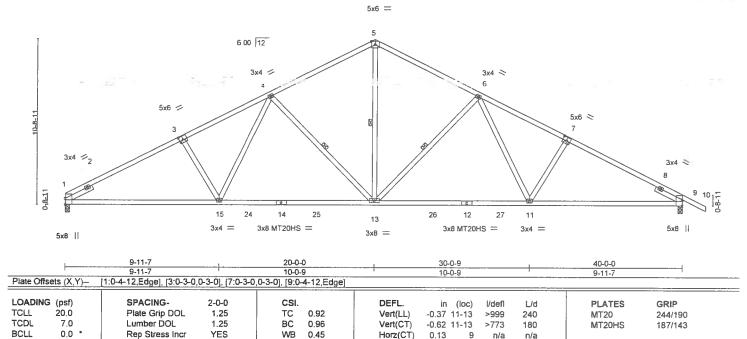
July 29,2019

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| Job | Truss | Truss Type | | Qty | Ply | HARRISON RES | | |
|-----------------------|---------------------------|------------|--------|------------------|------------|--------------------------|--------------------|----------------|
| 2033215 | T05 | COMMON | | _ | ١. | | | T17713754 |
| 2000210 | 103 | COMMON | | ' | ' | Job Reference (optional) | | |
| Builders FirstSource, | Jacksonville, FL - 32244, | | | | 8,240 s Ju | n 8 2019 MiTek Industrie | | 17 2019 Page 1 |
| | | | | ID.Na4?LzVoi51_v | U6na?cx?l | BzOClt-xmsMGCkS0YDn9 | PLAW41NPPEcvDW08E_ | Tlrz51N5ytHTG |
| | 7-7-0 | 13-3-7 | 20-0-0 | , 26-8-9 | | 32-5-0 | 40-0-0 | 41-6-0 |
| | 7-7-0 | 5-8-7 | 6-8-9 | 6-8-9 | | 5-8-7 | 7-7-0 | 1-6-0 |

Scale = 1 72 0



BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD

10.0

2x4 SP No.2 *Except*

12-14; 2x4 SP M 31 2x4 SP No.3

WEBS

SLIDER Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8

(lb/size) 1=1478/0-3-8, 9=1563/0-3-8 Max Horz 1=-241(LC 13) REACTIONS.

Max Uplift 1=-551(LC 12), 9=-602(LC 13)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown

Code FBC2017/TPI2014

TOP CHORD 1-3=-2515/1368, 3-4=-2355/1372, 4-5=-1707/1081, 5-6=-1707/1082, 6-7=-2346/1366,

7-9=-2505/1362

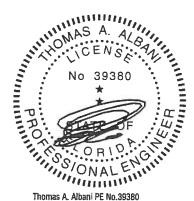
BOT CHORD 1-15=-1057/2171, 13-15=-814/1874, 11-13=-812/1871, 9-11=-1049/2159 WEBS

5-13=-658/1170, 6-13=-631/492, 6-11=-210/459, 7-11=-247/296, 4-13=-635/495,

4-15=-218/462, 3-15=-255/301

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 551 lb uplift at joint 1 and 602 lb uplift at joint 9.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code



Weight: 214 lb

5-13, 6-13, 4-13

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

1 Row at midpt

FT = 20%

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

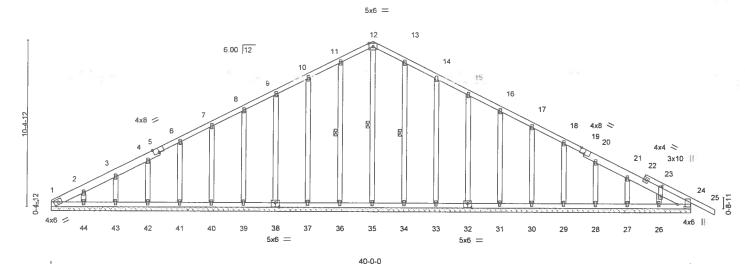
July 29,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MTex® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and nor-porty incorporate this design into the overall building designer must verify the applicability of design parameters and nor-porty incorporate this design into the overall building designer must verify the applicability of design parameters and nor-porty incorporate this design into the overall building designer must verify the applicability of design parameters and nor-porty incorporate this design into the overall building designer must verify the applicability of design parameters and nor-porty incorporate this design into the overall building designer must verify a true of the overall building designer must be sufficient to the overall



| Job | Truss | Truss Type | Qty | Ply | HARRISON RES | |
|-----------------------|-------------------------|------------------------|------------|------------|--|----------------------|
| 2033215 | T05G | Common Supported Gable | 1 | 1 | | T17713755 |
| | | | | | Job Reference (optional) | |
| Builders FirstSource, | Jacksonville, FL - 3224 | 4, | | 8 240 s Ju | n 8 2019 MiTek Industries, Inc. Mon Jul 29 | 06 53 19 2019 Page 1 |
| | | | ID Na4?LzV | oi51_vU6n | a?cx?BzOClt-t9_6humjY9UVPfKlBRQtUfhR | xKx3iypblHa8R_ytHTE |
| | | 20-0-0 | | | 40-0-0 | A1-6-0 |
| ' | | 20-0-0 | 1 | | 20-0-0 | 1-6-0 |

Scale = 1 69 4



40-0-0 Plate Offsets (X,Y)-[5:0-4-0,Edge], [19:0-4-0,Edge], [23:0-7-6,0-1-0], [24:0-2-0,0-5-6], [32:0-3-0,0-3-0], [38:0-3-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) L/d **PLATES** TCLL 20 0 Plate Grip DOL 1.25 TC 0.14 Vert(LL) -0.00 25 n/r 120 244/190 TCDL 7.0 Lumber DOI 1 25 BC 0.04 Vert(CT) -0.01 25 n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.14 Horz(CT) 0.01 24 BCDI Code FBC2017/TPI2014 10.0 Matrix-S Weight: 289 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

1-5,19-24; 2x6 SP No.2

BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. All bearings 40-0-0.

(lb) - Max Horz 1=-226(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 36, 37, 38, 39, 40, 41, 43, 34, 33, 32, 31, 30, 29, 27, 24

except 42=-100(LC 12), 44=-111(LC 12), 28=-101(LC 13), 26=-102(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 34, 33, 32, 31,

30, 29, 28, 27, 26, 24

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-300/98, 9-10=-96/257, 10-11=-118/321, 11-12=-137/375, 12-13=-137/375,

13-14=-118/321, 14-15=-96/257

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 36, 37, 38, 39, 40, 41, 43, 34, 33, 32, 31, 30, 29, 27, 24 except (it=lb) 42=100, 44=111, 28=101, 26=102
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

12-35, 11-36, 13-34

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 29,2019

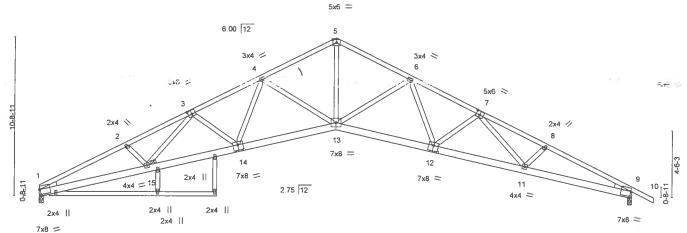
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and truss systems. see __ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria. VA 22314.



| Job | Truss | Truss Type | Qty | Ply | HARRISON RES |
|-----------------------|---------------------------|------------|------------------|----------------|---|
| 2033215 | T06 | SCISSORS | 5 | 1 | T17713756 |
| | <u>l</u> | | | | Job Reference (optional) |
| Builders FirstSource, | Jacksonville, FL - 32244, | | | 8 240 s Jur | 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 20 2019 Page 1 |
| | | | ID:Na4?Lz\ | /oi51_vU6na?cx | ?BzOCIt-LLYUvEmLJTcM0pvVI9x60tEV6k9VREZkXxKh_QytHTD |
| | -10-14 10-4-2 | 15-0-3 2 | 0-0-0 , 24-11-13 | , 29-7-1 | 40-0-0 41-6-0, |
| 5 | 10-14 4-5-4 | 4-8-1 4- | 11-13 4-11-13 | 4-8-1 | 4-5-4 5-10-14 1-6-0 |

Scale = 1,75,1



| | 7-3-2 7-3-2 | 12-0-0 4-8-15 | 13-4-2 1-4-2 | 20-0-0 6-7-15 | 26-7- 6-7-1 | 5 | | 32-8- 6-1- | -0 | 40-0-0 7-3-1 | — |
|---|---|------------------------------|------------------------|------------------------------|---|------------------------------|-------------------|-------------------------------|--------------------------|----------------------------------|-----------------------------|
| Plate Offsets (X,Y) | [1:0-0-7,0-4-5], [1:0-9-14 ,0-4-8] | I,0-0-1], [1:0- | -0-2,0-0-1], [3:0 | -3-0,0-3-0], | [7:0-3-0,0-3-0], [9:0 | -0-2,0-0 | -1], [9:0 |)-0-7,0-4- | 5], [9:0-9-14 | ,0-0-1], [12:0-4-0,0-4-8] | , [14:0-4-0 |
| LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0 | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/1 | 2-0-0 1.25 1.25 YES | CSI. TC BC WB | 0.63 0.54 0.87 x-AS | DEFL. Vert(LL) Vert(CT) Horz(CT) | in -0.39 -0.72 0.43 | (loc) 13 13 | l/defl >999 >664 n/a | L/d 240 180 n/a | PLATES MT20 Weight: 261 lb | GRIP 244/190 FT = 20% |

BRACING-

JOINTS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

1 Brace at Jt(s): 15

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2 *Except*

1-14,9-12: 2x6 SP M 26, 1-16: 2x4 SP No.3

WERS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 9=1563/0-3-8, 1=1478/0-3-8

Max Horz 1=-148(LC 8)

Max Uplift 9=-338(LC 13), 1=-310(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-4112/2149, 2-3=-3954/2112, 3-4=-3731/1949, 4-5=-2920/1509, 5-6=-2920/1509,

6-7=-3727/1945, 7-8=-3930/2094, 8-9=-4086/2131

1-15=-1805/3670, 14-15=-1663/3636, 13-14=-1312/3216, 12-13=-1310/3213, **BOT CHORD**

11-12-1655/3626, 9-11-1787/3643

5-13=-1121/2288, 6-13=-704/526, 6-12=-275/517, 7-12=-318/304, 4-13=-707/530,

4-14=-279/521, 3-14=-326/311

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II, Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 9, 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=338, 1=310.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

July 29,2019

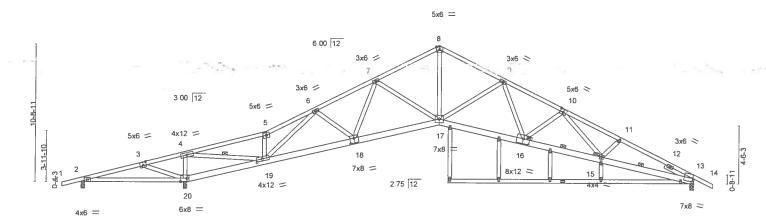
🔔 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE. Design valid for use only with MiTeN® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Dracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see. ASVITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Sufe 312, Alexandria, VA 22314.



6904 Parke East Blvd

| Job | Truss | Truss Type | Qty | Ply | HARRISON RES | |
|----------------------------|-------------------------|-----------------|------------|-------------|----------------------------------|--------------------------------|
| 2033215 | T08 | Roof Special | 4 | 1 | | T17713757 |
| | | <u> </u> | | | Job Reference (optional) | |
| Builders FirstSource, Ja | acksonville, FL - 32244 | | | 3 240 s Jur | 8 2019 MiTek Industries, Inc. Mi | on Jul 29 06:53 22 2019 Page 1 |
| | | | ID Na4?Lz\ | oi51_vU6i | na?cx?BzOClt-HjfFKvobr4s4G63tta | aza6IJm9Xkov811?Fpo2JytHTB |
| ₁ 1-6-0, 4-11-0 | 8-3-8 14-5- | 4 18-4-1 23-0-3 | 28-0-0 | 32-11-13 | , 37-8-1 , 42-1-6 | 48-0-0 49-6-0 |
| ካ-6-0' 4-11-0 | 3-4-8 6-2- | 3-10-2 4-8-2 | 4-11-13 | 4-11-13 | 4-8-4 4-5-5 | 5-10-10 1-6.0 |

Scale = 1 86 8



| | 8-1-12 8-3-8 8-1-12 0-1-12 | 14-5-14 6-2-6 | 21-4-1 6-10-2 | 28-0-0 6-7-15 | 28-8-8 0-8-8 | 34-7-15 5-11-7 | 40-9-1 6-1-2 | 48-0-0 7-2-15 | |
|---|---|---|--|---|-----------------------------------|--|--------------------------|----------------------------------|-----------------------------|
| Plate Offsets (X,Y)- | [3:0-3-0,0-3-4], [10:0-3-0 | 0,0-3-0], [13:0-0- | 10,0-3-5], [16:0-6-0,0-4-8 |], [18:0-4-0,0-4-8] | | | | | |
| LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0 | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/7 | 2-0-0 1.25 1.25 YES TPI2014 | CSI. TC 0.85 BC 0.93 WB 0.87 Matrix-AS | DEFL. Vert(LL) Vert(CT) Horz(CT) | in (-0.31 -0.59 16 0.32 | (loc) I/defl 17 >999 6-17 >811 13 n/a | L/d 240 180 n/a | PLATES MT20 Weight: 326 lb | GRIP 244/190 FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

WEBS

JOINTS

Structural wood sheathing directly applied.

4-19

Rigid ceiling directly applied. Except:

5-11-0 oc bracing: 13-15

6-2-0 oc bracing: 15-16

7-5-0 oc bracing: 16-17

1 Brace at Jt(s): 16, 15

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x6 SP No.2 *Except*

2-20: 2x4 SP No.2, 21-22,13-22: 2x4 SP No.3

WEBS 2x4 SP No.3 *Except*

4-19: 2x4 SP No.2 SLIDER Right 2x4 SP No.3 2-5-8

(lb/size) 2=-275/0-3-8, 20=2573/0-3-8, 13=1414/0-3-8 REACTIONS.

Max Horz 2=-229(LC 17)
Max Uplift 2=-393(LC 19), 20=-1012(LC 12), 13=-571(LC 13) Max Grav 2=77(LC 12), 20=2573(LC 1), 13=1414(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-892/2119, 3-4=-1259/2543, 4-5=-1375/655, 5-6=-1531/845, 6-7=-2586/1338,

7-8=-2324/1189, 8-9=-2324/1189, 9-10=-3129/1625, 10-11=-3383/1801,

11-13=-3501/1828

BOT CHORD 2-20=-2012/895, 19-20=-2653/1437, 18-19=-884/2192, 17-18=-839/2332,

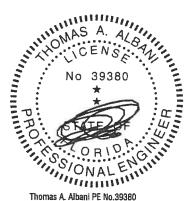
16-17=-1016/2667, 15-16=-1374/3100, 13-15=-1507/3117

WEBS 3-20=-459/471, 4-20=-1605/876, 4-19=-1849/3863, 5-19=-598/439, 6-19=-1132/525,

7-17=-393/324, 8-17=-837/1757, 9-17=-700/526, 9-16=-276/514, 10-16=-341/322

NOTES-(10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 7) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=393, 20=1012, 13=571.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

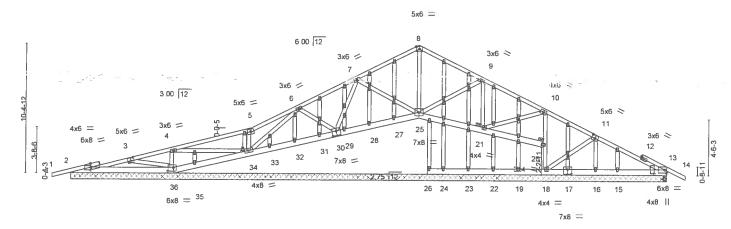
July 29,2019

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandra, VA 22314.



| Job | Truss | Truss Type | Qty | Ply | HARRISON RES | |
|-----------------------|---------------------------|-----------------------|----------------|------------|---|-----------|
| 2033215 | T08G | GABLE | 1 | 1 | | T17713758 |
| | | | | | Job Reference (optional) | |
| Builders FirstSource, | Jacksonville, FL - 32244, | | | 8.240 s Ju | in 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 28 20 | 19 Page 1 |
| | | | ID Na4?LzVoi51 | vU6na?cx | ?BzOClt-6t1WaztMQwcD_1W1Dq4_LZZwyyxOI?pvNBG | 6GvvtHT5 |
| 1-6-0 | | 14-5-14 18-4-1 23-0-3 | 28-0-0 | 33-1-9 | | 49-6-Q |
| 1-6-0' | 4-11-0 ' 3-4-8 ' | 6-2-6 3-10-2 4-8-2 | 4-11-13 | 5-1-9 | 4-10-7 4-3-0 5-9-0 | 1-6-0 |

Scale = 1:89.5



| | | 8-3-8 8-3-8 | 14-5-14 | 21-4-1 6-10-2 | 28-0-0 | 30-8-8 2-8-8 | 33-1-9 | <u>38-0-0</u> 4-10-7 | | 9-0-0 |
|-----------------|---------------|---|------------------|--------------------|----------------------|------------------------|----------------|-------------------------|--------------------------|-----------------|
| Plate Offse | ets (X,Y)- | [2:0-0-4,Edge], [2:0-0-4,0 [56:0-1-10,0-1-0] | -2-12], [3:0-3-0 | | | | | | 4:0-2-0,0-0-15], [36:0-5 | -4,0-3-8], |
| LOADING TCLL | (psf) 20.0 | SPACING- Plate Grip DOL | 2-0-0 1.25 | CSI. TC 0.38 | DEFL. Vert(LL) | in (loc) -0.05 2-36 | l/defl >999 | L/d 240 | PLATES MT20 | GRIP 244/190 |
| TCDL BCLL | 7.0 0.0 * | Lumber DOL Rep Stress Incr | 1.25 YES | BC 0.28 WB 0.21 | Vert(CT) Horz(CT) | -0.09 2-36 0.01 13 | | 180 n/a | | |
| 3CDL | 10.0 | Code FBC2017/T | PI2014 | Matrix-S | | | | - <u></u> | Weight: 398 lb | FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

JOINT\$

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2 *Except*

10-18: 2x4 SP No.3

WEBS 2x4 SP No.3 2x4 SP No.3

OTHERS

WEDGE

Right: 2x4 SP No.3

REACTIONS. All bearings 48-0-0.

(lb) -Max Horz 2=-221(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 15 except 2=-196(LC 8), 36=-341(LC 8), 18=-103(LC 13), 34=-205(LC 12), 30=-289(LC 12), 25=-197(LC 12),

16=-256(LC 13), 13=-150(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 13, 13, 26, 28, 29, 31, 32, 33,

35, 27, 15, 24, 23, 22, 19 except 2=324(LC 23), 36=647(LC 23), 18=508(LC 24),

34=383(LC 23), 30=360(LC 23), 25=681(LC 1), 16=312(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-286/298, 10-11=-90/280

BOT CHORD 35-36=-229/253, 34-35=-224/255, 29-30=-84/279, 28-29=-88/280, 27-28=-89/280,

25-27=-86/279, 18-20=-444/174, 10-20=-398/179 3-36=-402/326, 4-36=-349/287, 5-34=-257/260, 7-30=-261/166, 8-25=-332/122,

9-25=-324/361, 11-16=-278/330

NOTES-(11)

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4,2psf; BCDL=3,0psf; h=18ft; Cat. II, Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Bearing at joint(s) 25, 28, 29, 27 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. Except.

6-0-0 oc bracing: 18-20

1 Brace at Jt(s): 20, 21

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610

July 29,2019

Continued on page 2

🛦 WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and included building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Pracing indicated its to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucclings with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



6904 Parke East Blvd

| Job | Truss | Truss Type | Qty | Ply | HARRISON RES | |
|-------------------------|--------------------------|------------|-----|-------------|--|-----------|
| 2033215 | T08G | GABLE | 1 | 1 | | T17713758 |
| | | | | | Job Reference (optional) | |
| Builders FirstSource, J | acksonville, FL - 32244, | | 8 | 3 240 s Jui | n 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 28 2019 | Page 2 |

Jacksonville, FL - 32244,

8 240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 28 2019 Page 2 ID Na4?LzVoi51_vU6na?cx?BzOClt-6t1WaztMQwcD_1W1Dq4_LZZwyyxOI?pvNBG6GyytHT5

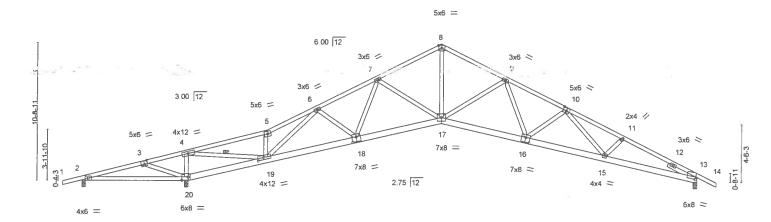
NOTES- (11)

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15 except (jt=lb) 2=196, 36=341, 18=103, 34=205, 30=289, 25=197, 16=256, 13=150,
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



| Job | Truss | Truss Type | | Qty | Ply | HARRISON RES | | | |
|----------------------------|--------------------------|---------------|--------------|------------|-------------|----------------------|---------------|-------------------|-------------|
| 2033215 | T09 | Roof Special | | 7 | 1 | | | | T17713759 |
| | | | | | | Job Reference (optio | nal) | | |
| Builders FirstSource, J | acksonville, FL - 32244, | | | | 8.240 s Jur | B 2019 MiTek Indus | tries Inc Mon | Jul 29 06:53 29 : | 2019 Page 1 |
| | | | ID. | la4?LzVoi5 | 1_vU6na?c | c?BzOClt-a4buoJt_BE | k4cB5DnYbDu | um5zOM8m1lm3d | cr0goPytHT4 |
| ₁ 1-6-0, 4-11-0 | 8-3-8 14-5-1 | 14 , 18-4-1 , | 23-0-3 , 28- | 0-0 | 32-11-12 | 37-7-15 | 42-1-4 | 48-0-0 | 49-6-0 |
| <u> </u> | 3-4-8 6-2-6 | 3-10-2 | 4-8-2 4-1 | -13 | 4-11-12 | 4-8-3 | 4-5-6 | 5-10-12 | 1.6.0 |

Scale = 1 86 8



| | - | 8-1-12 8-3-8 8-1-12 0-1-12 | 14-5-14 6-2-6 | + 21-4-1 6-10-2 | 28-0-0 6-7-15 | 34-7-14 6-7-14 | 40-9-0 6-1-1 | 48-0-0 7-3-0 |
|---|-------------------------------|--|--|--|----------------------|---|--------------------------|--|
| Plate Offse | ets (X,Y)- | [3:0-3-0,0-3-4], [10:0-3-0, | .0-3-0], [13:0-0- | 14,0-2-4], [16:0-4-0,0-4-8 |], [18:0-4-0,0-4-8], | [19:0-5-7,0-2-0], [20:0-0 | 6-0,0-3-0] | |
| LOADING TCLL TCDL BCLL BCDL | (psf) 20.0 7.0 0.0 * | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TI | 2-0-0 1.25 1.25 YES PI2014 | CSI. TC 0.85 BC 0.85 WB 0.87 Matrix-AS | , , | in (loc) l/defl -0.31 17 >999 -0.59 16-17 >810 0.32 13 n/a | L/d 240 180 n/a | PLATES GRIP MT20 244/190 Weight: 282 lb FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied

Rigid ceiling directly applied.

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x6 SP No.2 *Except*

2-20: 2x4 SP No.2

2x4 SP No.3 *Except* WEBS 4-19: 2x4 SP No.2

Right 2x4 SP No.3 2-5-8 SLIDER

REACTIONS. (lb/size) 2=-275/0-3-8, 20=2573/0-3-8, 13=1414/0-3-8

Max Horz 2=140(LC 11)

Max Uplift 2=373(LC 19), 20=571(LC 12), 13=321(LC 13) Max Grav 2=43(LC 12), 20=2573(LC 1), 13=1414(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-892/2119, 3-4=-1259/2543, 4-5=-1375/655, 5-6=-1531/845, 6-7=-2586/1338,

7-8=-2324/1189, 8-9=-2324/1189, 9-10=-3129/1625, 10-11=-3383/1801,

11-13=-3501/1828

BOT CHORD 2-20=-2012/895, 19-20=-2653/1437, 18-19=-884/2192, 17-18=-839/2332,

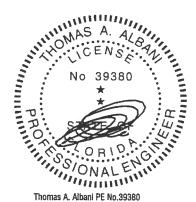
16-17=1016/2667, 15-16=1374/3100, 13-15=-1506/3117

WEBS 3-20=-459/471, 4-20=-1605/876, 4-19=-1849/3863, 5-19=-598/439, 6-19=-1132/525,

7-17=-393/324, 8-17=-837/1757, 9-17=-699/526, 9-16=-276/514, 10-16=-341/322

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4,2psf; BCDL=3,0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone, porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=373, 20=571, 13=321.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

July 29,2019

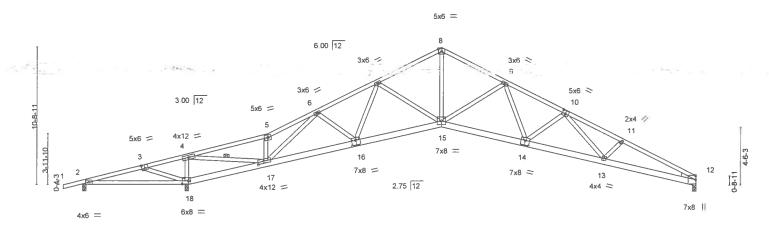
Marking - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the tabincation, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



| Job | Truss | Truss Type | Qty | Ply | HARRISON RES | | |
|-------------------------|--------------------------|-----------------|-------------|-------------|---------------------------|-----------------------------|-----------|
| | | 28 | | ! | | | T17713760 |
| 2033215 | T10 | Roof Special | 9 | 1 | 8.32 | | |
| | <u> </u> | | | | Job Reference (optional) | | |
| Builders FirstSource, J | acksonville, FL - 32244, | | | 3.240 s Jur | 8 2019 MiTek Industries | Inc Mon Jul 29 06:53:31:201 | 9 Page 1 |
| | | | ID:Na4?LzVo | i51_vU6na | r?cx?BzOClt-XSieD_vFjr_or | VFcuzehzBBJp9ptVCBM39V | ntHytHT2 |
| 71-6-0, 4-10-0 | 8-3-8 1 14-5-1 | 4 18-4-1 23-0-3 | 28-0-0 | 32-11-12 | , 37-7-15 , 42- | 1-4 , 48-0-0 | 1 |
| 1-6-0' 4-10-0 | 3-5-8 6-2-6 | 3.10.2 4.8.2 | A-11-13 | 1.11.12 | 483 4 | 5.6 5.10.12 | 7 |

Scale = 1 86.8



| | 8-1-12 | 8-3-8 | 14-5-14 | 21-4-1 | 28-0-0 | 34-7-1 | 4 | 40-9-0 | 48-0-0 | |
|---|--|---------------------|---------------------------------------|--|---|--|-------------------------------|--------------------------|----------------------------------|---------------------------------|
| | 8-1-12 | 0-1"12 | 6-2-6 | 6-10-2 | 6-7-15 | 6-7-14 | 4 | 6-1-1 | 7-3-0 | |
| Plate Offsets (X,Y)- | [3:0-3-0,0-3-4], [| 10:0-3-0,0 | -3-0], [12:0-0-1 | ,0-0-3], [12:0-4-0,Edge] | , [12:0-2-5,0-9-11] | [14:0-4-0,0-4-8 |], [16:0-4 | -0,0-4-8], [17:0-5- | 11,0-2-0], [18:0-6 | -0,0-3-0] |
| LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0 | SPACING Plate Grip Lumber D Rep Stres Code FB0 | DOL OL s Incr | 2-0-0 1.25 1.25 YES 12014 | CSI. TC 0.85 BC 0.94 WB 0,88 Matrix-AS | DEFL. Vert(LL) Vert(CT) Horz(CT) | in (loc) -0.31 15 -0.59 14-15 0.31 12 | l/defl >999 >807 n/a | L/d 240 180 n/a | PLATES MT20 Weight: 278 lb | GRIP 244/190 FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied

Rigid ceiling directly applied.

1 Row at midpt

LUMBER-

2x4 SP No.2 TOP CHORD

BOT CHORD 2x6 SP No.2 *Except*

2-18: 2x4 SP No.2

WEBS 2x4 SP No.3 *Except*

4-17: 2x4 SP No.2

WEDGE

Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=-280/0-3-8, 18=2584/0-3-8, 12=1328/0-3-8

Max Horz 2=149(LC 9)

Max Uplift 2=-374(LC 19), 18=-576(LC 12), 12=-291(LC 13)

Max Grav 2=47(LC 12), 18=2584(LC 1), 12=1328(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-939/2135, 3-4=-1311/2560, 4-5=-1380/649, 5-6=-1537/839, 6-7=-2604/1363,

7-8-2346/1225, 8-9-2346/1224, 9-10-3173/1673, 10-11-3499/1886,

11-12=-3656/1924

BOT CHORD 2-18=-2028/906, 17-18=-2670/1455, 16-17=-934/2205, 15-16=-900/2349,

14-15=-1092/2703, 13-14=-1461/3156, 12-13=-1640/3260 3-18=-459/473, 4-18=-1613/890, 4-17=-1892/3885, 5-17=-599/438, 6-17=-1143/555, WEBS

6-16=0/251, 7-15=392/311, 8-15=869/1777, 9-15=-717/536, 9-14=-286/531,

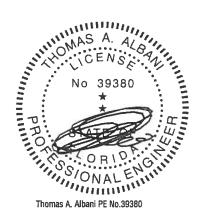
10-14=-357/331

NOTES-

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone, porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

 6) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=374, 18=576, 12=291.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

July 29,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314



6904 Parke East Blvd Tampa, FL 36610

Job Truss Truss Type Ply Qty HARRISON RES T17713761 2033215 T12 Common Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244 8 240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06:53 32 2019 Page 1 ID Na4?LzVoi51_vU6na?cx?BzOCII-?fG1QKwtU96fTfqoSg9wWPjexZFOEmfVlpEKPjytHT1 -1-6-0 17-0-0 18-6-0 4-1-4 Scale = 1 37.6 4x4 = 7 00 12 2x4 N 2x4 0 6 10 5x8 = 3x10 | 3x10 [] 17-0-0 8-6-0 Plate Offsets (X,Y)--[2:0-3-8,Edge], [8:0-6-2,Edge], [10:0-4-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI DEFL. in (loc) I/defl Ľ∕d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.14 10-17

-0.13 10-17

8 n/a

0.02

>999

>999

Rigid ceiling directly applied.

240

180

n/a

Structural wood sheathing directly applied

MT20

Weight: 92 lb

244/190

FT = 20%

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

7.0

10.0

0.0

WEBS 2x4 SP No.3 SLIDER

Left 2x6 SP No.2 1-11-8, Right 2x6 SP No.2 1-11-8

REACTIONS. (lb/size) 2=710/0-3-8, 8=710/0-3-8

Max Horz 2=182(LC 11)

Max Uplift 2=-279(LC 12), 8=-279(LC 13)

Lumber DOL

Rep Stress Incr

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-785/951, 4-5=-640/876, 5-6=-640/876, 6-8=-785/951

BOT CHORD 2-10=-718/641, 8-10=-724/641

WEBS 5-10=-721/409

NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

0.25

0.53

вс

WB 0.42

Matrix-AS

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

1.25

YES

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=279, 8=279,
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

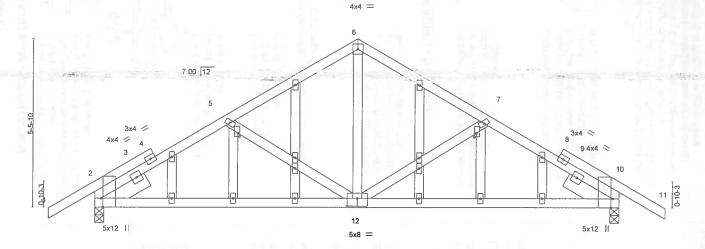
July 29,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see. ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Stately Information available from Truss Plate Institute. 218 N. Lee Street. Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty HARRISON RES T17713762 2033215 T12G GABLE Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244 8 240 s Jun 8 2019 MiTek Industries, Inc. Mon Jul 29 06 53 33 2019 Page 1 ID Na4?LzVoi51_vU6na?cx?BzOClt-TrqPdgwVFTEW4oP?0Og92cGn6zaazEwfWT_txAytHT0 8-6-0 12-7-4 17-0-0 18-6-0 1-6-0

Scale = 1 36.0



| Plate Offsets (X,Y)- | 2:0-3-8,Edge], [5:0-1-7,0-0-12], [7: | 8-6-0 D-1-7,0-0-12], [10:0-3-8,Edge], | 8-6-0 12:0-4-0,0-3-0] | |
|----------------------|--------------------------------------|--|-----------------------------|-------------------------|
| LOADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. in (loc) I/defl L | /d PLATES GRIP |
| TCLL 20,0 | Plate Grip DOL 1.25 | TC 0.35 | Vert(LL) 0.15 10-12 >999 24 | 40 MT20 244/190 |
| TCDL 7,0 | Lumber DQL 1.25 | BC 0.54 | Vert(CT) -0.13 2-12 >999 18 | 80 |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.35 | | n/a |
| BCDL 10,0 | Code FBC2017/TPI2014 | Matrix-AS | T Well | Weight: 117 lb FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

17-0-0

Structural wood sheathing directly applied

Rigid ceiling directly applied

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3 SLIDER Left 2x6 SP No.2 1-7-6, Right 2x6 SP No.2 1-7-6

REACTIONS. (lb/size) 2=702/0-3-8, 10=702/0-3-8

Max Horz 2=-171(LC 10)

Max Uplift 2=-286(LC 12), 10=-286(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-5=-809/948, 5-6=-617/838, 6-7=-617/837, 7-10=-809/947

BOT CHORD 2-12=-750/667, 10-12=-766/667 WEBS 6-12=-688/400, 7-12=-294/277, 5-12=-294/277

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=286, 10=286.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

July 29,2019

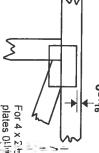


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully ambed teeth.



For 4 x 2 brientation, locate plates 011/1/16" from outside edge of truss.

00

o

This symbol indicates the required direction of slots in

*Plate location details available in MiTek 20/20 software or upon request.

connector plates.

PLATE SIZE

4 × 4

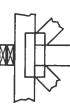
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

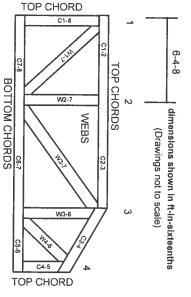
Industry Standards:

ANSI/TPI1: National Design Specification for Metal
Plate Connected Vood Truss Construction.

DSB-89: BCSI:

Design Standard for Bracing.
Building Component Safety Information,
Builde to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



Ņ

JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet; Mtl-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

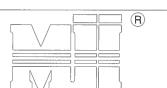
ω.

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

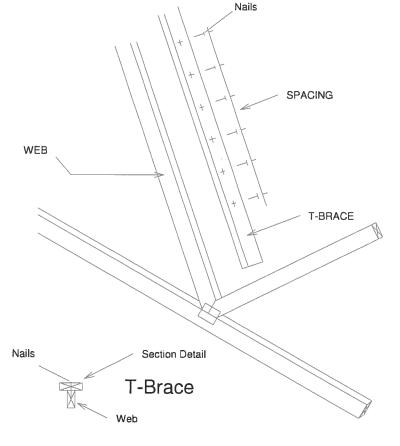
| A MITON AITHBIO | - | | | | | | | | |
|-------------------|-------------------|--------------|--|--|--|--|--|--|--|
| Nailing Pattern | | | | | | | | | |
| T-Brace size | Nail Size | Nail Spacing | | | | | | | |
| 2x4 or 2x6 or 2x8 | 10d (0.131" X 3") | 6" o.c. | | | | | | | |
| 2x4 or 2x6 or 2x8 | 10d (0.131" X 3") | 6" o.c. | | | | | | | |

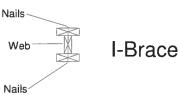
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

| All in out a street tracks | for One | for One-Ply Truss | | | | |
|----------------------------|-------------|------------------------------|--|--|--|--|
| | | Continuous ateral Bracing | | | | |
| Web Size | 1 | 2 | | | | |
| 2x3 or 2x4 | 2x4 T-Brace | 2x4 I-Brace | | | | |
| 2x6 | 2x6 T-Brace | 2x6 I-Brace | | | | |
| 2x8 | 2x8 T-Brace | 2x8 I-Brace | | | | |

| | | e Size -Ply Truss | | | | |
|------------|-------------------------|------------------------------|--|--|--|--|
| | Specified Rows of La | Continuous iteral Bracing | | | | |
| Web Size | 1 | 2 | | | | |
| 2x3 or 2x4 | 2x4 T-Brace | 2x4 I-Brace | | | | |
| 2x6 | 2x6 T-Brace | 2x6 I-Brace | | | | |
| 2x8 | 2x8 T-Brace | 2x8 T-Brace 2x8 I-Brace | | | | |

T-Brace / I-Brace must be same species and grade (or better) as web member.







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SCAB-BRACE DETAIL

MII-SCAB-BRACE

MíTek USA

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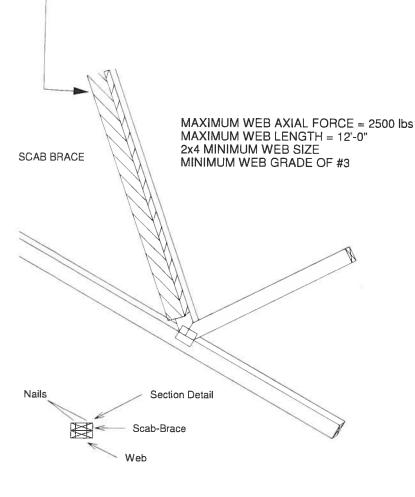


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

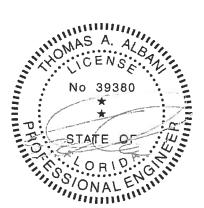
Scab must cover full length of web +/- 6".

THIS DETAIL IS NOT APLICABLE WHEN BRACING IS REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.

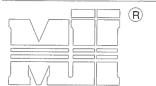


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STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

MiTek USA, Inc. Page 1 of 1

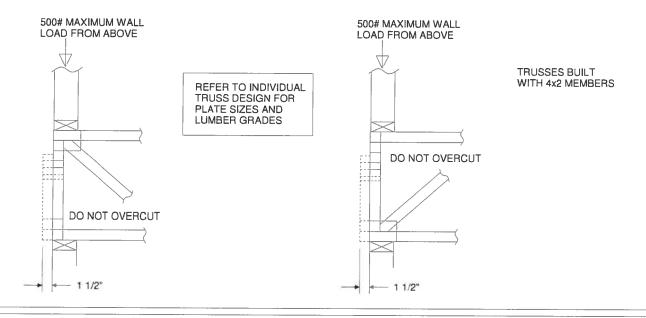


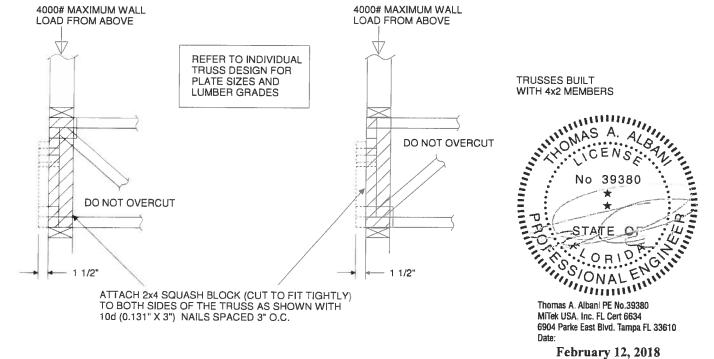
MiTek USA, Inc.

ENGINEERED BY A MiTek Affiliate

- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

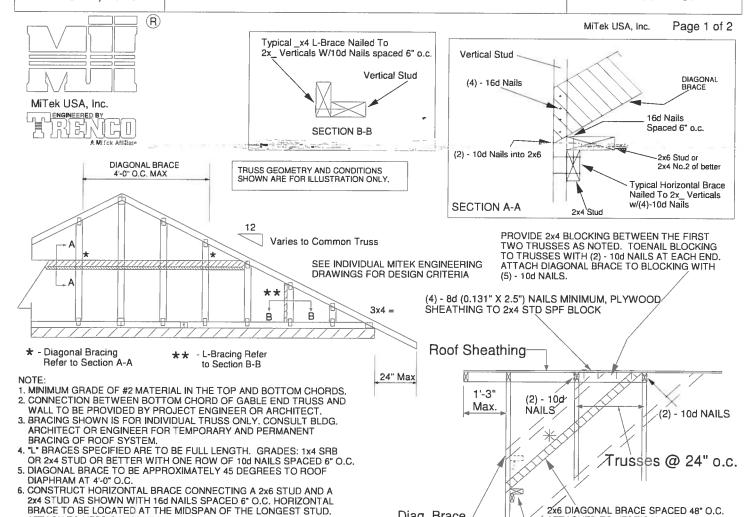
 3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPETTING OF THE WOOD.
- The first water a series 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
- 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X ORIENTATION ONLY.
 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





Standard Gable End Detail

MII-GE130-D-SP



Diag. Brace

at 1/3 points

End Wall

if needed

(REFER TO SECTION A-A) 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

06-01-13 BY SPIBIALSC.

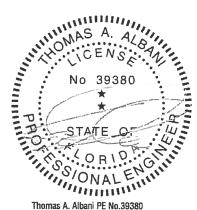
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species | Stud Spacing | Without Brace | 1x4 L-Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS | |
|---------------------------------|-----------------|---------------------|----------------|----------------|-------------------|---------------------------------------|--|
| and Grade | | Maximum Stud Length | | | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 3-9-13 | 4-1-1 | 5-9-6 | 7-1-3 | 11-5-7 | |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-5-4 | 3-6-8 | 5-0-2 | 6-10-8 | 10-3-13 | |
| 2x4 SP No. 3 / Stud | 24" O.C. | 2-9-11 | 2-10-11 | 4-1-1 | 5-7-6 | 8-5-1 | |

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



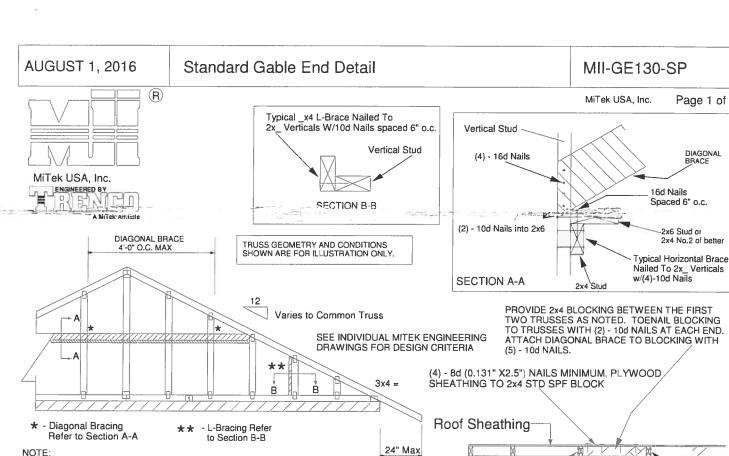
ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

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1'-3"

Max.

Diag. Brace

at 1/3 points

End Wall

if needed

(2) - 10d

NAILS,

NOTE

- MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
 CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT
- BRACING OF ROOF SYSTEM.

 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB
 OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

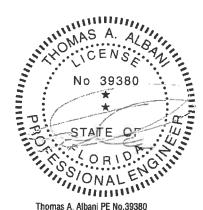
 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species | Stud Spacing | Without Brace | 1x4 L-Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS |
|---------------------------------|-----------------|------------------|----------------|----------------|-------------------|---------------------------------------|
| and Grade | | | ngth | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 4-0-7 | 4-5-6 | 6-3-8 | 8-0-15 | 12-1-6 |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-8-0 | 3-10-4 | 5-5-6 | 7-4-1 | 11-0-1 |
| 2x4 SP No. 3 / Stud | 24" O.C. | 3-0-10 | 3-1-12 | 4-5-6 | 6-1-5 | 9-1-15 |

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS



Page 1 of 2

DIAGONAL

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d

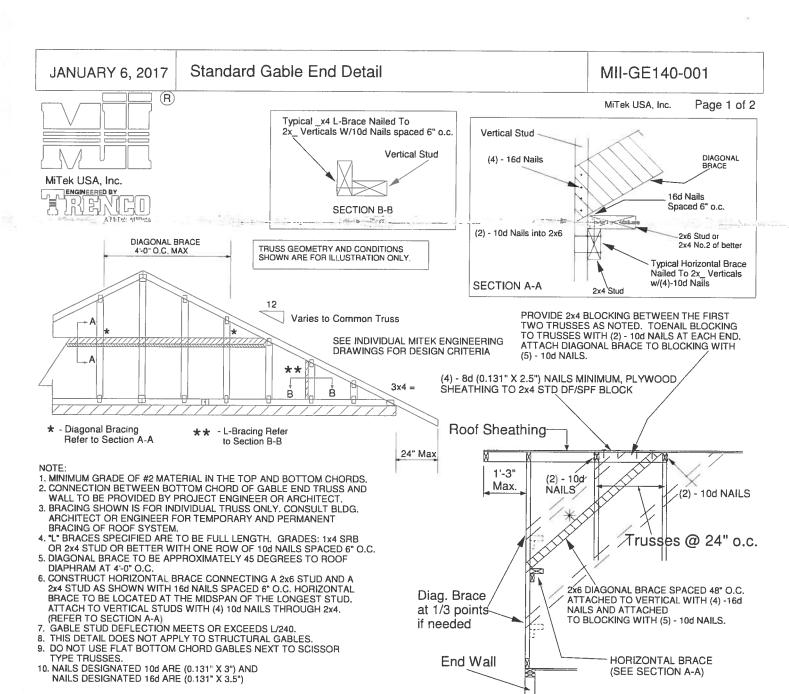
HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610



| Minimum Stud Size Species | Stud Spacing | Without Brace | 1x4 L-Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS | |
|---------------------------------|-----------------|---------------------|----------------|----------------|-------------------|---------------------------------------|--|
| and Grade | | Maximum Stud Length | | | | | |
| 2x4 DF/SPF Std/Stud | 12" O.C. | 3-10-1 | 3-11-7 | 5-7-2 | 7-8-2 | 11-6-4 | |
| 2x4 DF/SPF Std/Stud | 16" O.C. | 3-3-14 | 3-5-1 | 4-10-2 | 6-7-13 | 9-11-11 | |
| 2x4 DF/SPF Std/Stud | 24" O.C. | 2-8-9 | 2-9-8 | 3-11-7 | 5-5-2 | 8-1-12 | |

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

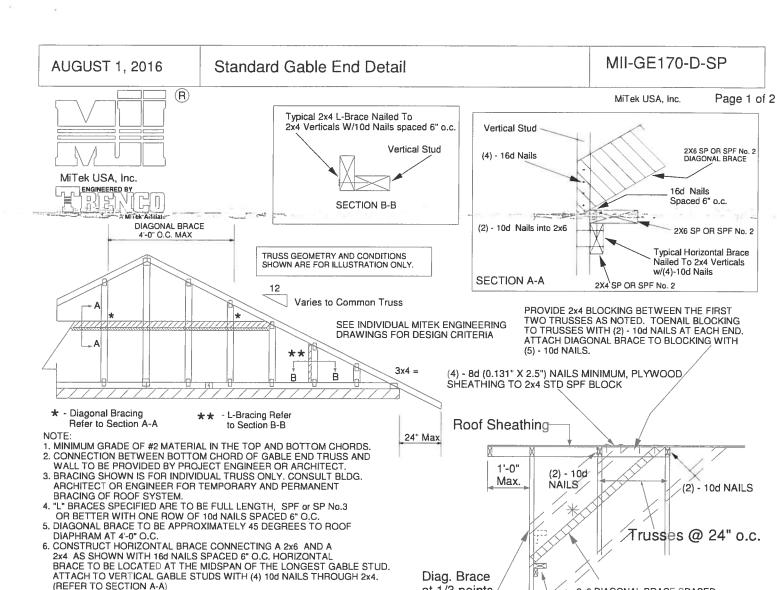
MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Gert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 19, 2018



at 1/3 points

End Wall

if needed

| Minimum Stud Size Species | Stud Spacing | Without Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS | | |
|---------------------------------|-----------------|---------------------|----------------|-------------------|---------------------------------------|--|--|
| and Grade | | Maximum Stud Length | | | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 3-9-7 | 5-8-8 | 6-11-1 | 11-4-4 | | |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-4-12 | 4-11-15 | 6-9-8 | 10-2-3 | | |
| 2x4 SP No. 3 / Stud | 24" O.C. | 2-9-4 | 4-0-7 | 5-6-8 | 8-3-13 | | |
| 2x4 SP No. 2 | 12" O.C. | 3-11-13 | 5-8-8 | 6-11-1 | 11-11-7 | | |
| 2x4 SP No. 2 | 16" O.C. | 3-7-7 | 4-11-5 | 6-11-1 | 10-10-5 | | |
| 2x4 SP No. 2 | 24" O.C. | 3-1-15 | 4-0-7 | 6-3-14 | 9-5-14 | | |

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

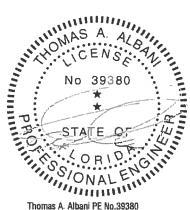
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH DURATION OF LOAD INCREASE : 1.60

TYPE TRUSSES.

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



2x6 DIAGONAL BRACE SPACED

48" O.C. ATTACHED TO VERTICAL WITH

HORIZONTAL BRACE

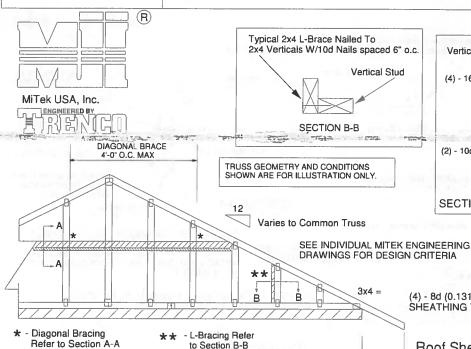
(SEE SECTION A-A)

(4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Standard Gable End Detail

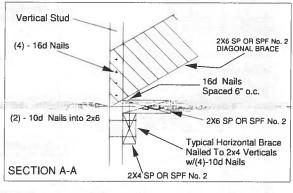
MII-GE180-D-SP



MiTek USA, Inc.

Page 1 of 2

(2) - 10d NAILS



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d NAILS

Roof Sheathing

1'-0"

Max.

24" Max

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3
OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4"-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

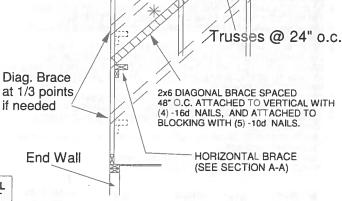
| NAILS DESIGNATED 16d ARE (0.131" X 3.5") | | | | | | | | | |
|--|-----------------|---------------------|----------------|-------------------|---------------------------------------|--|--|--|--|
| Minimum Stud Size Species | Stud Spacing | Without Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS | | | | |
| and Grade | . B. B. | Maximum Stud Length | | | | | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 3-7-12 | 5-4-11 | 6-2-1 | 10-11-3 | | | | |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-2-8 | 4-8-1 | 6-2-1 | 9-7-7 | | | | |
| 2x4 SP No. 3 / Stud | 24" O.C. | 2-7-7 | 3-9-12 | 5-2-13 | 7-10-4 | | | | |
| 2x4 SP No. 2 | 12" O.C. | 3-10-0 | 5-4-11 | 6-2-1 | 11-6-1 | | | | |
| 2x4 SP No. 2 | 16" O.C. | 3-5-13 | 4-8-1 | 6-2-1 | 10-5-7 | | | | |
| 2x4 SP No. 2 | 24" O.C. | 3-0-8 | 3-9-12 | 6-1-1 | 9-1-9 | | | | |

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

MiTek USA, Inc. Page 1 of 1

(R)

MiTek USA, Inc.

ENGINEERED BY

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE: 1.60

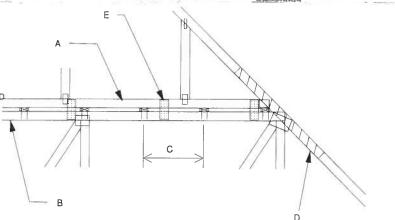
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING FNGINEER/DESIGNER ARE REQUIRED

IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

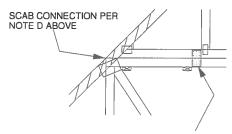
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W. (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

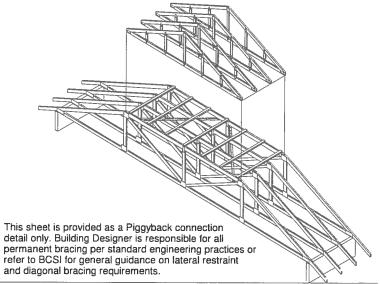


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

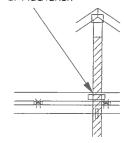
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH NAII-ON PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- AS SHOWN IN DETAIL.
 ATTACH 2 x 4'-0" SCAB TO EACH FACE OF
 TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
 SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
 VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
- (MINIMUM 2X4)
 THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 $^{\circ}$ O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEERATE OF AREASOURED.



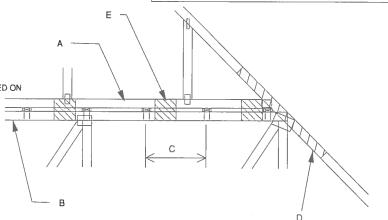
MiTek USA, Inc. ENGINEERED BY

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

- PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0.131" X 3.5") TOE-NAILED.
- BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- 2 X X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
SCONTINIOUS OVER INTERSECTION ALL FAST I, ET IN BOTH IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND

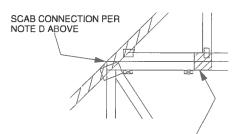
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft. E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH

ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 46" O.C. OR LESS. ATTACH WITH 3 -6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

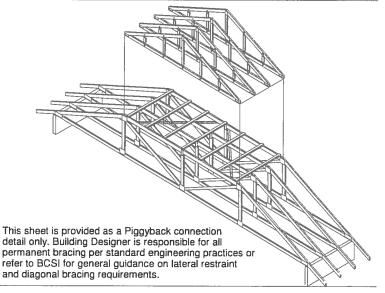


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

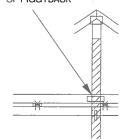
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TÒTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

2) ATTACH 2 × _ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS.

NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.

No 39380

STATE OF ST Thomas A. Albani PE No.39380 MiTek USA, Inc. Fl. Cert 6634 6904 Parke East Blvd. Tampa FL 33610 January 19, 2018

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

(R)

MiTek USA, Inc.

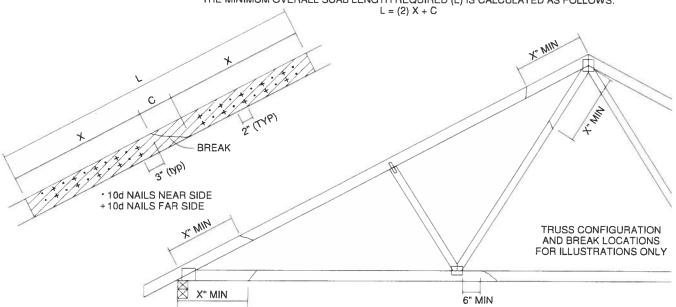
ENGINEERED BY A MiTek Affiliate

| | | | | | | | | MiTek US | SA, Inc. | Page 1 c |
|-----|--------------------|-------------|------|-------|----------|-----------|---------|----------|----------|----------|
| | UMBER OF | | | MAX | (ІМИМ FQ | RCE (lbs) | 15% LOA | D DURAT | ION | |
| | ACH SIDE REAK * | X INCHES | 5 | SP DF | | s | PF | | HF | |
| 2x4 | 2x6 | HOHES | 2x4 | 2x6 | 2x4 | 2x6 | 2x4 | 2x6 | 2x4 | 2x6 |
| 20 | 30 | 24" | 1706 | 2559 | 1561 | 2342 | 1320 | 1980 | 1352 | 2028 |
| 26 | 39 | | 2101 | 3601 | =2007 | 301 F | 72.3.7 | 2 2J46 | 1733 | 772050 |
| 32 | 48 | 36" | 2681 | 4022 | 2454 | 3681 | 2074 | 3111 | 2125 | 3187 |
| 38 | 57 | 42" | 3169 | 4754 | 2900 | 4350 | 2451 | 3677 | 2511 | 3767 |
| 44 | 66 | 48" | 3657 | 5485 | 3346 | 5019 | 2829 | 4243 | 2898 | 4347 |

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

- 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED. THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

- AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x ORIENTATION ONLY.

 THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

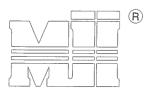
January 19, 2018

LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

NOTES: 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

SIDE VIEW

3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THE DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

NEAR SIDE NEAR SIDE

SIDE VIEW (2x3) 2 NAILS

| - | OE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) | | | | | | | | | |
|------|--|-------|------|------|------|-------|--|--|--|--|
| | DIAM. | SP | DF | HF | SPF | SPF-S | | | | |
| ā | .131 | 88.0 | 80.6 | 69.9 | 68.4 | 59.7 | | | | |
| LONG | .135 | 93.5 | 85.6 | 74.2 | 72.6 | 63.4 | | | | |
| .5" | .162 | 108.8 | 99.6 | 86.4 | 84.5 | 73.8 | | | | |
| 9 | | | | | | _ | | | | |
| LONG | .128 | 74.2 | 67.9 | 58.9 | 57.6 | 50.3 | | | | |
| | .131 | 75.9 | 69.5 | 60.3 | 59.0 | 51.1 | | | | |
| 25" | .148 | 81.4 | 74.5 | 64.6 | 63.2 | 52.5 | | | | |
| ю. | | | | | | | | | | |

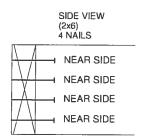
VALUES SHOWN ARE CAPACITY PER TOE-NAIL APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

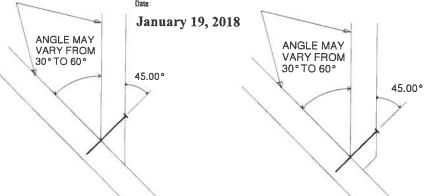
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

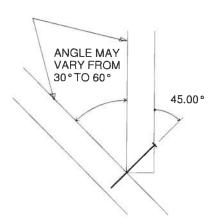
For load duration increase of 1.15:

(2x4)3 NÁILS NEAR SIDE 3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity **NEAR SIDE NEAR SIDE**







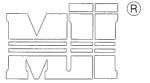


TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

MiTek USA, Inc.

Page 1 of 1

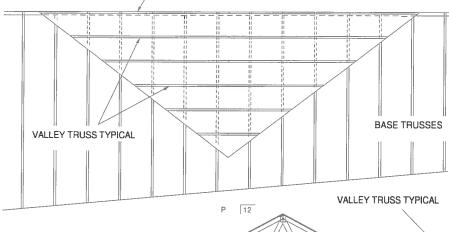


MiTek USA, Inc.



GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DE AMIN
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

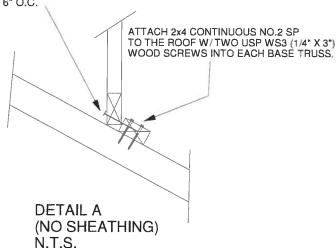


SEE DETAIL A BELOW (TYP.)

GABLE END, COMMON TRUSS OR GIRDER TROSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



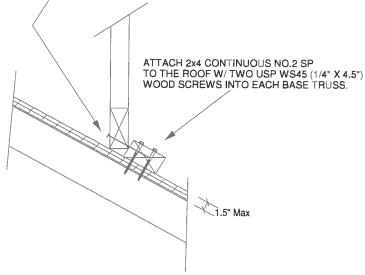
WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



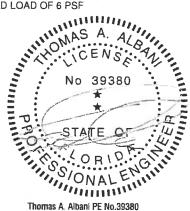
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

TRUSSED VALLEY SET DETAIL AUGUST 1, 2016 MII-VALLEY HIGH WIND2 R MiTek USA, Inc. Page 1 of 1 **GENERAL SPECIFICATIONS** 1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND MiTek USA, Inc. GABLE END, COMMON TRUSS OR GIRDER TRUSS ENGINEERED BY SECURE TO BASE TRUSSES AS PER DETAIL A 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS. LOTE AL LUMBERT 6. NAILING DONE PER NDS-01 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. BASE TRUSSES VALLEY TRUSS TYPICAL GABLE END, COMMON TRUSS OR GIRDER TRUSS VALLEY TRUSS TYPICAL P 12 SEE DETAIL A BELOW (TYP.) SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C. WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C ATTACH 2x4 CONTINUOUS NO.2 SP WIND DURATION OF LOAD INCREASE: 1.60 TO THE ROOF W/ TWO USP WS45 (1/4" X 4.5")



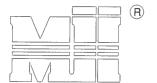
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



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MiTek USA, Inc.

Page 1 of 1

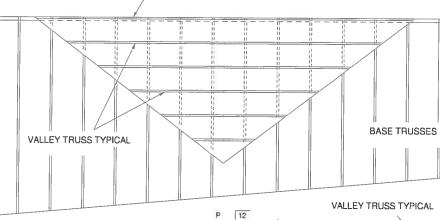


MiTek USA, Inc.

ENGINEERED BY 設以

GENERAL SPECIFICATIONS

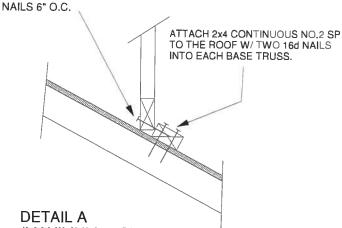
- 1. NAIL SIZE 16d (0.131" X 3.5")
 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING BASE THUSS SHALL BE DESIGNED WITH A FUNDLING A COLLING START OF THE VALLEY TRUSS SPACING OR GIPPER TPUSS 5. NAILE G DONE PEH NDS - 01
 - 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
 - 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS

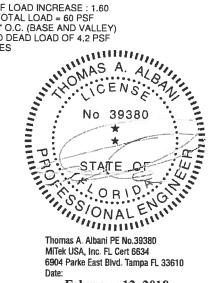
GABLE END, COMMON TRUSS OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d



DETAIL A (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT \pm 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE : 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES



TRUSSED VALLEY SET DETAIL MII-VALLEY AUGUST 1, 2016 (HIGH WIND VELOCITY) NOTE: VALLEY STUD SPACING NOT R MiTek USA, Inc. Page 1 of 1 TO EXCEED 48" O.C. SPACING MiTek USA, Inc. ENGINEERED BY A MITek Affiliate FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING NON-BEVELED EXPOSURE B or C **BOTTOM CHORD** WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF No 39380

STAILE OF THE STAILE MAX SPACING = 24" O.C. (BASE AND VALLEY) SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF NON-BEVELED 2'-10" ON AFFECTED TOP CHORDS. **BOTTOM CHORD** NOTES: - SHEATHING APPLIED AFTER **INSTALLATION OF VALLEY TRUSSES** - THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER. CLIP MUST BE APPLIED Thomas A. Albani PE No.39380 TO THIS FACE WHEN PITCH EXCEEDS 6/12.

(MAXIMUM 12/12 PITCH)

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January 19, 2018

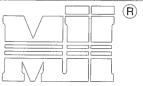


Standard Gable End Detail

MII-GE146-001

MiTek USA, Inc.

Page 1 of 2



MiTek USA, Inc.

ENGINEERED BY

Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 =

CHALE. " LE > 5"- " ;

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stuc

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

Roof Sheathing

- Diagonal Bracing Refer to Section A-A

L-Bracing Refer to Section B-B

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT
BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:
2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

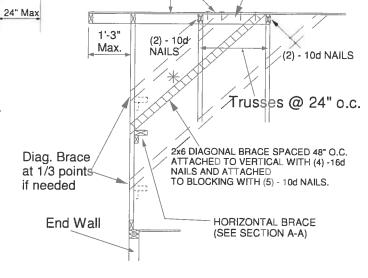
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species | Stud Spacing | Without Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS | |
|---------------------------------|-----------------|---------------------|----------------|-------------------|---------------------------------------|--|
| and Grade | | Maximum Stud Length | | | | |
| 2x4 SP No 3/Stud | 12" O.C. | 3-11-3 | 6-8-0 | 7-2-14 | 11-9-10 | |
| 2x4 SP No 3/Stud | 16" O.C. | 3-6-14 | 5-9-5 | 7-1-13 | 10-8-11 | |
| 2x4 SP No 3/Stud | 24" O.C. | 3-1-8 | 4-8-9 | 6-2-15 | 9-4-7 | |

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





MiTek USA, Inc. Ft. Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

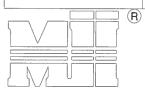
OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY A MITck Affiliate TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6

PITCH: 4/12 - 12/12 HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

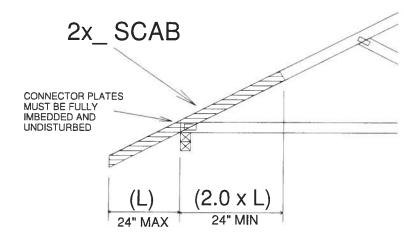
END BEARING CONDITION

NOTES:

1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

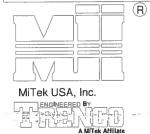
LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

MiTek USA, Inc.

Page 1 of 1

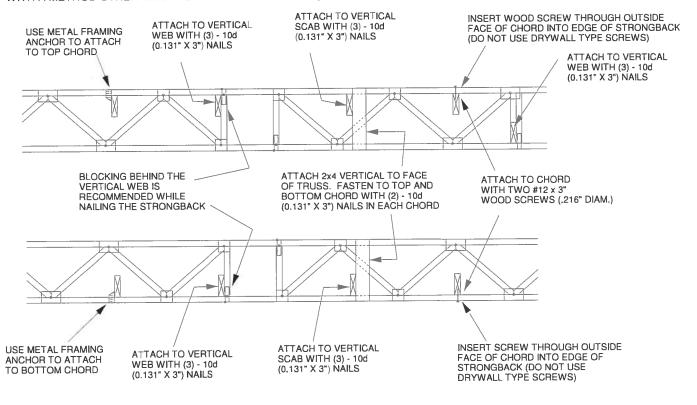
ACCORDING TO SERVICE

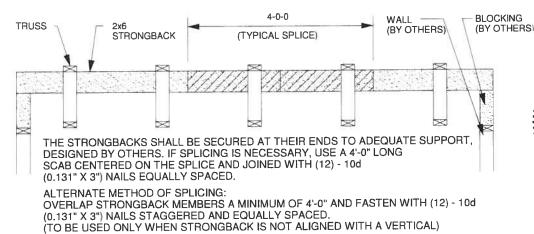


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR-TRUSS. The thing to the same

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





NO 39380

STATE OF

A Albani PE No.3'
Inc. FL Cert 6'
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6904 Parke East Blvd. Tampa FL 33610 Date

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