#### BUILDING PROFILE

Width $(ft) = 50$	Eave Height (ft) = 16	
Length (ft) = 200	Roof Slope (Rise/12) = $1.0:12$	

#### BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 20 / 7TH EDITION

THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY TH BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL FOUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE

OCCUPANCY/RISK CATEGORY	<u>II - Normal Je1.00</u>
WIND LOAD ULTIMATE	119 MPH NOMINAL 92.18 MPH WIND EXPOSURE B
CLOSURE TYPE	Partially Enclosed
INTERNAL WIND COEFFICIENT	-0.55 / 0.55
COLLATERAL DEAD LOAD	1 PSF
ROOF LIVE LOAD	20.00 PSF (REDUCIBLE Yes )
DEAD LOAD	2.000 PSF (FOR ROOF PANELS AND PURLINS)
SEISMIC	200000000 THO ACCUMULATION TO CONTROL OF THE CONTRO
SPECTRAL RESPONSE Ss 0.08	59 St 0.0507 Sds 0.0907 Sdt 0.0800
SITE CLASS d	DESIGN RISK CATEGORY B Cs 0.0302
RESPONSE MODIFICATION FACTOR, R	3.000* FRAMES 3.000* BRACING
BASIC SEISMIC FORCE RESISTING SYS	STEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYS	STEM (ENDWALLS) = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES
BASIC SEISMIC FORCE RESISTING SYS	STEM (FSW) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYS	STEM (BSW) = ORDINARY STEEL CONC. BRACED FRAMES
ANALYSIS PROCEDURE	= EQUIVALENT LATERAL FORCE PROCEDURE
SERVICE	ABILITY CRITERIA SEISMIC RESISTANCE.

	SERVICEABILITY		ISMIC RESISTANCE
	MINIMUM DESIG	N DEFLECTIONS	
Endwall Column	= 120	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 60
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

#### GENERAL NOTES

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.
- B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.
- 1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN"
- 2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL NEWSFESS"
- 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.
- 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"
- C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD—FORMED SECTIONS, CONFORM TO ASTM—A529 OR A572. FLANGES WITH THICKNESS OF ONE INCH OR LESS AND WIDTH OF 12" OR LESS CONFORM TO A529 WITH A MINIMUM YIELD POINT OF 55,000 psi, FLANGES GREATER THAN 1" IN THICKNESS OR 12" IN WIDTH CONFORM TO A572 WITH A MINIMUM YIELD
- POINT OF 50,000 psi. WEB MATERIAL CONFORMS TO ASTM-A529 WITH A MINIMUM YIELD POINT OF 55,000 psi. 2) MATERIAL PROPERTIES OF PIPE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD POINT OF 42,000 psi
- 3) MATERIAL PROPERTIES OF TUBE SECTIONS CONFORM TO ASTM-ASOO, GRADE B WITH A MINIMUM YIELD
- POINT OF 46 000 psi. 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A529 WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT CAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
- 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT
- 8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE
- "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A-325 BOLTS IF:
- a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC
- DESIGN CATEGORY" OF "D", "E" OR "F".

  b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS REVERSALS ON THE CONNECTIONS.
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 SC".

- 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.
- 11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36.
- ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.

  D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.

  E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS
- STIPULATED OTHERWISE IN THE CONTRACT.

#### APPROVAL NOTES

- THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:
- A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
- 1) BE MADE IN CONTRASTING INK.
- 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL DISAPPROVAL REJECTION OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER

#### SAFETY COMMITMENT

- THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.
- (C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.
- MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING, EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.
- E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE. ARE RECOMMENDED

#### ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS.
- (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, 13TH ED.)

  D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 13TH ED.)

  E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING
- MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT-URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

  F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH
- THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

  G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR.
- IMMEDIATELY UPON ARRIVAL CLAMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
- IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
- THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.
- H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.
- I) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, 13TH ED.)
- J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION, IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT. BEARING VALUES. TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

COMPONENTS & CLADDING (unfactored) Wall Field Values = 25.729 psf / -27.326 psf Wall Edge Values = 25.729 psf / -32.116 psf



## **BUILDINGS AND MORE**

#### FLORIDA PRODUCT APPROVAL NUMBER PRR ROOF PANEL

PBR WALL PANEL

36876.1

IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING

#### FRAMING COLORS

Rigid Frame:	RO	Red Oxide
Tange brace:	RO	Grey Primer Galvanized
ingle:	RO	

A	ngle:	RC	)					
			_				End	
		Grt	Pur	EvSt	Jmb	88	Col	Raf
V	SECTION:	RO	RO	RO	RO	RO	RO	RO
С	SECTION:	RO	RO	RO .	RO	RO	RO .	RO .
D	SECTION:	RO	RO	RO	RO	RO	RO	RO
Z	SECTION:	RO	RO	RO	RO	RO	RO	RO
E	SECTION:	RO	RO	RO	RÓ	RO	RO	RO
R	SECTION:	RO	RO	RO	RO	RO	RO	RO
W	SECTION:	RO	RO	RO	RO	RO	RO	RO

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.



BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY.

COLORS:

ROOF:

CUTTER

DOWNSPOUTS:

		16 15					
REV.	PAGE	DESCRIPTION					
	0	COVER PAGE	1				
	1	ANCHOR BOLT LAYOUT	1				
	1.1	ANCHOR BOLT DETAILS	٦٠				
	1:2	ANCHOR BOLT REACTIONS					
	2	ROOF FRAMING LAYOUT	7				
2.1-2.4 3 4		RIGID FRAME CROSS SECTION	1				
		SIDEWALL FRAMING LAYOUT ENDWALL FRAMING LAYOUT					
	6	ROOF PANELS & TRIM	-				
	6.1	ROOF PANEL DETAILS					
	7	SIDEWALL PANELS & TRIM					
	7.1	SIDEWALL PANEL DETAILS					
	8	ENDWALL PANELS & TRIM					
8,1		ENDWALL PANEL DETAILS					
	9	SPECIAL DETAILS					
	10	PARTITION DETAILS					
			_				

DRAWING INDEX

FOR OCCUPANCY (RISK) CAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC

THIS PROJECT IS DESIGNED AS A PARTIALLY ENCLOSED BUILDING AS DEFINED BY THE REFERENCED BUILDING CODE.

1.0 PSF COLL ONLY ALLOW LIGHTING AND HVAC DUCT TO HANG FROM ROOF SYSTEMS SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICITLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS CAN BE PERMITTED BY CODE.

	0	COVER PAGE		힣		
	1	ANCHOR BOLT LAYOUT		LENGT		
	1.1	ANCHOR BOLT DETAILS	٦.,			
	1.2	ANCHOR BOLT REACTIONS	ISSUE	BUILDING		
	2	ROOF FRAMING LAYOUT	-	≝		
1	2-1-2-4	RIGID FRAME CROSS SECTION	1			
	3	SIDEWALL FRAMING LAYOUT		S		
	4	ENDWALL FRAMING LAYOUT		REVISED		
	5-5.4	FRAMING DETAILS		2	11	
	6	ROOF PANELS & TRIM	$\vdash$			
	6.1	ROOF PANEL DETAILS				
	7	SIDEWALL PANELS & TRIM				
	7.1	SIDEWALL PANEL DETAILS				
	8	ENDWALL PANELS & TRIM				
	8.1	ENDWALL PANEL DETAILS				
	9	SPECIAL DETAILS				
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110		II, IBC PROVISIONS INDICATE THAT NO DRIFT LIMIT* PROVIDED THAT	1	JILDINGS	ST	

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CIT,

GALVALUME DRAWING STATUS

WALLS: FOR APPROVAL:
THESE DRAWINGS, BEING FOR APPROVAL, ARE BY
DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL
REPRESENTATION ONLY. THEIR PURPOSE IS TO
CONFIRM PROPER INTERPRETATION OF THE PROJECT
DOCUMENTS. ONLY DRAWINGS ISSUED FOR CONSTRUCTION"
CAN BE CONSIDERED AS COMPLETE. CALVALUME GABLE: CALVALUME EAVE: GALVALUME CORNER GALVALUME

8/23/23 SCALE DAR TITLE

NUMBER

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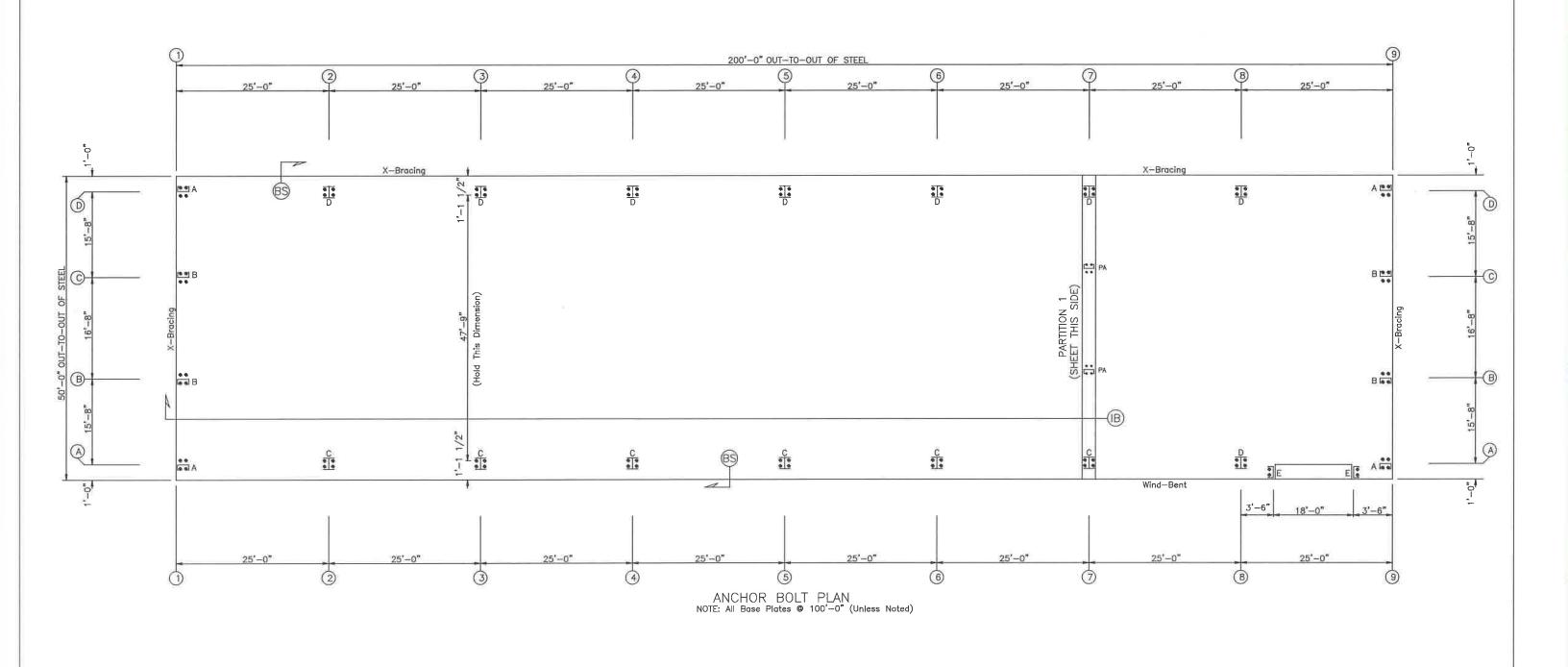
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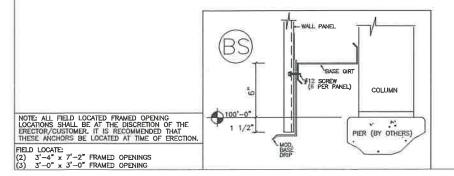
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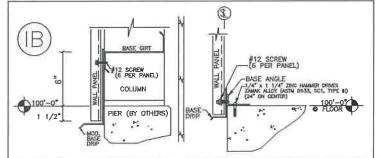
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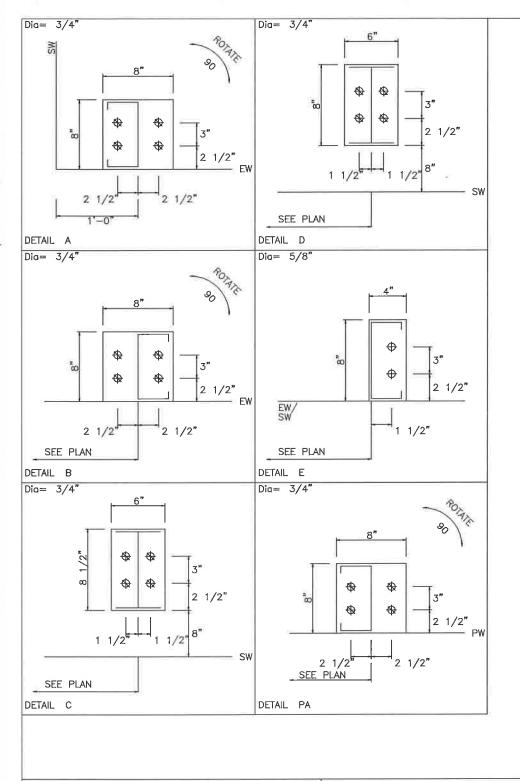
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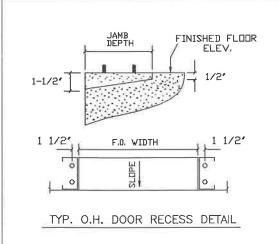


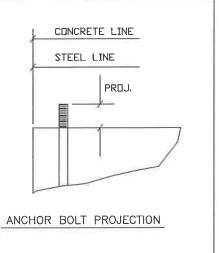




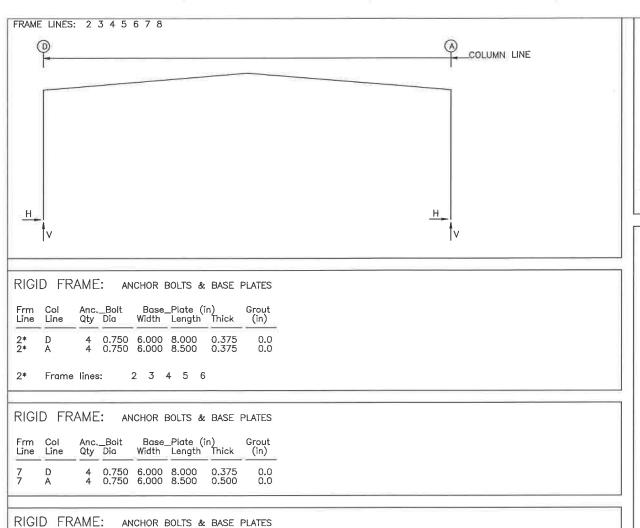
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LAKE CITY, FL	32055			
DRAWING NAME: ANCHOR BOLT [	ETAILS			
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	9	CD

RIGIE	FRAN	ЛЕ: ғ	BASIC COL	UMN REA	CTIONS (	k )							
Frame Line 2* 2* 7 7 8 8	Column Line D A D A D A	Horiz 0.9 -0.9 0.9 -0.9 0.9 -0.9	-Dead Vert 1.8 1.8 1.8 1.8 1.8	Colla Horîz 0.4 -0.4 0.4 -0.4 0.3 -0.3	teral— Vert 0.6 0.6 0.6 0.6 0.6	Horiz 4.2 -4.2 4.2 -4.2 -4.2 4.1 -4.1	Vert 7.5 7.5 7.5 7.5 7.5 7.5	Wind Horiz -6.8 8.7 -8.9 2.1 -8.7	Left1- Vert -15.2 -13.7 -16.6 -12.3 -16.6 -12.3	-Wind_ Horiz -1.7 10.1 -2.1 8.9 -1.9 8.7	Right1- Vert -12.0 -16.9 -12.2 -16.7 -12.3 -16.6	Wind Horiz -7.0 -2.5 -6.4 -0.4 -6.4 -0.4	_Left2 Vert -2.4 2.9 -1.9 2.5 -1.9 2.4
Frame Line 2* 2* 7 7 8	Column Line D A D A D A	-Wind_ Horiz -2.1 -1.0 0.4 6.4 0.4 6.4	Right2 Vert 0.8 -0.3 2.5 -1.9 2.4 -1.9	Wind. Horiz -1.5 10.4 -3.6 4.0 -3.4 3.8	Long1- Vert -14.0 -14.9 -15.4 -13.5 -15.4 -13.5	Wind Horiz -1.9 10.1 -4.0 3.6 -3.8 3.4	Long2- Vert -12.0 -16.9 -13.5 -15.5 -13.5 -15.4	-Seismi Horiz -0.1 -0.1 -0.1 -0.1 -0.1	ic_Left Vert -0.1 0.1 -0.1 -0.1 0.1	Seismic. Horiz 0.1 0.1 0.1 0.1 0.1 0.1	_Right Vert 0.1 -0.1 0.1 -0.1 -0.1 -0.1 -0.1		
2*	Frame lir	es:	2 3 4	5 6						_			

END	WAL	L COI	_UMN:	BASIC		REACTIONS (							
Frm Line 1 1 1	Col Line D C B A	Dead Vert 0.3 0.7 0.7 0.3	Collat Vert 0.1 0.2 0.2 0.1	Live Vert 1.7 4.5 4.5	Wind Left1 Vert -2.4 -5.9 -4.0 -1.8	Wind Right1 Vert -1.8 -4.0 -5.9 -2.4	Wind Left2 Vert -0.4 -1.6 0.2 0.2	Wind Right2 Vert 0.2 0.2 -1.6 -0.4	Wind Press Horz -1.4 -2.9 -2.9	Wind Suct Horz 1.6 3.1 3.1	Wind Long1 Vert -2.5 -5.7 -4.0 -1.8	Wind Long2 Vert -1.8 -4.0 -5.7 -2.5	Seis Left Vert 0.0 0.0 0.0
Frm Line 1 1 1	Col Line D C B A	Seis Right Vert 0.0 0.0 0.0	Seis Long Vert 0.0 0.0 0.0	E1PAT_ Horz 0.0 0.0 0.0 0.0		E1PAT_LL_2 Horz Vert 0.0 -0.: 0.0 1.9 0.0 4.9	Horz 2 0.0 9 0.0 9 0.0	Vert 2.0 2.2 2.2 2.0	0.0 - 0.0 0.0	L_4- /ert -0.2 2.3 2.3 -0.2			
Frm Line 9 9 9	Col Line A B C D	Dead Vert 0.3 0.7 0.7 0.3	Collat Vert 0.1 0.2 0.2 0.1	Live Vert 1.7 4.5 4.5	Wind Left1 Vert -2.4 -5.9 -4.0 -1.8	Wind Right1 Vert -1.8 -4.0 -5.9 -2.4	Wind Left2 Vert -0.4 -1.6 0.2 0.2	Wind Right2 Vert 0.2 0.2 -1.6 -0.4	Wind Press Horz -1.4 -2.9 -2.9	Wind Suct Horz 1.6 3.1 3.1	Wind Long1 Vert -2.5 -5.7 -4.0 -1.8	Wind Long2 Vert -1.8 -4.0 -5.7 -2.5	Seis Left Vert 0.0 0.0 0.0
Frm Line 9 9 9	Col Line A B C D	Seis Right Vert 0.0 0.0 0.0	Seis Long Vert 0.0 0.0 0.0	E2PAT_ Horz 0.0 0.0 0.0 0.0		E2PAT_LL_2 Horz Vert 0.0 -0 0.0 1.5 0.0 4.5 0.0 1.7	Horz 2 0.0 0 0.0 0 0.0	Vert 2.0 2.2 2.2 2.2 2.0	0.0 0.0 0.0	L_4- /ert -0.2 2.3 2.3 -0.2			

ANCHOR BOLT SUMMARY Qty Locate (in) Type (in) 5/8" A307 3/4" GR36 3/4" GR36 3/4" GR36

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

#### GENERAL NOTES

- 1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- 4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

				_
	DINC	DDACING	DEVCTIONS	

- 1			O DI							
	Wo	all —-	Col			ions(k —Sei			_Shea /ft)	г
	Loc	Line	Line	Horz	Vert	Horz	Vert	Wind	Séis	Note
- 1	L_EW F_SW R_EW	1 A 9	C,B 7,8 B,C	1.8 1.7 1.8	1.8 2.0 1.8	0.1 0.4 0.1	0.1 0.5 0.1			(b)
	B_SW	D	8,7 3,2	1.7	1.0 1.0	0.4 0.4	0.3 0.3			

(b)Wind bent in bay, base above finish floor

Reactions for seismic represent shear force, Eh

NOTES FOR REACTION	NS
Building reactions are based the following building data:  Width (ft) Length (ft) Eave Height (ft) Roof Slope (Rise/12) Dead Load (psf) Collateral Load (psf) Roof Live Load(psf) Frame Live Load(psf) Wind Speed (mph) Wind Code Exposure Closure Importance Wind Importance Seismic Seismic Coeff (Fa*Ss)	= 50.0 = 200.0 = 16.0/16.0 = 1.0/1.0 = 2.0 = 1.0 = 12.0 = 119.0 = FBC 20 (7TH EDITION) = PARTIALLY ENCLOSED = 1.00 = 1.00 = B

Frm Col Anc.\_Bolt Base\_Plate (in) Grout Line Line Qty Dia Width Length Thick (in)

Anc. Bolt Qty Dia

Frm Col

D

4 0.750 6.000 8.000 0.375 0.0 4 0.750 6.000 8.000 0.500 0.0

4 0.750 8.000 8.000 0.250 0.0

4 0.750 8.000 8.000 0.250

4 0.750 8.000 8.000 0.250

4 0.750 8.000 8.000 0.250

4 0.750 8.000 8.000 0.250

4 0.750 8.000 8.000 0.250

4 0.750 8.000 8.000 0.250

4 0.750 8.000 8.000 0.250

Base\_Plate (in) Grout Width Length Thick (in)

0.0

0.0

0.0

0.0

0.0

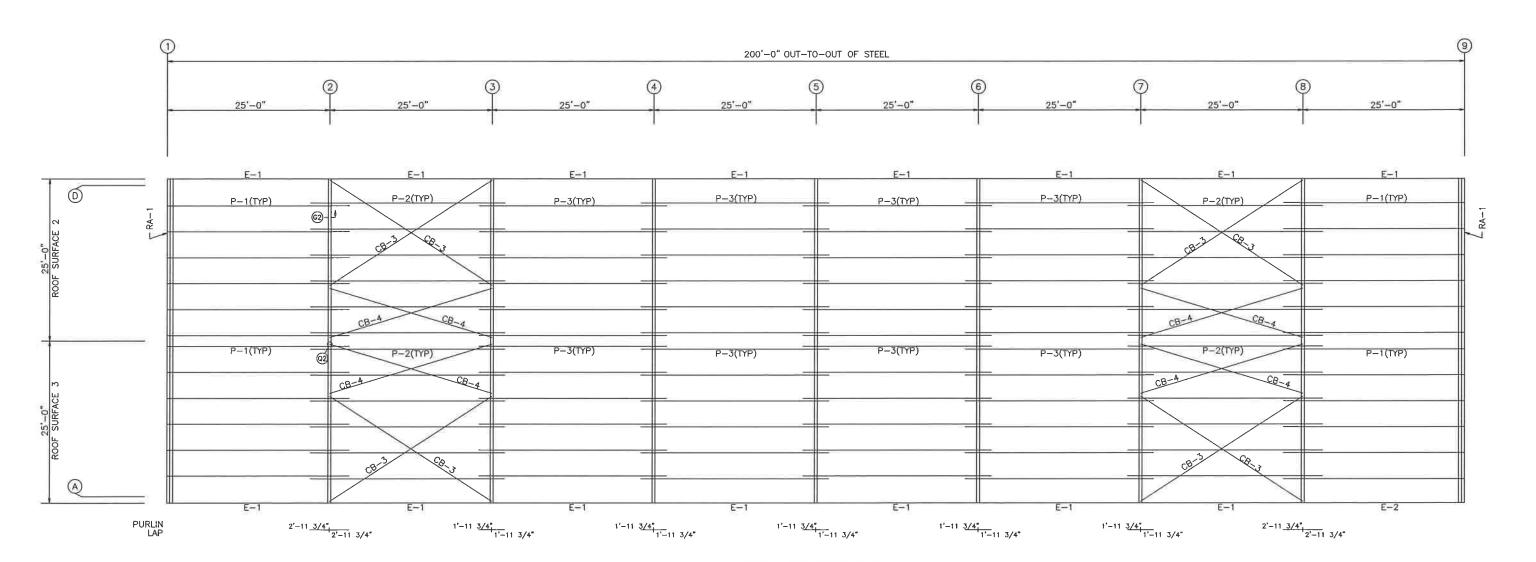
ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

ISSUE			DET	CHK	DATE	
BUILDINGS AND MO	DRE					
F TRAFFIC OPS BL	JILDINGS					
7966/8227			8/23/23			
LAKE CITY, FL 320	)55					
DRAWING NAME: ANCHOR BOLT REA						
PAGE 1.2	DRAWN BY: DAR	SP\	KED BY	SC	NONE	

MAXIMUM REACTION Part Dead Wind Id Orient Vert Horz

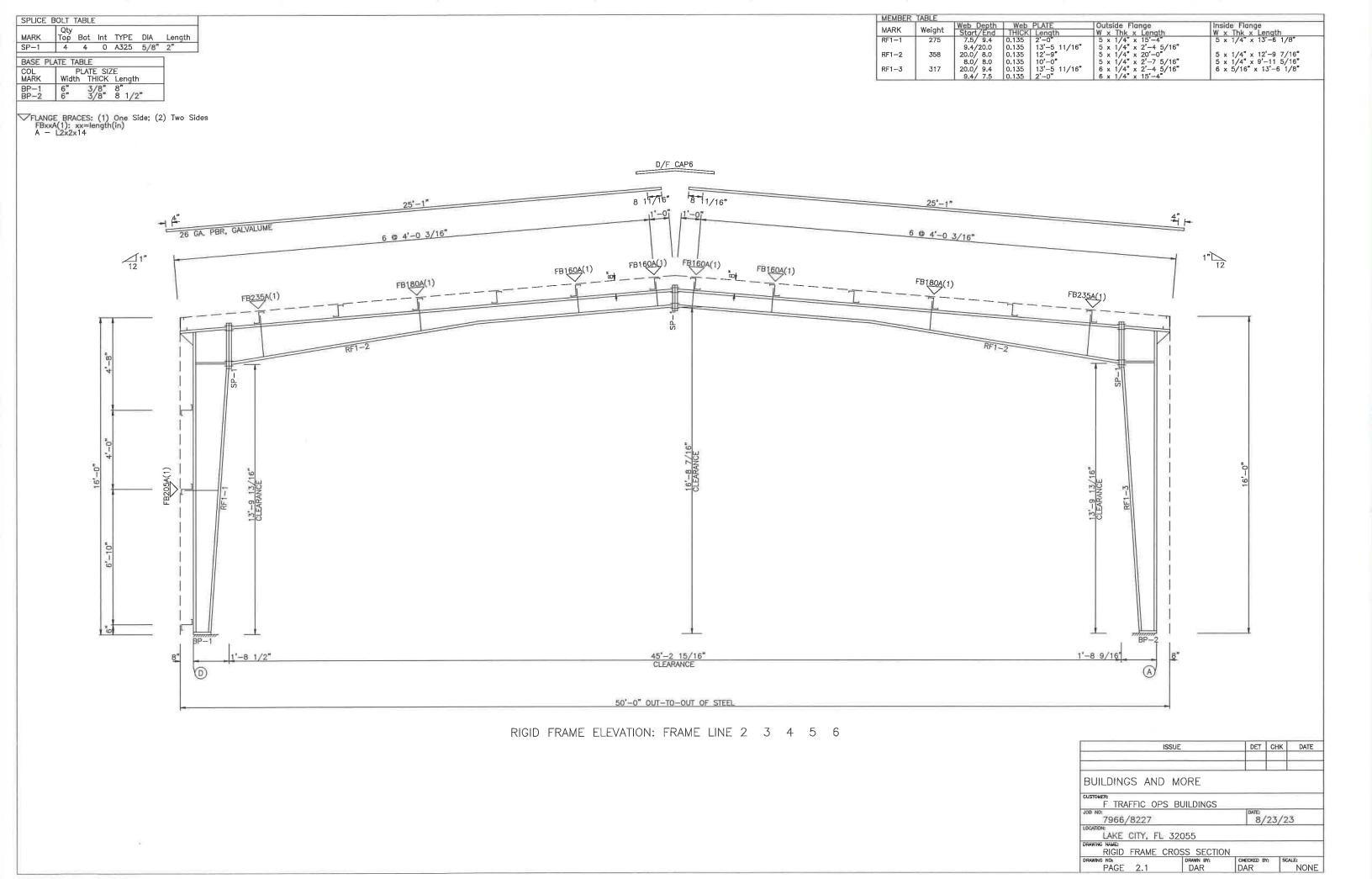
1 Trans. 0.1 1.9

	MEMBER ROOF P	TABLE LAN	
	MARK	PART	LENGTH
i	P-1	8x25Z14	27'-11 1/2"
	P-2	8x25Z16	29'-11 1/2"
	P-3	8x25Z16	28'-11 1/2"
	E-1	8LE14@1	24'-11 1/2"
	E-2	8LE14@1	24'-11 1/2"
	CB-3	1/4 CBL	29'-7"
	CB-4	1/4 CBL	26'-9"



ROOF FRAMING PLAN

	DET	снк	DATE	
BUILDINGS AND	MORE			
CUSTOMER: F TRAFFIC OP:	S BUILDINGS			
7966/8227		DATE:	23/2	3
LAKE CITY, FL	32055			
ROOF FRAMING	AND THE PARTY OF T	1000100000		
DRAWING NO: PAGE 2	DRAWN BY:	SPW	f: SC	NONE



MEMBER TABLE SPLICE BOLT TABLE Outside Flange
W x Thk x Length
5 x 1/4" x 15"-4"
5 x 1/4" x 2"-4 5/16"
5 x 1/4" x 2"-7 5/16"
5 x 1/4" x 2"-7 5/16"
5 x 1/4" x 15"-3 7/8" Inside Flange W x Thk x Length 5 x 1/4" x 13'-6 1/8" MARK Weight MARK | Qty | Top | Bot | Int | TYPE | DIA | Length | SP-1 | 4 | 4 | 0 | A325 | 5/8" | 2" RF2-1 280 5 x 1/4" x 12'-9 7/16" 5 x 1/4" x 9'-11 5/16" 5 x 3/8" x 13'-6" RF2-2 359 BASE PLATE TABLE PLATE SIZE
Width THICK Length
6" 3/8" 8"
6" 1/2" 8 1/2" 313 RF2-3 COL MARK FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14 D/F CAP6 8 11/16" 8-1/16" 41 1-1'-0" 26 GA. PBR, GALVALUME 6 9 4'-0 3/16" 6 @ 4'-0 3/16" 121" FB160A(1) FB160A(1) FB160A(1) FB160A(1) FB180A(1) FB180A(1) FB235A(1) FB235A(1) FB205A(1) 13'-9 13/16" CLEARANCE 13'-9 13/16" CLEARANCE BP-2 1'-8 5/8" 1'-8 1/2" D 50'-0" OUT-TO-OUT OF STEEL RIGID FRAME ELEVATION: FRAME LINE 7 ISSUE DET CHK DATE BUILDINGS AND MORE F TRAFFIC OPS BUILDINGS 8/23/23 7966/8227

LAKE CITY, FL 32055

RIGID FRAME CROSS SECTION

AWING NO: PAGE 2.2 CHECKED BY: SCALE: NONE

MEMBER TABLE SPLICE BOLT TABLE Outside Flange

W x Thk x Length

5 x 1/4\* x 15\*-4\*

5 x 1/4\* x 20\*-0\*

5 x 1/4\* x 20\*-0\*

5 x 1/4\* x 20\*-2 5/16\*

5 x 1/4\* x 2-9 3/16\*

5 x 1/4\* x 2-2 5/16\*

5 x 1/4\* x 15\*-3 7/8\* Web PLATE
THICK Length
0.135 13'-7 1/2"
0.188 1'-10"
0.135 12'-10 11/16"
0.135 10'-0"
0.188 1'-10"
0.135 13'-7 3/8" Inside Flange W x Thk x Length 5 x 1/4" x 13'-7 7/8" Web Depth Start/End 7.5/18.0 18.0/18.0 18.0/ 8.0 8.0/ 8.0 18.0/18.0 18.0/ 7.5 MARK Weight 
 MARK
 Qty Top
 Bot
 Int
 TYPE
 DIA
 Length

 SP-1
 4
 4
 0
 A325
 5/8"
 2"
 RF3-1 271 5 x 1/4" x 12'-11" 5 x 1/4" x 9'-11 5/16" 5 x 1/4" x 13'-7 3/4" RF3-2 356 272 RF3-3 FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in)
A - L2x2x14 D/F CAP6 8 11/16" 8 1/16" → A"

26 GA. PBR, GALVALUME 6 @ 4'-0 3/16" 6 @ 4'-0 3/16" 121" FB160A(1) FB160A(1) FB160A(1) FB160A(1) FB175A(1) FB175A(1) FB220A(1) FB220A(1) 13'-11 11/16" CLEARANCE 13'-11 11/16' CLEARANCE BP-2 45'-7" CLEARANCE 1'-6 1/2" 1'-6 1/2" 0 50'-0" OUT-TO-OUT OF STEEL RIGID FRAME ELEVATION: FRAME LINE 8 DET CHK DATE ISSUE BUILDINGS AND MORE F TRAFFIC OPS BUILDINGS 8/23/23 7966/8227

LAKE CITY, FL 32055

RIGID FRAME CROSS SECTION
G NO:
PAGE 2.3 DRAWN BY:
DAR

DAR SCALE:

SPLICE BOLTS

Splice Quan -----Bolt----Mark Top/Bot Type Dia Length

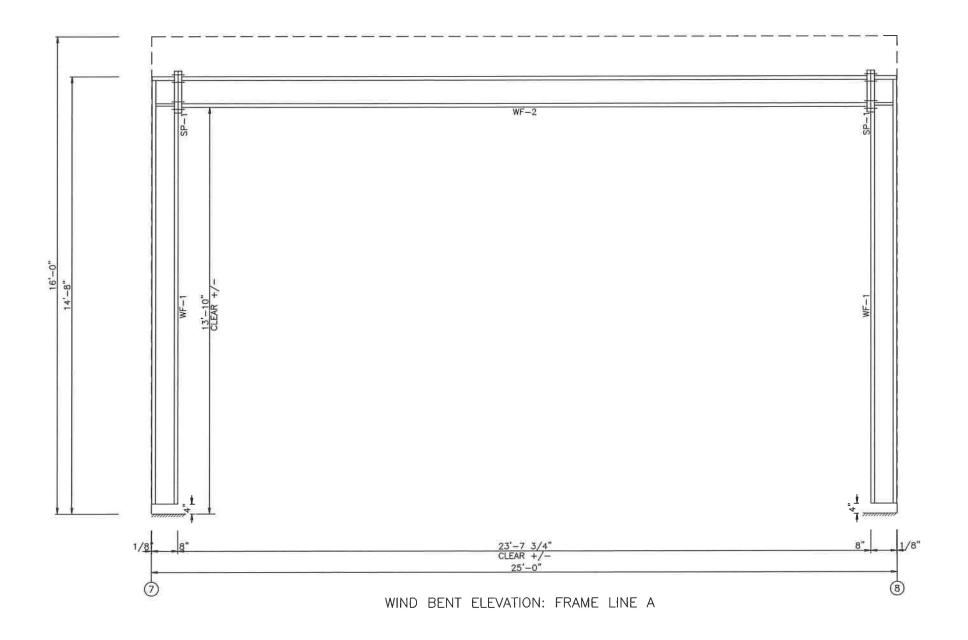
SP- 1 4 4 A325 5/8" 2"

 MEMBER
 SIZE
 TABLE

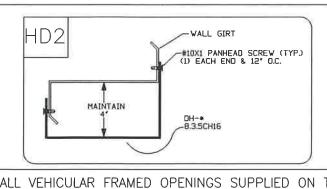
 MARK
 MEMBER
 LENGTH

 WF-2
 B10651
 23'-7, 1/2"

 WF-1
 B08541
 14'-4"



ISS	UILDINGS AND MORE			DATE
BUILDINGS AND	MORE			
CUSTOMER: F TRAFFIC OPS	BUILDINGS			
лов но: 7966/8227		DATE:	23/2	3
LAKE CITY, FL 3	32055			
RIGID FRAME CR		N		
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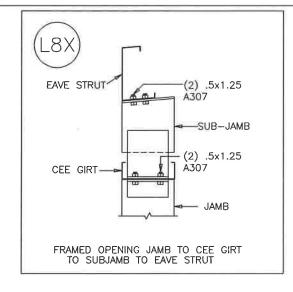


EC-6

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

RF3-1

2'-11 3/4" 2'-11 3/4"



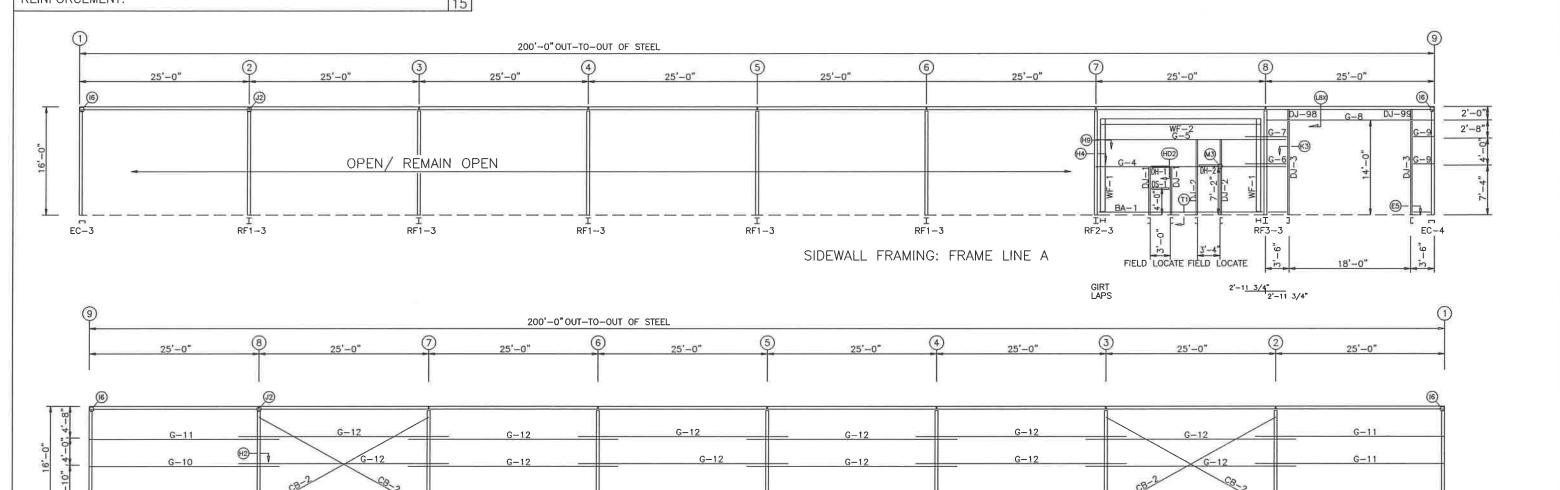
BOLT TABLE FRAME LINE A & D				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	5/8"	2"
WF-1 RF2-3	8	A325	5/8"	2"
WF-1 - RF3-3	8	A325	5/8"	2"

RF1-1

2'-11 3/4" 2'-11 3/4"

FRAME	LINE A & D	
MARK	PART	LENGTH
WF-1	B08541	14'-4"
WF-2	B10651	23'-7 1/2"
DJ-1	8X25C16	l 7'-4"
DJ-2	8X25C16	11'-4"
$DJ-\bar{3}$	8X35C12	14'-0"
DJ-98	8X35C12	1'-4 1/2"
DJ-99	8X35C12	1'-4 1/2" 1'-4 1/2"
DH-1	8.3.5CH6	3'-0" '/ 2
DH-2	8X25C16	3'-0" 3'-4"
DS-1	8X25C16	3'-0"
G-4	8x25Z16	28'-3 1/2"
G-5	8x25Z10	28'-3 1/2"
G-5 G-6	8x25Z12	
		SS (1) = 30
G-7	8x25Z12	
G-8	8x25C16	24'-11' 1/2"
G-9	8x25Z16	3'-1 1/2"
G-10	8x25Z14	27'-11' 1/2"
G-11	8x25Z16	27'-11 1/2"
G-12	8x25Z16	30'-11 1/2"
CB-2	1/4 CBL	29'-7"
	==	

MEMBER TABLE



G-12

RF1-1

2'-11 3/4" 2'-11 3/4"

RF1-1

2'-11 3/4"

SIDEWALL FRAMING: FRAME LINE D

RF1-1 2'-1<u>1 3/4"</u> 2'-11 3/4"

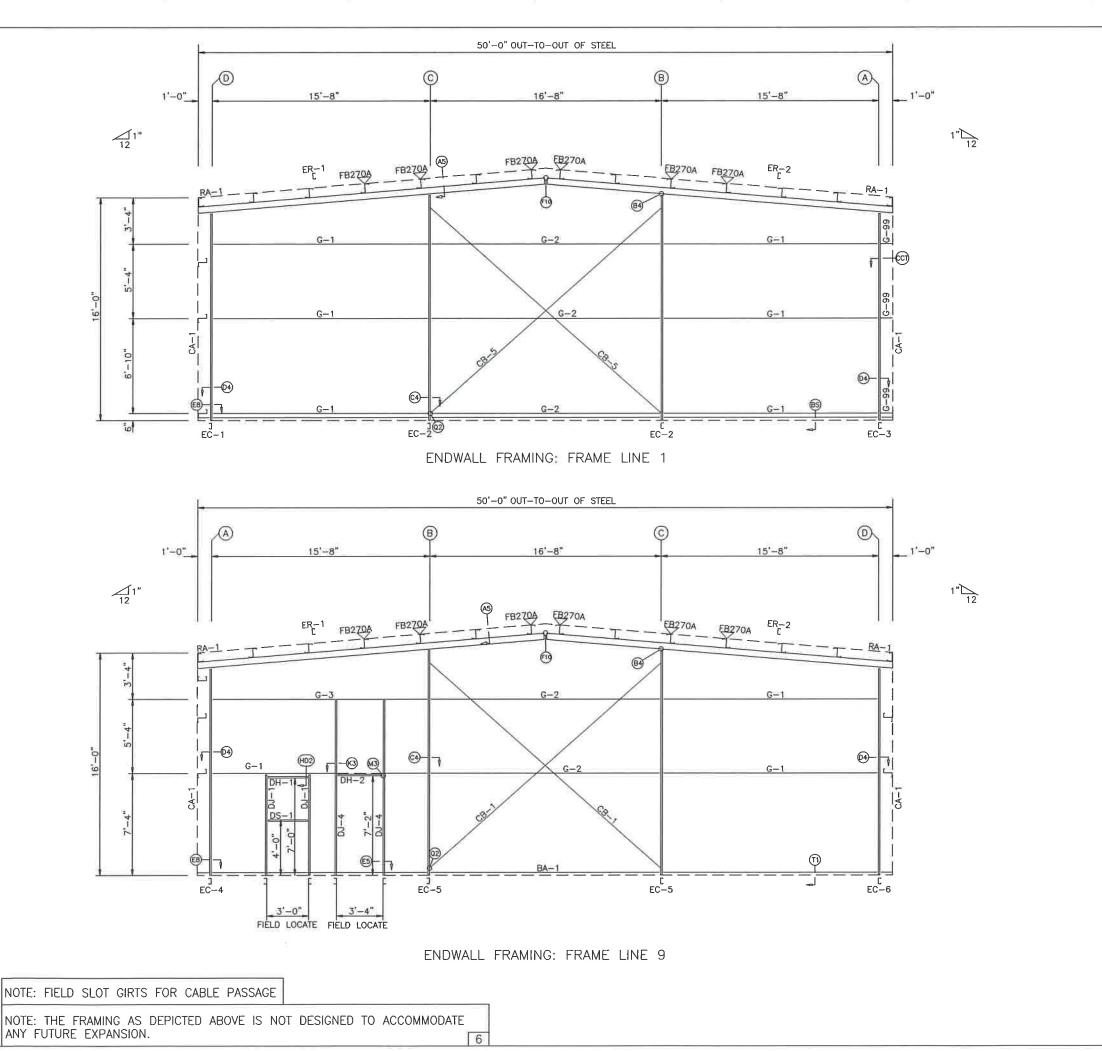
ISSUE	DET	снк	DATE	
BUILDINGS AND MO	ORE			
F TRAFFIC OPS BU	JILDINGS			
7966/8227	DATE:	8/23/23		
OCATION:			7	
LAKE CITY, FL 320	055			
RAWING NAME: SIDEWALL FRAMING	LAYOUT			
PAGE 3	DRAWN BY: DAR	DAR	Y: S0	NONE
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EC-1

G-11

RF1-1

2'-11 3/4" 2'-11 3/4"



BOLT TABLE FRAME LINE 1 & 9 LOCATION QUAN TYPE DIA 8 A325 5/8" 2 A325 5/8" ER-1/ER-2 Columns/Raf MEMBER TABLE FRAME LINE 1 & 9 MARK PART EC-1 8X35C16 LENGTH

14'-6 15/16"

15'-10 5/8"

14'-6 15/16"

15'-10 5/8"

14'-6 15/16"

25'-0 13/16"

25'-0 13/16"

7'-4"

12'-8"

3'-0"

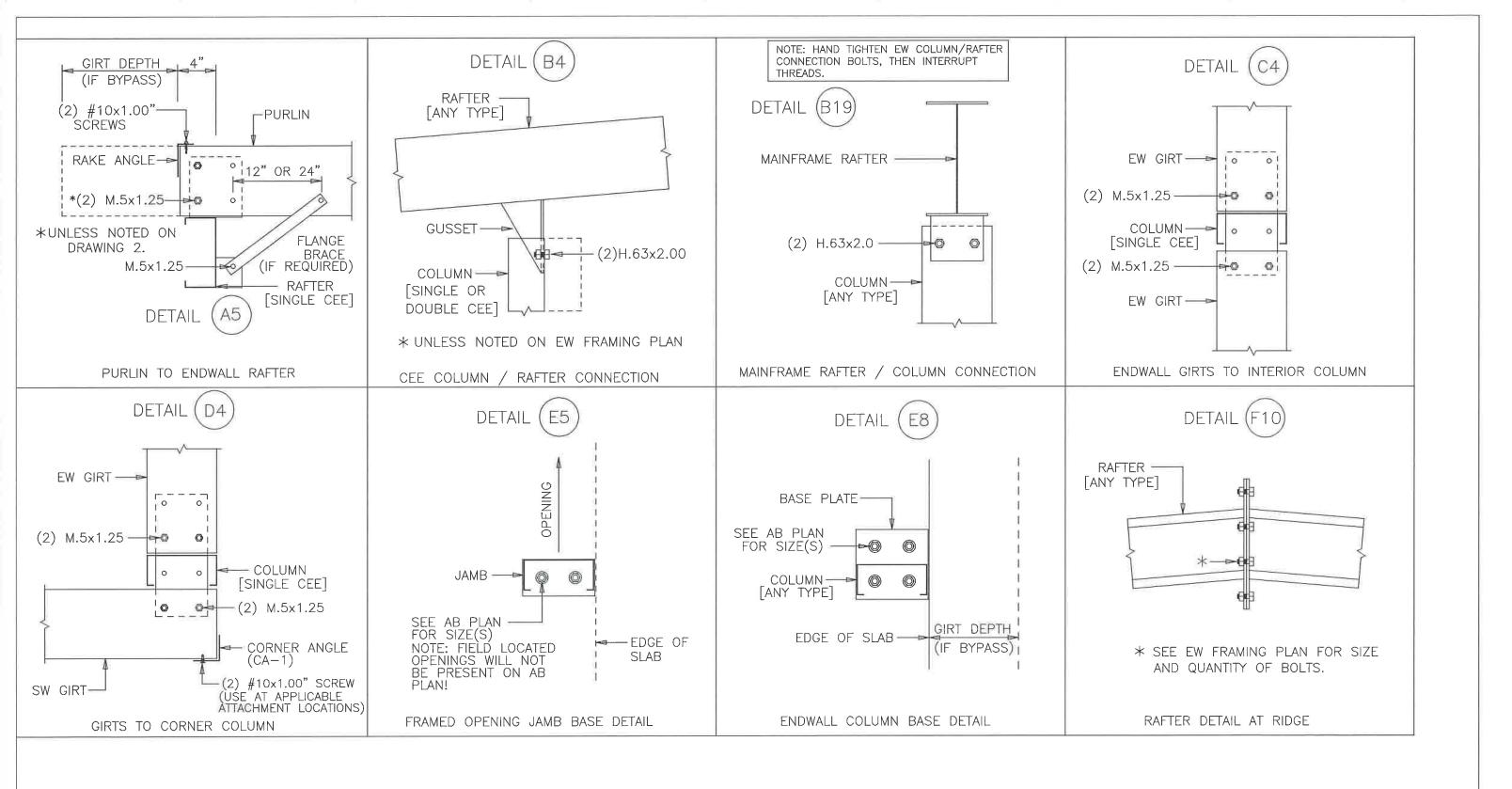
3'-0" EC-1 EC-2 EC-3 8X35C12 8X35C16 8X35C16 8X35C12 EC-4EC-5 8X35C16 8X35C12 EC-6 ER-18X35C12 ER-2 8X25C16 DJ-1DJ-48X25C16 8.3.5CH6 DH-1 8X25C16 DH-23'-0" 15'-3 1/2" 16'-7 1/2" 15'-3 1/2" 7 1/2" 23'-0" 22'-9" 8X25C16 8X25C16 8x25Z16 8x25Z16 8x25Z12 DS-1 G-G-28x25Z16 1/4 CBL 1/4 CBL G-99 CB-1(RIGHT SIDE) PANHEAD SCREW (TYP. U.N.) — #12 SCREW (24" O.C.) 0 3X3 ANGLE DET CHK DATE ISSUE BUILDINGS AND MORE F TRAFFIC OPS BUILDINGS

> 8/23/23 7966/8227

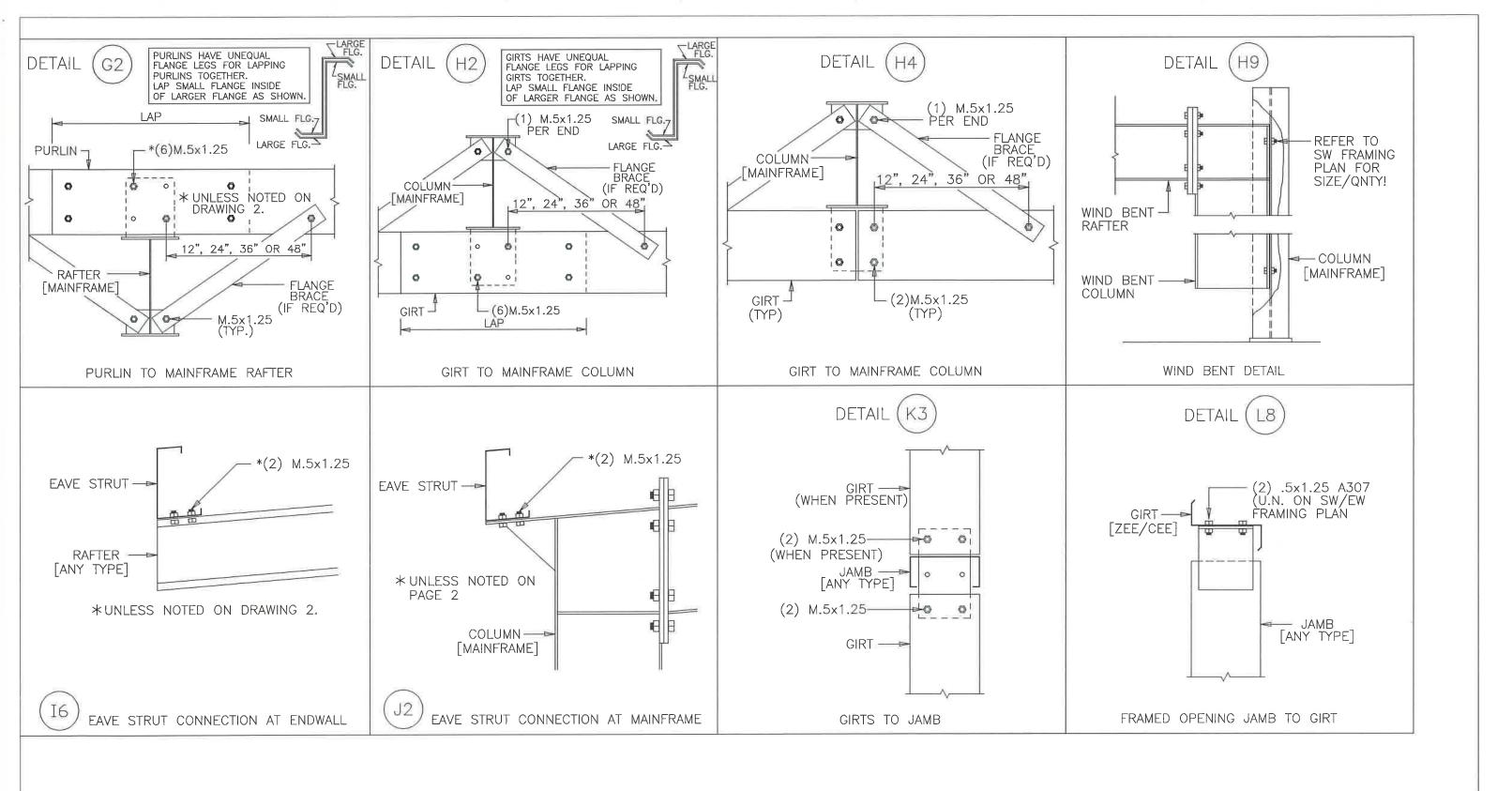
LAKE CITY, FL 32055

ENDWALL FRAMING LAYOUT DAR NONE

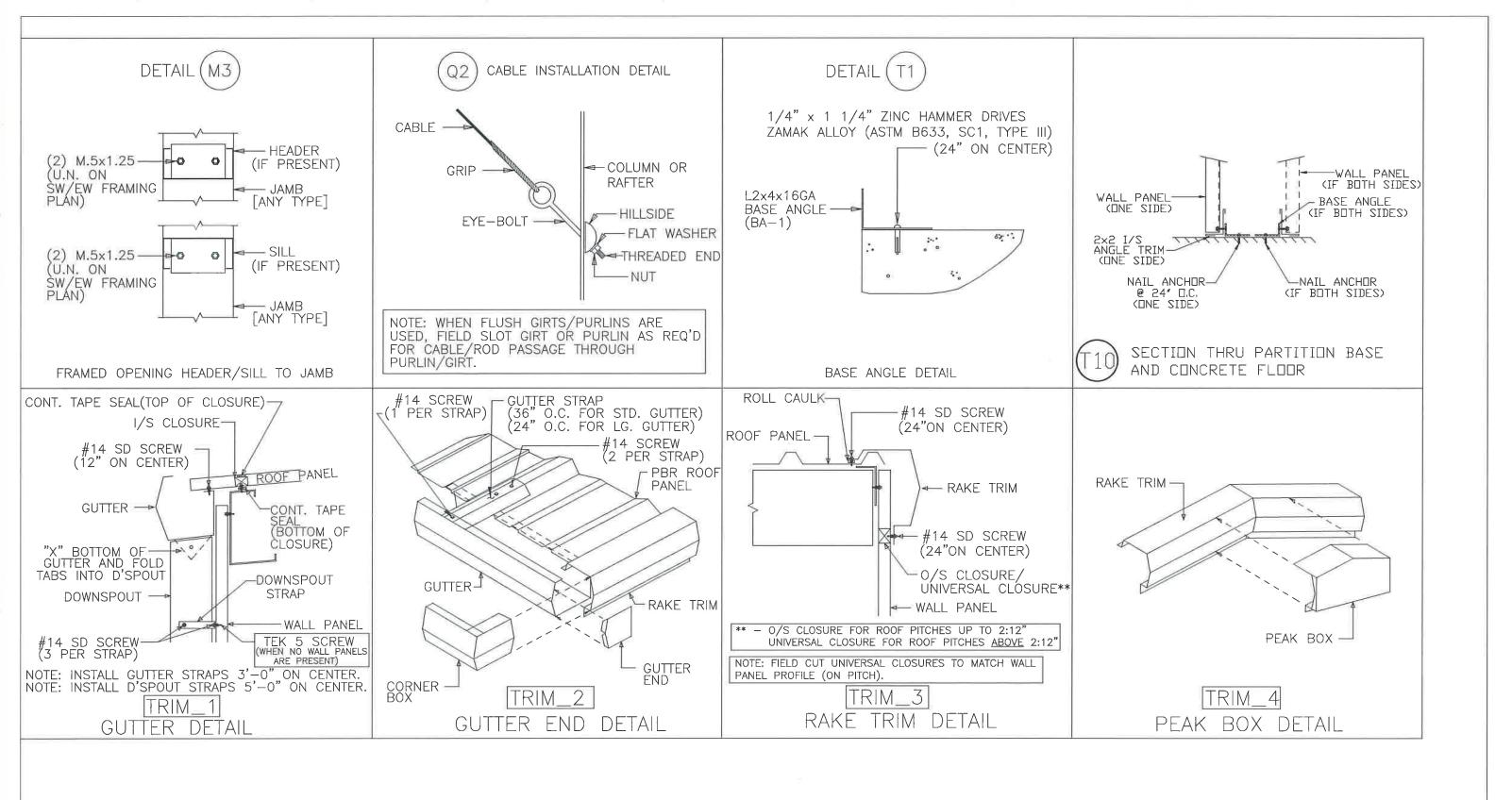
PAGE 4



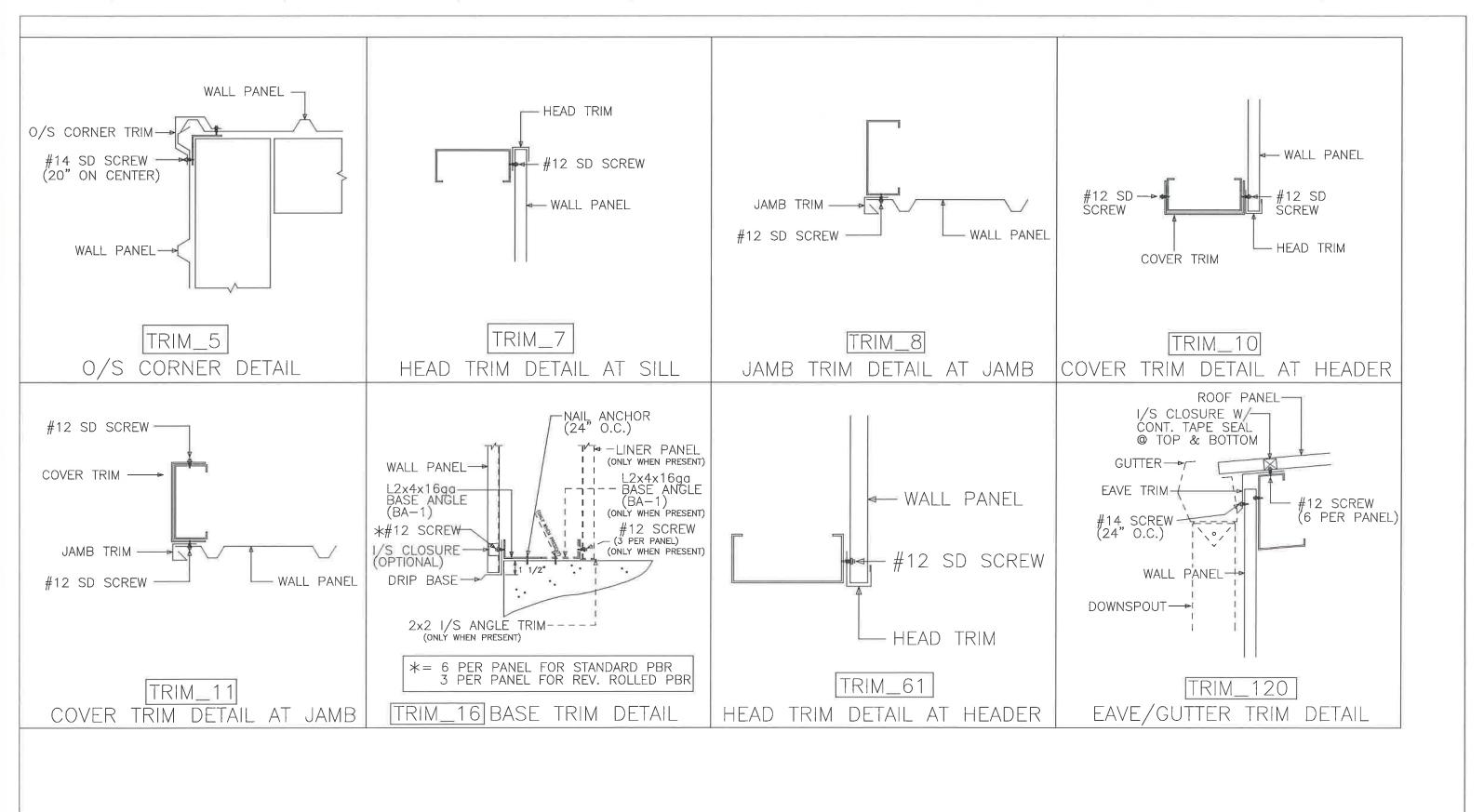
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UILDINGS AND	MORE			
STOMERE				
F TRAFFIC OPS	BUILDINGS			
7966/8227	8/2	8/23/23		
LAKE CITY, FL 3	32055		- 1/4	
FRAMING DETAIL	S	S		
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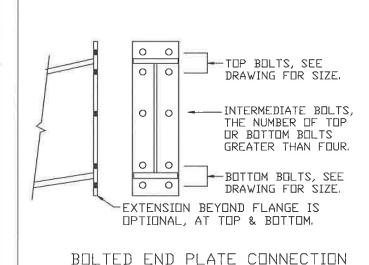
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USTOMER: F TRAFFIC OPS	BUILDINGS	1111			
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LAKE CITY, FL 3	2055	10-2	10		
FRAMING DETAILS					
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JILDINGS			
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	JILDINGS 055	DRE JILDINGS  DATE: 8/	DRE JILDINGS  BATE  8/23/2  DEANN BY: CHECKED BY: SC



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BUILDINGS AND	MORE			
F TRAFFIC OPS	BUILDINGS			
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LAKE CITY, FL 3	32055	11		
AWING NAME: FRAMING DETAIL:	S			
AWING NO: PAGE 5.3	DRAWN BY: DAR	DAR	SCA	NONE



# STRUCTURAL BOLTED CONNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

### TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

# MORTISE PREPPED PERSONNEL DOORS

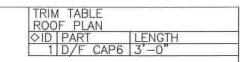
ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

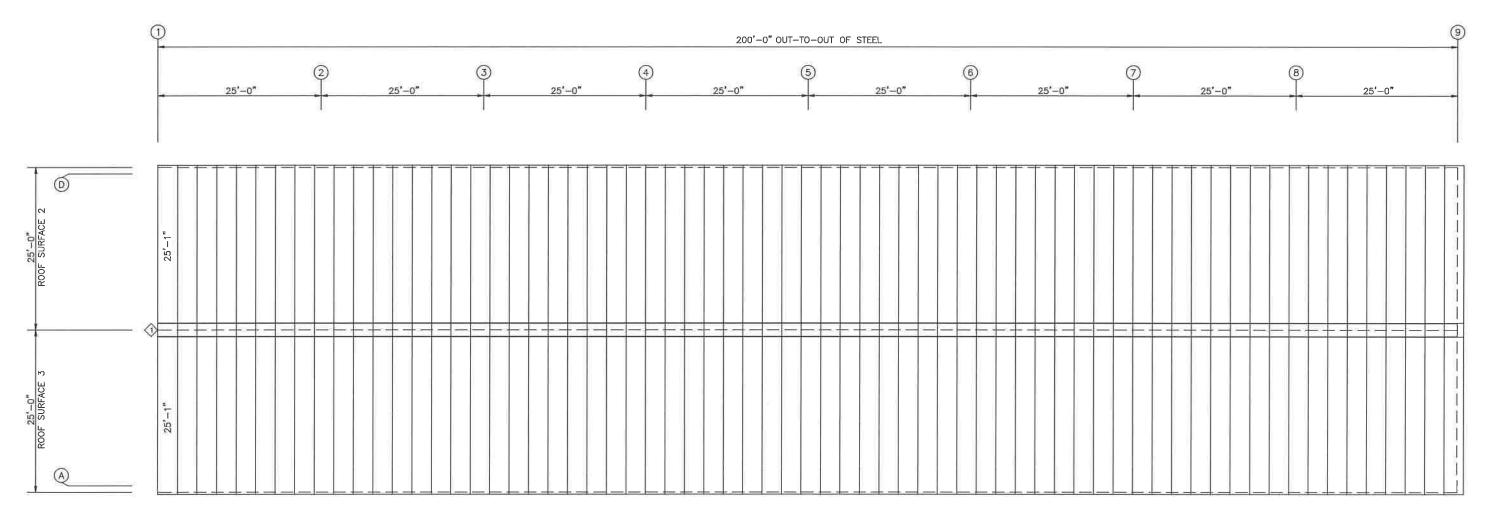
(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING. )

ANY FIELD MODIFICATIONS ARE THE RE— SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

BUILT-UP MEMBER LEGEND					
BEAM	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.	
B	08	5	4	1	
BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.	

IS	ISSUE		СНК	DATE
BUILDINGS AND	MORE			
F TRAFFIC OPS	BUILDINGS			
7966/8227		DATE:	23/2	3
LAKE CITY, FL	32055			
FRAMING DETAIL				
PAGE 5.4	DRAWN BY: DAR	DAR	: 50	NONE

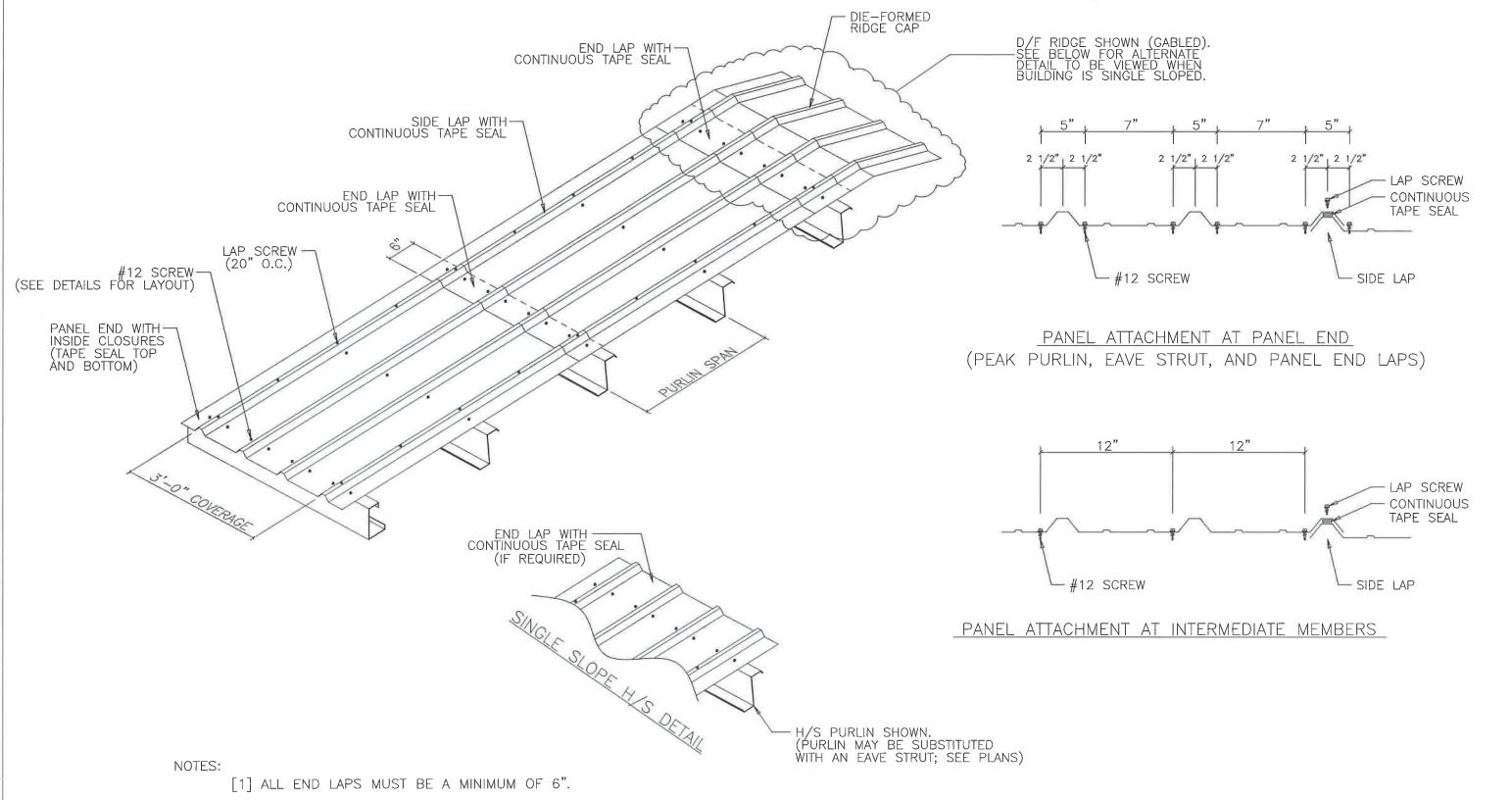




2'-<u>0"</u> Panel Start

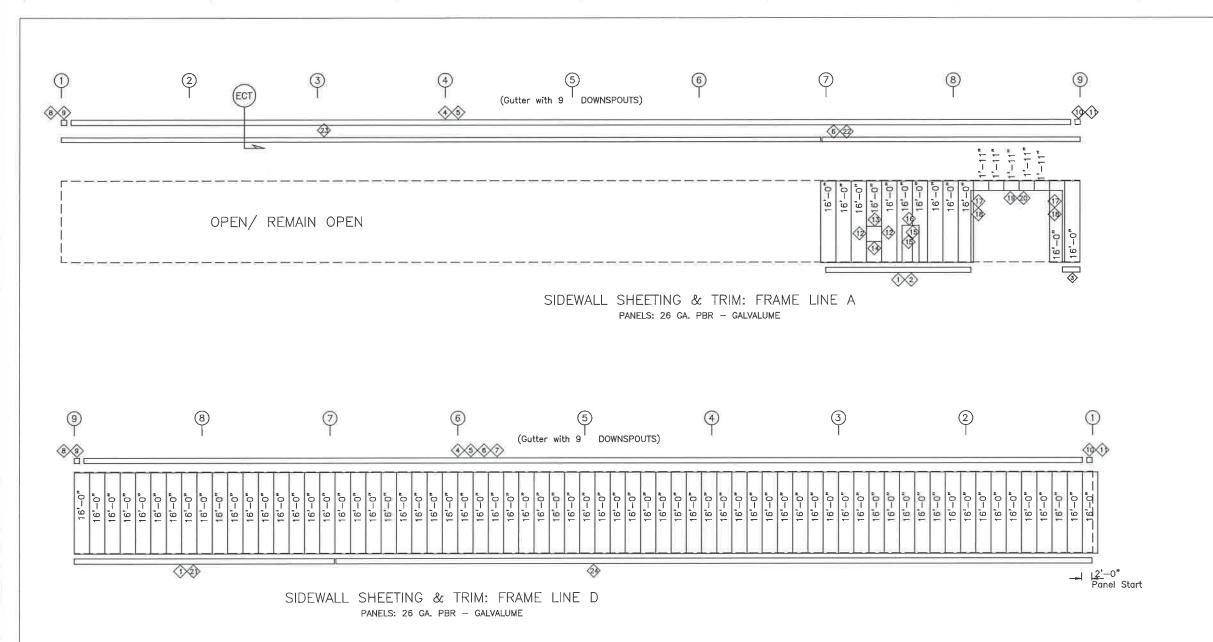
ROOF SHEETING PLAN PANELS: 26 GA. PBR – GALVALUME

ISSUE		DET	CHK	DATE
BUILDINGS AND MC	RE			
EUSTOMER: F TRAFFIC OPS BU	ILDINGS			
ов но: 7966/8227		DATE:	23/2	3
LAKE CITY, FL 320	55	A		
PRAWING NAME:				
ROOF PANELS & T	RIM			
PAGE 6	DAR	SPW	: 50	NONE



- [2] METAL SHAVINGS MUST BE SWEPT FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.

			-	
ISSUE		DET	СНК	DATE
				1
BUILDINGS AND MO	ORE			
F TRAFFIC OPS BU	JILDINGS			
7966/8227		DATE: 8/	/23/	23
LAKE CITY, FL 320	)55			
ROOF PANEL DETA	ILS			
AWING NO: PAGE 6.1	DRAWN BY: DAR	DAR	iri s	SCALE: NONE



TRIM TABLE FRAME LINE A & D FRAME LINE A & D

DID PART LENGTH

DRIP BASE 20'-3"

DRIP BASE 9'-1"

DRIP BASE 3'-9"

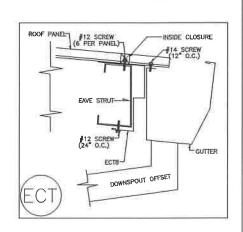
GUTTER 20'-3"

GUTTER 10'-3"

EAVE TRM 20'-3"

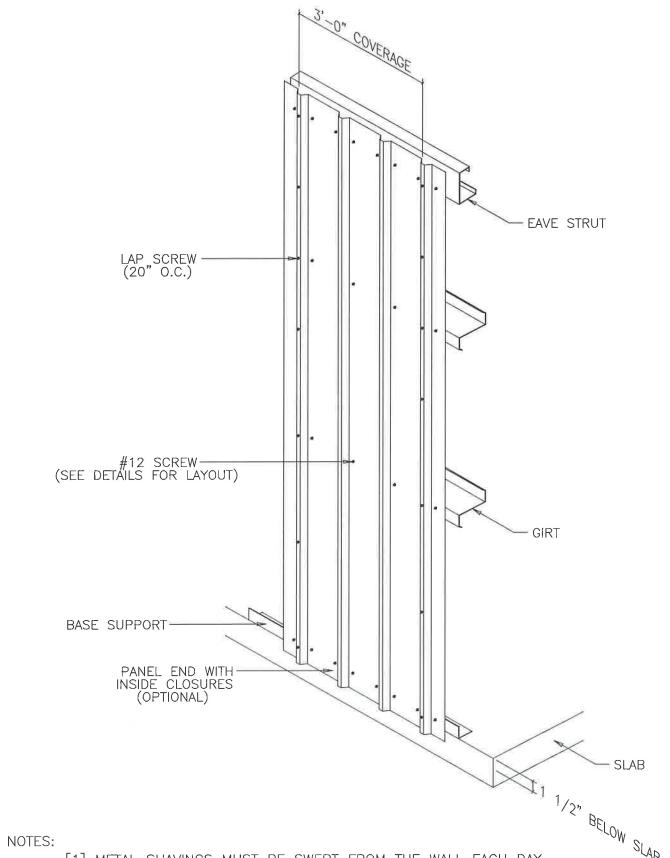
REVE TRM 10'-3"

BUTEND L 1" OID PART TRIM\_16 TRIM\_16 TRIM\_16 TRIM\_1 TRIM\_1 TRIM\_120 TRIM\_120 TRIM\_2 TRIM\_2 TRIM\_2 TRIM\_2 TRIM\_8 8 GUTEND L 9 CORBOX L 10 GUTEND R 11 CORBOX R 12 R JAMB 13 R HEAD 14 R HEAD TRIM\_61 TRIM\_7 15 R JAMB 16 R HEAD TRIM 8 TRIM\_61 16 R HEAD 3'-7"
17 CT8 14'-1"
18 R JAMB 14'-3"
19 CT8 18'-0"
20 R HEAD 18'-3"
21 DRIP BASE 10'-7"
22 EAVE TRM 10'-7"
23 ECT8 20'-3"
24 MOD. DRIP 20'-3" TRIM\_11 TRIM\_8 TRIM\_10 TRIM\_61 TRIM\_16 TRIM\_120 ECT



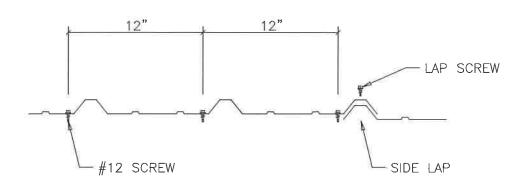
		DET		
	ISSUE		CHK	DATE
BUILDINGS AND	MORE			
F TRAFFIC OF	S BUILDINGS			
IOB NO:		DATE		
7966/8227		8/	23/2	3
LOCATIONS		111		
LAKE CITY, FL	32055			
FRAWING NAME:				
SIDEWALL PAN	IELS & TRIM			
DRAWING NO:	DRAWN BY:	CHECKED BY	r: SC/	NE:
PAGE 7	DAR	ISPW		NONE

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.



5" 7" 5" 7" 5" 5" LAP SCREW CONTINUOUS TAPE SEAL SIDE LAP

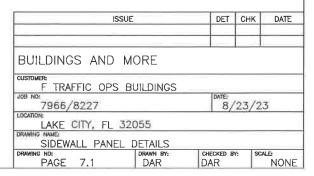
PANEL ATTACHMENT AT PANEL END (BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)

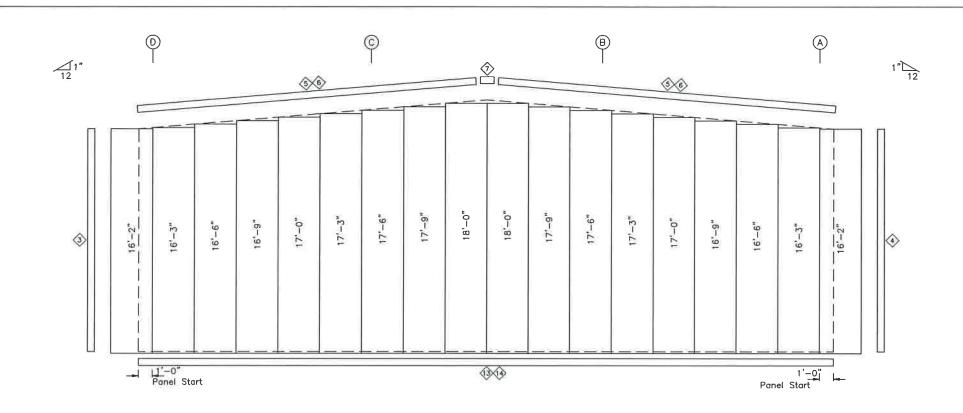


PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

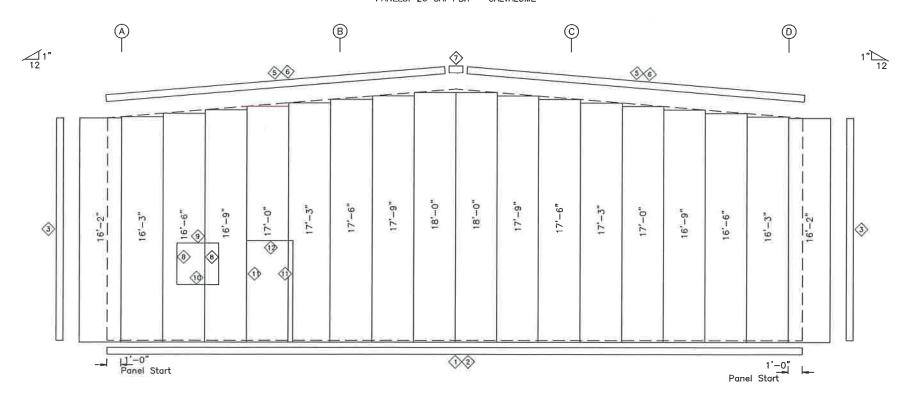
[1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.

[2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.





## ENDWALL SHEETING & TRIM: FRAME LINE 1 PANELS: 26 GA. PBR — GALVALUME

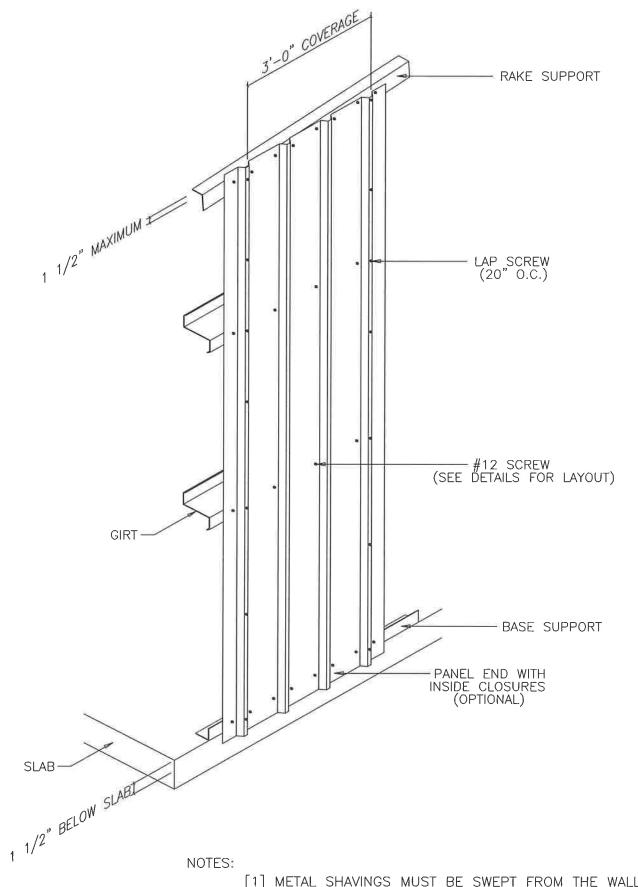


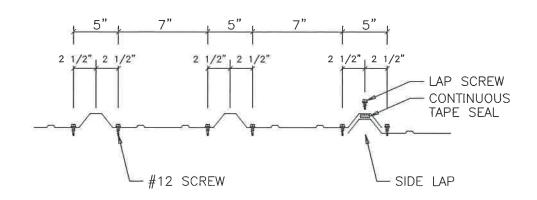
ENDWALL SHEETING & TRIM: FRAME LINE 9
PANELS: 26 GA. PBR - GALVALUME

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

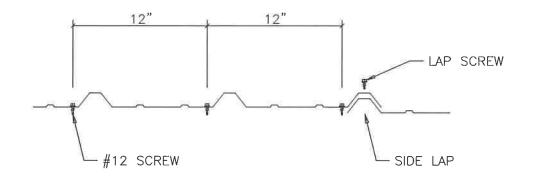
_								
	TRIM	1 TABLE						
	FRAME LINE 1 & 9							
		PART	LENGTH	DETAIL				
	1	DRIP BASE	20'-3"	TRIM. 16				
	2	DRIP BASE		TRIM_16				
	2 3	O/S CORN		TRIM_5				
	4	ССТВ	16'-2"	CCT				
	5	RAKE TRM	$\frac{1}{20}$ , $-\frac{1}{2}$ "	TRIM_3				
	6	RAKE TRM	20'-3" 5'-2"	TRIM 3				
	7	PEAK BOX	1,-4"	TRIM_4				
		R JAMB	3'-3"	TRIM_8				
	8		J -J					
	9	R HEAD	3'-3"	TRIM_61				
	10	R HEAD	3'-3"	TRIM_7				
	11	R JAMB	7'-5"	TRIM_8				
	12	R HEAD	3'-7"	TRIM_61				
	13	MOD. DRIP	20'-3"	IB				
	14	MOD. DRIP	10'-3"	IB				

ISSUE		DET	снк	DATE
BUILDINGS AND M	ORE			
CUSTOMER: F TRAFFIC OPS B	UILDINGS			
JOB NO: 7966/8227		DATE:	23/2	3
LAKE CITY, FL 32	055			
DRAWING NAME: ENDWALL PANELS	& TRIM			
DRAWING NO:	DRAWN BY:	CHECKED BY	r: SC/	ve: N∩NF





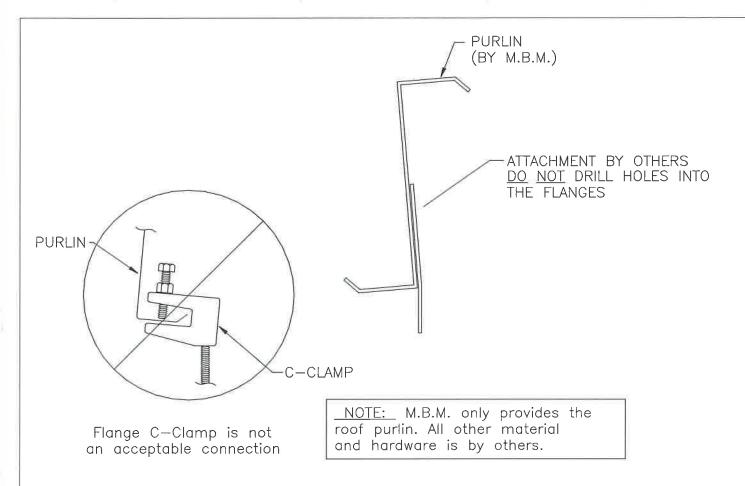
PANEL ATTACHMENT AT PANEL END (BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

- [1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.

ISSUE		DET	СНК	DATE
BUILDINGS AND M	ORE			
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ов NO: 7966/8227		BATE 8/	23/2	3
LAKE CITY, FL 320	055			
FRAMING NAME:  ENDWALL PANEL D	ETAILS			
PAGE 8.1	DRAWN BY: DAR	DAR	': SCA	NONE



Recommended Connection Detail

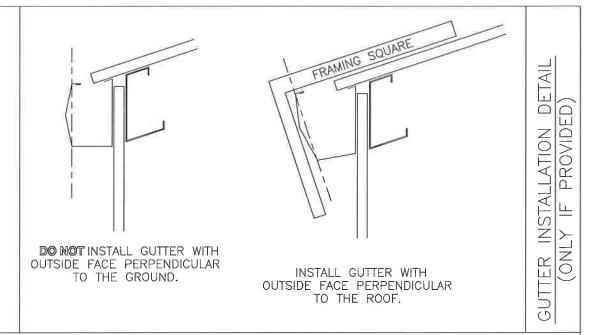
### NOTE

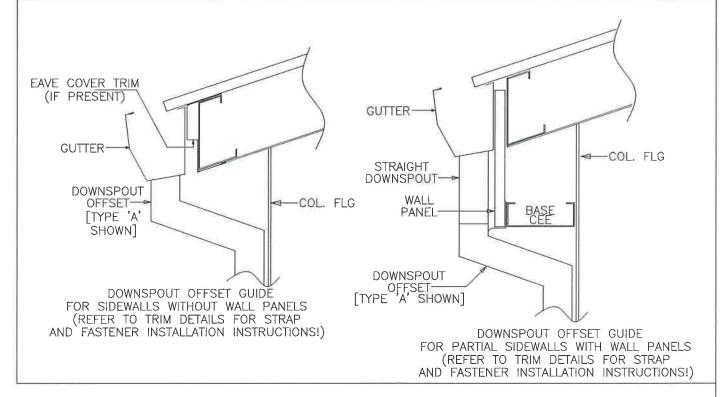
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS( A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS





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LAKE CITY, FL 3	2055			
DRAWING NAME: SPECIAL DETAILS				
DRAWING NO: PAGE 9	DRAWN BY: DAR	DAR	i: 50	NONE

