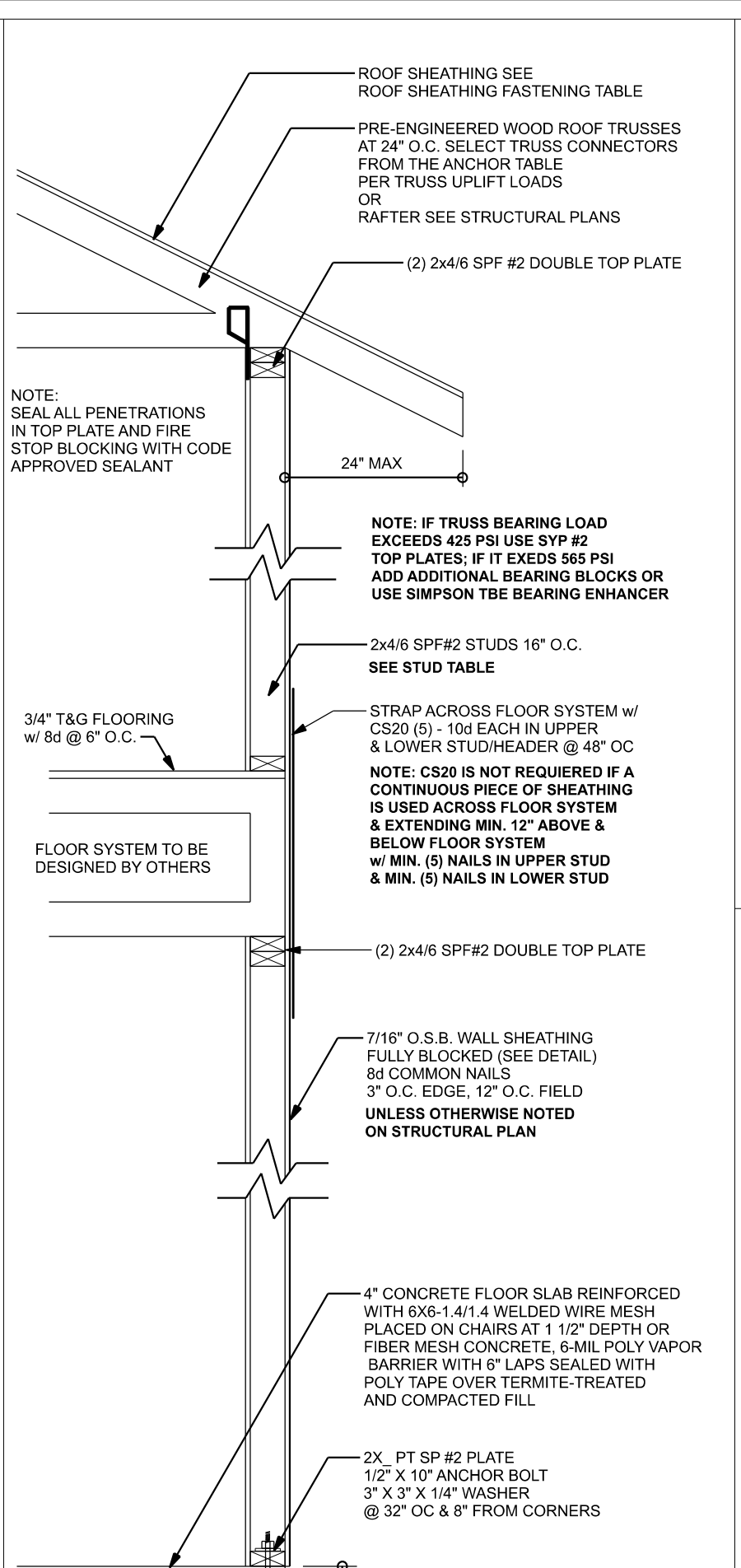
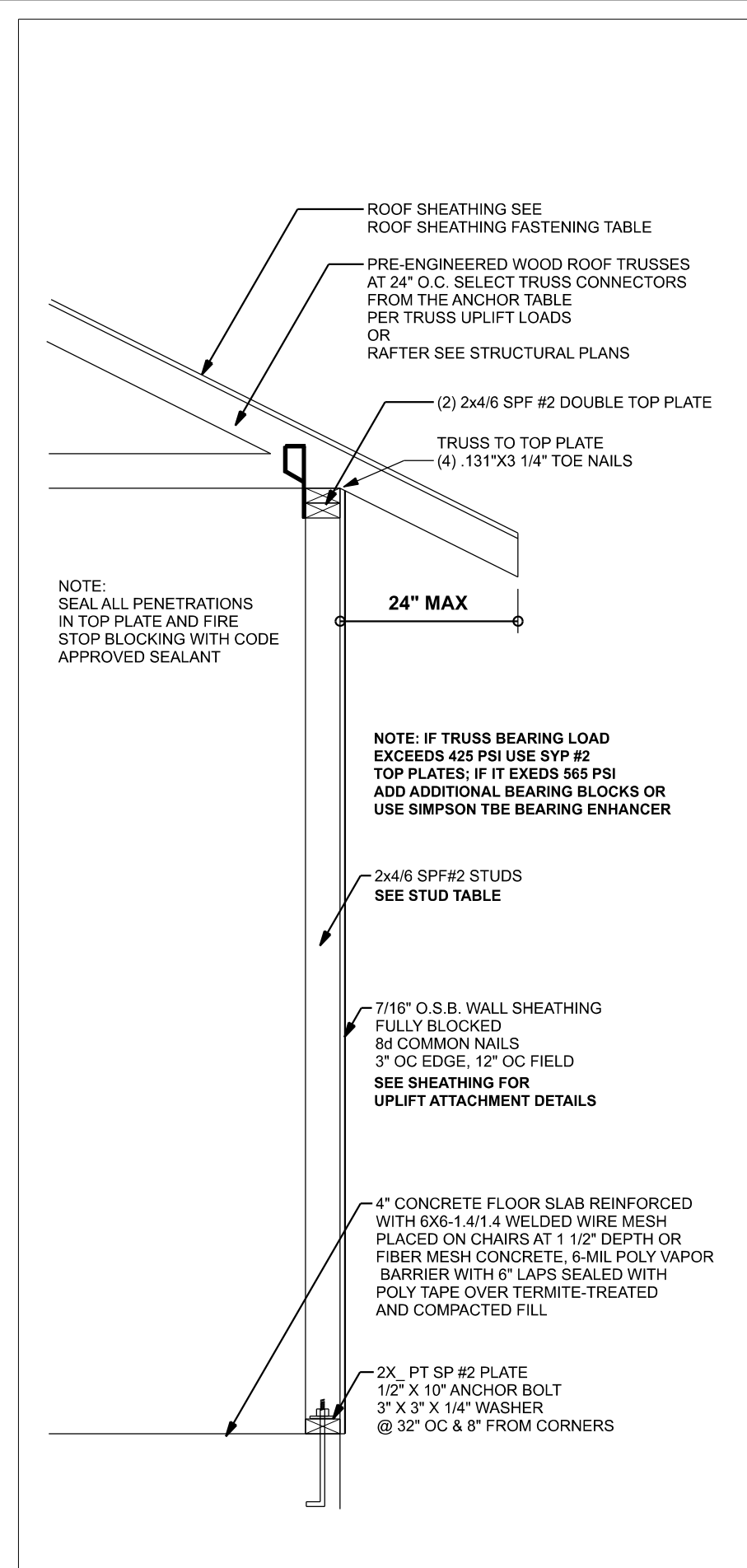
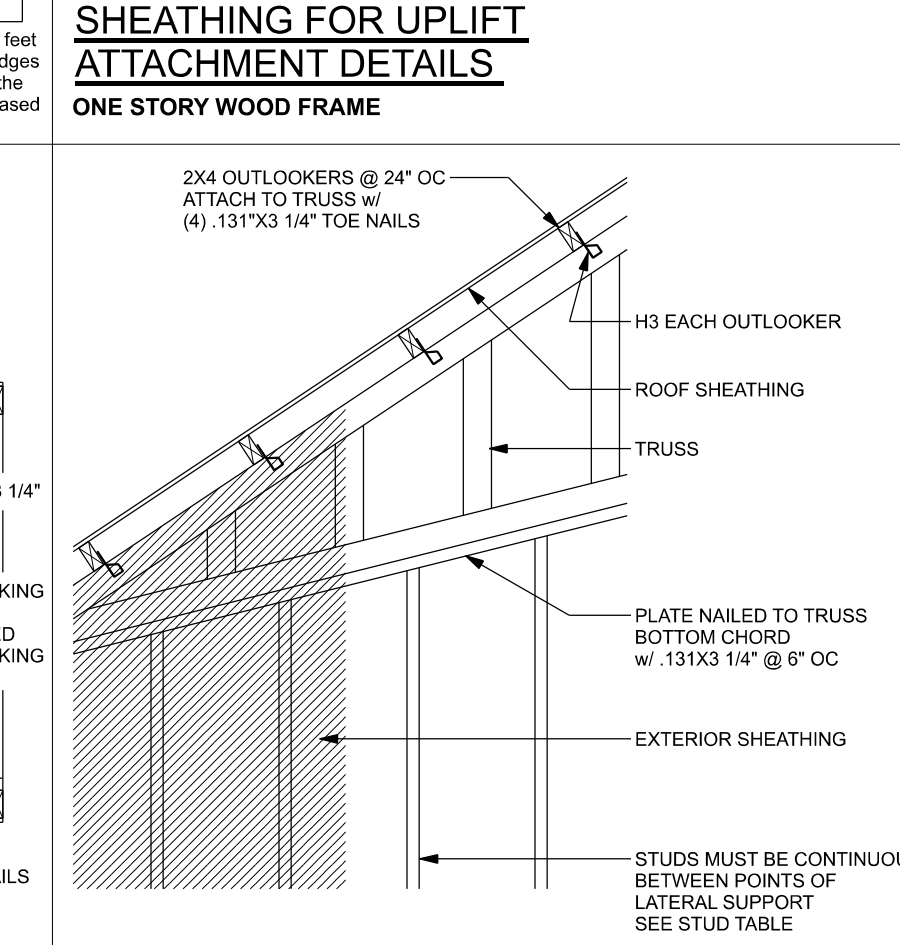
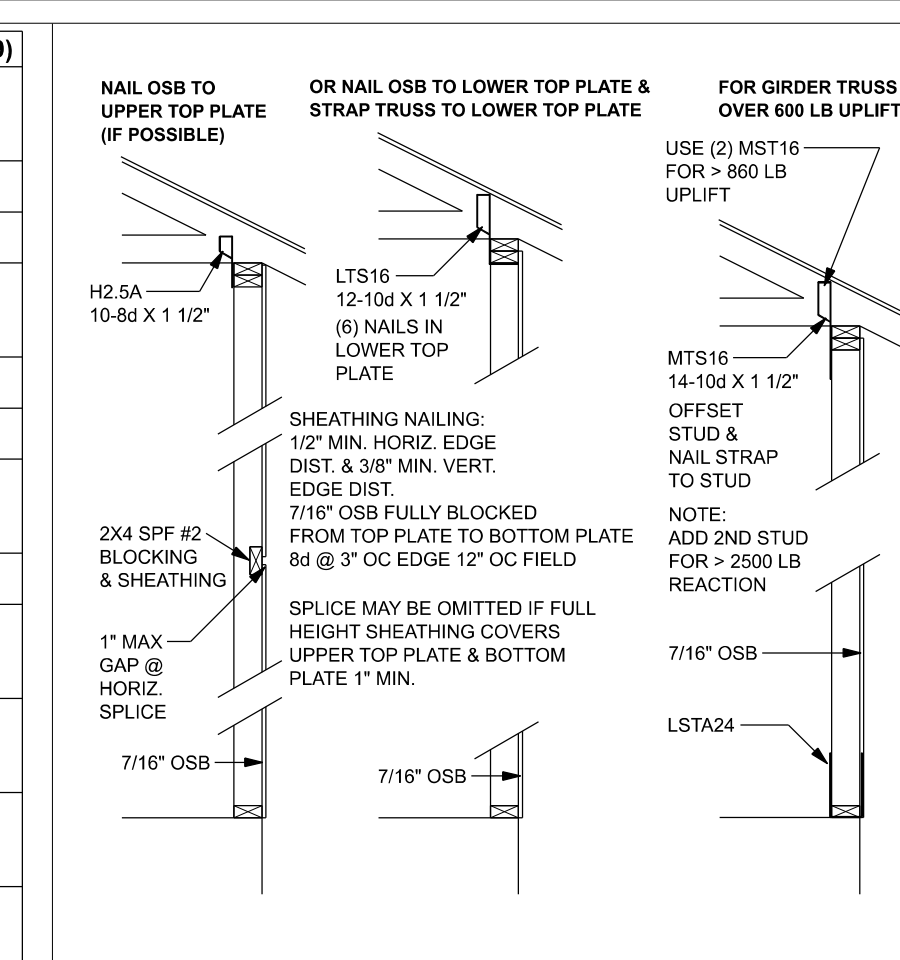




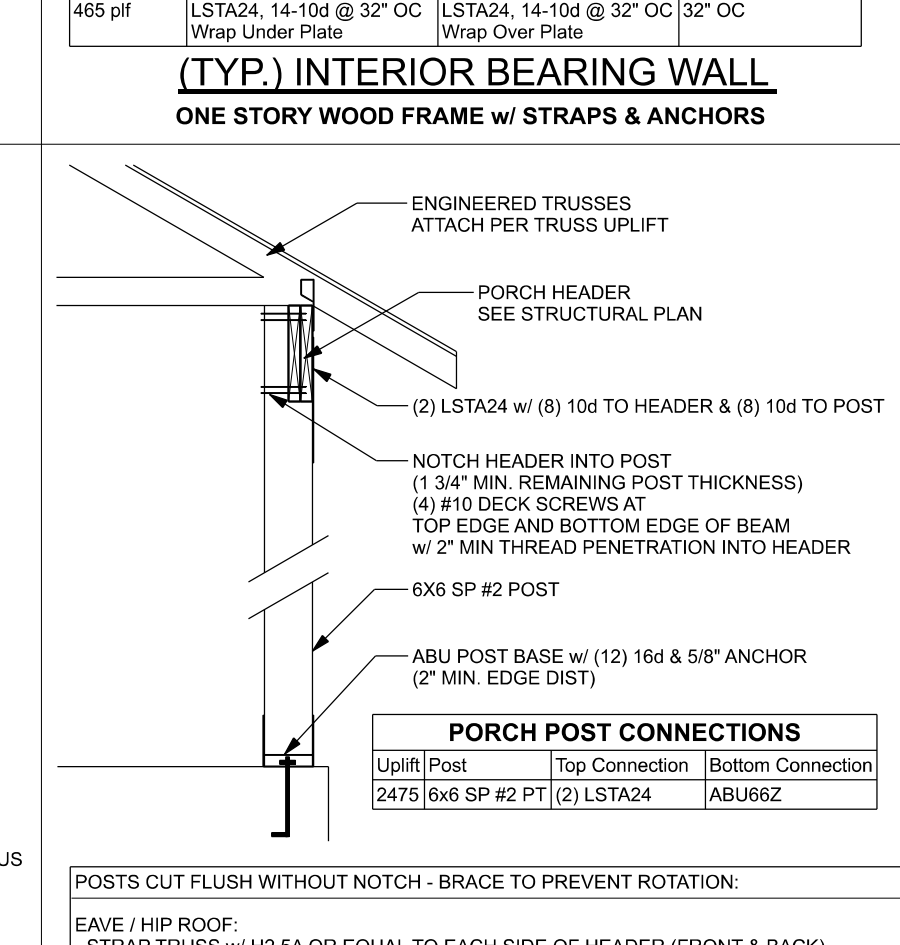
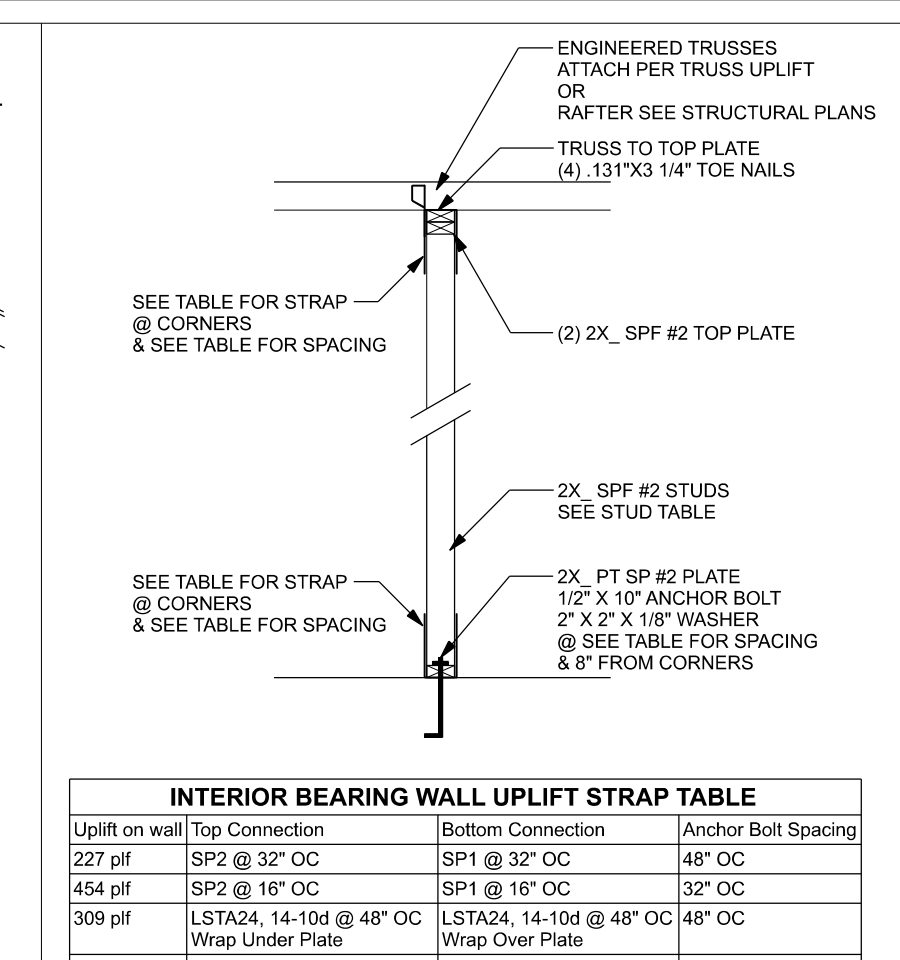
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood OR OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	12" oc
120 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



(TYP.) GABLE BRACING DETAIL
WOOD FRAME



(TYP.) PORCH POST
ONE STORY WOOD

CONNECTOR TABLE

Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pcf	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pcf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
309 pcf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
465 pcf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 12" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

Stud Height	Stud Spacing
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT
(1) 2x8 @ 16" OC	TO 22'-4" STUD HEIGHT

GRADE & SPECIES TABLE

Species	Fb	E
2x10 SP #2	925	1.4
2x10 SP #2	800	1.4
2x12 SP #2	750	1.4
GLB 24F-V3 SP	2600	1.9
LVL MICROLAM	2900	2.0
PSL PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x 4 W14, F8 + 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE LEFT DO NOT CUT W/WW OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ENGINEER'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS), UNO, ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH 318-16, U.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED CONCRETE.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, TRUSS PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR DESIGN APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

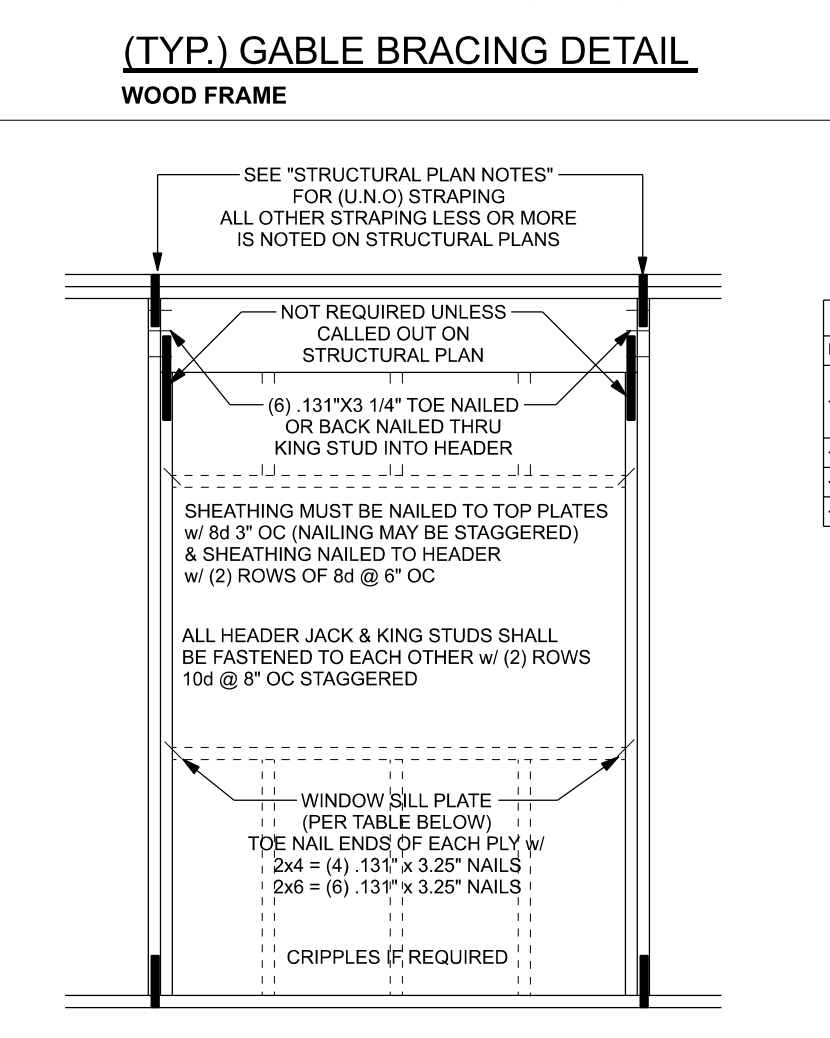
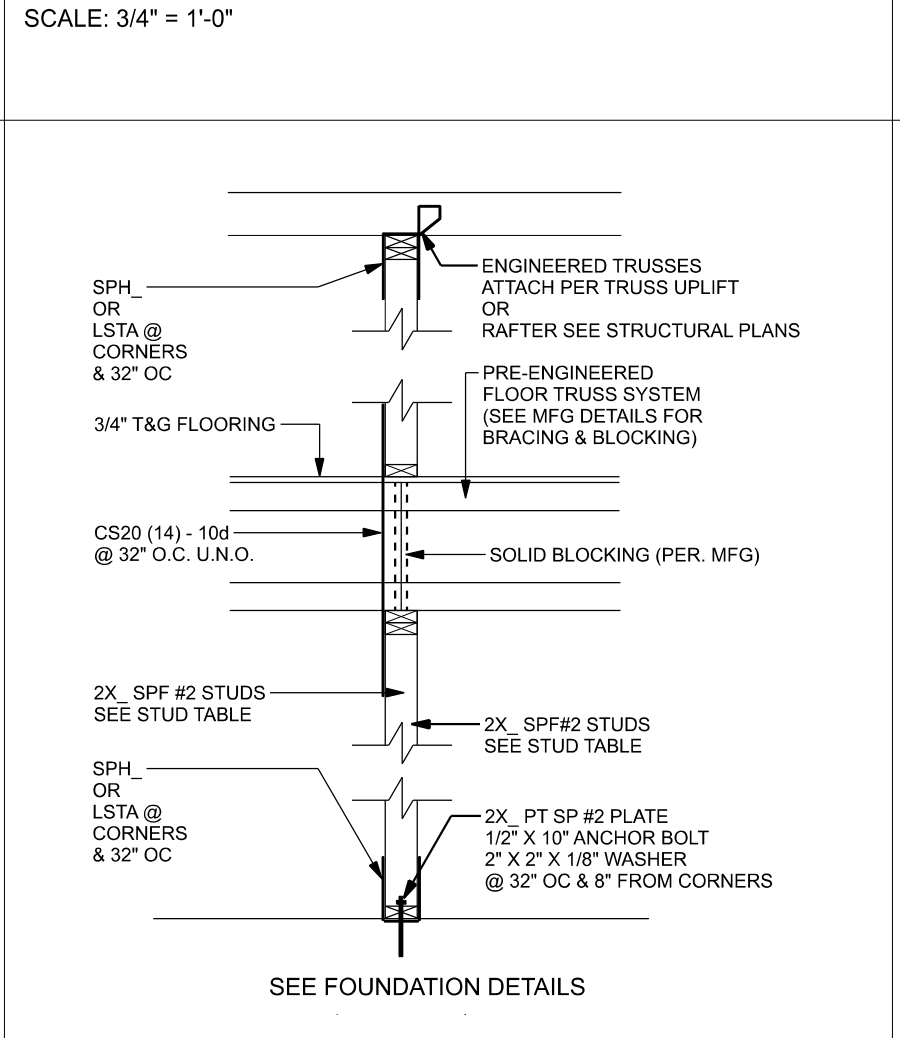
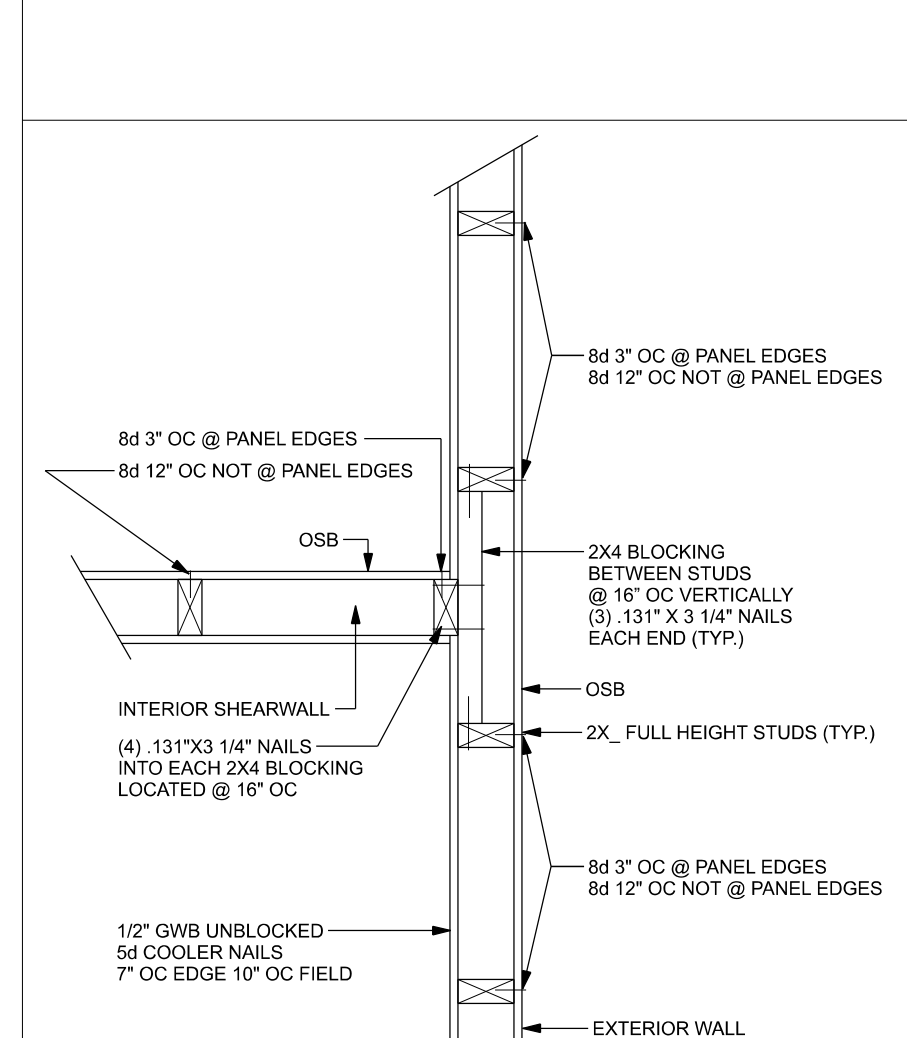
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

(TYP.) PORCH POST
ONE STORY WOOD

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



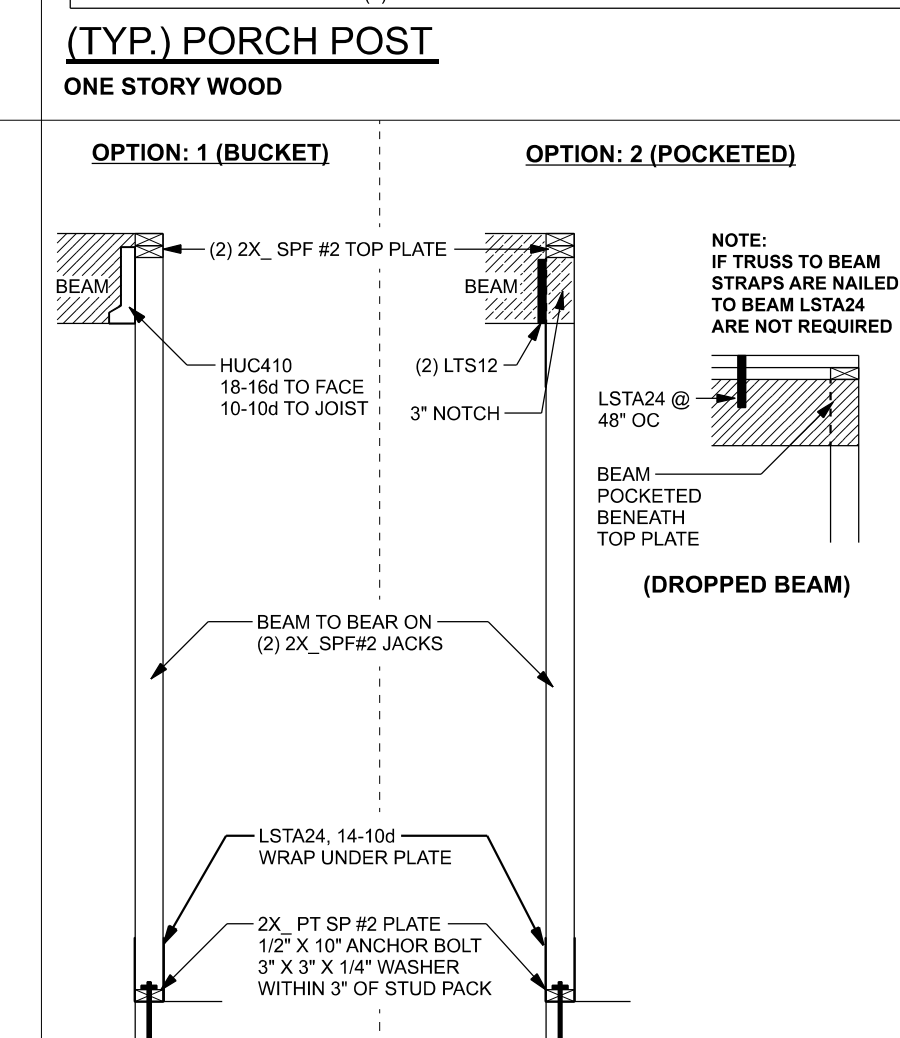
HEADER STRAP TABLE

Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 18-10d header to jacks	DTT22
< 1800	(2) MST24, 18-10d header to jacks	DTT22
< 2910	(2) MST24, 18-10d header to jacks	HTT4

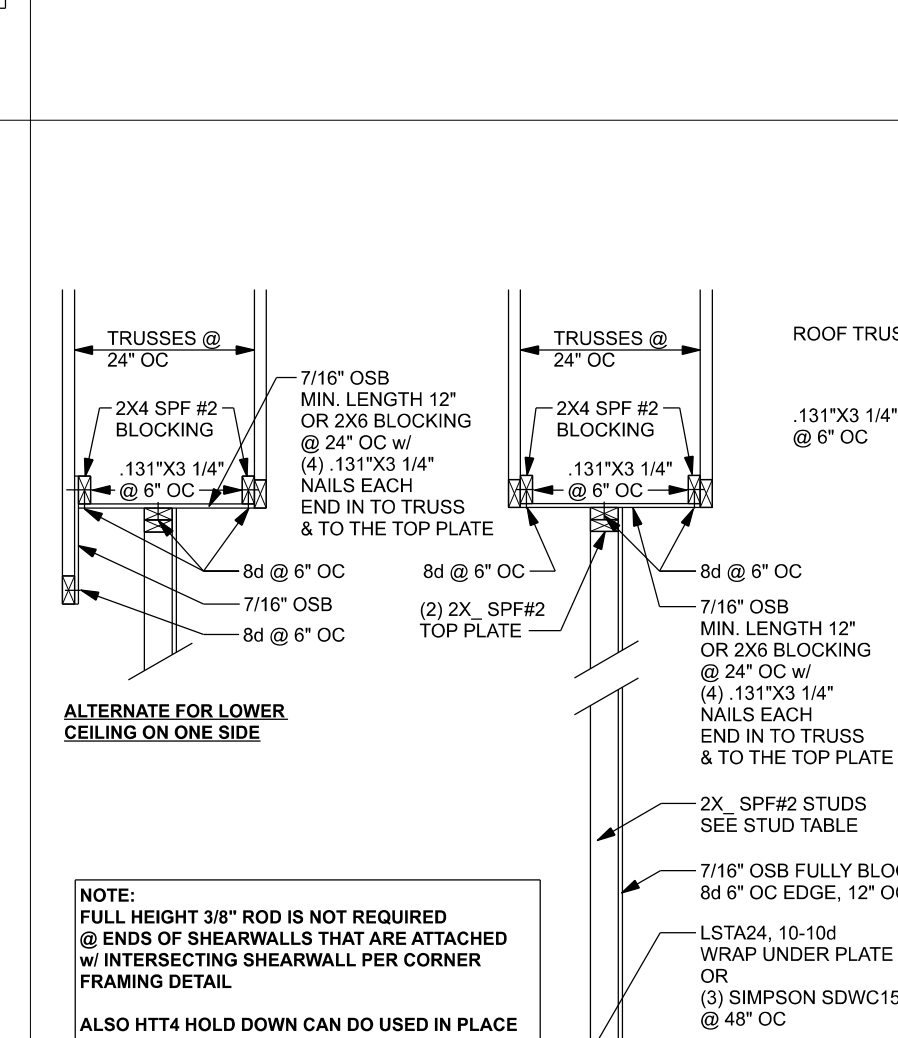
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-238
130 MPH EXP. C	5'-2"	7'-9"
	7'-9"	7'-7"
	7'-7"	11'-3"

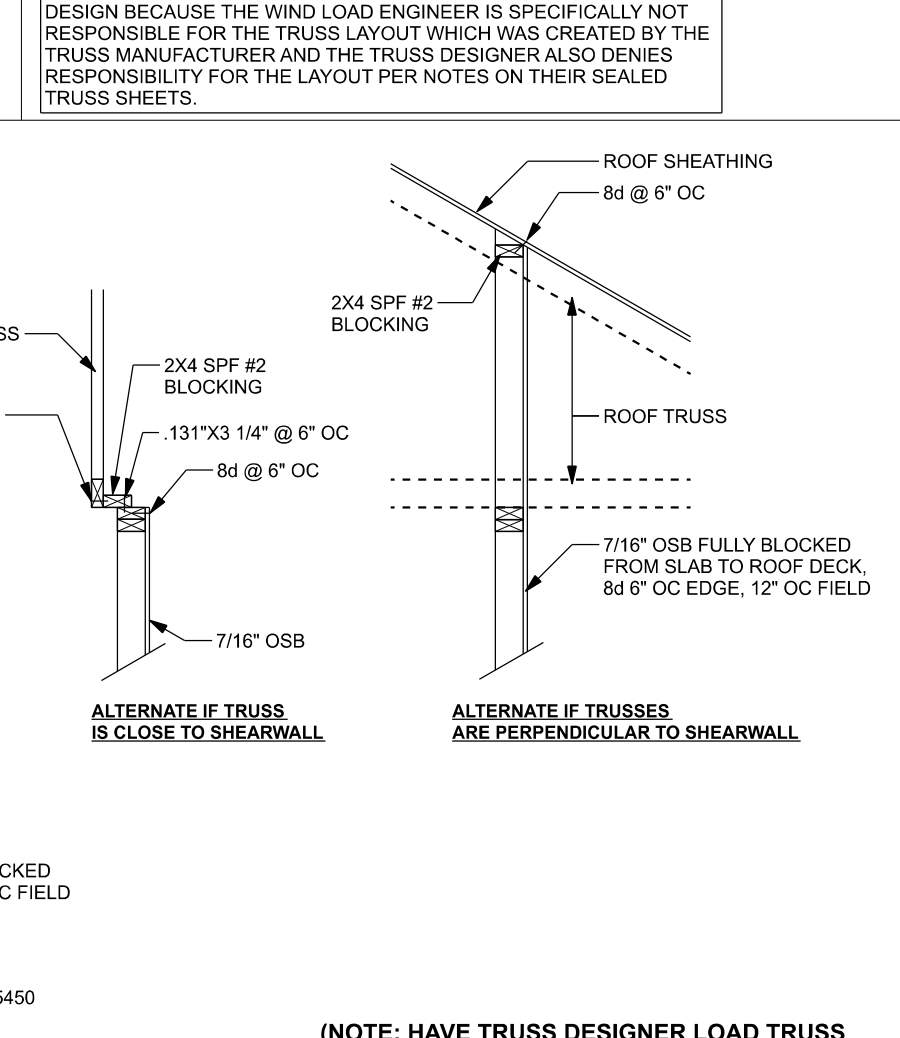
FOR OTHER WALL HEIGHTS (H) ALL SPAN SHALL BE DIVIDED BY (H/10)



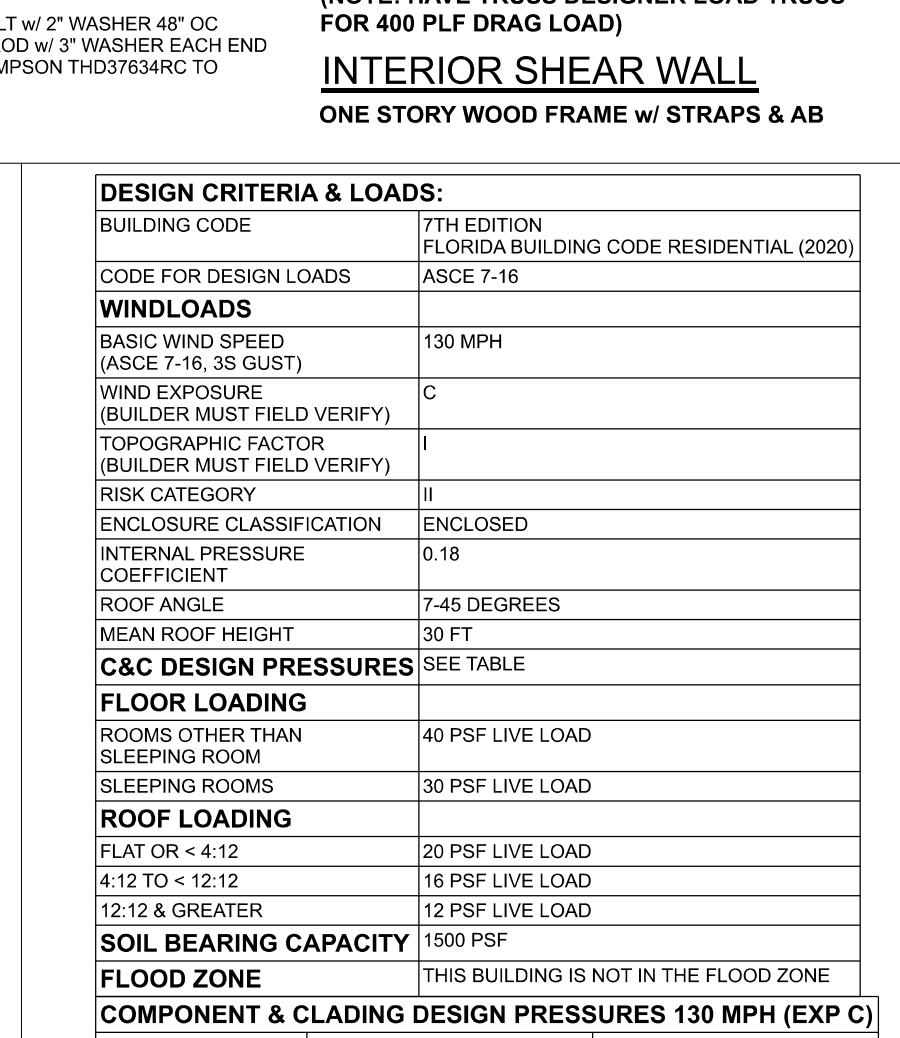
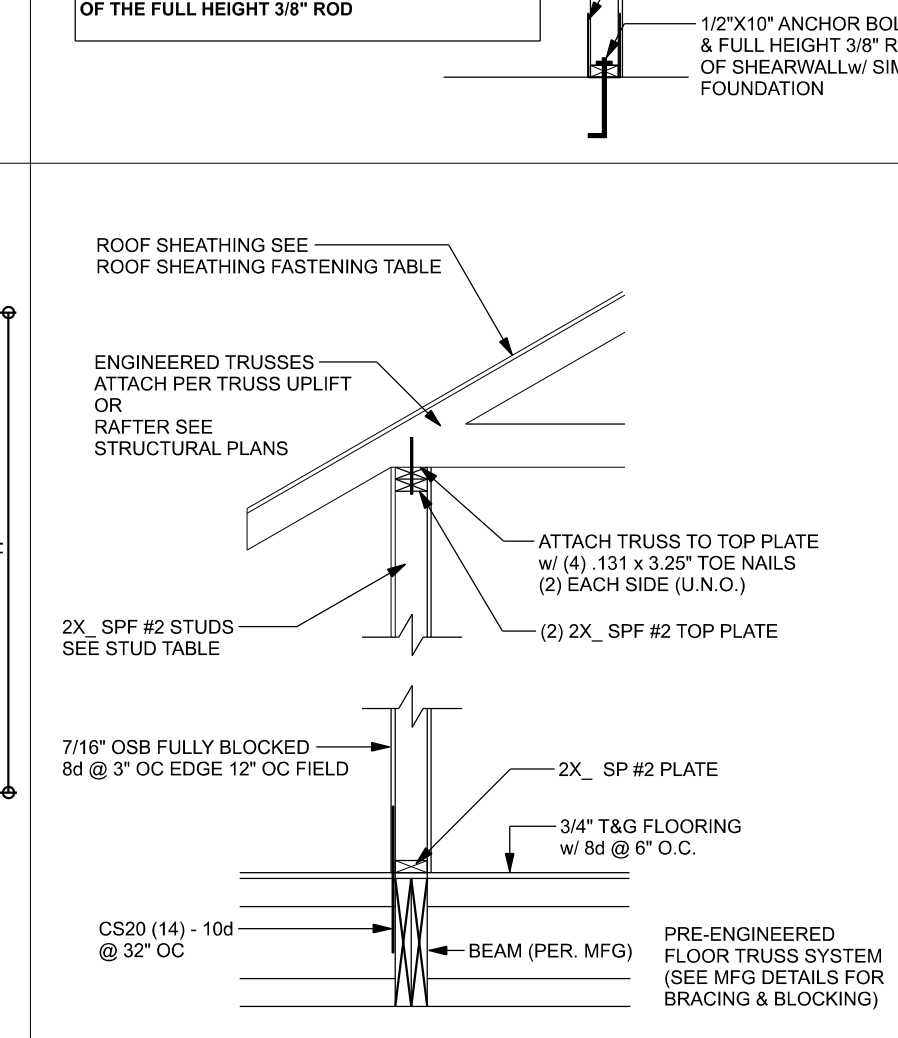
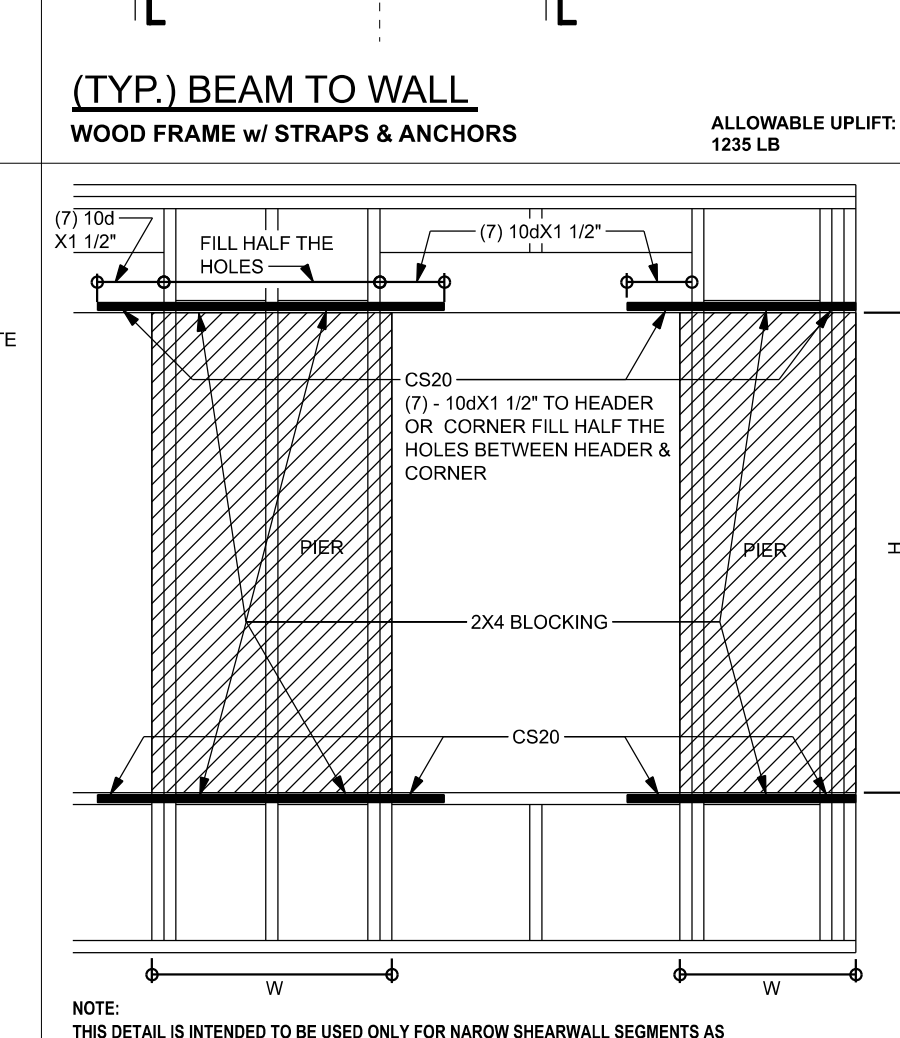
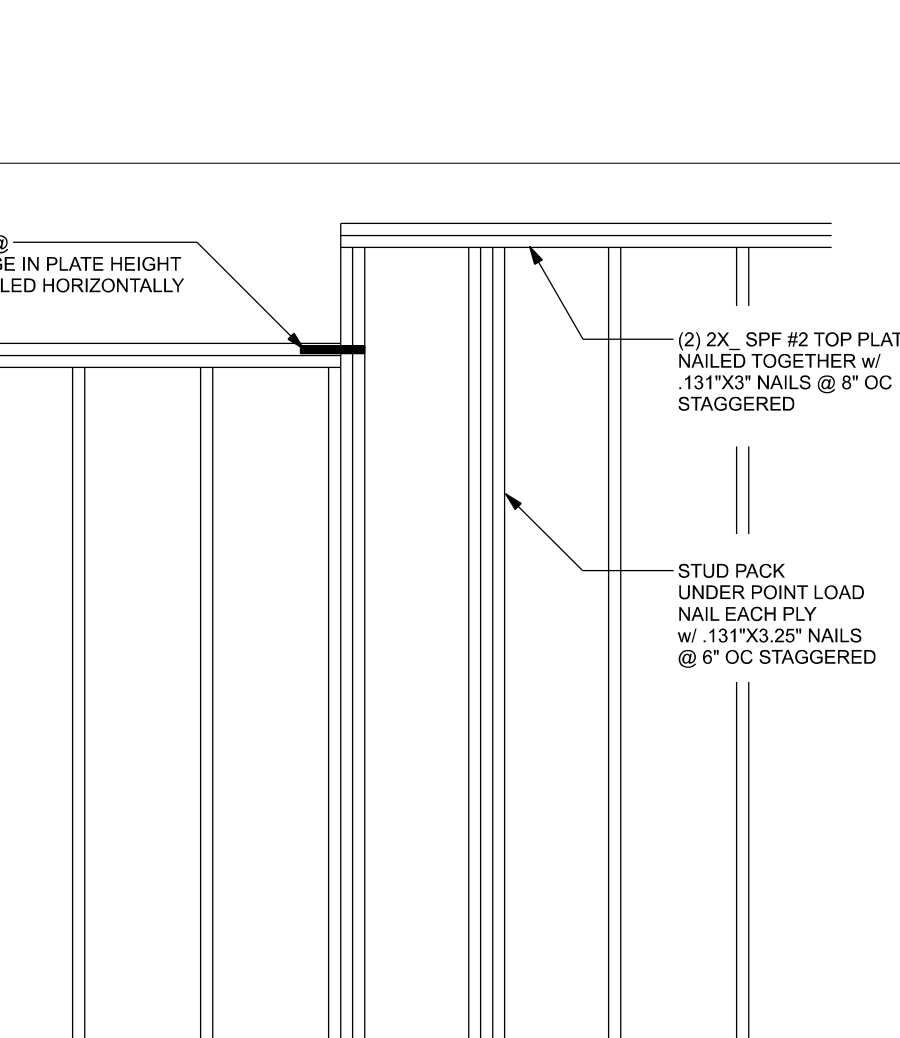
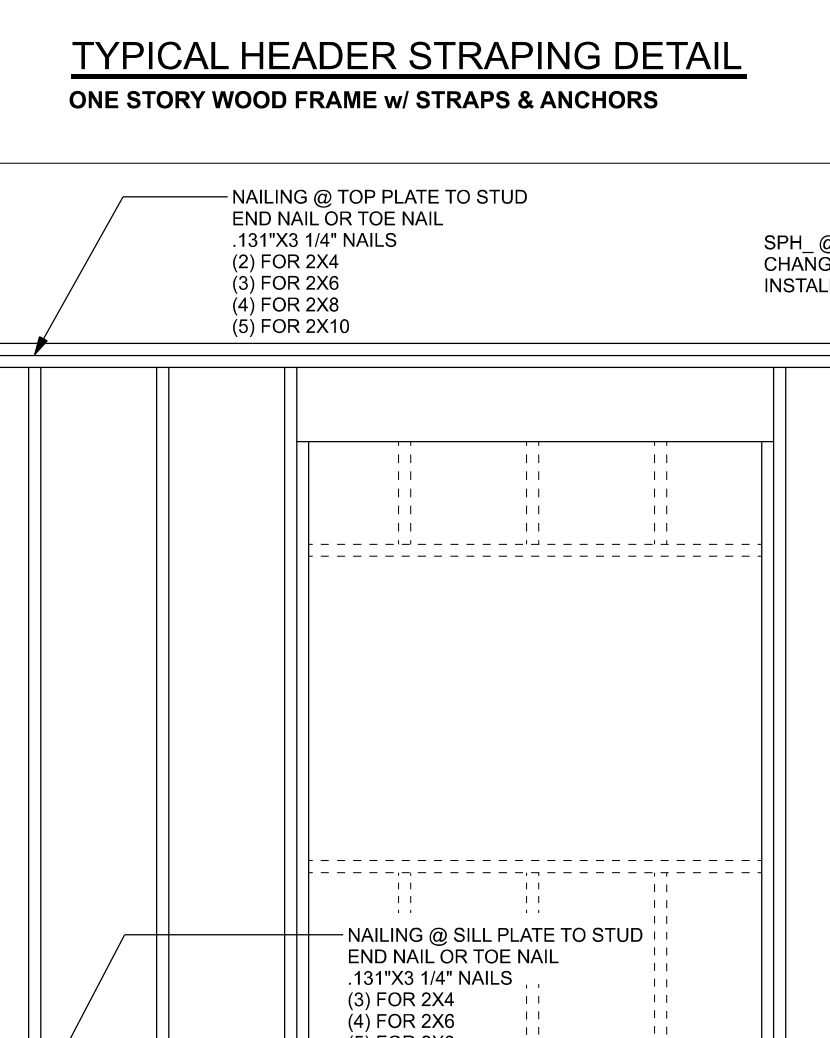
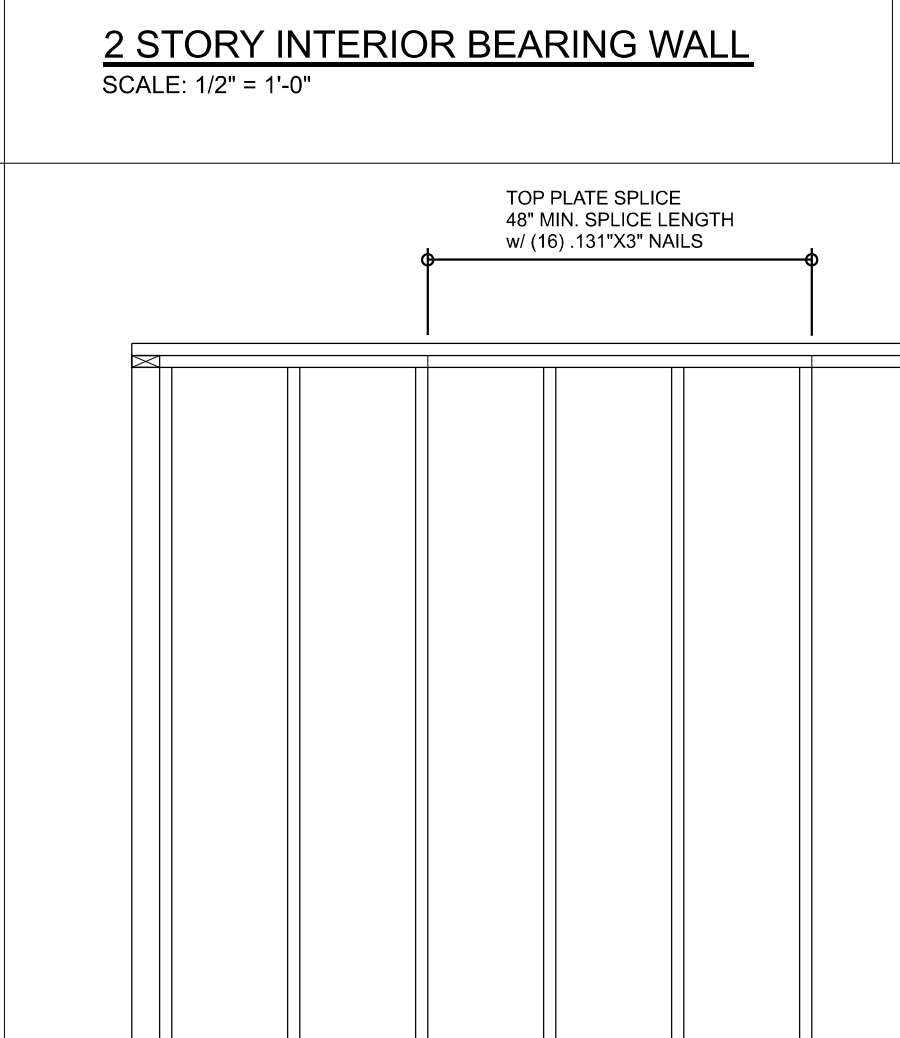
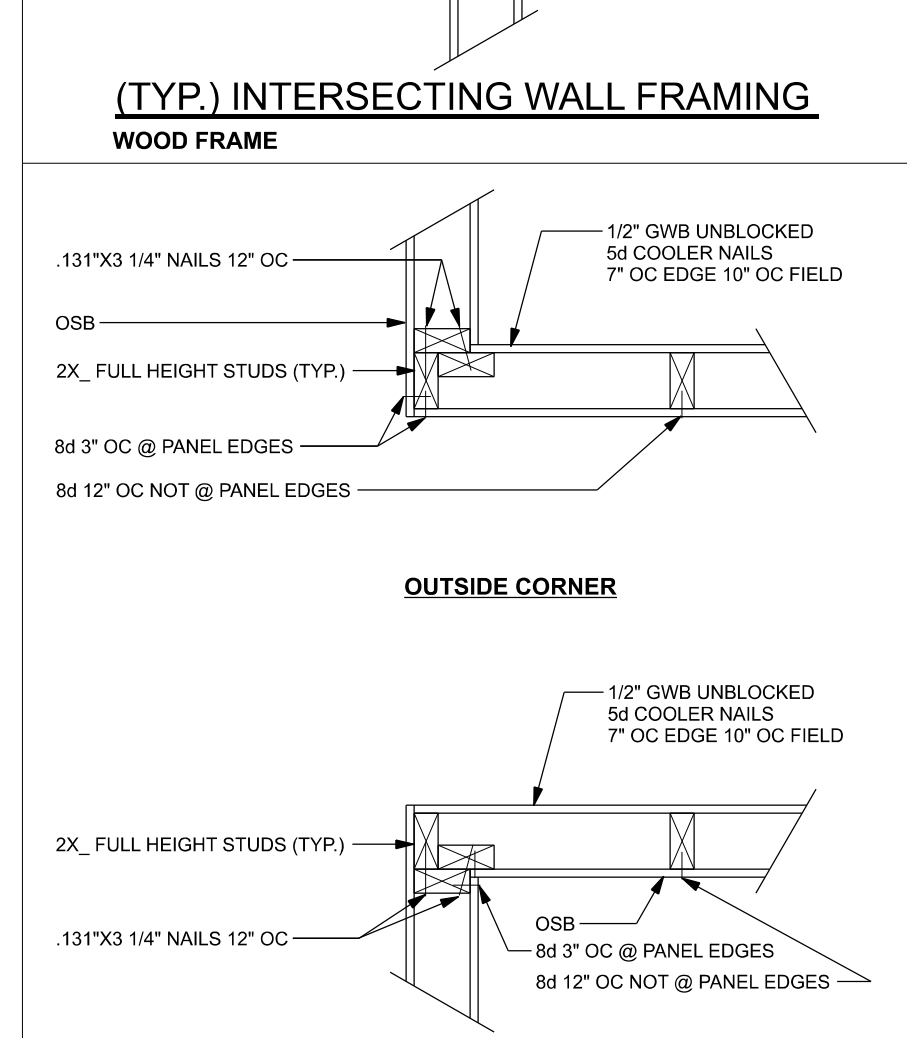
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) CORNER FRAMING
WOOD FRAME

(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

(TYP.) PORCH POST
ONE STORY WOOD

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

Blake Construction

Price Residence

PROJECT ADDRESS:
Lake City, FL

FL PE 53915

This item has been digitally signed and sealed by Mark Dissoway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01410C0000017E97DE07CA000746F0, CN=Mark D Dissoway

2023-02-24 16:14:28

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Dissoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

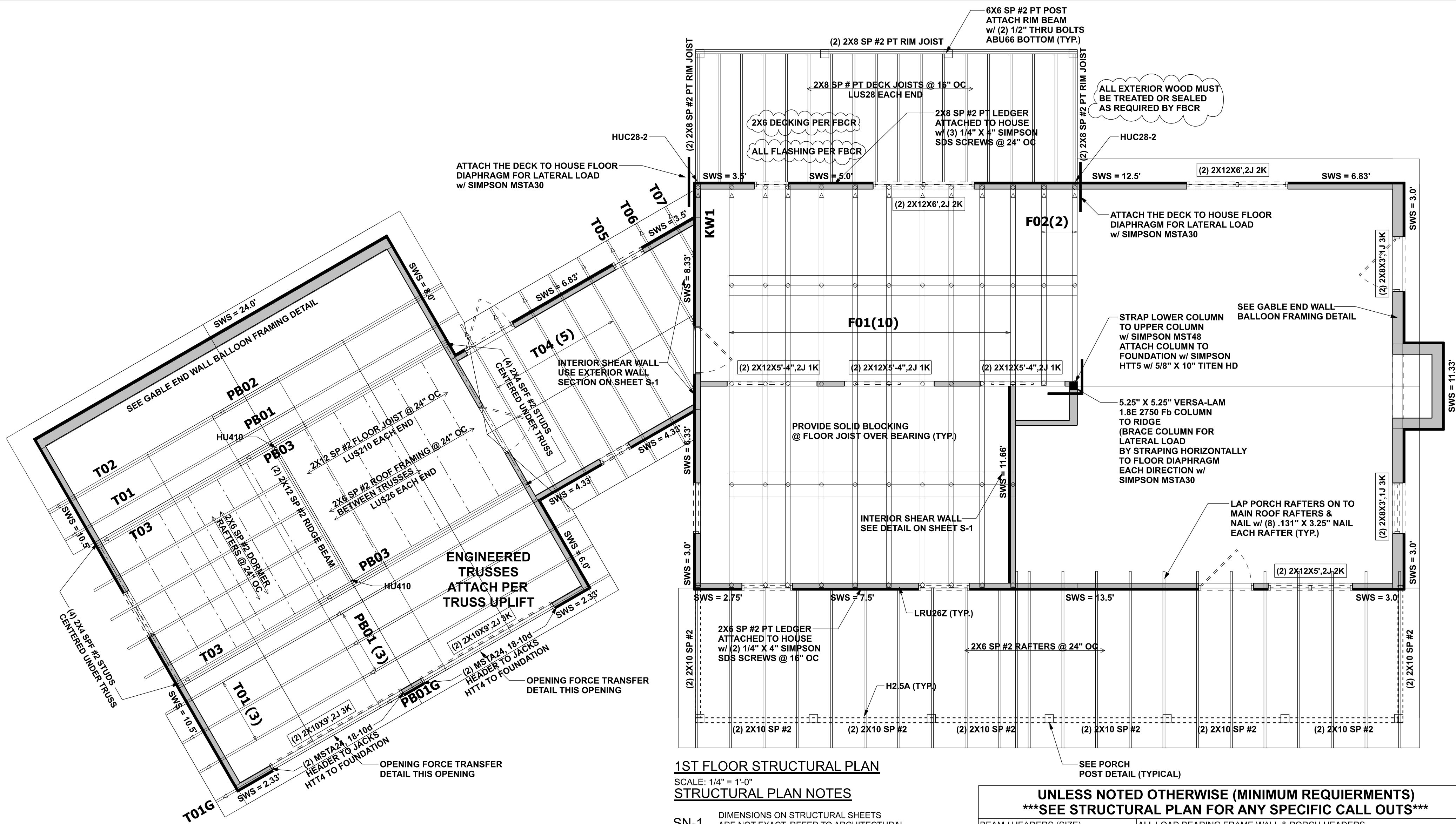
LIMITATION: This design is valid for one building, at specified location.

Mark Dissoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
dissowaydesign@gmail.com

JOB NUMBER:
220864

S-1
OF 7 SHEETS

OF 7 SHEETS



BONUS ROOM / GABLE END BRACING

SCALE: 1/2"=1'-0"

IS PER ENGINEERING

(6) 12dS
2X4 BLOCKING
(4) 12dS
PIGGY
2X4 SPF #2 GABLE BRACE, 6' O.C.
(4) 12dS
2X4 SPF #2 PLATE
2X4 BLOCKING
12dS, 12" O.C.
7/16" OSB FULLY BLOCKED
3" O.C. EDGE,
12" O.C. FIELD
2X4 SPF #2 STUDS SEE STUD TABLE
12dS, 12" O.C.
2X4 SPF #2 PLATE
3/4" T&G 8d, 6" O.C.
ATTIC TRUSS
24" O.C.
1/2" GYP 5d COOLER 7"
O.C. EDGES
UNBLOCKED

SEE GABLE END WALL
BALLOON FRAMING DETAIL

(2) MSTA24, 18-10d
HEADER TO JACKS
HTT4 TO FOUNDATION

(2) 1.75" X 20.00" LVL RIDGE BEAM

(4) 2X6 SPF #2 STUDS
CENTERED UNDER LVL
STRAP DOWN TO HEADER
BELOW w/ (2) HTS20

2X10 SP #2 RAFTERS @ 24" OC

CS20, 10-10d
RIDGE TENSION
STRAP (TYP.)
LRU210Z (TYP.)

STRAP LVL TO
STUD PACK
w/ (2) HTS20

STRAP LVL TO COLUMN
w/ (4) HTS20

5.25" X 5.25" VERSA-LAM
1.8E 2750 Fb COLUMN
CENTERED UNDER LVL

(4) 2X8 SPF #2 STUDS
CENTERED UNDER LVL
STRAP DOWN TO HEADER
BELOW w/ (2) HTS20

2X10 SP #2 RAFTERS @ 24" OC

SEE GABLE END WALL
BALLOON FRAMING DETAIL

(2) MSTA24, 18-10d
HEADER TO JACKS
HTT4 TO FOUNDATION

(3) 2X12S 2J 2K

(2) 2X8X6 1J 2K

H10 (TYP.)

JOB NUMBER:
220864

S-3
OF 7 SHEETS