Columbia County Building Permit PERMIT This Permit Must Be Prominently Posted on Premises During Construction 000026615 APPLICANT B. TRENT GIEBEIG PHONE 386.397.0545 SE HOLLY TERRACE ADDRESS 697 LAKE CITY 32025 OWNER ROBERT & BARBARA SCRAGG 386.752.0791 PHONE **ADDRESS** 554 SW MAY-FAIR LANE LAKE CITY FL 32025 **B. TRENT GIEBEIG PHONE** 386.397.0545 CONTRACTOR LOCATION OF PROPERTY SR. 247-S TO MAY-FAIR S.D, TR AND IT'S THE LAST JOB ON THE L @ MAY-FAIR LANE. ESTIMATED COST OF CONSTRUCTION 120300.00 TYPE DEVELOPMENT SFD/UTILITY HEATED FLOOR AREA 1600.00 TOTAL AREA 2406.00 HEIGHT 17.20 STORIES FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC RSF-2 MAX. HEIGHT 35 LAND USE & ZONING STREET-FRONT 25.00 SIDE 10.00 Minimum Set Back Requirments: 15.00 NO. EX.D.U. FLOOD ZONE XPP DEVELOPMENT PERMIT NO. SUBDIVISION PARCEL ID 11-48-16-02911-343 MAY-FAIR TOTAL ACRES LOT 43 BLOCK 000001519 R282811523 Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Conti ractor JTH 18"X32'MITERED 07-1006 BLK New Resident Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD. NOC ON FILE. Check # or Cash FOR BUILDING & ZONING DEPARTMENT ONLY (footer/Slab)

Temporary Power Monolithic date/app. by date/app. by date/app. by Under slab rough-in plumbing Sheathing/Nailing date/app. by date/app. by date/app. by Framing Rough-in plumbing above slab and below wood floor date/app. by date/app. by Electrical rough-in Heat & Air Duct Peri. beam (Lintel) date/app. by date/app. by date/app. by Permanent power C.O. Final Culvert date/app. by date/app. by date/app. by M/H tie downs, blocking, electricity and plumbing Pool date/app. by date/app. by Reconnection Utility Pole Pump pole date/app. by date/app. by date/app. by M/H Pole Travel Trailer Re-roof date/app. by date/app. by date/app. by 605.00 CERTIFICATION FEE \$ 12.03 SURCHARGE FEE \$ 12.03 **BUILDING PERMIT FEE \$** ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 _ TOTAL FEE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

INSPECTORS OFFICE

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

CLERKS OFFICE

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

NOTICE OF COMMENCEMENT

STATE OF: Florida COUNTY OF: Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, <u>Florida Statues</u>, the following information is provided in this Notice of Commencement:

	ruction of single family .
residence	
Owner Information:	
a. Name and Address: Robert L. a	nd Barbara J. Scragg.
13 Edmond Place, Palm	Coast FL 32164-6344
b. Interest in Property: Fee Simple	
 Name and Address of Fee Simple titlehol 	der (if other than Owner).
Contractor (Name and Address): Frent Gi	ebeig Construction, Inc.
697 SE Holly Terrace, Lake	City FL 32025
697 SE HOLLY Terrace, Lake	CICY, FH SEVES
Surety:	[1222년대] 121년대 122년 - 121년 122년대 122년
a. Name and Address: N/AS	
b. Amount of Bond: N/A	Inst:200812000456 Date:1/9/2008 Time:3:04 P
	DC,P.DeWitt Cason,Columbia County F
Lender (Name and Address): N/A	
In addition to himself, the Owner designates the	following person to recieve a copy of the Lieno (Name and Address):
Notice as provided in 713.13 (l)(b), Florida Statu	following person to recieve a copy of the Lieno les (Name and Address):
Notice as provided in 713.13 (l)(b), Florida Statu	es (Name and Address):
Notice as provided in 713.13 (l)(b), Florida Statu N/A Expiration date of Notice of Commencement (the	es (Name and Address):
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JOANNE ROUNTREE Commission DD 624371 Expires March 2, 2011 Bonded Thru Troy Fain Insurance 800-3

elett 3516
For Office Use Only Application # 080 + 42 Date Received 110 By De Permit # 15/9/26615
Zoning Official BLK Date 5.01.08 Flood Zone FEMA Map # NA Zoning RSF-2
Land Use RES La Des Elevation NA MFE adm & River NA Plans Examiner OK STH Date 1-14-08
Comments
NOC E EH Deed or PA Site Plan State Road Info Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No. AP 7/5/3/
Name Authorized Person Signing Permit Trent Gilblig Phone 397-0545
Address 697 SE Holly Terrace Lake City FL 32025
Owners Name Robert L and Barbara J Scragg Phone 752-0791
911 Address 554 SW Mayfair Lane Lake City FL 32024
Contractors Name Trent Ciebria Construction Inc Phone 397-0545
Address 697 SE Holly Terrace Lake City FL 32025
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address Freeman Design
Mortgage Lenders Name & Address
Circle the correct power company — FL Power & Light — Clay Elec. — Suwannee Valley Elec. — Progress Energy
Property ID Number 11-45-16-02911-343 Estimated Cost of Construction 100,000
Subdivision Name May farY Lot 43 Block Unit # Phase
Driving Directions 247 South right Into May Fair last job
on left SW Mayfair Lane
Number of Existing Dwellings on Property — O —
Construction of Total Acreage67 Lot Size
Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 17'2"
Actual Distance of Structure from Property Lines - Front 37 Side 3349 Side 3356 Rear 1944
Number of Stories Heated Floor Area 1600, 5 Total Floor Area 2406, Roof Pitch 6/12
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

0.2

386 7521284

Jan 09 00 01:54p Bishop Realty Columbia County Building Permit Application

NARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST NSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a light to enforce their claim for payment against your property. This claim is known as a construction lien. If your ight to enforce their claim for payment against your property. This claim is known as a construction lien. If your ight to enforce their claim for payment against your property. This claim is known as a construction lien. If your interest in falls to pay subcontractors or material suppliers or neglects to make other legally required payments, the seople who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor materials or other services which your contractor may have failed to pay.

AOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held esponsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in sending of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot or which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

<u>DWNERS CERTIFICATION</u>: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand he above written responsibilities in Columbia County for obtaining this Building Permit.

Robert Lean > Barbare 1. Xcrass	
Jwners Signature	The street of the street of the street of
written statement to the owner of all the above write	tand and agree that I have informed and provided this en responsibilities in Columbia County for obtaining
his Building Permit.	N
Test holds	Contractor's License Number 8R 282811503
Contractor's Signature (Permitee)	Competency Card Number 000141
Affirmed under penalty of parjury to by the Confractor and	d subscribed before the this TH day of January 2007.
Personally known X or Produced Identification	A CONTRACTOR OF THE PARTY OF TH
Elan K Jolan	SEAL:
State of Florida Notary Signature (For the Contractor)	EVAINE K. TOLAR ANY COMMISSION (DD 43632)

Page 2 of 2 (Both Pages must be submitted together.)

EXPIRES. October 2, 2009 stand True Normy President Standards

Revised 11-30-07

Columbia County Building Department Culvert Permit

Culvert Permit No. 000001519

DATE 01/1.	5/2008 PARCEL ID # 11-4		
APPLICANT	B. TRENT GIEBEIG	PHONE 386	.397.0545
ADDRESS _	697 SW HOLLY TERRACE	LAKE ITY	FL 32025
OWNER RO	DBERT & BARBARA SCRAGG	PHONE 386.	752.0791
ADDRESS _55	54 SW MAYFAIR LANE	LAKE CITY	FL 32024
CONTRACTO	R B. TRENT GIEBEIG	PHONE 386.	397.0545
LOCATION OF	F PROPERTY SR. 247-S TO MAYPFAIR S.D.,	TR AND IT'S LAST JOB ON	LEFT @
MAY-FAIR LANE			
SUBDIVISION/ SIGNATURE _	INSTALLATION REQUIREMENTS Culvert size will be 18 inches in diameter wird driving surface. Both ends will be mitered 4 thick reinforced concrete slab. INSTALLATION NOTE: Turnouts will be really a majority of the current and existing driving by the driveway to be served will be paved Turnouts shall be concrete or paved a miconcrete or paved driveway, whichever in current and existing paved or concreted. Culvert installation shall conform to the approximation.	foot with a 4 : 1 slope and quired as follows: iveway turnouts are pawdor formed with concrete hinimum of 12 feet wides greater. The width shaturnouts.	ed, or; ed, or; or the width of the Il conform to the
	Department of Transportation Permit instal	lation approved standard	ls.
	Other		

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



Prepared by: Michael H. Harrell Abstract & Title Services, Inc. 283 NW Cole Terrace Lake City, FL 32055

Warranty Deed

Individual to Individua

THIS WARRANTY DEED made the 1st day of June, 2006 by

Peter W. Glebeig, A Single Person

hereinafter called the grantor, to

Inst:2006013520 Date:06/06/2006 Time:11:53

Robert L. Scragg, and his wife, Barbara J. Scragg

Doc Stamp-Deed: 440.30

______DC,P.Dewitt Cason,Columbia County B:1085 P:2214

whose post office address is: 13 Edmond Place, Palm Coast, FL 32164-6344 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the grantee, all that certain land situate in COLUMBIA County, FLORIDA, viz: Parcel ID# P/O

Lot 43, May-Fair Unit 3, a subdivision according to the plat thereof filed in Plat Book 8, Pages 84-85, of the Public Records of Columbia County, Florida.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2005.

IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first above

Signed, sealed and delivered in our presence:

AINE

Printed Name

Peter W. Giebeig

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 1st day of June, 2006 by Peter W. Giebeig, A Single Person personally known to me or, if not personally known to for identification and who did not take an oath.

(SEAL)

DORIS M DRAKE MY COMMISSION # DD537517 EXPIRES: Apr. 5, 2010

My Commission Expires:

Notary Public

p. 2

Water Wells Pumps & Service Phone: (386) 752-6677 Fax: (386) 752-1477

Lynch Well Drilling, Inc.

173 SW Young Place Lake City, FL 32025 www.lynchwelldrilling.com

November 6, 2007

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the above-referenced well:

Size of Pump Motor:

1 Horse Power

Size of Pressure Tank:

81-Gallon Bladder Tank

Cycle Stop Valve Used:

No

Should you require any additional information, please contact us.

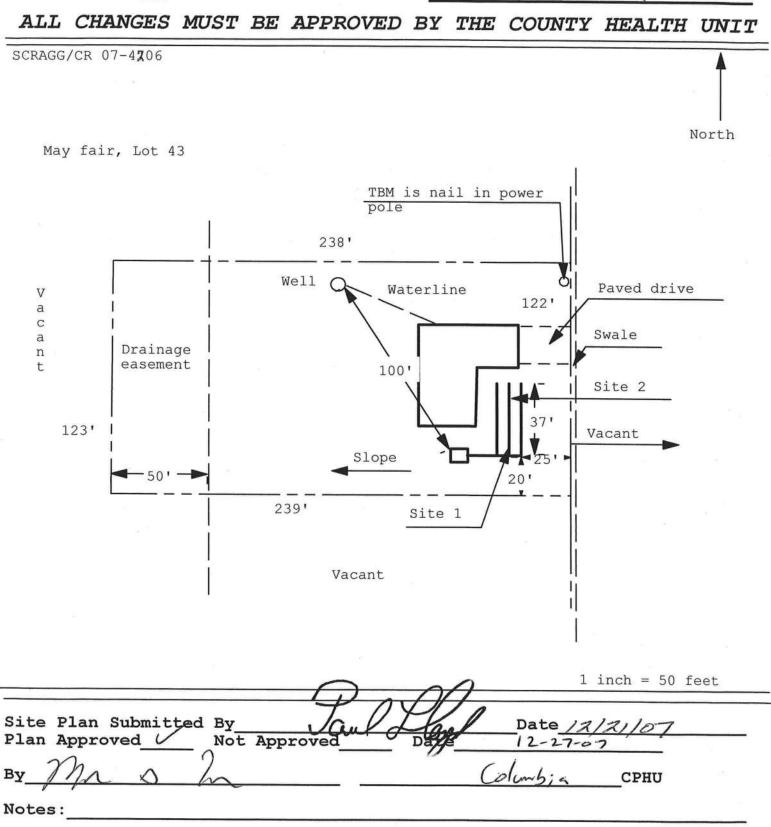
Sincerely,

Linda Newcomb

Lynch Well Drilling, Inc.

Linda Newcomb

Application	for	Onsite	Sewa	age	Dispo	osal	System
Construction	ı Pei	rmit.	Part	II	Site	Plan	_
Permit Appli	cat	ion Num	ber:		07-	1000	0



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Address: City, State: Owner:	ot#43 Mayfair Iorth	Builder: Permitting Office: Permit Number: Jurisdiction Number:	T. Geibeig Lake City
a. U-factor:	i-family Single family ulti-family 1	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump/Split b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	Cap: 32.0 kBtu/hr SEER: 13.00
Glass/F	Total as-built p Total base p	points: 17818 PAS	S
I hereby certify that the	plans and specifications covered by	Review of the plans and	THEST

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: (William)

DATE:

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT:

DATE:

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: ______
DATE: ____

SUMMER CALCULATIONS

ADDRESS: ,,,	PERMIT #:

BASE	AS	S-BUILT	
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area	Overhar Type/SC Ornt Le	ng en Hgt Area X SPM X SOF :	= Points
.18 1573.0 18.59 5264.0	2.Double, Clear W 1 3.Double, Clear E 1 4.Double, Clear E 1 5.Double, Clear N 1 6.Double, Clear N 1	1.0 6.0 45.0 38.52 0.97 1.0 6.0 10.0 38.52 0.97 1.0 6.0 24.0 42.06 0.97 1.0 6.0 30.0 42.06 0.97 1.0 6.0 5.0 19.20 0.98 1.0 6.0 8.0 19.20 0.98 1.0 6.0 15.0 19.20 0.98	1682.0 373.0 978.0 1223.0 93.0 149.0 280.0
	As-Built Total:	137.0	4778.0
WALL TYPES Area X BSPM = Points	Туре	R-Value Area X SPM =	Points
Adjacent 0.0 0.00 0.0 Exterior 1376.6 1.70 2340.2	1. Frame, Wood, Exterior	13.0 1376.6 1.50	2064.9
Base Total: 1376.6 2340.2	As-Built Total:	1376.6	2064.9
DOOR TYPES Area X BSPM = Points	Туре	Area X SPM =	Points
Adjacent 0.0 0.00 0.0 Exterior 59.4 6.10 362.3	1.Exterior Insulated 2.Exterior Insulated	33.0 4.10 26.4 4.10	135.3 108.2
Base Total: 59.4 362.3	As-Built Total:	59.4	243.5
CEILING TYPES Area X BSPM = Points	Type R-Va	alue Area X SPM X SCM =	Points
Under Attic 1573.0 1.73 2721.3	1. Under Attic	30.0 1573.0 1.73 X 1.00	2721.3
Base Total: 1573.0 2721.3	As-Built Total:	1573.0	2721.3
FLOOR TYPES Area X BSPM = Points	Туре	R-Value Area X SPM =	Points
Slab 234.0(p) -37.0 -8658.0 Raised 0.0 0.00 0.00	1. Slab-On-Grade Edge Insulation	0.0 234.0(p -41.20	-9640.8
Base Total: -8658.0	As-Built Total:	234.0	-9640.8
INFILTRATION Area X BSPM = Points		Area X SPM =	Points
1573.0 10.21 16060.3		1573.0 10.21	16060.3

SUMMER CALCULATIONS

ADDRESS: ,,,	PERMIT#:

	BASE		AS-BUILT	
Summer Ba	se Points: 1	8090.2	Summer As-Built Points:	16227.3
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	Cooling Points
18090.2	0.3250	5879.3	(sys 1: Central Unit 32000btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Int(AH),R6.0(INS) 16227 1.00 (1.00 x 1.147 x 0.91) 0.260 0.902 16227.3 1.00 1.044 0.260 0.902	3974.4 3974.4

WINTER CALCULATIONS

ADDRESS: , , ,	PERMIT #:

BASE	AS-BUILT
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X WPM X WOF = Point
.18 1573.0 20.17 5711.0	1.Double, Clear W 1.0 6.0 45.0 20.73 1.01 940.0
	2.Double, Clear W 1.0 6.0 10.0 20.73 1.01 208.0
	3.Double, Clear E 1.0 6.0 24.0 18.79 1.02 458.0
	4.Double, Clear E 1.0 6.0 30.0 18.79 1.02 572.0
Y .	5.Double, Clear N 1.0 6.0 5.0 24.58 1.00 122.0
	6.Double, Clear N 1.0 6.0 8.0 24.58 1.00 196.0
- 2 s	7.Double, Clear N 1.0 6.0 15.0 24.58 1.00 368.0
	As-Built Total: 137.0 2864.0
WALL TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points
Adjacent 0.0 0.00 0.0	1. Frame, Wood, Exterior 13.0 1376.6 3.40 4680.4
Exterior 1376.6 3.70 5093.4	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Base Total: 1376.6 5093.4	As-Built Total: 1376.6 4680.4
DOOR TYPES Area X BWPM = Points	Type Area X WPM = Points
Adjacent 0.0 0.00 0.0	1.Exterior Insulated 33.0 8.40 277.2
Exterior 59.4 12.30 730.6	2.Exterior Insulated 26.4 8.40 221.8
Base Total: 59.4 730.6	As-Built Total: 59.4 499.0
CEILING TYPES Area X BWPM = Points	Type R-Value Area X WPM X WCM = Points
Under Attic 1573.0 2.05 3224.6	1. Under Attic 30.0 1573.0 2.05 X 1.00 3224.6
Base Total: 1573.0 3224.6	As-Built Total: 1573.0 3224.6
FLOOR TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points
Slab 234.0(p) 8.9 2082.6	1. Slab-On-Grade Edge Insulation 0.0 234.0(p 18.80 4399.2
Raised 0.0 0.00 0.0	4000.2
Base Total: 2082.6	As-Built Total: 234.0 4399.2
INFILTRATION Area X BWPM = Points	Area X WPM = Points
1573.0 -0.59 -928.1	1573.0 -0.59 -928.1

WINTER CALCULATIONS

ADDRESS: ,,,	PERMIT #:

	BASE AS-BUILT				
Winter Base Points: 15914.2			Winter As-Built Points:	14739.2	
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	Heating Points	
15914.2	0.5540	8816.5	(sys 1: Electric Heat Pump 32000 btuh ,EFF(8.5) Ducts:Con(S),Con(R),Int(AF 14739.2 1.000 (1.000 x 1.169 x 0.93) 0.401 0.950 14739.2 1.00 1.087 0.401 0.950	f),R6.0 6107.0 6107.0	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT#:

BASE					AS-BUILT							
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multiplier	
3		2635.00		7905.0	20.0	0.94	3		1.00	2578.94	1.00	7736.8
	77				As-Built To	otal:						7736.8

	CODE COMPLIANCE STATUS											
	BASE							8	AS	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	S S				Total Points	
5879		8816		7905		22601	3974 6107 7737 17818					

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:
W. 200	

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 88.4

The higher the score, the more efficient the home.

1.	New construction or existing	New		2. Cooling systems		
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 32.0 kBtu/hr	_
3.	Number of units, if multi-family	1	_	**-	SEER: 13.00	_
4.	Number of Bedrooms	3	_	b. N/A		_
5.	Is this a worst case?	Yes				_
6.	Conditioned floor area (ft²)	1573 ft²	_	c. N/A		_
7.	Glass type 1 and area: (Label reqd.)	by 13-104.4.5 if not default)				_
a.	U-factor:	Description Area	1.	Heating systems		
	(or Single or Double DEFAULT)	7a. (Dble Default) 137.0 ft ²	_	a. Electric Heat Pump/Split	Cap: 32.0 kBtu/hr	_
b.	SHGC:				HSPF: 8.50	
	(or Clear or Tint DEFAULT)	7b. (Clear) 137.0 ft ²	_	b. N/A		2000
8.	Floor types					23-32
a.	Slab-On-Grade Edge Insulation	R=0.0, 234.0(p) ft		c. N/A		-
b.	N/A					====
c.	N/A		14	4. Hot water systems		===
9.	Wall types		2000 c	a. Electric Resistance	Cap: 20.0 gallons	
	Frame, Wood, Exterior	R=13.0, 1376.6 ft ²			EF: 0.94	
	N/A			b. N/A		
c.	N/A					
d.	N/A		_	c. Conservation credits		_
e.	N/A		-	(HR-Heat recovery, Solar		
10.	Ceiling types		-	DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 1573.0 ft ²	14	5. HVAC credits	PT, CF,	
	N/A	10 30.0, 1373.011		(CF-Ceiling fan, CV-Cross ventilation	100	-
	N/A		-	HF-Whole house fan,	•	
	Ducts		-	PT-Programmable Thermostat,		
	Sup: Con. Ret: Con. AH: Interior	Sup. R=6.0, 53.0 ft		MZ-C-Multizone cooling,		
	N/A	Sup. K-0.0, 55.0 II				
D.	N/A			MZ-H-Multizone heating)		
	rtify that this home has complie				THE STAN	3
	struction through the above ene				A TO	B
	his home before final inspection and on installed Code compliant		Display C	ard will be completed		E S
Buil	der Signature:		Date: _		ES THE	D
Add	ress of New Home:		City/FL	Zip:	COD WE TRUST	

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTMdesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

BUILDING INPUT SUMMARY REPORT

PROJECT	4	Owner: (blank) # of Units: 1 Builder Name: T. Geit Climate: North Permit Office: Lake C Jurisdiction #: (blank)	peig	Family Type: New/Existing: Bedrooms: Conditioned A Total Stories: Worst Case: Rotate Angle:	\rea:	New Lo 3 Si 1573 PI 1 Si Yes Co 270 Ci	ddress Type: ot #: ubdivision: atbook: creet: ounty: ty, St, Zip:	Street Address N/A N/A N/A (blank) Columbia	1
FLOORS	1	Floor Type Slab-On-Grade Edge Insula		ea/Perimeter Units 4.0(p) ft 1	DOORS	# Door Type 1 Insulated 2 Insulated	Orientation Exterior Exterior	33.0 ft ² 26.4 ft ²	Units 1 1
CEILINGS	1	Ceiling Type Under Attic	R-Val Area 30.0 1573.	Base Area Units 0 ft² 1573.0 ft² 1	COOLING	# System Type 1 Central Unit		Efficiency SEER: 13.00	Capacity 32.0 kBtu/hr
ပ	Cre	edit Multipliers: None			O	Credit Multipliers: Ce	il Fn, PT		
WALLS	1	Wall Type Frame - Wood		R-Val Area Units 13.0 1376.6 ft ² 1	HEATING	# System Type 1 Electric Heat Pump/S	plit	Efficiency HSPF: 8.50	Capacity 32.0 kBtu/hr
					ㅗ	Credit Multipliers: PT			
	# 1 2 3 4	Double Clear Double Clear Double Clear Double Clear	N 15.0 ft ² 1 N 5.0 ft ² 1 S 24.0 ft ² 1 S 30.0 ft ² 1	ength OH Hght Units .0 ft 6.0 ft 3 .0 ft 6.0 ft 2 .0 ft 6.0 ft 1 .0 ft 6.0 ft 1	DUCTS	# Supply Return Location 1 Cond. Cond.	Air Handler Location Interior	Supply R-Val 6.0	Supply Length 53.0 ft
	5	Double Clear Double Clear		.0 ft 6.0 ft 1		Credit Multipliers: No	ne		
	7	Double Clear	E 15.0 ft ² 1	.0 ft 6.0 ft 1	WATER	# System Type 1 Electric Resistance	EF Cap. 0.94 20.0	Conservation Ty None	vpe Con. EF 0.00
WINDOWS					REFR.	# Use Default? 1 Yes	Annual Operat	ing Cost Electr	ic Rate
MISC	,	Rater Name: Rater Certification #: Area Under Fluorescent Area Under Incandesce NOTE: Not all Rating in	nt: 1573.0	Class #: Duct Leakage Type Visible Duct Disco Leak Free Duct Sys HRV/ERV System F	nnect stem	Proposed: No	. I . I	Pool Size: 0 Pump Size: 0.0 Dryer Type: Ele Stove Type: Ele Avg Ceil Hgt:	ectric

EnergyGauge® (Version: FLRCPB v4.5.2)

Residential System Sizing Calculation

Summary

Project Title: Lot#43 Mayfair

Code Only Professional Version Climate: North

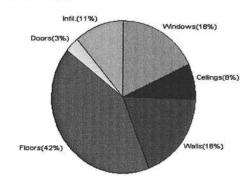
1/7/2008

				17772000					
Location for weather data: Gaine	sville - Def	aults: Latiti	ude(29) Altitude(152 ft.) Temp Ran	ge(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)									
Winter design temperature	33	F	Summer design temperature	92	F				
Winter setpoint	70	F	Summer setpoint	75	F				
Winter temperature difference	37	F	Summer temperature difference	17	F				
Total heating load calculation	24489	Btuh	Total cooling load calculation 17461						
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh				
Total (Electric Heat Pump)	130.7	32000	Sensible (SHR = 0.75)	151.6	24000				
Heat Pump + Auxiliary(0.0kW)	130.7	32000	Latent	491.9	8000				
10 (10 (10 (10 (10 (10 (10 (10 (10 (10 (Total (Electric Heat Pump)	183.3	32000				

WINTER CALCULATIONS

Winter Heating Load (for 1573 sqft)

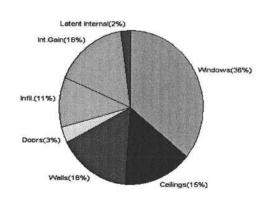
Load component			Load	
Window total	137	sqft	4410	Btuh
Wall total	1377	sqft	4521	Btuh
Door total	59	sqft	769	Btuh
Ceiling total	1573	sqft	1854	Btuh
Floor total	234	sqft	10216	Btuh
Infiltration	67	cfm	2719	Btuh
Duct loss			0	Btuh
Subtotal			24489	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			24489	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1573 sqft)

Load component			Load	
Window total	137	sqft	6291	Btuh
Wall total	1377	sqft	2871	Btuh
Door total	59	sqft	582	Btuh
Ceiling total	1573	sqft	2605	Btuh
Floor total		7.6	0	Btuh
Infiltration	34	cfm	625	Btuh
Internal gain			2860	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			15834	Btuh
Latent gain(ducts)		- 1	0	Btuh
Latent gain(infiltration)			1226	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occup	oants/othe	r)	400	Btuh
Total latent gain			1626	Btuh
TOTAL HEAT GAIN			17461	Btuh



Version 8
For Florida residences only

EnergyGauge® System Sizing

PREPARED BY: Notation Company

DATE: 1-7-08

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Project Title: Lot#43 Mayfair

Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F This calculation is for Worst Case. The house has been rotated 90 degrees.

1/7/2008

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	E	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	E	10.0	32.2	322 Btuh
3	2, Clear, Metal, 0.87	W	24.0	32.2	773 Btuh
4	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btuh
5	2, Clear, Metal, 0.87	S	5.0	32.2	161 Btuh
6	2, Clear, Metal, 0.87	S	8.0	32.2	258 Btuh
7	2, Clear, Metal, 0.87	S	15.0	32.2	483 Btuh
	Window Total		137(sqft)		4410 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1377	3.3	4521 Btuh
407	Wall Total		1377	7.5580410VX	4521 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		33	12.9	427 Btuh
2	Insulated - Exterior		26	12.9	342 Btuh
	Door Total		59	7.5	769Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1573	1.2	1854 Btuh
	Ceiling Total		1573		1854Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	234.0 ft(p)	43.7	10216 Btuh
	Floor Total		234		10216 Btuh
			Envelope Si	ubtotal:	21770 Btuh
Infiltration	Туре	ACH X Volu	ume(cuft) walls(sq	ft) CFM=	
	Natural	0.32	12584 1377	67.1	2719 Btuh
Ductload			(□	DLM of 0.000)	0 Btuh
All Zones		Sens	sible Subtotal A	II Zones	24489 Btuh

WHOLE HOUSE TOTALS

Subtotal Sensible	24489 Btuh
Ventilation Sensible	0 Btuh
Total Btuh Loss	24489 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title: Lot#43 Mayfair Code Only Professional Version Climate: North

1/7/2008

EQUIPMENT

1. Electric Heat Pump/Split

#(Outside) #(Inside)

32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details Project Title: Code C

Lot#43 Mayfair

Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F This calculation is for Worst Case. The house has been rotated 90 degrees.

1/7/2008

A			
Component	Loads	for Zone	#1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	E	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	E	10.0	32.2	322 Btuh
3	2, Clear, Metal, 0.87	W	24.0	32.2	773 Btuh
4	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btuh
5	2, Clear, Metal, 0.87	S	5.0	32.2	161 Btuh
6	2, Clear, Metal, 0.87	S	8.0	32.2	258 Btuh
7	2, Clear, Metal, 0.87	S	15.0	32.2	483 Btuh
	Window Total		137(sqft)		4410 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1377	3.3	4521 Btuh
	Wall Total		1377		4521 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		33	12.9	427 Btuh
2	Insulated - Exterior		26	12.9	342 Btuh
	Door Total		59		769Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1573	1.2	1854 Btuh
	Ceiling Total		1573		1854Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	234.0 ft(p)	43.7	10216 Btuh
	Floor Total		234		10216 Btuh
		z	one Envelope Su	btotal:	21770 Btuh
Infiltration	Туре	ACH X Volu	ume(cuft) walls(sqft) CFM=	
	Natural	0.32	12584 1377	67.1	2719 Btuh
Ductload	Average sealed, Supply(R6.	0-Cond.), Retu	ırn(R6.0-Cond)DI	LM of 0.000)	0 Btuh
Zone #1		Sens	sible Zone Subto	otal	24489 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title: Lot#43 Mayfair Code Only Professional Version

Climate: North

1/7/2008

	Subtotal Sensible Ventilation Sensible Total Btuh Loss	24489 Btuh 0 Btuh 24489 Btuh
--	--	------------------------------------

1. Electric Heat Pump/Split

#(Outside) #(Inside)

32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Project Title: Lot#43 Mayfair

Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

1/7/2008

This calculation is for Worst Case. The house has been rotated 90 degrees.

Component Loads for Whole House

	Type*		Over	hang	Win	dow Area	a(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		0 0 0	Shaded	Unshaded		
1	2, Clear, 0.87, B-D, N,F	E	1ft.	6ft.	45.0	0.0	45.0	19	55	2495	Btuh
2	2, Clear, 0.87, B-D, N,F	E	1ft.	6ft.	10.0	0.0	10.0	19	55	554	Btuh
3	2, Clear, 0.87, B-D, N,F	W	1ft.	6ft.	24.0	3.3	20.7	19	55	1208	Btuh
4	2, Clear, 0.87, B-D, N,F	W	1ft.	6ft.	30.0	4.2	25.9	19	55	1511	
5	2, Clear, 0.87, B-D, N,F	S	1ft.	6ft.	5.0	5.0	0.0	19	23	93	Btuh
6	2, Clear, 0.87, B-D, N,F	S	1ft.	6ft.	8.0	8.0	0.0	19	23	149	Btuh
7	2, Clear, 0.87, B-D, N,F	S	1ft.	6ft.	15.0	15.0	0.0	19	23	280	Btuh
	Window Total				137 (sqft)				6291	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	137	76.6		2.1	2871	Btuh
	Wall Total				515050		7 (sqft)			2871	
Doors	Туре						(sqft)		НТМ	Load	Dian
1	Insulated - Exterior						3.0		9.8	323	Btuh
2	Insulated - Exterior						5.4		9.8		Btuh
	Door Total		59 (sqft)			5.0		Btuh			
Ceilings	Type/Color/Surface		R-Value Area(sqft)			нтм	Load	Dian			
1	Vented Attic/DarkShingle		30.0			1573.0			1.7		Disk
	Ceiling Total						1.7		Btuh		
Floors		_	R-Value 1573 (sqft)			LITTA	2605	Btun			
	Type		K-V						HTM	Load	220 12
1	Slab On Grade			0.0				0.0		Btuh	
	Floor Total					234.	0 (sqft)			0	Btuh
						E	nvelope	Subtota	:	12350	Btuh
Infiltration	Туре		Δ	СН	Volum	e(cuft) v	wall area	(saft)	CFM=	Load	
	SensibleNatural		,	0.16	Volulli	12584	1377	(Sqit)	67.1	625	Btuh
Internal		(Occup	1000		310000000000000000000000000000000000000	cupant	-	Appliance	Load	Diuli
gain				2		X 23			2400	2860	Btuh
9											
						Se	ensible E	nvelope	e Load:	15834	Btuh
Duct load							(DGI	M of 0.0	00)	0	Btuh
						Ser	nsible Lo	ad All	Zones	15834	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)
Project Title:

Lot#43 Mayfair

Code Only **Professional Version** Climate: North

1/7/2008

WHOLE HOUSE TOTALS

		1	
	Sensible Envelope Load All Zones	15834	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	15834	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	15834	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	1226	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (2 people @ 200 Btuh per person)	400	Btuh
	Latent other gain	0	Btuh
	Latent total gain	1626	Btuh
	TOTAL GAIN	17461	Btuh

EQUIPMENT		
1. Central Unit	#	32000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default) (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value) (BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)

Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details Project Title: Code C

Lot#43 Mayfair

Professional Version Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F This calculation is for Worst Case. The house has been rotated 90 degrees.

1/7/2008

Component Loads for Zone #1: Main

	Type*		Over	hang	Win	dow Area	a(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, B-D, N,F	E	1ft.	6ft.	45.0	0.0	45.0	19	55	2495	Btuh
2	2, Clear, 0.87, B-D, N,F	E	1ft.	6ft.	10.0	0.0	10.0	19	55	554	Btuh
3	2, Clear, 0.87, B-D, N,F	W	1ft.	6ft.	24.0	3.3	20.7	19	55	1208	Btuh
4	2, Clear, 0.87, B-D, N,F	W	1ft.	6ft.	30.0	4.2	25.9	19	55	1511	Btuh
5 6 7	2, Clear, 0.87, B-D, N,F	S	1ft.	6ft.	5.0	5.0	0.0	19	23	93	Btuh
6	2, Clear, 0.87, B-D, N,F	S	1ft.	6ft.	8.0	8.0	0.0	19	23	149	Btuh
7	2, Clear, 0.87, B-D, N,F	S	1ft.	6ft.	15.0	15.0	0.0	19	23	280	Btuh
	Window Total				137 (sqft)				6291	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		НТМ	Load	
1	Frame - Wood - Ext			13.0/	0.09	137	76.6		2.1	2871	Btuh
	Wall Total					137	7 (sqft)			2871	Btuh
Doors	Туре -						(sqft)		НТМ	Load	
1	Insulated - Exterior					33	3.0		9.8	323	Btuh
2	Insulated - Exterior						5.4		9.8	259	
	Door Total		59 (sqft)				582	Btuh			
Ceilings	Type/Color/Surface		R-Value			Area(sqft)			HTM	Load	
1	Vented Attic/DarkShingle		30.0				3.0	1.7		2605	Btuh
	Ceiling Total				1573 (sqft)				2605		
Floors	Туре		R-Va	alue			ze		HTM	Load	
1	Slab On Grade			0.0		23	34 (ft(p))		0.0	0	Btuh
	Floor Total						0 (sqft)		E-17.	0	Btuh
						Z	one Enve	elope Su	ubtotal:	12350	Btuh
Infiltration	Туре		Δ	CH	Volum	e(cuft) y	wall area	(eaft)	CFM=	Load	
	SensibleNatural			0.16	VOIGITI	12584	1377	(Sqit)	33.6	625	Btuh
Internal		(Occup	Seattle to the second		Btuh/oc	cupant	-	Appliance	Load	
gain				2		X 23			2400	2860	Btuh
						S	ensible E	nvelope	Load:	15834	
Duct load	Average sealed, Supply	(R6.0-0	Cond.), Ret	urn(R6.	0-Cond)	(DGM c	of 0.000)	0	Btuh
							Sensib	le Zone	Load	15834	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Lot#43 Mayfair

Code Only Professional Version Climate: North

1/7/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones Sensible Duct Load	15834	
	Total Sensible Zone Loads	15834	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	15834	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	1226	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (2 people @ 200 Btuh per person)	400	Btuh
	Latent other gain	0	Btuh
	Latent total gain	1626	Btuh
	TOTAL GAIN	17461	Btuh

EQUIPMENT		
1. Central Unit	#	32000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

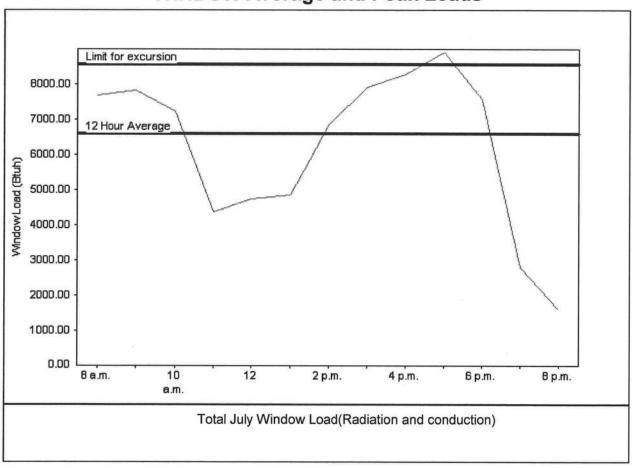
Project Title: Lot#43 Mayfair

Code Only Professional Version Climate: North

1/7/2008

Weather data for: Gainesville - Defaults					
Summer design temperature	92 F	Average window load for July	6577 Btuh		
Summer setpoint	75 F	Peak window load for July	8894 Btuh		
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	8550 Btuh		
Latitude	29 No	rth Window excursion (July)	344 Btuh		

WINDOW Average and Peak Loads



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: Libriday

DATE: 1-7-09

ACCA HRKURL J

EnergyGauge® FLRCPB v4.5.2



BRITT SURVEYING

830 West Duval Street • Lake City, FL 32055 Phone (386) 752-7163 • Fax (386) 752-5573

01/29/08

L-19060

To Whom It May Concern:

C/o: Trent Giebeig

Re: Lot 43 Mayfair Unit 3

The elevation of the foundation is found to be 154.20 feet. The recommended finished floor elevation is 149.00 feet as per the construction plans for Mayfair Unit 3. The highest adjacent grade is 152.57 feet on the building pad. The highest adjacent natural grade is 150.38 feet and the lowest adjacent grade is 149.80 feet. The centerline of the adjacent road SW Mayfair Lane is 154.10 feet. The elevations shown hereon are based on NGVD 29 Datum.

L. Scott Britt PLS #5757 Columbia County Building Department

Cu

Culvert Waiver

Cu

Culvert Waiver No. 000001519

DATE: 03/03/2008 BUILDING PERMIT NO.	26615		
APPLICANT B. TRENT GIEBEIG	PHONE 386	.397.0545	
ADDRESS 697 SW HOLLY TERRACE	LAKE ITY	FL	32025
OWNER ROBERT & BARBARA SCRAGG	PHONE 386.7	52.0791	
ADDRESS 554 SW MAYFAIR LANE	LAKE CITY	FL	32024
CONTRACTOR B. TRENT GIEBEIG	PHONE 386.3	397.0545	
LOCATION OF PROPERTY SR. 247-S TO MAYPFAIR S.D.,T	R AND IT'S LAST JOB ON	LEFT @	
MAY-FAIR LANE	-		
SUBDIVISION/LOT/BLOCK/PHASE/UNITMAY-FAIR		43	3
PARCEL ID # 11-4S-16-02911-343			
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC	Amount Pai	d _50.0	0
PUBLIC WORKS DEPARTMENT	NT USE ONLY		
HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION CULVERT WAIVER IS:	ON AND DETERMINED T	HAT THE	
APPROVED			
	NOT APPROVED -	NEEDS A	CULVERT PERMIT
COMMENTS:	NOT APPROVED -	NEEDS A	CULVERT PERMIT
P 11-1	NOT APPROVED -		CULVERT PERMIT

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160







OCCUPANCY

COLUMBIA COUNTY, FLORIDA

partment of Building and Zoning Inspection

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code. This Certificate of Occupancy is issued to the below named permit holder for the building

Parcel Number 11-4S-16-02911-343

Use Classification SFD/UTILITY

Building permit No. 000026615

Fire:

38.52

Permit Holder B. TRENT GIEBEIG

Waste: 100.50

Owner of Building ROBERT & BARBARA SCRAGG

Total: 139.02

Location: 554 SW MAY-FAIR LANE, LAKE CITY, FL

Date: 04/09/2008

yne X. Russ

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Clopay Building Products Co. 8585 Duke Blvd. Mason, OH 45040

SCOPE: This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code. This product is approved as described herein, and has been designed to comply with the Florida Building Code including the High Velocity Hurricane Zone.

DESCRIPTION: Sectional Garage Door 16'- 2" Wide.

APPROVAL DOCUMENT: Drawing No. 101300, titled "Double Car Hurricane Pan Door", dated 02/15/95 with last revision on 01/06/04, sheets 1 and 2 of 2, prepared by Clopay Building Products Co, signed and sealed by M. W. Westerfield, P.E., bearing the Miami-Dade County Product Control Revision stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

LIMITATION: This approval requires the manufacturer to do testing of all coils used to fabricate door panels under this Notice of Acceptance. A minimum of 2 specimens shall be cut from each coil and tensile tested according to ASTM E-8 by a Dade County approved laboratory selected and paid by the manufacturer. Every 3 months, four times a year, the manufacturer shall mail to this office: a copy of the tested reports with confirmation that the specimen were selected from coils at the manufacturer production facilities. And a notarized statement from the manufacturer that only coils with yield strength of 38000 psi or more shall be used to make door panels for Dade County under this Notice of Acceptance

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be remainded to the normal data of the normal data.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA renews NOA # 03-0829.05 and consists of this page, evidence page as well as the approval document mentioned above.

03/23/06

The submitted documentation was reviewed by Candido F. Font PE.

NOA No 05-1212.02 Expiration Date: March 26, 2007 Approval Date: March 23, 2006

Page 1

Clopay Building Products Co.

NOTICE OF ACCEPTANCE: EVIDENCE PAGE

A. DRAWINGS

1. Drawing prepared by Clopay Building Products Co., titled "Double Car Hurricane Pan Door", Drawing No. 101300, dated 02/15/95, with last revision on 01/06/2004, sheets 1 through 2 of 2, signed and sealed by M.W. Westerfield, PE.

B. TESTS

- 1. Test report of large missile impact test per PA 201 and cyclic wind pressure test per PA 203 of "Garage Door", prepared by Hurricane Engineering & Testing, Inc., report No. HETI 95-408, dated 01/25/95, signed and sealed by H. M. Medina, PE.
- 2. Test report of Uniform Static Air Pressure Test Per PA 202 on "Garage Door", prepared by Hurricane Engineering & Testing, Inc., report No. HETI 95-407, dated 01/24/95, signed and sealed by H. M. Medina, PE.
- 3. Test report of Forced Entry Resistance per section 3603.2(b)5 on "Garage Door" prepared by Hurricane Engineering Testing, Inc. report No. HETI 95-407f, dated 01/25/95, signed and sealed by H. M. Medina, PE.

C. CALCULATIONS

- 1. Calculations dated 01/20/95; pages 1 and 2, prepared by M. W. Westerfield, PE, signed and sealed by M. W. Westerfield, PE.
- 2. Calculations dated 02/24/95, page 1, prepared M.W. Westerfield, PE, signed and sealed by M.W. Westerfield, PE.

D. MATERIAL CERTIFICATIONS

- 1. Test report of Tensile Test per ASTM E 8, report No. HETI 94-T59, prepared by Hurricane Engineering & Testing, Inc., dated 02/06/95, signed and sealed by H.M. Medina, PE.
- 2. Test report of Salt Spray Test per ASTM D1654 & ASTM B117, report No. 9EM-1144, prepared by Q.C. Metallurgical, Inc., dated 06/03/99, signed and sealed by K. Grate.

E. STATEMENTS.

1. Affidavit of yield strength compliance prepared by R. D. Shifflett employed by Clopay Building Products Co., notarized on 01/11/2001 by B. H. Schuler.

F. QUALITY ASSURANCE.

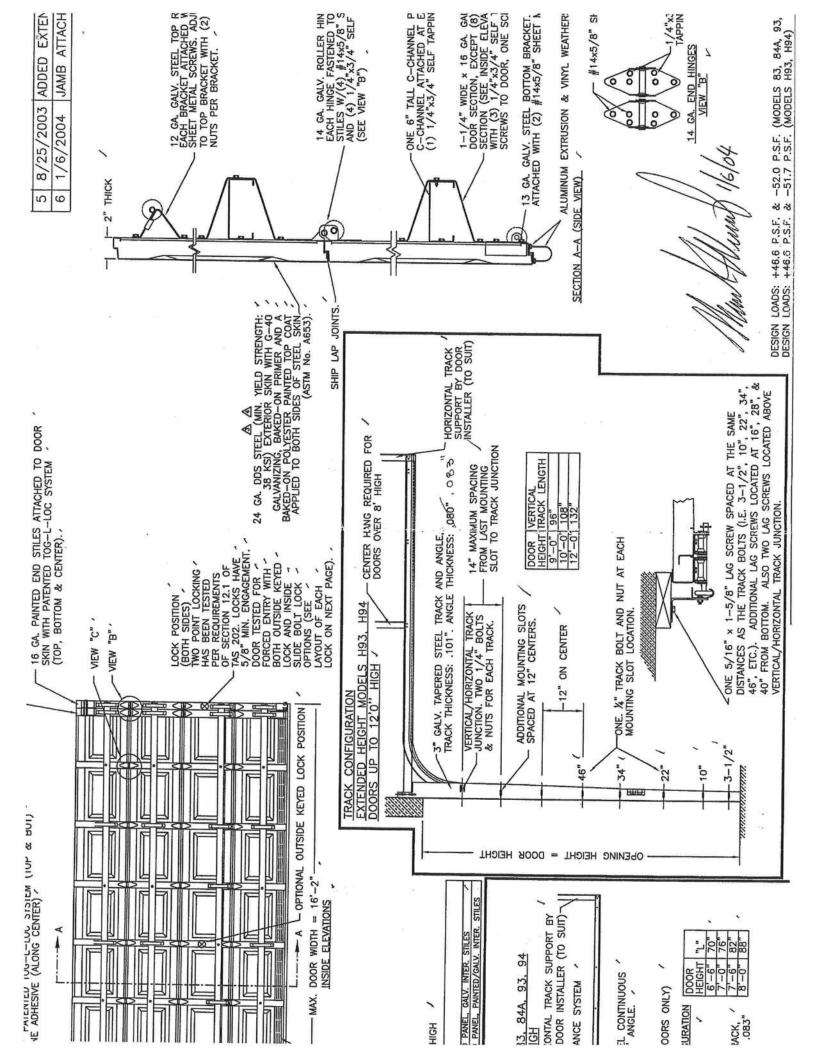
1. Building Code Compliance Office.

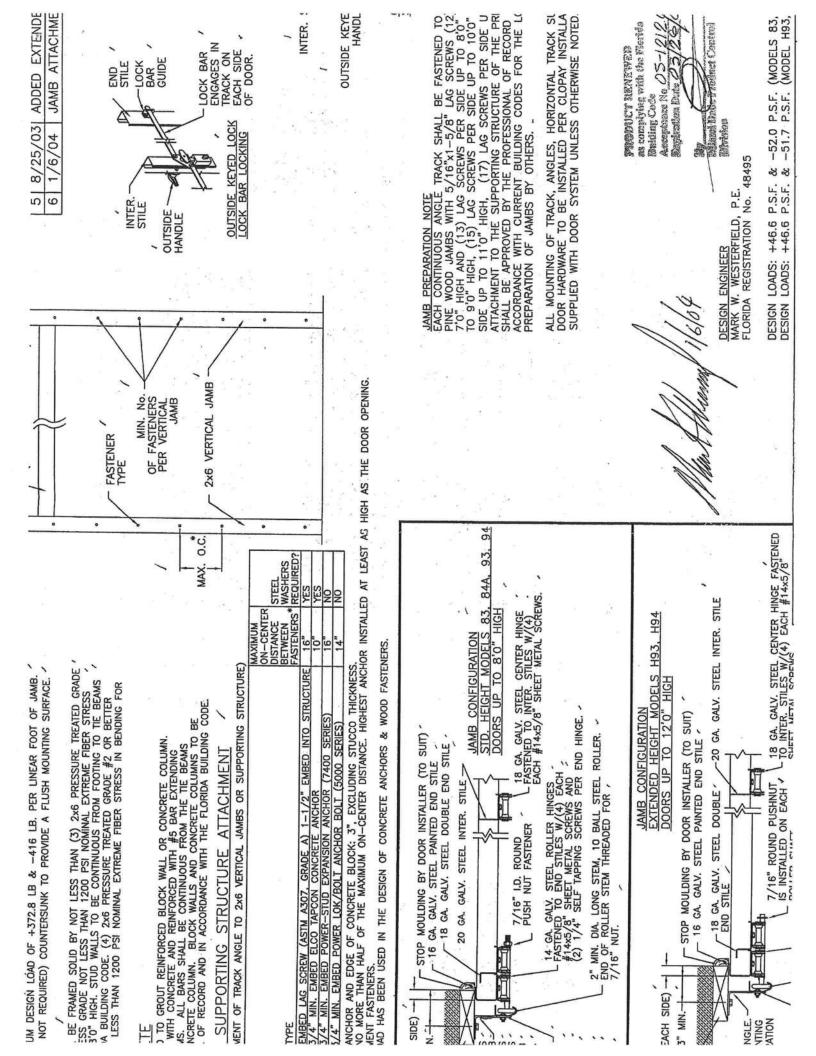
duct Control Division

Senior Product Control Division

NOA No 05-1212.02 Expiration Date: March 26, 2007

Approval Date: March 23, 2006





MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

MI Home Products, Inc. 650 West Market Street Gratz, PA 17030

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the Florida Building Code, including the High Velocity Hurricane Zone.

DESCRIPTION: Series "BetterBilt D185SH/D3185SH" Aluminum Single Hung Window

APPROVAL DOCUMENT: Drawing No. S-2422, titled "Non-Impact Single Hung Window Rectangle Circle Top & Oriel", sheets 1 through 5 of 5, prepared by RW Building Consultants, inc, dated 10/27/03 with revision "2", dated 02/10/04, signed and sealed by Wendell Haney, P.E., bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

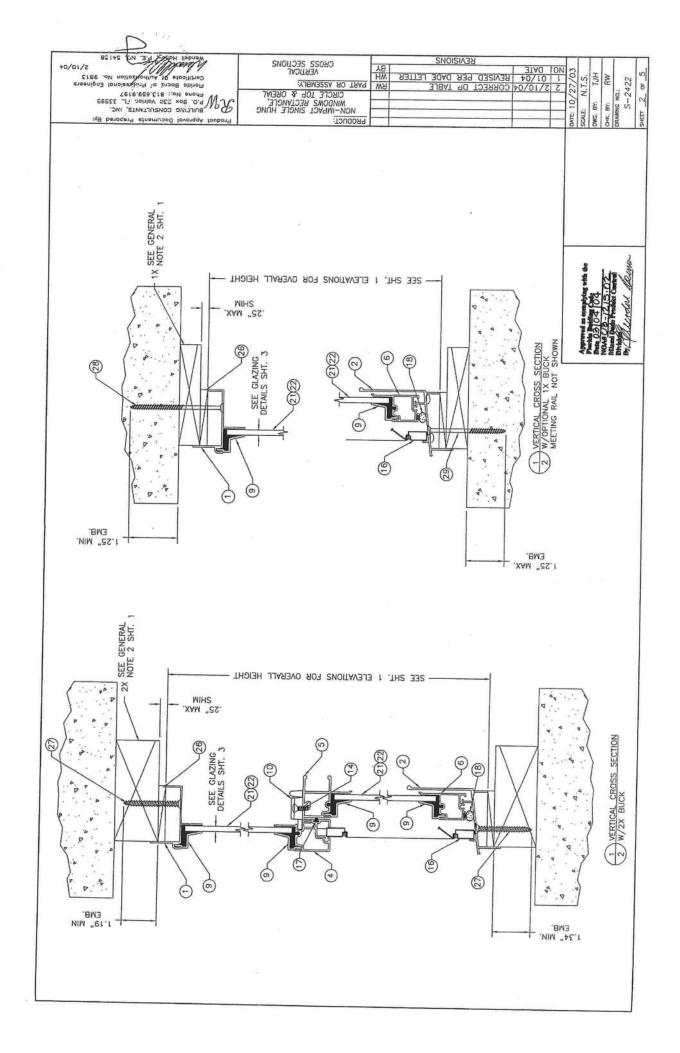
TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

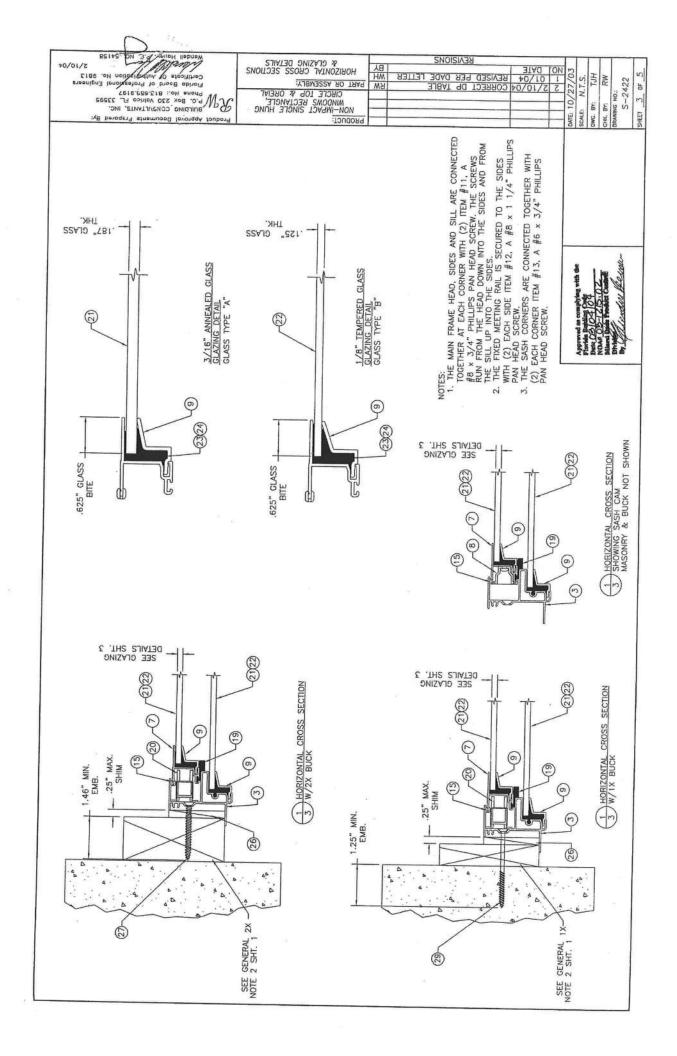
ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

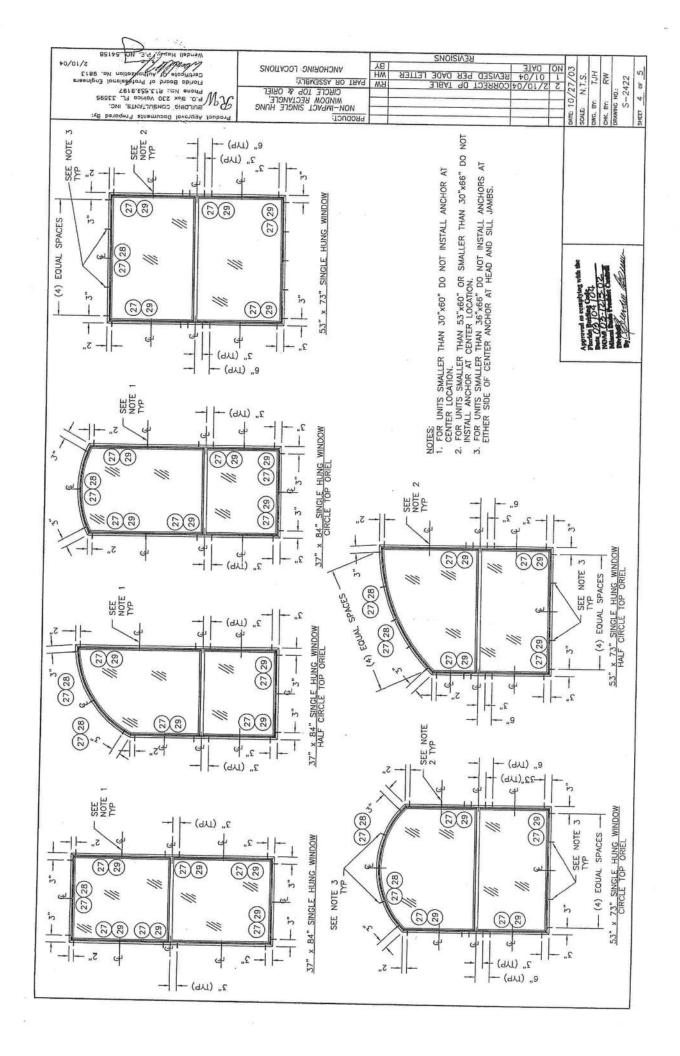
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

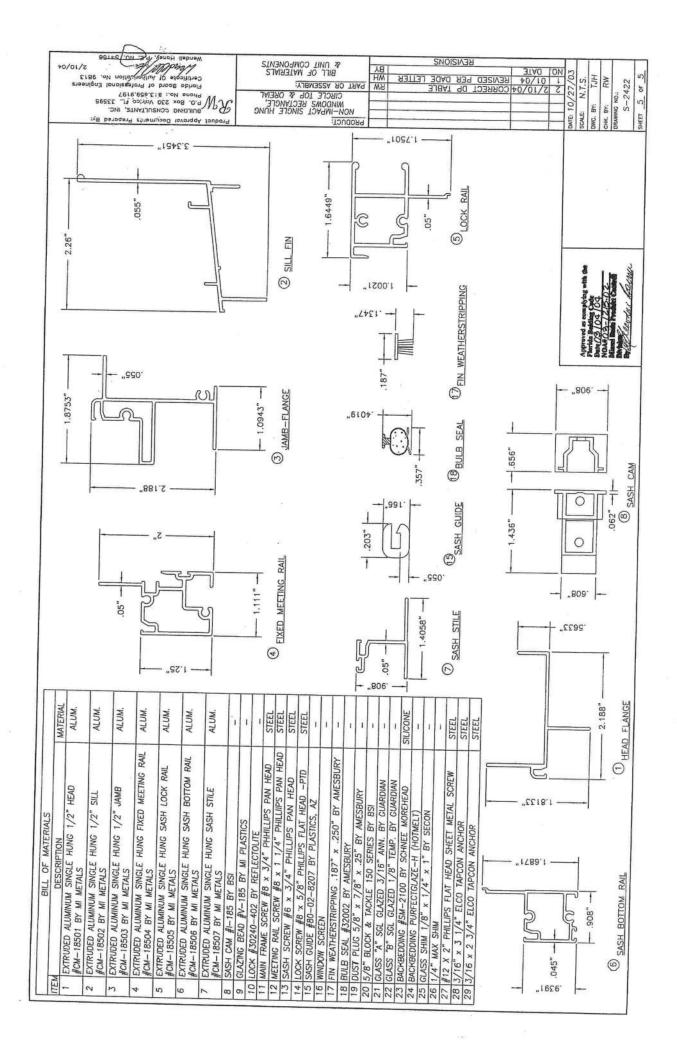
This NOA consists of this page 1 and evidence page E-1, as well as approval document mentioned above. The submitted documentation was reviewed by **Theodore Berman**, **P.E.**

NOA No 03-1215.02 Expiration Date: March 04, 2009 Approval Date: March 04, 2004 Page 1









MI Home Products, Inc.

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

A. DRAWINGS

Manufacturer's die drawings and sections.

Drawing No. S-2422, titled "Non-Impact Single Hung Window Rectangle Circle Top & Oriel", sheets 1 through 5 of 5, prepared by RW Building Consultants, inc, dated 10/27/03 with revision "2", dated 02/10/04, signed and sealed by Wendell Haney, P.E.

B. TESTS

1. Test reports on 1) Air Infiltration Test, per FBC, TAS 202-94

- 2) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
- 3) Water Resistance Test, per FBC, TAS 202-94

4) Forced Entry Test, per FBC 2411.3.2.1 and TAS 202-94

along with marked-up drawings and installation diagram of an aluminum single hung window, prepared by Architectural Testing, Inc., Test Report No. ATI 03056, dated 11/11/03, signed by Joseph A. Reed, P.E.

C. CALCULATIONS

- Anchor Calculations, ASTM-E1300-98, and structural analysis, prepared by R.W. Building Consultants, Inc., dated 12/11/03, signed and sealed by Lyndon F. Schmidt, P.E.
- 2. Revised Anchor Calculations, and structural analysis, prepared by R.W. Building Consultants, Inc., dated 02/10/04, signed and sealed by Lyndon F. Schmidt, P.E.

D. QUALITY ASSURANCE

1. Miami Dade Building Code Compliance Office (BCCO).

E. MATERIAL CERTIFICATIONS

1. None.

F. STATEMENTS

 Statement letter of conformance and no financial interest, dated December 09, 2003, signed and sealed by Lyndon F. Schmidt, P.E.

2. Statement letter of no financial interest with the laboratory that performed the Test Report No. **ATI 03056**, dated November 08, 2003, signed by Stu White, Design Engineering Manager.

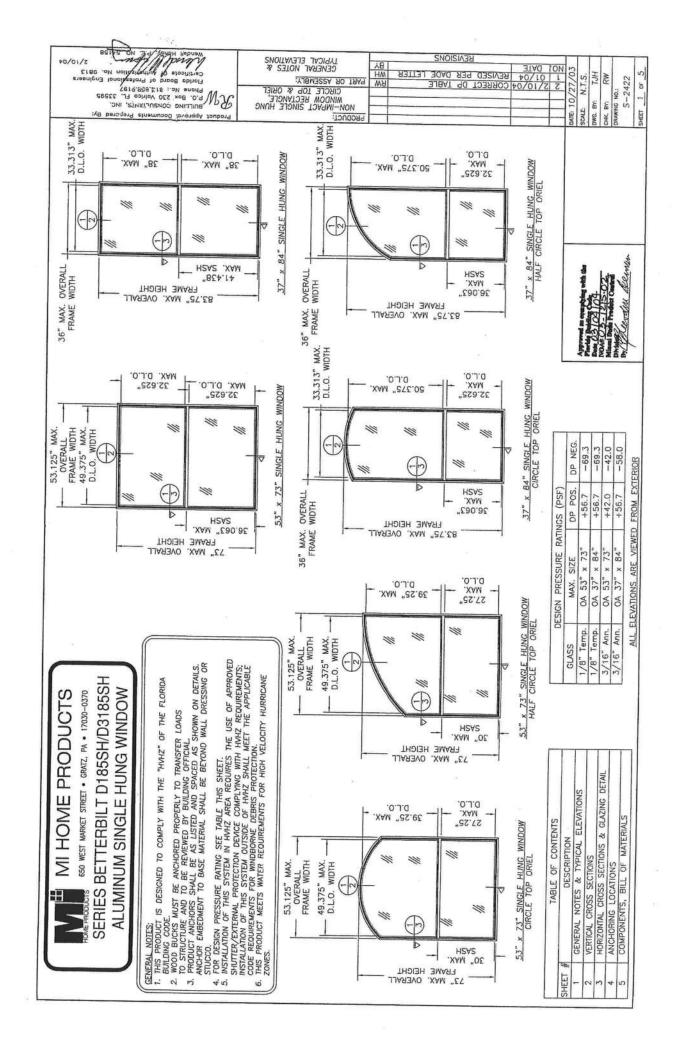
G. OTHER

1. Letter from the consultant stating that the product is in compliance with the Florida Building Code (FBC).

Theodore Berman, P.E. Deputy Director, Product Control Division

NOA No 03-1215.02

Expiration Date: March 04, 2009 Approval Date: March 04, 2004





MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corporation 108 Mutzfeld Road Butler, IN 46721

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by the BCCO and accepted by the Building Code and Product Review Committee (BCPRC) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The BCCO (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BCPRC reserves the right to revoke this acceptance, if it is determined by BCCO that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the South Florida Building Code, 1994 Edition for Miami-Dade County or Florida Building Code.

DESCRIPTION: Outswing Glazed Residential Steel Door w\Sidelites

APPROVAL DOCUMENT: Drawing No. S-2003, titled "Therma-Tru Wood edge Outswing", sheets 1 through 6 to 6, prepared by RW Consulting, dated 3/9/01, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

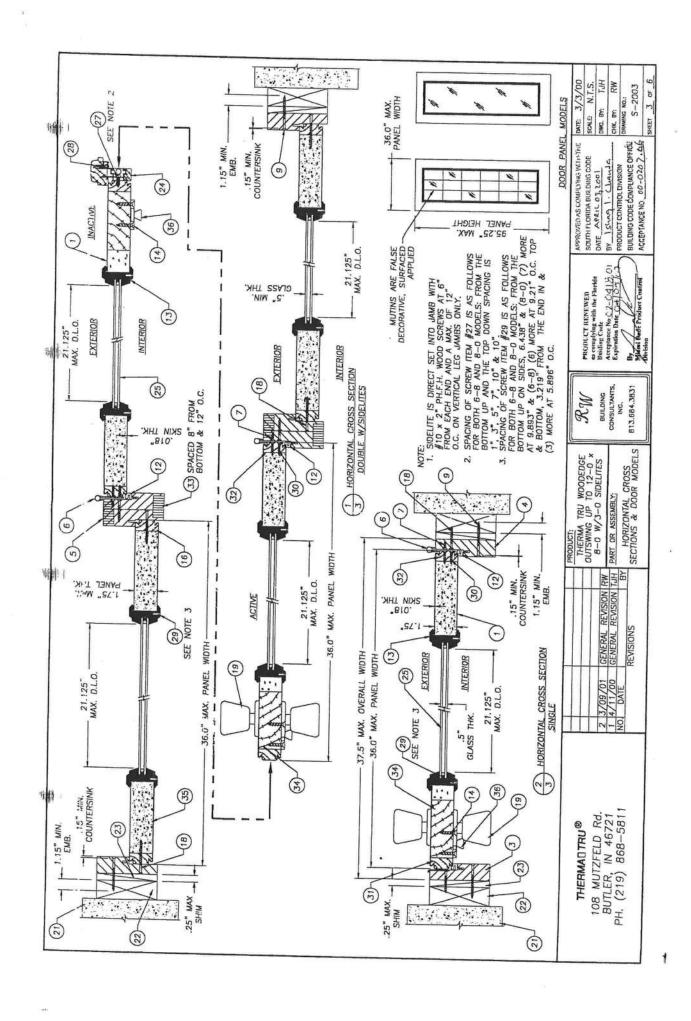
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

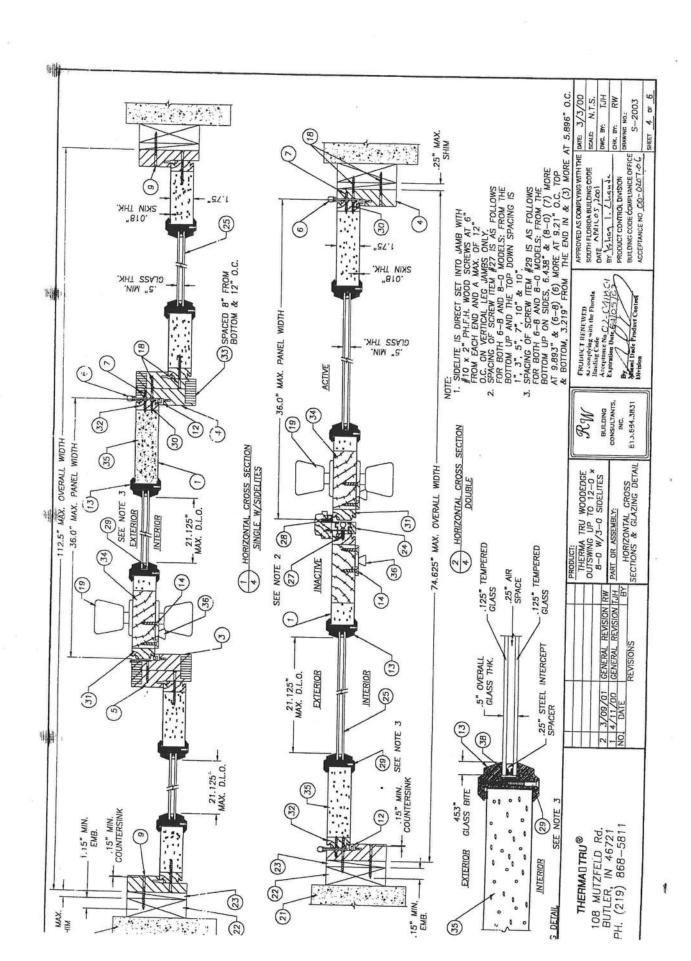
This NOA renews NOA # 00-0207.06 and, consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by Raul Rodriguez.

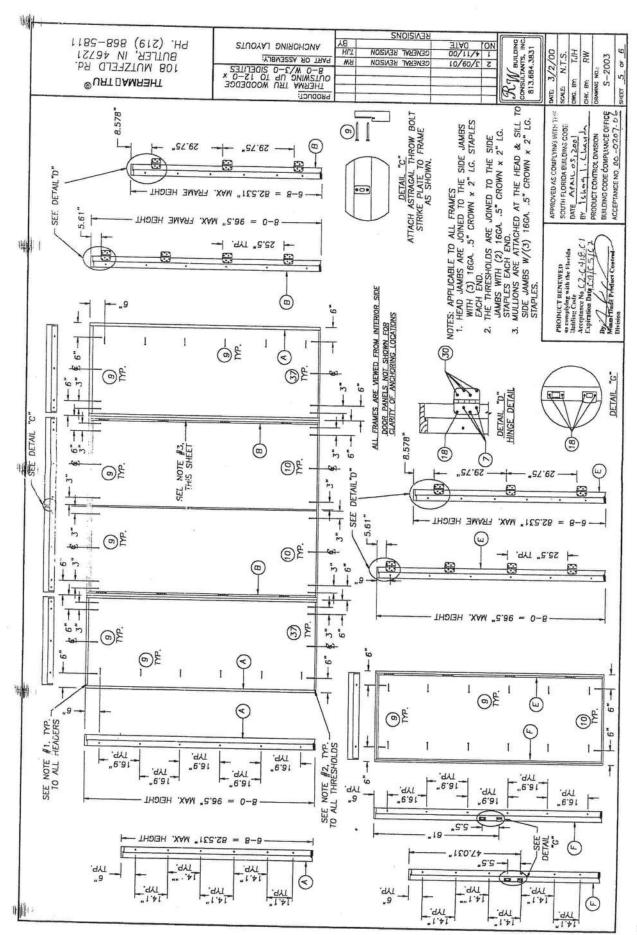


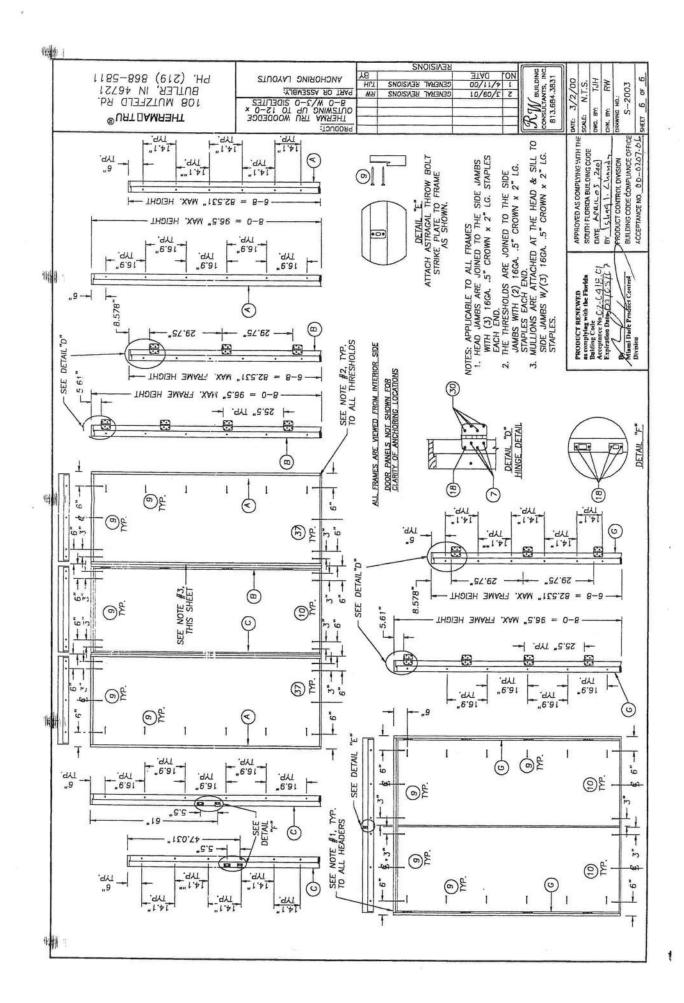
NOA No 02-0418.01 Expiration Date: April 05, 2007 Approval Date: May 23, 2002

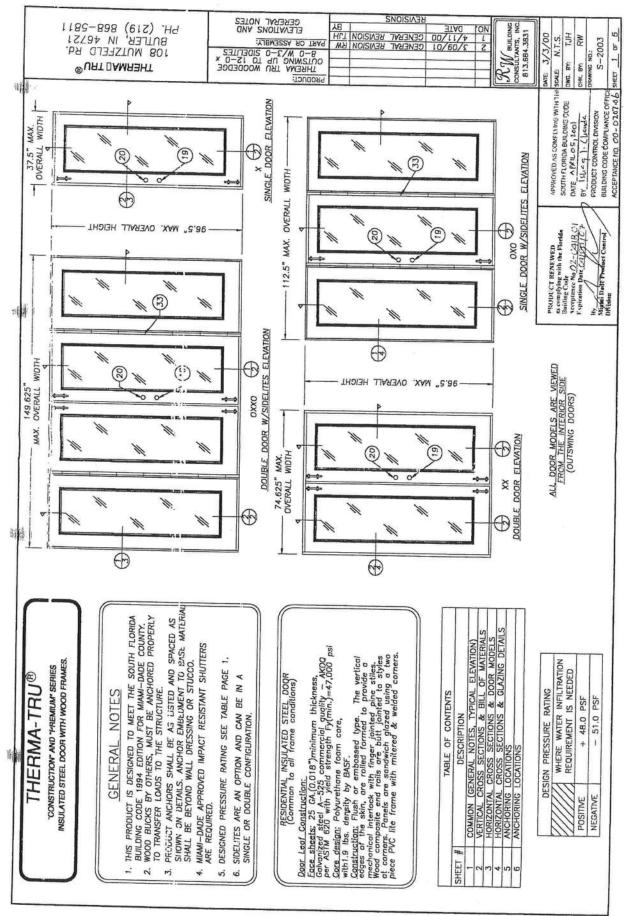
Page 1

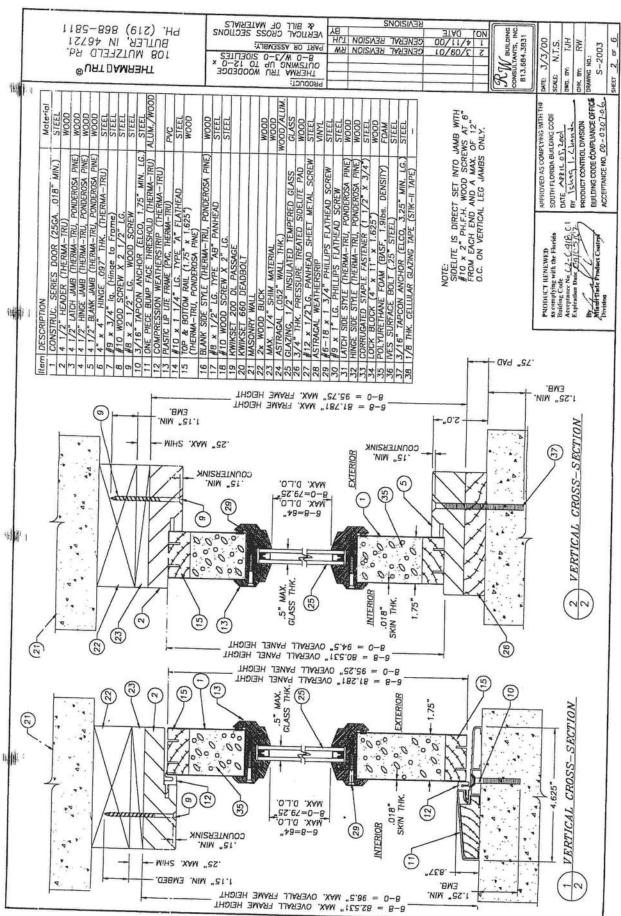














BUILDING CODE COMPLIANCE OFFICE (BCCO) PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Tamko Roofing Products, Inc. P.O. Box 1404 Joplin, MO 64802

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: TAMKO Heritage Declaration & Heritage XL Roof Shingles

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This consists of pages 1 through 4.

The submitted documentation was reviewed by Frank Zuloaga, RRC



NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03

Page 1 of 4

ROOFING ASSEMBLY APPROVAL

Category:

Roofing

Sub-Category:

07310 Composition Shingles

Materials

Dimensional

Deck Type:

Wood

1. SCOPE:

This approves Tamko Heritage Declaration and Heritage XL Asphalt Shingles, manufactured by Tamko Roofing Products, Inc. as described in this Notice of Acceptance.

2. PRODUCT DESCRIPTION

Product	Dimensions	<u>Test</u> Specifications	Product Description
Heritage Declaration & Heritage XL	12" x 36"		A heavy weight dimensional asphalt shingle.

3. EVIDENCE SUBMITTED:

Test Agency	Test Identifier	Test Name/Report	Date
PRI Asphalt Technologies, Inc.	TAS 100	TAP-066-02-01 TAP-073-02-01	01/09/03 05/20/03
Underwriters Laboratories, Inc. Underwriters Laboratories, Inc.	ASTM D 3462 TAS 107	R2919 03CA08442	06/12/03 06/12/03

4. LIMITATIONS

- 4.1 Fire classification is not part of this acceptance; refer to a current Approved Roofing Materials Directory for fire ratings of this product.
- 4.2 Shall not be installed on roof mean heights in excess of 33 ft.
- 4.3 All products listed herein shall have a quality assurance audit in accordance with the Florida Building Code and Rule 9B-72 of the Florida Administrative Code.

5. INSTALLATION

- 5.1 Shingles shall be installed in accordance with Roofing Application Standard RAS 115.
- 5.2 The manufacturer shall provide clearly written application instructions.
- 5.3 Exposure and course layout shall be in compliance with Detail 'A', attached.
- 5.4 Nailing shall be in compliance with Detail 'B', attached.

6. LABELING

5.1 Shingles shall be labeled with the Miami-Dade Logo or the wording "Miami-Dade County-Product Control Approved".

7. BUILDING PERMIT REQUIREMENTS

- 7.1 Application for building permit shall be accompanied by copies of the following:
 - 7.1.1 This Notice of Acceptance.
 - 7.1.2 Any other documents required by the Building Official or the applicable Building Code in order to properly evaluate the installation of this system.

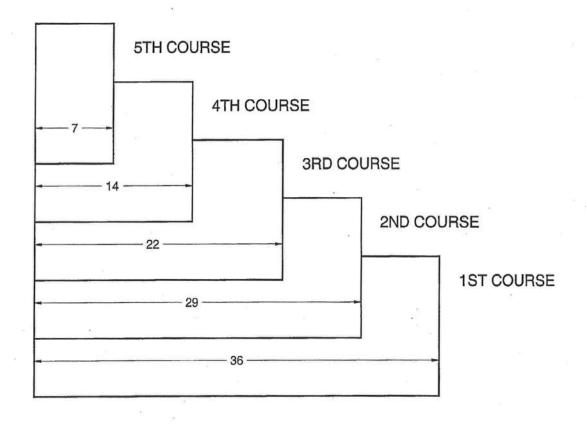


NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03 Page 2 of 4

DETAIL A

HERITAGE DECLARATION & XL

All dimensions are in inches.



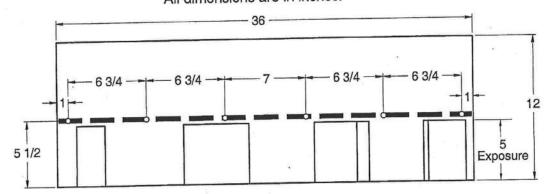


NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03 Page 3 of 4

DETAIL B

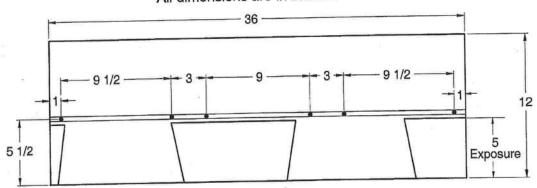
HERITAGE DECLARATION

12" x 36" LAMINATED SHINGLE All dimensions are in inches.



HERITAGE XL

12" x 36" LAMINATED SHINGLE All dimensions are in inches.



END OF THIS ACCEPTANCE



NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03

Page 4 of 4



MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corporation 108 Mutzfeld Road Butler, IN 46721

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by the BCCO and accepted by the Building Code and Product Review Committee (BCPRC) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

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DESCRIPTION: Outswing Glazed Residential Steel Door w\Sidelites

APPROVAL DOCUMENT: Drawing No. S-2003, titled "Therma-Tru Wood edge Outswing", sheets 1 through 6 to 6, prepared by RW Consulting, dated 3/9/01, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

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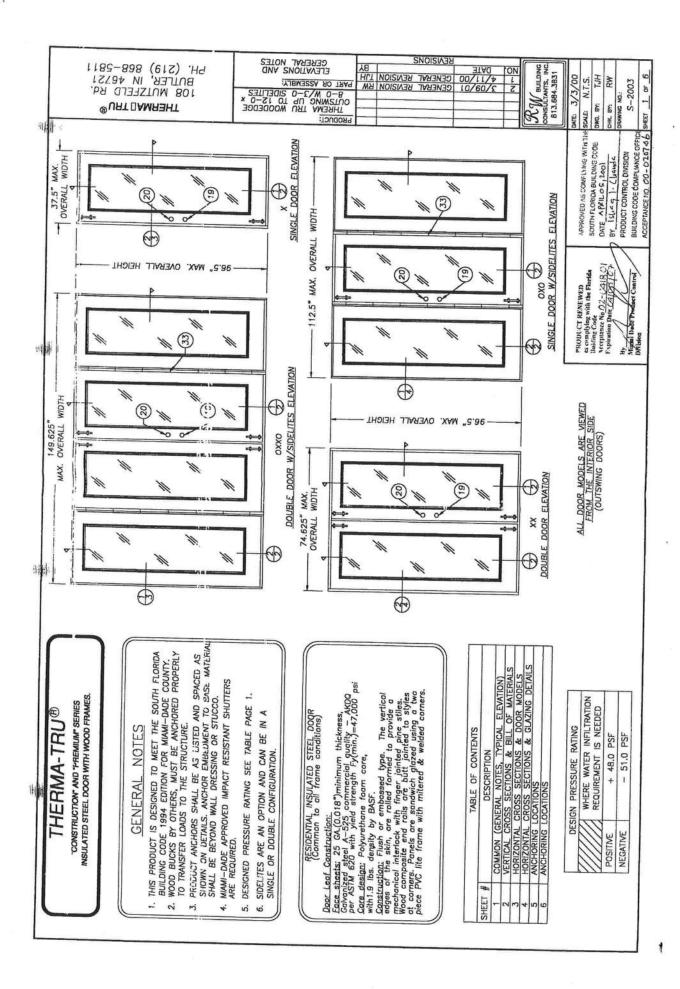
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

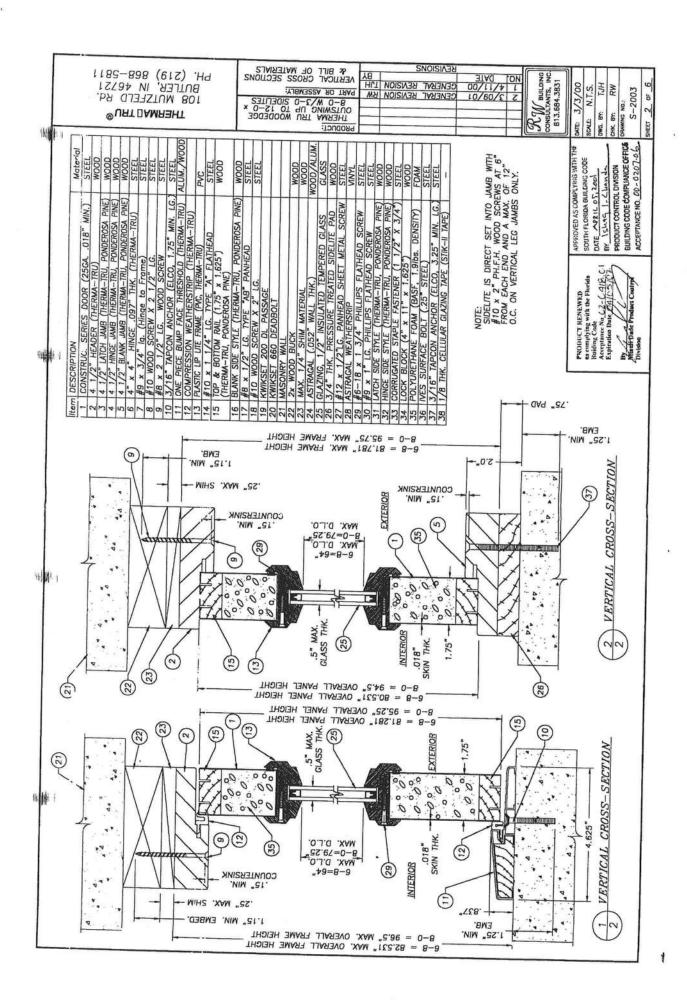
This NOA renews NOA # 00-0207.06 and, consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by Raul Rodriguez.

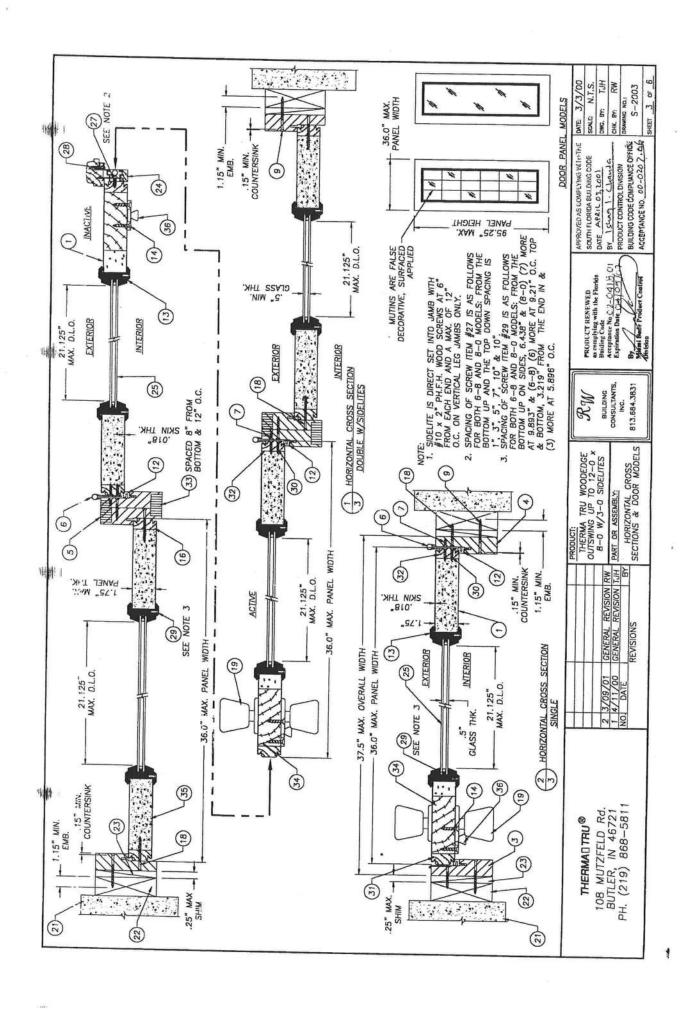


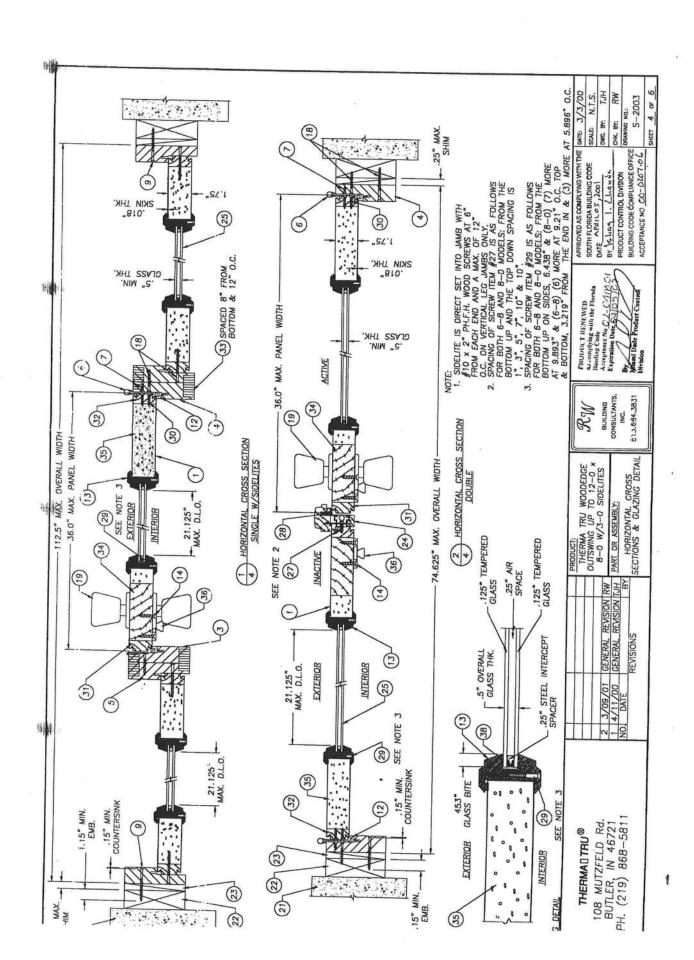
NOA No 02-0418.01 Expiration Date: April 05, 2007 Approval Date: May 23, 2002

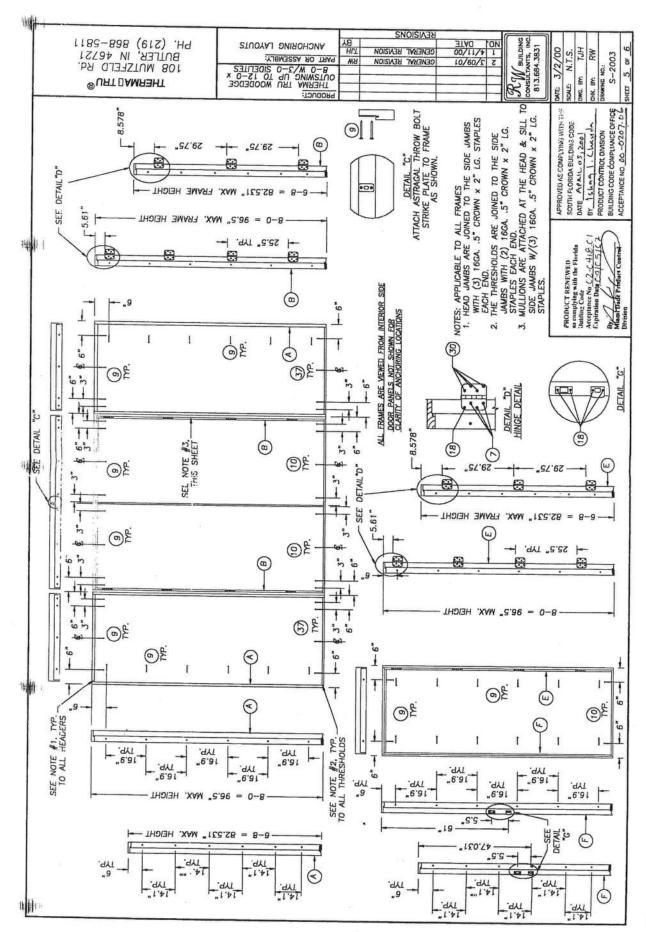
Page 1

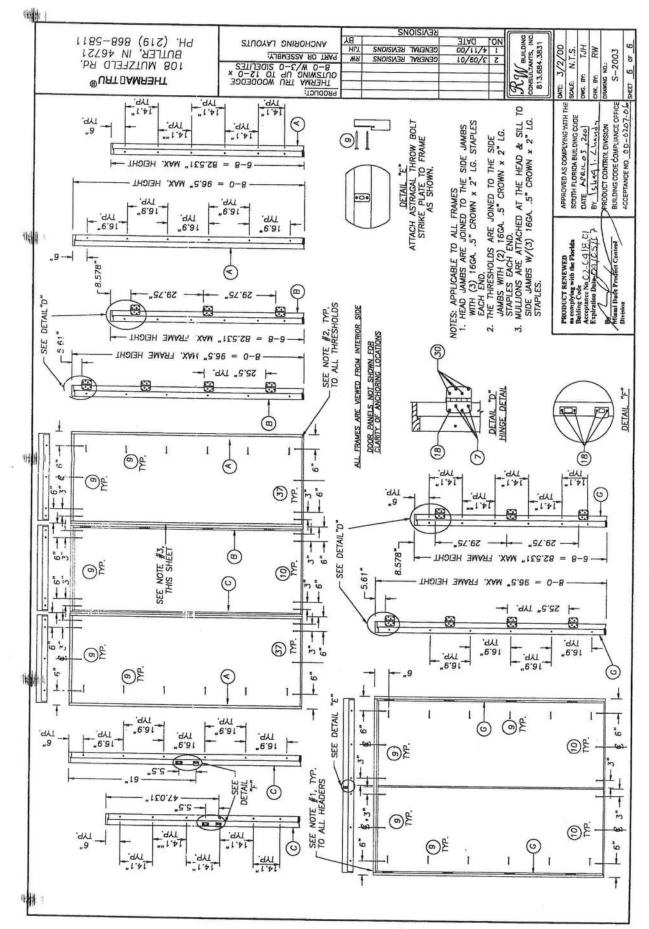














Project Information for:

L260933

Builder:

GIEBEIG HOMES

Lot:

Subdivision:

MAY-FAIR **COLUMBIA**

County: Truss Count:

30

Design Program: MiTek 20/20 6.3 Building Code:

FBC2004/TPI2002

Truss Design Load Information: Gravity:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

Brian T. Giebeig Florida Registered Residential Contractor License No. RR282811523 Address: Trent Giebeig Construction, Inc. 462 Southwest Fairlington Court Lake City, Florida 32025

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T23

T24

Date

11/16/07

11/16/07

Drwg. #

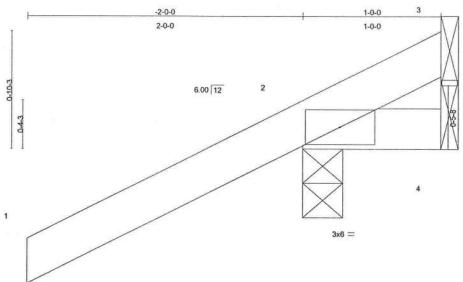
J1910514

J1910515

No.	Drwg. #	Truss ID	Date	No
1	J1910486	CJ1	11/16/07	29
2	J1910487	CJ3	11/16/07	30
3	J1910488	CJ5	11/16/07	
4	J1910489	EJ5	11/16/07	
5	J1910490	EJ7	11/16/07	
6	J1910491	HJ7	11/16/07	
7	J1910492	HJ9	11/16/07	
8	J1910493	T01GB	11/16/07	
9	J1910494	T03	11/16/07	
10	J1910495	T04	11/16/07	
11	J1910496	T05	11/16/07	
12	J1910497	T06	11/16/07	
13	J1910498	T07	11/16/07	
14	J1910499	T08	11/16/07	
15	J1910500	T09	11/16/07	
16	J1910501	T10	11/16/07	
17	J1910502	T11	11/16/07	
18	J1910503	T12	11/16/07	
19	J1910504	T13	11/16/07	
20	J1910505	T14	11/16/07	
21	J1910506	T15	11/16/07	
22	J1910507	T16	11/16/07	
23	J1910508	T17	11/16/07	
24	J1910509	T18	11/16/07	
25	J1910510	T19	11/16/07	
26	J1910511	T20	11/16/07	
27	J1910512	T22	11/16/07	
28	J1910513	T22G	11/16/07	

Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	CJ1	ROOF TRUSS	14	1	J1910486
				33.0	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:03 2007 Page 1



1-0-0

LOADIN	G (psf)	SPACING	2-0-0	CSI	5-6-6-6-6-6-	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)	3 57.					Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1)

Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3. Continued on page 2

Julius Lee Truse Oesign Engineer Florida PE No. 3-1869 1109 Ceestal Bay Blvd

November 16,2007

Scale: 1.5"=1"

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or tIHE-91 Handling Installing and Bracing Recommendation autilable from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910486
L260933	CJ1	ROOF TRUSS	14	1	
					Job Reference (optional)

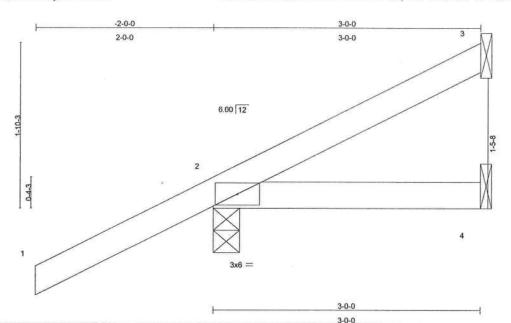
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:03 2007 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	СЈЗ	ROOF TRUSS	14	1	J1910487
	000	TOO! TROOS	14		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:03 2007 Page 1



LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES** GRIP in (loc) I/defI **TCLL** 20.0 Plates Increase 1.25 TC 0.29 Vert(LL) 0.01 2-4 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.012-4 >999 240 BCLL 10.0 WB * Rep Stress Incr YES 0.00 Horz(TL) -0.00n/a n/a BCDL Code FBC2004/TPI2002 5.0 (Matrix) Weight: 13 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)

Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4. Continued on page 2

Julius Les Truss Design Engineer Florida PE No. 34869 1100 Casstal Bay Blvd Boynton Beach, FL 33436

November 16,2007

Scale = 1:12.5

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation autibable from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	CJ3	ROOF TRUSS	14	1	J1910487
L200933	033	ROOF IROSS	14		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:03 2007 Page 2

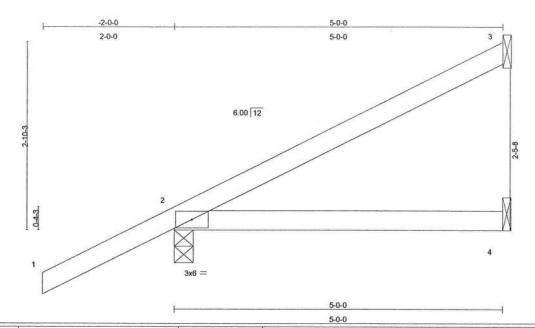
LOAD CASE(S) Standard

ulius Lee ruse Design Engineer londe PE No. 24866 109 Ceastal Bay Blvd loynton Besch, FL 22425



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	CJ5	ROOF TRUSS	10	1	J1910488
		Noor mood	.0		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:04 2007 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)						Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4) Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-88/36

BOT CHORD

2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

Julius Lee Truss Cosign Engineer Florida PE No. 34869 1409 Casstal Bay Blod Bovitton Beach, FL 33435

November 16,2007

Scale = 1:16.9

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
1.000000	CIE	DOOF TRUES	10		J1910488
L260933	CJ5	ROOF TRUSS	10	1	Job Reference (optional)

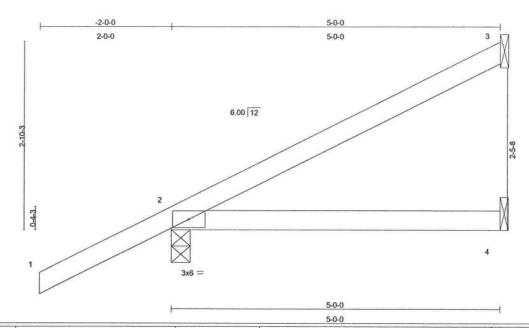
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:04 2007 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	EJ5	ROOF TRUSS	7	1	J1910489
		11001 111000			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:05 2007 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	3 3					Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4) Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-88/36

BOT CHORD

2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

November 16,2007

Scale = 1:16.9

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	EJ5	DOOF TRUCC	-		J1910489
L200933	EJS	ROOF TRUSS	/	1	Job Reference (optional)

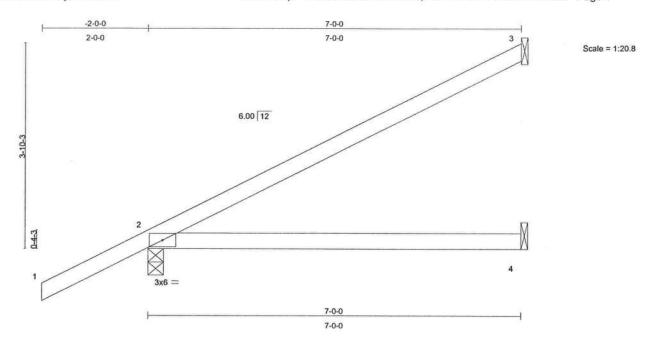
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:05 2007 Page 2

LOAD CASE(S) Standard



Jớb	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - CANNON CREEK PL LOT 15
L260933	EJ7	MONO TRUSS	25	1	J1910490
- INCAVATOR- STREET	1117975 (85)				Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 16 08:21:28 2007 Page 1



LOADIN	G (psf)	1	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.50	Vert(LL)	0.33	2-4	>250	360	MT20	244/190
TCDL	7.0	1	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.16	2-4	>501	240		200000000000000000000000000000000000000
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	1	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

oc pun

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-94(load case 6), 2=-224(load case 6), 4=-65(load case 5) Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-131/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.58

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Truse Ossign Engineer Florida PE No. 3-1868 1109 Crestal Bay Blvd Boynton Beach, FL 33435

November 16,2007

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - CANNON CREEK PL LOT 15
L260933	EJ7	MONO TRUSS	25	1	J1910490
L200933	Lor	WONO TROSS	25	_ '	Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 16 08:21:28 2007 Page 2

NOTES

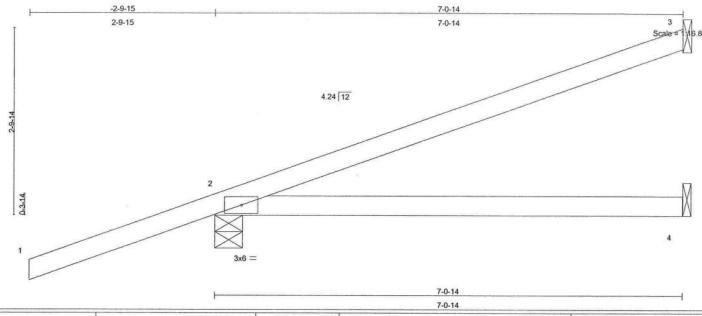
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 224 lb uplift at joint 2 and 65 lb uplift at joint 4.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34869 1109 Caestal Bay Blvd Bovnton Besch, FL 23436



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
		all outside of the Charles			J191049
_260933	HJ7	ROOF TRUSS	2	1	
	\$155.4E1645	CAMPAGNOSCA ON DIRECTOR			Job Reference (optional)



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.14	2-4	>596	240	B40/7/HUGBCRNA6/Y	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)			*/76.01	A. 30.3911		Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

7-0-14 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=185/Mechanical, 2=335/0-4-15, 4=38/Mechanical

Max Horz 2=167(load case 3)

Max Uplift 3=-145(load case 3), 2=-248(load case 3)

Max Grav 3=185(load case 1), 2=335(load case 1), 4=97(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-68/43

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.44

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 3 and 248 lb uplift at joint 2.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 Continued on page 2

Julius Lee Truss Design Engineer Florida PE No. 34866 1 100 Ceastal Bay Blvd Boynton Besch, FL 33436

November 16,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	НЈ7	ROOF TRUSS	2	1	J191049
L200933	1137	ROOF INUSS	2	1	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:06 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=-0(F=5, B=5)-to-4=-18(F=-4, B=-4)

Julius Les Truss Design Engineer Florida PE No. 3-1869 1-106 Ceastal Bay Blvd Boynton Besch, FL 23-126



Job Truss Truss Type Qty Ply GEIBEIG HOMES - MAY-FAIR LOT 48 J1910492 L260933 HJ9 ROOF TRUSS Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:06 2007 Page 1 9-10-13 -2-9-15 4-3-0 2-9-15 4-3-0 5-7-13 4.24 12 3x6 = 0-3-14 6 5 2x4 || 3x6 = 4-3-0 9-10-13 4-3-0 5-7-13 LOADING (psf) SPACING 2-0-0 CSI DEFL GRIP (loc) I/defl L/d **PLATES** TCLL 20.0 Plates Increase 1.25 TC 0.60 Vert(LL) 0.09 6-7 >999 360 MT20 244/190 0.40 7.0 BC TCDL Lumber Increase 1.25 Vert(TL) -0.116-7 >999 240 BCLL 10.0 Rep Stress Incr NO WB 0.36 Horz(TL) 0.01 5 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 45 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. 2 X 4 SYP No.3 **WEBS BOT CHORD** Rigid ceiling directly applied or 7-11-7 oc

bracing.

REACTIONS (lb/size) 4=267/Mechanical, 2=453/0-4-15, 5=220/Mechanical

Max Horz 2=269(load case 3)

Max Uplift 4=-233(load case 3), 2=-399(load case 3), 5=-183(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/50, 2-3=-650/365, 3-4=-105/65

BOT CHORD

2-7=-538/603, 6-7=-538/603, 5-6=0/0

WEBS

3-7=-89/186, 3-6=-627/559

JOINT STRESS INDEX

2 = 0.76, 3 = 0.22, 6 = 0.17 and 7 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 399 lb uplift at joint 2 and 183 lb uplift at joint 5.

Continued on page 2

November 16,2007

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	НЈ9	ROOF TRUSS	5	1	J191049.
		Noor mose			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:06 2007 Page 2

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

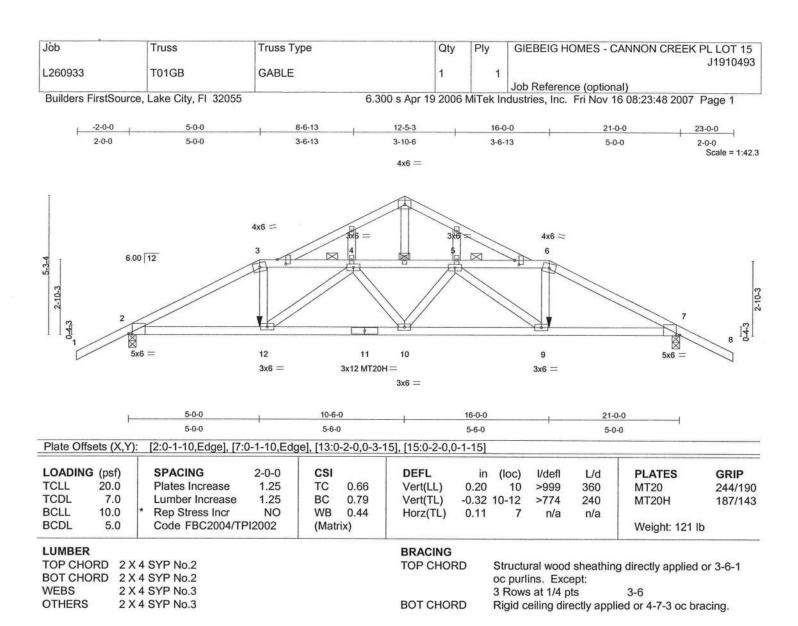
Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee Truse Design Engineer Florida PE No. 34889 1198 Ceastal Bay Blvd Boynton Beach, FL 33436





REACTIONS (lb/size) 2=1681/0-3-8, 7=1681/0-3-8

Max Horz 2=73(load case 5)

Max Uplift 2=-892(load case 5), 7=-892(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3085/1556, 3-4=-2736/1435, 4-5=-3741/1940, 5-6=-2736/1435,

6-7=-3085/1556, 7-8=0/47

BOT CHORD 2-12=-1342/2680, 11-12=-1901/3704, 10-11=-1901/3704, 9-10=-1885/3704,

7-9=-1315/2680

WEBS 3-12=-502/1004, 4-12=-1265/726, 4-10=0/127, 5-10=0/127, 5-9=-1265/726, 6-9=-502/1004

JOINT STRESS INDEX

2 = 0.77, 3 = 0.70, 4 = 0.39, 4 = 0.34, 5 = 0.39, 5 = 0.34, 6 = 0.70, 7 = 0.77, 9 = 0.66, 10 = 0.39, 11 = 0.81, 12 = 0.66, 13 = 0.34, 14 = 0.25, 15 = 0.34, 16 = 0.34, 17 = 0.34 and 18 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.

Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Julius Lee Truss Design Engineer Florida PE No. 34868 1109 Ceastal Bay Blvd Boynton Beach, FL 33425

November 16,2007

donting adequate drainage to prevent water ponding.

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Jòb	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - CANNON CREEK PL LOT 15
L260933	T01GB	GABLE	1	1	J1910493
					Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 16 08:23:49 2007 Page 2

NOTES

- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 892 lb uplift at joint 2 and 892 lb uplift at joint 7.
- 11) Girder carries hip end with 5-0-0 end setback.
- 12) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

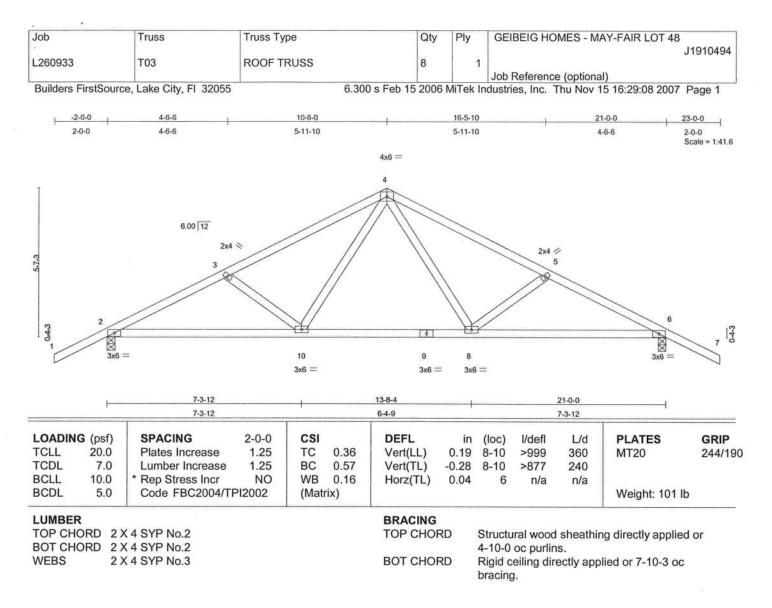
Vert: 1-3=-54, 3-6=-178(F=-124), 6-8=-54, 2-12=-10, 9-12=-17(F=-7), 7-9=-10

Concentrated Loads (lb)

Vert: 12=-187(F) 9=-187(F)

Julius Lee Truse Design Engineer Florida PE No. 34869 1100 Ceastal Bay Blvd





REACTIONS (lb/size) 2=970/0-3-8, 6=970/0-3-8

Max Horz 2=-98(load case 7)

Max Uplift 2=-293(load case 6), 6=-293(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1598/872, 3-4=-1396/799, 4-5=-1396/799, 5-6=-1598/872, 6-7=0/47

BOT CHORD 2-10=-620/1364, 9-10=-318/941, 8-9=-318/941, 6-8=-620/1364

WEBS 3-10=-248/224, 4-10=-242/498, 4-8=-242/498, 5-8=-248/224

JOINT STRESS INDEX

2 = 0.69, 3 = 0.33, 4 = 0.83, 5 = 0.33, 6 = 0.69, 8 = 0.42, 9 = 0.58 and 10 = 0.42

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 293 lb uplift at joint 2 and 293 lb uplift at joint 6. Continued on page 2

Julius Lee Truss Design Engineer Flonda PE No. 24868 1 109 Geestal Bay Blvd Bovnton Besch, FL 23436

November 16,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T03	ROOF TRUSS	g	1	J1910494
2200000	100	NOO! TROOS	0		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:08 2007 Page 2

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

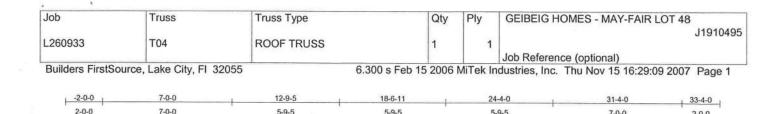
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

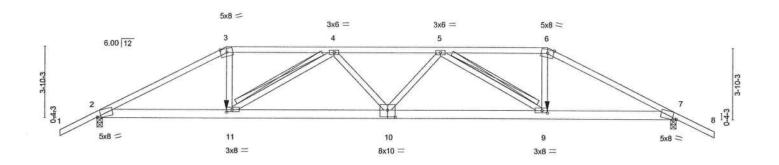
Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Lee Truse Design Engineer Flonda PE No. 34869 1100 Ceastal Bay Blvd Boynton Besch, FL 33436





5-9-5



		7-0-0		8-8-0		,	8-8-0			7	7-0-0	
Plate Of	fsets (X,Y	(): [2:0-2-7,Edge], [7:	0-2-7,Edg	e], [9:0-	3-8,0-1-8	3], [10:0-5-0,0	-4-8], [1	1:0-3-8	,0-1-8]			
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.29	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.55	9-10	>674	240	I ANDROMEN	20001000405050
BCLL	10.0	* Rep Stress Incr	NO	WB	0.45	Horz(TL)	0.13	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 170 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

2-3-14 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-6-8 oc

bracing.

5-9-5

WEBS T-Brace: 2 X 4 SYP No.3 -

4-11, 5-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

7-0-0

31-4-0

2-0-0 Scale = 1:59.9

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2171/0-3-8, 7=2171/0-3-8

7-0-0

7-0-0

5-9-5

Max Horz 2=-79(load case 6)

Max Uplift 2=-683(load case 5), 7=-683(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/51, 2-3=-4189/1339, 3-4=-3726/1244, 4-5=-5084/1652, 5-6=-3726/1244, TOP CHORD

6-7=-4189/1339, 7-8=0/51

BOT CHORD 2-11=-1162/3670, 10-11=-1643/5000, 9-10=-1623/5000, 7-9=-1131/3670

WEBS 3-11=-391/1356, 4-11=-1590/619, 4-10=0/243, 5-10=0/243, 5-9=-1590/619,

6-9=-391/1356

JOINT STRESS INDEX

2 = 0.79, 3 = 0.76, 4 = 0.45, 5 = 0.45, 6 = 0.76, 7 = 0.79, 9 = 0.85, 10 = 0.79 and 11 = 0.85

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
1.000000	TO 4	DOOF TOUGS			J1910495
L260933	T04	ROOF TRUSS	1	1	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:10 2007 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 683 lb uplift at joint 2 and 683 lb uplift at joint 7.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

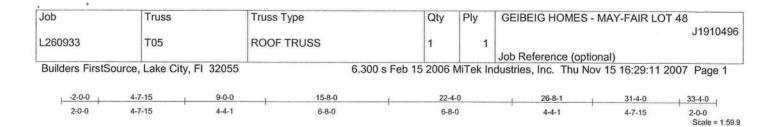
Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-11=-10, 9-11=-22(F=-12), 7-9=-10

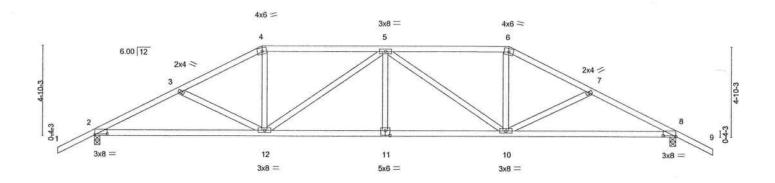
Concentrated Loads (lb)

Vert: 11=-411(F) 9=-411(F)

Julius Lee Truss Oesign Engineer Flonda PE No. 34888 1 109 Cassial Bay Blvd Bovnion Beach, FL 33436







		9-0-0		6-8-	0	5.5	6-8-0			9-0-0		
Plate Offsets (X,Y): [2:0-8-0,0-0-10], [8:0-8-0,0-0-10], [11:0-3-0,0-3-0]												
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.15	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.29	8-10	>999	240		

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRII
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.15	8-10	>999	360	MT20	244/
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.29	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002 (Matrix)		rix)	, ,					Weight: 160 lb)
										-		

15-8-0

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		4-7-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 7-6-3 oc
			bracing.

REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

Max Horz 2=89(load case 6)

Max Uplift 2=-271(load case 6), 8=-271(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1832/959, 3-4=-1607/860, 4-5=-1410/832, 5-6=-1410/832,

6-7=-1607/860, 7-8=-1832/959, 8-9=0/47

BOT CHORD 2-12=-690/1571, 11-12=-690/1715, 10-11=-690/1715, 8-10=-690/1571

WEBS 3-12=-198/193, 4-12=-142/418, 5-12=-465/206, 5-11=0/158, 5-10=-465/206,

6-10=-142/418, 7-10=-198/193

JOINT STRESS INDEX

2 = 0.65, 3 = 0.33, 4 = 0.68, 5 = 0.56, 6 = 0.68, 7 = 0.33, 8 = 0.65, 10 = 0.56, 11 = 0.40 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee Truss Design Engineer Floride PE No. 24889 1100 Ceastal Bay Blvd Boynton Beach, FL 33435

31-4-0

3) Provide adequate drainage to prevent water ponding.

Continued on page 2

November 16,2007

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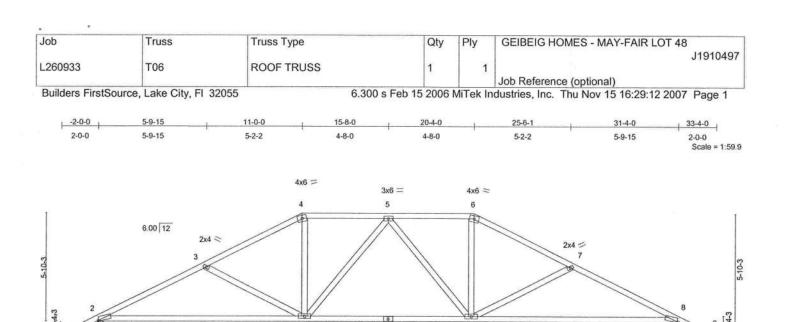
Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T05	ROOF TRUSS	1	1	J1910496
	1.000000000				Job Reference (optional)

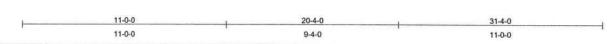
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:11 2007 Page 2

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2 and 271 lb uplift at joint 8.

LOAD CASE(S) Standard







11

3x6 =

10

3x8 =

12

3x8 =

Plate Of	fsets (X,Y	'): [2:0-0-10,Edge], [8	3:0-0-10,E	dge]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.31	8-10	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.57	8-10	>654	240	A12A 7.75.78	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	P12002	(Mat	rix)		2000 CO (000 CO)	AECV.	2.30,460.0	NAC-SEC	Weight: 158 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

3x8 =

BRACING

TOP CHORD Structural wood sheathing directly applied or

4-6-1 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-6-2 oc bracing.

REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

Max Horz 2=-101(load case 7)

Max Uplift 2=-285(load case 6), 8=-285(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1806/974, 3-4=-1490/827, 4-5=-1282/803, 5-6=-1282/803,

6-7=-1490/827, 7-8=-1806/974, 8-9=0/47

BOT CHORD 2-12=-694/1546, 11-12=-502/1376, 10-11=-502/1376, 8-10=-694/1546

WEBS 3-12=-314/289, 4-12=-144/397, 5-12=-263/116, 5-10=-263/116, 6-10=-144/397,

7-10=-314/289

JOINT STRESS INDEX

2 = 0.89, 3 = 0.33, 4 = 0.61, 5 = 0.38, 6 = 0.61, 7 = 0.33, 8 = 0.89, 10 = 0.56, 11 = 0.48 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

Continued on page 2

November 16,2007



3x8 =

Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T06	ROOF TRUSS	1	1	J191049
1					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:12 2007 Page 2

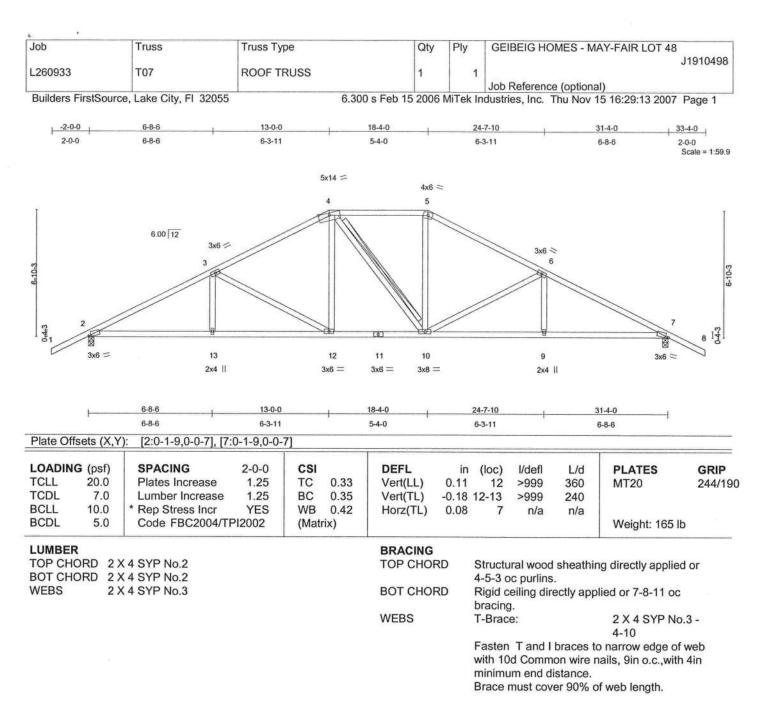
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 285 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34865 1199 Coestal Bay Blvd





REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=-113(load case 7)

Max Uplift 2=-297(load case 6), 7=-297(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1817/950, 3-4=-1360/802, 4-5=-1151/786, 5-6=-1360/802,

6-7=-1817/950, 7-8=0/47

BOT CHORD 2-13=-668/1545, 12-13=-668/1545, 11-12=-374/1150, 10-11=-374/1150,

9-10=-668/1545, 7-9=-668/1545

WEBS 3-13=0/212, 3-12=-453/335, 4-12=-124/318, 4-10=-152/153, 5-10=-124/318,

6-10=-453/335, 6-9=0/212

JOINT STRESS INDEX

2 = 0.78, 3 = 0.39, 4 = 0.79, 5 = 0.68, 6 = 0.39, 7 = 0.78, 9 = 0.33, 10 = 0.56, 11 = 0.38, 12 = 0.34 and 13 = 0.33

Continued on page 2

November 16,2007



PE No. 34868

Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T07	ROOF TRUSS	1	1	J1910498
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:13 2007 Page 2

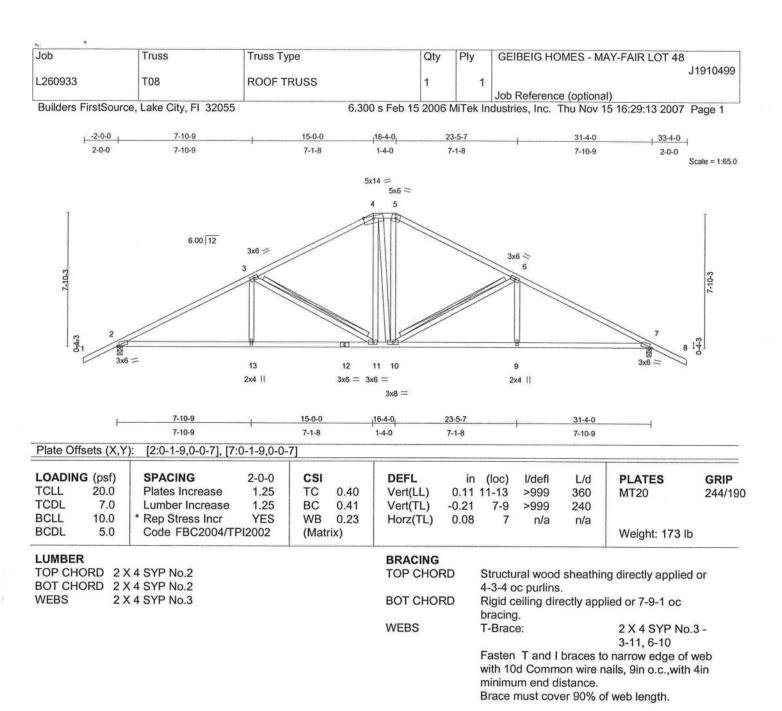
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 297 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 3-1869 1100 Ceestal Bay Blvd Boynton Beach, FL 33435





REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=-125(load case 7)

Max Uplift 2=-308(load case 6), 7=-316(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1781/949, 3-4=-1235/765, 4-5=-1030/763, 5-6=-1239/767,

6-7=-1780/948, 7-8=0/47

BOT CHORD 2-13=-652/1504, 12-13=-652/1504, 11-12=-652/1504, 10-11=-298/1026,

9-10=-651/1503, 7-9=-651/1503

WEBS 3-13=0/250, 3-11=-558/407, 4-11=-164/290, 5-10=-162/364, 6-10=-552/403,

6-9=0/248, 4-10=-202/240

JOINT STRESS INDEX

2 = 0.77, 3 = 0.39, 4 = 0.79, 5 = 0.59, 6 = 0.39, 7 = 0.77, 9 = 0.33, 10 = 0.57, 11 = 0.34, 12 = 0.52 and 13 = 0.33 Continued on page 2

November 16,2007

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T08	ROOF TRUSS	1	1	J1910499
			100		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:14 2007 Page 2

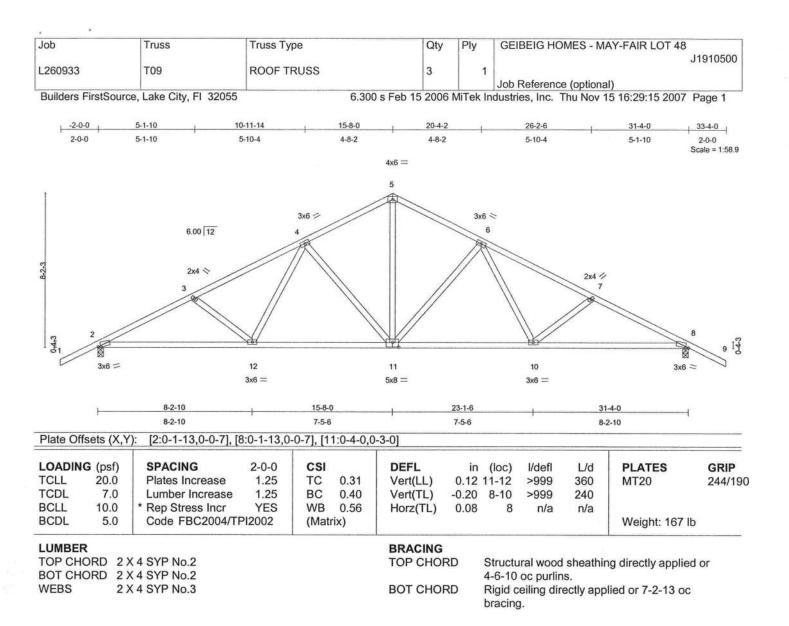
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 316 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34869 1 109 Geestal Bay Blvd Bovnton Beach, FL 33435





REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

Max Horz 2=-128(load case 7)

Max Uplift 2=-311(load case 6), 8=-311(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1850/1019, 3-4=-1619/933, 4-5=-1157/778, 5-6=-1157/778,

6-7=-1619/933, 7-8=-1850/1019, 8-9=0/47

BOT CHORD 2-12=-746/1590, 11-12=-493/1265, 10-11=-493/1265, 8-10=-746/1590

WEBS 3-12=-268/260, 4-12=-111/328, 4-11=-452/357, 5-11=-512/750, 6-11=-452/357,

6-10=-111/328, 7-10=-268/260

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.41, 5 = 0.36, 6 = 0.41, 7 = 0.33, 8 = 0.76, 10 = 0.44, 11 = 0.37 and 12 = 0.44

NOTES

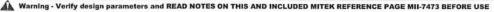
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp
B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Trues Design Engineer Florida FE No. 34898 1109 Chestal Bay Blvd Boynton Beach, Ft 33435

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Coliver page 2

November 16,2007



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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T09	ROOF TRUSS	3	1	J1910500
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:15 2007 Page 2

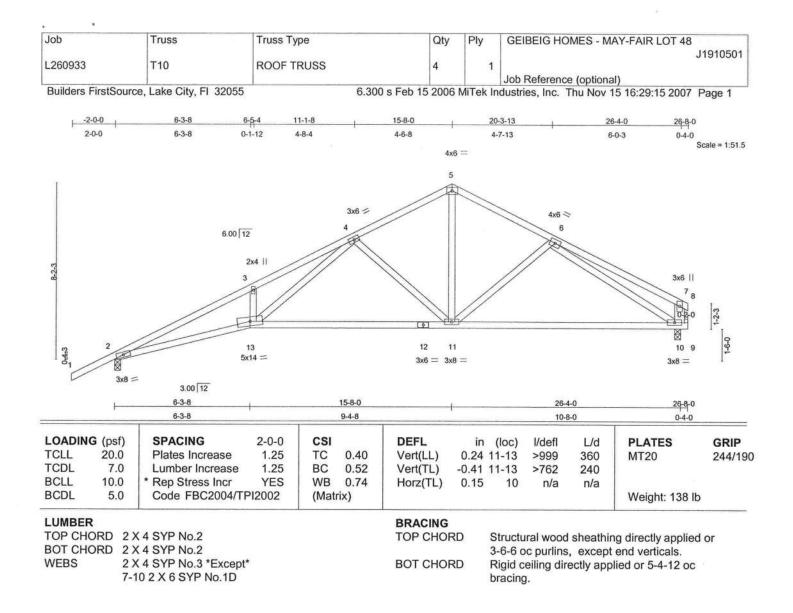
NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 311 lb uplift at joint 8.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=956/0-3-8, 10=857/0-3-8

Max Horz 2=190(load case 6)

Max Uplift 2=-284(load case 6), 10=-180(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2708/1450, 3-4=-2678/1612, 4-5=-1003/653, 5-6=-1007/653.

6-7=-371/235, 7-8=0/10, 7-10=-324/260

BOT CHORD 2-13=-1342/2405, 12-13=-686/1289, 11-12=-686/1289, 10-11=-510/959, 9-10=0/0

3-13=-249/274, 4-13=-835/1385, 4-11=-595/455, 5-11=-382/599, 6-11=-205/210,

6-10=-842/488

JOINT STRESS INDEX

Continued on page 2

2 = 0.63, 3 = 0.33, 4 = 0.87, 5 = 0.41, 6 = 0.28, 7 = 0.56, 10 = 0.87, 11 = 0.56, 12 = 0.47 and 13 = 0.69

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

November 16,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T10	ROOF TRUSS	4	1	J1910501
		WW. 750-270-001 - 4500-050-750-750-654			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:15 2007 Page 2

NOTES

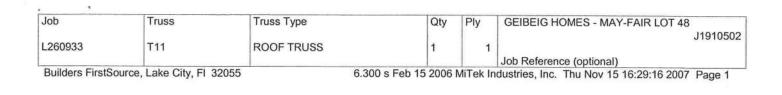
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 2 and 180 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34869 1100 Cassial Bay Blvd Bovnton Besch El. 23426





6-8-13

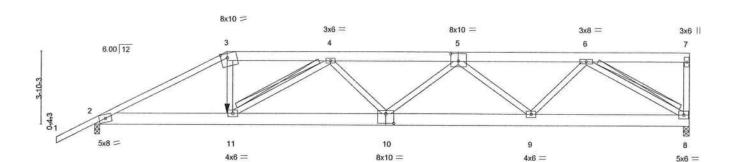




Plate Offsets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES** GRIP in (loc) 20.0 TCLL 1.25 TC Plates Increase 0.53 Vert(LL) -0.2110 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.27 Vert(TL) -0.40 10-11 >933 240 BCLL 10.0 Rep Stress Incr WB 0.88 NO Horz(TL) 0.08 8 n/a n/a **BCDL** Code FBC2004/TPI2002 (Matrix) 5.0 Weight: 220 lb

LUMBER TOP CHORD 2 X 6 SYP No.1D *Except* 1-3 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP 2400F 2.0E **BOT CHORD WEBS** 2 X 4 SYP No.3

BRACING TOP CHORD Structural wood sheathing directly applied or

25-10-13

6-8-13

31-4-0

5-5-3

Scale = 1:58.4

2-9-8 oc purlins, except end verticals. Rigid ceiling directly applied or 8-11-13 oc bracing.

WEBS T-Brace: 2 X 4 SYP No.3 -4-11, 6-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=2194/0-3-8, 2=2140/0-3-8

Max Horz 2=162(load case 5)

Max Uplift 8=-755(load case 4), 2=-666(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-4181/1361, 3-4=-3745/1268, 4-5=-5112/1715, 5-6=-3909/1295,

6-7=-79/23, 7-8=-277/136

BOT CHORD 2-11=-1249/3685, 10-11=-1775/5042, 9-10=-1750/4987, 8-9=-1061/2959

WEBS 3-11=-403/1381, 4-11=-1638/611, 4-10=0/239, 5-10=0/232, 5-9=-1427/603,

6-9=-354/1440, 6-8=-3400/1226

JOINT STRESS INDEX

-2-0-0

2-0-0

3-5-6

3-5-6

3-6-10

5-5-3

2 = 0.85, 3 = 0.71, 4 = 0.46, 5 = 0.33, 6 = 0.87, 7 = 0.41, 8 = 0.84, 9 = 0.61, 10 = 0.64 and 11 = 0.62

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T11	ROOF TRUSS	1	1	J1910502
		Accesses to the second	26		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:16 2007 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 755 lb uplift at joint 8 and 666 lb uplift at joint 2.
- 7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

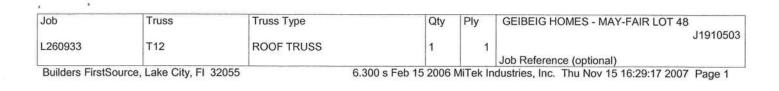
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-3=-54, 3-7=-118(F=-64), 2-11=-10, 8-11=-22(F=-12)

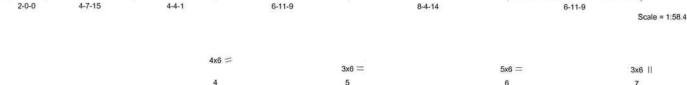
Concentrated Loads (lb) Vert: 11=-411(F)

> Julius Lee Trues Design Engineer Flood See No. 2 dess





15-11-9



24-4-7

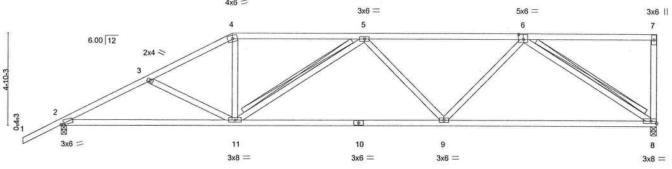




Plate Of	tsets (X,Y	(): [2:0-1-13,0-0-7], [6	5:0-3-0,0-3	3-0]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.25	8-9	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.45	8-9	>822	240	(101.0.002)	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	Control of the Contro				A1145.2004	Weight: 160 lb	

LUMBER	BRACING		
TOP CHORD 2 X 4 SYP No.2	TOP CHORD	Structural wood s	heathing directly applied or
BOT CHORD 2 X 4 SYP No.2			except end verticals.
WEBS 2 X 4 SYP No.3	BOT CHORD	dinakt Milk - man Milk 191	tly applied or 6-4-5 oc
		bracing.	
	WEBS	T-Brace:	2 X 4 SYP No.3 -

5-11, 6-8 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

31-4-0

6-11-9

REACTIONS (lb/size) 8=989/0-3-8, 2=1113/0-3-8 Max Horz 2=195(load case 6)

Max Uplift 8=-270(load case 5), 2=-265(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1846/924, 3-4=-1622/824, 4-5=-1423/798, 5-6=-1520/785, 6-7=-63/9

7-8=-160/110

BOT CHORD 2-11=-969/1582, 10-11=-965/1730, 9-10=-965/1730, 8-9=-671/1207

WEBS 3-11=-191/195, 4-11=-130/428, 5-11=-369/201, 5-9=-309/265, 6-9=-168/510,

6-8=-1396/797

JOINT STRESS INDEX

-2-0-0

2-0-0

4-7-15

4-7-15

4-4-1

2 = 0.80, 3 = 0.33, 4 = 0.66, 5 = 0.36, 6 = 0.68, 7 = 0.34, 8 = 0.65, 9 = 0.36, 10 = 0.67 and 11 = 0.56

November 16,2007

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T12	ROOF TRUSS	1	1	J1910503
				1	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:17 2007 Page 2

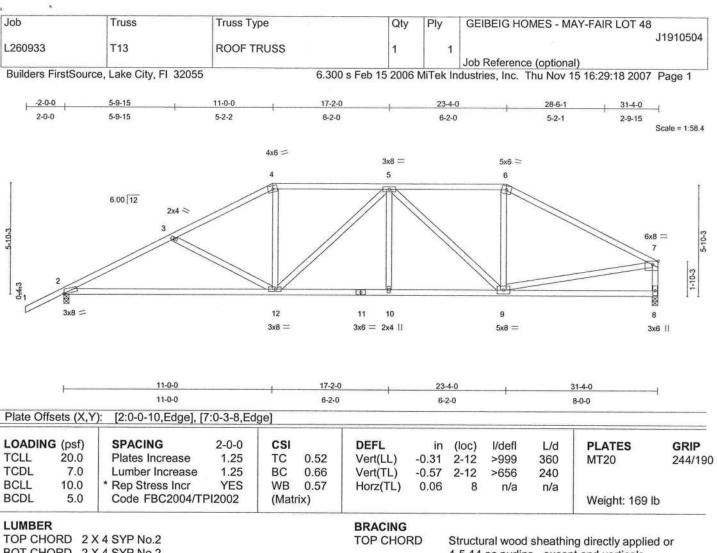
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 270 lb uplift at joint 8 and 265 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34889 1199 Coestal Bay Blvd. Boynton Besch, Ft. 33435





BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

7-8 2 X 4 SYP No.2

4-5-14 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-9-7 oc

bracing.

BOT CHORD

REACTIONS (lb/size) 2=1113/0-3-8, 8=989/0-3-8

Max Horz 2=147(load case 6)

Max Uplift 2=-285(load case 6), 8=-167(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1811/973, 3-4=-1502/831, 4-5=-1296/809, 5-6=-1102/723,

6-7=-1324/714, 7-8=-943/556

BOT CHORD 2-12=-853/1550, 11-12=-667/1393, 10-11=-667/1393, 9-10=-667/1393,

8-9=-167/218

WEBS 3-12=-302/280, 4-12=-119/387, 5-12=-254/119, 5-10=0/124, 5-9=-492/196,

6-9=-18/281, 7-9=-362/897

JOINT STRESS INDEX

2 = 0.89, 3 = 0.33, 4 = 0.73, 5 = 0.56, 6 = 0.74, 7 = 0.64, 8 = 0.47, 9 = 0.41, 10 = 0.33, 11 = 0.46 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Coffities is designed for C-C for members and forces, and for MWFRS for reactions specified.

Truss Design Engineer Florida PE No. 34869 1100 Casstal Bay Blvd Bovnton Basch, Ft. 33436

November 16,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or Itlls-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T13	ROOF TRUSS	1	1	J191050-
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:18 2007 Page 2

NOTES

3) Provide adequate drainage to prevent water ponding.

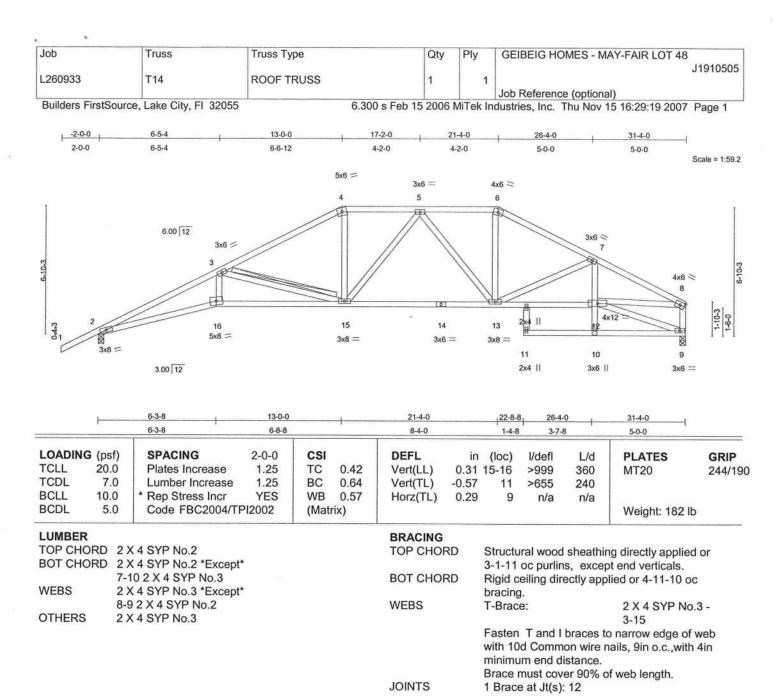
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 167 lb uplift at joint 8.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=1121/0-3-8, 9=1019/0-3-8

Max Horz 2=158(load case 6)

Max Uplift 2=-292(load case 6), 9=-164(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3387/1749, 3-4=-1782/963, 4-5=-1537/939, 5-6=-1389/840,

6-7=-1600/871, 7-8=-2147/1051, 8-9=-1022/543

BOT CHORD 2-16=-1584/3027, 15-16=-1510/2868, 14-15=-677/1527, 13-14=-677/1527,

12-13=-891/1896, 10-12=0/199, 7-12=-11/397, 10-11=0/0, 9-10=-47/17

3-16=-327/781, 3-15=-1389/855, 4-15=-166/459, 5-15=-159/94, 5-13=-325/142,

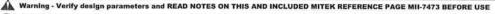
6-13=-173/436, 7-13=-571/338, 8-12=-808/1775, 9-12=-28/110

Trues Design Engineer Florida FE No. 34868 1109 Ceastel Bay Blvd Boynton Beach, FL 33436

JOINT STRESS INDEX

WEBS

2 = 0.77, 3 = 0.56, 4 = 0.58, 5 = 0.39, 6 = 0.57, 7 = 0.43, 8 = 0.72, 9 = 0.39, 10 = 0.38, 11 = 0.33, 12 = 0.77, 13 = 0.56, 14 = 0.54, 15 = 0.61, 16 = 0.90 and 17 = 0.33 November 16,2007 Continued on page 2



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T14	ROOF TRUSS	1	1	J1910505
			20		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:19 2007 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

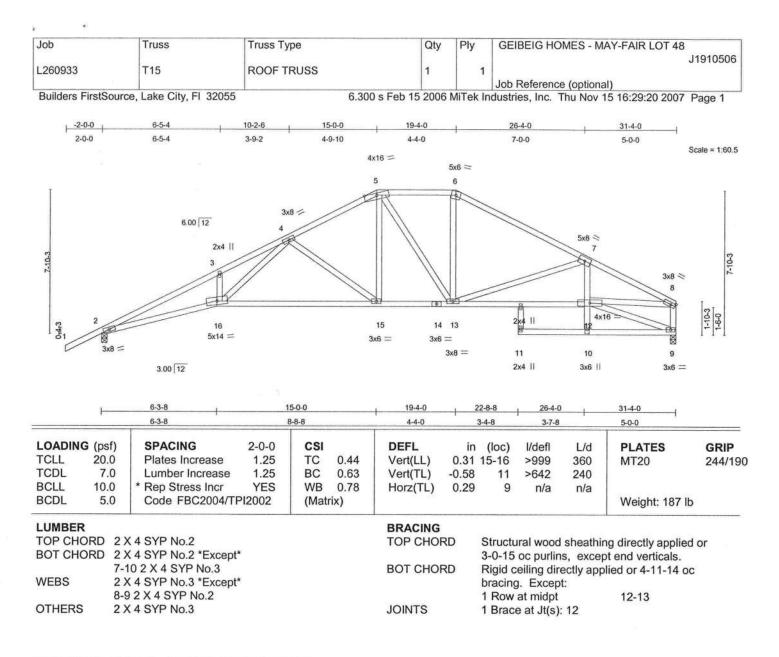
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 164 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonds PE No. 34866 1 100 Gessial Bay Blvd





REACTIONS (lb/size) 2=1121/0-3-8, 9=1019/0-3-8

Max Horz 2=170(load case 6)

Max Uplift 2=-302(load case 6), 9=-177(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3367/1728, 3-4=-3309/1864, 4-5=-1506/882, 5-6=-1260/827,

6-7=-1489/837, 7-8=-2160/1069, 8-9=-1019/541

BOT CHORD 2-16=-1560/3004, 15-16=-942/1862, 14-15=-543/1304, 13-14=-543/1304,

12-13=-951/1964, 10-12=0/196, 7-12=0/450, 10-11=0/0, 9-10=-81/17

3-16=-202/231, 4-16=-804/1449, 4-15=-708/503, 5-15=-263/497, 5-13=-228/107,

6-13=-113/368, 7-13=-748/452, 8-12=-840/1800, 9-12=0/147

Julius Design Engineer Truss Design Engineer Flonds PE No. 24869 1100 Ceastal Bay Blvd. Boynton Besch, FL 33435

JOINT STRESS INDEX

2 = 0.77, 3 = 0.33, 4 = 0.72, 5 = 0.63, 6 = 0.59, 7 = 0.51, 8 = 0.94, 9 = 0.38, 10 = 0.41, 11 = 0.33, 12 = 0.69, 13 = 0.57, 14 = 0.47, 15 = 0.34, 16 = 0.82 and 17 = 0.33

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T15	ROOF TRUSS	1	1	J1910506
L200303	1115	Kool IKoo	'		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:20 2007 Page 2

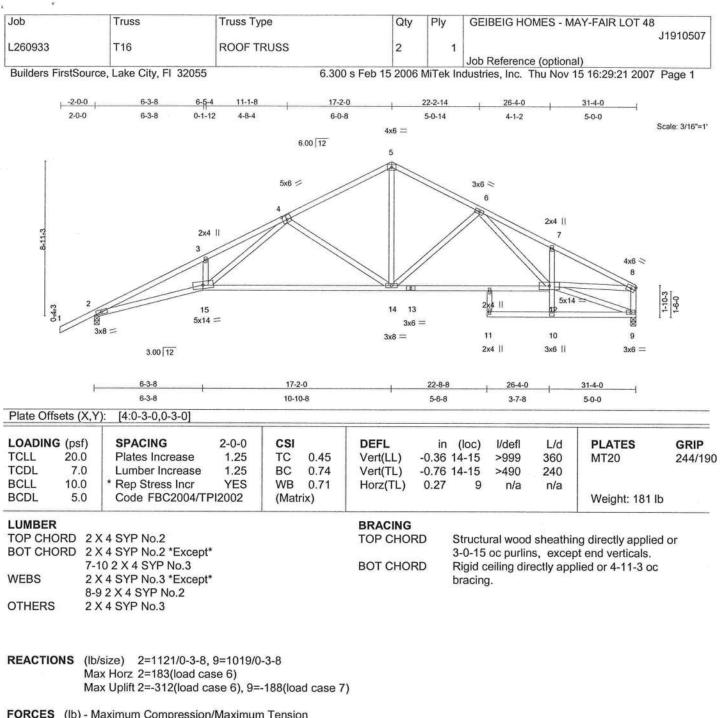
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 177 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34869 1109 Ceastal Bay Blvd





FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3381/1752, 3-4=-3330/1901, 4-5=-1327/806, 5-6=-1309/814,

6-7=-1970/1104, 7-8=-1964/988, 8-9=-1027/557

BOT CHORD 2-15=-1582/3019, 14-15=-894/1745, 13-14=-667/1420, 12-13=-667/1420,

10-12=0/194, 7-12=-232/238, 10-11=0/0, 9-10=-122/0

WEBS 3-15=-213/248, 4-15=-868/1565, 4-14=-748/554, 5-14=-462/803, 6-14=-428/315,

6-12=-208/552, 8-12=-748/1609, 9-12=-21/191

JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.69, 5 = 0.66, 6 = 0.39, 7 = 0.33, 8 = 0.75, 9 = 0.40, 10 = 0.45, 11 = 0.33, 12 = 0.47, 13 = 0.65, 14 = 00.56, 15 = 0.93 and 16 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T16	ROOF TRUSS	2	1	J1910507
	10	Moor mood			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:21 2007 Page 2

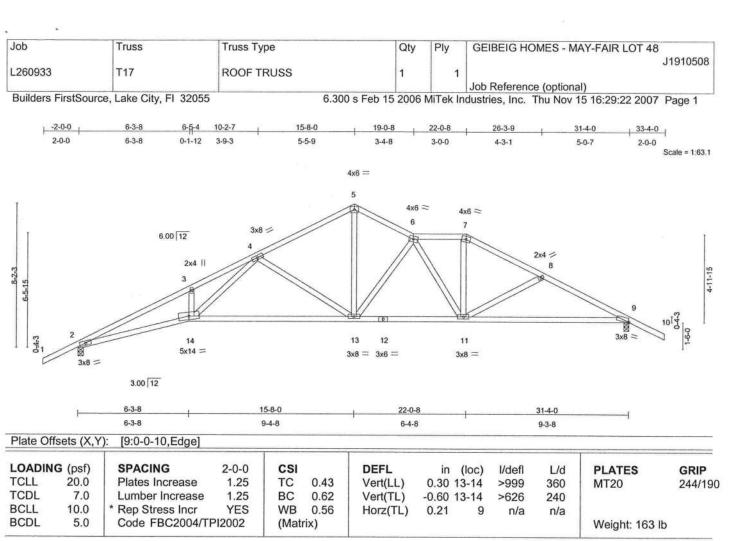
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 2 and 188 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 34869 1100 Cesstal Bay Blvd Boynton Beach, FL 33436





LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-1-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-2-13 oc

bracing.

REACTIONS (lb/size) 2=1109/0-3-8, 9=1109/0-3-8

Max Horz 2=158(load case 6)

Max Uplift 2=-311(load case 6), 9=-310(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=

1-2=0/46, 2-3=-3322/1665, 3-4=-3262/1801, 4-5=-1429/851, 5-6=-1378/861,

6-7=-1375/848, 7-8=-1576/884, 8-9=-1826/991, 9-10=0/47

BOT CHORD

2-14=-1416/2963, 13-14=-843/1831, 12-13=-628/1538, 11-12=-628/1538,

9-11=-717/1566

WEBS

3-14=-201/230, 4-14=-749/1437, 4-13=-732/506, 5-13=-532/933, 6-13=-542/355,

6-11=-314/188, 7-11=-208/452, 8-11=-246/219

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.72, 5 = 0.54, 6 = 0.34, 7 = 0.47, 8 = 0.33, 9 = 0.70, 11 = 0.57, 12 = 0.54, 13 = 0.56 and 14 = 0.84

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

৪১ নিমের্ভিক রব্দুপুরুষ্ট্র drainage to prevent water ponding.

Julius Lee Truss Design Engineer Flonda PE No. 34866 1406 Ceastal Bay Blvd Goynton Beach, FL 33435

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T17	ROOF TRUSS	1	1	J1910508
		11.000			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:22 2007 Page 2

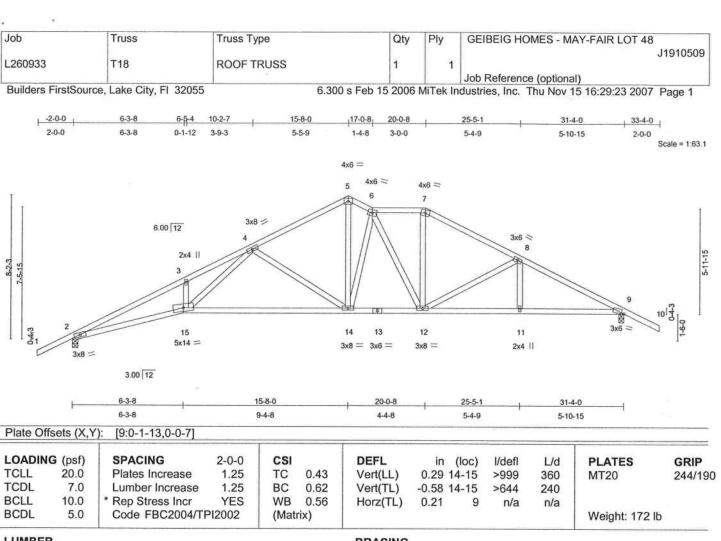
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 310 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 24869 1109 Ceastal Bay Sive





LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-1-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-2-14 oc

bracing.

REACTIONS (lb/size) 2=1109/0-3-8, 9=1109/0-3-8

Max Horz 2=158(load case 6)

Max Uplift 2=-311(load case 6), 9=-310(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/46, 2-3=-3322/1663, 3-4=-3263/1800, 4-5=-1432/849, 5-6=-1314/838,

6-7=-1250/823, 7-8=-1455/852, 8-9=-1835/957, 9-10=0/47

BOT CHORD

2-15=-1415/2963, 14-15=-839/1829, 13-14=-479/1316, 12-13=-479/1316,

11-12=-681/1565, 9-11=-681/1565

WEBS

3-15=-204/232, 4-15=-752/1438, 4-14=-725/503, 5-14=-478/867, 6-14=-439/248,

6-12=-222/58, 7-12=-153/343, 8-12=-379/261, 8-11=0/191

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.72, 5 = 0.59, 6 = 0.44, 7 = 0.60, 8 = 0.39, 9 = 0.76, 11 = 0.33, 12 = 0.60, 13 = 0.47, 14 = 0.67 and 15 = 0.47, 15 = 0.4= 0.83

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Coffiliase a designed for C-C for members and forces, and for MWFRS for reactions specified.

EL DOMOS

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occes. For general guidance regarding storage, delivery, erection and bracing, consult BCS-11 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T18	ROOF TRUSS	1	1	J1910509
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:23 2007 Page 2

NOTES

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

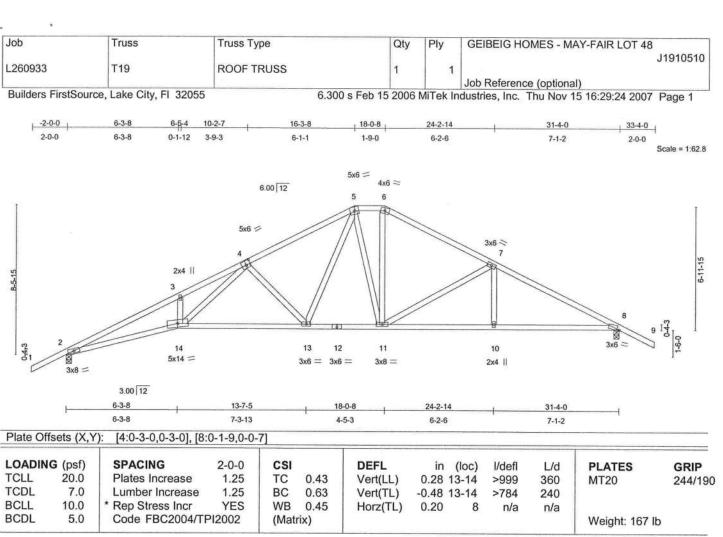
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 310 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Plonda PE No. 34869 1 100 Ceastal Bay Blvd





LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-1-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-2-15 oc

bracing.

REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

Max Horz 2=162(load case 6)

Max Uplift 2=-313(load case 6), 8=-293(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3302/1671, 3-4=-3237/1800, 4-5=-1618/972, 5-6=-1129/789,

6-7=-1336/806, 7-8=-1804/952, 8-9=0/47

BOT CHORD 2-14=-1422/2944, 13-14=-852/1846, 12-13=-387/1152, 11-12=-387/1152,

10-11=-664/1529, 8-10=-664/1529

WEBS 3-14=-189/217, 4-14=-742/1390, 4-13=-682/488, 5-13=-353/613, 5-11=-266/137,

6-11=-211/389, 7-11=-475/343, 7-10=0/223

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.56, 5 = 0.48, 6 = 0.62, 7 = 0.39, 8 = 0.78, 10 = 0.33, 11 = 0.67, 12 = 0.39, 13 = 0.53 and 14 = 0.82

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

8) মিনেগারিক কৰি প্রধান্ত drainage to prevent water ponding.

Julius Lee Truss Design Engineer Florida PE No. 34888 1409 Ceastel Bay Blvd Boynton Besch, FL 33495

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T19	ROOF TRUSS	1	1	J191051
LLUUUUU	1.10	Noor mood			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:24 2007 Page 2

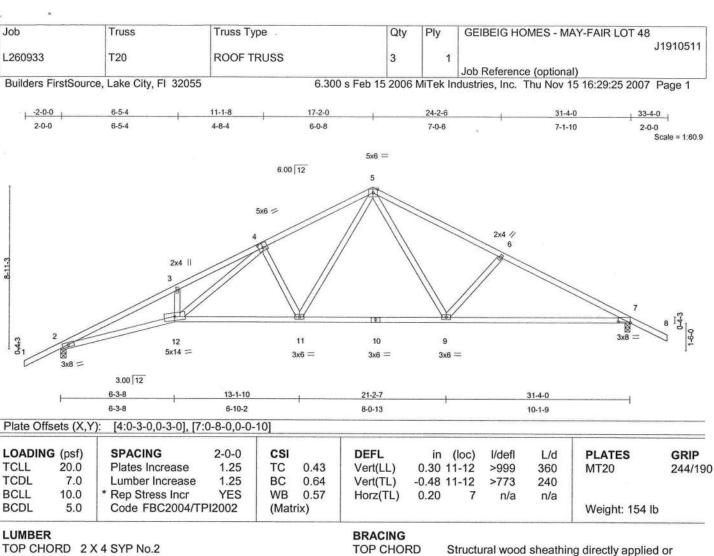
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2 and 293 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truss Cesign Engineer Florida PE No. 2-1868 1 100 Cestal Bay Blod Boynton Beach, FL 23435





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 TOP CHORD

3-1-7 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-2-8 oc

bracing.

REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=167(load case 6)

Max Uplift 2=-317(load case 6), 7=-298(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/46, 2-3=-3301/1691, 3-4=-3256/1843, 4-5=-1680/1043, 5-6=-1527/924,

6-7=-1783/981, 7-8=0/47

BOT CHORD

2-12=-1441/2944, 11-12=-775/1724, 10-11=-353/1082, 9-10=-353/1082,

7-9=-690/1516

WEBS

3-12=-219/254, 4-12=-843/1504, 4-11=-621/460, 5-11=-433/730, 5-9=-227/434,

6-9=-367/343

JOINT STRESS INDEX

Continued on page 2

2 = 0.76, 3 = 0.33, 4 = 0.67, 5 = 0.63, 6 = 0.33, 7 = 0.65, 9 = 0.43, 10 = 0.38, 11 = 0.57 and 12 = 0.86

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation authorities from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T20	ROOF TRUSS	2	1	J1910511
LEGOGG		TROOF TROOG			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:25 2007 Page 2

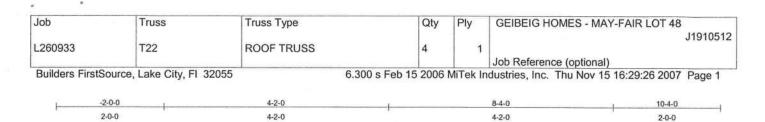
NOTES

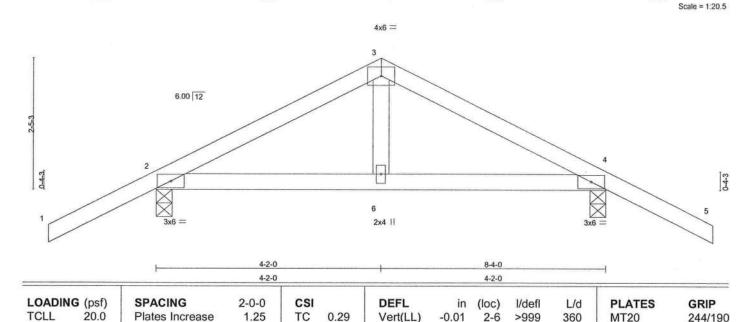
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2 and 298 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida Pie No. 34866 1100 Caastal Bay Blyd Boynton Beach, FL 30436







LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2

7.0

10.0

5.0

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 BRACING

Vert(TL)

Horz(TL)

-0.01

-0.00

TOP CHORD

Structural wood sheathing directly applied or

Weight: 36 lb

6-0-0 oc purlins.

>999

n/a

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

240

n/a

bracing.

2-6

REACTIONS (lb/size) 2=373/0-3-8, 4=373/0-3-8

Max Horz 2=-60(load case 7)

Lumber Increase

Code FBC2004/TPI2002

* Rep Stress Incr

Max Uplift 2=-260(load case 6), 4=-260(load case 7)

1.25

YES

BC

WB

(Matrix)

0.09

0.04

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-294/448, 3-4=-294/448, 4-5=0/47

BOT CHORD 2-6=-242/212, 4-6=-242/212

WEBS 3-6=-204/128

JOINT STRESS INDEX

2 = 0.57, 3 = 0.35, 4 = 0.57 and 6 = 0.09

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee Truse Design Engineer Florida PE No. 24888 1109 Caestal Bay Blvd Boyston Beach, Et 22428

November 16,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation authorities from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



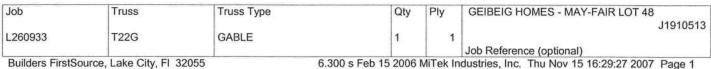
Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910512
L260933	T22	ROOF TRUSS	4	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:26 2007 Page 2

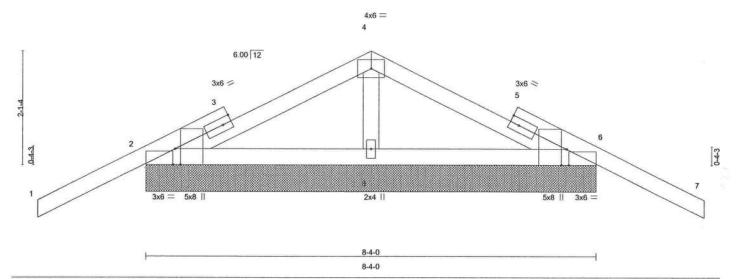
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 2 and 260 lb uplift at joint 4.

LOAD CASE(S) Standard









LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.01	7	n/r	120	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.02	7	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	6	n/a	n/a		
BCDL 5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		8-4-0 oc purlins.
OTHERS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc
			bracing.

REACTIONS (lb/size) 2=227/8-4-0, 6=227/8-4-0, 8=294/8-4-0

Max Horz 2=-63(load case 7)

Max Uplift 2=-204(load case 6), 6=-214(load case 7), 8=-69(load case 6) Max Grav 2=239(load case 10), 6=239(load case 11), 8=294(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-12/35, 3-4=0/98, 4-5=0/98, 5-6=-4/35, 6-7=0/47

BOT CHORD 2-8=-48/104, 6-8=-48/104

WEBS 4-8=-247/143

JOINT STRESS INDEX

2 = 0.39, 2 = 0.00, 3 = 0.00, 3 = 0.22, 4 = 0.30, 5 = 0.00, 5 = 0.22, 6 = 0.39, 6 = 0.00 and 8 = 0.09

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal Coffithe for page MiTek "Standard Gable End Detail"

November 16,2007



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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
	T000	0.00			J1910513
L260933	T22G	GABLE	1	1	TOTAL CES SI
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:27 2007 Page 2

NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2, 214 lb uplift at joint 6 and 69 lb uplift at joint 8.

LOAD CASE(S) Standard

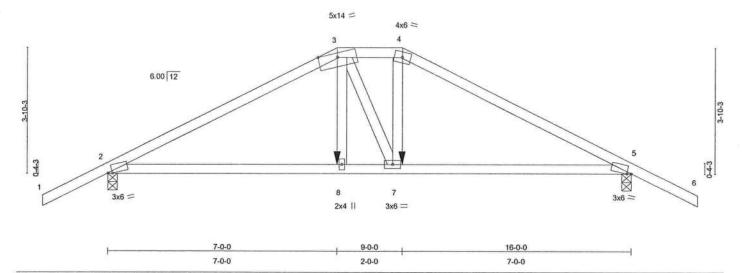
Julius Lee Truss Ossian Engineer Flondá FE No. 34889 1186 Caestal Bay Blos 4000000 Beach, FL 38435





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:27 2007 Page 1





LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.12	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.14	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 72 lb	

LUMBER	2
--------	---

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-4-10 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-5-5 oc

bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8

Max Horz 2=77(load case 5)

Max Uplift 2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/806, 5-6=0/47

BOT CHORD 2-8=-675/1504, 7-8=-684/1523, 5-7=-658/1507

WEBS 3-8=-262/480, 3-7=-146/159, 4-7=-303/592

JOINT STRESS INDEX

2 = 0.77, 3 = 0.87, 4 = 0.76, 5 = 0.77, 7 = 0.38 and 8 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Trues Design Engineer Florida PE No. 24868 1109 Ceastal Bay Blvd Boynton Beach, FL 33425

November 16,2007

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T23	ROOF TRUSS	1	1	J1910514
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:27 2007 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

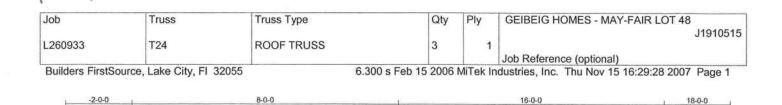
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

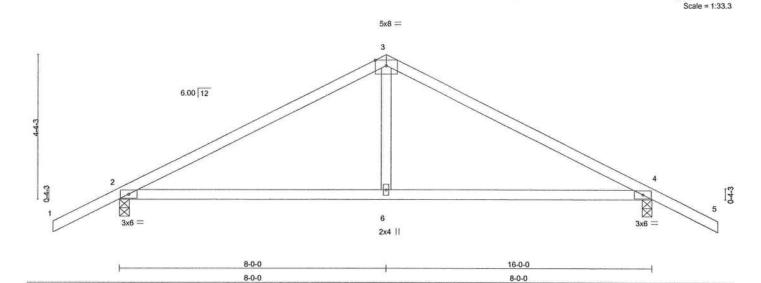
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10

Concentrated Loads (lb)

Vert: 8=-411(F) 7=-411(F)







LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.24	2-6	>779	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.14	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.14	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)						0.03000	Weight: 63 lb	

1		M	D		
_	u	IVI	D	_	м

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WFBS 2 X 4 SYP No.3

2-0-0

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

8-0-0

2-0-0

BOT CHORD

Rigid ceiling directly applied or 7-1-0 oc

bracing.

REACTIONS (lb/size) 2=619/0-3-8, 4=619/0-3-8

Max Horz 2=-83(load case 7)

Max Uplift 2=-404(load case 6), 4=-404(load case 7)

8-0-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-720/981, 3-4=-720/981, 4-5=0/47

BOT CHORD 2-6=-689/562, 4-6=-689/562

WEBS 3-6=-489/273

JOINT STRESS INDEX

2 = 0.69, 3 = 0.93, 4 = 0.69 and 6 = 0.19

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

November 16,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
1 000000	T04	DOOF TOUGO			J1910515
L260933	T24	ROOF TRUSS	3	1	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:28 2007 Page 2

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 4.

LOAD CASE(S) Standard

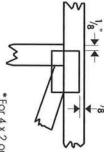


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless plates to both sides of truss and dimensions indicate otherwise securely seat Dimensions are in inches. Apply



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates

PLATE SIZE

4 × 4

perpendicular to slots. Second dimension is the length parallel The first dimension is the width

LATERAL BRACING



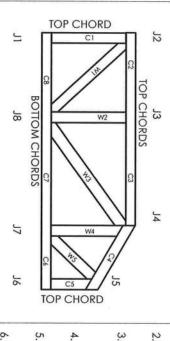
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA ICBO 96-31, 96-67

9667, 9432A 3907, 4922

SBCCI

WISC/DILHR 960022-W, 970036-N

561

NER





MiTek Engineering Reference Sheet: MII-7473

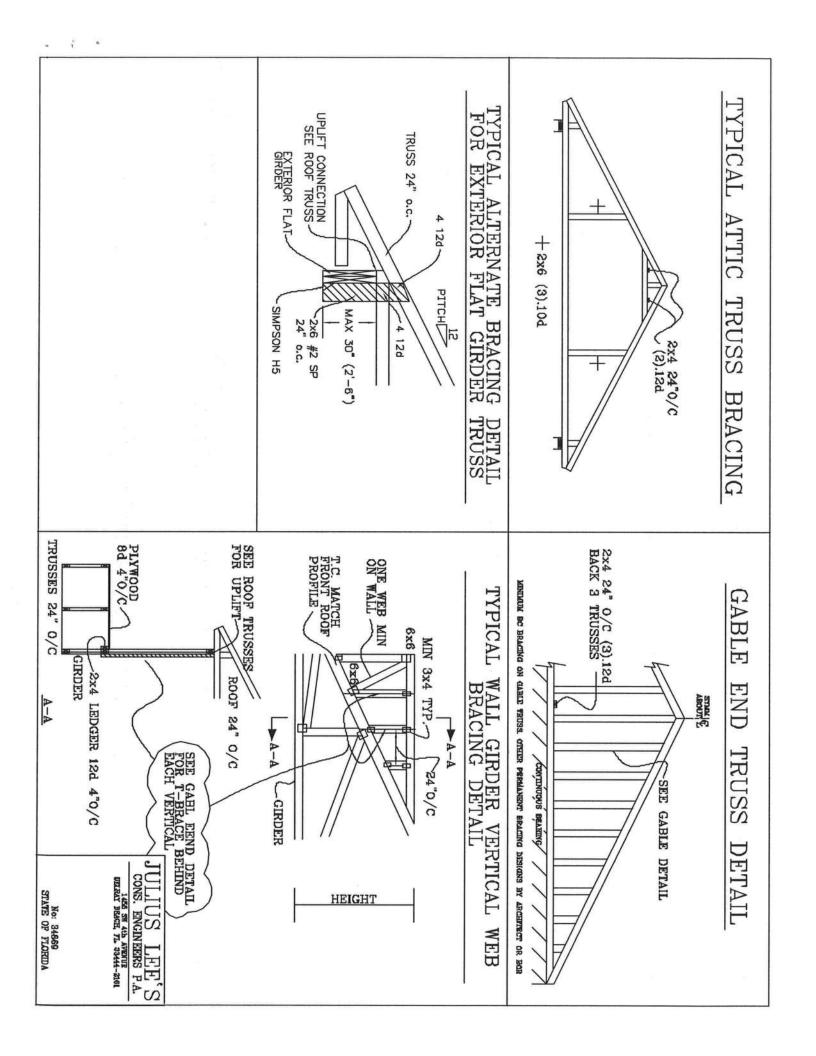
General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- 4 Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- 6 Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not preservative treated lumber. applicable for use with fire retardant or
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 00 Plate type, size and location dimensions shown indicate minimum plating requirements
- 9 grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Boffom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Anchorage and / or load transferring others unless shown connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials.
- Do not cut or after truss member or plate engineer. without prior approval of a professional
- Care should be exercised in handling, erection and installation of trusses.
- © 1993 MiTek® Holdings, Inc.

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED, CONNECT IMACONAL BRACE FOR SAGE AT RACH END. MAX WEB VERTICAL MAX GABLE LENGTH PENCIN IS 14. VERTICAL LENGTH SHOWN IN TABLE ASSOVE. SPACING SPECIES 24" 12" O.C. 16 O.C. O.C. CONNECT DIAGONAL AT GABLE VERTICAL SPF DEI SPF DFL SPF SP SP H Ħ ASCE STUD STANDARD #1 #2 #3 STUD #1 / #2 #8 STUD STANDARD #1 #2 #2 #3 STUD STANDARD \$1 / #2 \$4 \$1 / #2 STANDARD STANDARD GRADE STANDARD STUD HEA 3 が語は BRACE 7-02: ***WARDING*** TRUSSES REDUIRE EXTREME CARE IN FABRIDATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFEX TO SESS 1-40 (BULLING COMPIDENT SAFETY INFORMATION, PUBLISHED BY 1P) (TRUSS FLATE COM, MADISON, V. 25779) AND TITCA (VICID TRUSS COLORED OF METEORA, 6300 ENTERPRISE (UDICATED, TOP CAUDE STALL PRACTICES PRIOR TO PERTORNING THESE FINCTIONS, UNLESS OTHERVISE (UDICATED, TOP CAUDE STALL HAVE PROPERLY ATTACHED STRUCTURED. STRUCTURAL PARELS AND BUTTON CHORD SHALL HAVE A PROPERLY ATTACHED RECEIVED. #8 GABLE TRUBS BRACES 130 ZX4 EF #ZN, DF-L #Z, SPF #/#Z, DR BETTER DIAGONAL BRACT; ENVCLE OR DOUBLE CUT (AS SELTWA) AT GROUP A (1) 1X4 "L" BRACE * 4, 0<u>7</u> 0, 0, 0, MPH 0 7 7 0 0 0 0 0 4 5 0 0 7 7 0 0 0 0 0 0 4 5 0 GROUP B WIND (1) 2X4 "L" BRACE . GROUP A B B B B 7, 10, SPEED, GROUP B 9 5 5 10° REFER 15 18 H (2) 2X4 "L" BRACE ** GROUP A 10 6 5 10' 6" விவிவிலின் விவிவிவினி விவ்விவில் விவிவிவி MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH CONTINUOUS EXA MEN OR BETTER GROUP B 10' 5" 10' 5" 11' 2" 11' 2" 10 HEIGHT, 0 ULIUS LEE'S cons. ENGINEERS P.A. SNEWER (1) 2X0 GROUP A DELEVA BEACH L 23444-5161 10' 4" 10' 10" 50 No: 34869 STATE OF FLORIDA A r, ENCLOSED, GROUP B 14, 0 14, 0 14, 0 14, 0 14, 0 BRACE * Ñ GROUP A 20 20 22 Ľ. \mathbf{H} MAX MAX. HRACE GROUP B 13' 3" II 11' 8" 12 TOT. 1.00, SPACING 1 Ē ATTACH EACH "L" BRACE WITH 104 NAIS. * POR. (1) "L" BRACE, SPACE WAILS AP 2" O.C. * POR. (2) "L" BRACE, AND 4" O.C. BETTEN ZONES. ** FOR. (2) "L" BRACES: SPACE WAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETTEN ZONES. GABLE END SUPPOSTS LOAD FEON 4' 0" DUTLIDNERS WITH S' 0" DVENBANG, DR 12" PLYWOOD OVERHANG. PROVIDE UPLET CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). T." BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER: LENGTH. LIVE LOAD DEPLECTION CRITERIA IS L/240. SPRUCE-POUI-INB #1 / #2 STANDARD #3 STUD DOUGLAS FIR-LARCH BRACING GROUP SPECIES EXPOSURE CABLE TRUSS SOUTHLINGY PINE 60 GREATER THAN 1' 6' BUT LESS THAN 11' 8' GREATER THAN 11' 6' ARRIACAT TRACE 24.0" PRAY, SPLICE, AND HEEL PLATES. CERVCINELLE CABLE VERTICAL PSF DATE DRWG REF GROUP B: M W BLE GROUP DETAIL PLATE SIZES MITEX STD GABLE 16 E HI C 11/26/09 ASCR7-02-CAB13015 A: SOUTHERN PINE 23 23 NO SPLICE STANDARD STUD AND NOTES 7 STANDARD GRADES: COLS

			_
		MAX GABLE VERTICAL LENGTH 12" O.C. 16" O.C. 24" O.C. 25 g	
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		CONNESCT DIAGONAL AT THE STATE OF VERNICAL LENGTH SHOWN.	2
STRUCT	HANNAN HANNAN HANE HANE HANE HANE	GRADE GRADE GRADE \$1 #2	2
SOL PANELS	DIGEN TRUSS REFER TO CICA, 6900 EN UNCTIONS. L		
AND HOTTON CH	WAYARUNIA TRUSCE REBURE EXTREME CARE IN FARRICATING, HANDLING, SUPPING, INSTALLING AND BRACHM. BETER TO BEZE 1-43 GOULING COMPIDENT SAFETY (BIFDRATIDA), PURA MADIO BY TTY (TRUSCE PLATE INSTITULE), 393 THYRATRI IN, SULTE 20), MENISSAN, VIC SON'NO VITA, MADIO TRUSCE COLACTIO TARGICA, 6-300 ENTERPRISE LA, MIGIGON, VI SCHIED HAR THE PROPERTY ANTONEO TRUSCE (TARTICA).		
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VE A PROPERLY	TOP CHICA SHE	(1) 2X4 "L" BRACE & GROUP A GROUP B G. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6. 6.	
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OF FLORIDA	S LEE'S	BRACE • (2) 228 1 GROUP B GROUP A 10 7 12 12 3 9 1 1 12 3 9 1 1 12 3 11 1 1 12 3 8 9 1 1 12 3 11 1 1 12 3 8 9 1 1 12 3 11 1 1 12 3 8 9 1 1 12 3 11 1 1 12 3 11 1 1 12 3 11 1 1 12 3 11 1 1 12 3 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
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MAX. TOT. LD.		HAACE 11 12 13 2 11 12 13 2 11 12 13 2 2 11 14 0 0 11 14	
LD. 60 PSF	REF 1 DATE 1	BRACING GROUP SPEN GROUP SPENCE-PING-PIN FILL JAZ STANDARD FOR 12 STANDARD GROUN GROUN GROUN GROUN GROUN GROUN GROUN HEX-P FILL BOUTHERY PINS FILL SOUTHERY PINS FILL SOUTHERY PINS FILL GABLE TRUSS D CABLE TRUSS D CABLE TRUSS D FOR 13 "L" BRACE N FOR (2) "L" BRACE N FOR (3) "L" BRACE N FOR (3) "L" BRACE N FOR (4) "L" BRACE N FOR (5) "L" BRACE N FOR (6) "L" BRACE N FOR (7) "L" BRACE N GABLE VERTICAL L'NOTH TERMY A' 0"- LESS THAN 1." 0"- LESS THAN 1." 0"- LESS THAN 1." 0"- GREATER TO COLMON 1 FRANK, SPALER, AND N	TOPOGLED
	REF ASCEY-02-GAB13030 DATE 11/26/09 DWG myek 57D gable 30' e ht	CIES AND GRADES: A: HEM-FIR #2 STUD #3 STUD STUD #3 STUD #4 STUD STUD STUD #4 STUD #4 STUD #4 STUD #4 STUD #4 STUD STUD #4 STUD STUD #4 #4 STUD STUD #4 STUD #4 STUD STUD #4 #4 STUD STUD #4 #4 STUD #4 STUD #4 STUD #4 STUD #4 STUD #4 STUD #4 #4 STUD #4 STUD #4 STUD #4 STUD #4 STUD #4 #4 #4 STUD #4 #4 ** ** ** ** ** ** ** **	



BOT CHORD 2X4 2X4 200 222 BETTER BETTER

PIGGYBACK DETAIL

TYPE

SNAGS

å

\$

30

2

88

58

REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURILINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED HENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO BUGINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BIDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TO DIE-6 PSF, WIND BC DI=6 PSF
110 MPH WIND, 30' MEAN HGT, FEG
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TO DI-5 PSF, WIND BC DI-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND HC DL=6 PSF

Ħ C

584

8

6XG

Ħ >

4X8

8XB

500

BX6

284

2.5X4

2.6X4

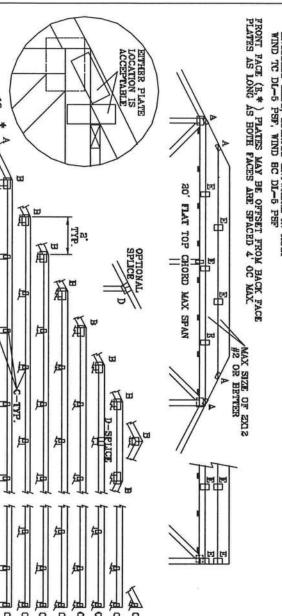
335

1.5X8

1.5X4 9X9

1.5X4

1.5X4



2 m	0' TO 7'9"	WEB LENGTH	
1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB	NO BRACING	REQUIRED BRACING	WEB BRACING CHART

ATTACH THULOX PLATES WITH (8) 0.120" X 1.575" NAILS, OR EQUAL PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR THULOX

4XB OR SXB TRULOX AT 4'
HOTATED VEHTICALLY

00,

INFORMATION

MED LENGTH	REQUIRED BRACING
o' To 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES MEMBER. OR BETTER, AND 80% LENGTH MEMBER. ATTACH WITH 8d NAILS AT 4"
10' TO 14'	2x4 "T" BEACE, SAME GRADE, SPECIES MEMBER, OR BETTER, AND 80% LENGTH MEMBER. ATTACH WITH 16d NAILS AT 4

	ACE PER PLY. A			٥	٥	
4 OC OR LESS.	20° X 1.375" NAILS PER FACE PER PLY. 4' OC OR LESS.	0	0	a	٥	
4 OC OR LESS.	BACK SPECIAL PLATE TO EACH TRUSS FACE 4 OC OR LESS.	•	٥	0	٥	(
4 0C 0R	20" X 1.975" NAILS PER FACE PER PLY. BACK SPECIAL PLATE TO EACH TRUSS FACE 4' OC OR LESS.	۰	0	٥	•))
֡	BACK SPECIAL PLATE TO EACH TRUSS FACE				ESS.	4 OC OR

ULIUS LEE'S CONS. ENGINEERS P.A. MAX LOADING DATE REF 834,017 & 847,045

THIS DRAWING

REPLACES DRAWINGS

634,016

8 1/4"

DINEARY BRACH, IL. 33444—2161 No: 34868 STATE OF FLORIDA SPACING 1.15 1.33 DUR. 1.25 DUR. 47 PSF 15 DUR. 50 PSF 55 PSF AT 33 DUR. FAC. 24.0 AT FAC. AT FAC. DRWG MITEK STD H 09/12/07 PIGGYBACK PIGG)

SWAZNAKINGSM TRACSICS REQUIRE COTRIDE EARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BAACING REFER TO INCID INTO GUILLING COMPONENT AREIT INFORMITHAN, PUBLICINED BY TPI CIRILISS PACING TO INCID I

*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

MAX V 12

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 2X4 SP #3 OR BETTER. OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF. FHC 2004 110 MPH, ASCE 7-02 110 MPH WIND ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR OR (3) 16d FOR ENCLOSED

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VALLEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

* ++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

LARGER AS REQ'D

12 MAX. WZX4

W2X4

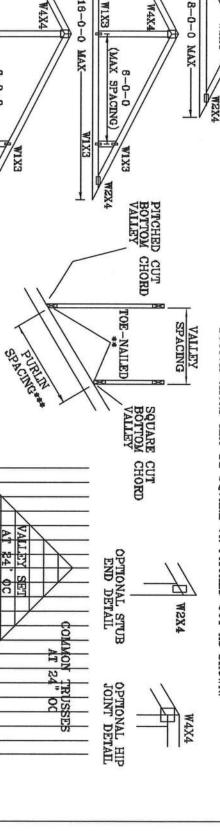
12

4-0-0

MAX

NOT EXCEED 12'0"

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



W1X3

MIX3

12 MAX.

12

W4X4

12 MAX

12

₩4X4

W1X3 W1X3

W1X3 W5X4/SPL

(MAX SPACING

W1X3

W2X4

COMMON TRUSSES

24

00

PARTIAL FRAMING

6-0-0

STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT
STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT
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SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.

20-0-0 MAX

(++)

No: 34869 STATE OF FLORIDA			DECLING.	1455 SW 41 PERFORMAN DELIKAT BEACH, 1	CONS	SULIUL	
PLORIDA	9			W AUG. AVENUE	ENGINEERS P.A.	S, HHI	
SPACING	DUR.FAC. 1.25	TOT. LD.	BC LL	BC DL	TC DL	TC II	
	EST.	32 40	0	U	-Z	20	HI
24"	1.25	40	0	Ç	15	20	THIS DRAWING
		PSF	PSF	PSF	PSF	PSF	PING
			PSF -ENG JL	PSF DRWG	PSF DATE	PSF REF	REPLACES
			JL .	VALTRUSS1103	11/26/09	VALLEY DETAIL	ES DRAWING A105

TOE-NAIL DETAIL

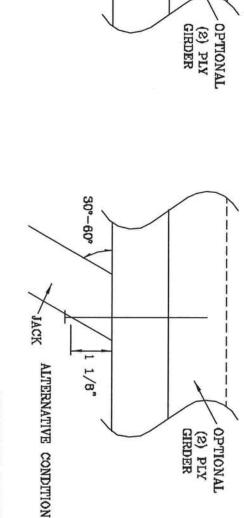
TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

5 493#	4 394#	3 296#	2 197#	TOE-NAILS 1 I	-3
3#	4#	8#	7#	1 PLY	SOUTHERN PINE
639#	511#	383#	256#	2 PLIES 1 PLY	N PINE
452#	361#	271#	181#		DOUGLAS
585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
390#	312#	234#	156#	1 PLY	
507#	406#	304#	203#	2 PLIES	HEM-FIR
384#	307#	230#	154#	1 PLY	SPRUCE
496#	397#	298#	189#	2 PLIES	SPRUCE PINE FIR



1/B

JACK

THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RECED BEILING	TE, 583 I'ONDERCI DR., SUITE 200, NADISON, WI 337(9) AND VICA (WOOD TRUS 300 ENTERPRISE LM, NADISON, VI 337(9) FIR SAFETY PRACTICES PRIDE TO PER 300 ENTERPRISE CM, NADISON, VI 337(9) FIR SAFETY PRACTICES PRIDE TO PER	BRACING. RETER TO BISSI 1-43 CHULLING COMPONENT SAFETY (NYTHANDLING, SHIPPING, DISTALLING AND BRACING, RETER TO BISSI 1-43 CHULLING COMPONENT SAFETY (NYTHANDLING, SHIPPING, DISTALLING AND	
STATE OF FLORIDA	No: 34869			DELPAY SEACH, PL SSHIII-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF REF
			-ENG JL	DRWG	DATE	REF
			JL	DRWG CNTONAIL1103	DATE 09/12/07	TOE-NAIL

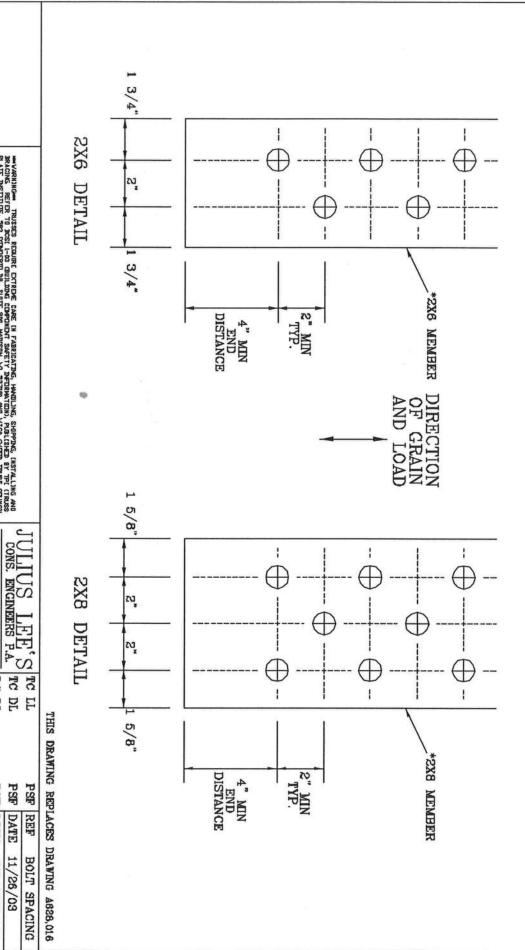
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PAITERNS SHOWN BELOW. APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



C

DELEGY SEACH, FL 33444-2161

BC DL TC DL

PSF PSF

DRWG -ENG

CNBOLTSP1103

DATE

11/26/03

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No: 34869 STATE OF FLORIDA

SPACING DUR. FAC TOT. LD.

TRULOX CONNECTION

SHOWN (+). 11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE

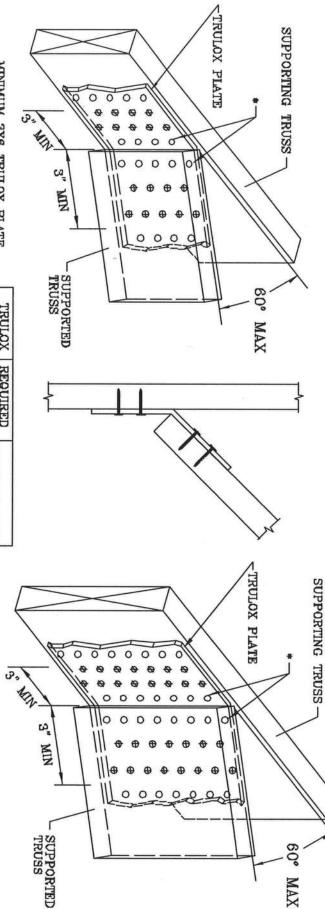
NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN THIS DETAIL FOR LUMBER, PLATES, AND OTHER

MAX



MINIMUM 3X6 TRULOX PLATE

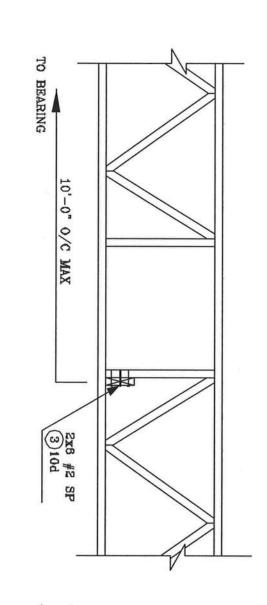
TRULOX PLATE SIZE 3X6 5X8 REQUIRED PER TRUSS NAILS 16 9 MAXIMUM LOAD UP OR DOWN 990# 350#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

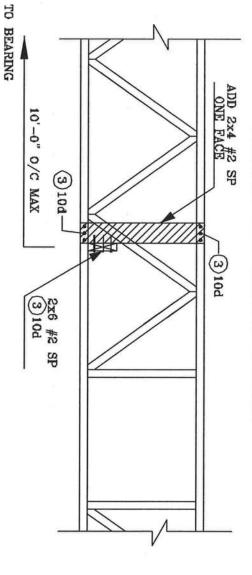
MINIMUM 5X6 TRULOX PLATE

	PUSE MUSICALED, THE CHURC SHALL HAVE PROPERTY ATTACHED RIGHT CELLING	DR. SUITE EDG, NADISCH, VI. 33719) AND VICA (VIIII) TRUSS COUNCIL MADISCH, VI. 33719) FOR SAFETY PRACTICES PRIOR TO PERFURNING	DITUDE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND DITUDE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND		
No: 34869 State of Florida		DELEAT BEACH, IL SEASON SIED		JULIUS LEE'S	
7	-ENG JL	DRWG CNTRULOX1103	DATE 11/26/03	REF TRULOX	

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S cons. engineers p.a.

No: 34869 STATE OF FLORIDA c^{y}

