

CONNECTOR TABLE					
Uplift SP		Truss Connector		To Plate	To Truss/Rafter
806	505	SDWC15600	-	-	-
416	290	H3	4-8x1 1/2"	4-8x1 1/2"	
615	540	H2 5A	5-8x1 1/2"	5-8x1 1/2"	
1340	1015	H10A	9-1x1 1/2"	9-1x1 1/2"	
720	820	LT5-12-20	8-10x1 1/2"	8-10x1 1/2"	
1000	860	MT5-12-30	7-10x1 1/2"	7-10x1 1/2"	
1450	1245	HT5-12-30	12-10x1 1/2"	12-10x1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LS7A21	8-10x1	8-10x1	
1640	1465	MS7A28	9-10x1	9-10x1	
1030	1030	CS20	7-10x1	7-10x1	
Uplift SP	Uplift SPF	Steel Plate Ties	To Stud	To Plate	
585	535	SP1	8-10x1	4-10x1	
1065	805	SP2	8-10x1	8-10x1	
771	771	LS7A24	10-10x1	wrap under or over plate	
1235	1235	LS7A24	14-10x1	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stewrnall	Stud / Post	Anchor	
1825	1800	DT722	8-SDS 14"x1 1/2"	12"x12" Titen HD	
4235	3640	HT74	18-16x52 1/2"	12"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	Stud / Post	Anchor	
1825	1800	DT722	8-SDS 14"x1 1/2"	12"x12" Titen HD	
4235	3640	HT74	18-16x52 1/2"	12"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stewrnall to Mono	Stud	Anchor	
1900		ABJ442	12-16x1	5/8"x12" Drill & Epoxy	
2475		ABJ662	12-16x1	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	Stud	Anchor	
1900		ABJ442	12-16x1	5/8"x7" Drill & Epoxy	
2475		ABJ662	12-16x1	5/8"x7" Drill & Epoxy	

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

**THIS STUD BEARING TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)**

(1) 2x4 @ 16" OC	To 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	To 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	To 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	To 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE		
	Fd	E
2x8	SP #2	925 1.4
2x10	SP #2	800 1.4
2x12	SP #2	750 1.4
GLB	24F-V3 SP	2600 1.9
LSL	TIMBERSTRAND	1700 1.7
LSL	MICROLAM	2950 2.0
PSL	PARALAM	2900 2.0

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FLOOR TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT, AND CONNECTIONS TO BEARING LOCATIONS. TRUSSES TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S DESIGN OF TRUSSES AND CONNECTIONS. THE TRUSS MANUFACTURER IS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR DESIGN OF TRUSSES AND CONNECTIONS. TRUSS MANUFACTURER'S SYSTEMS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION. CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VARIATION OBSERVATION OR SOILS TEST PROVIDES OTHER BEARING CAPACITY).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI. WELDED WIRE REINFORCED SLAB: #8 @ 1' x W1.4 @ 1' x W1.4 @ 2' BS. DEPENDS ON THE SLAB, SUPPORTED WITH APPROVED METHODS OR SUPPORTS AT SPACINGS NOT TO EXCEED 7'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT SHALL BE LESS THAN 12" TO 12" TO INCREASE DOSEAGE AMOUNTS FROM 0.75 TO 1.0 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. REBAR TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302.2. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT AND NOT LESS THAN 12" TO 12" TO INCREASE DOSEAGE AMOUNTS FROM 0.75 TO 1.0 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ENGINEER'S DISCRETION. CONTROL JOINTS SHALL BE PLACED TO AVOID CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 40 KSI. ALL LAP SPICES 40" / DE. 2' MIN. OVERLAP. REBAR DETAIL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-96, U. N. 1.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNLESS NOTED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTIONS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTION TO BEARING LOCATIONS AND UPLIFT LOCATIONS. FOR EXAMPLE, NOT ENFORCEMENT AN EQUIVALENT TYPE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED WITHOUT THE MANUFACTURER'S WRITTEN APPROVAL. THE REQUIRED UPLIFT AND REACTION LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: 3/32" ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN ANCHOR BOLT MANUFACTURER'S LITERATURE. ANCHOR BOLTS TO BE SET IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER SHALL BE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND SURFACE SLOPE. THE WIND LOAD ENGINEER IS NOT RESPONSIBLE FOR THE DETAILS OF THE FOUNDATION DESIGN.

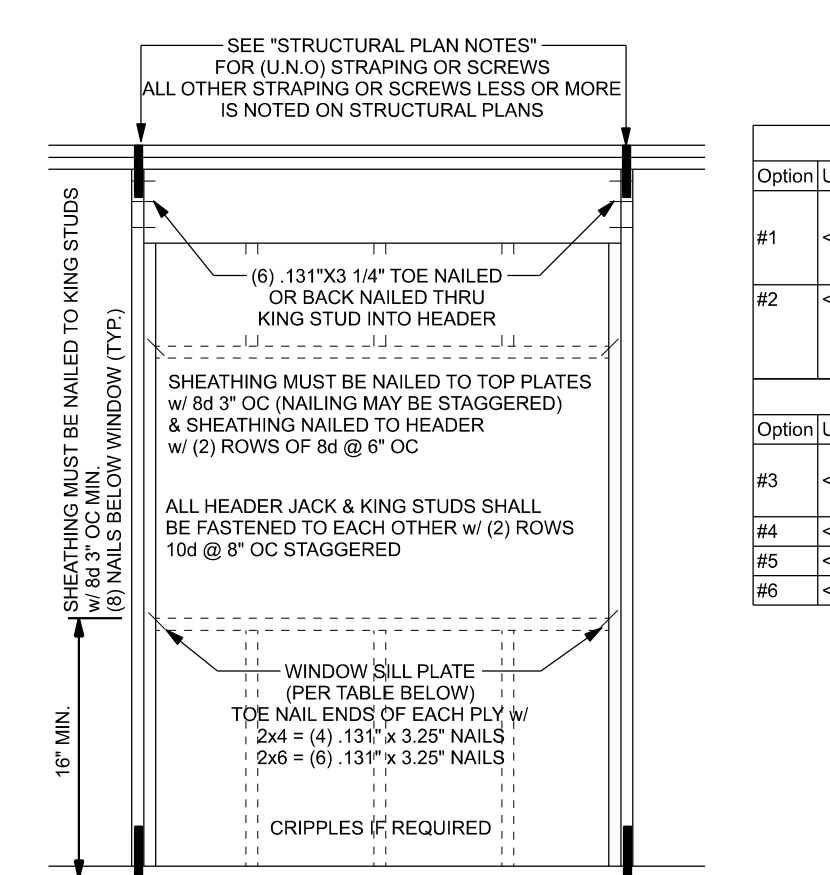
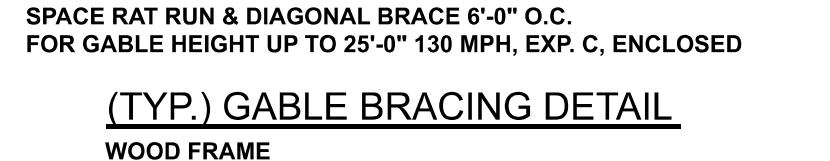
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FLOOR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER FOR CLARIFICATION.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BEARING DETAILS, TRUSSES TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

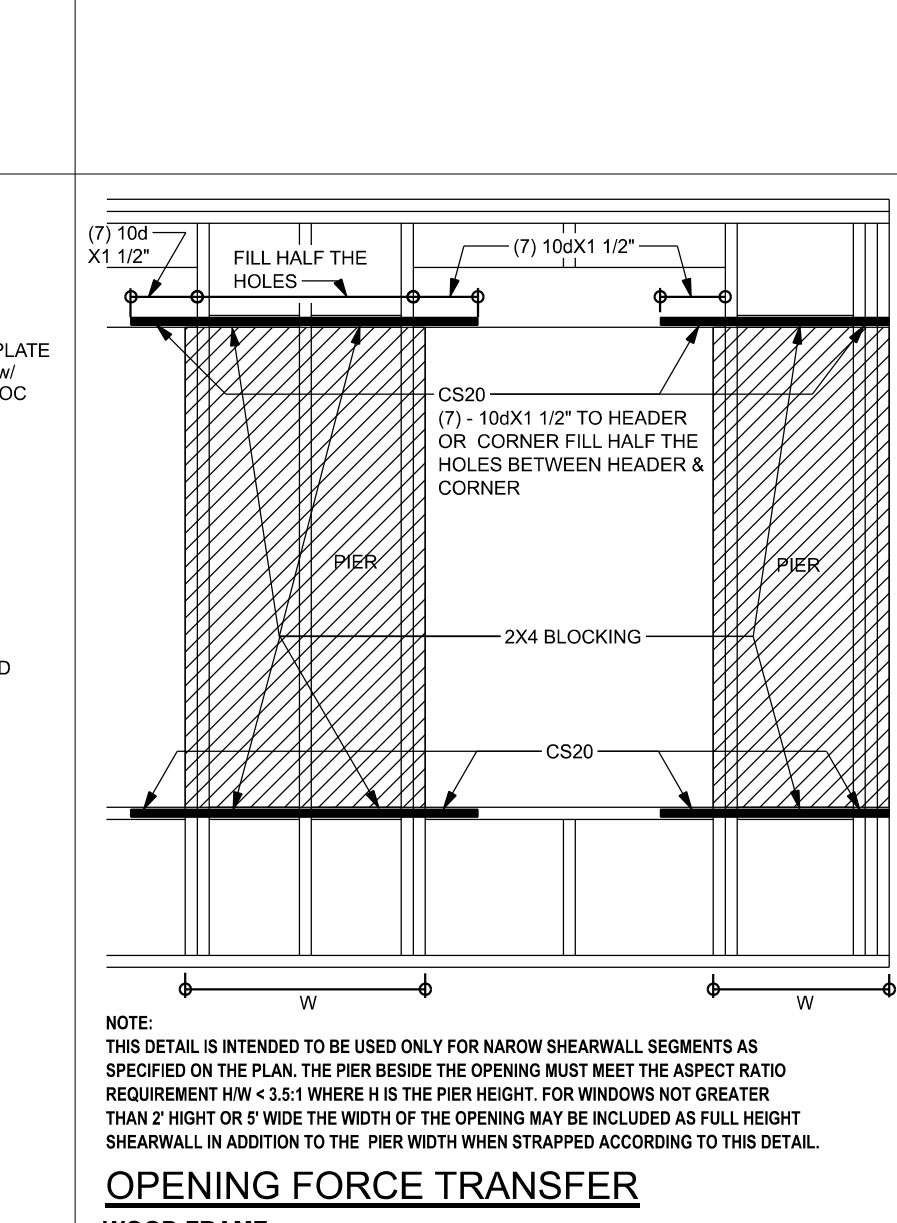
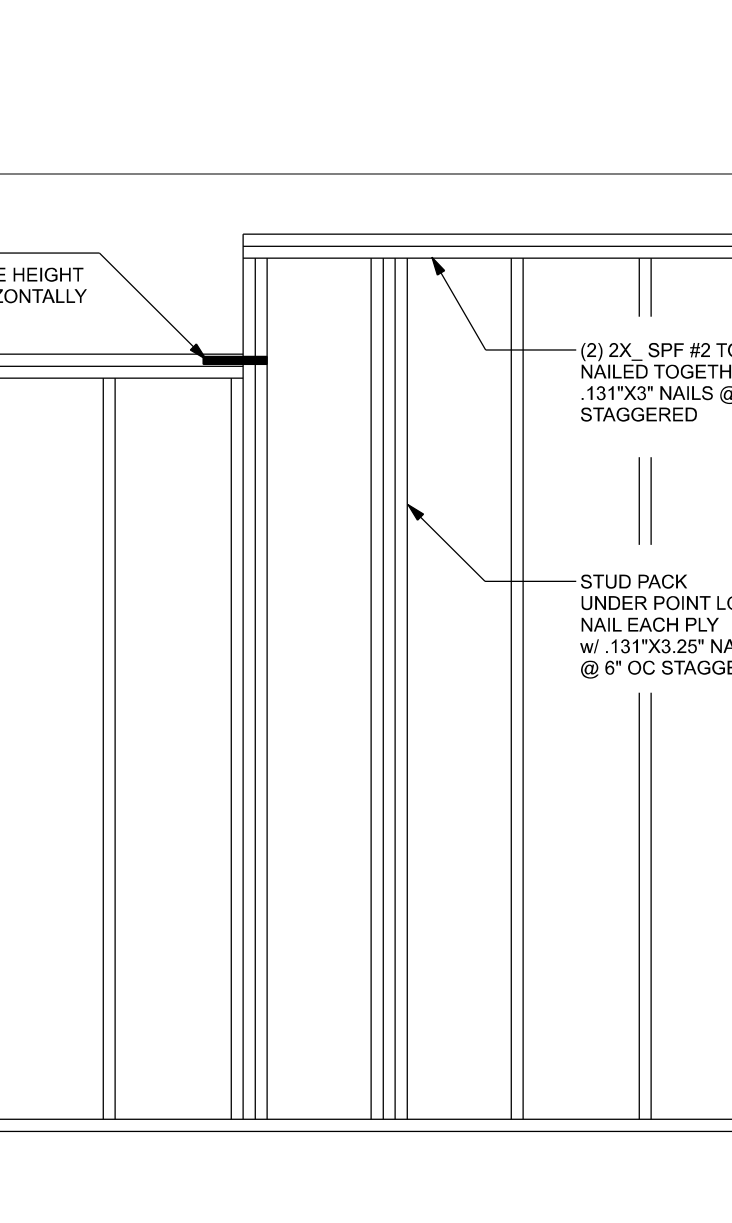
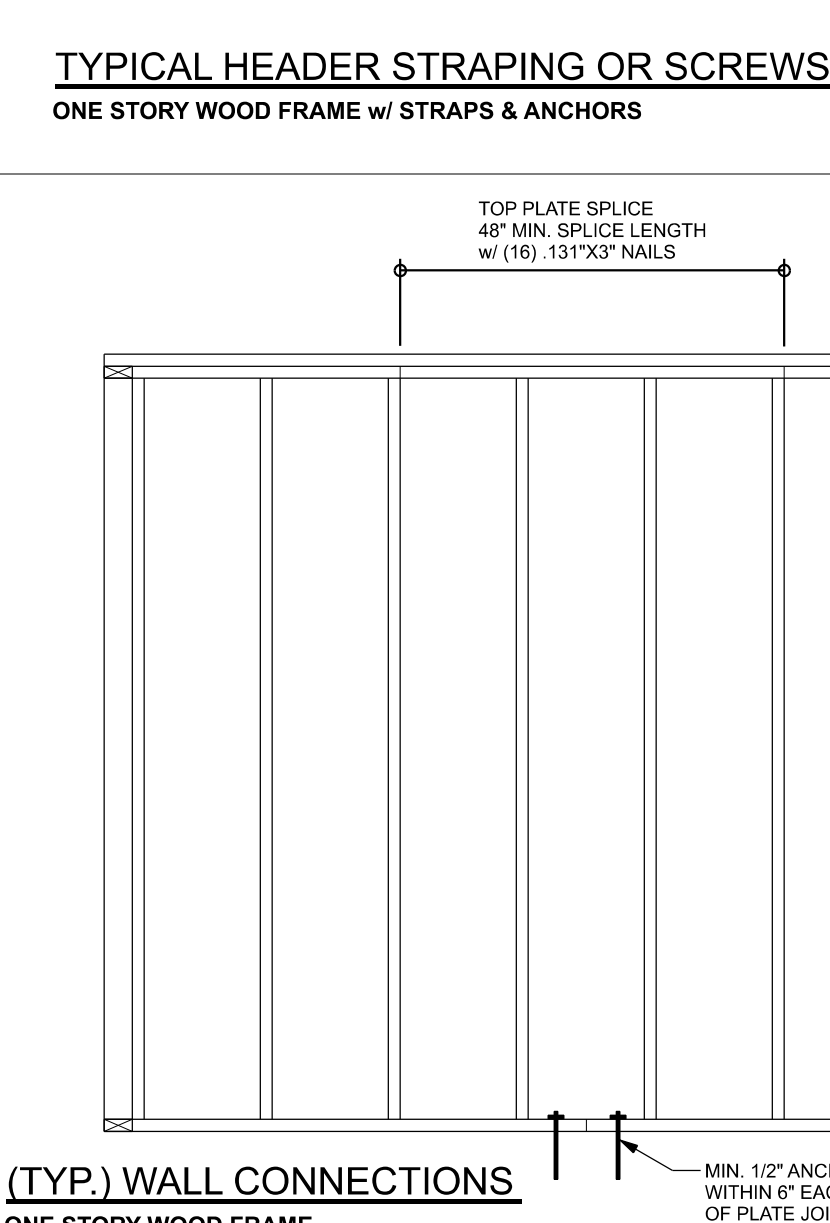
**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO OBTAIN ALL THE DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND TO OBTAIN THE REQUIRED UPLIFT AND REACTION LOADS FROM A PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIREMENTS.



<b>HEADER SCREWS TABLE</b>				
Top Connection		Bottom Connection		
Attach King stud to top plate w/ (1) Simpson SDWC1560	Attach king stud to bottom plate w/ (2) Simpson SDWC1545 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud at all door locations			
Attach king stud to top plate w/ (2) Simpson SDWC1560	Attach king stud to bottom plate w/ (3) Simpson SDWC1545 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud at all door locations			
<b>HEADER STRAP TABLE</b>				
Top Connection		Bottom Connection		
L5	MSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud at all door locations		
M5	MSTA24, 18-10d header to jacks	DTT22		
(2)	MSTA24, 18-10d header to jacks	DTT22		
(2)	MSTA24, 18-10d header to jacks	HTT4		

SILL PLATE SPANS FOR 10"-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.238
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
50-MPH (130-MPH)	6'-10"	7'-0"	10'-2"	14'-0"	444'-0"



**2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT**

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 131X3 1/4" ON PER TABLE BELOW:

DOOR WIDTH	3/8"x4" LAG	16d STAGGER	(2) ROWS OF 131X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC

2X6 SP #2 DOOR BUCK

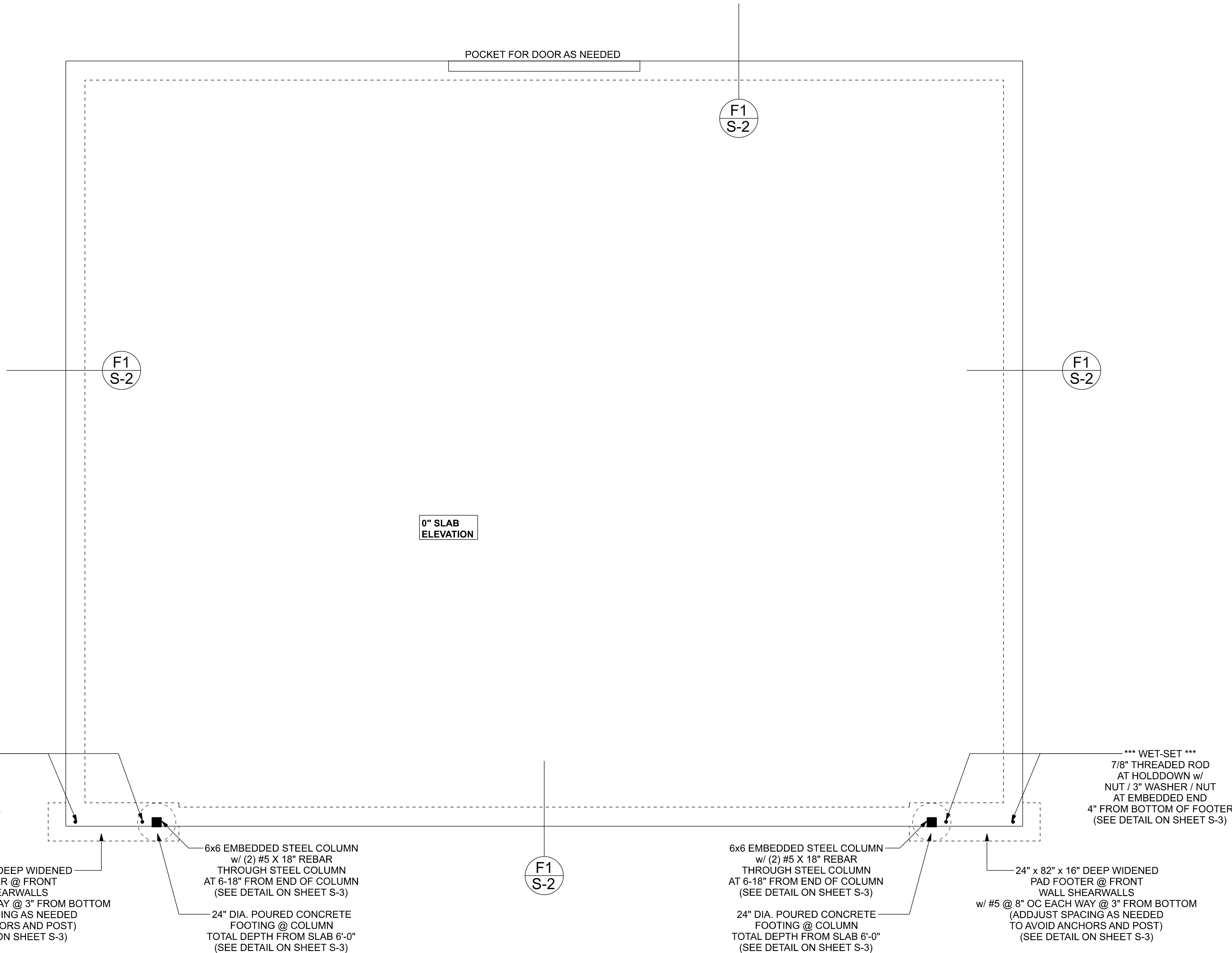
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(TYP.) GARAGE DOOR BUCK INSTALLATION

WOOD FRAME

<b>DESIGN CATEGORY &amp; LOADS:</b>			
BUILDING CODE		7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)	
CODE FOR DESIGN LOADS		ASCE 7-16	
<b>WINDLOADS</b>			
WIND MAX SPEED (ASCE 7-16, 38 GUST)		130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)		C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)		I	
RISK CATEGORY		II	
ENCLOSURE CLASSIFICATION		ENCLOSED	
INTERNAL PRESSURE COEFFICIENT		0.18	
ROOF ANGLE		7-45 DEGREES	
MEAN ROOF HEIGHT		30 FT	
<b>C&amp;C DESIGN PRESSURES</b>		SEE TABLE	
<b>FLORID LADING</b>			
ROOMS OTHER THAN SLEEPING ROOM		40 PSF LIVE LOAD	
SLEEPING ROOMS		30 PSF LIVE LOAD	
<b>ROOF LADING</b>			
FLAT ROOF		20 PSF LIVE LOAD	
4:12 TO 12:12		16 PSF LIVE LOAD	
12:12 & GREATER		12 PSF LIVE LOAD	
<b>SOIL BEARING CAPACITY</b>		1500 PSF	
<b>FLOOD ZONE</b>		THIS BUILDING IS NOT IN THE FLOOD ZONE	



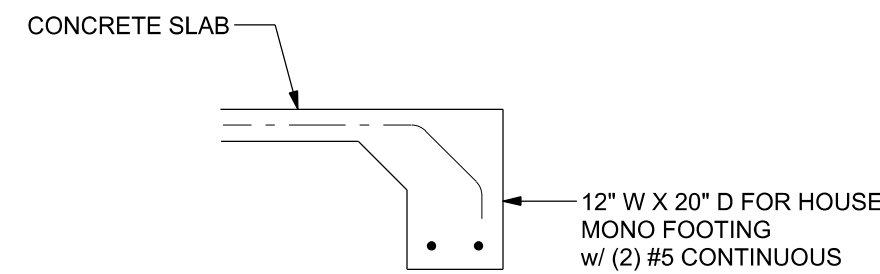


#### FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

#### FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN - 2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN.
- FN - 3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6x6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE-APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).



F1  
S-2

MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL

Bryan Zecher Construction

Tim Marshall Hanger

PROJECT ADDRESS:  
160 SW Orange Blossom Ct.  
Lake City, Florida

FL PE 53915  
This item has been digitally signed and sealed by Mark Disoway P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01410C0000017E97DE07CA000746F0, CN=Mark d Disoway  
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DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

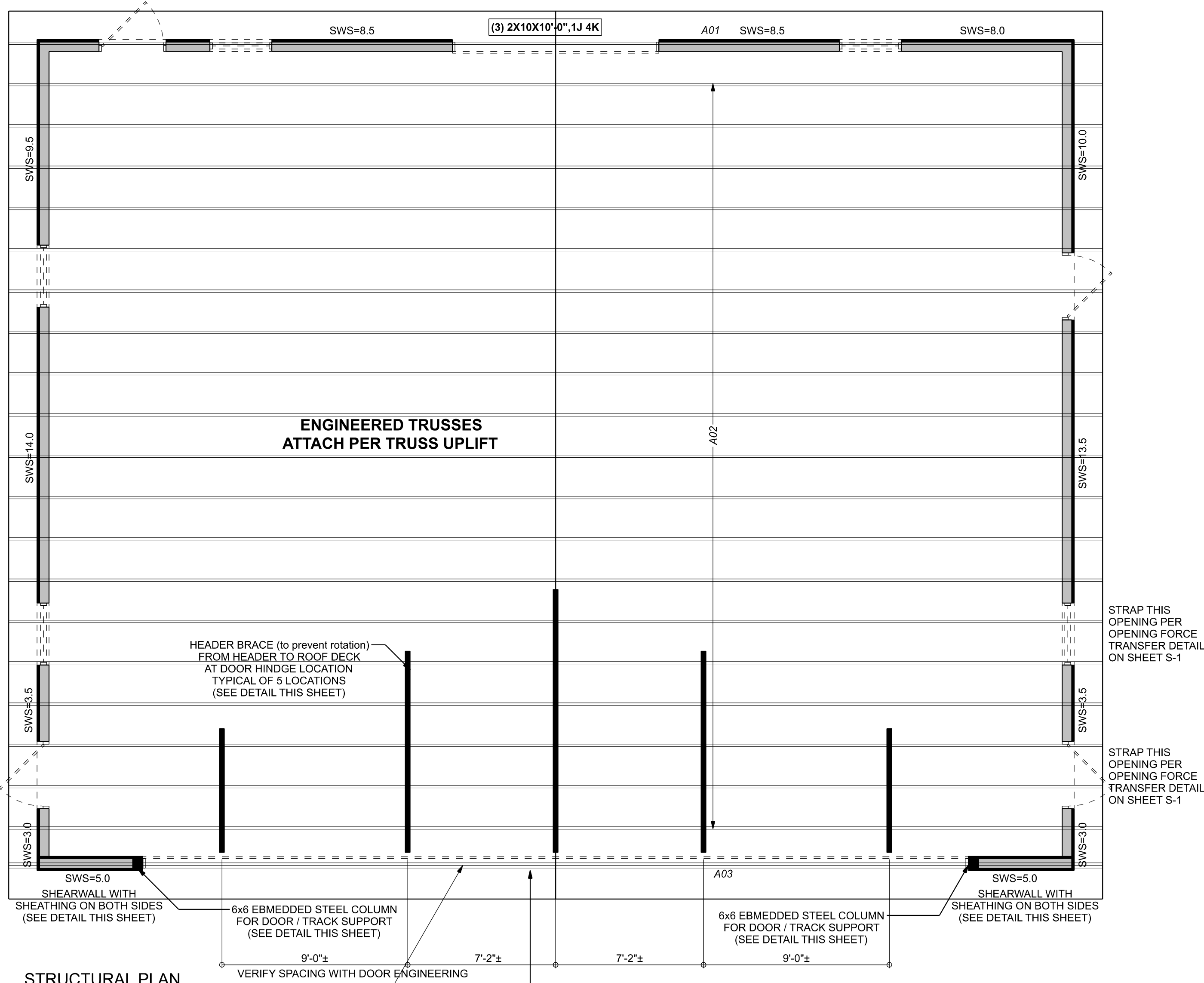
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Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

JOB NUMBER:  
230368

S-2  
OF 3 SHEETS

HORIZONTAL (in line with wall)		
	0-5'	45-50'
CASE #1	+2600 LB	+2600 LB
CASE #2	-2600 LB	-2600 LB

SCALE: 1/2" = 1'-0"



SCALE: 1/4" = 1'-0"

\*\*\* GABLE END TRUSS A03 \*\*\*

THE FOLLOWING DESIGN CHANGES MUST BE MADE:

- MUST BE REVISED TO HAVE BEARING ONLY ON 4'-6" WALLS AT ENDS
- SUPPORT VERTICAL DEAD LOAD OF HEADER AND KNEE WALL BELOW (50 PLF PER FOOT)
- SUPPORT VERTICAL DEAD LOAD OF THE 5 CENTER HANGER DOOR HINGE LOADS (600 LBS PER HINGE)
- TRUSS MUST BE DESIGNED FOR DRAG PER TABLE (THIS SHEET)

SN-1	<p>DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS</p> <p>PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-103.</p>
SN-2	<p>BCSI-B1, BCSI-B2, &amp; BCSI-B3, BCSI-B1, BCSI-B2, &amp; BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE</p>

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER. W.B. HOWLAND CO.  
JOB 22-8407

**(2) 2X6X0'.1J 1K** ← HEADER/BEAM CALL-OUT (U.N.O.)

- NUMBER OF KING STUDS EACH SIDE OF OPENING (FULL LENGTH)
- NUMBER OF JACK STUDS EACH SIDE OF OPENING (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

Tim Marshall Hanger

**PROJECT ADDRESS:**  
60 SW Orange Blossom C  
Lake City, Florida

PE 53915  
Digitally signed and sealed by  
on digital signature date.  
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C=US, O=Florida  
dnQualifier=A014  
10C0000017E97  
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0, CN=Mark d  
Disosway  
2023-03-31 12:  
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**LIMITATION:** This design is valid for one building, at specified location.

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**S-3**