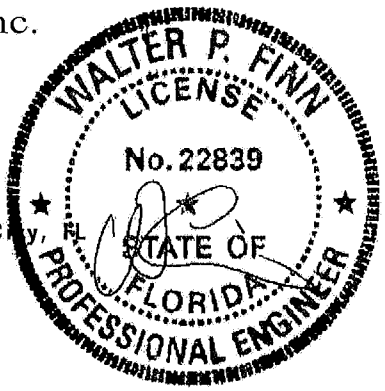
ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V2T487-Z0207082342



Truss Fabricator	Anderson Truss Company
Job Identification	13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL
Truss Count	66
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

01/07/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-12030EC1-DEFLCAMB-PB16010-CNNAILSP-

#	Ref	Description	Drawing#	Date
1	19080	A 25'4" Common	14007001	01/07/14
2	19081	A1 12'9"8 Common	14007002	01/07/14
3	19082	AGE 25'4" Gable	14007036	01/07/14
4	19083	B 38'4" Stepdown	14006042	01/06/14
5	19084	B1 38'4" Stepdow	14006056	01/06/14
6	19085	BG 38'4" Stepdow	14007037	01/07/14
7	19086	C 34'10" Common	14007003	01/07/14
8	19087	C1 33'4" Common	14007004	01/07/14
9	19088	CGE 34'10" Gable	14007005	01/07/14
10	19089	CJ1 1' Jack	14006045	01/06/14
11	19090	CJ3 3' Jack	14006044	01/06/14
12	19091	CJ3A 3' Jack	14006053	01/06/14
13	19092	CJ5 5' Jack	14006043	01/06/14
14	19093	CJ5A 5' Jack	14007006	01/07/14
15	19094	D1 39'2"8 Stepdo	14007007	01/07/14
16	19095	D10 47' Special	14007008	01/07/14
17	19096	D11 47' Special	14007009	01/07/14
18	19097	D2 39'2"8 Stepdo	14007010	01/07/14
19	19098	D3 39'2"8 Stepdo	14007011	01/07/14
20	19099	D4 47' Stepdown	14007012	01/07/14
21	19100	D5 47' Special	14007013	01/07/14
22	19101	D6 47' Special	14007014	01/07/14
23	19102	D7 47' Special	14007015	01/07/14
24	19103	D8 47' Special	14007016	01/07/14
25	19104	D9 47' Special	14007017	01/07/14
26	19105	EJ7 7' End Jack	14006046	01/06/14
27	19106	H11 38'4" Stepdo	14006055	01/06/14
28	19107	H11A 51' Stepdow	14007018	01/07/14
29	19108	H13 38'4" Stepdo	14006058	01/06/14
30	19109	H13A 47' Stepdow	14007019	01/07/14
31	19110	H15 38'4" Stepdo	14006063	01/06/14
32	19111	H15A 47' Stepdow	14007020	01/07/14
33	19112	H17 38'4" Stepdo	14006065	01/06/14
34	19113	H17A 47' Stepdow	14007021	01/07/14
35	19114	H19A 47' Stepdow	14007022	01/07/14
36	19115	H7 38'4" Stepdow	14006047	01/06/14
37	19116	H7A 51' Stepdown	14006052	01/06/14
38	19117	H9 38'4" Stepdow	14006050	01/06/14
39	19118	H9A 51' Stepdown	14006051	01/06/14
40	19119	HJ7 9'10"13 Hip	14006048	01/06/14
41	19120	HJ7A 9'10"13 Hip	14006060	01/06/14
42	19121	J10 9'10"8 Mono	14006062	01/06/14
43	19122	J10B 9'10"8 Mono	14006061	01/06/14
44	19123	J7 6'10"8 End Ja	14006068	01/06/14
45	19124	J7B 7' End Jack	14007023	01/07/14
46	19125	MH11 47' Common	14006064	01/06/14
47	19126	MH13 47' Common	14006059	01/06/14
48	19127	MH15 47' Common	14007024	01/07/14
49	19128	MH17 47' Mono Hi	14007025	01/07/14
50	19129	MH19 47' Mono Hi	14007026	01/07/14
51	19130	MH7 48'10" Mono	14007039	01/07/14
52	19131	MH9 48'10" Comm	14006049	01/06/14
53	19132	MHG10 9'10"8 Mon	14007038	01/07/14
54	19133	PBA1 20'10"7 Ste	14007027	01/07/14
55	19134	PBA10 32'2"4 Mon	14007028	01/07/14
56	19135	PBA2 20'10"7 Ste	14007029	01/07/14
57	19136	PBA3 20'10"7 Ste	14007030	01/07/14
58	19137	PBA4 20'10"7 Ste	14007031	01/07/14
59	19138	PBA5 16'10"7 Com	14007032	01/07/14
60	19139	PBA6 13'11"5 Com	14006066	01/06/14
61	19140	PBA7 15'10"1 Ste	14007033	01/07/14
62	19141	PBA8 11'10"1 Ste	14007034	01/07/14
63	19142	PBA9 30'5"11 Mon	14007035	01/07/14
64	19143	PBB1 8'2"7 Stepd	14006057	01/06/14
65	19144	PBB2 8'2"7 Comm	14006054	01/06/14
66	19145	PBB3 8'2"7 Comm	14006067	01/06/14

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - A 25 4 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

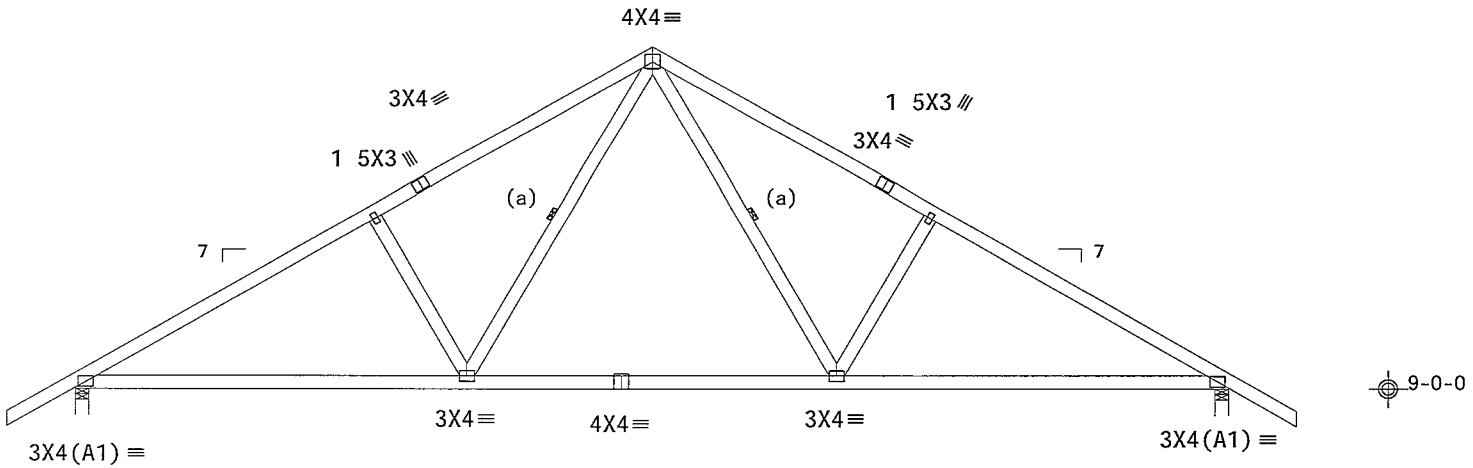
Bottom chord checked for 10 00 psf non-concurrent live load

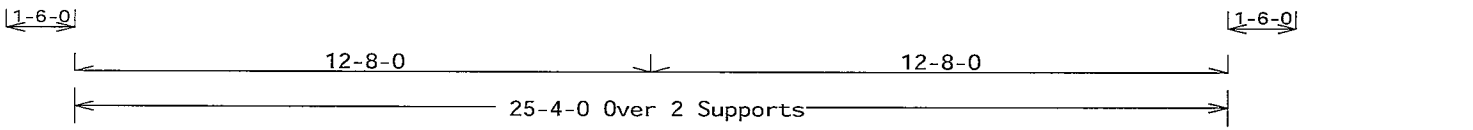
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1131 U=41 W=3 5"
RL=135/-135

R=1132 U=41 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 0806.13 QTY:11 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabr cat ng handl ng shipp ng nstalling and bracing Refer to and follow the latest edition of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices prior to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restra nt of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any dev at on from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping nstallation bracing of trusses Apply plates to each face of truss and posit on as shown above and on the Joint Deta ls unless noted otherwise Refer to drawings 160A-Z for standard plate pos t ons A seal on this draw ng or cover page l sting this drawing ndicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitab l ity and use of th s des gn for any structure is the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20.0 PSF	REF	R9114- 19080
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007001
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338351
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - A1 12 9 8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

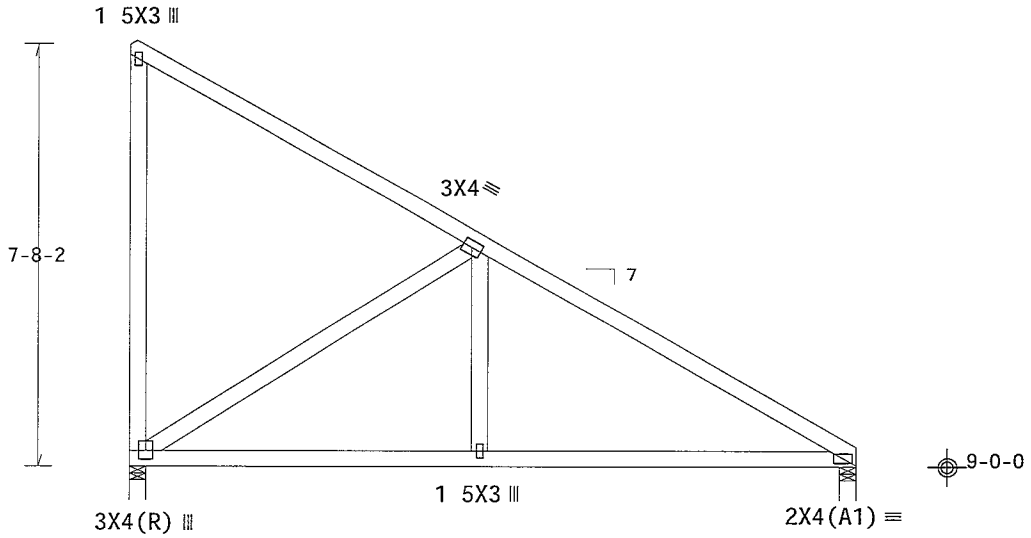
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge



12-9-8 Over 2 Supports

R=482 U=3 W=3 5"
RL=28/-81

R=493 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

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Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handl ng shipping install ng and bracing Refer to and follow the latest edition of BCSI (Build ng Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of web shall have brac ng installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pping installation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this draw ng or cover page listing th s draw ng nd cates acceptance of profess onal engineering responsibility solely for the design shown The suitabil ty and use of th s des gn for any structure is the respons b l ty of the Build ng Des gner per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www ccsafe org

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19081
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007002
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	338355
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - AGE 25 4 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

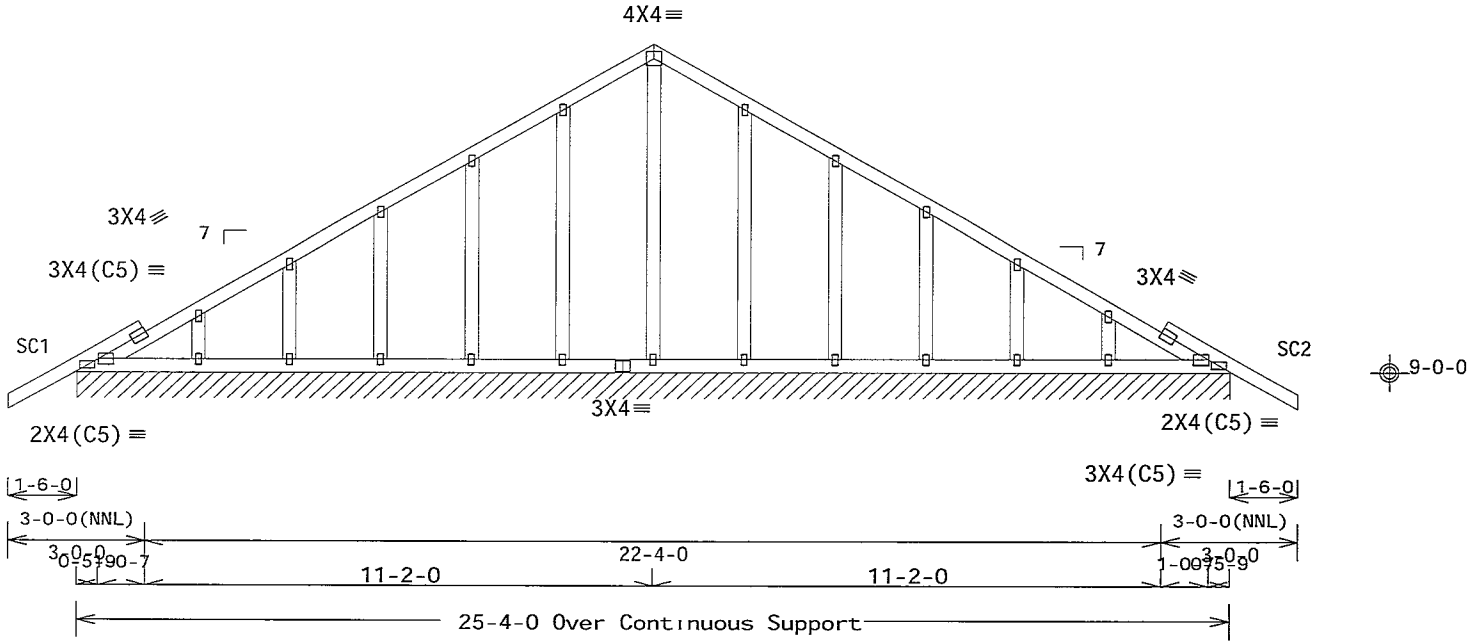
Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=193 PLF U=21 PLF W=25-4-0
RL=12/-12 PLF

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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01/07/2014

TC LL	20 0 PSF	REF	R9114- 19082
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007036
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	338347
DUR.FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - B 38 4 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)

TC- From	56 plf at	-1 50 to	56 plf at	15 06
TC- From	56 plf at	15 06 to	56 plf at	23 27
TC- From	56 plf at	23 27 to	56 plf at	39 83
BC- From	5 plf at	-1 50 to	5 plf at	0 00
BC- From	20 plf at	0 00 to	20 plf at	8 33
BC- From	20 plf at	8 33 to	20 plf at	15 50
BC- From	60 plf at	15 50 to	60 plf at	17 83
BC- From	20 plf at	17 83 to	20 plf at	20 17
BC- From	20 plf at	20 17 to	20 plf at	38 33
BC- From	5 plf at	38 33 to	5 plf at	39 83

BC- 200 00 lb Conc Load at 5 50

Wind loads and reactions based on MWFRS

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

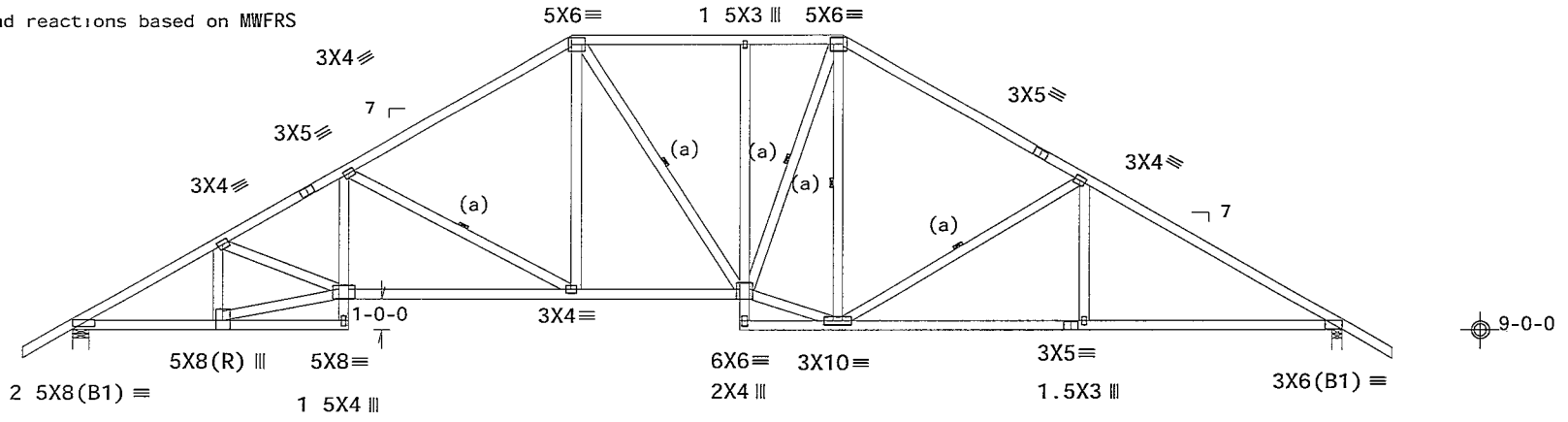
(a) Continuous lateral restraint equally spaced on member

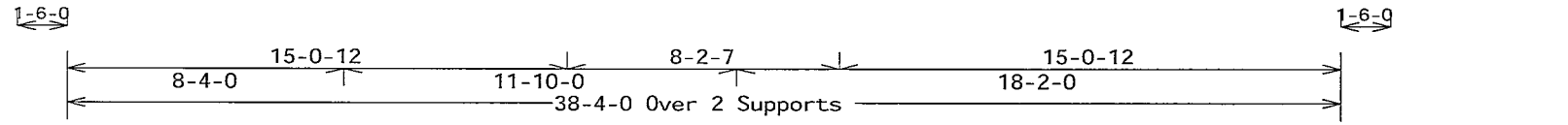
In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge





R=1777 U=13 W=6" (6" min)
RL=163/-163

R=1621 U=6 W=3 5" (3 5" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 03/26/13 QTY·3 FL/-/5/-/-/R/- Scale = 1875"/Ft.

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19083
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006042
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	337628
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - B1 38'4" Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge

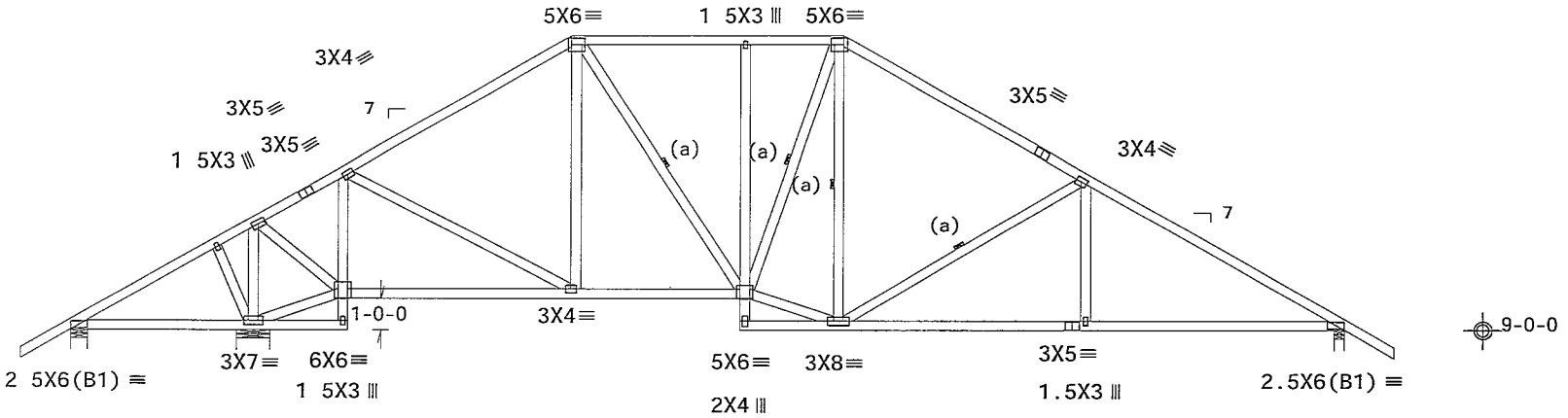
Negative reaction(s) of -237# MAX (See below) from a non-wind load case requires uplift connection

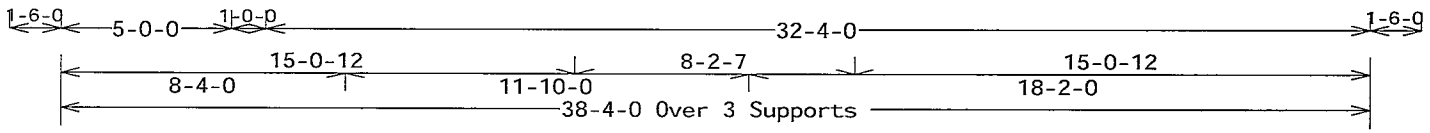
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=122/-237 U=77 W=6" (6' min)
RL=163/-163 R=1941 U=0 W=12' (12 min)

R=1321 U=0 W=3 5" (3 5' min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 0886 13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft

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FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this design any fa lure to bu ld the truss n conformance w th ANSI/TPI 1 or for hand! ng sh pp ng installation brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this draw ng or cover page l st ng th s draw ng nd cates acceptance of profess ona eng neer ng respons b l ty solely for the des gn shown The su tab l ity and use of th s design for any structure s the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more nformat on see Th s job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20.0 PSF	REF	R9114- 19084
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006056
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	337606
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - BG 38 4 Stepdown Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x6 SP SS-13B
Webs 2x4 SP #3-13B W2 W14 2x6 SP #2-13B
W3 W9 W13 W16 W18 2x4 SP #2-13B W11 2x4 SP #1-13B
Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 500
Rt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 500

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge

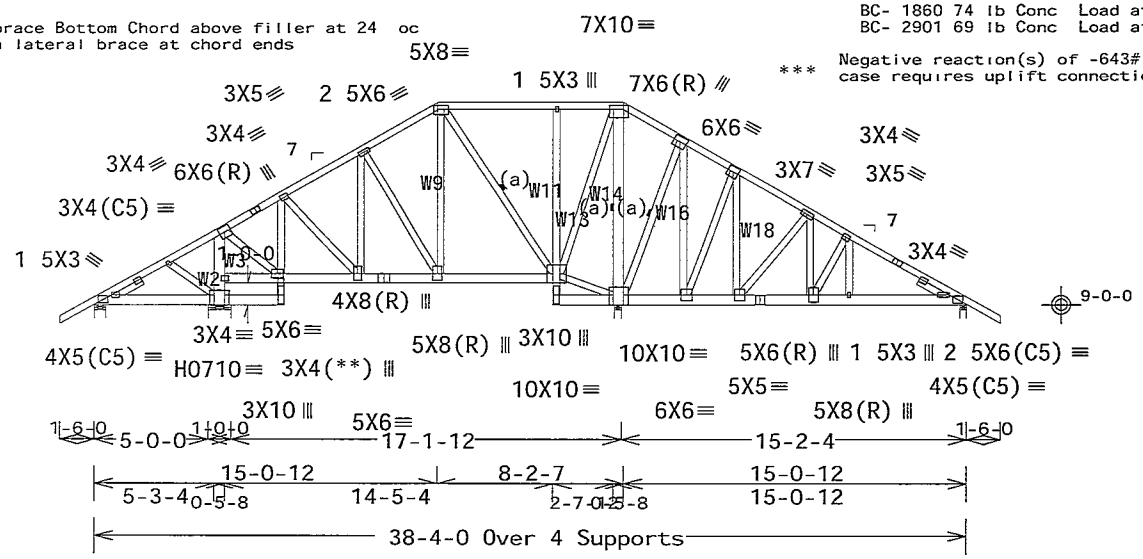
Laterally brace Bottom Chord above filler at 24 oc including a lateral brace at chord ends

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131 x3", min nails
Top Chord 1 Row @12 00 o c
Bot Chord 2 Rows @ 5 00 o c (Each Row)
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at -1 50 to 56 plf at 6 99
TC- From 28 plf at 6 99 to 28 plf at 15 06
TC- From 28 plf at 15 06 to 28 plf at 23 27
TC- From 28 plf at 23 27 to 28 plf at 31 33
TC- From 56 plf at 31 33 to 56 plf at 35 36
TC- From 56 plf at 35 36 to 56 plf at 39 83
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 5 27
BC- From 10 plf at 5 27 to 10 plf at 12 73
BC- From 10 plf at 12 73 to 10 plf at 20 17
BC- From 10 plf at 20 17 to 10 plf at 22 81
BC- From 10 plf at 22 81 to 10 plf at 29 27
BC- From 20 plf at 29 27 to 20 plf at 38 33
BC- From 5 plf at 38 33 to 5 plf at 39 83
BC- 1785 31 lb Conc Load at 7 40 9 40,11 40 17 40 19 40 21 40 23 40,25 40
BC- 1783 56 lb Conc Load at 13 40,15 40
BC- 1739 00 lb Conc Load at 27 40
BC- 1860 74 lb Conc Load at 29 40
BC- 2901 69 lb Conc Load at 31 33

*** Negative reaction(s) of -643# MAX (See below) from a non-wind load case requires uplift connection



*** R=-643 Rw=131 U=40 W=6 '
RL=163/-163

R=7516 U=19 W=12'

R=17000 U=501 W=3 5"

R=2517 U=111 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03 04 [illegible] QTY.1 FL/-/5/-/-/R/- Scale = 125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20 0 PSF	REF	R9114- 19085
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10 0 PSF	DRW	HCUSR9114 14007037
BC LL	0 0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	339051
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - C 34 10 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

Left and right cantilevers are exposed to wind

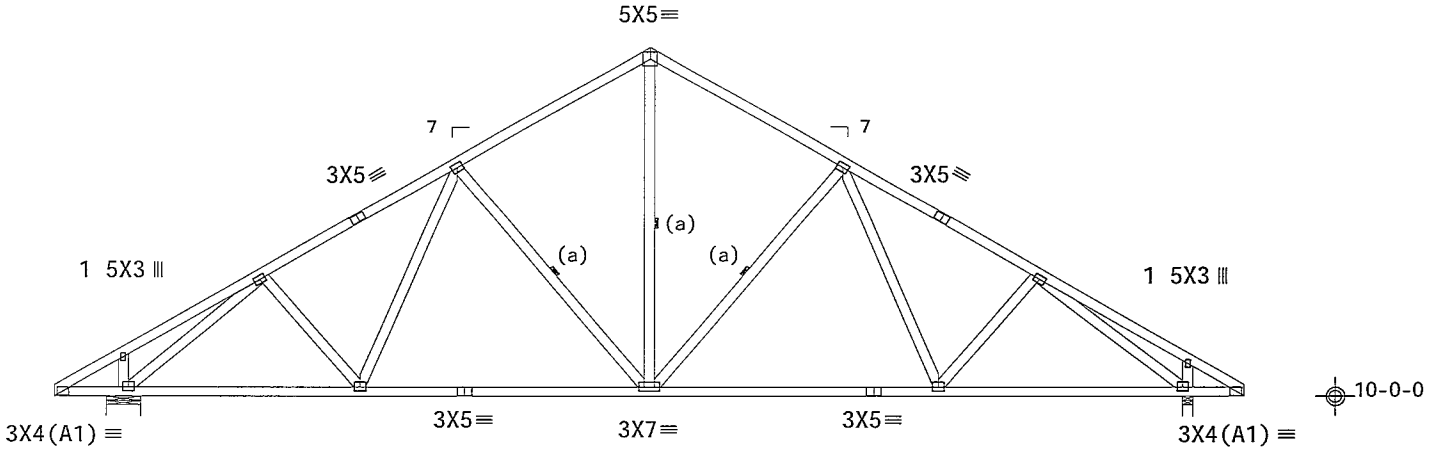
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 44 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 [illegible]6019 QTY:1 FL/-/5/-/-/R/- Scale = 1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20 0 PSF	REF	R9114- 19086
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007003
BC LL	0 0 PSF	HC-ENG	JB/WPF
TOT LD.	37.0 PSF	SEQN-	338342
DUR.FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - C1 33 4 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Right cantilever is exposed to wind

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24"-wide clearance

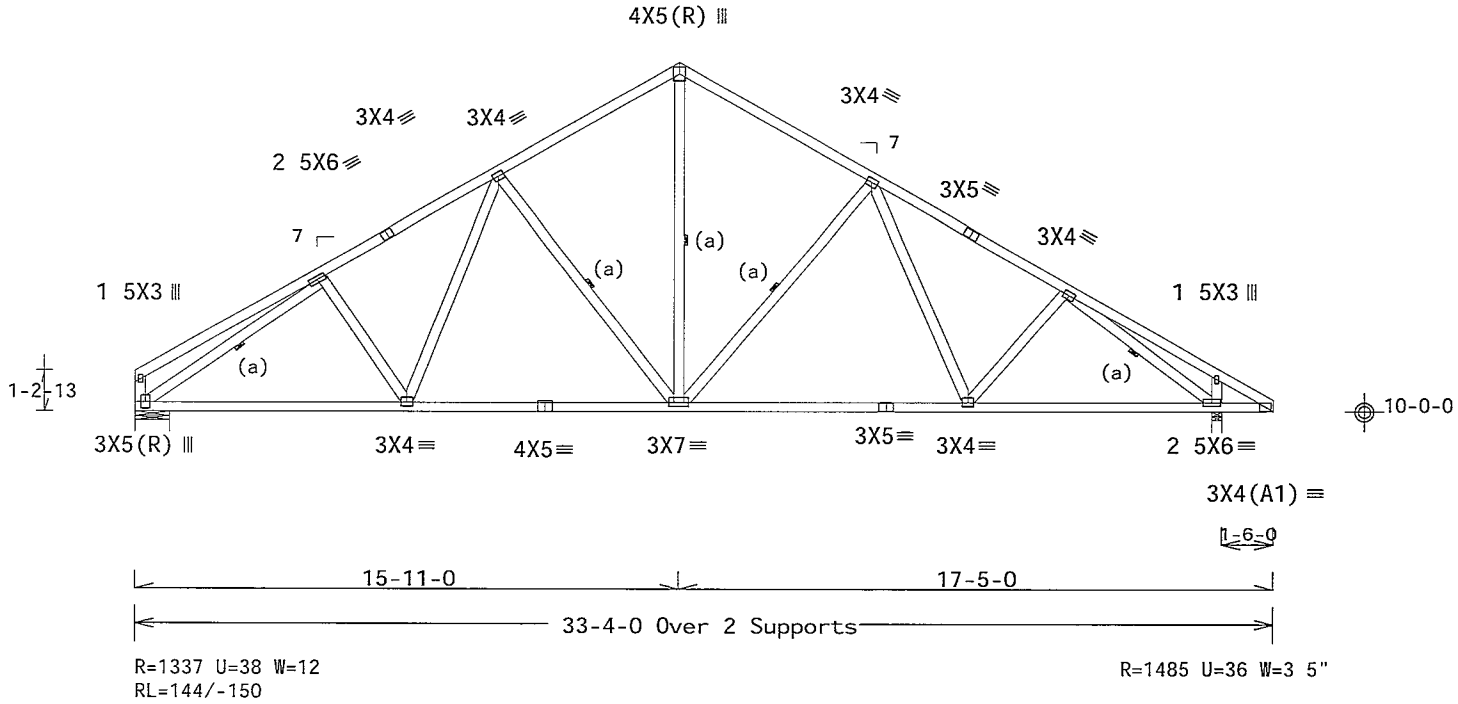
120 mph wind, 15 44 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 QTY:2 FL/-/5/-/-/R/- Scale =.1875"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20 0 PSF	REF	R9114- 19087
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007004
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338334
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - CGE 34 10 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

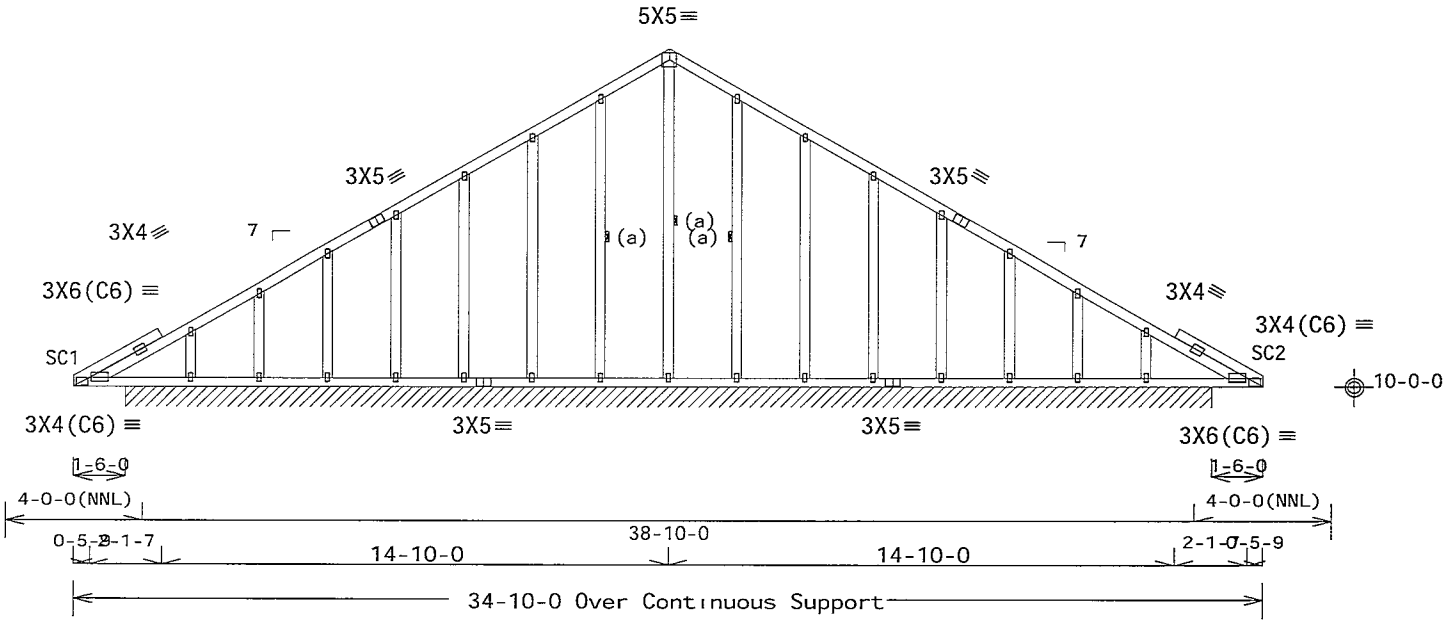
Gable end supports 8 max rake overhang

120 mph wind, 15 24 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Left and right cantilevers are exposed to wind

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



R=83 PLF U=1 PLF W=31-10-0
RL=5/-5 PLF

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 0226 13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft

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Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19088
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007005
BC LL	0.0 PSF		HC-ENG JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338320
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - CJ1 1 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

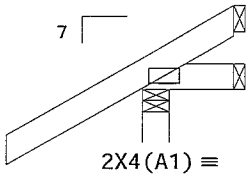
Bottom chord checked for 10 00 psf non-concurrent live load

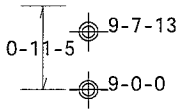
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

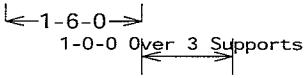
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

R=-51 Rw=18 U=37 (1 5' min)





R=8 Rw=11 U=9 (1 5' min)



R=232 U=30 W=3 5 (3 5' min)
RL=24

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0336 13 QTY:10 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19089
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006045
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337580
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - CJ3 3 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

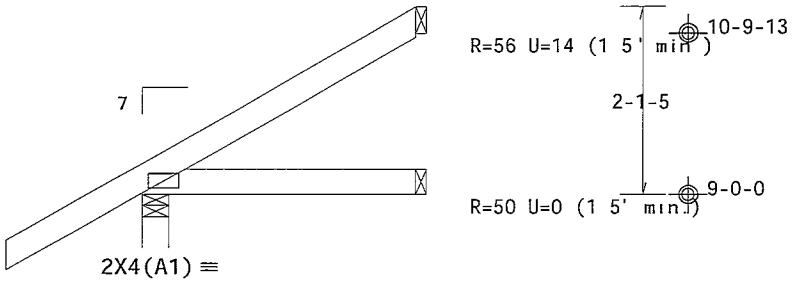
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

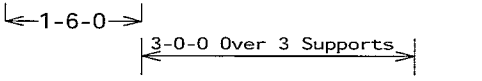
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=240 U=11 W=3 5' (3 5" min)
RL=43/-23

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]43 QTY:23 FL/-/5/-/-/R/- Scale =.5"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19090
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006044
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337585
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - CJ3A 3' Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

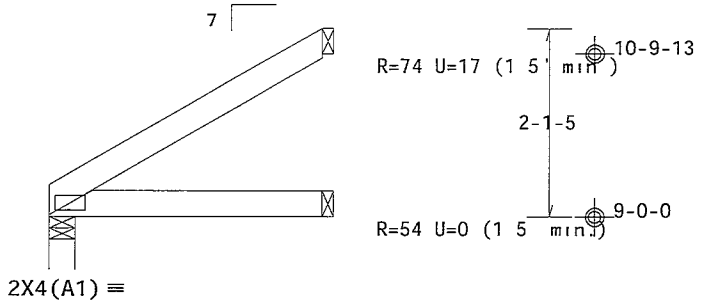
Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

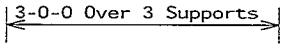
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=120 U=0 W=3 5' (3 5' min)
RL=28

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]43 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling sh pp ng install ng and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall prov de temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be responsible for any deviation from this des n any failure to build the truss in conformance w th ANSI/TPI 1 or for handling shipping installation brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate posit ons A seal on this draw ng or cover page list ng th s draw ng indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of th s design for any structure s the responsib l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see This job s goneral notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19091
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006053
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337722
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - CJ5 5' Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

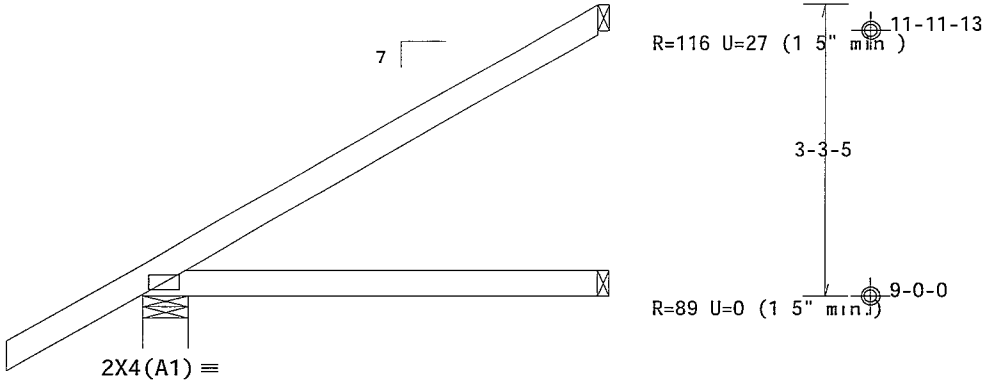
Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

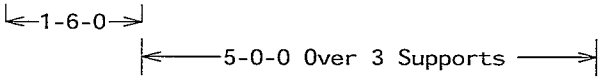
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=304 U=6 W=6" (6" min)
RL=62/-27

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04 OPOC 13 | QTY:9 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19092
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006043
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337582
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - CJ5A 5 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

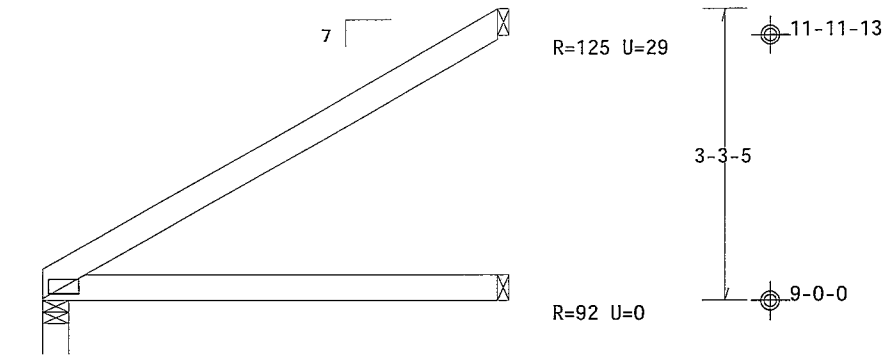
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0806.13 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
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Trusses requ re extreme care n fabr cat ng handl ng shipping nstalling and bracing Refer to and follow the latest ed tion of BCSI (Bu lding Component Safety Information by TPI and WTCA) for safety pract ces pr or to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of web shall have brac ng nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from th s design any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handl ng shipp ng nstallat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos t ons A seal on this drawing or cover page listing th s draw ng nd cates acceptance of professional engineering respons b l ty solely for the des gn shown The su tab l ity and use of this design for any structure is the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see This Job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19093
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007006
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338872
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D1 39 2 8 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

MWFRS loads based on trusses located at least 15 00 ft from roof edge

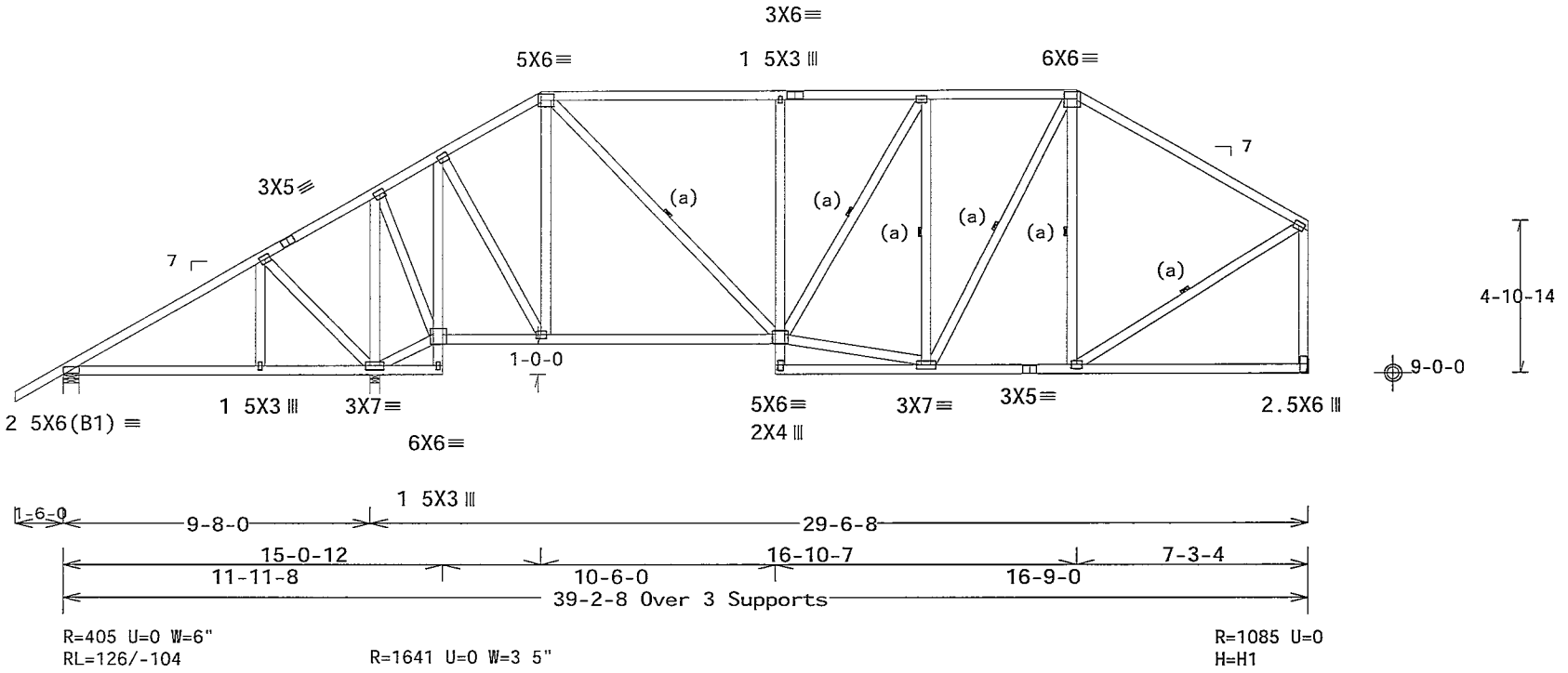
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cating handl ng sh pp ng nstall ng and bracing Refer to and follow the latest ed t on of BCSI (Bu lding Component Safety Information by TPI and WTCA) for safety pract ces pr or to performing these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from th s desi any fa lure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng installat on bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page l sting th s drawing nd cates acceptance of profess onal engineering respons b l ty solely for the design shown The su tabil ty and use of th s des gn for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19094
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007007
BC LL	0.0 PSF		HC-ENG JB/WPF
TOT LD.	37.0 PSF	SEQN-	338860
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - D10 47 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

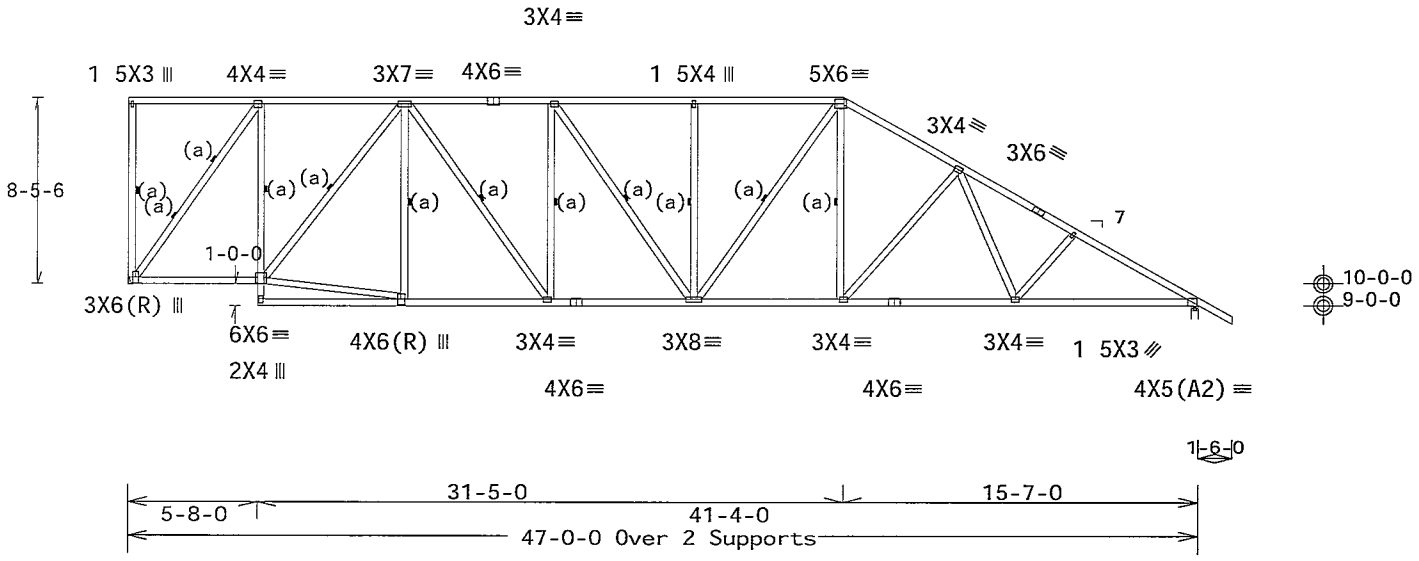
Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 17" DL 0 23" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:1 FL/-/5/-/-/R/- Scale = 125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19095
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007008
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37 0 PSF	SEQN-	338584
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - D11 47 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING' note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

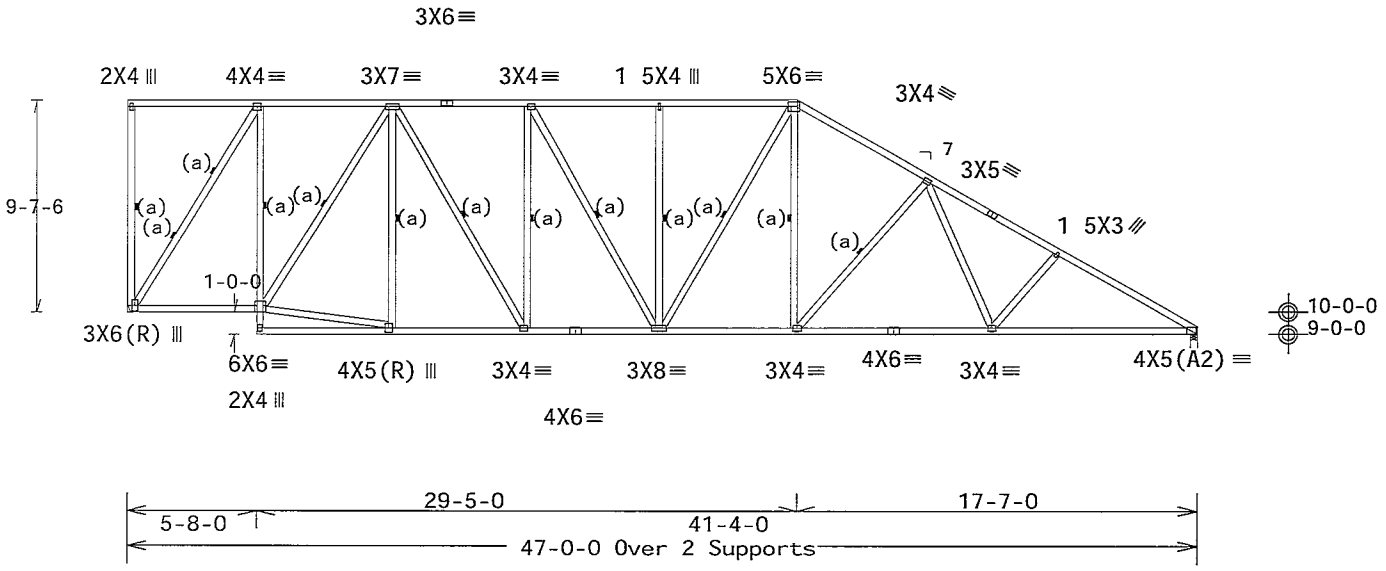
Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 15" DL 0 21" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge



R=1785 U=49
RL=52/-113
H=H1

R=1796 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 ER6215 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19096
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007009
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338701
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D2 39 2 8 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge

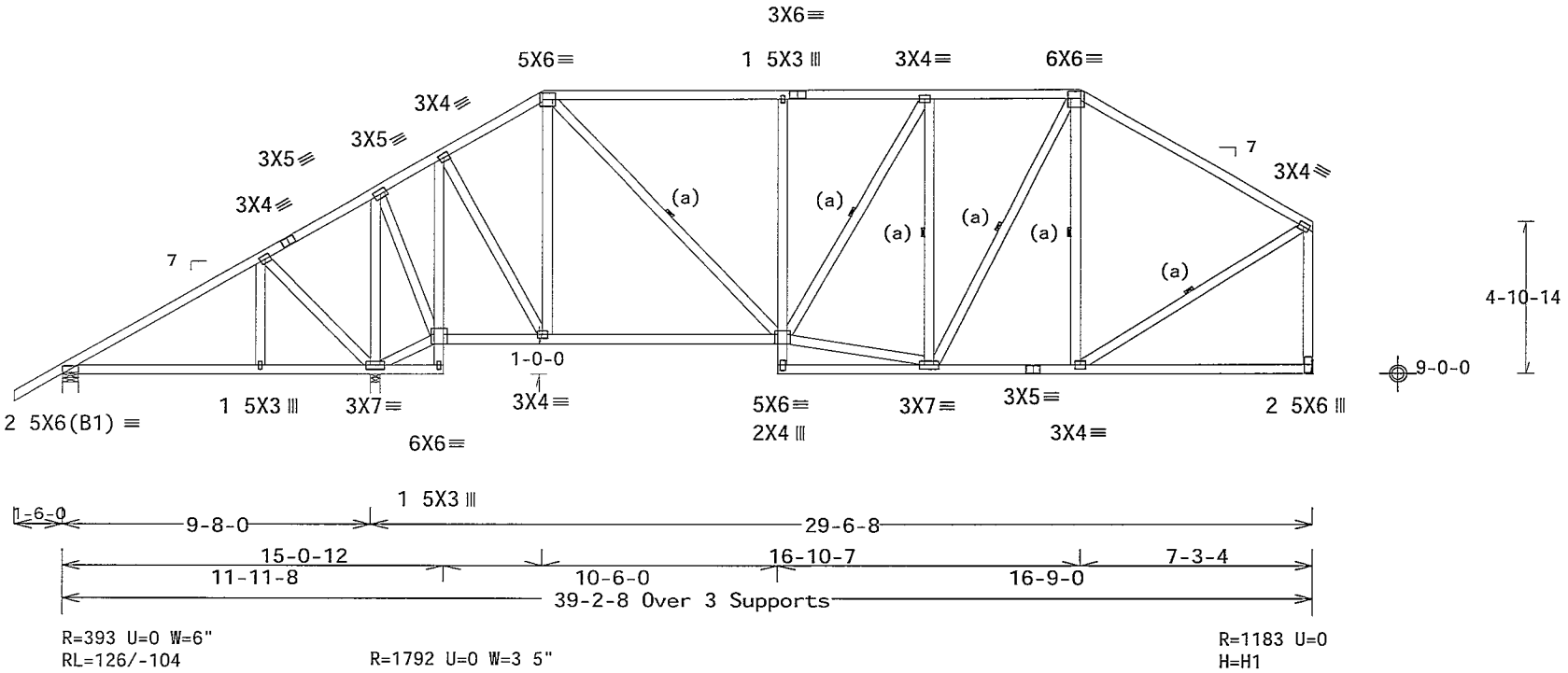
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 R6 10 QTY:2 FL/-/5/-/-/R/- Scale = 1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19097
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007010
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338862
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - D3 39 2 8 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

MWFRS loads based on trusses located at least 30 00 ft from roof edge

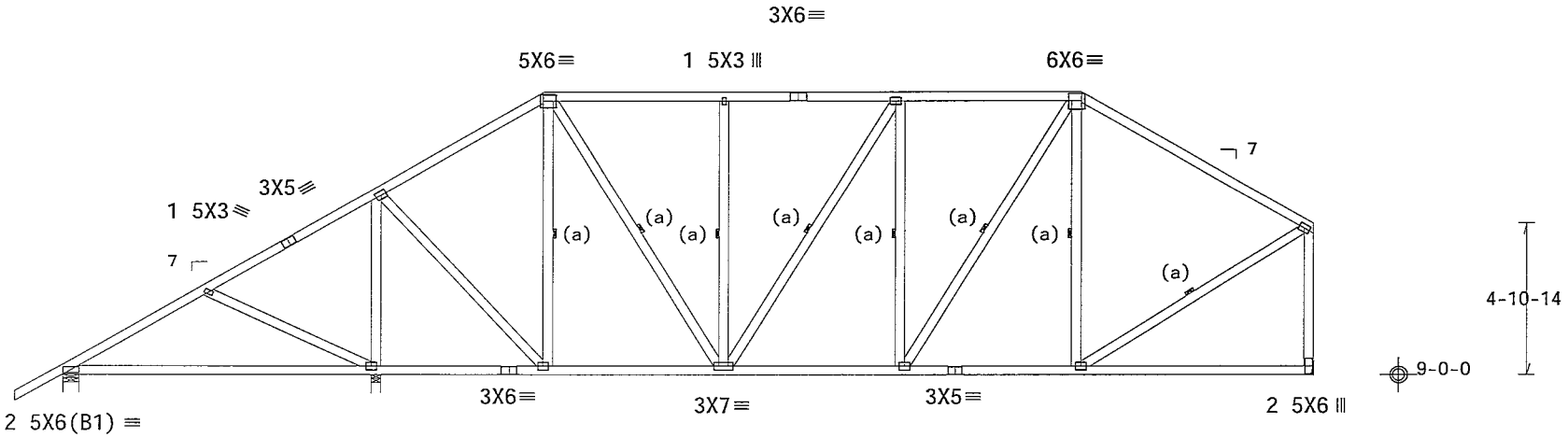
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care in fabr cat ng handl ng shipping install ng and brac ng Refer to and follow the latest edition of BCSI (Build ng Component Safety Informat on by TPI and WTCA) for safety practices prior to perform ng these functions Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rigid ceil ng Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this des n any failure to build the truss in conformance w th ANSI/TPI 1 or for handl ng sh pping installation bracing of trusses Apply plates to each face of truss and posit on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s drawing or cover page listing this drawing ndicates acceptance of profess onal eng neer ng respons bil ty solely for the des gn shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Building Des gner per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19098
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007011
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338871
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D4 47 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 30 00 ft from roof edge

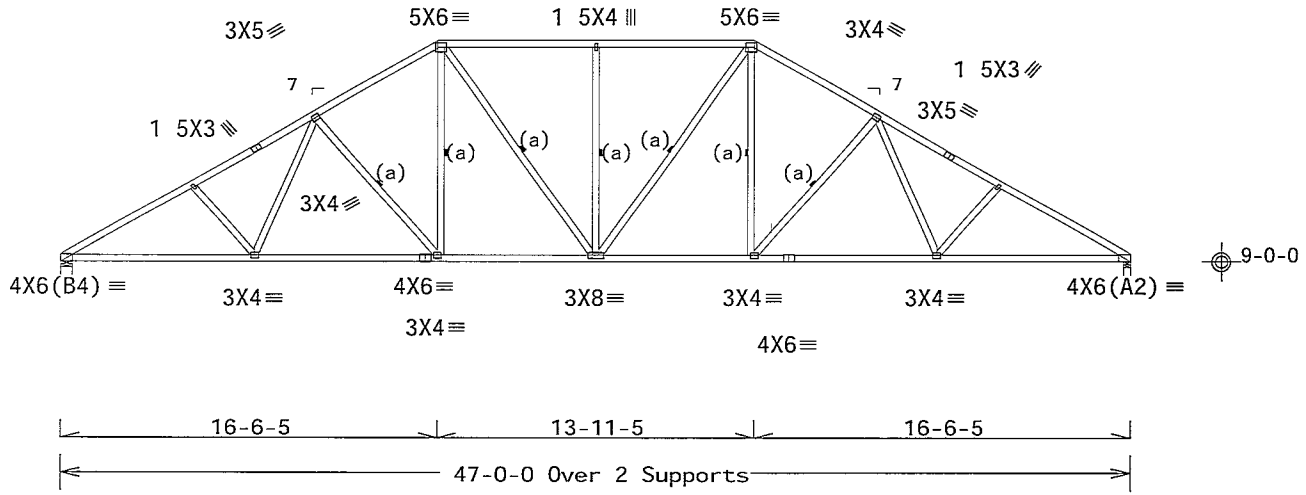
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]13 QTY:2 FL/-/5/-/-/R/- Scale = 125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care n fabr cating handl ng sh pp ng nstalling and brac ng Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety pract ces pr or to performing these funct ons Installers shall prov de temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rig d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this design any failure to bu ld the truss in conformance with ANSI/TPI 1 or for hand ng sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Dota ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s draw ng or cover page I st ng this draw ng ind cates acceptance of profess onal eng neer ng responsibil ty solely for the design shown The suitab lity and use of th s design for any structure s the responsibility of the Bu ld ng Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19099
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007012
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338854
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D5 47 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B W2 2x4 SP #2-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

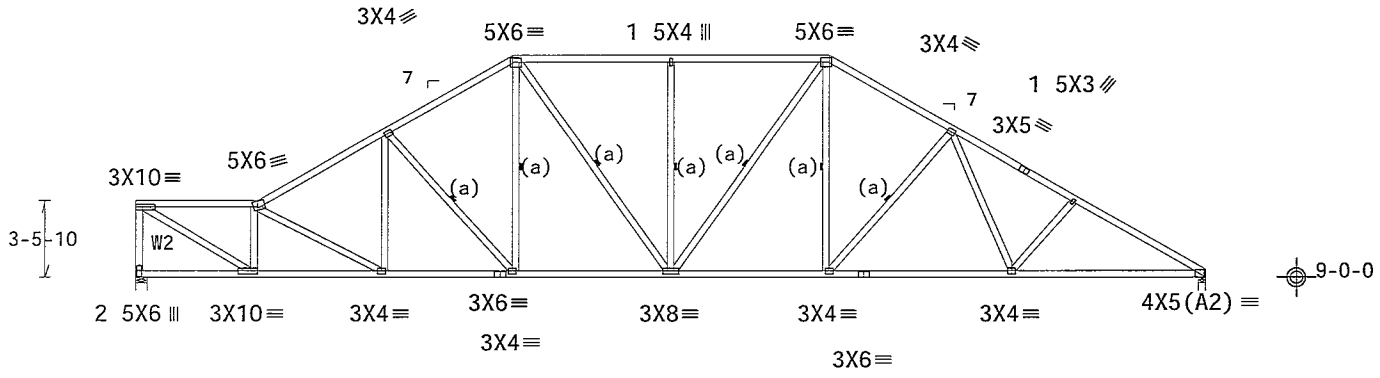
Wind loads and reactions based on MWFRS with additional C&C member design

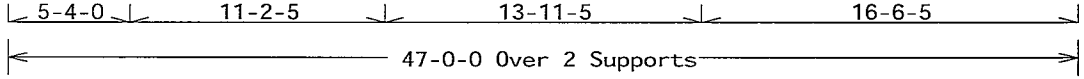
Max JT VERT DEFL LL 0 18" DL 0 25" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 30 00 ft from roof edge





R=1785 U=0 W=6"
RL=120/-139

R=1796 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 03:26:13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
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Trusses requ re extreme care n fabr cat ng handl ng shipping nstall ng and brac ng Refer to and follow the latest edit on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety pract ces pr or to perform ng these funct ons Installers shall provide temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s design any failure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng shipp ng installat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on th s draw ng or cover page l st ng th s draw ng nd cates acceptance of professional eng neering respons b l ty solely for the des gn shown The su tab l ity and use of th s design for any structure s the responsib l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19100
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10 0 PSF	DRW	HCUSR9114 14007013
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37.0 PSF	SEQN-	338428
DUR FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D6 47 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

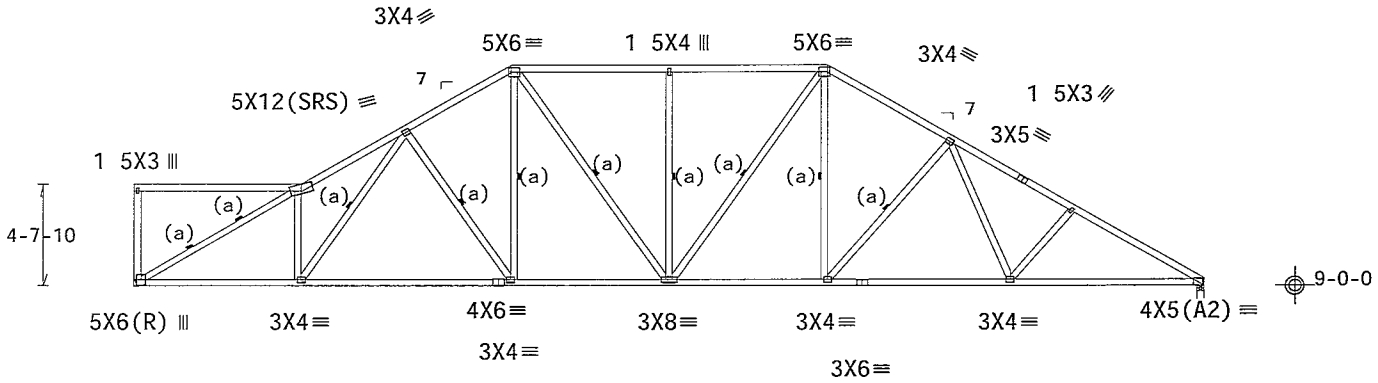
Wind loads and reactions based on MWFRS with additional C&C member design

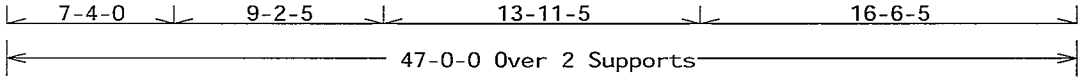
Max JT VERT DEFL LL 0 18" DL 0 25" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 30 00 ft from roof edge





R=1785 U=0
RL=108/-133
H=H1

R=1796 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 [illegible] 13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20 0 PSF	REF	R9114- 19101
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007014
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37 0 PSF	SEQN-	338434
DUR.FAC	1 25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D7 47 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

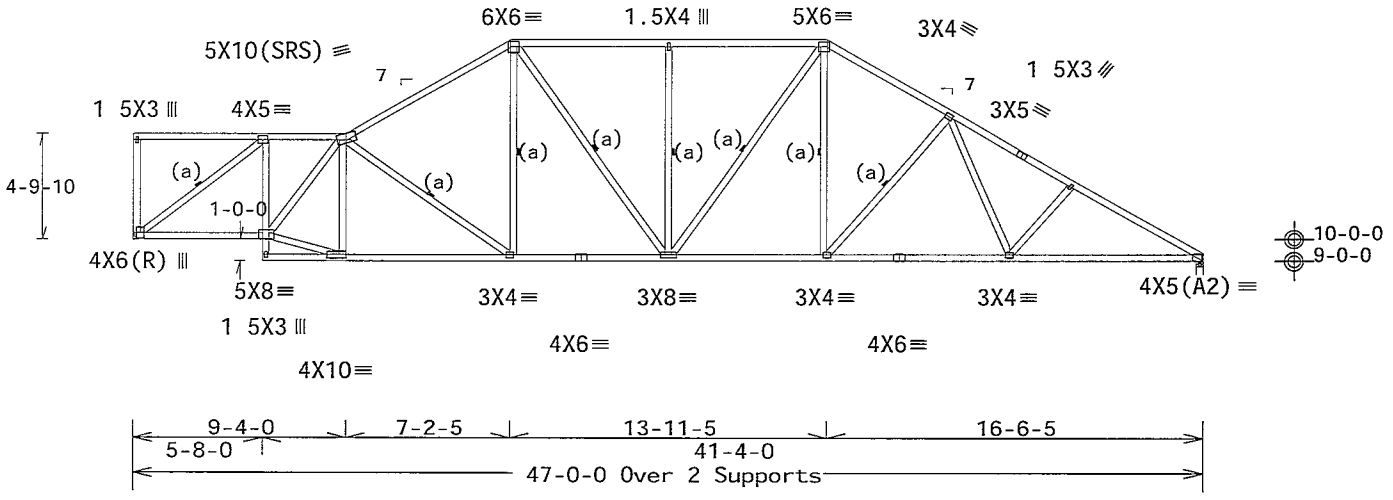
Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL· 0 18" DL 0 24" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 30 00 ft from roof edge



R=1785 U=0
RL=95/-127
H=H1

R=1796 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 19102
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007015
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	338440
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

01/07/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D8 47 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

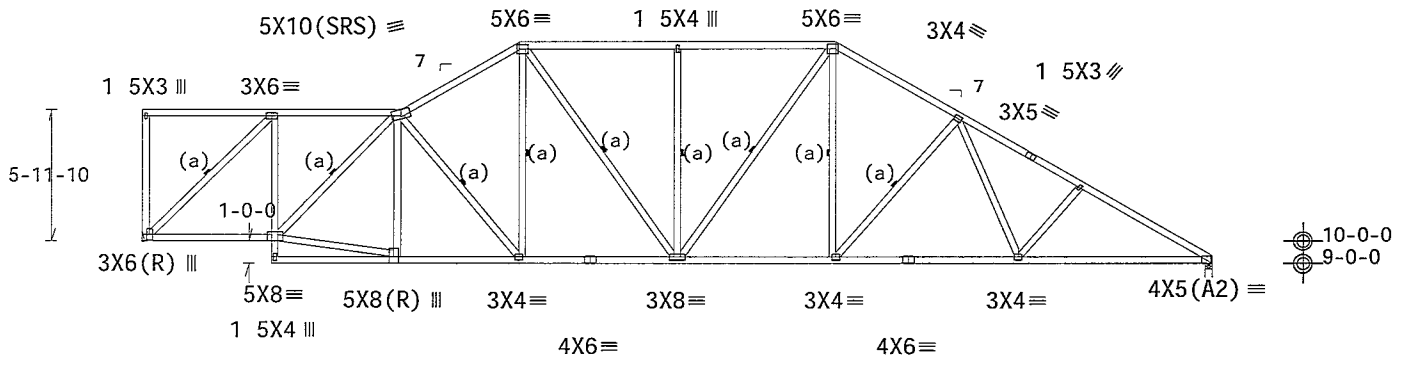
Wind loads and reactions based on MWFRS with additional C&C member design

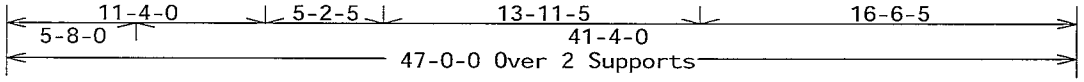
Max JT VERT DEFL LL 0 17" DL 0 23" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge





R=1785 U=0
RL=82/-121
H=H1

R=1796 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 0326 13 QTY·1 FL/-/5/-/-/R/- Scale = 125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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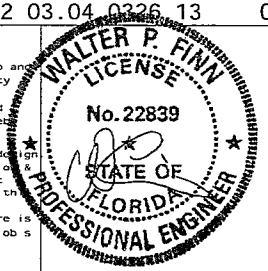


01/07/2014

TC LL	20 0 PSF	REF	R9114- 19103
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007016
BC LL	0 0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	338446
DUR.FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - D9 47 Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING' note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

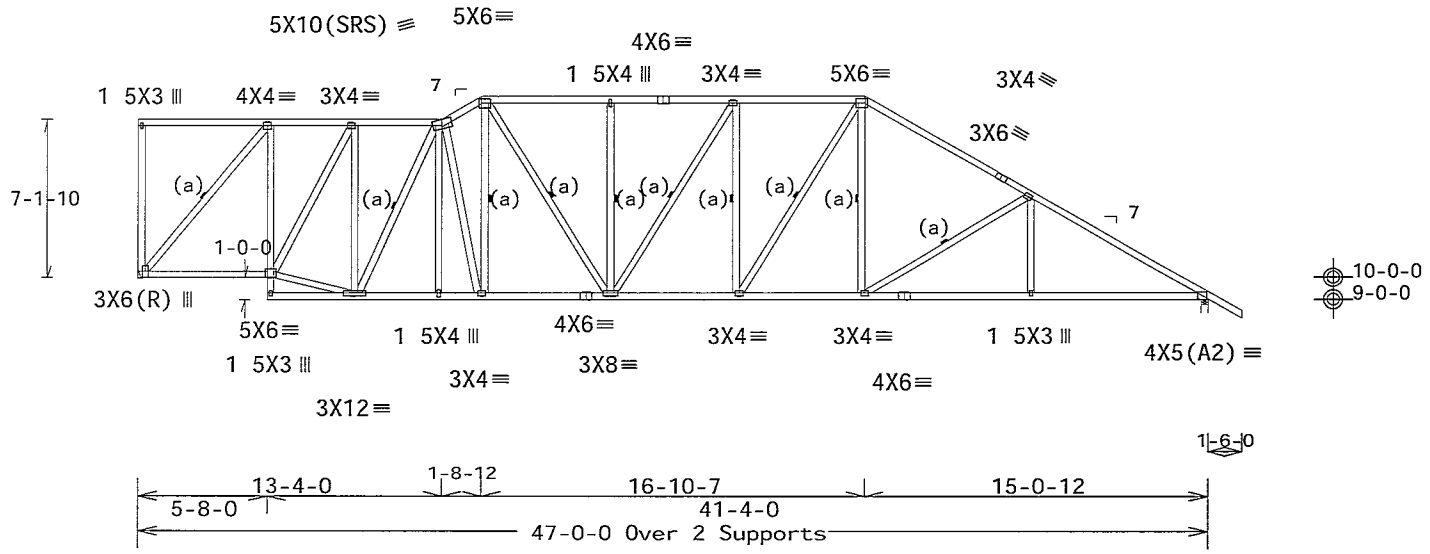
Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 17" DL 0 23" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 [illegible] QTY.1 FL/-/5/-/-/R/- Scale = 125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabr cat ng handl ng sh pp ng nstall ng and brac ng Refer to and follow the latest ed t on of BCSI (Bu lding Component Safety Information by TPI and WTCA) for safety pract ces pr or to performing these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from th s desig any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handl ng shipp ng installat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos t ons A seal on this draw ng or cover page l sting th s drawing nd cates acceptance of professional eng neering respons b l ty solely for the des gn shown The su tab l ity and use of this design for any structure is the respons b l ty of the Bu lding Des gnor per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19104
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007017
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37 0 PSF	SEQN-	338454
DUR.FAC.	1 25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - EJ7 7 End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

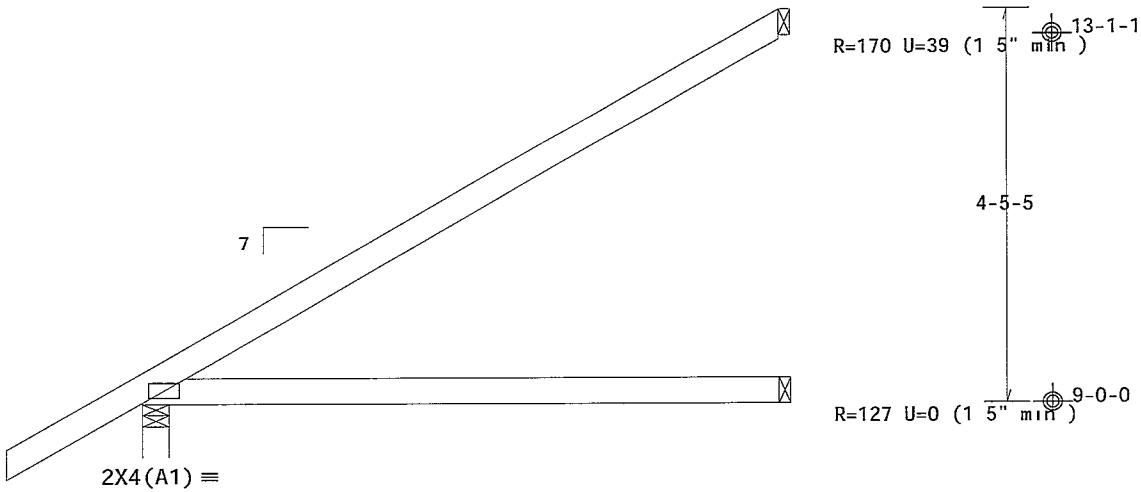
Lumber grades designated with '13B use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0336 13 QTY:34 FL/-/5/-/-/R/- Scale =.5"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from this de gn any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping installation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherw se Refer to drawings 160A-Z for standard plate pos t ons A seal on thi draw ng or cover page listing this drawing indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20 0 PSF	REF	R9114- 19105
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006046
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337579
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - H11 38 4' Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

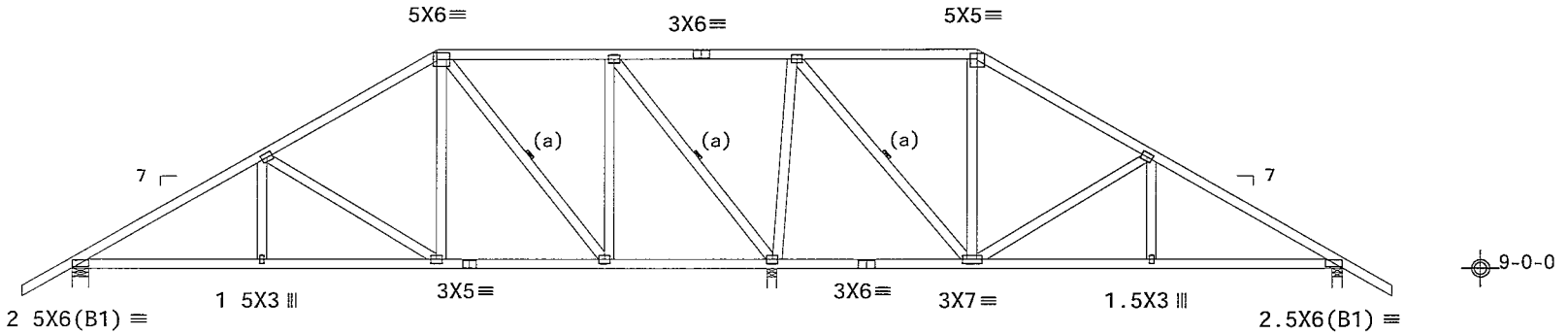
MWFRS loads based on trusses located at least 7 50 ft from roof edge

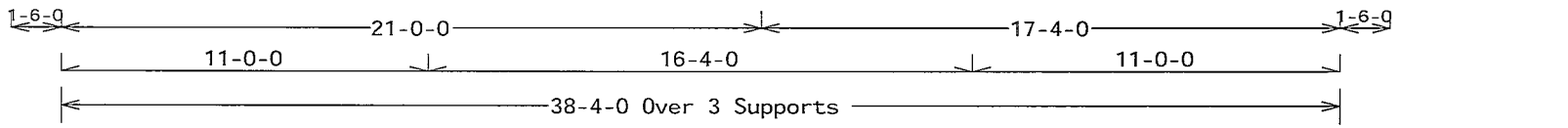
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=794 U=38 W=6' (6 min)
RL=126/-126

R=1752 U=60 W=3 5" (3 5' min)

R=641 U=31 W=3 5' (3 5" min)

Note All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 0336 13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu lding Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from this design any fa lure to build the truss in conformance with ANSI/TPI 1 or for handl ng sh pp ng nstallation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to drawings 160A-Z for standard plate pos t ons A seal on this draw ng or cover page listing this draw ng indicates acceptance of profess onal eng neering responsibility solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20.0 PSF	REF	R9114- 19106
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006055
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337599
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - H11A 51 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Calculated horizontal deflection is 0 11" due to live load and 0 15' due to dead load

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge

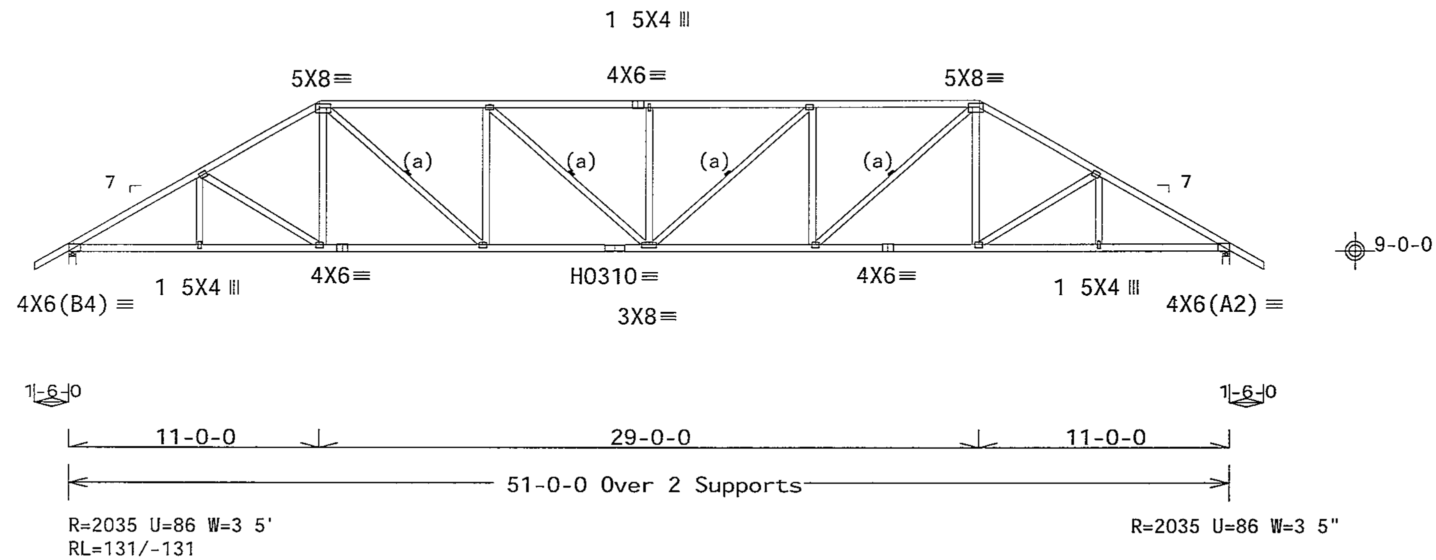
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See "WARNING" note below



Note All Plates Are 3X4 Except As Shown.

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19107
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007018
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338385
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - H13 38'4 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

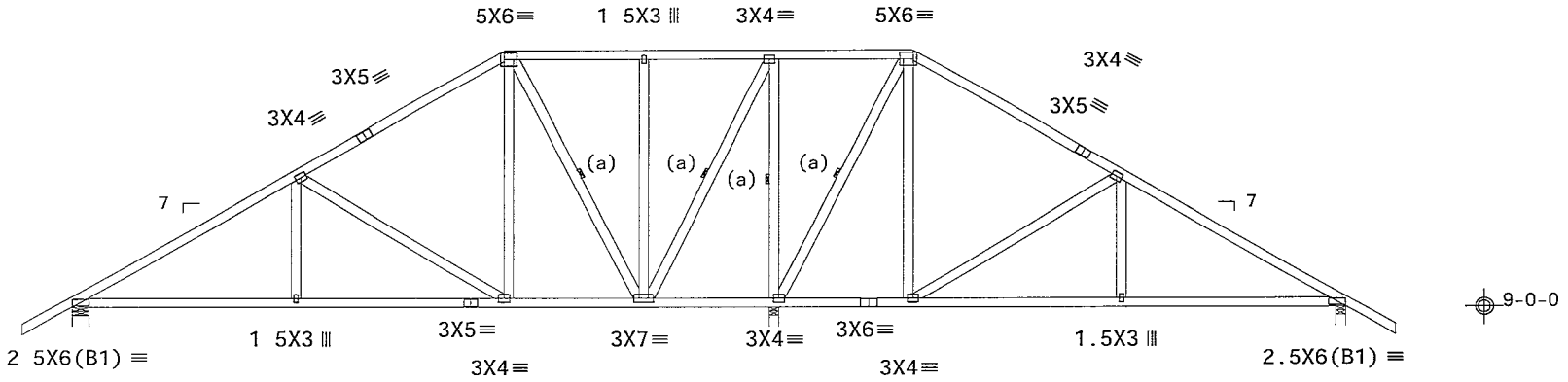
MWFRS loads based on trusses located at least 7 50 ft from roof edge

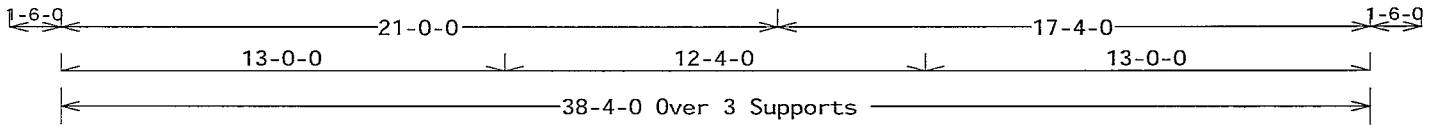
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=803 U=38 W=6" (6" min)
RL=144/-144

R=1763 U=55 W=3 5" (3 5" min)

R=624 U=32 W=3 5" (3 5" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19108
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006058
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337595
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - H13A 47 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

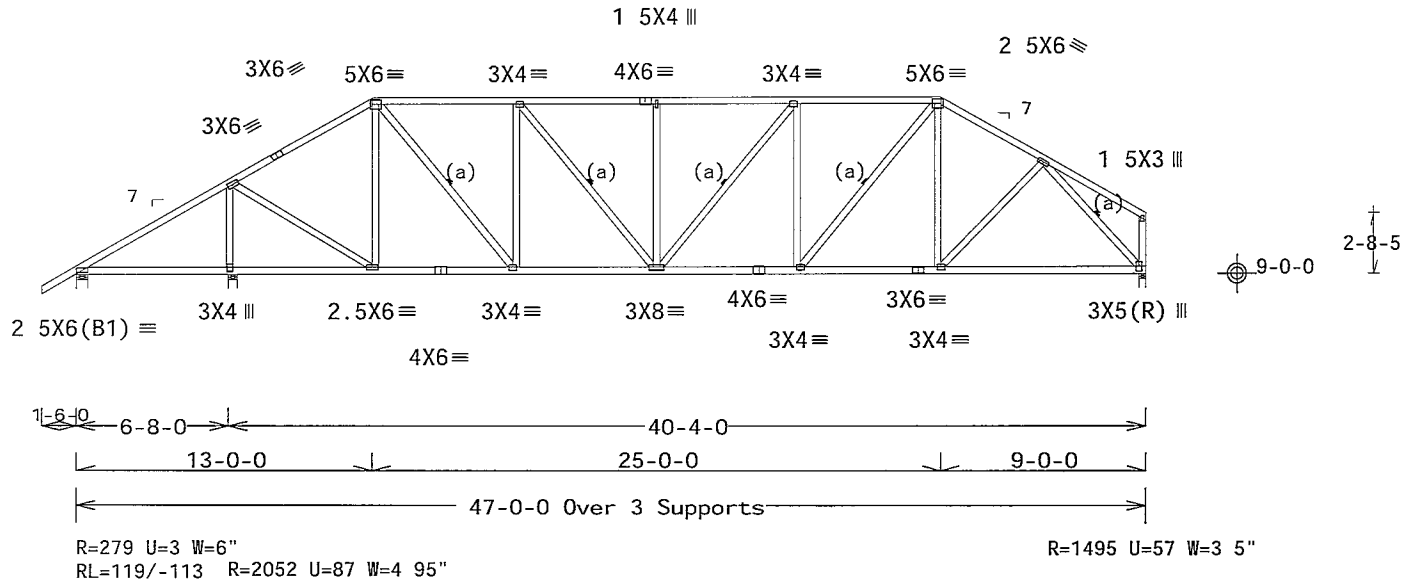
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0806.13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19109
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007019
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338389
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - H15 38'4' Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B B1 2x4 SP M-30
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

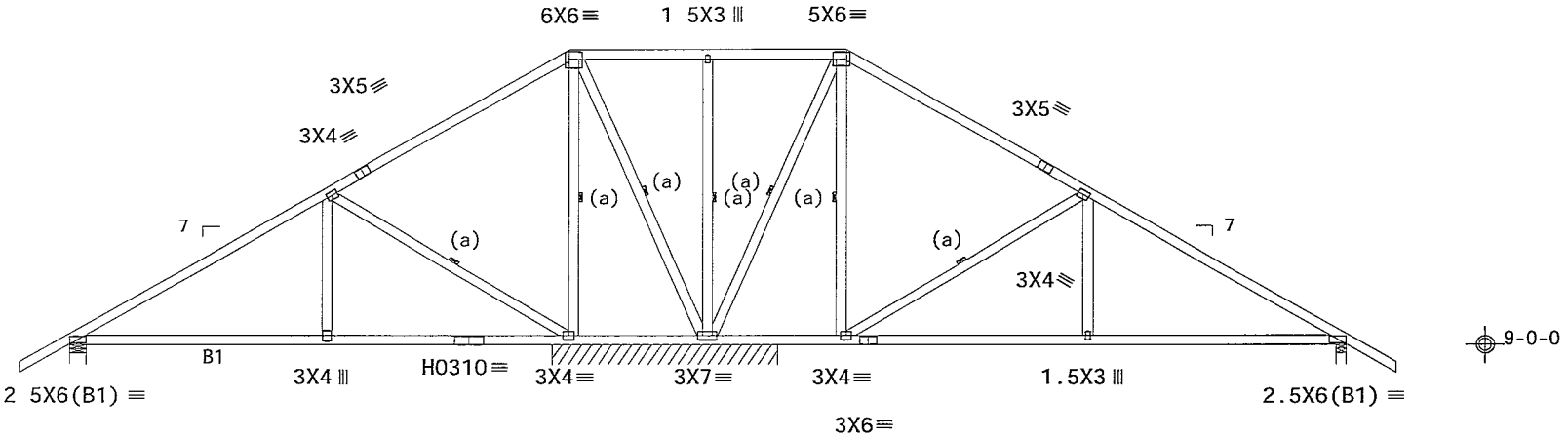
MWFRS loads based on trusses located at least 15 00 ft from roof edge

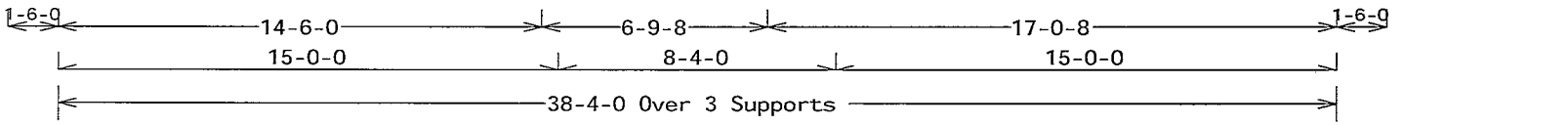
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=752 U=0 W=6" (6" min)
RL=163/-163

R=396 PLF U=0 PLF W=6-9-8

R=680 U=21 W=3 5" (3 5" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 19110
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006063
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD.	37 0 PSF	SEQN-	337588
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

01/07/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - H15A 47 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

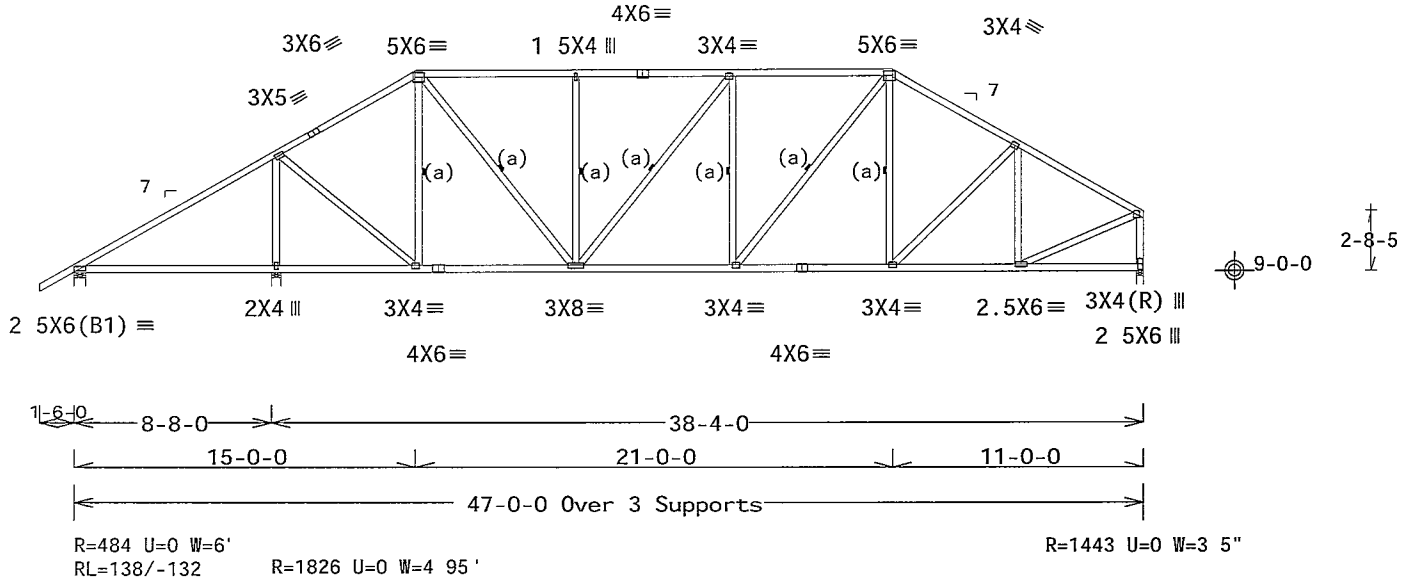
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 0826.13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricat ng handling sh pp ng insta ng and brac ng Refer to and follow the latest edition of BCSI (Build ng Component Safety Informat on by TPI and WTCA) for safety practices prior to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from this design any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handl ng sh pp ng installation brac ng of trusses Apply plates to each face of truss and pos tion as shown above and on the Joint Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s draw ng or cover page I st ng this draw ng indicates acceptance of profess onal eng neer ng responsibil ty solely for the design shown The suitabil ty and use of th s des gn for any structure s the responsibil ty of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19111
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007020
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338395
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - H17 38 4" Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B B1 2x4 SP M-30
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)

TC- From	56 plf at -1 50 to	56 plf at 6 99
TC- From	56 plf at 6 99 to	56 plf at 17 00
TC- From	56 plf at 17 00 to	56 plf at 21 33
TC- From	56 plf at 21 33 to	56 plf at 29 97
TC- From	56 plf at 29 97 to	56 plf at 39 83
BC- From	5 plf at -1 50 to	5 plf at 0 00
BC- From	20 plf at 0 00 to	20 plf at 12 00
BC- From	20 plf at 12 00 to	20 plf at 24 00
BC- From	20 plf at 24 00 to	20 plf at 38 33
BC- From	5 plf at 38 33 to	5 plf at 39 83

BC- 200 00 lb Conc Load at 5 50

Wind loads and reactions based on MWFRS with additional C&C member design

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

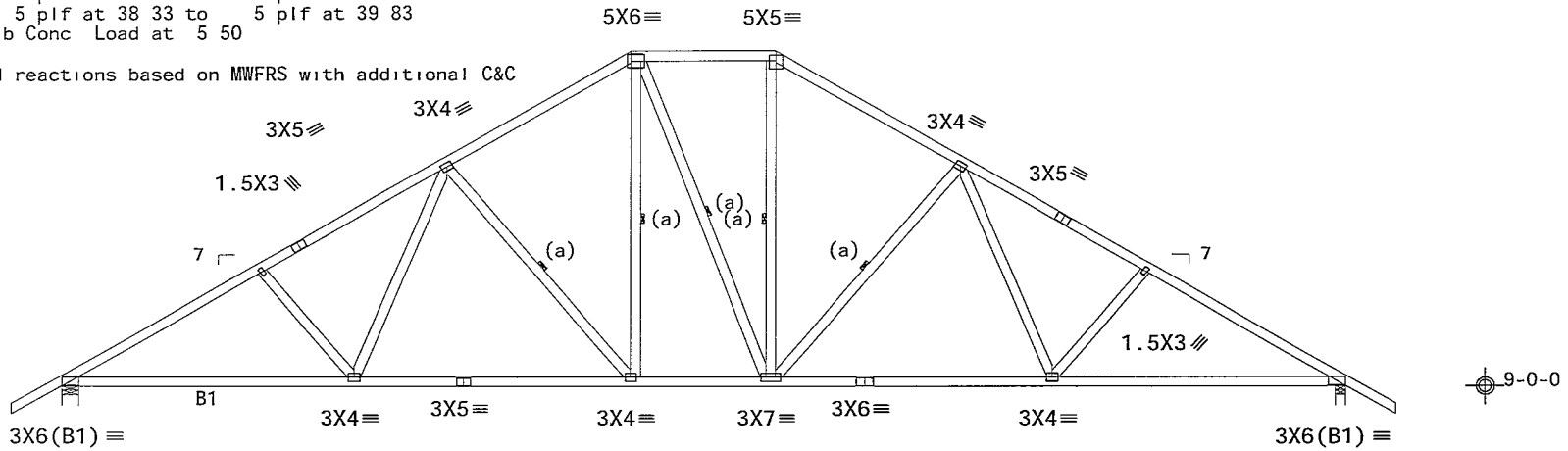
(a) Continuous lateral restraint equally spaced on member

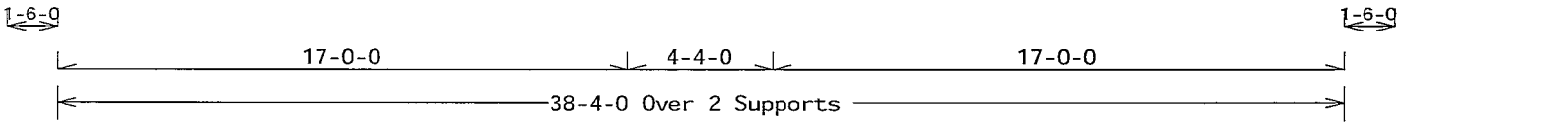
In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge





R=1724 U=0 W=6" (6" min)
RL=181/-181

R=1580 U=0 W=3 5" (3 5" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04-[illegible]13 | QTY:1 | FL/-/5/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19112
TC DL	7 0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006065
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337598
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - H17A 47' Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING' note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

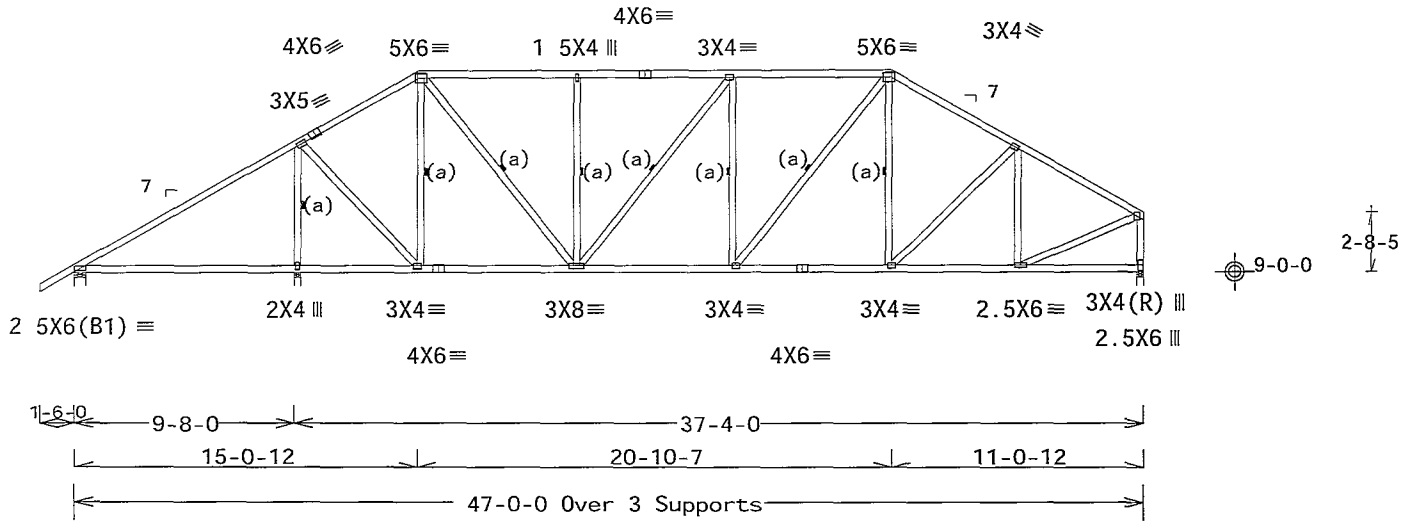
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 Job # QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19113
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007021
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	338400
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - H19A 47 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

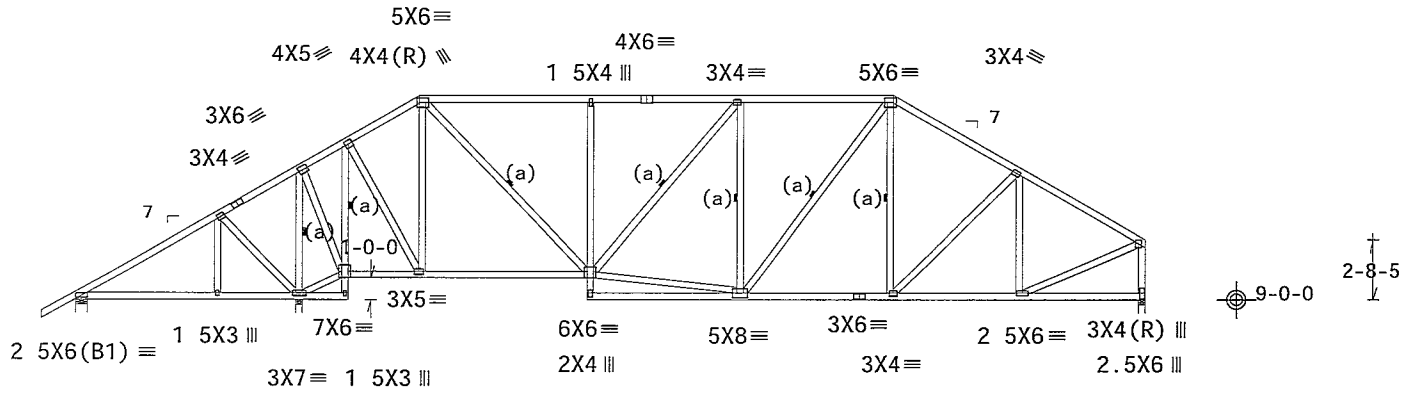
Wind loads and reactions based on MWFRS with additional C&C member design

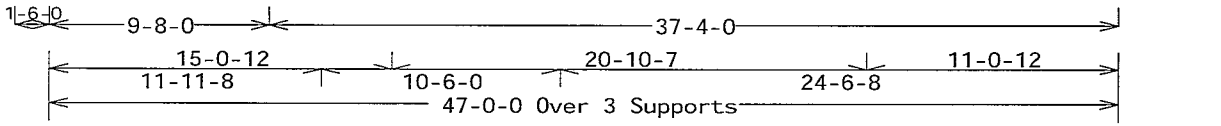
Right end vertical not exposed to wind pressure

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge





R=329 U=0 W=6"
RL=139/-133

R=2266 U=0 W=3 5"

R=1473 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:3 FL/-/5/-/-/R/- Scale =.125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19114
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007022
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338404
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - H7 38'4 Stepdown Hip Girder)

Top chord 2x4 SP #1-13B T3 2x4 SP M-30
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B W8 2x4 SP #2-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Brg blocks 0 131 x3 min nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	21 000	1	17	11	Rigid Surface

Brg block to be same size and species as chord
Refer to drawing CNNAILSP0109 for more information

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

(a) Continuous lateral restraint equally spaced on member

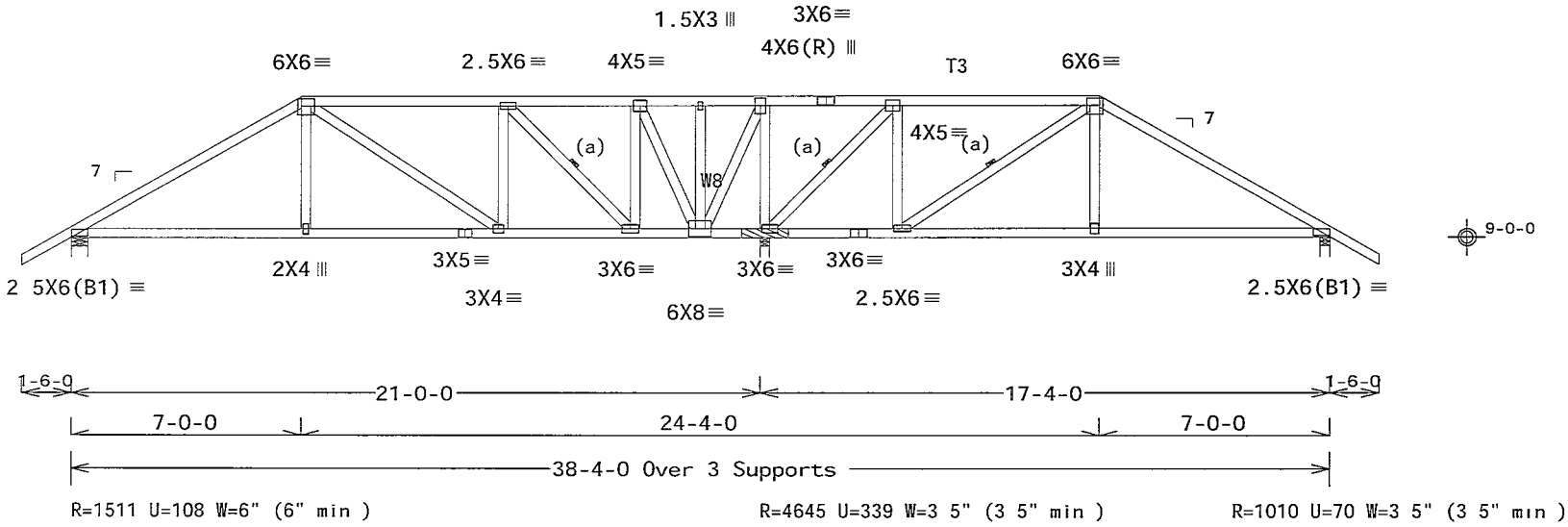
Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

WARNING! This truss is not symmetric but its exterior geometry makes erection error more probable It is imperative that this truss be installed properly Truss manufacturer is to mark this truss for proper erection

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at -1 50 to 56 plf at 7 00
TC- From 28 plf at 7 00 to 28 plf at 23 00
TC- From 28 plf at 23 00 to 28 plf at 31 33
TC- From 56 plf at 31 33 to 56 plf at 39 83
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 12 00
BC- From 10 plf at 12 00 to 10 plf at 24 00
BC- From 10 plf at 24 00 to 10 plf at 31 30
BC- From 20 plf at 31 30 to 20 plf at 38 33
BC- From 5 plf at 38 33 to 5 plf at 39 83
TC- 236 91 lb Conc Load at 7 03,31 30
TC- 169 81 lb Conc Load at 9 06,11 06,13 06 15 06 17 06 19 06 19 27 21 27 23 27 25 27,27 27,29 27
BC- 455 39 lb Conc Load at 7 03 31 30
BC- 127 34 lb Conc Load at 9 06,11 06,13 06 15 06 17 06 19 06 19 27 21 27 23 27,25 27 27 27,29 27

In lieu of structural panels use purlins to brace all flat TC @ 24 OC



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19115
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006047
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	338137
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - H7A 51 Stepdown Hip Girder)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B W2 W16 2x4 SP #1-13B
W4 W14 2x4 SP #2-13B
Rt Splice Block 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at -1 50 to 56 plf at 7 00
TC- From 28 plf at 7 00 to 28 plf at 19 00
TC- From 28 plf at 19 00 to 28 plf at 31 00
TC- From 28 plf at 31 00 to 28 plf at 44 00
TC- From 56 plf at 44 00 to 56 plf at 52 50
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 14 00
BC- From 10 plf at 14 00 to 10 plf at 28 00
BC- From 10 plf at 28 00 to 10 plf at 40 00
BC- From 10 plf at 40 00 to 10 plf at 43 97
BC- From 20 plf at 43 97 to 20 plf at 51 00
BC- From 5 plf at 51 00 to 5 plf at 52 50
TC- 236 91 lb Conc Load at 7 03 43 97
TC- 169 81 lb Conc Load at 9 06 11 06 13 06 15 06 17 06 19 06 21 06 23 06 25 06 25 94 27 94 29 94 31 94 33 94 35 94 37 94 39 94 41 94
BC- 455 39 lb Conc Load at 7 03 43 97
BC- 127 34 lb Conc Load at 9 06 11 06,13 06 15 06 17 06 19 06 21 06 23 06 25 06 25 94 27 94 29 94 31 94 33 94 35 94 37 94 39 94 41 94
(51 06 12 88) (53 06 12 88) PLB- 1046 15 lb Conc Load at (

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131 x3 min nails
Top Chord 1 Row @12 00 o c
Bot Chord 1 Row @12 00 o c
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Calculated horizontal deflection is 0 14 due to live load and 0 18" due to dead load

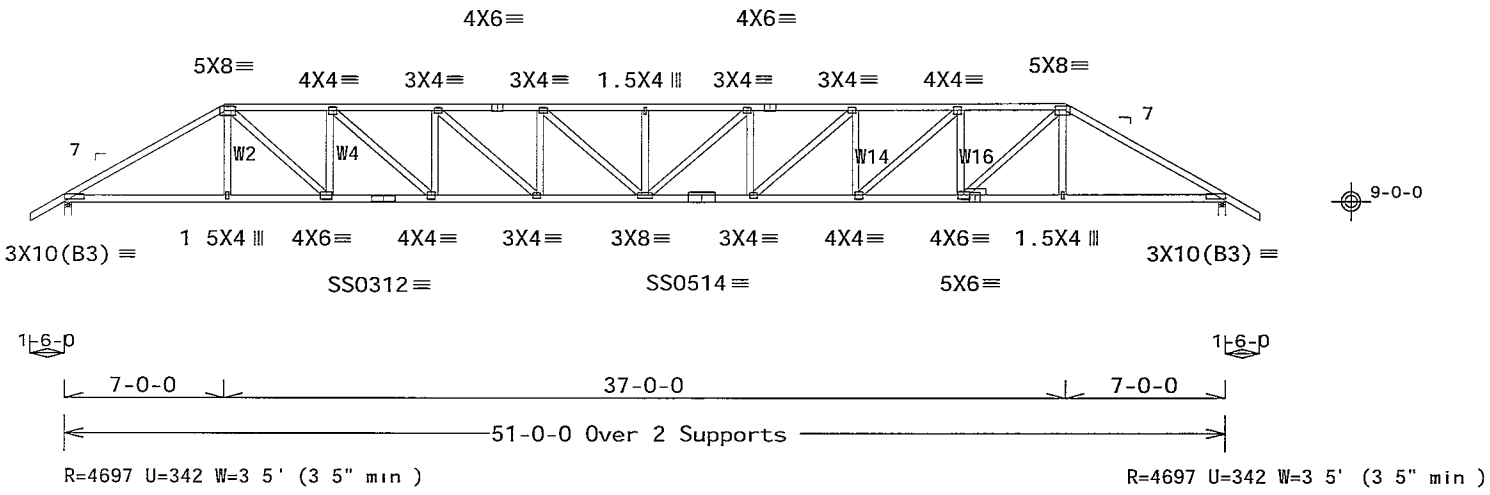
In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Calculated vertical deflection is 0 63 due to live load and 0 80 due to dead load at X = 25-6-0

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 OBRG 13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19116
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006052
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD	37.0 PSF	SEQN-	337602
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - H9 38 4 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

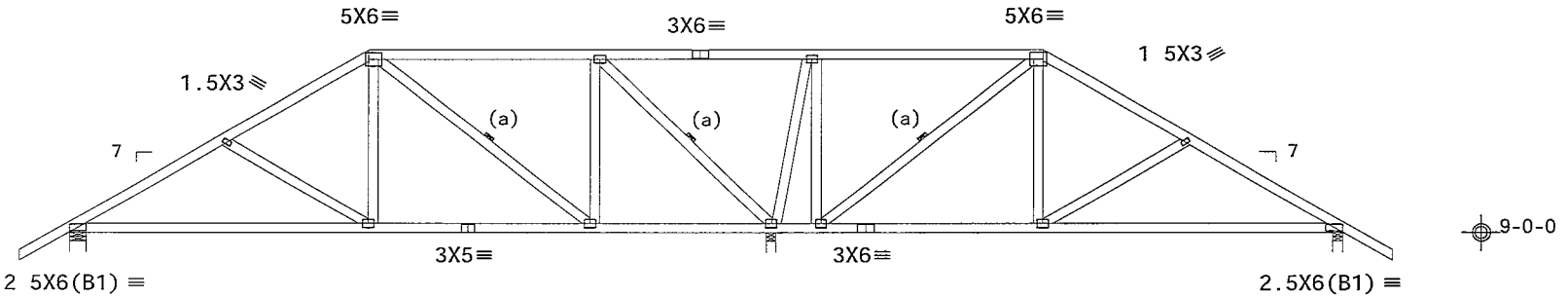
WARNING! This truss is not symmetric, but its exterior geometry makes erection error more probable It is imperative that this truss be installed properly Truss manufacturer is to mark this truss for proper erection

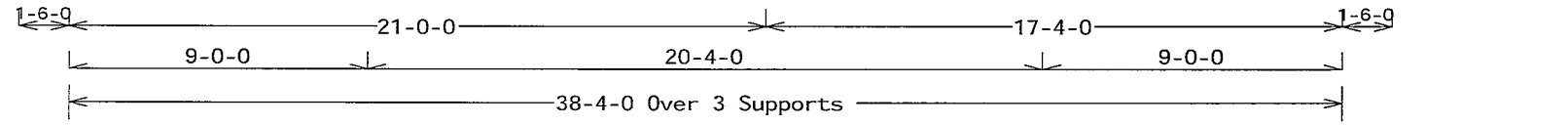
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=815 U=31 W=6" (6" min)
RL=108/-108

R=1702 U=80 W=3 5" (3 5" min)

R=668 U=22 W=3 5" (3 5" min)

Note All Plates Are 3X4 Except As Shown

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricat ng handling shipp ng nstall ng and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety pract ces prior to performing these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ceiling Locations shown for permanent lateral restraint of web shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu lding Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for hand l ng sh pp ng installation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page l sting th s drawing nd cates acceptance of profess onal engineering respons b l ty for the des gn shown The su tab l ity and use of th s design for any structure is the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20 0 PSF	REF	R9114- 19117
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006050
BC LL	0 0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	337590
DUR.FAC.	1 25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - H9A 51 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Calculated horizontal deflection is 0 13" due to live load and 0 17" due to dead load

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

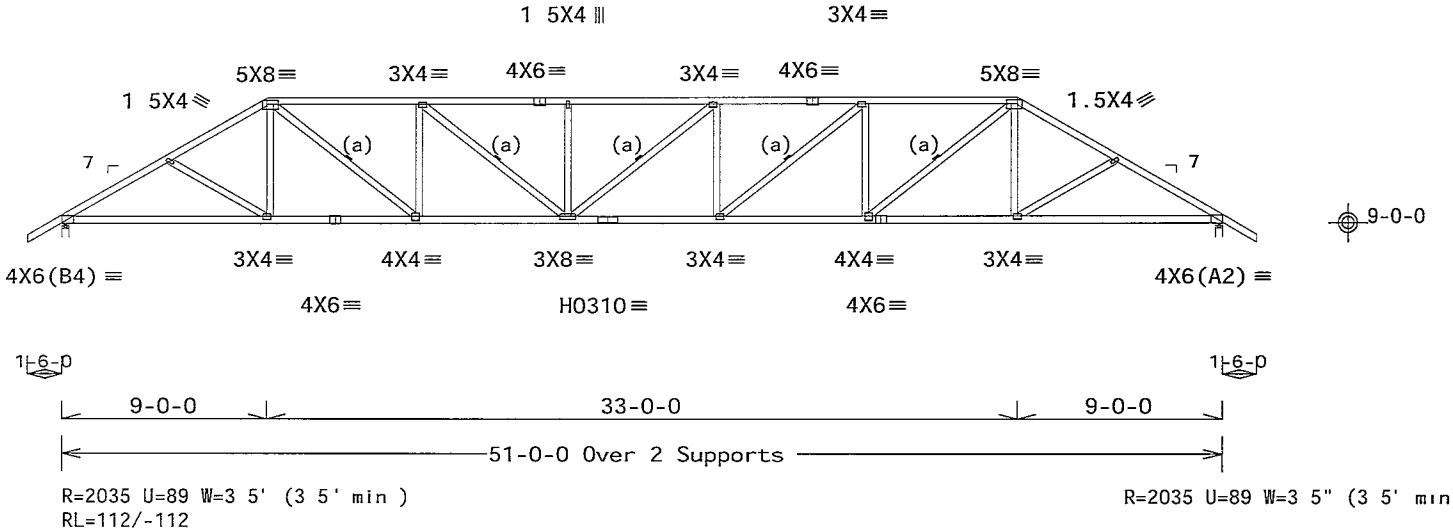
Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Calculated vertical deflection is 0 45" due to live load and 0 59' due to dead load at X = 22-2-12

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03 04 0326 13 QTY.1 FL/-/5/-/-/R/- Scale = 125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng hand ng shipping nstall ng and brac ng Refer to an follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety pract ces prior to perform ng these funct ons Installers shall provide temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restra nt of web shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s dc any fa lure to bu ld the truss in conformance w th ANSI/TPI 1 or for handl ng shipp ng nstallat o brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on th draw ng or cover page l st ng th s draw ng nd cates acceptance of profess onal eng neer ng respons b l ty solely for the des gn shown The su tab l ity and use of th s design for any structure s the responsib l ty of the Bu ld ng Des gner per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20 0 PSF	REF	R9114- 19118
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006051
BC LL	0 0 PSF	HC-ENG	SSB/WPF
TOT.LD	37 0 PSF	SEQN-	337597
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

01/07/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - HJ7 9 10 13 Hip Jack Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

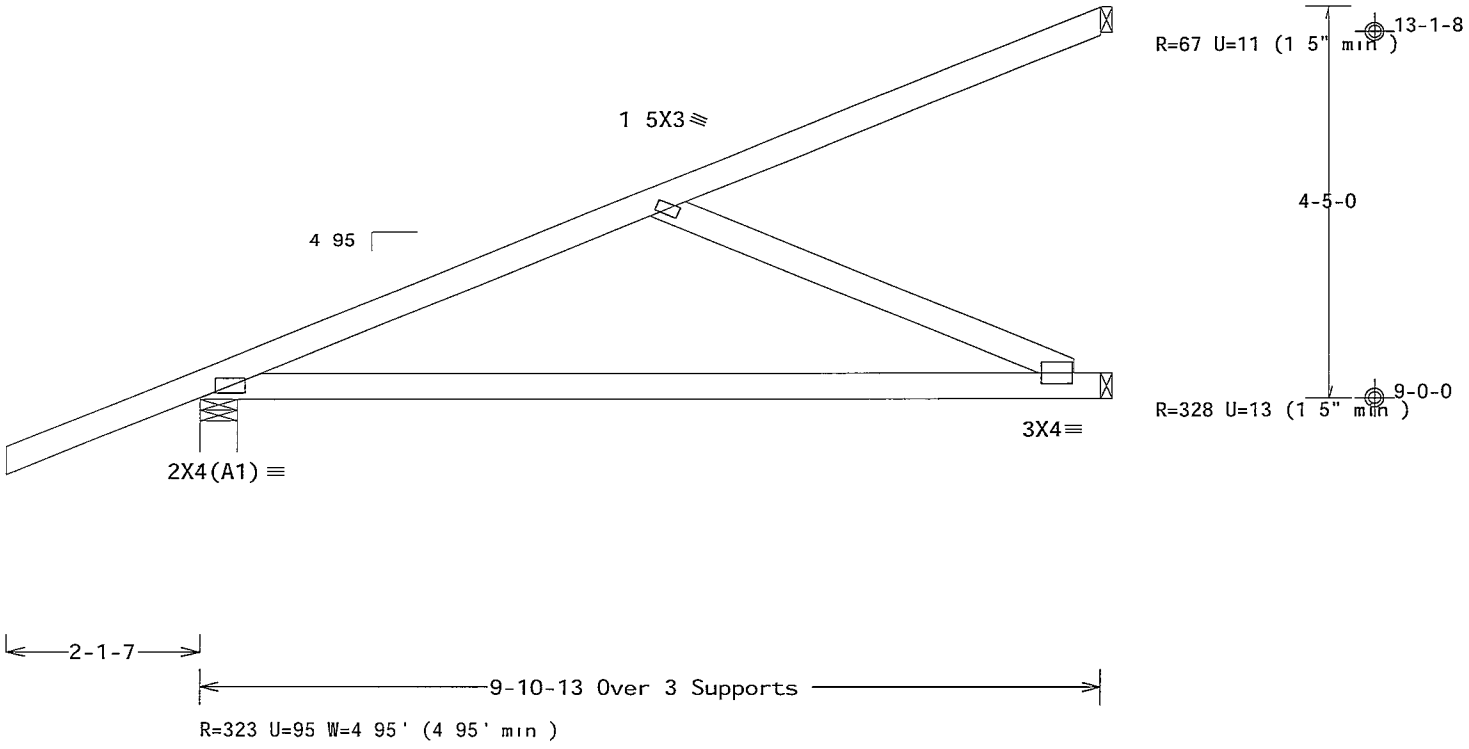
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 12 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 90
BC- From 0 plf at -2 12 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 90
TC- -34 00 lb Conc Load at 1 48
TC- 112 38 lb Conc Load at 4 31
TC- 231 19 lb Conc Load at 7 13
BC- 15 06 lb Conc Load at 1 48
BC- 99 91 lb Conc Load at 4 31
BC- 178 08 lb Conc Load at 7 13

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 03.13 QTY:4 FL/-/5/-/-/R/- Scale = 5"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from this design any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handl ng sh pp ng nstallation brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Deta is unless noted otherwise Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s draw ng or cover page l st ng this draw ng nd cates acceptance of profess onal eng neer ng responsibility solely for the des gn shown The suitability and use of this design for any structure s the responsibil ty of the Bu ilding Des gner per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19119
TC DL	7 0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006048
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD	37 0 PSF	SEQN-	337601
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - HJ7A 9'10'13 Hip Jack Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with '13B use design values approved 1/30/2013 by ALSC

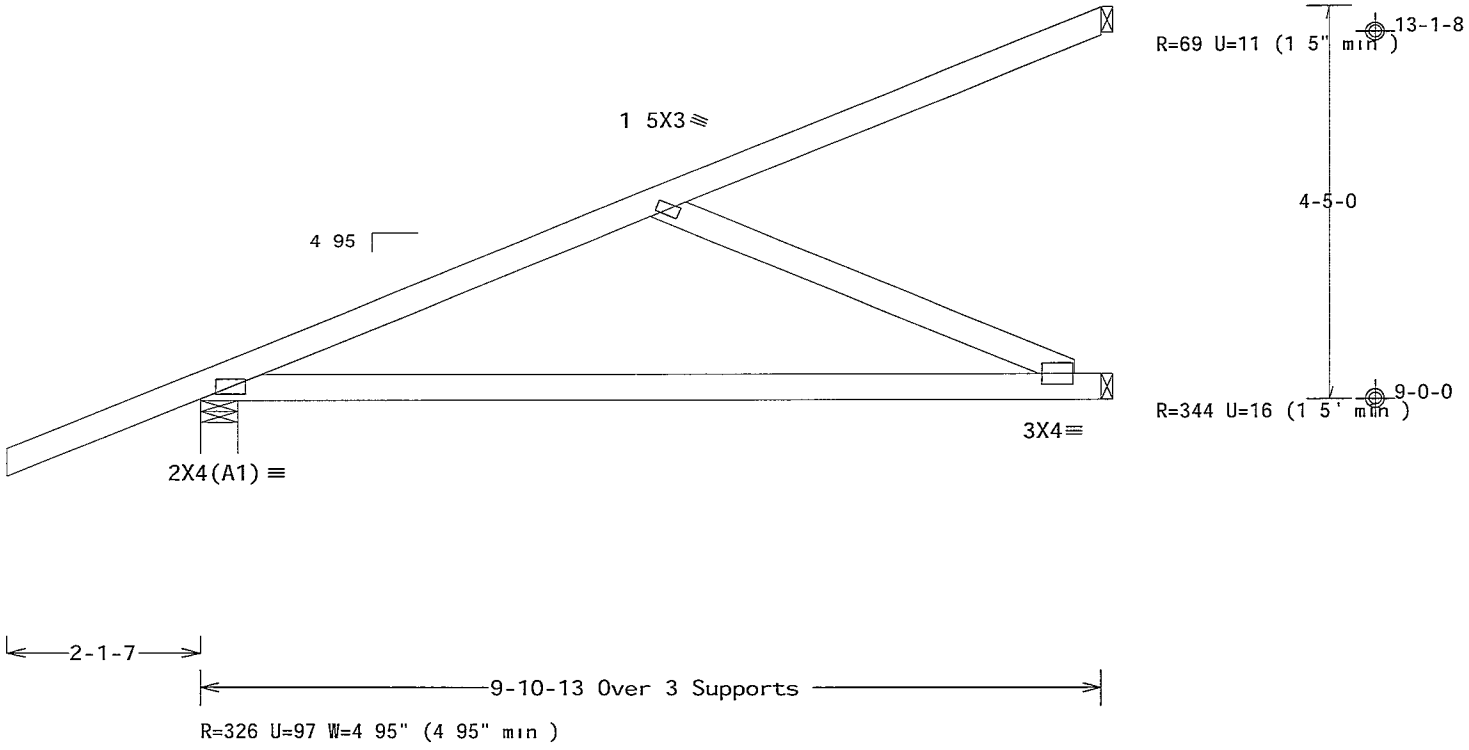
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 12 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 90
BC- From 0 plf at -2 12 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 90
TC- -34 00 lb Conc Load at 1 48
TC- 129 74 lb Conc Load at 4 31
TC- 240 88 lb Conc Load at 7 13
BC- 15 06 lb Conc Load at 1 48
BC- 103 77 lb Conc Load at 4 31
BC- 180 92 lb Conc Load at 7 13

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04 03:26:13 | QTY:1 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any deviation from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping nstallation bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard p ate pos t ons A seal on this drawing or cover page listing th s drawing nd cates acceptance of profess onal engineering respons b l ty solely for the design shown The suitability and use of th s des gn for any structure is the responsib lity of the Build ng Des gner per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www ccsafe org

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19120
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006060
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337735
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - J10 9'10'8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

These support conditions used at bearings indicated
(H1) = LU24 w/ (2)2x6 SP #1 Dense-13B supporting member
(4) 0 148 x3 nails into supporting member,
(2) 0 148 x1 5 nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

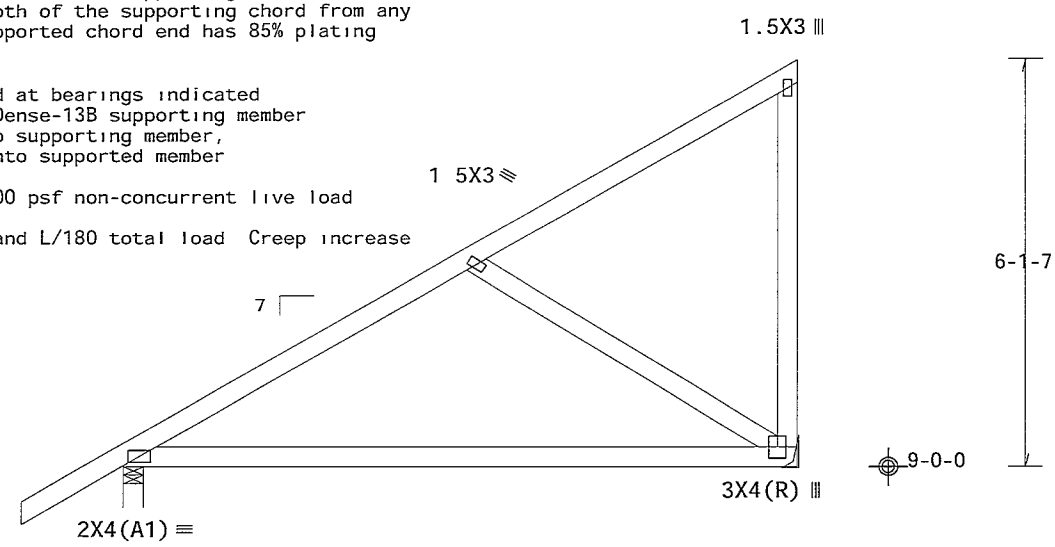
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 OBRG-13 QTY:4 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19121
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006062
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD	37.0 PSF	SEQN-	337726
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - J10B 9 10 8 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (2)2x6 SP #1 Dense-13B supporting member
(4) 0 148 x3 nails into supporting member
(2) 0 148 x1 5 nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 30 00 ft from roof edge

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

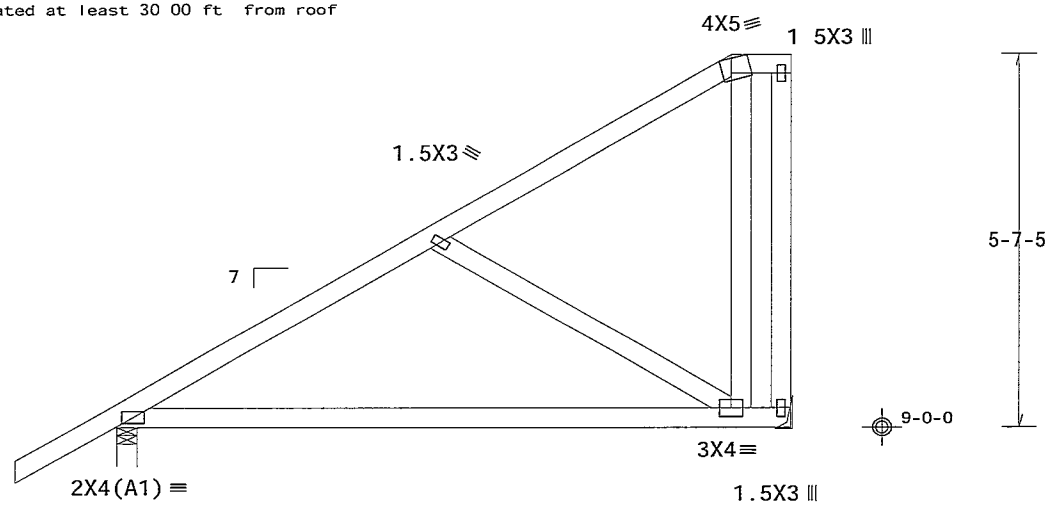
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QBBC-13 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20 0 PSF	REF	R9114- 19122
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006061
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	337742
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - J7 6 10 8 End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

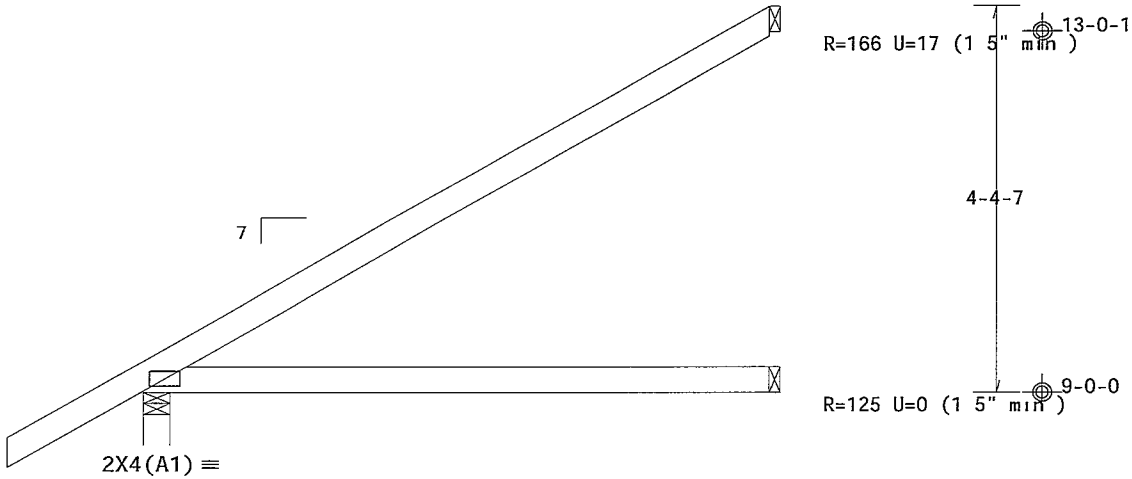
Bottom chord checked for 10 00 psf non-concurrent live load

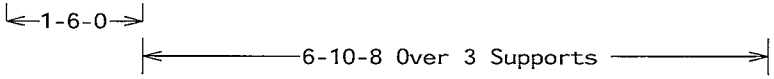
MWFRS loads based on trusses located at least 30 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=371 U=0 W=3 5 (3 5' min)
RL=54/-31

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]43 QTY:15 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19123
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006068
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337719
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - J7B 7' End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

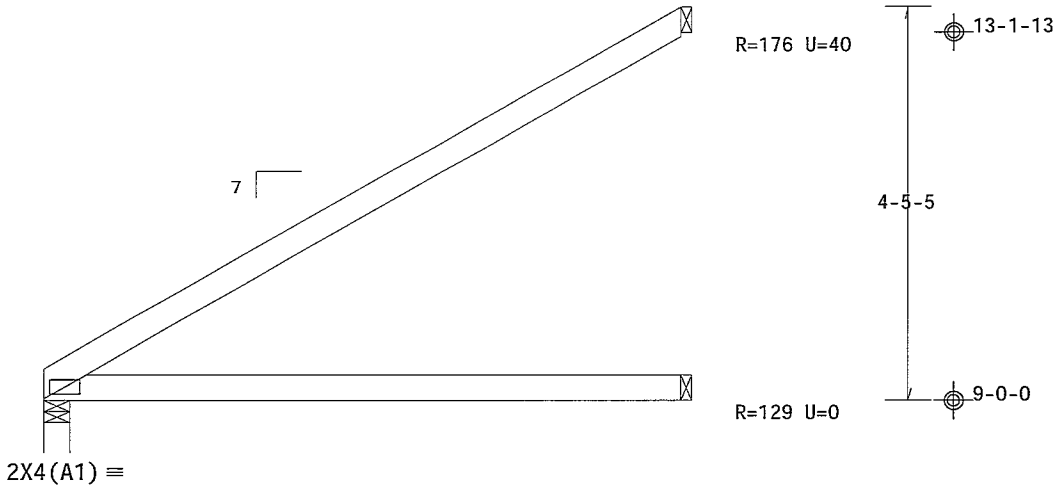
Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:2 FL/-/5/-/-/R/- Scale = 5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care in fabr cat ng handl ng shipping nstall ng and brac ng Refer to and follow the latest edit on of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for safety practices pr or to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rig d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any dev at on from this des n any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s draw ng or cover page l st ng th s draw ng nd cates acceptance of profess onal eng neer ng respons bil ty solely for the design shown The su tab l ty and use of th s des gn for any structure s the responsibil ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19124
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007023
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	338873
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - MH11 47' Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

End verticals not exposed to wind pressure

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder

These support conditions used at bearings indicated
(H1) = LU24 w/ (3)2x8 SP SS-13B supporting member
into supporting member,
into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

120 mph wind, 15 70 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 26" DL 0 33" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

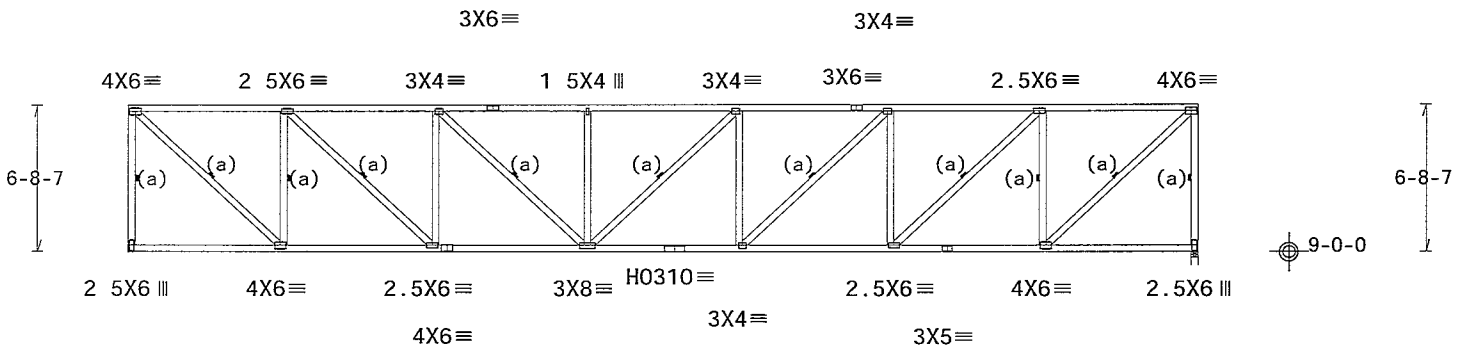
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING" note below

MWFRS loads based on trusses located at least 7 85 ft from roof edge



47-0-0 Over 2 Supports

R=1739 U=87
H=H1

R=1739 U=87 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12 03.04 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care in fabr cat ng handl ng sh pping install ng and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety pract ces prior to performing these funct ons Installers shall prov do temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from th s des g any fa lure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstal at on & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos t ons A seal on th s drawing or cover page l sting th s draw ng nd cates acceptance of profess ona eng neering respons b l ty solely for the des gn shown The su tab l ty and use of th s des gn for any structure is the respons b l ty of the Build ng Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19125
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006064
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	337808
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - MH13 47 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder

These support conditions used at bearings indicated
(H1) = LU24 w/ (3)2x8 SP SS-13B supporting member
into supporting member,
into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 21" DL 0 28" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

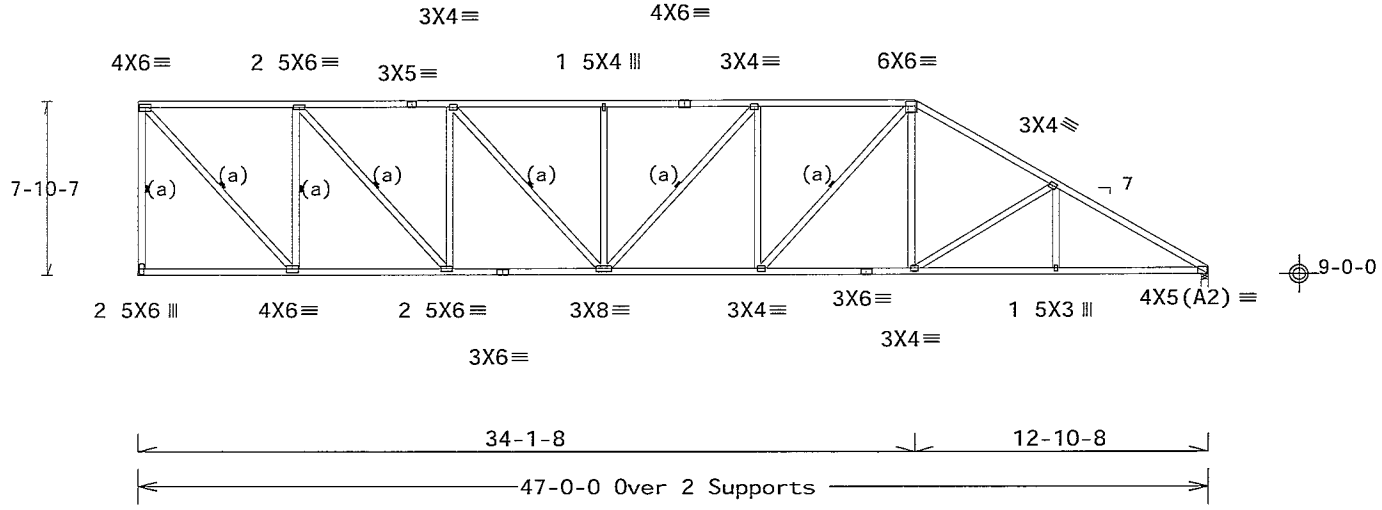
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0886 13 QTY:2 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19126
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006059
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37 0 PSF	SEQN-	337810
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - MH15 47 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

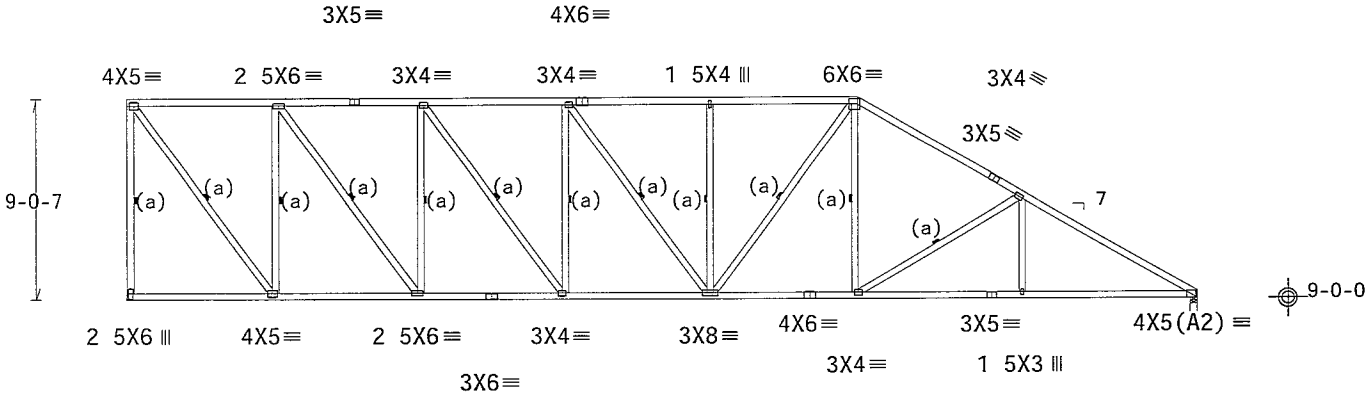
Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 18" DL 0 25" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



R=1785 U=91
RL=44/-143
H=H1

R=1796 U=52 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20.0 PSF	REF	R9114- 19127
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007024
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338705
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - MH17 47 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required, by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

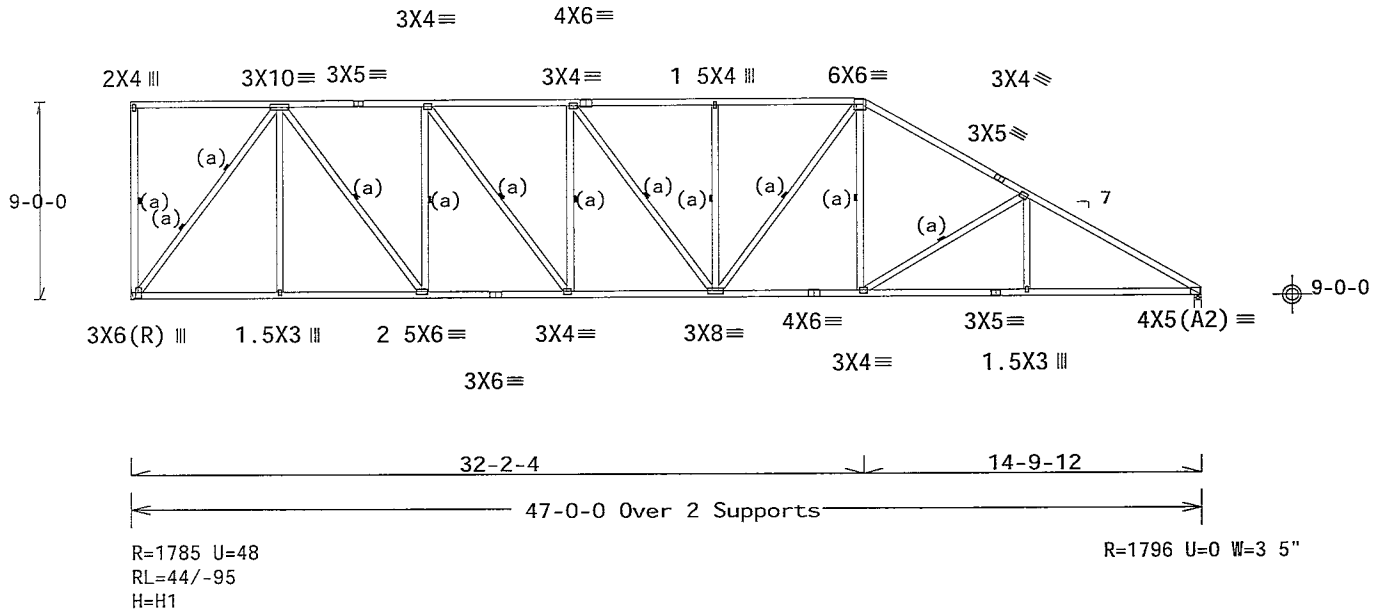
Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 17" DL 0 23" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19128
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007025
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	338478
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - MH19 47' Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B" use design values approved 1/30/2013 by ALSC

Left end vertical not exposed to wind pressure

(J) Hanger Support Required by others

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See 'WARNING note below

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

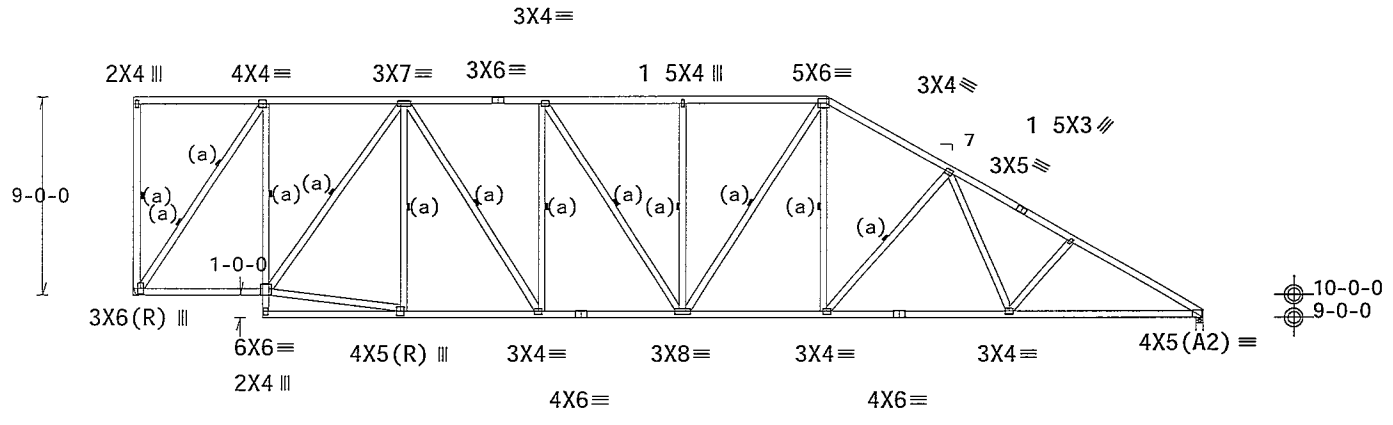
Wind loads and reactions based on MWFRS with additional C&C member design

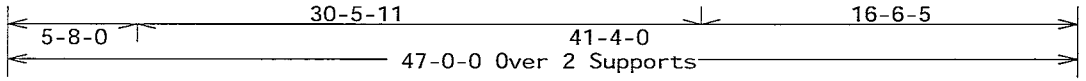
Max JT VERT DEFL LL 0 16" DL 0 22" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 15 00 ft from roof edge





R=1785 U=48
RL=49/-106
H=H1

R=1796 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19129
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007026
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338472
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - MH7 48 10" Mono Hip Girder)

Top chord 2x4 SP #1-13B T1 T2 2x4 SP M-30
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B W8 2x4 SP #2-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 37 38
TC- From 56 plf at 37 38 to 56 plf at 40 38
TC- From 56 plf at 40 38 to 56 plf at 48 83
BC- From 20 plf at 0 00 to 20 plf at 48 83
TC- 166 48 lb Conc Load at 1 09 3 09 5 09 6 94 8 94 10 94 12 94 14 94 16 94 18 94 20 94 22 94 24 94 26 94 28 94
BC- 124 96 lb Conc Load at 1 09 3 09, 5 09, 6 94 8 94 10 94 12 94 14 94 16 94 18 94 20 94,22 94,24 94 26 94 28 94
BC- 1126 12 lb Conc Load at 37 56
BC- 362 23 lb Conc Load at 39 56 41 56 43 56 45 56 47 56

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

The TC of this truss shall be braced with attached spans at 24 OC in lieu of structural sheathing

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131 x3 , min nails
Top Chord 1 Row @12 00" o c
Bot Chord 1 Row @12 00 o c
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

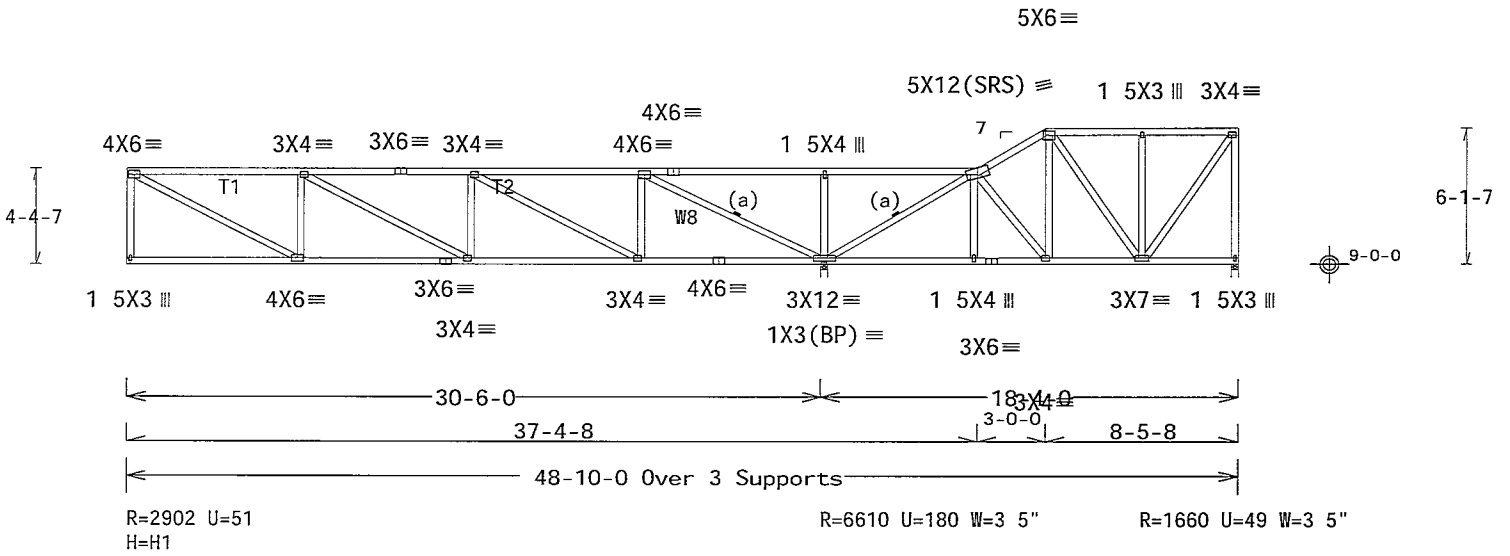
End verticals not exposed to wind pressure

Max JT VERT DEFL LL 0 15 DL 0 21" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

(J) Hanger Support Required, by others

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING note below



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care in fabr cat ng handl ng shipping install ng and brac ng Refer to and follow the latest ed t on of BCSI (Bu lding Component Safety Informat on by TPI and WTCA) for safety practices prior to perform ng these funct ons Installers shall prov de temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from this des any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng installation brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate posit ons A seal on this draw ng or cover page listing this draw ng ind cates acceptance of profess onal eng neer ng responsibility solely for the design shown The su tabil ty and use of th s design for any structure s the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20 0 PSF	REF	R9114- 19130
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007039
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37.0 PSF	SEQN-	338880
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - MH9 48 10' Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B W2 2x4 SP #2-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

End verticals not exposed to wind pressure

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder

These support conditions used at bearings indicated
(H1) = LU24 w/ (3)2x8 SP SS-13B supporting member
into supporting member,
into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

WARNING Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses See WARNING" note below

120 mph wind, 15 41 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Max JT VERT DEFL LL 0 40" DL· 0 55" See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

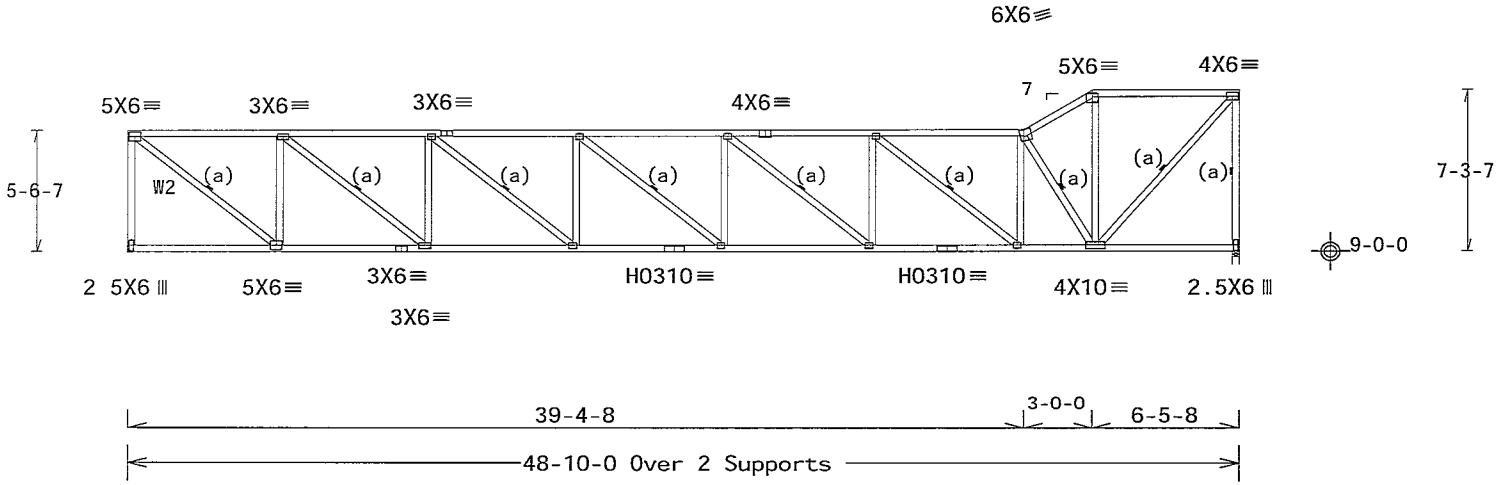
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 71 ft from roof edge



Note All Plates Are 3X4 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care in fabr cat ng handl ng shipp ng nstalling and brac ng Refer to and follow the latest edition of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for safety pract ces pr or to perform ng these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this des any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handl ng sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s draw ng or cover page l st ng this draw ng ind cates acceptance of professional eng neer ng responsibility solely for the design shown The suitab l ity and use of th s design for any structure s the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page ITW-BCG www twbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19131
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006049
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD	37.0 PSF	SEQN-	338087
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - MHG10 9'10"8 Mono Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (2)2x4 SP #1-13B supporting member
(4) 0 148 x3 nails into supporting member
(2) 0 148 x1 5 nails into supported member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

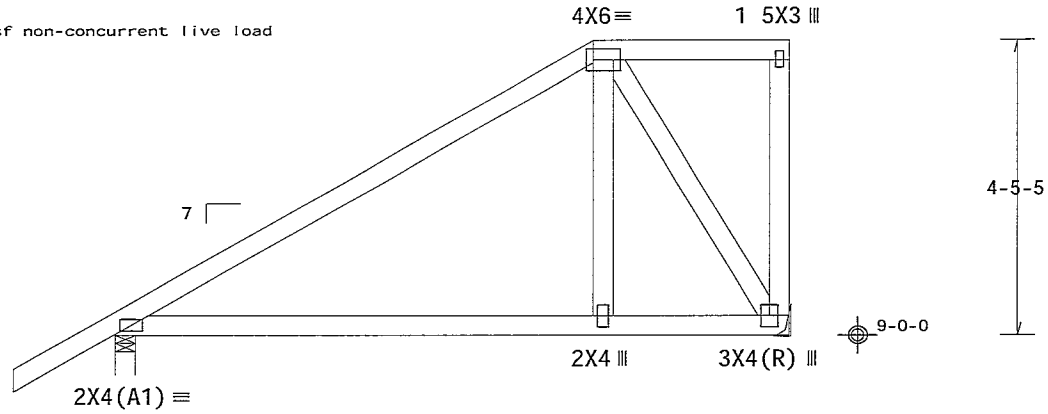
Bottom chord checked for 10 00 psf non-concurrent live load

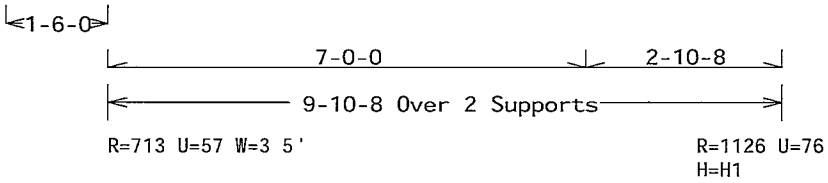
Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at -1 50 to 56 plf at 7 00
TC- From 56 plf at 7 00 to 56 plf at 9 88
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 9 88
TC- 245 25 lb Conc Load at 7 03
TC- 176 17 lb Conc Load at 9 06
BC- 472 92 lb Conc Load at 7 03
BC- 129 25 lb Conc Load at 9 06

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 03.04 0326 13 | QTY:1 | FL/-/5/-/-/R/- | Scale = 375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu lding Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from this de gn any fa lure to bu ld the truss in conformance w th ANSI/TPI 1 or for hand ng sh pp ng nstallat on & bracing of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on th draw ng or cover page l st ng this draw ng indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitab lity and use of th s des gn for any structure s the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www twbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20 0 PSF	REF	R9114- 19132
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10 0 PSF	DRW	HCUSR9114 14007038
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	338875
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - PBA1 20'10"7 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

120 mph wind, 18 71 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

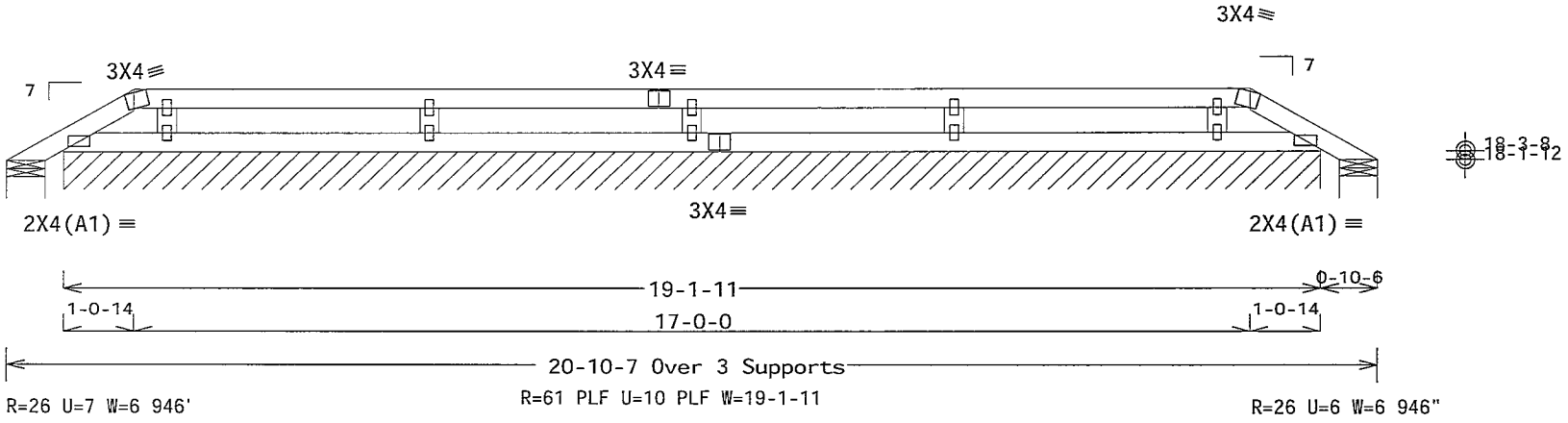
Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur.Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 1 94
TC- From 56 plf at 1 94 to 56 plf at 18 94
TC- From 56 plf at 18 94 to 56 plf at 20 87
BC- From 4 plf at 0 00 to 4 plf at 20 87

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 9 36 ft from roof edge



Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 [illegible] QTY.1 FL/-/5/-/-/R/- Scale = 375"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20 0 PSF	REF	R9114- 19133
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10 0 PSF	DRW	HCUSR9114 14007027
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	338493
DUR.FAC	1 25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - PBA10 32 2'4 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24 OC, all BC @ 24 OC

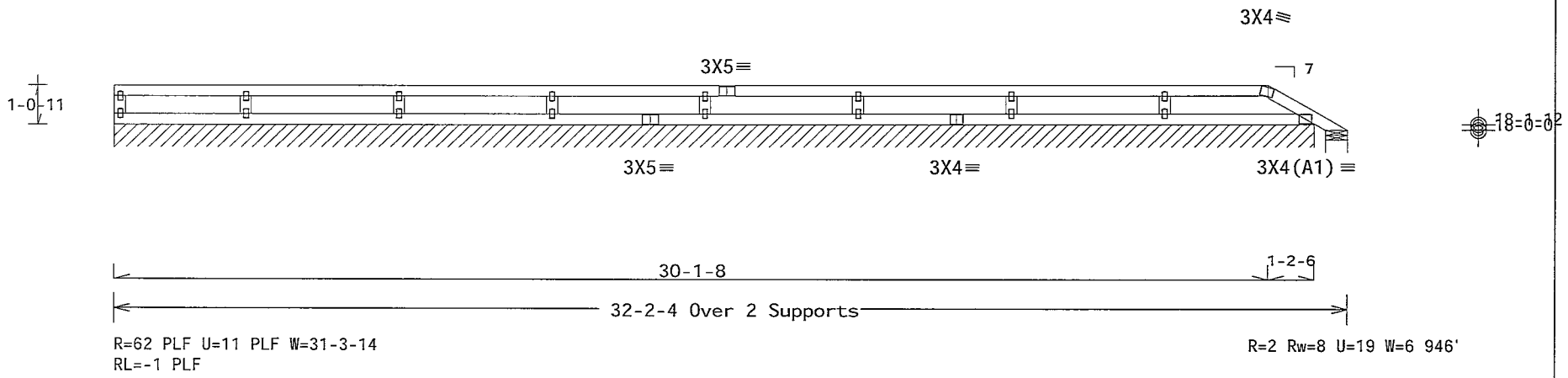
MWFRS loads based on trusses located at least 9 30 ft from roof edge

Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 30 13
TC- From 56 plf at 30 13 to 56 plf at 32 19
BC- From 4 plf at 0 00 to 4 plf at 32 19

120 mph wind, 18 60 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=62 PLF U=11 PLF W=31-3-14
RL=-1 PLF

R=2 Rw=8 U=19 W=6 946'

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible] QTY.1 FL/-/5/-/-/R/- Scale = 25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shal not be respons ble for any dev at on from th s des g any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng insta at on & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on this draw ng or cover page l st ng th s draw ng nd cates acceptance of profess onal eng neering respons b lity solely for the des gn shown The su tab lity and use of this design for any structure s the responsib l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19134
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10 0 PSF	DRW	HCUSR9114 14007028
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	338497
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - PBA2 20 10 7 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 19 29 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

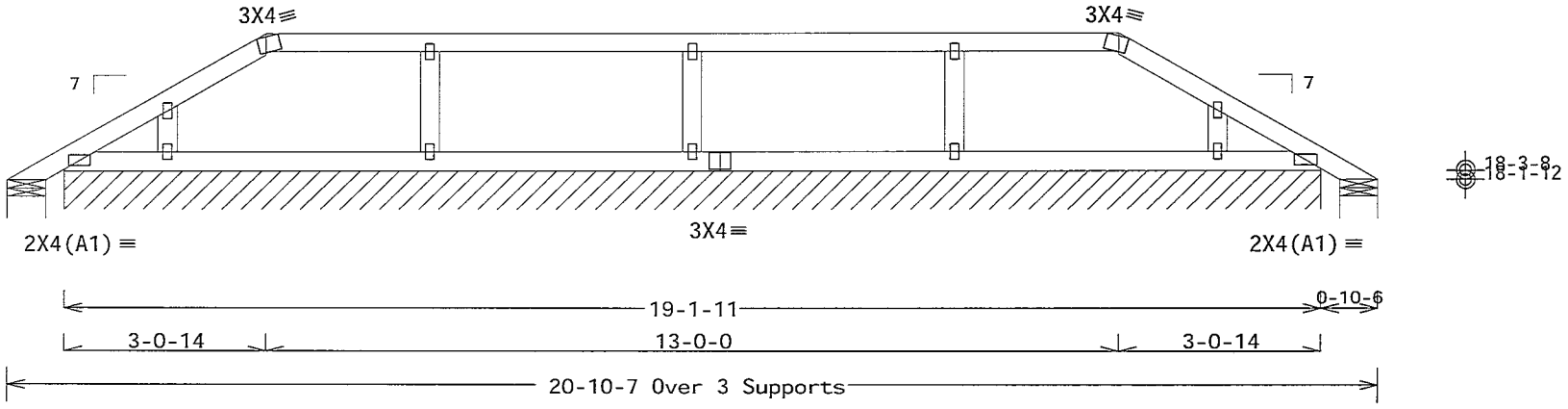
Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 3 94
TC- From 56 plf at 3 94 to 56 plf at 16 94
TC- From 56 plf at 16 94 to 56 plf at 20 87
BC- From 4 plf at 0 00 to 4 plf at 20 87

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24' OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 19 29 ft from roof edge



Note All Plates Are 1 5X3 Except As Shown
Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03 04 [illegible] QTY·1 FL/-/5/-/-/R/- Scale = 375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20 0 PSF	REF	R9114- 19135
TC DL	7 0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007029
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338484
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - PBA3 20 10 7 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 19 88 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

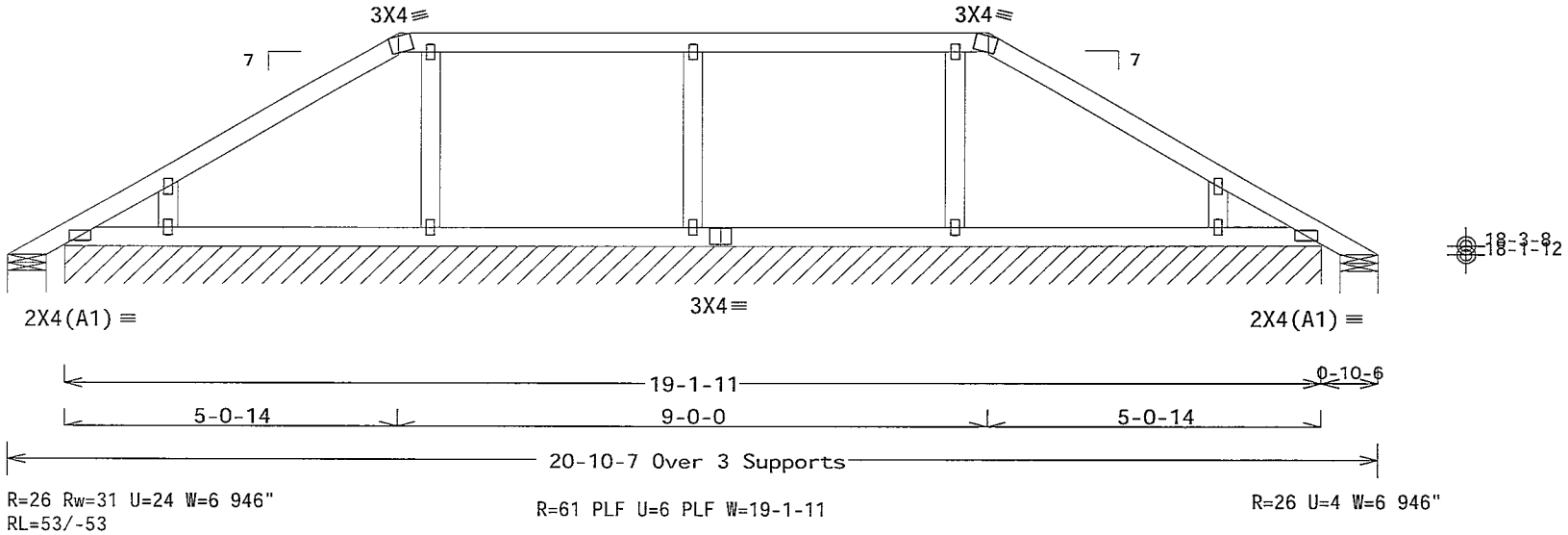
Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 5 94
TC- From 56 plf at 5 94 to 56 plf at 14 94
TC- From 56 plf at 14 94 to 56 plf at 20 87
BC- From 4 plf at 0 00 to 4 plf at 20 87

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24' OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 19 88 ft from roof edge



Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03.04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale = 375"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20 0 PSF	REF	R9114- 19136
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007030
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338501
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - PBA4 20'10 7 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 20 46 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

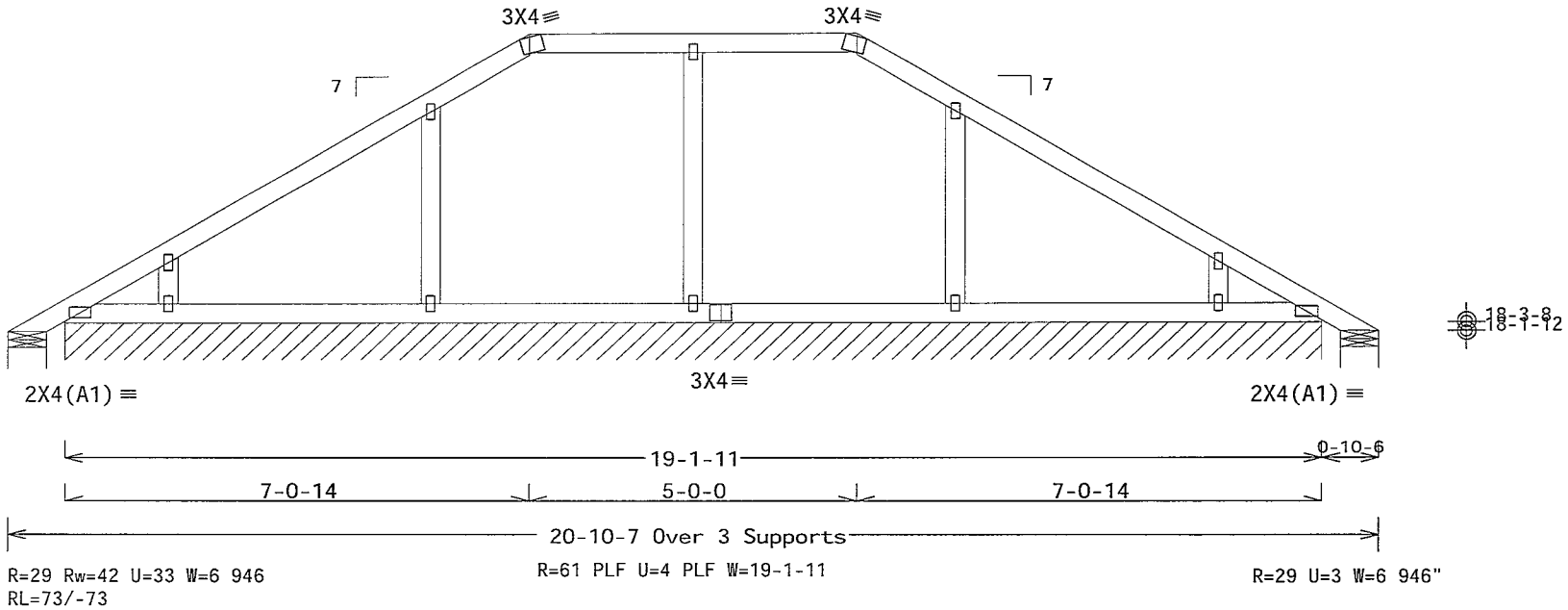
Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 7 94
TC- From 56 plf at 7 94 to 56 plf at 12 94
TC- From 56 plf at 12 94 to 56 plf at 20 87
BC- From 4 plf at 0 00 to 4 plf at 20 87

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 20 46 ft from roof edge



Note All Plates Are 1 5X3 Except As Shown

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]13 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19137
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007031
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	338495
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - PBA5 16 10'7 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24' OC

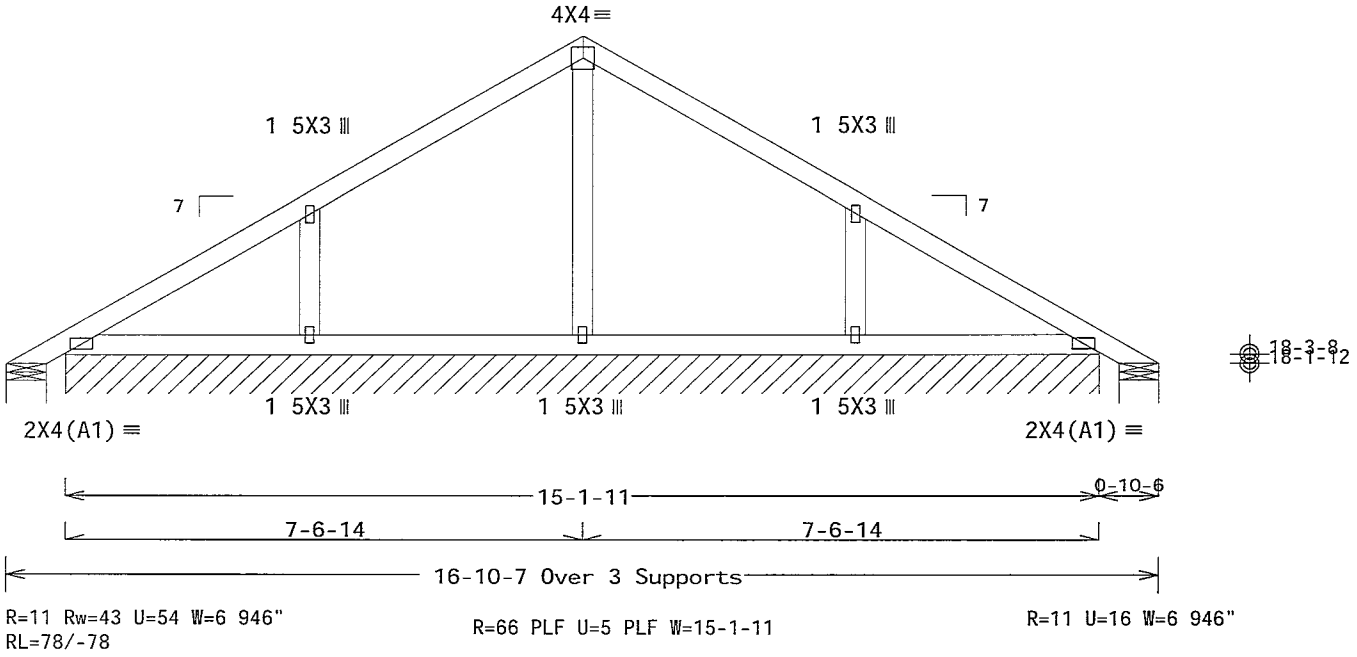
MWFRS loads based on trusses located at least 20 61 ft from roof edge

Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 8 44
TC- From 56 plf at 8 44 to 56 plf at 16 87
BC- From 4 plf at 0 00 to 4 plf at 16 87

120 mph wind, 20 61 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]13 QTY:5 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any deviation from th s design any fa lure to bu ld the truss in conformance w th ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this draw ng or cover page t sting th s draw ng ind cates acceptance of professional engineering responsib l ity solely for the des gn shown The su tab l ity and use of this des gn for any structure is the respons b l ty of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

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01/07/2014

TC LL	20 0 PSF	REF	R9114- 19138
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007032
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37 0 PSF	SEQN-	338491
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - PBA6 13'11 5 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24' OC

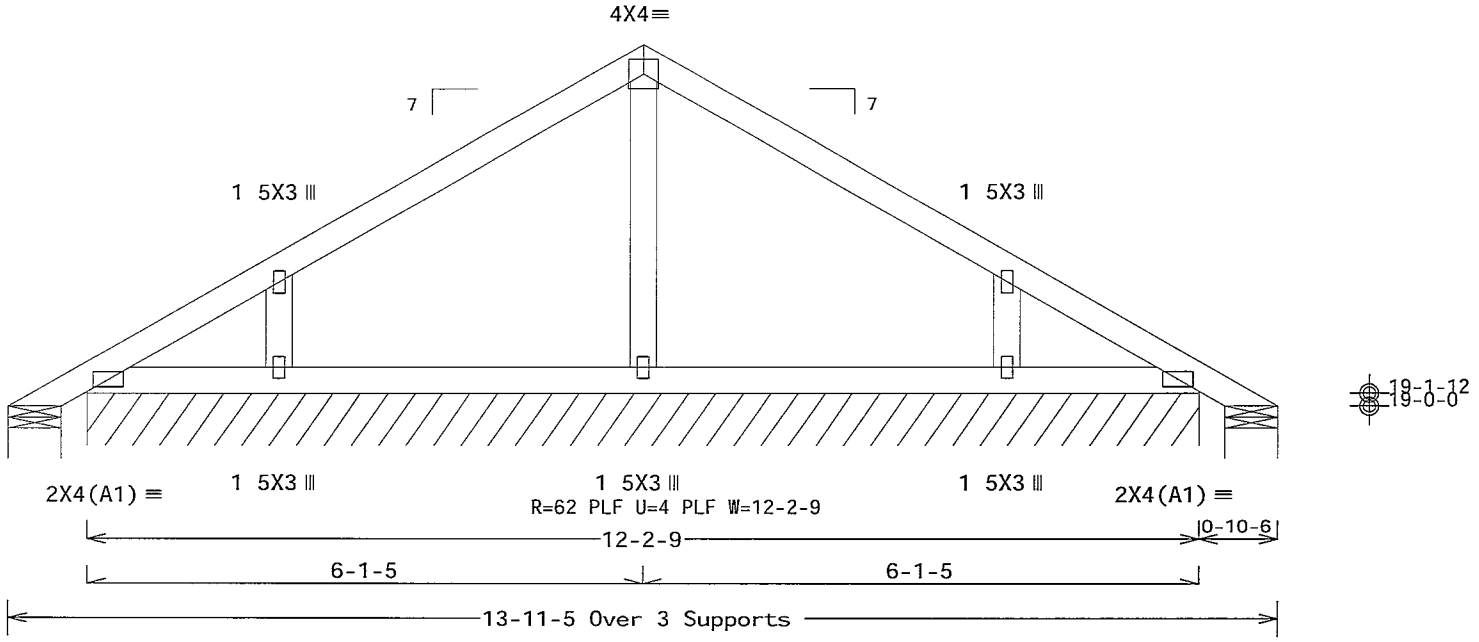
MWFRS loads based on trusses located at least 21 03 ft from roof edge

Refer to drawing PB160100212 for piggyback detail Top chord of supporting truss under piggyback to be braced @ 24" O C , unless otherwise specified

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 6 97
TC- From 56 plf at 6 97 to 56 plf at 13 94
BC- From 4 plf at 0 00 to 4 plf at 13 94

120 mph wind, 21 03 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=24 Rw=35 U=29 W=6 946' (6 946" min)
RL=64/-64

R=24 U=4 W=6 946" (6 946" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0806.13 QTY:6 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any dev at on from this design any failure to build the truss in conformance w th ANSI/TPI 1 or for handl ng sh pping nstallat on bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Detail is unless noted otherwise Refer to drawings 160A-Z for standard plate posit ons A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitabil ty and use of th s design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19139
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006066
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337878
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - PBA7 15 10'1 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

120 mph wind, 20 58 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

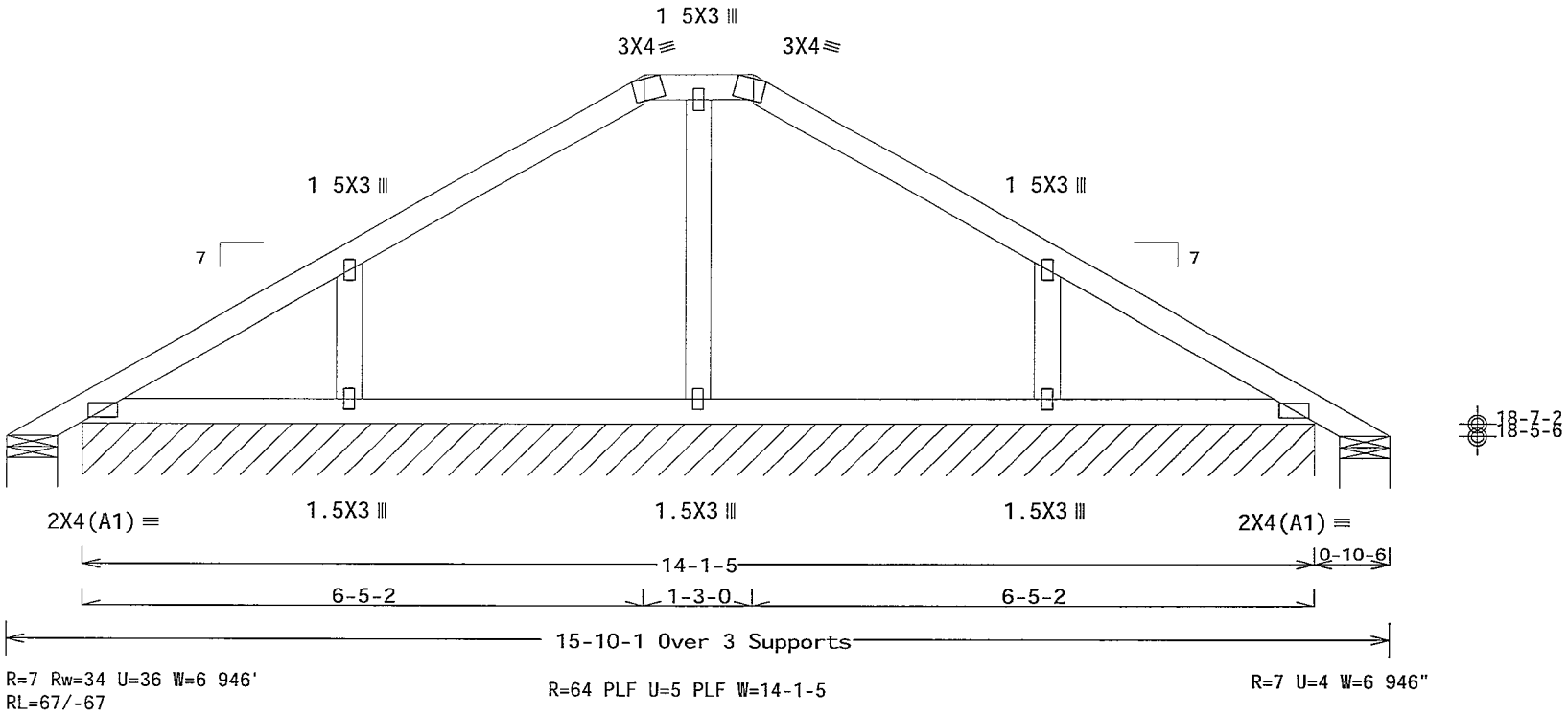
Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 7 29
TC- From 56 plf at 7 29 to 56 plf at 8 54
TC- From 56 plf at 8 54 to 56 plf at 15 84
BC- From 4 plf at 0 00 to 4 plf at 15 84

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 20 58 ft from roof edge



Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Orlando FL, 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 19140
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007033
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	338587
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

01/07/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City, FL - PBA8 11'10"1 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 20 58 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

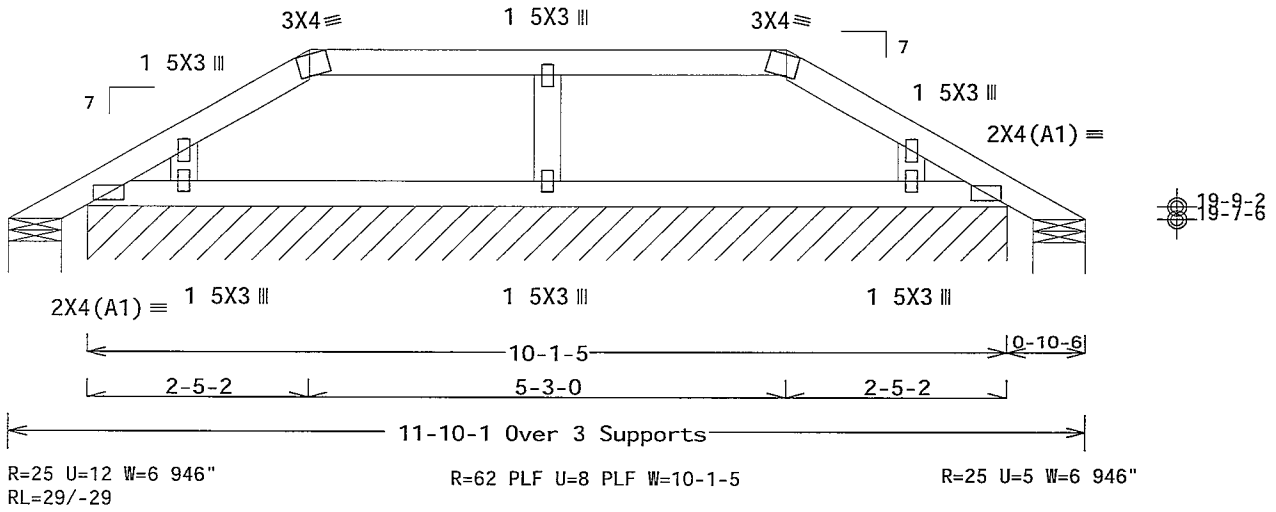
Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 3 29
TC- From 56 plf at 3 29 to 56 plf at 8 54
TC- From 56 plf at 8.54 to 56 plf at 11 84
BC- From 4 plf at 0 00 to 4 plf at 11 84

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24 OC, all BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 20 58 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20 0 PSF	REF	R9114- 19141
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14007034
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	338693
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

01/07/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - PBA9 30 5"11 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24 OC all BC @ 24 OC

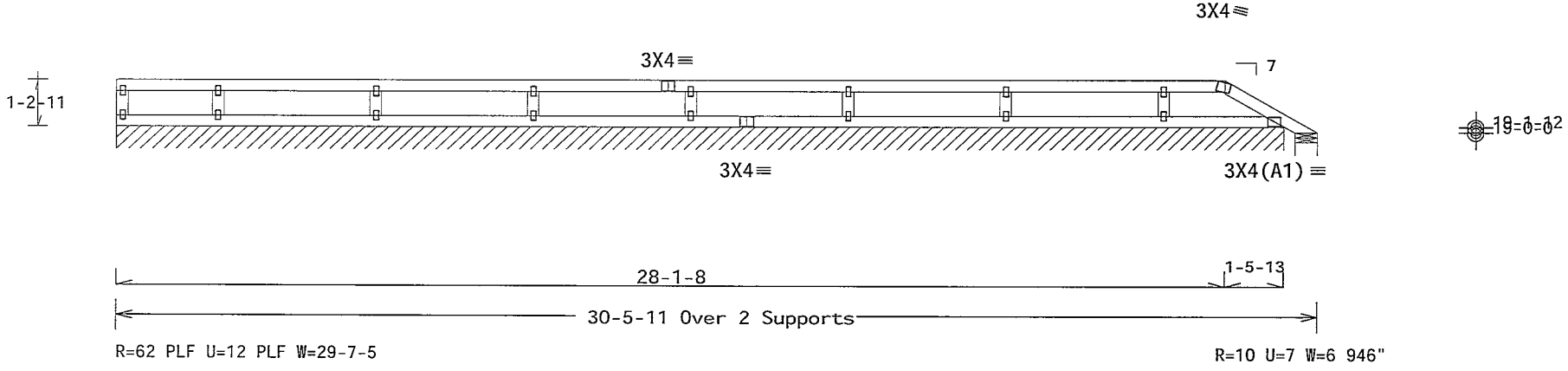
MWFRS loads based on trusses located at least 19 68 ft from roof edge

Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 28 13
TC- From 56 plf at 28 13 to 56 plf at 30 47
BC- From 4 plf at 0 00 to 4 plf at 30 47

120 mph wind, 19 68 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note· All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 [illegible]13 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/07/2014

TC LL	20.0 PSF	REF	R9114- 19142
TC DL	7.0 PSF	DATE	01/07/14
BC DL	10 0 PSF	DRW	HCUSR9114 14007035
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	338490
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - PBB1 8 2 7 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

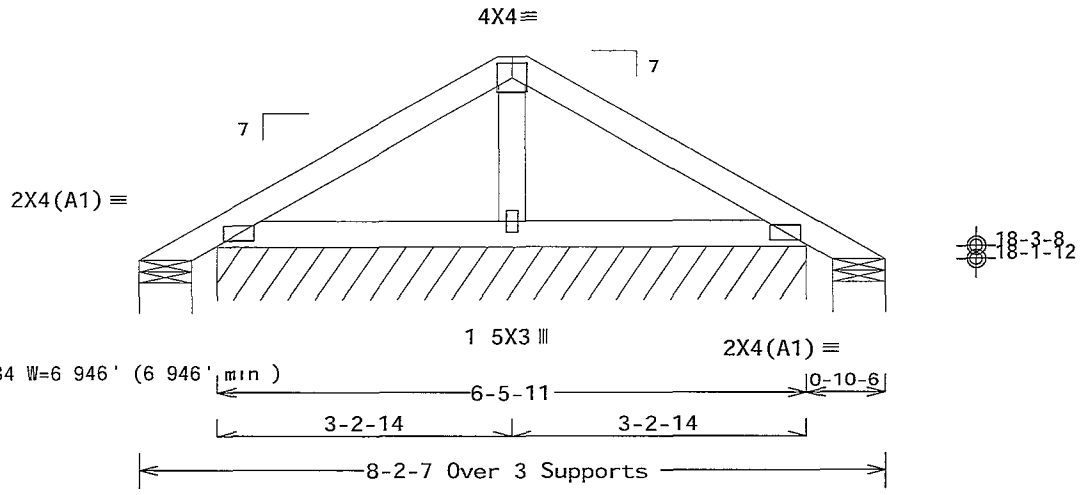
MWFRS loads based on trusses located at least 19 29 ft from roof edge

Refer to drawing PB160100212 for piggyback detail Top chord of supporting truss under piggyback to be braced @ 24' 0 C , unless otherwise specified

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 4 10
TC- From 56 plf at 4 10 to 56 plf at 8 21
BC- From 4 plf at 0 00 to 4 plf at 8 21

120 mph wind, 19 29 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0326 13 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care in fabr cat ng handl ng shipp ng nstall ng and brac ng Refer to and follow the latest edition of BCSI (Bu ld ng Component Safety informat on by TPI and WTCA) for safety practices pr or to performing these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restra nt of web shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shal not be respons ble for any dev ation from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pp ng installation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on th drawing or cover page listing th s draw ng nd cates acceptance of profess onal engineering respons b l ty solely for the des gn shown The su tab lity and use of this design for any structure is the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/07/2014

TC LL	20.0 PSF	REF	R9114- 19143
TC DL	7 0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006057
BC LL	0 0 PSF	HC-ENG	SSB/WPF
TOT.LD	37 0 PSF	SEQN-	337615
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - PBB2 8 2 7 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

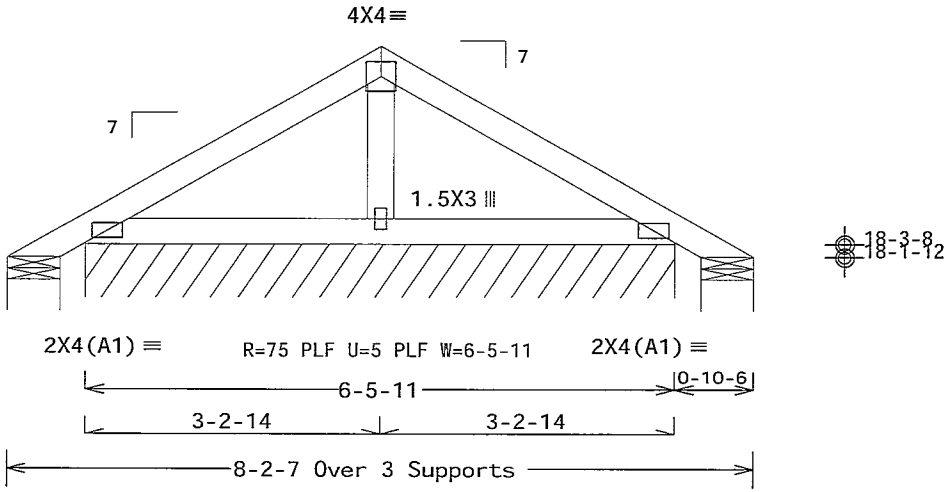
MWFRS loads based on trusses located at least 19 34 ft from roof edge

Refer to drawing PB160100212 for piggyback detail Top chord of supporting truss under piggyback to be braced @ 24' O C , unless otherwise specified

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 4 10
TC- From 56 plf at 4 10 to 56 plf at 8 21
BC- From 4 plf at 0 00 to 4 plf at 8 21

120 mph wind, 19 34 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=2 Rw=25 U=35 W=6 946' (6 946" min)
RL=36/-36

R=2 Rw=7 U=17 W=6 946" (6 946" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:3 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19144
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006054
BC LL	0 0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	337609
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

(13-296A--BRYAN ZECHER /Greenbrier 1087F - Willia -- Lake City FL - PBB3 8 2 7 Common Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24 OC

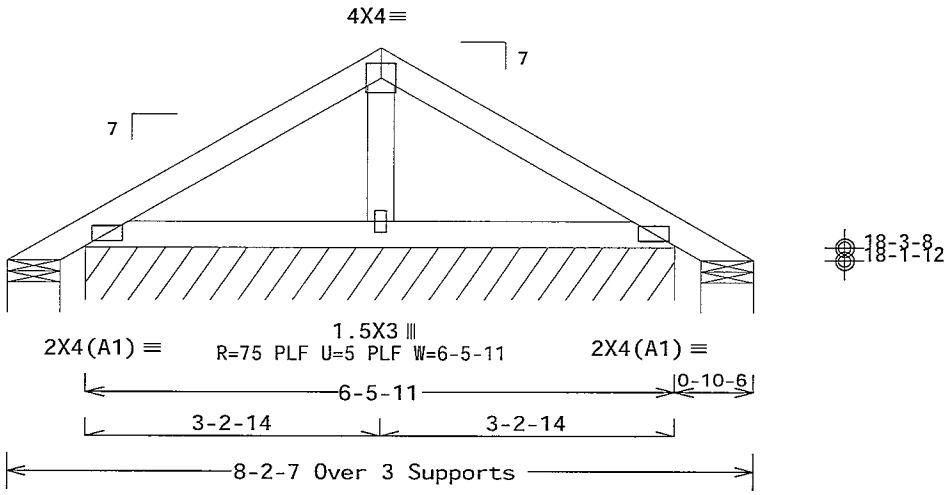
MWFRS loads based on trusses located at least 19 34 ft from roof edge

Refer to drawing PB160100212 for piggyback detail Top chord of supporting truss under piggyback to be braced @ 24" O C , unless otherwise specified

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 4 10
TC- From 56 plf at 4 10 to 56 plf at 8 21
BC- From 4 plf at 0 00 to 4 plf at 8 21

120 mph wind, 19 34 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=2 Rw=25 U=35 W=6 946' (6 946" min)
RL=36/-36

R=2 Rw=7 U=17 W=6 946' (6 946" min)

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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01/07/2014

TC LL	20.0 PSF	REF	R9114- 19145
TC DL	7.0 PSF	DATE	01/06/14
BC DL	10.0 PSF	DRW	HCUSR9114 14006067
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD.	37.0 PSF	SEQN-	337616
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V2T487_Z02

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail s only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum a ternative reinforcement, re-run design with approprate reinforcement type

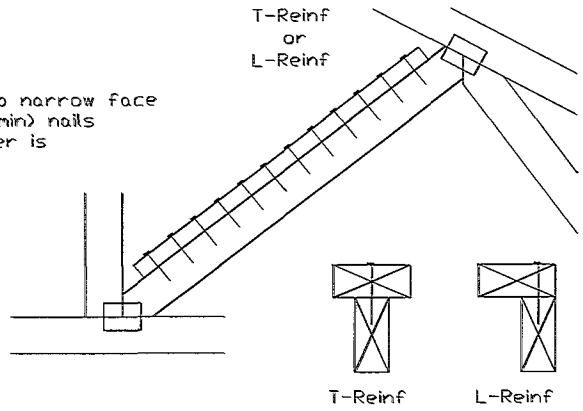
Web Member Size	Specified CLR Restraint	Alternat ve Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement l reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer s sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

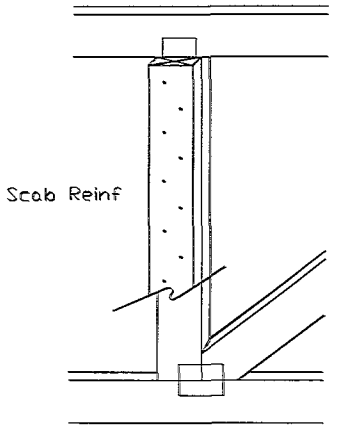
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face Attach with 10d (0.128"x3.0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0128"x30",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length



TWV
Building Components Group Inc.

Earth City MO 63045

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TC LL	PSF	REF CLR Subst
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT LD	PSF	
DUR FAC		
SPACING		

01/07/2014

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Partially Enclosed Exposure C Kzt = 1.00
Or 100 mph Wind Speed 15 Mean Height Enclosed, Exposure D Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" oc	SPF HF	#1 / #2	4 10"	8' 2"	8 6"	9 8"	10 1"	11 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4 7"	7' 9"	8 3"	9 7"	9 11"	11 5"	11' 10"	14 0"	14' 0"	14' 0"	14' 0"
		Stud	4 7"	8' 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14' 0"	14' 0"	14' 0"
		Standard	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4 11"	8' 3"	8' 7"	9 9"	10' 1"	11 7"	12 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4 10"	8' 2"	8 6"	9 8"	10 1"	11 6"	12 0"	14 0"	14' 0"	14' 0"	14' 0"
		#3	4 7"	6' 11"	7 4"	9 3"	9 10"	11 5"	11' 10"	14 0"	14' 0"	14' 0"	14' 0"
		Stud	4 7"	6 11"	7 4"	9 3"	9 10"	11 5"	11 10"	14 0"	14' 0"	14' 0"	14' 0"
		Standard	4 7"	6 0"	6 4"	8 0"	8 6"	10 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" oc	SPF HF	#1 / #2	5 6"	9' 5"	9 9"	11 1"	11 6"	13 2"	13 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5 3"	9 3"	9 9"	10 11"	11 4"	13 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5 3"	9 3"	9 7"	10 11"	11 4"	13' 0"	13 7"	14 0"	14' 0"	14' 0"	14' 0"
		Standard	5 3"	9' 3"	9 7"	10 11"	11 4"	13 0"	13 7"	14 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5 8"	9 5"	9 10"	11 2"	11 7"	13 3"	13 10"	14 0"	14 0"	14' 0"	14' 0"
		#2	5 6"	9 5"	9 9"	11 1"	11' 6"	13 2"	13 9"	14 0"	14' 0"	14' 0"	14' 0"
		#3	5 3"	8' 6"	9 0"	10 11"	11 4"	3' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5 3"	8 6"	9 0"	10 11"	11 4"	3 0"	13 7"	14 0"	14 0"	14' 0"	14 0"
		Standard	5 3"	7' 4"	7 10"	9 9"	10 5"	13 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" oc	SPF HF	#1 / #2	6 1"	10 4"	10 8"	12 2"	12 8"	13' 2"	14 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"
		Stud	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14' 0"	14 0"	14' 0"
		Standard	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14' 0"	14' 0"
	SP DFL	#1	6 2"	10 5"	10 9"	12' 3"	12 9"	14 0"	14 0"	14 0"	14 0"	14 0"	14' 0"
		#2	6 1"	10 4"	10 8"	12 2"	12 8"	14' 0"	14 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5 9"	9 9"	10 5"	12' 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14' 0"
		Stud	5 9"	9 9"	10 5"	12' 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14' 0"
		Standard	5 9"	8 6"	9 0"	11 3"	12 1"	14 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades.

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace space nails at 2" o.c in 18" end zones and 4" o.c between zones.

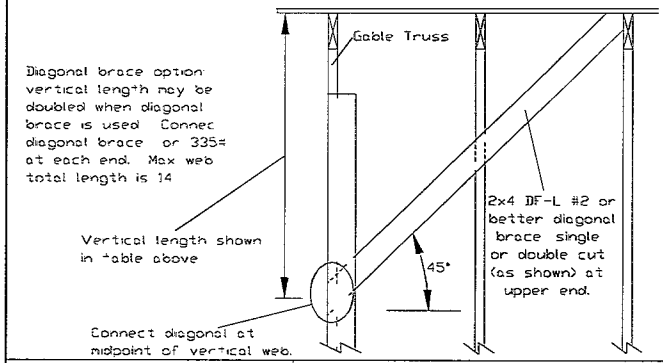
** For (2) "L" braces: space nails at 3" o.c in 18" end zones and 6" o.c. between zones.

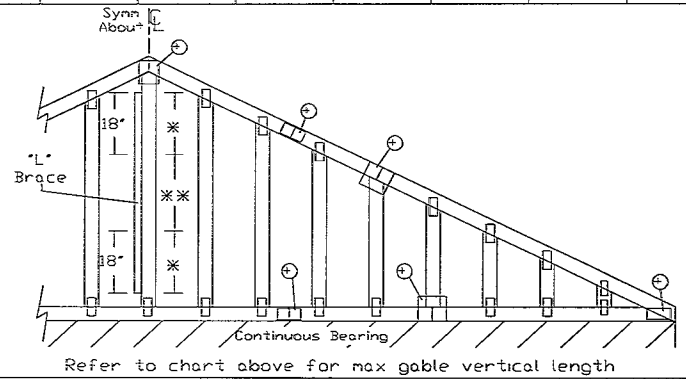
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4 0", but less than 11 6"	2X4
Greater than 11 6"	2.5X4

+ Refer to common truss design for peak, splice and heel plates.

Refer to the Building Designer for conditions not addressed by this detail





Refer to chart above for max gable vertical length

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ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

Building Components Group Inc.
Earth City MO 63045

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

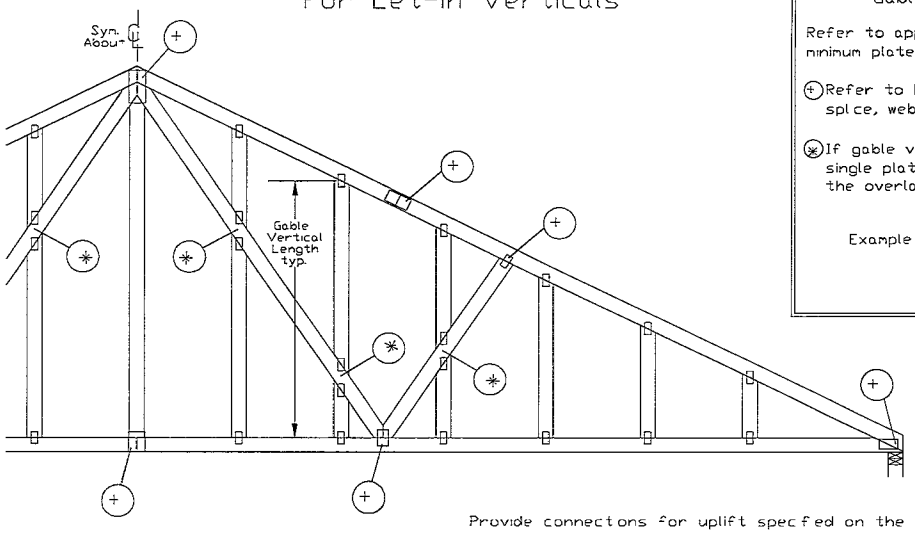
01/07/2014

MAX TOT LD 60 PSF

MAX SPACING 24 0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

Gable Detail
For Let-in Verticals



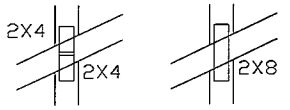
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

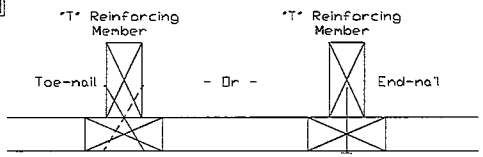
(+) Refer to Engineered truss design for peak splice, web and heel plates

(*) If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web

Example



"T" Reinforcement Attachment Detail

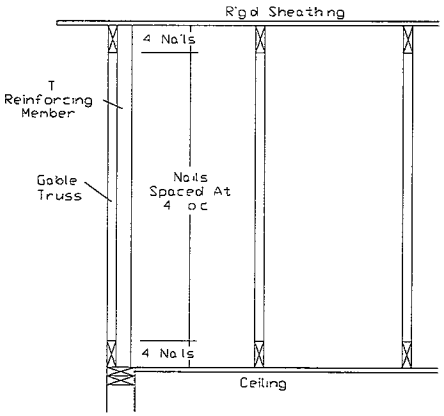


Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3." min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" o.c plus
- (4) toenails in the top and bottom chords.



This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109, A10015020109 A14015020109
A13030020109, A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212 A14015ENC100212 A16015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212, A20015PED100212,
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord

"T" reinforcing member material must match size specie and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24"oc SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8 7" = 11 2"

Building Components Group Inc.

Earth City MO 63045

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

14

MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24 0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

01/07/2014

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber plates, and other information not shown on this detail

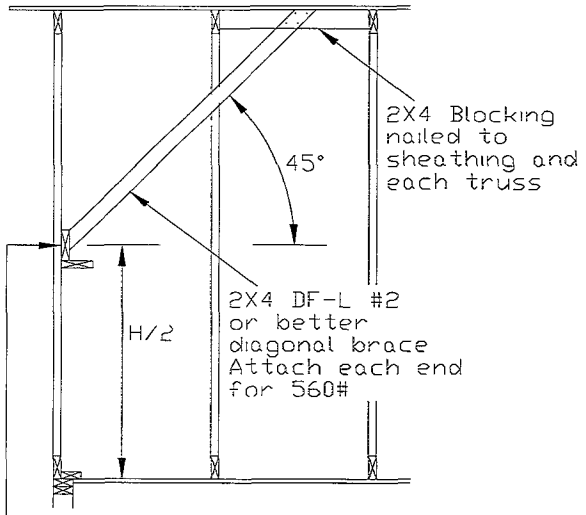
Nails 10d box or gun (.0128"x3",min) nails

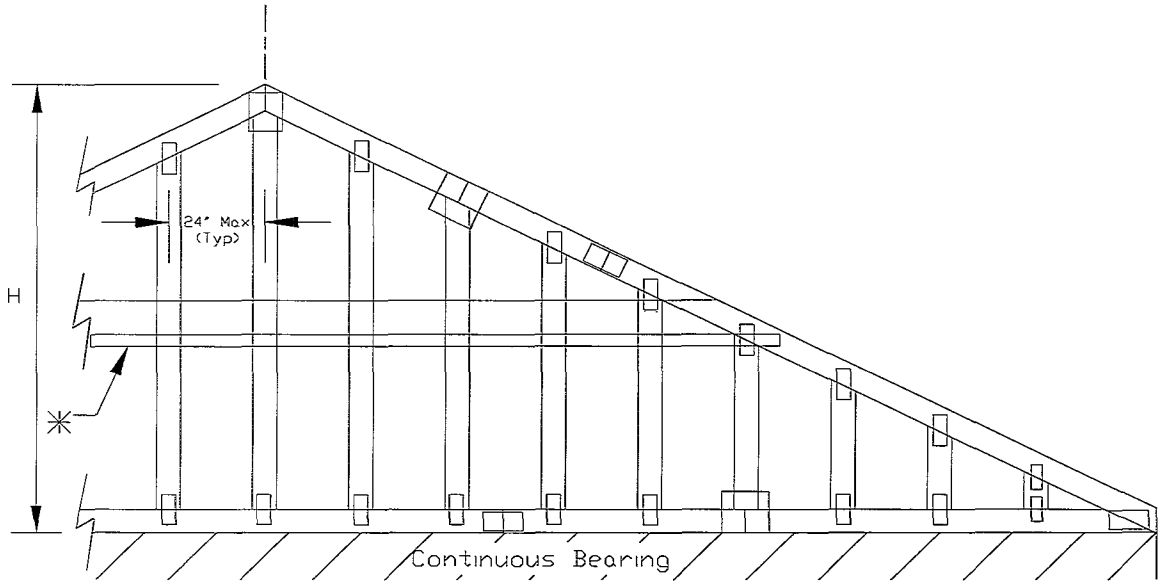
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (.0128" x 3", min) nails @ 6" o.c.





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Earth City MO 63045

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14

MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

01/07/2014

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 Mph Wind Speed 30 Mean Height Partially Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 30' Mean Height Enclosed, Exposure B, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" oc	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 8"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	7' 6"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 10"	8' 2"	9' 3"	9' 8"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
		Standard	4' 3"	5' 7"	5' 11"	7' 5"	7' 11"	10' 0"	10' 9"	11' 8"	12' 5"	14' 0"	14' 0"
16" oc	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	9' 0"	9' 4"	10' 7"	11' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	6' 10"	7' 3"	9' 1"	9' 8"	12' 4"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" oc	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	3' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 11"	9' 11"	10' 3"	11' 8"	12' 2"	13' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	7' 10"	8' 4"	10' 6"	11' 3"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades.

Group A

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c in 18" end zones and 6" o.c. between zones.

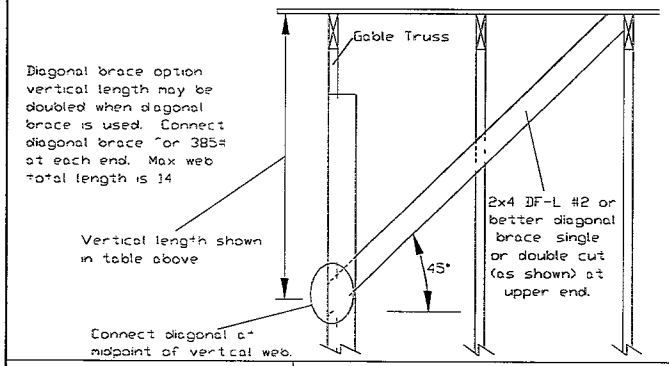
"L" bracing must be a minimum of 80% of web member length.

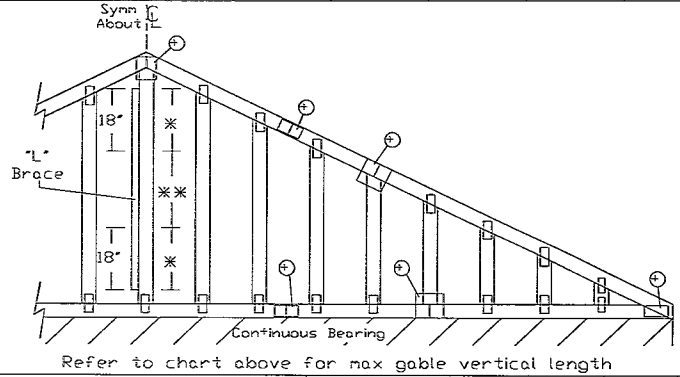
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail





Refer to chart above for max gable vertical length

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14

MAX TOT LD 60 PSF

MAX SPACING 240

REF ASCE7-10-GAB12030

DATE 2/14/12

DRWG A12030ENC100212

01/07/2014

Commentary Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads Providing camber has the following advantages

- Helps to ensure level ceilings and floors after dead loads are applied
- Facilitates drainage to avoid ponding on flat or low slope roofs
- Compensates for different deflection characteristics between adjacent trusses
- Improves appearance of garage door headers and other long spans that can appear to sag
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system The Building Designer shall also specify any dead load live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads

The amount of camber is dependent on the truss type, span, loading, application, etceteras

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration

The following tables are provided as guidelines for limiting deflection and estimating camber Conditions or codes may exist that require exceeding these recommendations or past experience may warrant using more stringent limitations

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0 75" (horizontal)	1 25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1 00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1 5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0 25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0 25 x Deflection from Live Load) + (1 5 x Design Dead Load Deflection)

Note The actual dead load may be considerably less than the design dead load

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14

REF	DEFLEC/CAMB
DATE	8/2/13
DRWG	DEFLCAMB0813

Piggyback Detail - ASCE 7-10 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

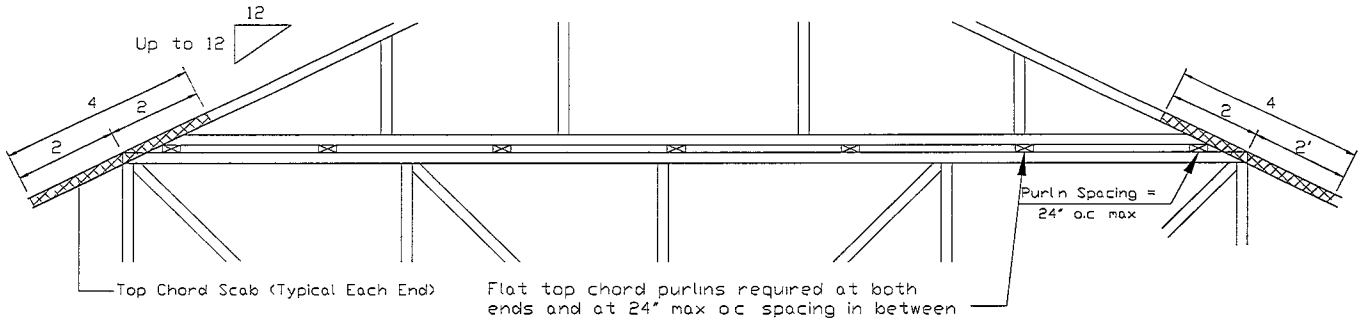
160 mph Wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof Exp C Wind DL= 5.0 psf (min) Kzt=1.0
Or 140 mph wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof Exp D wind DL= 5.0 psf (min) Kzt=1.0

Note Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins and lateral bracing for out of plane loads over gable ends

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola steeple chimney or drag strut loads

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications

Detail A Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

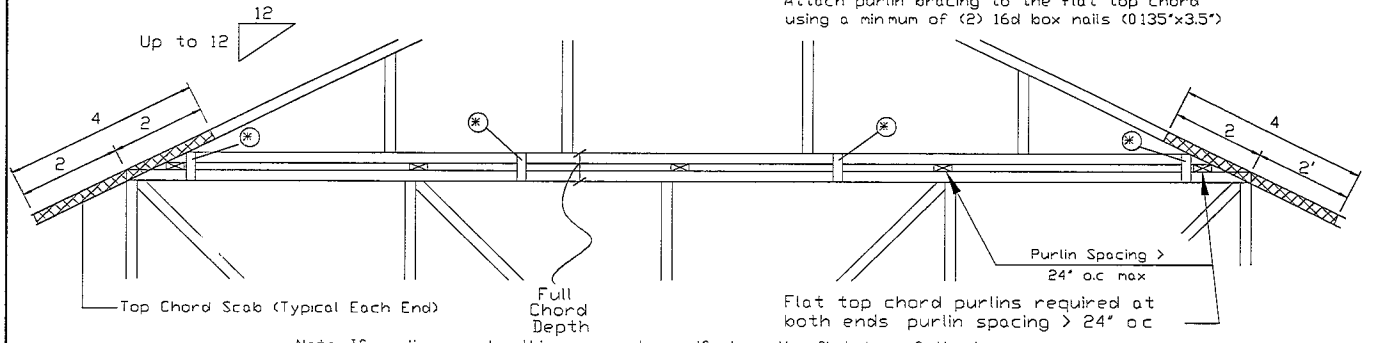
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5")

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails Note Nailing thru holes of wave plate is acceptable

Detail B Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5")



Note If purlins or sheathing are not specified on the flat top of the base truss purlins must be installed at 24" o.c. max and use Detail A

* In addition provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member and 3X10 Trulox plates for 2x6 and larger chord members Attach to each face @ 8" o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord Trulox plates may be staggered 4" o.c. front to back faces

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face) Attach @ 8" o.c. with (8) 6d common (0.113"x2") nails per gusset (4) in cap bottom chord and (4) in base truss top chord Gussets may be staggered 4" o.c. front to back faces

2x4 Vertical Scabs
2x4 SPF #2 full chord depth scabs (each face) Attach @ 8" o.c. with (6) 10d box nails (0.128"x3") per scab (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4" o.c. front to back faces

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8" o.c. Attach teeth to piggyback at time of fabrication Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply Piggyback plates may be staggered 4" o.c. front to back faces

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

14

REF	PIGGYBACK
DATE	2/14/12
DRWG	PB160100212
SPACING	24.0"

01/07/2014

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL

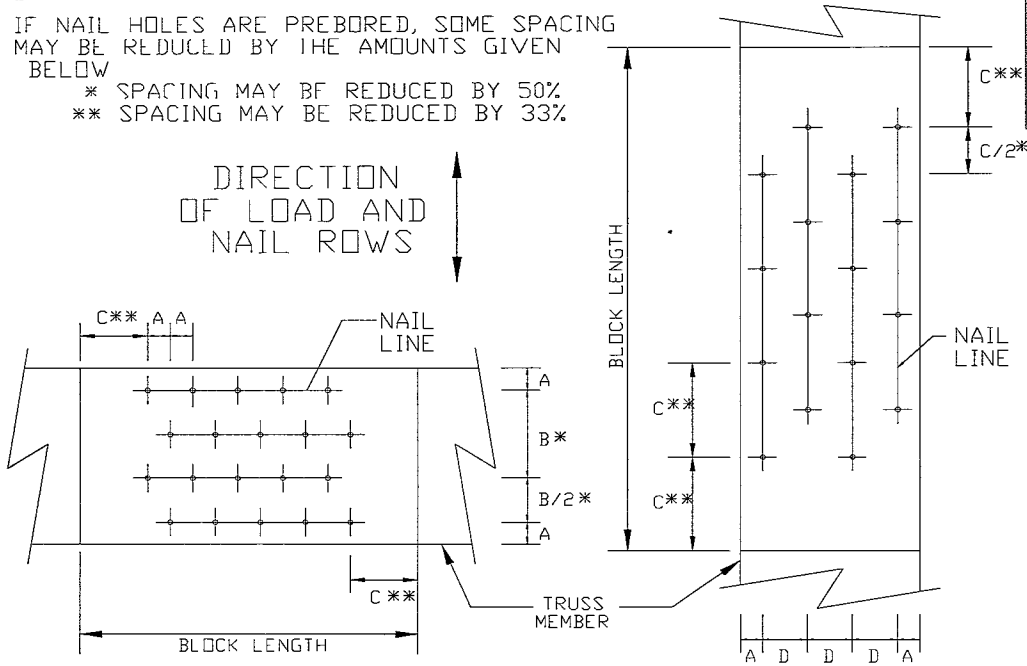
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0113 X 25",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0128"X 325",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0135"X 35",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0131"X 25",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0148"X 325",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0162"X 35",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0120"X 25",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0131"X 25",MIN)	7/8"	1 5/8"	2"	1"
GUN (0120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0131"X 3",MIN)	7/8"	1 5/8"	2"	1"



ITW Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shal provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7 See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR
ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with TPI or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/V/H,S) galv steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2
ITW BCG: www.itwbcg.com, TPI: www.tpinst.com, WTCA: www.sbcindustry.com, ICC: www.iccsafe.org

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REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAILSP0109

01/07/2014