

DATE 10/03/2008

Columbia County Building Permit
This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000027397

APPLICANT PATRICK WILSON PHONE 904-296-1490
ADDRESS 6800 SOUTHPOINT PRKWY # 300 JACKSONVILLE FL 32216
OWNER MARONDA HOMES INC PHONE 904-296-1490
ADDRESS 379 SW TIMBER RIDGE CT LAKE CITY FL 32025
CONTRACTOR THEODORE BROCK PHONE 904-296-1490
LOCATION OF PROPERTY 90 W, L 247, R 252 B, L TIMBER RIDGE DR, 13TH
LOT ON RIGHT
TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 174000.00
HEATED FLOOR AREA 2920.00 TOTAL AREA 3480.00 HEIGHT 25.00 STORIES 2
FOUNDATION CONCRTE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB
LAND USE & ZONING RSF-2 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO. _____

PARCEL ID 10-4S-16-02856-113 SUBDIVISION TIMBERLANDS
LOT 13 BLOCK 1 PHASE _____ UNIT _____ TOTAL ACRES 0.50

000001673 _____ CBC1256382 _____
Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number _____ Applicant/Owner/Contractor _____
PERMIT 08-0598 BK WR Y
Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident _____

COMMENTS: MFE SET PER S/D 104.0', ELEVATION CONFIRMATION REQUIRED AT SLAB

NOC ON FILE

Check # or Cash 938128

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
date/app. by _____ date/app. by _____ date/app. by _____
Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
date/app. by _____ date/app. by _____ date/app. by _____
Framing _____ Rough-in plumbing above slab and below wood floor _____
date/app. by _____ date/app. by _____
Electrical rough-in _____ Heat & Air Duct _____ Peri. beam (Lintel) _____
date/app. by _____ date/app. by _____ date/app. by _____
Permanent power _____ C.O. Final _____ Culvert _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H tie downs, blocking, electricity and plumbing _____ Pool _____
date/app. by _____ date/app. by _____
Reconnection _____ Pump pole _____ Utility Pole _____
date/app. by _____ date/app. by _____ date/app. by _____
M/H Pole _____ Travel Trailer _____ Re-roof _____
date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 870.00 CERTIFICATION FEE \$ 17.40 SURCHARGE FEE \$ 17.40
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ _____
FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 1004.80
INSPECTORS OFFICE Z. Hockley CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

938128

1673

Columbia County Building Permit Application

For Office Use Only Application # 0809-31 Date Received 9/17/08 By LA Permit # 27397
 Zoning Official BK Date 24.09.08 Flood Zone X plat Land Use RES. Low Density Zoning RSF-2
 FEMA Map # N/A Elevation N/A MFE 104.04 River N/A Plans Examiner (u) Date 9/22/08
 Comments Elevation Confirmation Letter Required at Slab
☐ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☒ State Road Info ☐ Parent Parcel #
☐ Dev Permit # ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter
 IMPACT FEES: EMS \$29.88 Fire \$78.63 Corr \$409.16 Road/Code \$1,046.00/211
 School \$1,500.00 = TOTAL \$3,063.67 ck 939009

Septic Permit No. 08-0598

Fax (904) 332-1035

Name Authorized Person Signing Permit Theodore Brock / Patrick Wilson Phone (904) 290-1490Address 6800 Southpoint Pkwy #300 Jacksonville, FL 32216Owners Name Maronda Homes Inc. of Florida Phone (904) 290-1490911 Address 379 SW Timber Ridge Ct Lake City, FL 32025Contractors Name Theodore C. Brock Phone (904) 290-1490Address 6800 Southpoint Pkwy #300 Jacksonville, FL 32216Fee Simple Owner Name & Address N/ABonding Co. Name & Address N/AArchitect/Engineer Name & Address Thomas Ponce 4005 Maronda Way Sanford, FL 32711Mortgage Lenders Name & Address Bank of America 250 Park Ave S #400 Winter Park, FL 32789Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress EnergyProperty ID Number 10-45-10-02850-113 Estimated Cost of Construction \$130,500.00Subdivision Name Timber lands Lot 13 Block 1 Unit Phase Driving Directions Hwy 90, left on 247 South; Right on 252B; left on Timber Ridge Dr.,
13th Lot on Right.Number of Existing Dwellings on Property 0

Construction of Residential Single Family Dwelling Total Acreage 1/2 Lot Size 27,200
 Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 25.0'
 Actual Distance of Structure from Property Lines - Front 50.0' Side 38.0' Side 52.0' Rear 57.0'
 Number of Stories 2 Heated Floor Area 2920 Total Floor Area 3480 Roof Pitch 12/6

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.)

Revised 1-10-08

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

Steve Hagg

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Theodore C. Brock

Contractor's License Number

CBC1250382

Columbia County

Competency Card Number

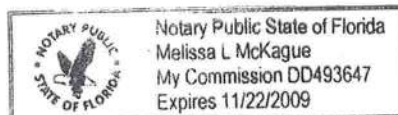
Affirmed under penalty of perjury to by the Contractor and subscribed before me this 22 day of July 2008.

Personally known X or Produced Identification

State of Florida Notary Signature (For the Contractor)

Melissa L. McKague

SEAL:



This Instrument Prepared by and Return to :

Amy Wesp
SOUTHERN TITLE HOLDING
COMPANY, LLC.
3943 BAY MEADOWS ROAD
JACKSONVILLE, Florida 32217

as a necessary incident to the fulfillment of conditions
contained in a title insurance commitment issued by it.

Property Appraisers Parcel L.D. (Folio) Number(s):

R02856-000

Grantee(s) L.D.#(s):

File No: JX0812085

Inst: 200812010775 Date: 6/6/2008 Time: 1:05 PM

Doc Stamp Deed: 6293.00

CC, P. DeWitt Cason, Columbia County Page 1 of 1 B:1151 P:2385

WARRANTY DEED
(CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

\$899,000.00

WITNESSETH: That said grantor, for and in consideration of the sum of ~~\$48,000~~ Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9, PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.

(Whenever used herein the terms "grantor" and "grantee" included all the parties to this instrument, and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation.)

In Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereunto affixed, by its proper officers thereunto duly authorized, the day and year first above written.

Signed, sealed and delivered in our presence:

ATTEST:

Secretary

RML HOLDINGS INC.

Witness Signature: Worth D. Morris

Printed Name: WORTH D. MORRIS

Witness Signature: Jody M. Goble, ACP

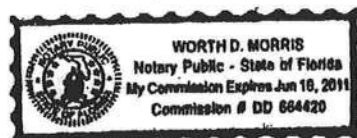
Printed Name: Jody M. Goble

BY: Robert R. Lardizabal
ROBERT R. LARDIZABAL, PRESIDENT

STATE OF FLORIDA
COUNTY OF DUVAL

The foregoing instrument was acknowledged before me this 28th day of May, 2008, by ROBERT R. LARDIZABAL as PRESIDENT of RML HOLDINGS INC., A FLORIDA CORPORATION, on behalf of the corporation. He/she is personally known to me or who has produced driver license(s) as identification.

My Commission Expires:



Printed Name: WORTH D. MORRIS
Notary Public
Serial Number



STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-0598

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.

See
Attached
Site Plan



Notes: _____

Site Plan submitted by: [Signature] _____

Signature

agent
Title

Plan Approved ☒

Not Approved ☐

Date 9/15/08

By [Signature] Columbin County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

08-0598

LEGAL DESCRIPTION:

LOT THIRTEEN (13) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "9", PAGES 28-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

CERTIFIED TO:

1) MARIONA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BENCHMARK NOTE:

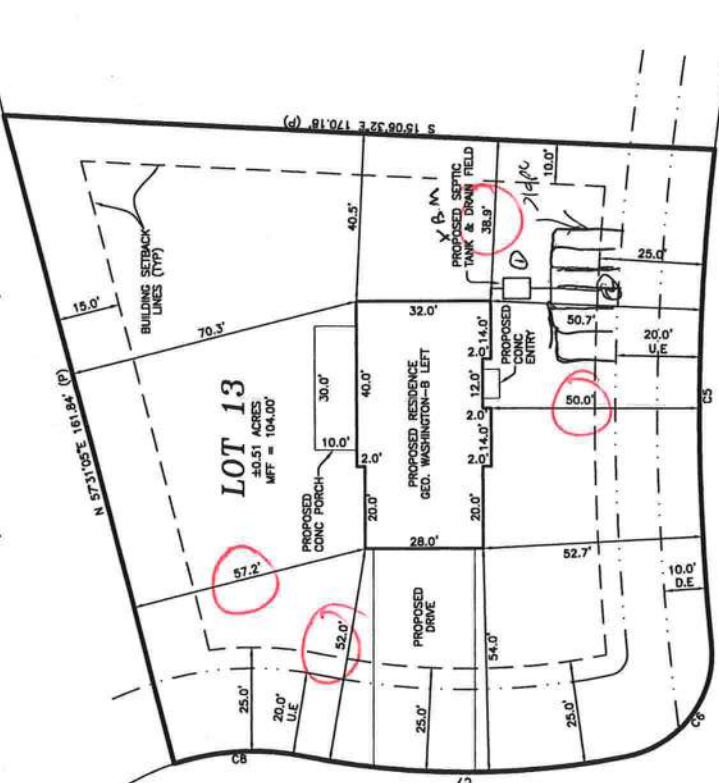
ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.75'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRIT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN.

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE ADJACENT LANDS. ANY UNDETERMINED BOUNDARIES FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DERIVED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE PRIOR TO ANY NEW CONSTRUCTION. THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



- LEGEND:**
- FOUND 1/2" REBAR NO IDENTIFICATION
 - FOUND 1/2" REBAR & CAP L.B. 6884
 - SET 1/2" REBAR & CAP L.B. 6884
 - FOUND 3/4" IRON PIPE
 - FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
 - SET 4" X 4" CONC. MON. P.S.M. 5582
 - X SET NAIL & DISK P.S.M. 5582
 - X FOUND NAIL & DISK P.S.M. 5582
 - FOUND 6" X 6" S.R.D. R/W MON.
 - E CATY RISER
 - TELEPHONE PEDESTAL
 - WOOD POWER POLE
- ABBREVIATIONS:**
- A/C AIR CONDITIONER
 - ASPH ASPHALT
 - CLUT CLUTCH
 - C/V CABLE TELEVISION
 - C/B CONCRETE BLOCK
 - CLF CHAIN LINK FENCE
 - CM CONCRETE MONUMENT
 - CONC CONCRETE
 - ELEC ELECTRIC
 - ELEV ELEVATION
 - FND FOUND
 - FENCE FENCE
 - LI LICENSED SURVEYOR BUSINESS
 - LI LICENSED SURVEYOR
 - MANH MANHOLE
 - O.U. OVERHEAD UTILITIES
 - P.P. PLAT BOOK
 - P.U. PUBLIC UTILITIES EASEMENT
 - TRANS TRANSFORMER
 - TYP TYPICAL
 - WV WATER VALVE

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA SURVEYING BOARD, PUBLISHED IN THE FLORIDA SURVEYING STATUTES, AND CHAPTER 473, FLORIDA ADMINISTRATIVE CODE.

BY: *[Signature]*
JAMES E. BRINKMAN, PSM - P.A. CERT# 5582
DATE: 8/14/08

BRINKMAN SURVEYING & MAPPING INC.
4807 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'	DRAWN BY: ZL
DATE: 8/14/08	CHECKED BY: J.B.
FIELD WORK COMPLETED ON ****	FIELDBOOK **, PAGE **
PREPARED FOR: MARIONA	DRAWING NUMBER 163-08

S.W. TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

CURVE TABLE:

CURVE	RADIUS	TANGENT	LENGTH	DELTA	CHORD	CHORD BEARING
C6(P)	171.14'	18.75'	18.75'	171.14°	171.14'	S 70°33'18" W
C7(P)	25.00'	22.75'	33.92'	84°13'31.9"	33.66'	N 71°16'22" W
C7(P)	200.00'	39.54'	78.07'	22°21'55.5"	77.58'	N 17°47'16" W
C8(P)	60.00'	15.88'	31.04'	29°38'27.7"	30.70'	N 21°25'32" W

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP (FIRM) 12010C0101, THE SUBJECT PROPERTY IS IN FLOOD ZONE "X", WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS DETERMINED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAP (FIRM) 12010C0101, THE SUBJECT PROPERTY IS IN FLOOD ZONE "X", WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS DETERMINED FROM SAID MAP. REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY PLATS THAT MAY BE RECORDED IN THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. THE SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE EASEMENTS OR RESTRICTIONS THAT MAY EFFECT THIS SURVEY. THE SURVEYOR DOES NOT GUARANTEE THE ACCURACY OR PRESENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

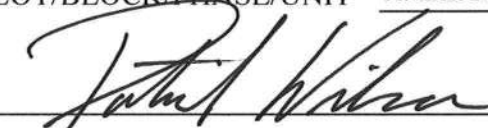
**Columbia County Building Department
Culvert Permit**

**Culvert Permit No.
000001673**

DATE 10/03/2008 PARCEL ID # 10-4S-16-02856-113
APPLICANT PATRICK WILSON PHONE 904-296-1490
ADDRESS 6800 SOUTHPPOINT PRKWY #300 JACKSONVILLE FL 32216
OWNER MARONDA HOMES INC OF FLORIDA PHONE 904-296-1490
ADDRESS 379 SW TIMBER RIDGE CT LAKE CITY FL 32025
CONTRACTOR THEODORE BROCK PHONE 904-296-1490
LOCATION OF PROPERTY 90, L 247 S, R 252 B, L TIMBER RIDGE DR, 13TH LOT ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBERLANDS 13 1

SIGNATURE



INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALLATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



27397

NOTICE OF COMMENCEMENT

Tax Parcel Identification Number 10-45-10-02856-113

County Clerk's Office Stamp or Seal

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

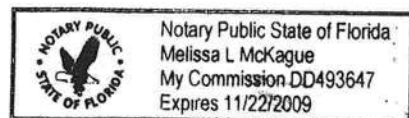
1. Description of property (legal description): 131 Timberlands
a) Street (job) Address: 379 SW Timber Ridge Dr.
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Name and address of fee simple titleholder (if other than owner):
c) Interest in property:
4. Contractor Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Telephone No.: (904) 290-1490 Fax No. (Opt.): (904) 332-0315
5. Surety Information
a) Name and address:
b) Amount of Bond:
c) Telephone No.: Fax No. (Opt.):
6. Lender
a) Name and address:
b) Phone No.:
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Southern Title Holding Co LLC 3943 Bay Meadows Rd Jax FL 32217
b) Telephone No.: (904) 739-2205 Fax No. (Opt.):
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name and address:
b) Telephone No.: Fax No. (Opt.):
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 22 day of July, 2008, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
fact) for Maronda Homes Inc of Florida (name of party on behalf of whom instrument was executed).

Personally Known ☒ OR Produced Identification TypeNotary Signature Melissa L McKague Notary Stamp or Seal:

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
Signature of Natural Person Signing (in line #10 above.)

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

13/1 TM

Project Name: **GEORGE WASHINGTON GAINESVILLE**
Address: **379 SW Timber Ridge Dr.**
City, State: **Lake City, FL 32055**
Owner: **ELECTRIC**
Climate Zone: **North**

Builder: **MARONDA HOMES**
Permitting Office: **COLUMBIA**
Permit Number: **27397**
Jurisdiction Number: **221500**

1. New construction or existing	New	___
2. Single family or multi-family	Single family	___
3. Number of units, if multi-family	1	___
4. Number of Bedrooms	5	___
5. Is this a worst case?	Yes	___
6. Conditioned floor area (ft²)	2920 ft²	___
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		___
a. U-factor:	Description Area	___
(or Single or Double DEFAULT)	7a(Sngle Default) 313.0 ft²	___
b. SHGC:		___
(or Clear or Tint DEFAULT)	7b. (Clear) 313.0 ft²	___
8. Floor types		___
a. Slab-On-Grade Edge Insulation	R=0.0, 196.0(p) ft	___
b. Raised Wood, Adjacent	R=13.0, 640.0ft²	___
c. N/A		___
9. Wall types		___
a. Concrete, Int Insul, Exterior	R=4.1, 736.0 ft²	___
b. Frame, Wood, Exterior	R=13.0, 1108.0 ft²	___
c. Frame, Wood, Adjacent	R=13.0, 206.0 ft²	___
d. N/A		___
e. N/A		___
10. Ceiling types		___
a. Under Attic	R=19.0, 1944.0 ft²	___
b. N/A		___
c. N/A		___
11. Ducts		___
a. Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 150.0 ft	___
b. Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 200.0 ft	___
12. Cooling systems		___
a. Central Unit	Cap: 40.5 kBtu/hr	___
	SEER: 13.00	___
b. Central Unit	Cap: 22.6 kBtu/hr	___
	SEER: 13.00	___
c. N/A		___
13. Heating systems		___
a. Electric Heat Pump	Cap: 40.5 kBtu/hr	___
	HSPF: 8.10	___
b. Electric Heat Pump	Cap: 22.6 kBtu/hr	___
	HSPF: 8.20	___
c. N/A		___
14. Hot water systems		___
a. Electric Resistance	Cap: 50.0 gallons	___
	EF: 0.90	___
b. N/A		___
c. Conservation credits		___
(HR-Heat recovery, Solar		___
DHP-Dedicated heat pump)		___
15. HVAC credits	PT, ___	___
(CF-Ceiling fan, CV-Cross ventilation,		___
HF-Whole house fan,		___
PT-Programmable Thermostat,		___
MZ-C-Multizone cooling,		___
MZ-H-Multizone heating)		___

Glass/Floor Area: 0.11

Total as-built points: 36781

Total base points: 37723

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Walter Campbell

DATE: 07/24/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa McRague

DATE: 07/24/08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	2920.0	18.59	9771.0	1.Single, Clear	E	1.0	8.0	40.0	47.92	0.99	1894.0
				2.Single, Clear	E	1.0	6.0	30.0	47.92	0.97	1391.0
				3.Single, Clear	W	1.0	6.0	30.0	43.84	0.97	1273.0
				4.Single, Clear	W	1.0	6.0	16.0	43.84	0.97	679.0
				5.Single, Clear	W	1.0	6.0	16.0	43.84	0.97	679.0
				6.Single, Clear	N	1.0	6.0	30.0	21.73	0.97	635.0
				7.Single, Clear	W	1.0	6.0	15.0	43.84	0.97	636.0
				8.Single, Clear	W	1.0	6.0	15.0	43.84	0.97	636.0
				9.Single, Clear	W	1.0	5.0	6.0	43.84	0.95	249.0
				10.Single, Clear	W	1.0	6.0	20.0	43.84	0.97	849.0
				11.Single, Clear	E	1.0	6.0	30.0	47.92	0.97	1391.0
				12.Single, Clear	E	1.0	6.0	30.0	47.92	0.97	1391.0
				13.Single, Clear	NW	1.0	3.0	5.0	29.42	0.88	128.0
				14.Single, Clear	NW	1.0	6.0	30.0	29.42	0.97	858.0
				As-Built Total:				313.0	12689.0		
WALL TYPES				Area X BSPM = Points		Type		R-Value	Area X SPM = Points		
Adjacent	206.0	0.70	144.2	1. Concrete, Int Insul, Exterior		4.1		736.0	1.13		835.4
Exterior	1844.0	1.70	3134.8	2. Frame, Wood, Exterior		13.0		1108.0	1.50		1662.0
				3. Frame, Wood, Adjacent		13.0		206.0	0.60		123.6
Base Total:	2050.0		3279.0	As-Built Total:				2050.0	2621.0		
DOOR TYPES				Area X BSPM = Points		Type		Area X SPM = Points			
Adjacent	18.0	2.40	43.2	1.Exterior Insulated				20.0	4.10		82.0
Exterior	20.0	6.10	122.0	2.Adjacent Wood				18.0	2.40		43.2
Base Total:	38.0		165.2	As-Built Total:				38.0	125.2		
CEILING TYPES				Area X BSPM = Points		Type		R-Value	Area X SPM X SCM = Points		
Under Attic	1816.0	1.73	3141.7	1. Under Attic		19.0		1944.0	2.34 X 1.00		4549.0
Base Total:	1816.0		3141.7	As-Built Total:				1944.0	4549.0		
FLOOR TYPES				Area X BSPM = Points		Type		R-Value	Area X SPM = Points		
Slab	196.0(p)	-37.0	-7252.0	1. Slab-On-Grade Edge Insulation		0.0		196.0(p)	-41.20		-8075.2
Raised	640.0	-3.99	-2553.6	2. Raised Wood, Adjacent		13.0		640.0	0.63		400.0
Base Total:			-9805.6	As-Built Total:				836.0	-7675.2		

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT			
INFILTRATION Area X BSPM = Points				Area X SPM = Points			
2920.0 10.21 29813.2				2920.0 10.21 29813.2			
Summer Base Points: 36364.5				Summer As-Built Points: 42122.1			
Total Summer Points	X	System Multiplier	= Cooling Points	Total Component (System - Points)	X	Cap Ratio (DM x DSM x AHU)	X Duct Multiplier X System Multiplier X Credit Multiplier = Cooling Points
36364.5	0.3250	11818.5		(sys 1: Central Unit 40500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS)			
				42122	0.64	(1.08 x 1.147 x 0.86)	0.260 0.950 7157.9
36364.5	0.3250	11818.5		(sys 2: Central Unit 22600btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS)			
				42122	0.36	(1.08 x 1.147 x 0.86)	0.260 0.950 3994.3
36364.5	0.3250	11818.5		42122.1	1.00	1.072	0.260 0.950 11152.2

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2920.0	20.17	10601.0	1.Single, Clear	E	1.0	8.0	40.0	26.41	1.01	1066.0
				2.Single, Clear	E	1.0	6.0	30.0	26.41	1.02	805.0
				3.Single, Clear	W	1.0	6.0	30.0	28.84	1.01	872.0
				4.Single, Clear	W	1.0	6.0	16.0	28.84	1.01	465.0
				5.Single, Clear	W	1.0	6.0	16.0	28.84	1.01	465.0
				6.Single, Clear	N	1.0	6.0	30.0	33.22	1.00	997.0
				7.Single, Clear	W	1.0	6.0	15.0	28.84	1.01	436.0
				8.Single, Clear	W	1.0	6.0	15.0	28.84	1.01	436.0
				9.Single, Clear	W	1.0	5.0	6.0	28.84	1.01	175.0
				10.Single, Clear	W	1.0	6.0	20.0	28.84	1.01	581.0
				11.Single, Clear	E	1.0	6.0	30.0	26.41	1.02	805.0
				12.Single, Clear	E	1.0	6.0	30.0	26.41	1.02	805.0
				13.Single, Clear	NW	1.0	3.0	5.0	32.93	1.01	165.0
				14.Single, Clear	NW	1.0	6.0	30.0	32.93	1.00	987.0
				As-Built Total:				313.0	9060.0		
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	206.0	3.60	741.6	1. Concrete, Int Insul, Exterior	4.1		736.0	6.42	4725.1		
Exterior	1844.0	3.70	6822.8	2. Frame, Wood, Exterior	13.0		1108.0	3.40	3767.2		
				3. Frame, Wood, Adjacent	13.0		206.0	3.30	679.8		
Base Total:				As-Built Total:		2050.0		9172.1			
DOOR TYPES Area X BWPM = Points				Type			Area X WPM = Points				
Adjacent	18.0	11.50	207.0	1.Exterior Insulated			20.0	8.40	168.0		
Exterior	20.0	12.30	246.0	2.Adjacent Wood			18.0	11.50	207.0		
Base Total:				As-Built Total:		38.0		375.0			
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1816.0	2.05	3722.8	1. Under Attic	19.0		1944.0	2.70 X 1.00	5248.8		
Base Total:				As-Built Total:		1944.0		5248.8			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	196.0(p)	8.9	1744.4	1. Slab-On-Grade Edge Insulation	0.0		196.0(p)	18.80	3684.8		
Raised	640.0	0.96	614.4	2. Raised Wood, Adjacent	13.0		640.0	3.25	2080.0		
Base Total:				As-Built Total:		836.0		5764.8			

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
INFILTRATION Area X BWPM = Points				Area X WPM = Points					
2920.0 -0.59 -1722.8				2920.0 -0.59 -1722.8					
Winter Base Points: 22977.2				Winter As-Built Points: 27897.9					
Total Winter Points	X	System Multiplier	= Heating Points	Total Component (System - Points)	X	Cap Ratio	X	Duct Multiplier (DM x DSM x AHU)	X System Multiplier X Credit Multiplier = Heating Points
				(sys 1: Electric Heat Pump 40500 btuh ,EFF(8.1) Ducts:Unc(S),Con(R),Int(AH),R6.0 27897.9 0.642 (1.060 x 1.169 x 0.88)0.421 0.950 7840.0 (sys 2: Electric Heat Pump 22600 btuh ,EFF(8.2) Ducts:Unc(S),Con(R),Int(AH),R6.0 27897.9 0.358 (1.060 x 1.169 x 0.88)0.416 0.950 4321.6					
22977.2		0.5540	12729.4	27897.9	1.00	1.095	0.419	0.950	12161.2

Residential Whole Building Performance Method A - Details

PERMIT #:

CODE COMPLIANCE STATUS							
BASE				AS-BUILT			
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	
11818		12729		13175		37723	
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	
11152		12161		13468		36781	

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	✓
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.2

The higher the score, the more efficient the home.

ELECTRIC, , , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 40.5 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	5	b. Central Unit	Cap: 22.6 kBtu/hr
5. Is this a worst case?	Yes		SEER: 13.00
6. Conditioned floor area (ft ²)	2920 ft ²	c. N/A	
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAULT)	7a.(Single Default) 313.0 ft ²	a. Electric Heat Pump	Cap: 40.5 kBtu/hr
b. SHGC:			HSPF: 8.10
(or Clear or Tint DEFAULT)	7b. (Clear) 313.0 ft ²	b. Electric Heat Pump	Cap: 22.6 kBtu/hr
8. Floor types			HSPF: 8.20
a. Slab-On-Grade Edge Insulation	R=0.0, 196.0(p) ft	c. N/A	
b. Raised Wood, Adjacent	R=13.0, 640.0ft ²		
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Concrete, Int Insul, Exterior	R=4.1, 736.0 ft ²		EF: 0.90
b. Frame, Wood, Exterior	R=13.0, 1108.0 ft ²	b. N/A	
c. Frame, Wood, Adjacent	R=13.0, 206.0 ft ²		
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar	
10. Ceiling types		DHP-Dedicated heat pump)	
a. Under Attic	R=19.0, 1944.0 ft ²	15. HVAC credits	PT,
b. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
c. N/A		HF-Whole house fan,	
11. Ducts		PT-Programmable Thermostat,	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 150.0 ft	MZ-C-Multizone cooling,	
b. Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 200.0 ft	MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Melinda Morale

Date: 07/24/08

Address of New Home: 379 SW Timber Ridge Dr City/FL Zip: Lake City, FL 32055



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5.2)



Project Summary

Entire House

MARONDA HOMES

Job: GEORGE WASHINGTON...
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: GEORGE WASHINGTON 1ST FLOOR

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	18282 Btuh
Ducts	1741 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	20023 Btuh

Sensible Cooling Equipment Load Sizing

Structure	12214 Btuh
Ducts	3683 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	15420 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

Latent Cooling Equipment Load Sizing

Structure	2941 Btuh
Ducts	815 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	3756 Btuh

	Heating	Cooling
Area (ft²)	1256	1256
Volume (ft³)	9950	9950
Air changes/hour	0.45	0.23
Equiv. AVF (cfm)	75	38

Equipment total load	19176 Btuh
Req. total capacity at 0.76 SHR	1.7 ton

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H324AKA

Efficiency	8.2 HSPF
Heating input	
Heating output	0 Btuh @ 47°F
Temperature rise	0 °F
Actual air flow	0 cfm
Air flow factor	0.000 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Cooling Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Cond	N4H324AKA
Coil	FSM4X2400A
Efficiency	13 SEER
Sensible cooling	17176 Btuh
Latent cooling	5424 Btuh
Total cooling	22600 Btuh
Actual air flow	595 cfm
Air flow factor	0.037 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.81

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



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Right-Suite Residential 6.0.90 RSR21115

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2008-Jul-23 10:47:07

Page 1



Building Analysis Entire House MARONDA HOMES

Job: GEORGE WASHINGTON...
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: GEORGE WASHINGTON 1ST FLOOR

Design Conditions

Location:

Gainesville, FL, US
Elevation: 151 ft
Latitude: 30°N

Outdoor:

Dry bulb (°F)
Daily range (°F)
Wet bulb (°F)
Wind speed (mph)

Heating

33
-
-
15.0

Cooling

92
19 (M)
77
7.5

Indoor:

Indoor temperature (°F)
Design TD (°F)
Relative humidity (%)
Moisture difference (gr/lb)

Heating

70
37
30
10.6

Cooling

75
17
50
52.0

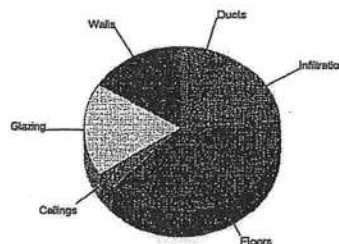
Infiltration:

Method
Construction quality
Fireplaces

Simplified
Average
0

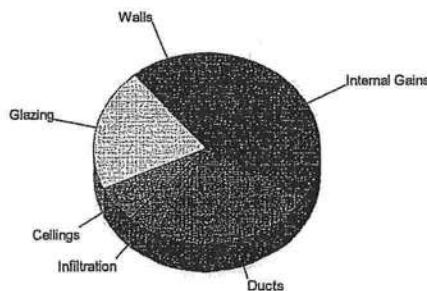
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.3	3201	16.0
Glazing	47.0	3665	18.3
Doors	0.0	0	0.0
Ceilings	1.8	707	3.5
Floors	12.9	7688	38.4
Infiltration	3.7	3021	15.1
Ducts		1741	8.7
Piping		0	0.0
Humidification		0	0.0
Ventilation		0	0.0
Adjustments		0	0.0
Total		20023	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	1731	10.9
Glazing	42.7	3327	20.9
Doors	0.0	0	0.0
Ceilings	2.6	1006	6.3
Floors	0.0	0	0.0
Infiltration	0.9	709	4.5
Ducts		3683	23.2
Ventilation		0	0.0
Internal gains		5440	34.2
Blower		0	0.0
Adjustments		0	0.0
Total		15897	100.0



Overall U-value = 0.229 Btuh/ft²-°F

Data entries checked.



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Right-Suite Residential 6.0.90 RSR21115

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2008-Jul-23 10:47:07

Page 1



Project Summary

Entire House

MARONDA HOMES

Job: GEORGE WASHINGTON...
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: GEORGE WASHINGTON 2ND FLOOR

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	26460 Btuh
Ducts	3964 Btuh
Central vent (100 cfm)	4048 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	34472 Btuh

Sensible Cooling Equipment Load Sizing

Structure	20442 Btuh
Ducts	8518 Btuh
Central vent (100 cfm)	1860 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	29895 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft ²)	1665	1665
Volume (ft ³)	14776	14776
Air changes/hour	0.38	0.20
Equiv. AVF (cfm)	94	49

Latent Cooling Equipment Load Sizing

Structure	3332 Btuh
Ducts	1956 Btuh
Central vent (100 cfm)	3516 Btuh
Equipment latent load	8803 Btuh
Equipment total load	38699 Btuh
Req. total capacity at 0.76 SHR	3.3 ton

Heating Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Model	N4H342AKA
Efficiency	8.1 HSPF
Heating input	0 Btuh @ 47°F
Heating output	0 °F
Temperature rise	0 cfm
Actual air flow	0.000 cfm/Btuh
Air flow factor	0.60 in H2O
Static pressure	
Space thermostat	

Cooling Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Cond	N4H342AKA
Coil	FSM4X4200A
Efficiency	13 SEER
Sensible cooling	30780 Btuh
Latent cooling	9720 Btuh
Total cooling	40500 Btuh
Actual air flow	1270 cfm
Air flow factor	0.044 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.78

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



wrightsoft

Right-Suite Residential 6.0.90 RSR21115

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Page 1



Building Analysis Entire House MARONDA HOMES

Job: GEORGE WASHINGTON...
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: GEORGE WASHINGTON 2ND FLOOR

Design Conditions

Location:

Gainesville, FL, US
Elevation: 151. ft
Latitude: 30°N

Outdoor:

Dry bulb (°F)
Daily range (°F)
Wet bulb (°F)
Wind speed (mph)

Heating

33
-
15.0

Cooling

92
19 (M)
77
7.5

Indoor:

Indoor temperature (°F)
Design TD (°F)
Relative humidity (%)
Moisture difference (gr/lb)

Heating

70
37
30
10.6

Cooling

75
17
50
52.0

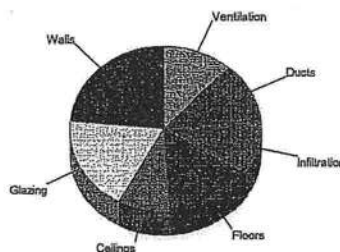
Infiltration:

Method
Construction quality
Fireplaces

Simplified
Average
0

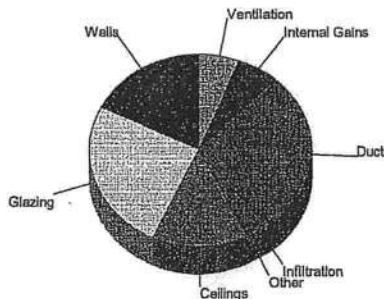
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	3.8	8111	23.5
Glazing	47.0	6391	18.5
Doors	0.0	0	0.0
Ceilings	1.8	3066	8.9
Floors	8.2	5104	14.8
Infiltration	1.7	3788	11.0
Ducts		3964	11.5
Piping		0	0.0
Humidification		0	0.0
Ventilation		4048	11.7
Adjustments		0	0.0
Total		34472	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.5	5422	17.6
Glazing	57.7	7842	25.4
Doors	0.0	0	0.0
Ceilings	2.6	4363	14.2
Floors	0.1	60	0.2
Infiltration	0.4	916	3.0
Ducts		8518	27.6
Ventilation		1860	6.0
Internal gains		1840	6.0
Blower		0	0.0
Adjustments		0	0.0
Total		30820	100.0



Overall U-value = 0.134 Btuh/ft²-°F

Data entries checked.



wrightsoft

Right-Suite Residential 6.0.90 RSR21115

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Page 1

Duct System Summary

Entire House

MARONDA HOMES

Job: GEORGE WASHINGTON ...
 Date:
 By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: GEORGE WASHINGTON 1ST FLOOR

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.48 / 0.12 in H2O	0.48 / 0.12 in H2O
Lowest friction rate	1.935 in/100ft	1.935 in/100ft
Actual air flow	595 cfm	595 cfm
Total effective length (TEL)	31 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
FAMILY ROOM	h 5917	150	150	1.935	7	x0	VIFx	25.0	0.0	st1
KITCHEN	c 3868	135	135	2.419	7	x0	VIFx	20.0	0.0	ST2
BATH	c 825	25	25	9.677	4	x0	VIFx	5.0	0.0	ST3
DINING	c 3908	135	135	4.839	7	x0	VIFx	10.0	0.0	ST4
LIVING	c 3772	150	150	1.935	7	x0	VIFx	25.0	0.0	ST5

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	150	150	1.935	615	7	0 x 0	VinlFlx	
ST2	Peak AVF	135	135	2.419	534	7	0 x 0	VinlFlx	
ST3	Peak AVF	25	25	9.677	349	4	0 x 0	VinlFlx	
ST4	Peak AVF	135	135	4.839	539	7	0 x 0	VinlFlx	
ST5	Peak AVF	150	150	1.935	520	7	0 x 0	VinlFlx	

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	595	595	6.0	1.935	557	16	x0 0		VIFx	

Bold/italic values have been manually overridden

Duct System Summary

Entire House

MARONDA HOMES

Job: GEORGE WASHINGTON ...
 Date:
 By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: GEORGE WASHINGTON 2ND FLOOR

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.43 / 0.17 in H2O	0.43 / 0.17 in H2O
Lowest friction rate	2.857 in/100ft	2.857 in/100ft
Actual air flow	1270 cfm	1270 cfm
Total effective length (TEL)	21 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER TOILET	h 273 8	25	25	4.286	4	x0	VIFx	10.0	0.0	st3
HALLWAY	h 449 4	105	105	4.286	6	x0	VIFx	10.0	0.0	ST2
BDR #4	c 297 9	135	135	4.286	6	x0	VIFx	10.0	0.0	ST2
BDR #3	c 258 3	135	135	4.286	6	x0	VIFx	10.0	0.0	ST2
BATH	c 142 8	31	31	2.857	4	x0	VIFx	15.0	0.0	st1
BDR #2	h 271 7	115	115	8.571	6	x0	VIFx	5.0	0.0	st1
LOFT	c 397 5	170	170	8.571	7	x0	VIFx	5.0	0.0	st1
BDR #5	c 355 3	175	175	4.286	7	x0	VIFx	10.0	0.0	ST2
MASTER BATH	h 187 4	34	34	8.571	6	x0	VIFx	5.0	0.0	st3
MASTER BDR-A	c 337 3	170	170	4.286	7	x0	VIFx	10.0	0.0	st3
MASTER BDR	c 337 3	175	175	4.286	7	x0	VIFx	10.0	0.0	st3

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	720	720	2.857	557	14	0 x 0	VinlFlx	st1
ST2	Peak AVF	550	550	4.286	684	12	0 x 0	VinlFlx	
st3	Peak AVF	404	404	4.286	568	12	0 x 0	VinlFlx	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1270	1270	6.0	2.857	582	18	x0 0		VIFx	

27397

FIELD DENSITY WORKSHEET

CLIENT MARIONDA HOMES

DATE 7 Oct 08

PROJECT NAME TIMBER LAIRS LAKE CITY

PROJECT NO. _____

EARTH CONTRACTOR Lot # 13 JOB # 97mo1301

PERMIT NO. _____

COMPACTION REQUIREMENT (%) 95% ☐ Standard Proctor ☒ Modified Proctor

TESTED BY JHL

COMPACTION REQUIREMENT (%) 95% ☒ Modified Proctor FIELD CONTACT

TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____

☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☒ Building Footing ☐ Other _____[illegible]

REMARKS _____

- * Density failed to meet minimum project requirement

** Retest indicates minimum density requirement was obtained.

() Client is aware of unsatisfactory test results.

**COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST
FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006
Supplements and One (1) and Two (2) Family Dwellings**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE- AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS:

- Two (2) complete sets of plans containing the following:
- All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

Site Plan information including:

- Dimensions of lot or parcel of land
- Dimensions of all building set backs
- Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required:

- Plans or specifications must meet state compliance with FRC Chapter 3
- The following information must be shown as per section FRC
- Basic wind speed (3-second gust), miles per hour
- Wind importance factor and nature of occupancy
- Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifiably designed by the registered design professional.

Elevations Drawing including:

- All side views of the structure
- Roof pitch
- Overhang dimensions and detail with attic ventilation
- Location, size and height above roof of chimneys
- Location and size of skylights with Florida Product Approval
- Number of stories
- e) Building height from the established grade to the roof's highest peak

Floor Plan including:

- Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- All exterior and interior shear walls indicated
- Shear wall opening shown (Windows, Doors and Garage doors)
- Emergency escape and rescue opening in each bedroom (net clear opening shown)
- Safety glazing of glass where needed
- Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- Plans must show and identify accessibility of bathroom (see FRC 322)

All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

Foundation Plans Per FRC 403:

- a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling.
- d) Assumed load-bearing value of soil _____ (psf)
- e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

CONCRETE SLAB ON GRADE Per FRC R506

- Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports

PROTECTION AGAINST TERMITES Per FRC 320:

- Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606

- Show all materials making up walls, wall height, and Block size, mortar type
- Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

- Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers
- Girder type, size and spacing to load bearing walls, stem wall and/or piers
- Attachment of joist to girder
- Wind load requirements where applicable
- Show required under-floor crawl space
- Show required amount of ventilation opening for under-floor spaces
- Show required covering of ventilation opening.
- Show the required access opening to access to under-floor spaces
- Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- Show Draft stopping, Fire caulking and Fire blocking
- Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- Provide live and dead load rating of floor framing systems (psf).

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

ROOF SYSTEMS:

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

ROOF SHEATHING FRC Table R602,3(2) FRC 803

- Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

- Include all materials which will make up the roof assemblies covering; with Florida Product Approval numbers for each component of the roof assemblies covering.

FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

- Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

- All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- Ceiling fans
- Smoke detectors
- Service panel, sub-panel, location(s) and total ampere ratings

- On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- Appliances and HVAC equipment and disconnects
- Arc Fault Circuits (AFCI) in bedrooms
- Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

Private Potable Water

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED. A development permit will also be required. The permit cost is \$50.00.
- Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____ **Project Name:** _____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org


Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection


Contractor or Contractor's Authorized Agent Signature

 9/17/08
Print Name Date

Location

Permit # (FOR STAFF USE ONLY)

COLUMBIA COUNTY OFFICE OF ALTERNATE

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-113

Building permit No. 000027397

Use Classification SFD, UTILITY

Fire: 64.20

Permit Holder THEODORE BROCK

Waste: 167.50

Owner of Building MARONDA HOMES INC

Total: 231.70

Location: 379 SW TIMBER RIDGE CT., LAKE CITY, FL

Date: 12/17/2008

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



Theresa Dickel by J. W. L.

*Corrected
12/30/08 instead of 12/30/07*
COLUMBIA COUNTY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

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Use Classification SFD, UTILITY

Fire: 64.20

Permit Holder THEODORE BROCK

Waste: 167.50

Owner of Building MARONDA HOMES INC

Total: 231.70

Location: 379 SW TIMBER RIDGE DR., LAKE CITY, FL

Date: 12/30/2008



Harry Davis by GY
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Maronda Systems

Fax (407) 321-3913

Fax (407) 321-3913

Design Criteria: TPI Design: Matrix Analysis MiTek software

PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL
9TM01301	13-1	379 SW TIMBER RIDGE DR	JAX-9TM	GEOB5 LEFT


GEORGE WASHINGTON B&E

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone. Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.

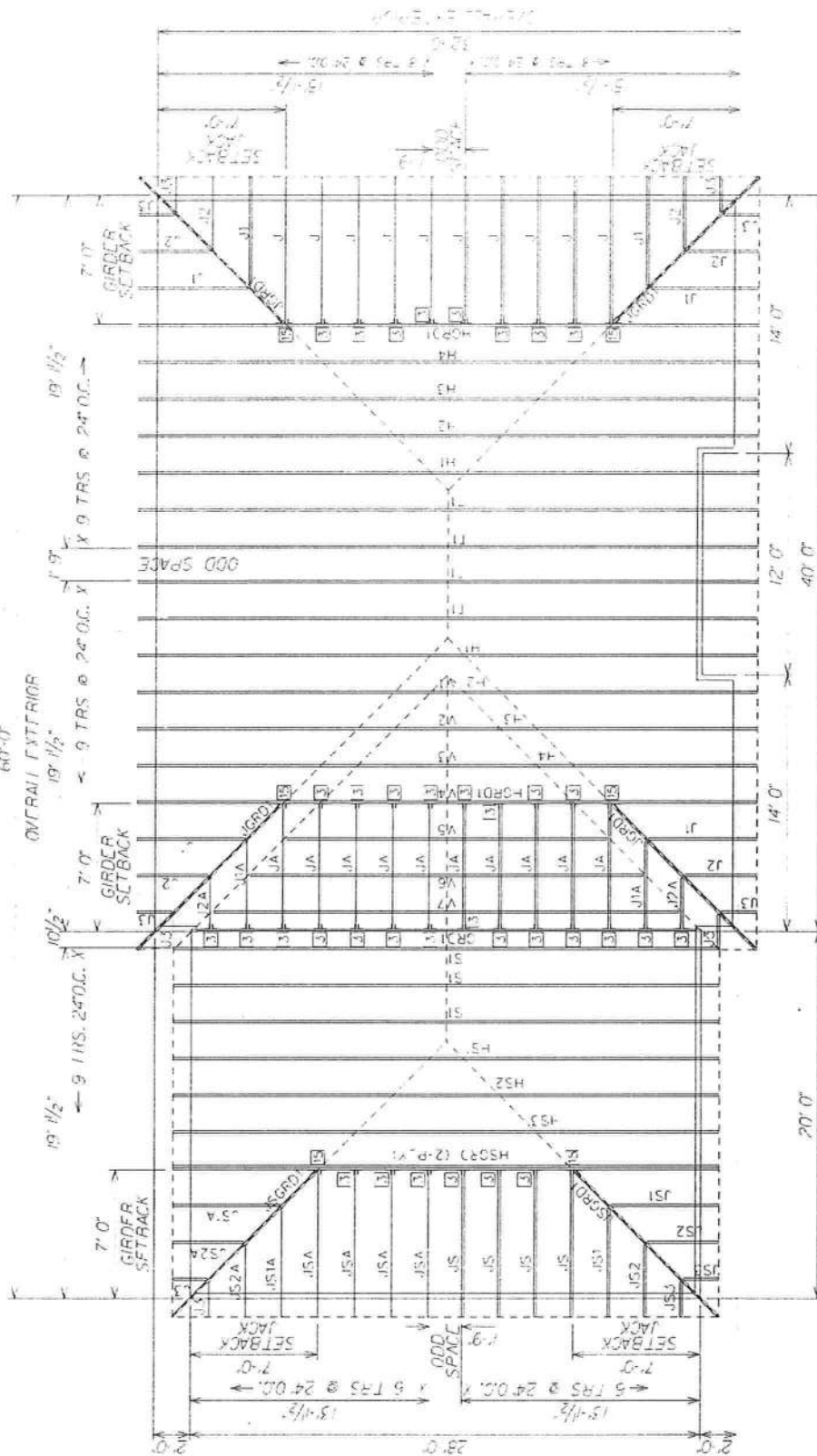
I, **Tomas Ponce P.E.** the Architect/Engineer of Record for the above referenced lot
Have reviewed the package and confirmed that it matches the physical and structural
Parameters found on the set of permit drawings.



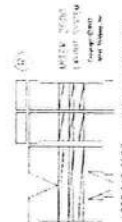
Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwgs:	46
Layout	10/30/07	01/30/08	V1	10/25/07	01/30/08	Roof Loads- TC Live: 16.0 psf TC Dead: 7.0 psf BC Live: 10.0 psf BC Dead: 10.0 psf Total 43.0 psf	
V	07/27/05	01/30/08	V2	10/25/07	01/30/08		
HIP	11/02/06	01/30/08	V3	10/25/07	01/30/08		
T1	10/25/07	01/30/08	V4	10/25/07	01/30/08		
H1	10/25/07	01/30/08	V5	10/25/07	01/30/08		
H2	10/25/07	01/30/08	V6	10/25/07	01/30/08		
H3	10/25/07	01/30/08	V7	10/25/07	01/30/08	DurFac- Lbr: 1.25 DurFac- Plt: 1.25 O.C. Spacing: 24.0"	
H4	10/25/07	01/30/08					
HGRD1	10/25/07	01/30/08					
GRD1	10/25/07	01/30/08					
S1	10/25/07	01/30/08					
HS1	10/25/07	01/30/08					
HS2	10/25/07	01/30/08				Floor Loads- TC Live: 40.0 psf TC Dead: 10.0 psf BC Live: 0.0 psf BC Dead: 5.0 psf Total 55.0 psf DurFac- Lbr: 1.00 DurFac- Plt: 1.00 O.C. Spacing: 24.0"	
HS3	10/25/07	01/30/08	Floor Layout	09/27/07	01/30/08		
HSGRD	10/25/07	01/30/08	FA	07/26/07	01/30/08		
JGRD1	10/25/07	01/30/08	FB	07/26/07	01/30/08		
JSGRD1	10/25/07	01/30/08	FC	07/26/07	01/30/08		
J	10/25/07	01/30/08	FD	07/26/07	01/30/08		
JS	10/25/07	01/30/08	FE	07/26/07	01/30/08	DurFac- Lbr: 1.00 DurFac- Plt: 1.00 O.C. Spacing: 24.0"	
JSA	10/25/07	01/30/08	FF	07/26/07	01/30/08		
JA	10/25/07	01/30/08	FG	07/26/07	01/30/08		
J1	10/25/07	01/30/08					
J1A	10/25/07	01/30/08					
JS1	10/25/07	01/30/08					
JS1A	10/25/07	01/30/08					
J2	10/25/07	01/30/08					
J2A	10/25/07	01/30/08					
JS2	10/25/07	01/30/08	INV #	DESC	QNTY		
JS2A	10/25/07	01/30/08	50060.0114	THD48			
J3	10/25/07	01/30/08	50060.0047	THD28			
JS3	10/25/07	01/30/08	50060.0110	JUS26	36		
			50060.0058	THJ26	6		
			50060.0049	THD28-2			
			SEAT PLATES				
			FLOOR SEAT PLATES			49	
			DATE:			JUL 28 2008	

HARDWARE LEGEND

- 1 HUS26
- 2 HUS28
- 3 JUS26
- 4 MP61
- 5 MP-A1 & MP-A11
- 6 SKH26 L/R
- 7 SKH26 I/R
- 8 SUS26
- 9 SUS28
- 10 TH026
- 11 TH028
- 12 TH028-2
- 13 TH028-3
- 14 TH048
- 15 HH26**
- 16 LTW12



SCALE: 1/8" = 1'-0"
DATE: 10-30-2007



DESIGNER: MKH
CHECKER: MKH

LOADING: BC2004 / 1 P12002

TC LIVE	16.00	SNOW LOAD	0.00
TC DEAD	7.00	LUMBER DOJL	1.25
BC LIVE	10.00	PLATE DOJL	1.25
BC DEAD	10.00	WIND	175
TOTAL	43.00	SPACING	2'-0"


GEO. WASHINGTON "B & E" - FL

GARAGE: 1111



JUL 28 2008

HARDWARE MANUFACTURED
BY USP
HARDWARE MANUFACTURED
BY SIMPSON
HARDWARE MANUFACTURED
BY CULLUM



MTK 2000
L'AVANTAGE
DE LA
TECHNOLOGIE

LOADING-FBC2004/TPI2002

TTC LIVE	40.00	SNOW LOAD	N/A
TTC DEAD	10.00	LUMBER DOL	1.00
IBC LIVE	0.00	PLATE DOL	1.00
IBC DEAD	5.00	WIND	N/A
TOTAL	55.00	SPACING	2'-0"

GARAGE : LEFT

THD28-2

THD48

PERMIT
FDO

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

Warning! The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

Warning! Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

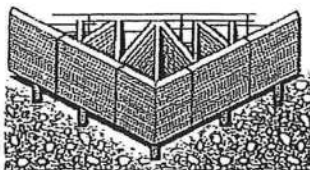
HANDLING — MANEJO

Warning! Avoid lateral bending. — Evite la flexión lateral.



Warning! The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



Warning! If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

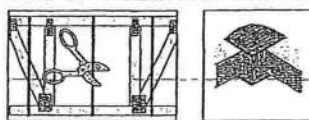
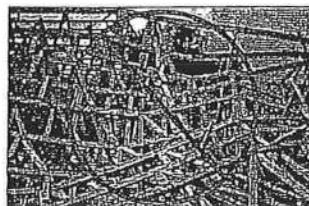
Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenamiento de los trusses en área de trabajo.

NOTAS GENERALES

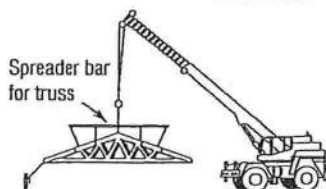
Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arrioste Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



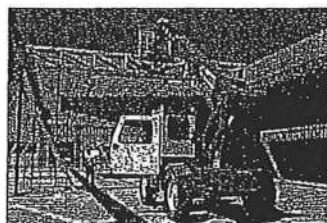
Warning! Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



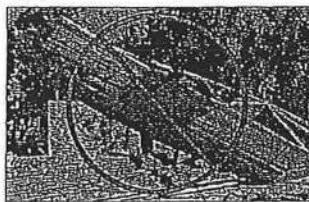
Warning! Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



Warning! Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



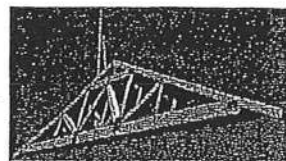
Warning! Do not store on uneven ground.

No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Warning! Don't overload the crane.
Advertencia! ¡No sobrecarga la grúa!
Warning! Never use banding alone to lift a bundle. Do not lift a group of individually banded bundles.
Nunca use sólo los empaques para levantar un paquete. No levante un grupo de empaques individuales.



Warning! A single lift point may be used for bundles with trusses up to 45'. Two lift points may be used for bundles with trusses up to 60'. Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies. Puede usar dos puntos de levantar para paquetes más de 60 pies. Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

Warning! Do not over load supporting structure with truss bundle.
Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

Warning! Place truss bundles in stable position. Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

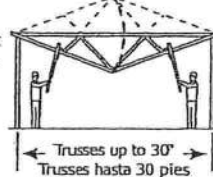
Warning! Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



Warning! Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



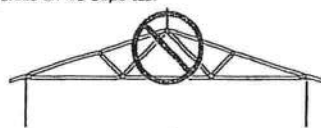
HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALES

Warning! Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

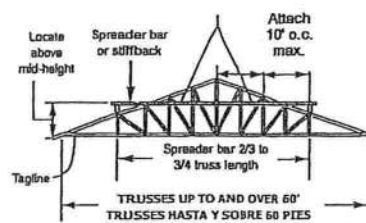
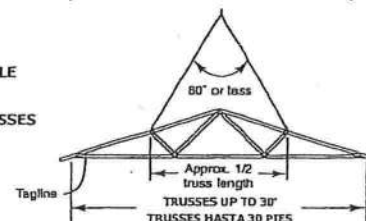
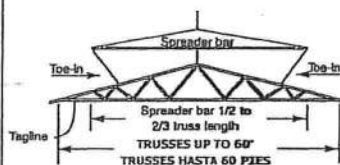
Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

Warning! Using a single pick-point at the peak can damage the truss.

Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

Warning! Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

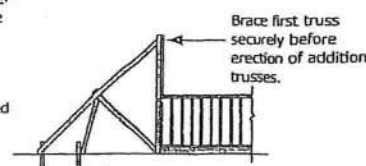
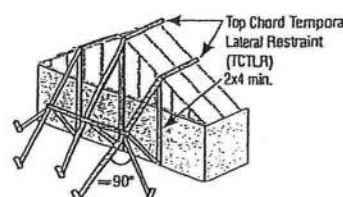
Vea el resumen BCSI B2 - Restricción/Arrioste Temporal y Instalación de los Trusses para más información.

Warning! Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).



Warning! Do not walk on unbraced trusses. No camine en trusses sueltos.



Brace first truss securely before erection of addition trusses.

STEPS TO SETTING TRUSSES

AS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

TOP CHORD — CUERDA SUPERIOR

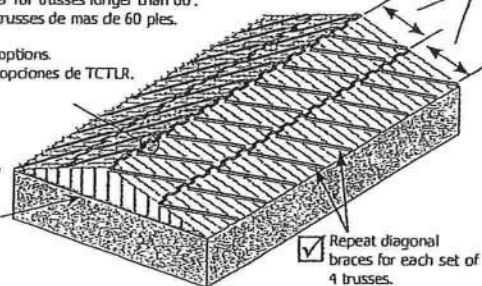
Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCLTR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un Ingeniero para trusses de mas de 60 pies.

See BCSI-B2 for TCLTR options.
Vea el BCSI-B2 para las opciones de TCLTR.

Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.

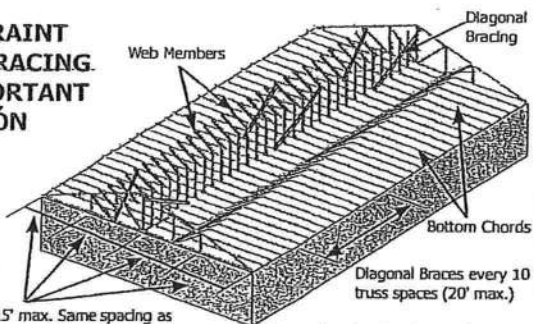


Ground bracing not shown for clarity.

Repeat diagonal braces for each set of 4 trusses.
Repita los arriostres diagonales para cada grupo de 4 trusses.

WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

! LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT !
! LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES !



Some chord and web members not shown for clarity.

BOTTOM CHORD — CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses.

Bottom chords

Diagonal Braces every 10 truss spaces (20' max.)

10'-15' max.

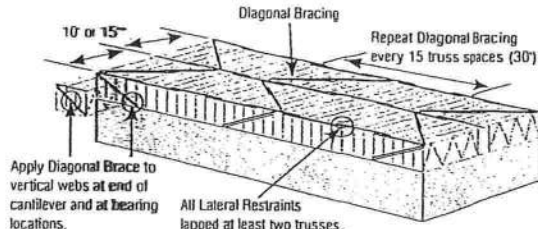
Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES

LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses for more information.

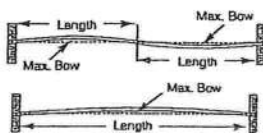
Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.



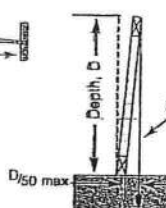
*Top chord Temporary Lateral Restraint spacing shall be 10' o.c. max. for 3x2 chords and 15' o.c. for 4x2 chords.

INSTALLING — INSTALACION

Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.



D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥8'

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.

Material	Height
Gypsum Board	12"
Plywood or OSB	16"
Asphalt Shingles	2 bundles
Concrete Block	8"
Clay Tile	3-4 ties high

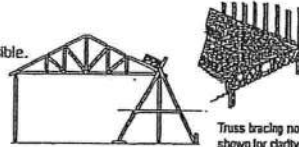


Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.

Never stack materials near a peak.
Nunca amontone material cerca del pico.

Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.

Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

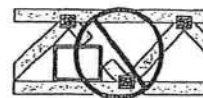


ALTERATIONS — ALTERACIONES

Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.
Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.



Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations are based on the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WITCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbndustry.com



TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinst.org

831WARN11x17 20061115

This document summarizes the information provided in Section B1 of the 2006 Edition of Building Component Safety Information (BCSI) - Guide to Good Practice for Handling, Installing, Restraint & Bracing of Metal Plate Connected Wood Trusses.

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

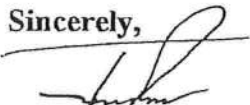
From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or designs furnished to the truss designer by the client or the correctness or accuracy of this information as it

my role to a specific project and accept no responsibility or exercise no control with regard to the design, construction, installation, testing, handling, shipment and liquidation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI-1 1995 and ASCE 7-97 and is incorporated as part of the building design by a Building Designer. When reviewed for approval by a building designer, the design loadings shown must be checked to make sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special stipulated loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not braced laterally by a properly applied rafter ceiling, they should be braced at a maximum spacing of 19' 0". Connector plates shall be designed and fabricated in accordance with the provisions measured from 20 gauge hot dipped galvanized steel. Refer to ASTM A 653. Grade 40 unless otherwise specified.

FABRICATION-NOTES-

As to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize assuming responsibility for such verification. Any discrepancies are to be noted before cutting or fabricating. Plates are to be milled or lathed before cutting or fabricating. Plates are to be treated before bolting or welding or disassembled. Members shall be cut for their fitting need to wood or steel. Connector plates shall be treated on both faces of the joint unless otherwise shown. A 5/4 plate is 5" wide x 10" long. A 6/4 plate is 6" wide x 8" long. Splice (bolts) are to parallel to the plate length specified. Double end on end members shall meet at the example of this web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with any redundant treated member unless otherwise shown. For additional information on Quality Control refer to ANS/PTP 1-1985.

PRECAUTIONARY NOTES

All handling and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal preconstruction bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and collapsing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought to design. Concentration of connection loads greater than the design loads shall not be applied to trusses at any time. The design of the connection of two trusses shall be applied to trusses with a fire all fastening and everything is designed.

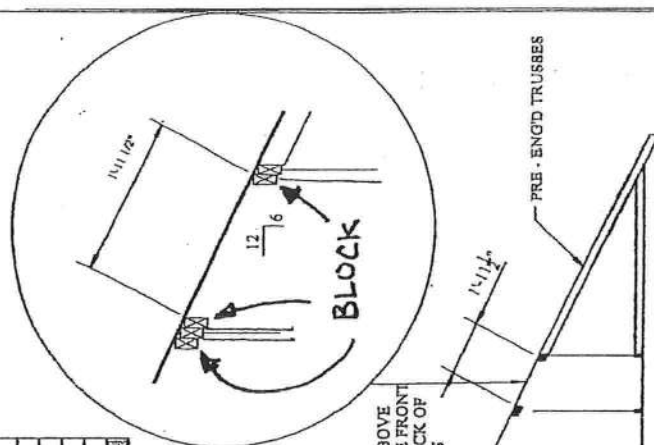
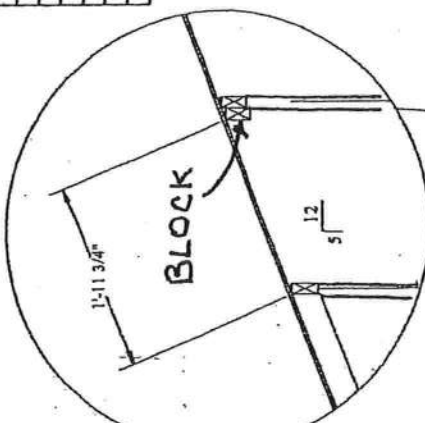
HIP TRUSS BLOCKING REQUIREMENTS

APA-FORM NO. TT-083

SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 74" O.C.

Roof Slope	Panel/Span Rating			48/24
	24/18	32/18	40/20	
≤ 1:12	OK	OK	OK	OK
≤ 2:12	OK	OK	OK	OK
≤ 3:12	OK	OK	OK	OK
≤ 4:12	OK	OK	OK	OK
≤ 5:12	OK	OK	OK	OK
≤ 6:12	OK	OK	OK	OK
≤ 8:12	OK	OK	OK	OK
≤ 7:12	OK	OK	OK	OK
≤ 8:12	OK	OK	OK	OK
≤ 9:12	OK	OK	OK	OK
≤ 10:12	OK	OK	OK	OK
≤ 11:12	OK	OK	OK	OK
≤ 12:12	OK	OK	OK	OK
≤ 13:12	OK	OK	OK	OK
≤ 14:12	OK	OK	OK	OK

NOTE: SHADED AREA REQUIRE IX LUMBER BLOCKING BETWEEN TRUSS AT UN SUPPORTED PANTII. EDDING IN HIT ROOF AREA



6/12 OR ABOVE
(2) 2X ONE FRONT
& ONE BACK OF
HIP TRUSS

2X BLOCKING TYP
NAILED W/ 10D 12" O.C.

5/12 (1) 2X
TO THE FRONT OF
HIP TRUSS

7/16" OSB ROOF SHEATHING

NOTE: NO BLOCKING IS REQUIRED ON 4/12 AND BELOW PITCHED ROOFS

Maronda Systems
4005 MARONDA WAY
Baltimore, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
357 Medallion Pl. Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ALL PARTICIPATING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, but similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only in raftere bracing length. Provisions must be made to anchor lateral bracing at ends and specified members only by the building designer. Additional bracing of the overall structure may be required. (See HB-91 (LTP) for specific details.) For more information, contact building designer (Truss Photo Institute, 171 E. Boston at 343)
Onorio Drive, Madison, Wisconsin 53719)
Component Engineering By Truss Engineering Co., P.A., 181 Southside Rd., Edgemoor, NC 77932

Eng Job:	WO: HIPDETAIL	
Dwg:	TI: HIP	
Dsgnr: TLY	Chk:	11/2/2006
TC Live	16.0	psf
TC Dead	7.0	psf
BC Live	10.0	psf
BC Dead	10.0	psf
TOTAL	43.0	psf
		lbr DF: 1.25
		Flt DF: 1.25
		O.C.: 2- 0- 0
		TPI-02/FEC-04
		Code: FTA
		V4.7.32-0

Trenco

818 Soundside Rd
Edenton, NC 27932

Re: GEOWASHINGTON
GEOWASHINGTON_FLORIDA_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4478918 thru E4478952

My license renewal date for the state of Florida is February 28, 2009.



October 25, 2007

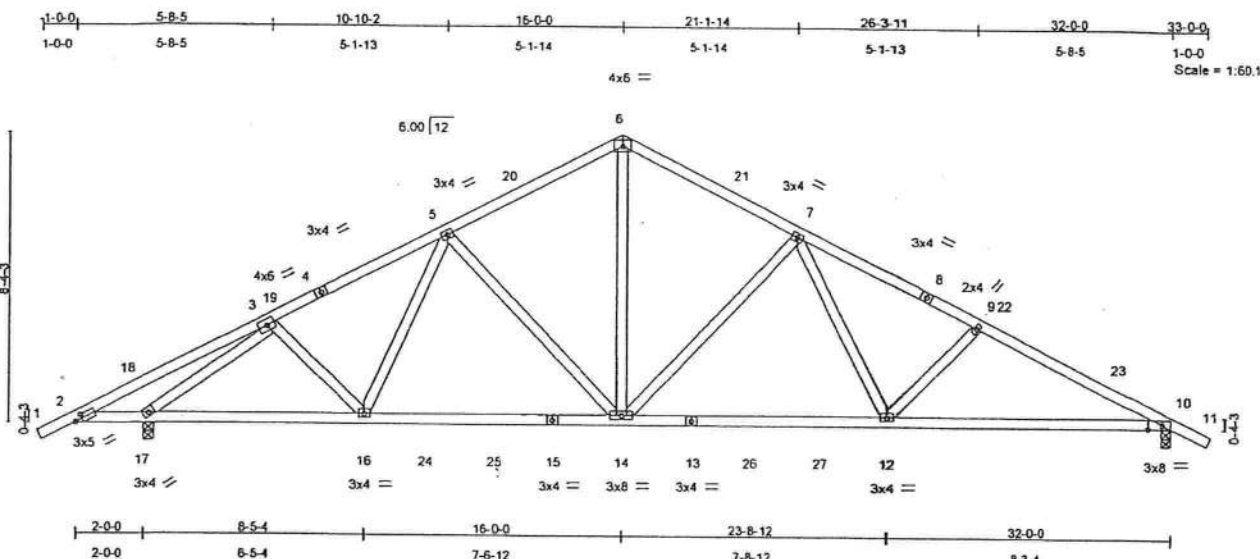
Lassiter, Frank

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.
Engineering services provided by Truss Engineering Company.

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478945
GEOWASHINGTON	T1	COMMON	4	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MITek Industries, Inc. Thu Oct 25 08:56:35 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.36	Vert(LL)	-0.19 12-14	>999	240	MT20	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.72	Vert(TL)	-0.34 14-16	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.82	Horz(TL)	0.09 10	n/a	n/a		
BCCL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
5-14 2 X 4 SYP No.2, 6-14 2 X 4 SYP No.2, 7-14 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 10=1411/0-3-8, 17=1596/0-3-8
Max Horz 17=144(LC 5)
Max Uplift 10=497(LC 7), 17=545(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/21, 2-18=406/157, 3-18=396/262, 3-19=1896/621, 4-19=1887/623, 4-5=1841/641, 5-20=1530/644,
6-20=1435/656, 6-21=1434/656, 7-21=1530/645, 7-8=2237/790, 8-22=2286/772, 9-22=2295/770, 9-23=2431/837,
10-23=2462/818, 10-11=0/21
BOT CHORD 2-17=184/489, 16-17=334/1500, 16-24=329/1565, 24-25=329/1565, 15-25=329/1565, 14-15=329/1565,
13-14=400/1740, 13-26=400/1740, 26-27=400/1740, 12-27=400/1740, 10-12=597/2156
WEBS 3-16=0/237, 5-16=0/235, 5-14=386/205, 6-14=285/1076, 7-14=619/309, 7-12=120/615, 9-12=227/277,
3-17=2125/1040

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCCL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 13-0-0, Exterior(2) 13-0-0 to 16-0-0, Interior(1) 19-0-0 to 30-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 497 lb uplift at joint 10 and 545 lb uplift at joint 17.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on October 25, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.

October 25, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MI-7473 BEFORE USE.

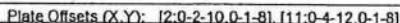
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, DSI-89 and CS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:20 2007 Page 1



LUMBER

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-8-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 8-0-0 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension

NOTES

- LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on October 25, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.

Maronda Homes Inc., Sanford, Fla.

7.010 s Oct 16 2007 MiTek Industries, Inc. Thu Oct 25 08:56:21 2007 Page 1



Weight: 164 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-7-5 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 7-1-5 oc bracing.
WEBS	1 Row at midpt 4-10

Max Horiz 2=116(LC 4)

TOP CHORD	1-2=0/21, 2-14=2603/1017, 14-15=2556/1021, 3-15=2491/1035, 3-16=1992/892, 4-16=1925/913, 4-5=1713/805, 5-17=1802/813, 6-17=1968/892, 6-18=2470/1035, 18-19=2535/1021, 7-19=2582/1017, 7-8=0/21
BOT CHORD	2-13=764/2271, 12-13=764/2271, 12-20=490/1734, 11-20=490/1734, 10-11=490/1734, 9-10=764/2252, 7-9=764/2252
WEBS	3-13=0/285, 3-12=626/315, 4-12=77/612, 4-10=156/92, 5-10=77/576, 6-10=628/316, 6-9=0/288

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-10-13 to 1-11-13, Interior(1) 1-11-13 to 8-9-1, Exterior(2) 8-9-1 to 23-2-15, Interior(1) 23-2-15 to 30-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 519 lb uplift at joint 2 and 519 lb uplift at joint 7.

This document was originally issued by Lassiter, Frank on October 25, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.

October 25, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Miteltec connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS1/771 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Affiliat

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478921
GEOWASHINGTON	HIP	HIP	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:22 2007 Page 1

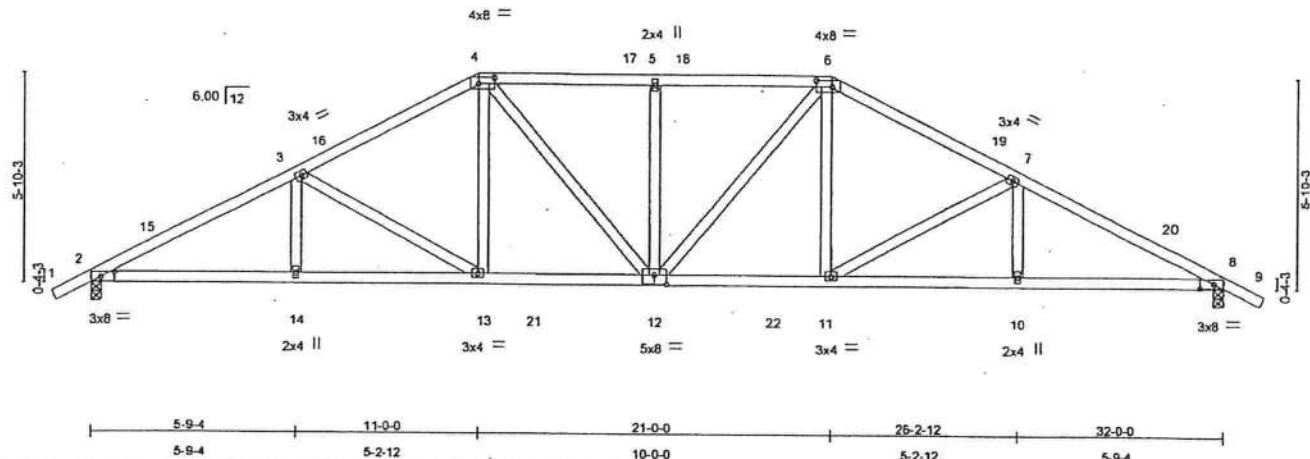
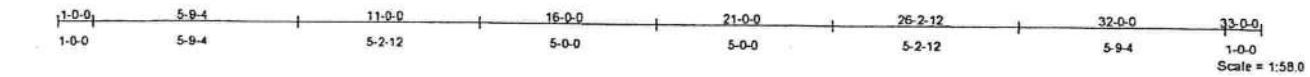


Plate Offsets (X,Y): [2:0-4-12:0-1-8], [4:0-5-4-0-2-0], [6:0-5-4-0-2-0], [8:0-4-12:0-1-8], [12:0-4-0-0-3-0]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		PLATES	
TCLL	18.0	Plates Increase 1.25		TC 0.29		in (loc) l/defl L/d		MT20	GRIP
TCDL	7.0	Lumber Increase 1.25		BC 0.54		Vert(LL) -0.15 11-12 >999 240			
BCLL	10.0	Rep Stress Incr YES		WB 0.33		Vert(TL) -0.29 11-12 >999 180			
BCDL	10.0	Code FBC2004/TP12002		(Matrix)		Horz(TL) 0.12 8 n/a n/a			
								Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
4-12 2 X 4 SYP No.2, 6-12 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-11-4 oc bracing.

REACTIONS (lb/size) 2=1478/0-3-8, 8=1478/0-3-8
Max Horz 2=97(LC 4)
Max Uplift 2=516(LC 6), 8=516(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/21, 2-15=2665/1074, 3-15=2624/1089, 3-16=2168/974, 4-16=2113/992, 4-17=2093/1037, 5-17=2092/1037, 5-18=2092/1037, 6-18=2093/1037, 6-19=2113/992, 7-19=2168/974, 7-20=2624/1089, 8-20=2665/1074, 8-9=0/21
BOT CHORD 2-14=823/2328, 13-14=823/2328, 13-21=596/1902, 12-21=596/1902, 12-22=596/1902, 11-22=596/1902, 10-11=823/2328, 8-10=823/2328
WEBS 3-14=0/236, 3-13=504/269, 4-13=66/497, 4-12=203/376, 6-12=203/376, 6-11=66/497, 7-11=504/269, 7-10=0/236, 5-12=228/268

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 6-9-1, Exterior(2) 6-9-1 to 11-0-0, Interior(1) 15-2-15 to 16-9-1, Exterior(2) 21-0-0 to 33-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 516 lb uplift at joint 2 and 516 lb uplift at joint 8.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on October 25, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.

October 25, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

7.010 b Oct 16 2007 MiTek Industries, Inc. Thu Oct 25 08:56:23 2007 Page 1



Weight: 159 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-7-1 oc purfins.
BOT CHORD	Rigid ceiling directly applied or 6-8-1 oc bracing.

$$\text{Max Up lift}_2 = 519(\text{LC } 6), 8 = 519(\text{LC } 7)$$

2-12=886/2163, 11-12=676/1995, 10-11=676/1995, 8-10=886/2163
3-12=209/253, 4-12=17/446, 4-11=342/597, 5-11=341/383, 6-11=342/597, 6-10=18/446, 7-10=209/253

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125 mph (3-second gust); h=25ft; TCDF=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-13 to 1-11-3, Interior(1) 1-11-3 to 4-7-5, Exterior(2) 4-7-5 to 9-0-0, Interior(1) 13-2-15 to 18-9-1, Exterior(2) 23-0-0 to 33-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 519 lb uplift at joint 2 and 519 lb uplift at joint 8.

This document was originally issued by Lassiter, Frank on October 25, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MTI-7473 BEFORE USE.

Design valid for use only with Hiltel connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design to other situations and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members. Proper bracing for entire truss structure is responsibility of building designer. Truss designer is responsible for overall structure. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/PTI Quality Criteria, DSI-89 and BCS/1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478923
GEOWASHINGTON	HGRD1	HIP	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7,010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:24 2007 Page 1

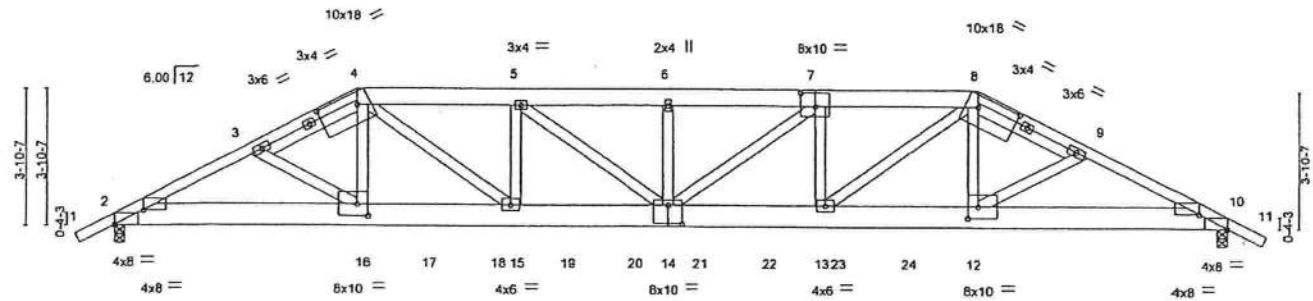


Plate Offsets (X,Y): [2:0-9-15,0-4-12], [2:0-0-0,0-0-4], [4:1-1-12,0-4-0], [7:0-5-0,0-4-8], [8:1-1-12,0-4-0], [8:0-0-0,0-0-0], [9:0-0-0,0-0-0], [10:0-0-0,0-0-4], [10:0-9-15,0-4-12], [12:0-3-8,0-4-0], [14:0-5-0,0-6-4], [16:0-3-8,0-4-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.77	Vert(LL)	0.54	14	>701	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.82	Vert(TL)	-0.71	14	>535	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.76	Horz(TL)	0.15	10	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 237 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 "Except"	TOP CHORD Structural wood sheathing directly applied or 2-0-14 oc purlins.
4-7 2 X 6 SYP No.2, 7-8 2 X 6 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-1-3 oc bracing.
BOT CHORD 2 X 8 SYP No.1D	
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=3178/0-3-8, 10=3178/0-3-8
Max Horz 2=80(LC 6)
Max Uplift 2=1993(LC 4), 10=1993(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=6552/4405, 3-4=6603/4524, 4-5=7774/5418, 5-6=8329/5780, 6-7=8329/5780, 7-8=7776/5420, 8-9=6604/4525, 9-10=6552/4406, 10-11=0/26
BOT CHORD 2-16=3937/5832, 16-17=4128/6074, 17-18=4128/6074, 15-18=4128/6074, 15-19=5347/7769, 19-20=5347/7769, 14-20=5347/7769, 14-21=5322/7791, 21-22=5322/7791, 13-22=5322/7791, 13-23=4085/6075, 23-24=4085/6075, 12-24=4085/6075, 10-12=3894/5832
WEBS 3-16=264/341, 4-16=716/1051, 4-15=1554/2125, 5-15=460/314, 5-14=505/741, 6-14=200/145, 7-14=492/722, 7-13=465/318, 8-13=1556/2127, 8-12=718/1053, 9-12=264/341

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1993 lb uplift at joint 2 and 1993 lb uplift at joint 10.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 785 lb down and 745 lb up at 7-0-0, 243 lb down and 249 lb up at 9-0-12, 243 lb down and 249 lb up at 11-0-12, 243 lb down and 249 lb up at 13-0-12, 243 lb down and 249 lb up at 15-0-12, 243 lb down and 249 lb up at 16-11-4, 243 lb down and 249 lb up at 18-11-4, 243 lb down and 249 lb up at 20-11-4, and 243 lb down and 249 lb up at 22-11-4, and 785 lb down and 745 lb up at 24-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE M1-7473 BEFORE USE.
Design void for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478925
GEOWASHINGTON	HGRD1	HIP	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:24 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 4-8=-46, 8-11=-46, 2-10=-40

Concentrated Loads (lb)

Vert: 16=-785(F) 12=-785(F) 17=-243(F) 18=-243(F) 19=-243(F) 20=-243(F) 21=-243(F) 22=-243(F) 23=-243(F) 24=-243(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.

Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ICS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofre Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27632

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4476918
GEOWASHINGTON	GRD1	COMMON	1	1	Job Reference (optional)	

Meranda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:19 2007 Page 1

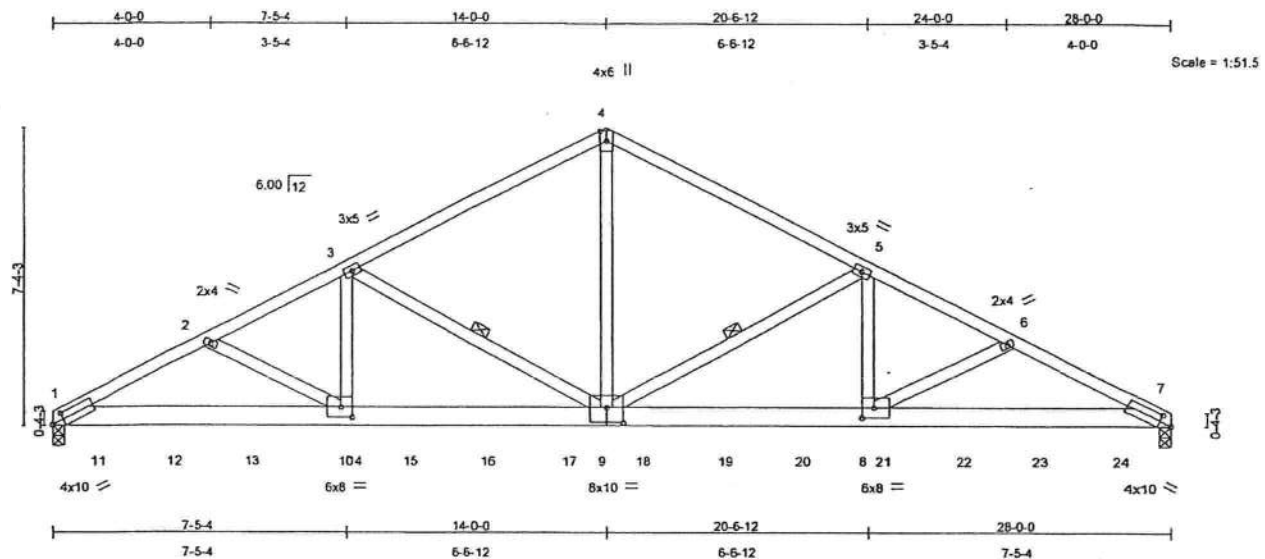


Plate Offsets (X,Y): [1:0-3-9-0-2-0] [7:0-3-9-0-2-0] [8:0-3-8-0-3-0] [9:0-5-0-0-4-8] [10:0-3-8-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.52	Vert(LL)	0.28	9-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.76	Vert(TL)	-0.43	9-10	>765	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.84	Horz(TL)	0.12	7	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 164 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-11 oc bracing.
WEBS 1 Row at midpt 3-9, 5-9

REACTIONS

(lb/size) 1=2667/0-3-8, 7=2746/0-3-8
Max Horz 1=115(LC 4)
Max Uplift 1=1058(LC 5), 7=969(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=5035/2042, 2-3=4876/1984, 3-4=3373/1275, 4-5=3373/1275, 5-6=4876/1607, 6-7=5035/1680
BOT CHORD 1-11=1882/4473, 11-12=1882/4473, 12-13=1882/4473, 13-14=1882/4473, 10-14=1882/4473, 10-15=1771/4336, 15-16=1771/4336, 16-17=1771/4336, 9-17=1771/4336, 9-18=1326/4336, 18-19=1326/4336, 19-20=1326/4336, 8-20=1326/4336, 8-21=1457/4473, 21-22=1457/4473, 22-23=1457/4473, 23-24=1457/4473, 7-24=1457/4473
WEBS 2-10=154/125, 3-10=577/1220, 3-9=1595/887, 4-9=935/2624, 5-9=1595/500, 5-8=222/1220, 6-8=154/147

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1058 lb uplift at joint 1 and 969 lb uplift at joint 7.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 78 lb down and 56 lb up at 1-1-13, 164 lb down and 92 lb up at 3-1-13, 247 lb down and 126 lb up at 5-0-12, 83 lb down and 105 lb up at 28-10-3, 295 lb down and 186 lb up at 7-0-12, 295 lb down and 186 lb up at 9-0-12, 295 lb down and 186 lb up at 11-0-12, 295 lb down and 186 lb up at 13-0-12, 247 lb down and 60 lb up at 14-11-4, 247 lb down and 60 lb up at 16-11-4, 247 lb down and 60 lb up at 18-11-4, 247 lb down and 60 lb up at 20-11-4, 247 lb down and 80 lb up at 22-11-4, and 191 lb down and 132 lb up at 24-10-3, and 78 lb down and 19 lb up at 28-9-2 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=46, 4-7=46, 1-7=40

Continued on page 2

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478918
GEOWASHINGTON	GRD1	COMMON	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:19 2007 Page 2

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 7=78(F) 11=78(F) 12=164(F) 13=247(F) 14=247(F) 15=247(F) 16=247(F) 17=247(F) 18=247(F) 19=247(F) 20=247(F) 21=247(F) 22=247(F)
23=164(F) 24=78(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, B58-89 and IBC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondrio Drive, Madison, WI 53719.

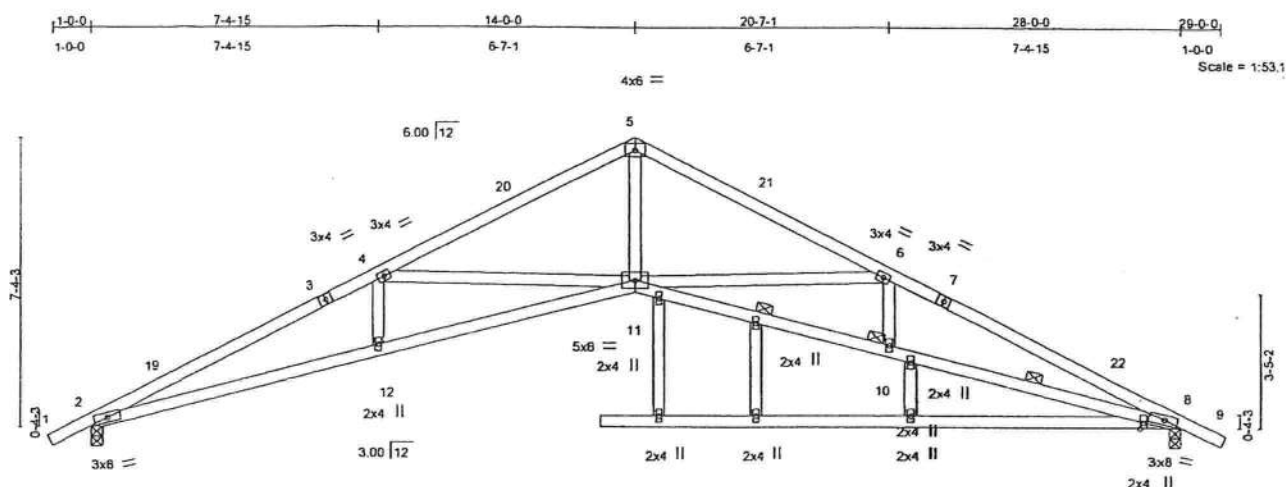
ENGINEERING BY
TRENCO
A Mitek Affiliate

618 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478944
GEOWASHINGTON	S1	SCISSORS	3	1	Job Reference (optional)	

Maranda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:34 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.63	Vert(LL)	-0.37 10-11	>897	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.93	Vert(TL)	-0.75 10-11	>446	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.70	Horz(TL)	0.53 8	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
4-11 2 X 4 SYP No.2, 6-11 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-0 oc purins.
BOT CHORD Rigid ceiling directly applied or 6-1-11 oc bracing. Except:
6-2-0 oc bracing: 8-10
6-4-0 oc bracing: 10-11
JOINTS 1 Brace at J(s): 10

REACTIONS (lb/size) 2=1247/0-3-8, 8=1247/0-3-8
Max Horz 2=126(LC 4)
Max Uplift 2=476(LC 6), 8=476(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/20, 2-19=3855/1247, 3-19=3807/1254, 3-4=3733/1270, 4-20=2706/896, 5-20=2633/907, 5-21=2633/907,
6-21=2706/896, 6-7=3733/1270, 7-22=3807/1254, 8-22=3855/1247, 8-9=0/20
BOT CHORD 2-12=1001/3514, 11-12=1000/3507, 10-11=1000/3507, 8-10=1001/3514
WEBS 4-12=0/296, 4-11=1057/521, 5-11=451/2043, 6-11=1057/527, 6-10=0/296

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 11-0-0, Exterior(2) 11-0-0 to 14-0-0, Interior(1) 17-0-0 to 26-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 476 lb uplift at joint 8.

LOAD CASE(S) Standard

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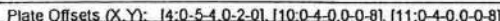
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-B7 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27832

Maronda Homes Inc., Sanford, FL

7,010 & Oct 16 2007 MTEK Industries, Inc. Thu Oct 25 08:56:25 2007 Page 1



LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-7 oc bracing. Except:
5-11-0 oc bracing: 7-9

REACTIONS

(lb/size) 2=1247/0-3-8, 7=1247/0-3-8
Max Horz 2=117(LC 5)
Max Uplift 2=-476(LC 6), 7=-476(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD
1-2=0/20, 2-15=3927/1333, 15-16=3877/1338, 3-16=3812/1352, 3-17=2819/991, 4-17=2753/1002, 4-5=2506/584,
5-18=2760/1003, 6-18=2825/992, 6-19=3808/1351, 19-20=3873/1337, 7-20=3824/1332, 7-8=0/20
BOT CHORD
2-12=1084/3580, 11-12=1084/3572, 10-11=582/2500, 9-10=1084/3569, 7-9=1083/3577
WEBS
3-12=0/286, 3-11=894/508, 4-11=239/1052, 4-10=140/182, 5-10=238/1053, 5-10=986/512, 6-9=0/284

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDF=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1.0-13 to 1-11-3, Interior(1) 1-11-3 to 10-0-0, Exterior(2) 10-0-0 to 18-0-0, Interior(1) 18-0-0 to 26-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 476 lb uplift at joint 7.

LOAD CASE(S) Standard

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October 25, 2007

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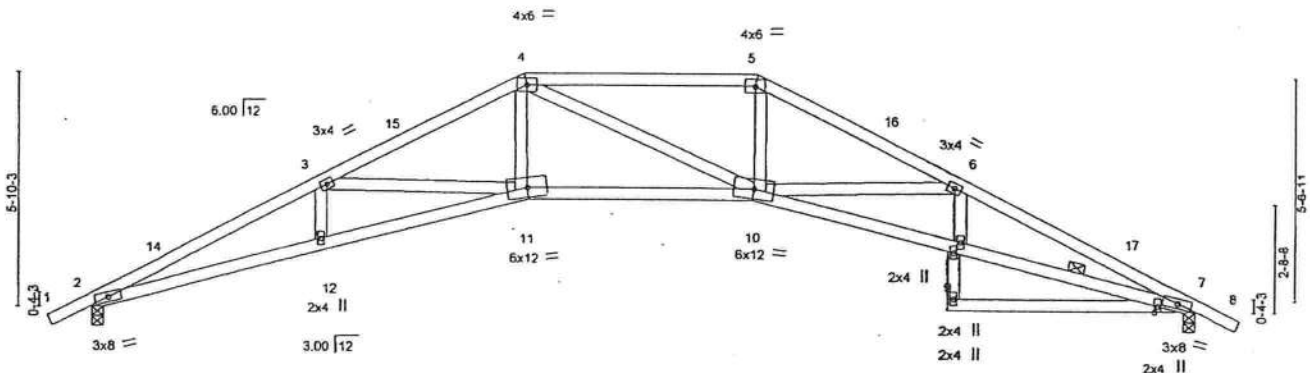
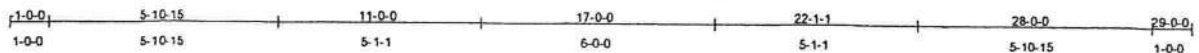
ENGINEERING BY
TRENCO
A Miltek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478925
GEOWASHINGTON	HS2	SPECIAL	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7,010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:26 2007 Page 1



LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	16.0	Plates Increase	1.25	TC	0.42	ln (loc)	l/defl	L/d			
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(LL)	-0.37 10-11	>906	240	MT20	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Vert(TL)	-0.74 10-11	>452	180		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Horz(TL)	0.51 7	n/a	n/a		
										Weight: 138 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
4-10 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-8 oc bracing. Except:
5-8-0 oc bracing: 7-9

REACTIONS

(lb/size) 2=1247/0-3-8, 7=1247/0-3-8
Max Horz 2=99(LC 5)
Max Uplift 2=474(LC 6), 7=474(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/20, 2-14=3964/1462, 3-14=3917/1478, 3-15=3177/1222, 4-15=3129/1233, 4-5=2830/1183, 5-16=3132/1233,
6-16=3179/1221, 6-17=3916/1478, 7-17=3963/1462, 7-8=0/20
BOT CHORD 2-12=1207/3603, 11-12=1207/3603, 10-11=828/2828, 9-10=1207/3602, 7-9=1207/3603
WEBS 3-12=0/217, 3-11=684/409, 6-10=681/417, 6-9=0/216, 4-11=248/1134, 5-10=248/1134, 4-10=167/172

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-10-13 to 1-11-3, Interior(1) 1-11-3 to 8-0-7, Exterior(2) 8-0-7 to 19-11-9, Interior(1) 19-11-9 to 26-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 7.

LOAD CASE(S) Standard

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ENGINEERING BY
TRENCO
A Mitek Alliance

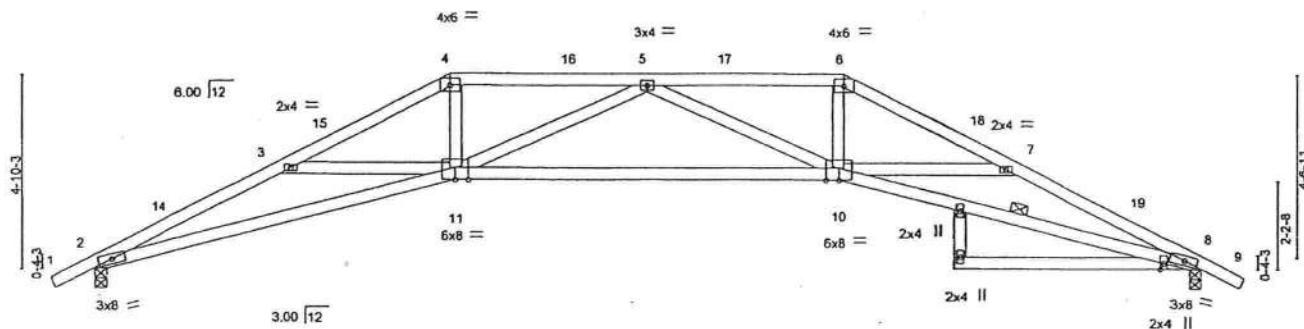
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478926
GEOWASHINGTON	HS3	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7,010 s Oct 16 2007 MTEK Industries, Inc. Thu Oct 25 08:56:26 2007 Page 1



Scale = 1:52.1



9-1-6		18-10-8		28-0-0			
9-1-6		9-9-0		9-1-8			
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC .043	Vert(LL) -0.49 10-11	>680 240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.98	Vert(TL) -1.02 10-11	>327 180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.45	Horz(TL) 0.52 8	n/a n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)				
						Weight: 136 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 "Except"
2-11 2 X 4 SYP No.1D, 8-10 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
5-9-0 oc bracing: 8-10

REACTIONS

(lb/size) 2=1247/0-3-8, 8=1247/0-3-8
Max Horz 2=80(LC 5)
Max Uplift 2=476(LC 6), 8=476(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/20, 2-14=-3927/1609, 3-14=-3898/1625, 3-15=-3656/1338, 4-15=-3626/1349, 4-16=-3317/1294, 5-16=-3317/1294
5-17=-3307/1291, 6-17=-3307/1291, 6-18=-3616/1346, 7-18=-3654/1335, 7-19=-3887/1621, 8-19=-3916/1605,
8-9=0/20
BOT CHORD 2-11=-1352/3601, 10-11=-1206/3537, 8-10=-1349/3591
WEBS 3-11=-215/416, 4-11=-326/1388, 5-11=-367/395, 5-10=-377/396, 6-10=-325/1384, 7-10=-216/423

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-13 to 1-11-3, Interior(1) 1-11-3 to 6-0-0, Exterior(2) 6-0-0 to 9-0-0, Interior(1) 12-0-0 to 16-0-0, Exterior(2) 18-0-0 to 29-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 476 lb uplift at joint 8.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on October 25, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above. October 25, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEK Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478927
GEOWASHINGTON	MSGRD	SPECIAL	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Thu Oct 25 08:56:27 2007 Page 1

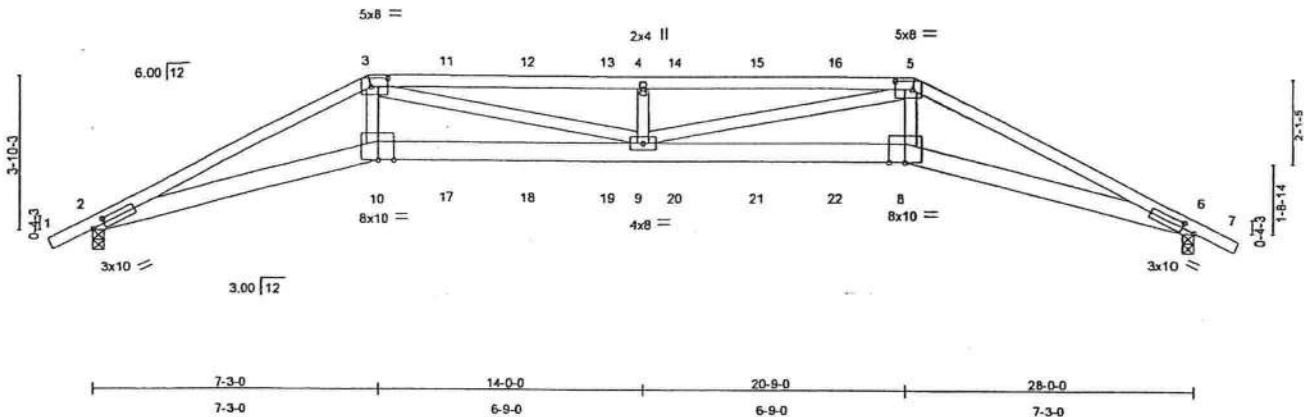
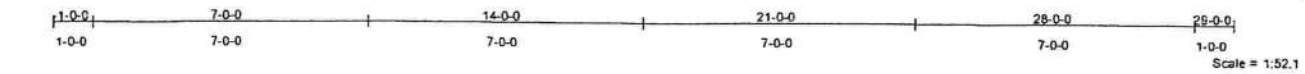


Plate Offsets (X,Y): [2:0-3-14,0-1-8], [3:0-5-4,0-2-8], [5:0-5-4,0-2-8], [6:0-3-14,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.69	Vert(L)	0.79	9	>422	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.78	Vert(TL)	-1.12	9	>296	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.58	Horz(TL)	0.54	6	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 282 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.2 "Except"
8-10 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-7 oc bracing.

REACTIONS

(lb/size) 2=2703/0-3-8, 6=2701/0-3-8
Max Horz 2=79(LC 8)
Max Uplift 2=1617(LC 7), 6=1615(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/23, 2-3=10301/6336, 3-11=12187/7570, 11-12=12187/7570, 12-13=12187/7570, 4-13=12187/7570,
4-14=12187/7570, 14-15=12187/7570, 15-16=12187/7570, 5-16=12187/7570, 5-6=10289/6244, 6-7=0/23
BOT CHORD 2-10=5788/9429, 10-17=5869/9569, 17-18=5869/9569, 18-19=5869/9569, 9-19=5869/9569, 9-20=5718/9558,
20-21=5718/9558, 21-22=5718/9558, 8-22=5718/9559, 6-8=5637/9419
WEBS 4-9=378/447, 3-9=1788/2863, 3-10=2110/3640, 5-8=2069/3632, 5-9=1804/2875

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.80 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1617 lb uplift at joint 2 and 1615 lb uplift at joint 6.

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Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and IBCS11 Building Component Safety Information available from Truss Plate Institute, 583 O'Onofre Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEK AFFILIATE

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478927
GEOWASHINGTON	HSGRD	SPECIAL	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:27 2007 Page 2

NOTES

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 18 lb down and 89 lb up at 7-0-7, 18 lb down and 89 lb up at 9-0-12, 18 lb down and 89 lb up at 11-0-12, 18 lb down and 89 lb up at 13-0-12, 18 lb down and 89 lb up at 14-9-12, 18 lb down and 89 lb up at 16-11-4, and 18 lb down and 89 lb up at 18-11-4, and 18 lb down and 89 lb up at 20-11-9 on top chord, and 788 lb down and 625 lb up at 7-3-0, 198 lb down and 132 lb up at 9-0-12, 198 lb down and 132 lb up at 11-0-12, 198 lb down and 132 lb up at 13-0-12, 198 lb down and 132 lb up at 14-9-12, 198 lb down and 132 lb up at 16-11-4, and 198 lb down and 132 lb up at 18-11-4, and 788 lb down and 625 lb up at 20-9-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-46, 3-5=-46, 5-7=-46, 2-10=-40, 8-10=-40, 6-8=-40

Concentrated Loads (lb)

Vert: 3=-18(B) 5=-18(B) 10=-788(B) 8=-788(B) 11=-18(B) 12=-18(B) 13=-18(B) 14=-18(B) 15=-18(B) 16=-18(B) 17=-198(B) 18=-198(B) 19=-198(B) 20=-198(B) 21=-198(B) 22=-198(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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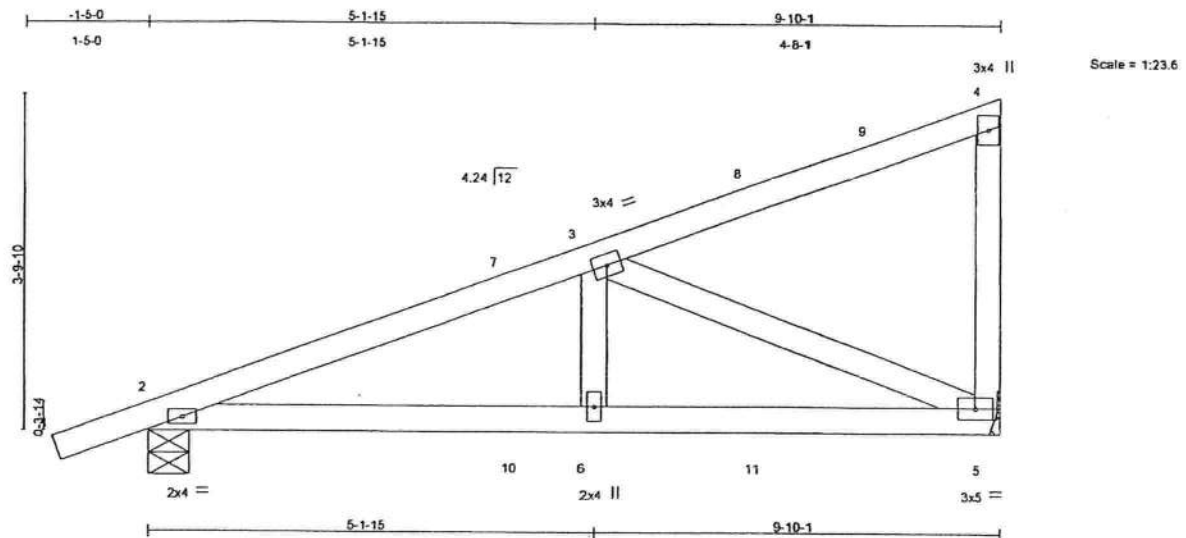
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478635
GEOWASHINGTON	UGRD1	MONO TRUSS	4	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:30 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.03	2-6	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.33	Vert(TL)	-0.06	5-6	>999	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.36	Horz(TL)	0.01	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 46 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-3-5 oc bracing.

REACTIONS (lb/size) 5=576/Mechanical, 2=550/0-5-11
Max Horz 2=227(LC 3)
Max Uplift 5=385(LC 3), 2=343(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/15, 2-7=-927/468, 3-7=-853/446, 3-8=-119/91, 8-9=-53/0, 4-9=-24/0, 4-5=-125/180
BOT CHORD 2-10=-574/838, 6-10=-574/838, 6-11=-574/838, 5-11=-574/838
WEBS 3-6=0/304, 3-5=-842/569

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 1'-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 5 and 343 lb uplift at joint 2.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 4 lb down and 64 lb up at 4-3-4, 4 lb down and 64 lb up at 4-3-4, and 55 lb down and 162 lb up at 7-1-3, and 55 lb down and 162 lb up at 7-1-3 on top chord, and 14 lb down at 4-3-4, 14 lb down at 4-3-4, and 54 lb down at 7-1-3, and 54 lb down at 7-1-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=46, 2-5=40

Concentrated Loads (lb)

Vert: 7=7(F=-4, B=-4) 8=110(F=-55, B=-55) 10=28(F=-14, B=-14) 11=108(F=-54, B=-54)

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

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ENGINEERING BY
TRENCO
A Mitek Affiliate

816 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E 4478943
GEOWASHINGTON	JSGRD1	MONO SCISSOR	2	1	Job Reference (optional)	
Meranda Homes Inc., Sanford, FL						
7.010 s Oct 15 2007 Mitek Industries, Inc. Thu Oct 25 08:56:34 2007 Page 1						

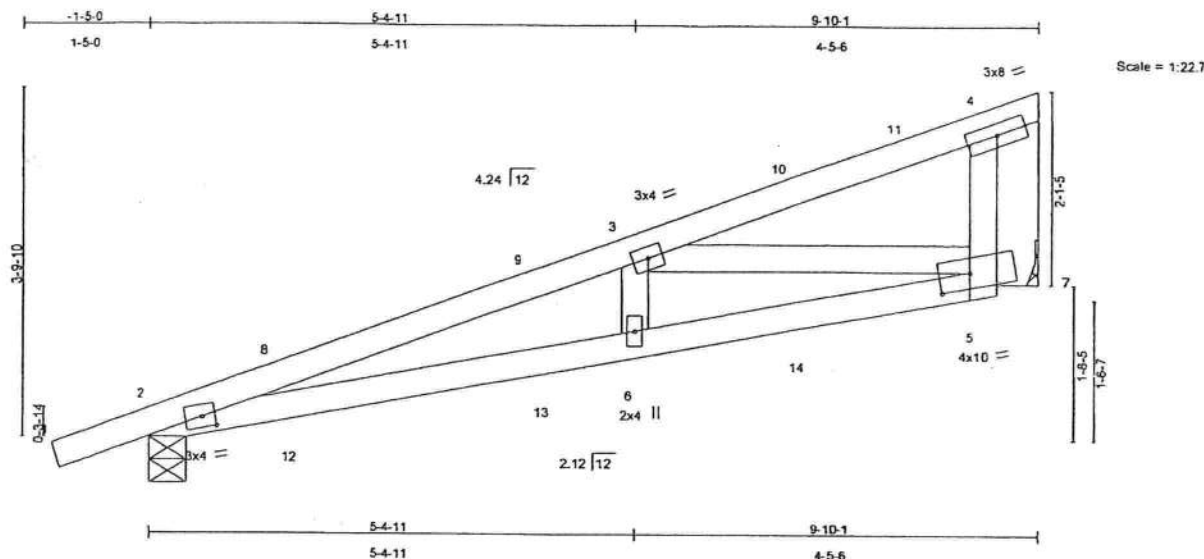


Plate Offsets (X,Y): [2-0-1-12-0-1-8], [5-0-4-0-0-2-0]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		in (loc) l/defl L/d		PLATES GRIP	
TCLL 18.0		Plates Increase 1.25		TC 0.52		Vert(LL) 0.07 2-6		>999 240		MT20 244/190	
TCDL 7.0		Lumber Increase 1.25		BC 0.46		Vert(TL) -0.12 2-6		>938 180			
BCLL 10.0		Rep Stress Incr NO		WB 0.32		Horz(TL) 0.03 7		n/a n/a			
BCDL 10.0		Code FBC2004/TPI2002		(Matrix)						Weight: 44 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-6-1 oc bracing.
WEBS 2 X 4 SYP No.3 "Except"	
4-5 2 X 6 SYP No.2	

REACTIONS (lb/size) 2=521/0-4-15, 7=541/Mechanical
Max Horz 2=219(LC 3)
Max Uplift 2=251(LC 3), 7=373(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/14, 2-8=1530/820, 8-9=1458/813, 3-9=1458/813, 3-10=364/279, 10-11=298/176, 4-11=273/182, 4-5=220/488
BOT CHORD 2-12=911/1412, 12-13=903/1414, 6-13=902/1437, 6-14=916/1404, 5-14=909/1437
WEBS 3-6=0/267, 3-5=1096/851, 4-7=579/418, 5-7=207/207

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 373 lb uplift at joint 7.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 6 lb down and 71 lb up at 4-4-12, 6 lb down and 71 lb up at 4-4-12, 57 lb down and 168 lb up at 7-2-11, 57 lb down and 168 lb up at 7-2-11, and 59 lb down at 1-6-12, and 59 lb down at 1-6-12 on top chord, and 21 lb up at 1-6-12, 21 lb up at 1-6-12, 16 lb down at 4-4-12, 16 lb down at 4-4-12, and 56 lb down at 7-2-11, and 56 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

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October 25, 2007

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ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478943
GEOWASHINGTON	JSGRD1	MONO SCISSOR	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MiTek Industries, Inc. Thu Oct 25 08:56:34 2007 Page 2

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Concentrated Loads (lb)

Vert: 9=-12(F=-6, B=-6) 10=-114(F=-57, B=-57) 12=42(F=21, B=21) 13=-32(F=-16, B=-16) 14=-112(F=-56, B=-56)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSI-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

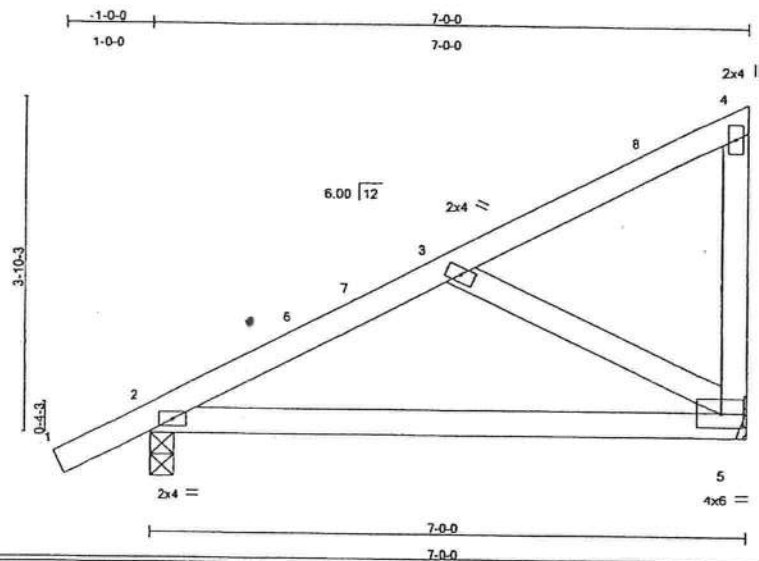
ENGINEERING BY
TRENCO
A MITEK Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478928
GEOWASHINGTON	J	MONO TRUSS	10	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:28 2007 Page 1



Scale: 1/2"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.11	2-5	>757	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	-0.27	2-5	>303	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.09	Horz(TL)	-0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 34 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=349/0-3-8, 5=283/Mechanical
Max Horz 2=240(LC 6)
Max Uplift 2=181(LC 6), 5=157(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-6=220/130, 6-7=181/136, 3-7=155/142, 3-8=72/0, 4-8=63/21
BOT CHORD 2-5=331/165
WEBS 4-5=62/133, 3-5=186/375

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 2-7-5, Exterior(2) 2-7-5 to 6-10-4 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2 and 157 lb uplift at joint 5.

LOAD CASE(S) Standard

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WARNING - Verify design parameters and READ NOTES on THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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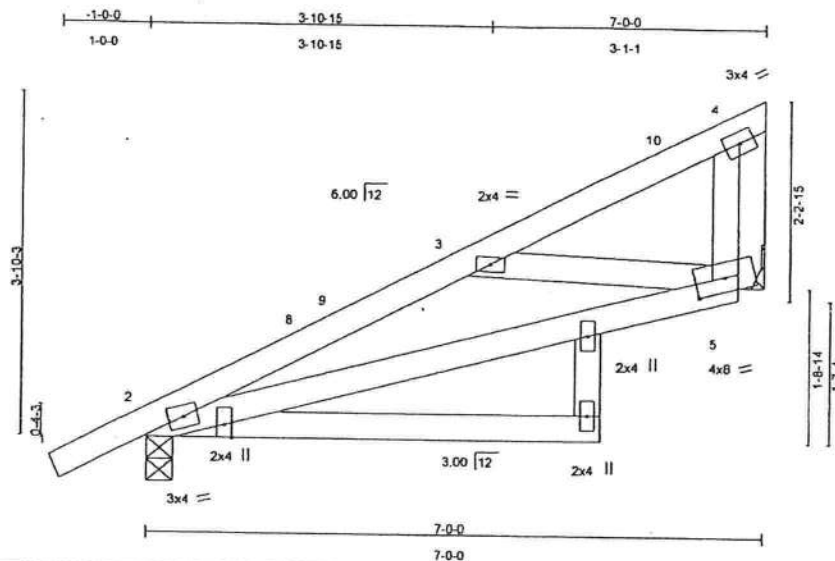
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	City	Ply	GEOWASHINGTON_FLORIDA_125	E4478942
GEOWASHINGTON	JSA	MONO SCISSOR	4	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:33 2007 Page 1



Scale = 1/23.4

Plate Offsets (X,Y): [5-0-4-0-0-1-15]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.09	2-5	>839	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.53	Vert(TL)	-0.23	2-5	>336	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.14	Horz(TL)	-0.01	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 42 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purfins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=337/0-3-8, 5=271/Mechanical
Max Horz 2=233(LC 6)
Max Uplift 2=176(LC 6), 5=156(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/20, 2-8=332/396, 8-9=293/396, 3-9=285/403, 3-10=63/0, 4-10=54/18
BOT CHORD 2-5=575/293
WEBS 3-5=260/552, 4-5=54/117

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 2-3-13, Exterior(2) 2-3-13 to 6-6-12 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 2 and 156 lb uplift at joint 5.

LOAD CASE(S) Standard

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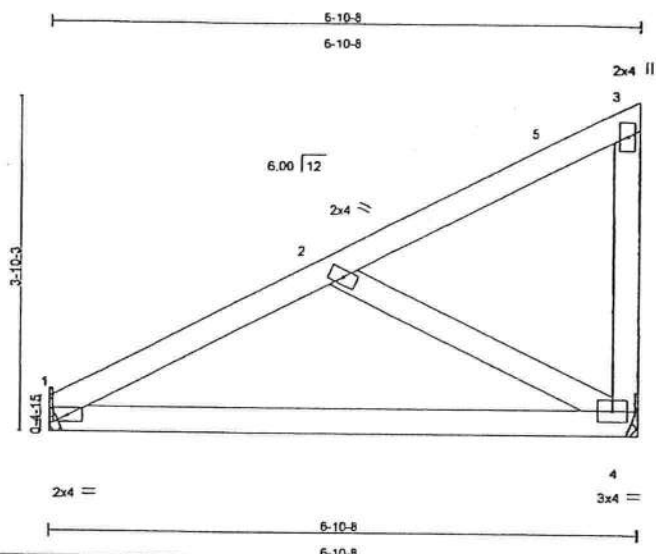
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	City	Ply	GEOWASHINGTON_FLORIDA_125	E4478934
GEOWASHINGTON	JA	MONO TRUSS	10	1		

Maronda Homes Inc., Sanford, FL

7,010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:30 2007 Page 1



Scale: 1/2"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.21	Vert(LL)	-0.10	1-4	>775	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.45	Vert(TL)	-0.26	1-4	>310	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.11	Horz(TL)	-0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 32 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-8-5 oc bracing.

REACTIONS (lb/size) 1=286/Mechanical, 4=286/Mechanical
Max Horz 1=200(LC 6)
Max Uplift 1=79(LC 6), 4=168(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=225/188, 2-5=720, 3-5=61/21
BOT CHORD 1-4=397/181
WEBS 3-4=62/131, 2-4=202/445

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 1 and 168 lb uplift at joint 4.
- 6) Attach with (2) 16d (0.162" x 3.5") toe-Nails at Joints 3 and 4.

LOAD CASE(S) Standard

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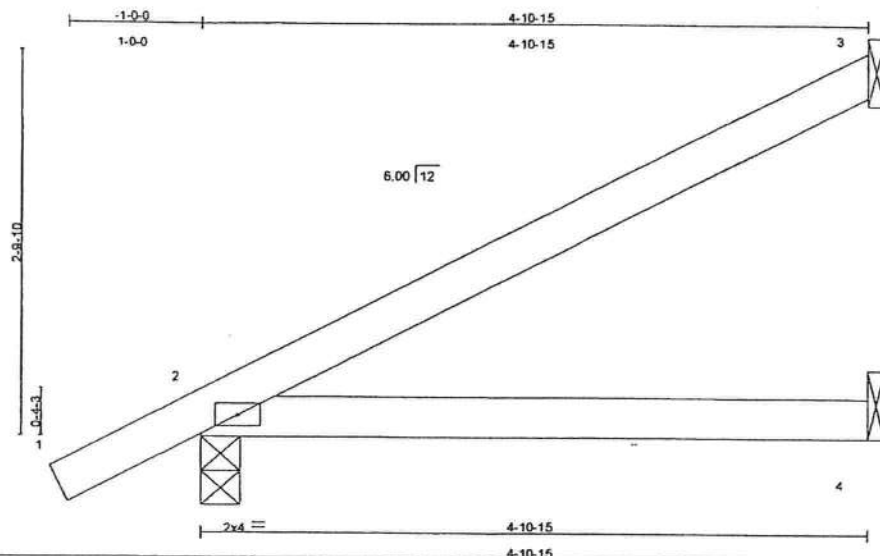
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818 Soundside Road
Edenton, NC 27932

Job GEOWASHINGTON	Truss J1	Truss Type JACK	Qty 6	Ply 1	GEOWASHINGTON_FLORIDA_125	E4478929
Maronda Homes Inc., Sanford, FL			Job Reference (optional)			

7.010 s Oct 16 2007 MTEK Industries, Inc. Thu Oct 25 08:56:28 2007 Page 1



Scale = 1:15.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.36	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.21	Vert(TL)	-0.06	2-4	>879	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
										Weight: 17 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=101/Mechanical, 2=265/0-3-8, 4=94/Mechanical
Max Horz 2=188(LC 6)
Max Uplift 3=145(LC 6), 2=158(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=125/37
BOT CHORD 2-4=0/0

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 3 and 158 lb uplift at joint 2.
- 6) Attach with (2) 16d (0.162" x 3.5") toe-Nails at joints 3 and 4.

LOAD CASE(S) Standard

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October 25,2007

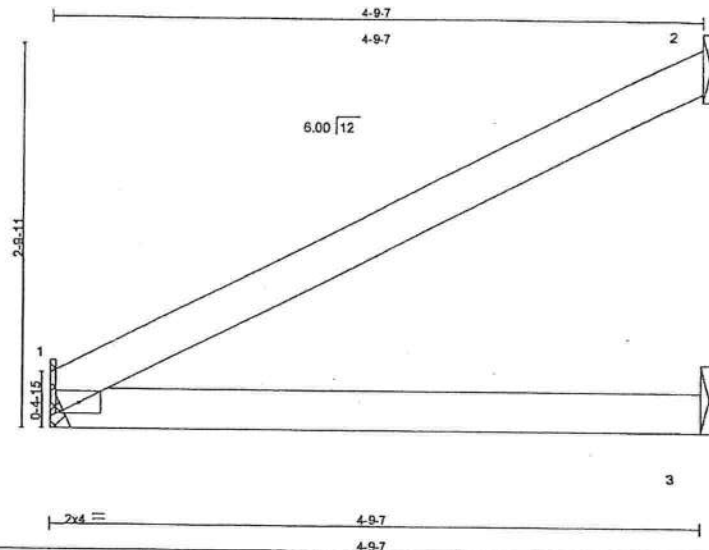
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ENGINEERING BY
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A Mitek Affiliate
818 Soundside Road
Edenton, NC 27832

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478930
GEOWASHINGTON	J1A	JACK	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 18 2007 Mitek Industries, Inc. Thu Oct 25 08:58:28 2007 Page 1



Scale = 1:15.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.41	Vert(LL)	-0.02	1-3	>999	240	
TCDL 7.0	Lumber Increase	1.25	BC 0.21	Vert(TL)	-0.06	1-3	>909	180	
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 15 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-9-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=200/Mechanical, 2=107/Mechanical, 3=93/Mechanical
Max Horz 1=147(LC 6)
Max Uplift 1=52(LC 6), 2=158(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=132/40
BOT CHORD 1-3=0/0

NOTES (7)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1 and 158 lb uplift at joint 2.
- 7) Attach with (2) 16d (0.162" x 3.5") toe-Nails at joints 2 and 3.

LOAD CASE(S) Standard

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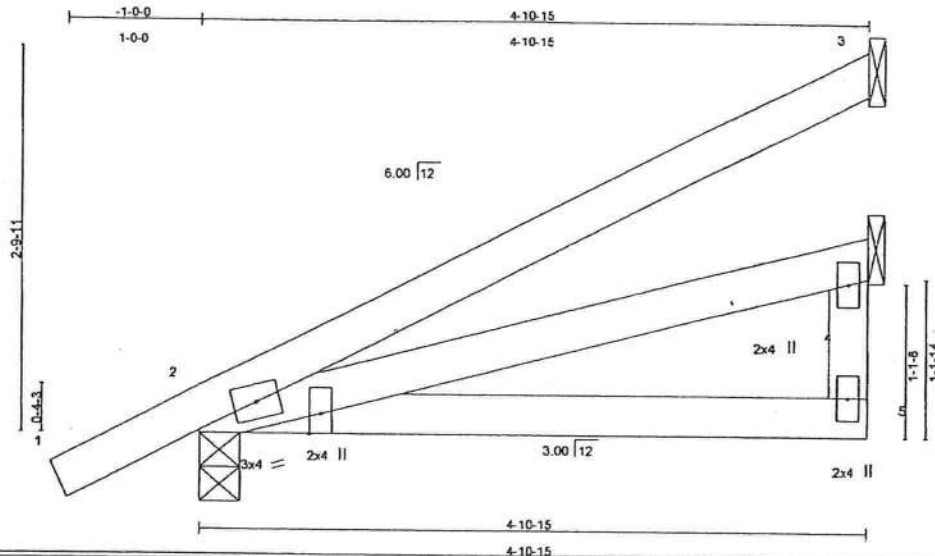
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ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4476938
GEOWASHINGTON	US1A	MONO SCISSOR	2	1	Job Reference (optional)	
Maronde Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:32 2007 Page 1



Scale = 1:15.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.36	Vert(LL)	-0.03	2-4	>999	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.07	2-4	>787	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrx)						
									Weight: 26 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=101/Mechanical, 2=263/0-3-8, 4=92/Mechanical
Max Horz 2=187(LC 6)
Max Uplift 3=146(LC 6), 2=157(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/20, 2-3=128/37
BOT CHORD 2-4=18/18
WEBS 4-5=0/0

NOTES (7)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 3 and 157 lb uplift at joint 2.
- 7) Attach with (2) 16d (0.162" x 3.5") toe-Nails at joints 3 and 4.

LOAD CASE(S) Standard

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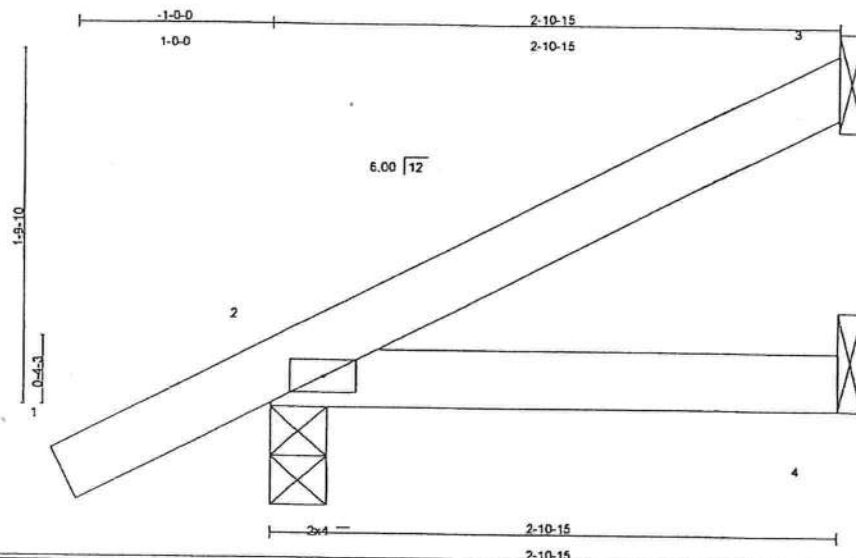
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A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_123	E4478931
GEOWASHINGTON	J2	JACK	6	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Thu Oct 25 08:56:29 2007 Page 1



Scale = 1:10.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 15.0	Plates Increase	1.25	TC 0.12	Vert(LL)	-0.00	2-4	>999	240	
TCDL 7.0	Lumber Increase	1.25	BC 0.07	Vert(TL)	-0.01	2-4	>999	180	
BCCL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
								Weight: 11 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=50/Mechanical, 2=184/0-3-8, 4=54/Mechanical
Max Horz 2=128(LC 6)
Max Uplift 3=69(LC 6), 2=144(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=65/18
BOT CHORD 2-4=0/0

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 3 and 144 lb uplift at joint 2.
- 6) Attach with (2) 16d (0.162" x 3.5") toe-Nails at joints 3 and 4.

LOAD CASE(S) Standard

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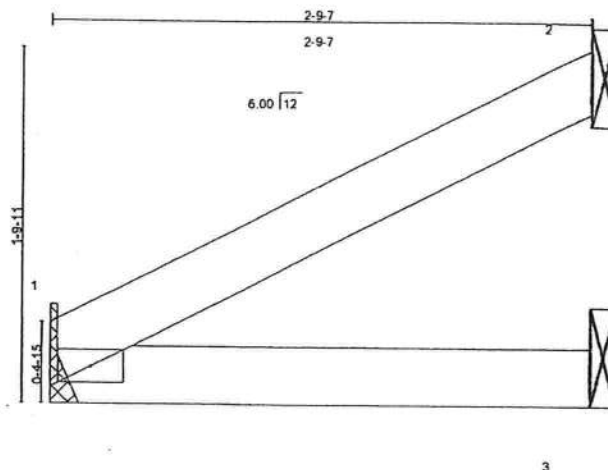
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
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ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478032
GEOWASHINGTON	J2A	JACK	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Thu Oct 25 08:56:29 2007 Page 1



Scale = 1:10.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.00	1-3	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.07	Vert(TL)	-0.01	1-3	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 9 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-9-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=114/Mechanical, 2=61/Mechanical, 3=53/Mechanical
Max Horz 1=87(LC 6)
Max Uplift 1=28(LC 6), 2=92(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=79/24
BOT CHORD 1-3=0/0

NOTES (7)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1 and 92 lb uplift at joint 2.
- 7) Attach with (2) 16d (0.162" x 3.5") toe-Nails at joints 2 and 3.

LOAD CASE(S) Standard

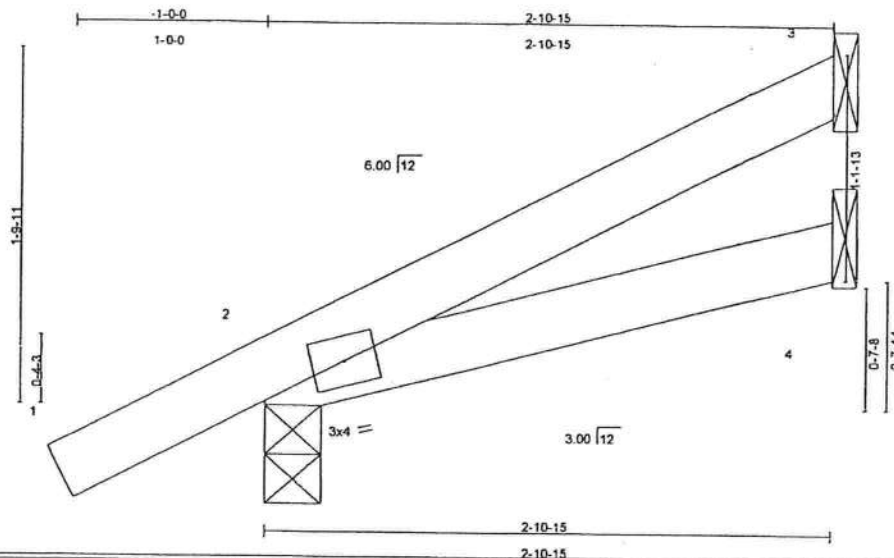
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October 25,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MU-7473 BEFORE USE.
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ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27832

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478639
GEOWASHINGTON	JS2	MONO SCISSOR	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:32 2007 Page 1



Scale = 1:10.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.12	Vent(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.07	Vent(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 11 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=50/Mechanical, 2=184/0-3-8, 4=54/Mechanical
Max Horz 2=128(LC 6)
Max Uplift 3=70(LC 6), 2=143(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/20, 2-3=67/18
BOT CHORD 2-4=10/10

NOTES (7)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 3 and 143 lb uplift at joint 2.
- 7) Attach with (2) 16d (0.162" x 3.5") toe-Nails at joints 3 and 4.

LOAD CASE(S) Standard

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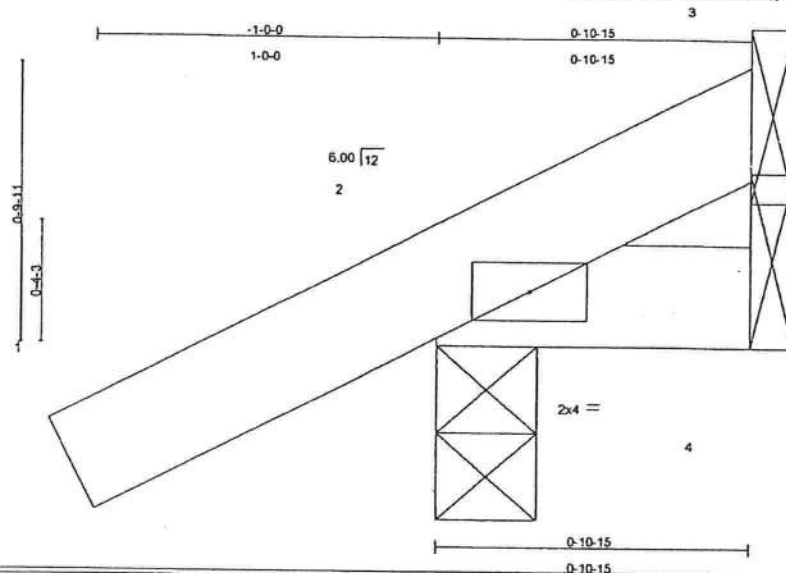
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenboro, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478933
GEOWASHINGTON	J3	JACK	10	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Thu Oct 25 08:58:30 2007 Page 1



Scale = 1:6.0

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.10	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 5 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 0-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=119/0-3-8, 4=17/Mechanical, 3=12/Mechanical
Max Horz 2=70(LC 6)
Max Uplift 2=154(LC 6), 3=12(LC 1)
Max Grav 2=119(LC 1), 4=17(LC 1), 3=33(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=25/16
BOT CHORD 2-4=0/0

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 2 and 12 lb uplift at joint 3.
- 6) Attach with (2) 16d (0.162" x 3.5") toe-Nails at joints 3 and 4.

LOAD CASE(S) Standard

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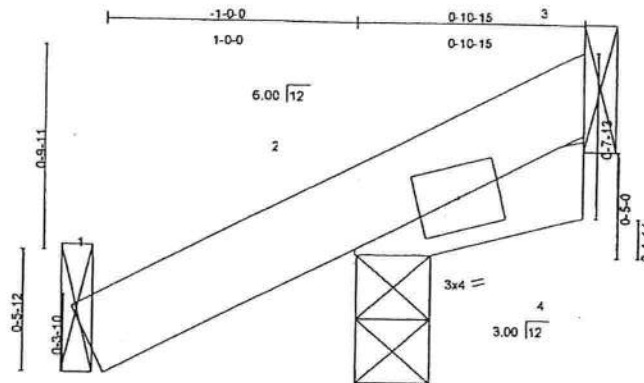
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.
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ENGINEERING BY
TRENCO
A MITEK Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478941
GEOWASHINGTON	JS3	MONO SCISSOR	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 M/Tek Industries, Inc. Thu Oct 25 08:56:33 2007 Page 1



Scale = 1:8.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.08	Ver(LL)	-0.00	2	n/r	120	MT20	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.03	Ver(TL)	-0.00	4	n/r	120		
BCCL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	1	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 5 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 0-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=2/Mechanical, 2=143/0-3-8, 2=143/0-3-8
Max Horz 2=52(LC 7)
Max Uplift 1=5(LC 2), 2=75(LC 7)
Max Grav 1=16(LC 7), 2=143(LC 1), 2=143(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=5/24, 2-3=22/0
BOT CHORD 2-4=9/0

NOTES (8)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCCL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 1 and 75 lb uplift at joint 2.
- 7) Non Standard bearing condition. Review required.
- 8) Attach with (2) 16d (0.162" x 3.5") toe-nails at joints 3 and 4.

LOAD CASE(S) Standard

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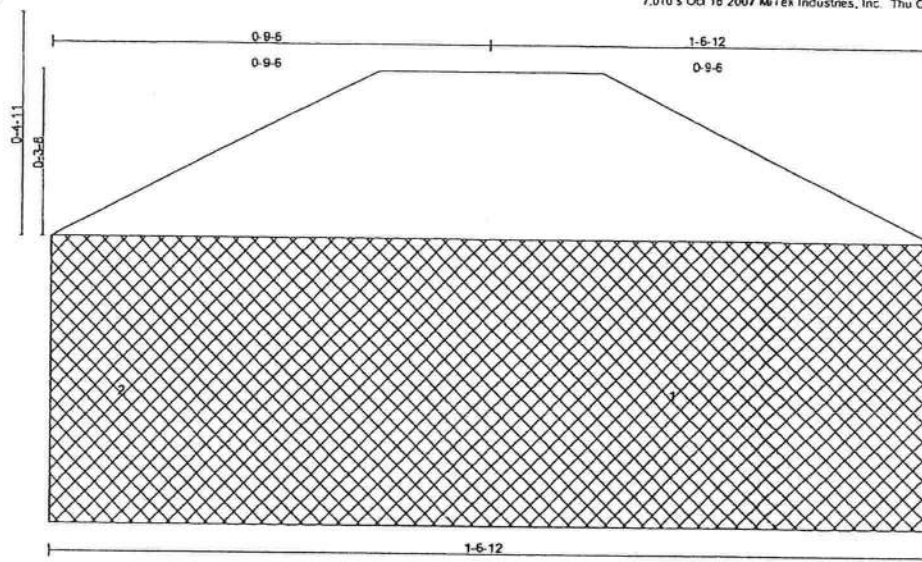
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478946
GEOWASHINGTON	V1	VALLEY	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7,010 s Oct 18 2007 MTEK Industries, Inc. Thu Oct 25 08:55:35 2007 Page 1



Scale = 1:3.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	Vdefl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.00	TC 0.00	Vert(LL)	n/a	-	n/a	999		
TCDL 7.0	Lumber Increase	1.00	BC 0.01	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00		n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 2 lb	

LUMBER

BOT CHORD 2 X 4 SYP No.2

BRACING

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=20/1-6-12, 1=20/1-6-12

FORCES (lb) - Maximum Compression/Maximum Tension

BOT CHORD 1-2=0/0

NOTES

- 1) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 2) Gable requires continuous bottom chord bearing.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

LOAD CASE(S) Standard

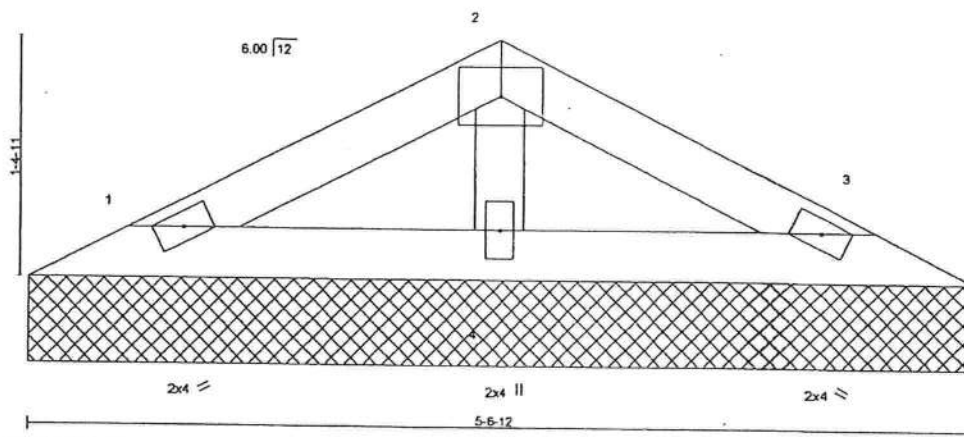
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ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478947
GEOWASHINGTON	V2	VALLEY	1	1	Job Reference (optional)	
Maronde Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:38 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.03	Horz(TL)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 17 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=88/5-6-12, 3=88/5-6-12, 4=197/5-6-12
Max Horz 1=20(LC 4)
Max Uplift 1=58(LC 6), 3=58(LC 6), 4=54(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=30/44, 2-3=30/44
BOT CHORD 1-4=1/12, 3-4=1/12
WEBS 2-4=89/137

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) automatic zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 58 lb uplift at joint 1, 58 lb uplift at joint 3 and 54 lb uplift at joint 4.

LOAD CASE(S) Standard

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ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MITek Industries, Inc. Thu Oct 25 08:56:35 2007 Page 1



Weight: 31 lb

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

Max Horz 1=39(LC 5)
Max Uplift 1=-76(LC 6), 3=-81(LC 7), 4=-151(LC 6)
Max Grav 1=153(LC 10), 3=153(LC 11), 4=418(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-5=58/68, 2-5=3/77, 2-6=3/77, 3-6=58/68
BOT CHORD 1-4=4/29, 3-4=4/29
WEBS 2-4=204/258

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf, BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 1, 81 lb uplift at joint 3 and 151 lb uplift at joint 4.

LOAD CASE(S) Standard

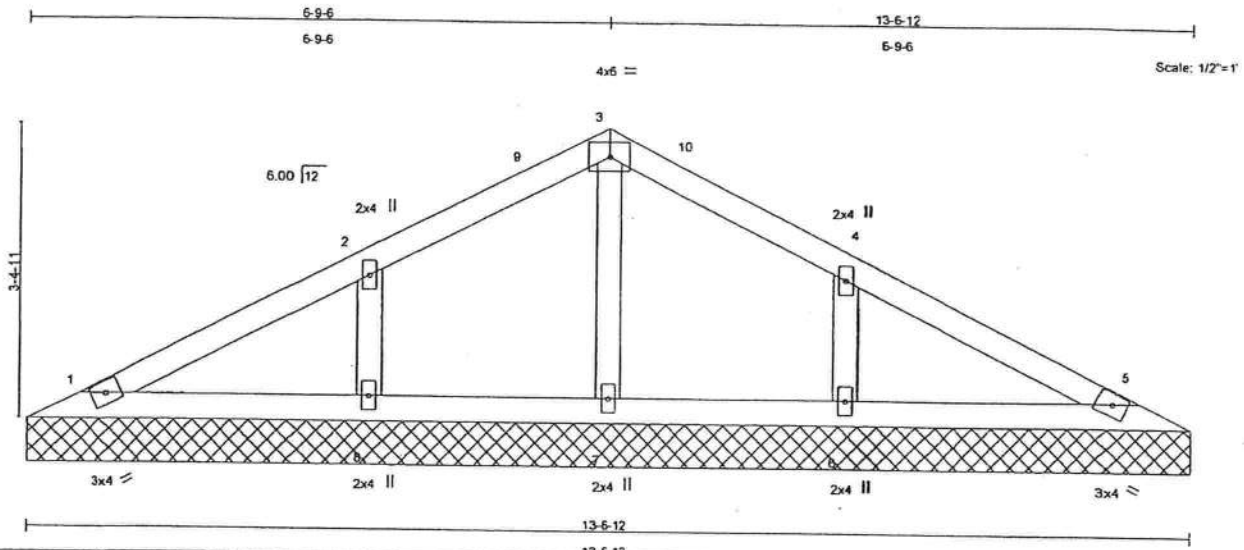
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ENGINEERING BY
TRENCO
A Miltek Affiliat

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON, FLORIDA, 125	E4478948
GEOWASHINGTON	V4	VALLEY	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						7.010 s Oct 16 2007 MITek Industries, Inc. Thu Oct 25 08:56:37 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.07	Horz(TL)	0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
										Weight: 49 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=123/13-6-12, 5=123/13-6-12, 7=183/13-6-12, 6=316/13-6-12, 8=316/13-6-12
Max Horz 1=58(LC 5)
Max Uplift 1=38(LC 7), 5=44(LC 7), 6=197(LC 7), 8=197(LC 6)
Max Grav 1=123(LC 1), 5=123(LC 1), 7=183(LC 1), 6=319(LC 11), 8=319(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-44/41, 2-9=-47/124, 3-9=-5/133, 3-10=-47/124, 4-5=-35/32
BOT CHORD 1-8=0/59, 7-8=0/59, 6-7=0/59, 5-6=0/59
WEBS 3-7=-94/41, 4-6=-170/265, 2-8=-170/265

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 44 lb uplift at joint 5, 197 lb uplift at joint 6 and 197 lb uplift at joint 8.

LOAD CASE(S) Standard

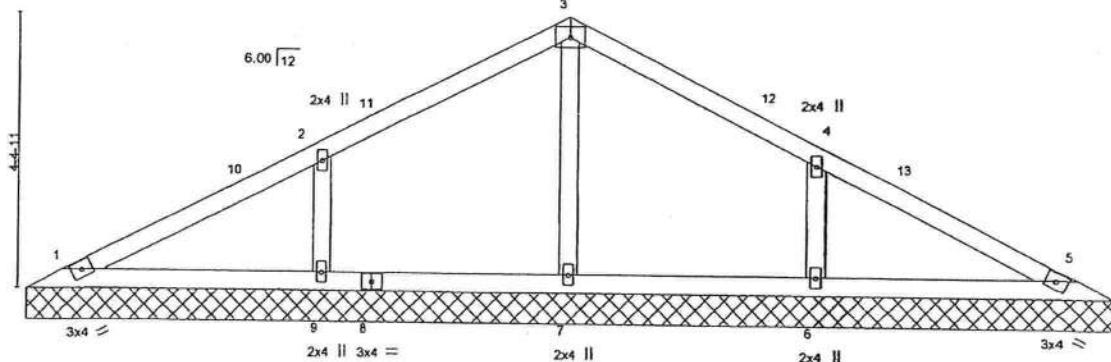
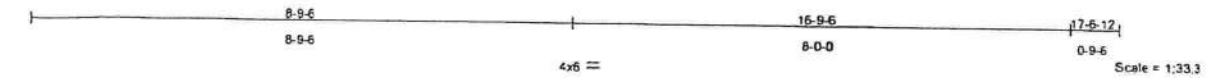
This document was originally issued by Lassiter, Frank on October 25, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above. October 25, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4476950
GEOWASHINGTON	V5	VALLEY	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 & Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:38 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.15	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.07	Horz(TL)	0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
										Weight: 64 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=151/17-6-12, 5=151/17-6-12, 7=294/17-6-12, 6=404/17-6-12, 9=404/17-6-12
Max Horz 1=77(LC 5)
Max Uplift 1=48(LC 6), 5=57(LC 7), 6=230(LC 7), 9=230(LC 6)
Max Grav 1=151(LC 1), 5=151(LC 1), 7=294(LC 1), 6=411(LC 11), 9=411(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-10=60/47, 2-10=46/54, 2-11=66/133, 3-11=21/147, 3-12=21/147, 4-12=66/133, 4-13=14/37, 5-13=48/15
BOT CHORD 1-9=0/70, 8-9=0/70, 7-8=0/70, 6-7=0/70, 5-6=0/70
WEBS 3-7=150/95, 4-6=220/269, 2-9=220/269

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-7-7 to 3-7-7, Interior(1) 3-7-7 to 5-9-6, Exterior(2) 5-9-6 to 8-9-6, Interior(1) 11-9-6 to 13-11-6 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1, 57 lb uplift at joint 5, 230 lb uplift at joint 6 and 230 lb uplift at joint 9.

LOAD CASE(S) Standard

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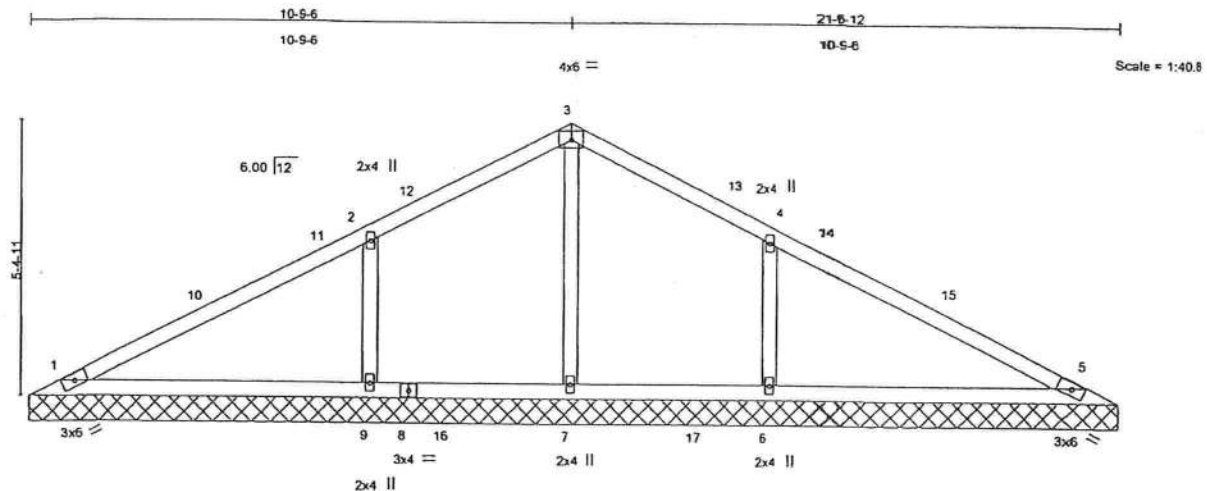
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
816 Soundside Road
Eden, NC 27632

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4476951
GEOWASHINGTON	V6	VALLEY	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:38 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	U/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.08	Horz(TL)	0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 82 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 8-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=219/21-6-12, 5=219/21-6-12, 7=347/21-6-12, 9=584/21-6-12, 6=584/21-6-12
Max Horz 1=95(LC 5)
Max Uplift 1=80(LC 6), 5=89(LC 7), 9=288(LC 6), 6=288(LC 7)
Max Grav 1=219(LC 1), 5=219(LC 1), 7=347(LC 1), 9=585(LC 10), 6=585(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-10=72/56, 10-11=60/57, 2-11=51/68, 2-12=70/169, 3-12=50/182, 3-13=50/182, 4-13=70/169, 4-14=11/65,
14-15=21/17, 5-15=54/16
BOT CHORD 1-9=0/95, 8-9=0/95, 8-16=0/95, 7-16=0/95, 7-17=0/95, 6-17=0/95, 5-6=0/95
WEBS 3-7=100/22, 2-9=295/332, 4-6=295/332

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-7-7 to 3-7-7, Interior(1) 3-7-7 to 7-9-6, Exterior(2) 7-9-6 to 10-9-6, Interior(1) 13-9-6 to 17-11-6 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 1, 89 lb uplift at joint 5, 288 lb uplift at joint 9 and 288 lb uplift at joint 6.

LOAD CASE(S) Standard

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October 25, 2007

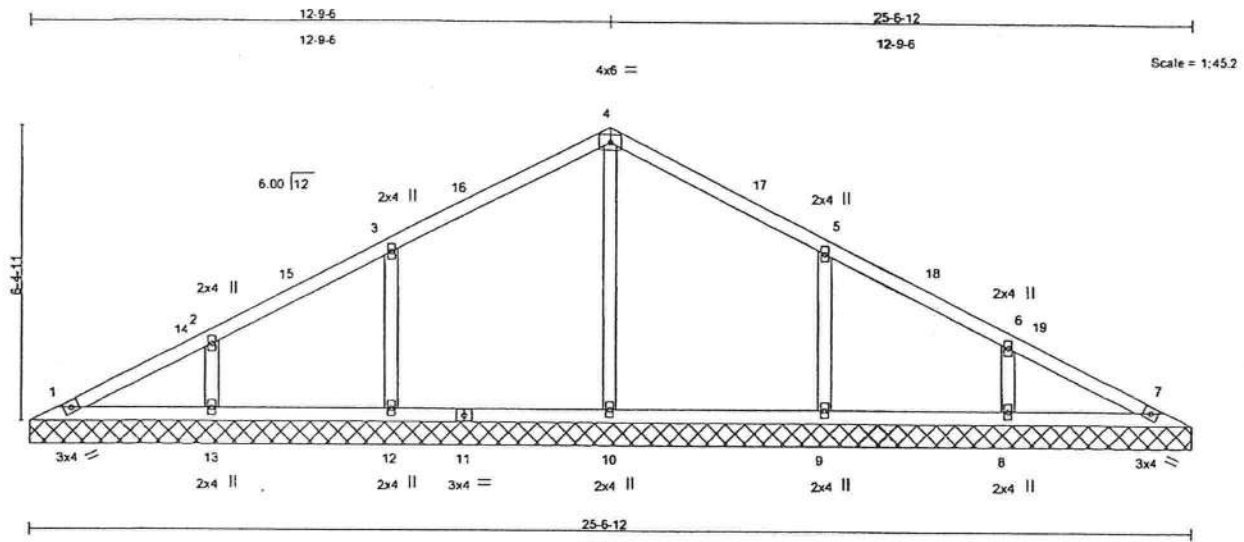


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
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ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEOWASHINGTON_FLORIDA_125	E4478952
GEOWASHINGTON	V7	VALLEY	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						
7.010 s Oct 16 2007 Mitek Industries, Inc. Thu Oct 25 08:56:39 2007 Page 1						



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.18	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.24	Vert(TL)	n/a	-	n/a		
BCLL 10.0	Lumber Increase 1.25	WB 0.13	Horz(TL)	0.00	7	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TP12002						Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purtins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=128/25-6-12, 7=128/25-6-12, 10=616/25-6-12, 8=328/25-6-12, 9=473/25-6-12, 13=328/25-6-12, 12=473/25-6-12
Max Horz 1=114(LC 4)
Max Uplift 1=18(LC 4), 7=23(LC 7), 8=213(LC 7), 9=142(LC 7), 13=212(LC 6), 12=143(LC 6)
Max Grav 1=128(LC 1), 7=128(LC 1), 10=616(LC 1), 8=328(LC 1), 9=484(LC 1), 13=328(LC 1), 12=484(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-14=114/57, 2-14=102/58, 2-15=68/89, 3-15=27/97, 3-16=76/194, 4-16=29/206, 4-17=29/206, 5-17=76/194,
5-18=19/82, 6-18=68/69, 6-19=55/23, 7-19=66/9
BOT CHORD 1-13=0/96, 12-13=0/96, 11-12=0/96, 10-11=0/96, 9-10=0/96, 8-9=0/96, 7-8=0/96
WEBS 4-10=202/105, 6-8=183/244, 5-9=213/236, 2-13=183/244, 3-12=213/235

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-7-7 to 3-7-7, Interior(1) 3-7-7 to 9-9-6, Exterior(2) 9-9-6 to 12-9-6, Interior(1) 15-9-6 to 21-11-6 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1, 23 lb uplift at joint 7, 213 lb uplift at joint 8, 142 lb uplift at joint 9, 212 lb uplift at joint 13 and 143 lb uplift at joint 12.

LOAD CASE(S) Standard

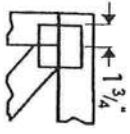
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October 25, 2007

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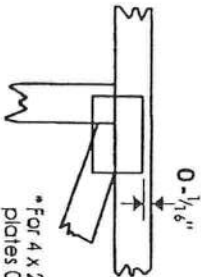
ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27832

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



* For 4 x 2 orientation, locate plates 0 1/4" from outside edge of truss.



* This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

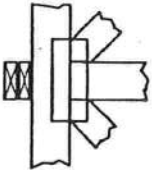
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use L, I or Eliminator bracing if indicated.

BEARING



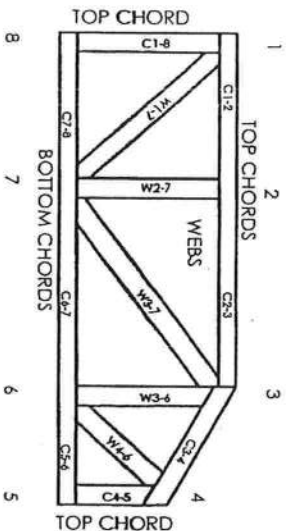
Indicates location where bearings (support) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information.
Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Mitek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative L, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft, spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

RE: ELEV_F - GEORGE WASHINGTON FL

Trenco

818 Soundside Rd
Edenton, NC 27932

Site Information:

Project Customer: MARONDA HOMES Project Name: GEORGE WASHINGTON
Lot/Block: SANFORD Subdivision: SANFORD
Address: SANFORD
City: SANFORD State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 7.0
Wind Code: N/A Wind Speed: N/A mph Floor Load: 55.0 psf
Roof Load: N/A psf

This package includes 7 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	E4261208	FA	7/26/07
2	E4261209	FB	7/26/07
3	E4261210	FC	7/26/07
4	E4261211	FD	7/26/07
5	E4261212	FE	7/26/07
6	E4261213	FF	7/26/07
7	E4261214	FG	7/26/07

The truss drawing(s) referenced above have been prepared by
TRENCO under my direct supervision based on the parameters
provided by Maronda Homes-Pittsburgh, PA.

Truss Design Engineer's Name: Strzyzewski, Marvin
My license renewal date for the state of Florida is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of
professional engineering responsibility solely for the truss
components shown. The suitability and use of this component
for any particular building is the responsibility of the building
designer, per ANSI/TPI-2002 Chapter 2.

Marvin A. Strzyzewski, P.E. #43144
Truss Engineering Company
818 Soundside Rd.
Edenton, NC 27932
FL Cert. #7239

Job	Truss	Truss Type	Qty	Ply	GEORGE WASHINGTON FL	E4261208
ELEV_F	FA	FLOOR	12	1	ELEVATION F Job Reference (optional)	
Maronda Homes, Inc, Pittsburgh, PA			7.000 s May 29 2007 Mitek Industries, Inc. Wed Jul 25 09:40:44 2007 Page 1			

0-1-8
 1-1-3-0
 1-2-4
 1-2-0
 1-2-0
 0-10-4
 2-0-0
 1-2-0
 0-1-8
 Scale = 1:56.0

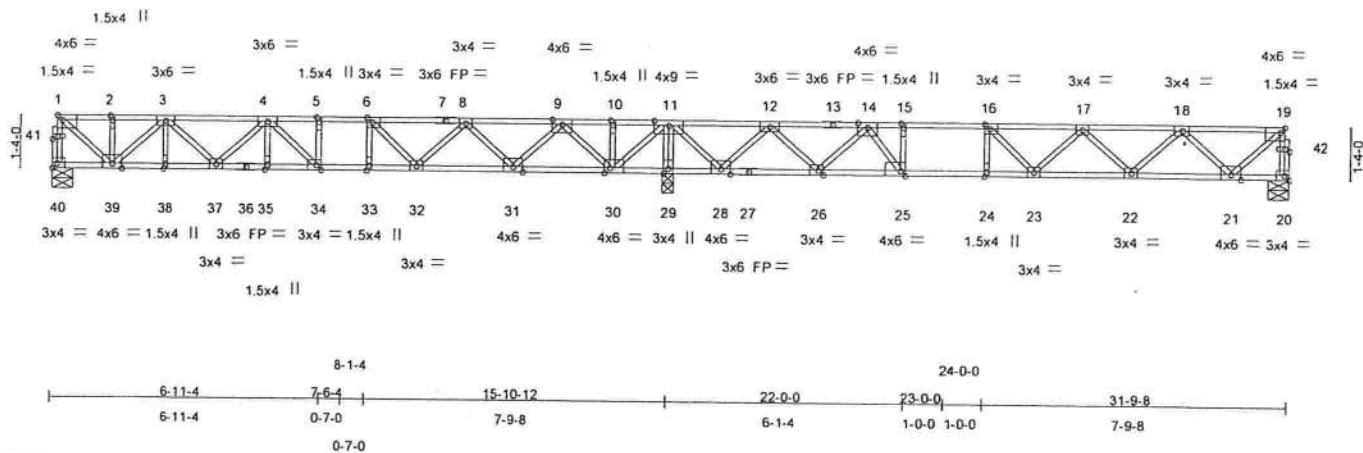


Plate Offsets (X,Y): [1:Edge,0-1-8], [6:0-1-8,Edge], [16:0-1-8,Edge], [19:0-1-8,Edge], [25:0-1-8,Edge], [26:0-1-8,Edge], [30:0-1-12,Edge], [34:0-1-8,Edge], [41:0-1-8,0-0-12], [42:0-1-8,0-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.99	Vert(LL)	-0.18 23-24	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.95	Vert(TL)	-0.28 23-24	>672	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.56	Horz(TL)	0.04 20	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 170 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2 "Except"
 20-27 4 X 2 SYP No.1
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS

(lb/size) 40=707/0-6-8, 20=707/0-6-8, 29=2044/0-3-8
 Max Grav40=755(LC 2), 20=771(LC 3), 29=2044(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 40-41=-748/0, 1-41=-747/0, 20-42=-767/0, 19-42=-766/0, 1-2=-818/0, 2-3=-818/0, 3-4=-1786/0, 4-5=-2156/0, 5-6=-2156/0, 6-7=-1898/0, 7-8=-1898/0, 8-9=-1081/294, 9-10=0/901, 10-11=0/901, 11-12=0/1132, 12-13=-1159/493, 13-14=-1159/493, 14-15=-2130/3, 15-16=-2130/3, 16-17=-2210/0, 17-18=-1792/0, 18-19=-758/0
 BOT CHORD 39-40=0/39, 38-39=0/1443, 37-38=0/1443, 36-37=0/2113, 35-36=0/2113, 34-35=0/2113, 33-34=0/2156, 32-33=0/2156, 31-32=-99/1625, 30-31=-521/508, 29-30=-1965/0, 28-29=-1965/0, 27-28=-726/630, 26-27=-726/630, 25-26=-238/1732, 24-25=-3/2130, 23-24=-3/2130, 22-23=0/2153, 21-22=0/1416, 20-21=0/40
 WEBS 5-34=-83/58, 6-33=-44/194, 11-29=-2015/0, 15-25=-496/0, 16-24=-314/0, 1-39=0/1038, 2-39=-124/0, 3-39=-832/0, 3-38=0/16, 3-37=0/466, 4-37=-445/0, 4-35=-12/97, 4-34=-335/221, 11-30=0/1391, 10-30=-107/27, 9-30=-1207/0, 9-31=0/865, 8-31=-819/0, 8-32=0/503, 6-32=-580/0, 11-28=0/1339, 12-28=-1227/0, 12-26=0/822, 14-26=-912/0, 14-25=0/956, 19-21=0/976, 18-21=-916/0, 18-22=0/522, 17-22=-503/0, 17-23=-123/90, 16-23=0/416

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 4) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Mario Stappardi

July 26, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8CSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job ELEV_F	Truss FB	Truss Type FLOOR	Qty 1	Ply 1	GEORGE WASHINGTON FL ELEVATION F Job Reference (optional)	E4261209
Maronda Homes, Inc, Pittsburgh, PA			7.000 s May 29 2007 Mitek Industries, Inc. Wed Jul 25 09:40:45 2007 Page 1			

0-1-8

1-1-3-0 0-11-6

1-1-4-6 1-1-2-0 1-1-2-0

1-1-12 1-1-2-0 1-1-2-8

0-1-8
Scale = 1:56.0

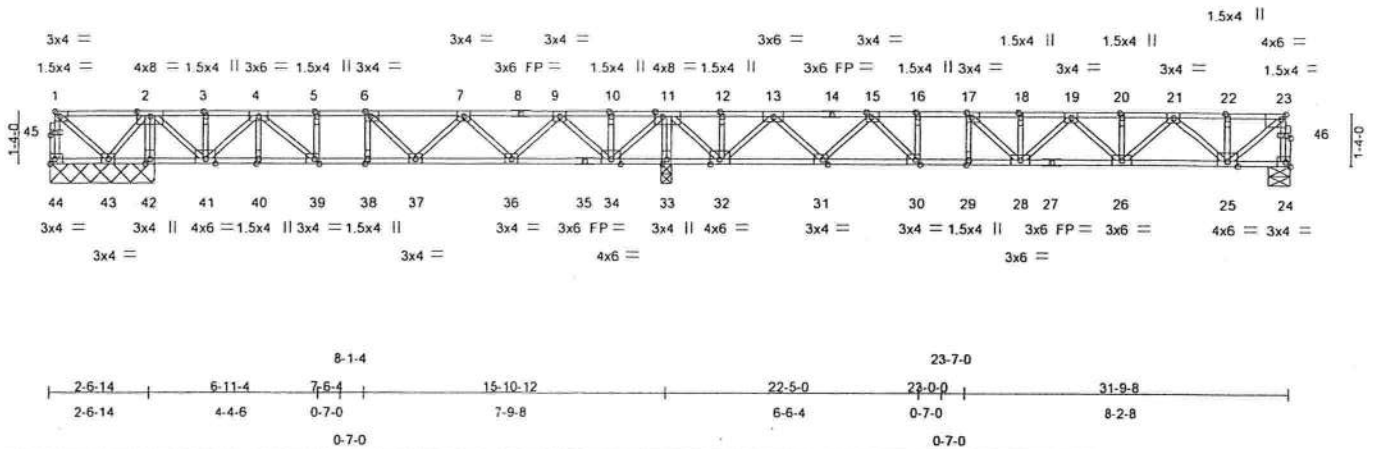


Plate Offsets (X,Y): [6:0-1-8,Edge], [11:0-2-4,Edge], [15:0-1-8,Edge], [17:0-1-8,Edge], [23:0-1-8,Edge], [30:0-1-8,Edge], [31:0-1-8,Edge], [32:0-2-12,Edge], [39:0-1-8,Edge], [45:0-1-8-0-12], [46:0-1-8-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.60	Ver(TL)	-0.13 28-29	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.77	Ver(TL)	-0.20 28-29	>954	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.57	Horz(TL)	0.02 24	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 178 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 44=-160/2-8-6, 24=700/0-6-8, 42=1038/2-8-6, 33=1897/0-3-8, 43=-19/2-8-6
Max Uplift44=-286(LC 5), 43=-69(LC 5)
Max Grav44=79(LC 4), 24=750(LC 4), 42=1198(LC 2), 33=1898(LC 3), 43=51(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 44-45=-73/294, 1-45=-73/293, 24-46=-745/0, 23-46=-744/0, 1-2=0/352, 2-3=-34/30, 3-4=-34/30, 4-5=-986/63, 5-6=-986/63, 6-7=-1017/221, 7-8=-609/570, 8-9=-609/570, 9-10=0/1160, 10-11=0/1160, 11-12=0/963, 12-13=0/963, 13-14=-1014/333, 14-15=-1014/333, 15-16=-1981/0, 16-17=-1981/0, 17-18=-2178/0, 18-19=-2178/0, 19-20=-1798/0, 20-21=-1798/0, 21-22=-820/0, 22-23=-820/0
BOT CHORD 43-44=-15/4, 42-43=-688/0, 41-42=-683/0, 40-41=0/519, 39-40=0/519, 38-39=-63/986, 37-38=-63/986, 36-37=-375/965, 35-36=-798/232, 34-35=-798/232, 33-34=-2056/0, 32-33=-2056/0, 31-32=-555/457, 30-31=-119/1575, 29-30=0/1981, 28-29=0/1981, 27-28=0/2048, 26-27=0/2048, 25-26=0/1378, 24-25=0/38
WEBS 2-42=-1169/0, 5-39=-227/32, 6-38=-139/93, 11-33=-1866/0, 16-30=-360/0, 17-29=-225/0, 1-43=-463/0, 2-43=-16/507, 11-34=0/1180, 10-34=-112/1, 9-34=-969/0, 9-36=0/652, 7-36=-615/0, 7-37=0/311, 6-37=-331/46, 11-32=0/1428, 12-32=-95/8, 13-32=-1209/0, 13-31=0/840, 15-31=-858/0, 15-30=0/796, 23-25=0/1041, 22-25=-151/0, 21-25=-758/0, 21-26=0/571, 20-26=-139/0, 19-26=-341/7, 19-28=-28/176, 18-28=-263/0, 17-28=0/551, 2-41=0/927, 3-41=-146/30, 4-41=-670/0, 4-40=-44/51, 4-39=-123/601

NOTES

- Unbalanced floor live loads have been considered for this design.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 44 and 69 lb uplift at joint 43.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Marion A. Stroup

July 26, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

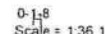
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Maronda Homes, Inc. Pittsburgh, PA

7,000 s May 29 2007 MiTek Industries, Inc. Wed Jul 25 09:40:46 2007 Page 1



LUMBER		BRACING	
TOP CHORD	4 X 2 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-6-2 oc purlins, except end verticals.
BOT CHORD	4 X 2 SYP No.2 *Except* 13-21 4 X 2 SYP No.1D	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 19-20,18-19,17-18.
WEBS	4 X 2 SYP No.3		

REACTIONS (lb/size) 24=1115/0-3-8, 13=1109/0-6-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	24-25=-1108/0, 25-26=-1108/0, 1-26=-1108/0, 13-27=-1104/0, 12-27=-1102/0, 1-2=-1092/0, 2-3=-2834/0, 3-4=-4016/0, 4-5=-4016/0, 5-6=-4587/0, 6-7=-4631/0, 7-8=-4631/0, 8-9=-3995/0, 9-10=-2872/0, 10-11=-2872/0, 11-12=-1141/0
BOT CHORD	23-24=0/0, 22-23=0/2115, 21-22=0/3517, 20-21=0/3517, 19-20=0/4587, 18-19=0/4587, 17-18=0/4587, 16-17=0/4408, 15-16=0/3562, 14-15=0/2154, 13-14=0/057
WEBS	5-19=-61/234, 6-18=-211/93, 1-23=0/1485, 2-23=-1423/0, 2-22=0/999, 3-22=-950/0, 3-20=0/678, 4-20=-168/81, 5-20=-938/0, 12-14=0/1474, 11-14=-1408/0, 11-15=0/999, 9-15=-960/0, 9-16=0/601, 8-16=-574/0, 8-17=0/335, 7-17=-222/22, 6-17=-438/411

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10'-0" oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 4) CAUTION. Do not erect truss backwards.

LOAD CASE(S) Standard

Maria Stroganoff

July 26, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D5B-89 and BC311 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEL & MITEL COMPANY

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEORGE WASHINGTON FL	E4261211
ELEV_F	FD	FLOOR	6	1	ELEVATION F Job Reference (optional)	
Maronda Homes, Inc, Pittsburgh, PA			7.000 s May 29 2007 MiTek Industries, Inc. Wed Jul 25 09:40:47 2007 Page 1			

0-1-8



0-1-8
Scale = 1/28.0

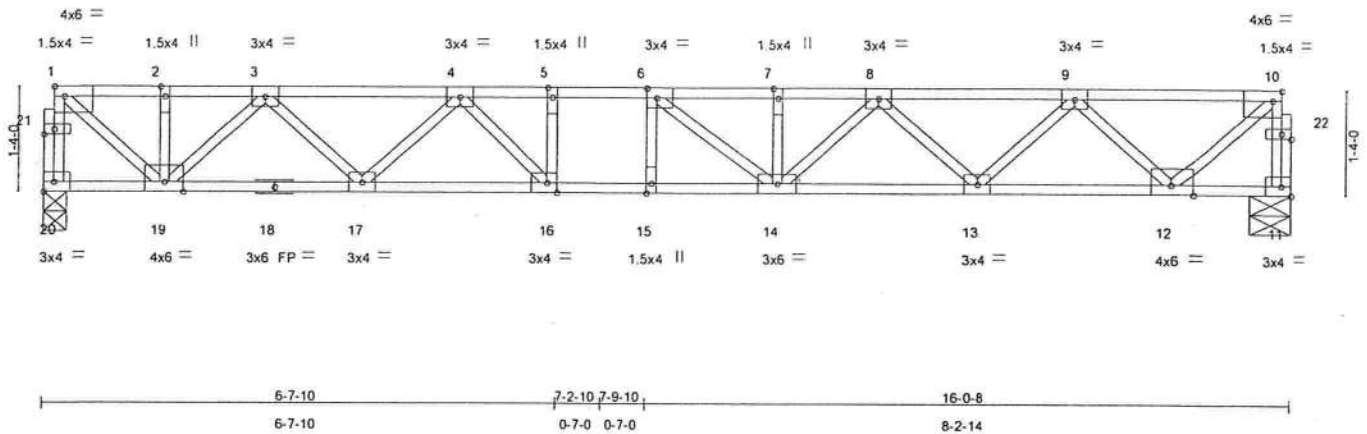


Plate Offsets (X,Y): [1:Edge,0-1-8], [6:0-1-8,Edge], [10:0-1-8,Edge], [16:0-1-8,Edge], [21:0-1-8,0-0-12], [22:0-1-8,0-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.42	Vert(LL)	-0.16 14-15	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.77	Vert(TL)	-0.24 14-15	>784	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.48	Horz(TL)	0.05 11	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 87 lb

LUMBER
TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 20=862/0-3-8, 11=862/0-6-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 20-21=-858/0, 1-21=-857/0, 11-22=-858/0, 10-22=-857/0, 1-2=-949/0, 2-3=-949/0, 3-4=-2107/0, 4-5=-2790/0, 5-6=-2790/0, 6-7=-2758/0, 7-8=-2758/0, 8-9=-2075/0, 9-10=-864/0

BOT CHORD 19-20=0/44, 18-19=0/1658, 17-18=0/1658, 16-17=0/2546, 15-16=0/2790, 14-15=0/2790, 13-14=0/2504, 12-13=0/1623, 11-12=0/44

WEBS 5-16=-250/0, 6-15=-127/59, 1-19=0/1204, 2-19=-121/0, 3-19=-964/0, 3-17=0/625, 4-17=-611/0, 4-16=-4/545, 10-12=0/1115, 9-12=-1055/0, 9-13=0/628, 8-13=-596/0, 8-14=0/346, 7-14=-199/8, 6-14=-352/221

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Mario Stapp

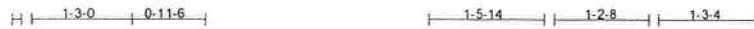
July 26, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEORGE WASHINGTON FL	E4261212
ELEV_F	FE	FLOOR	1	1	ELEVATION F Job Reference (optional)	
Maronda Homes, Inc, Pittsburgh, PA			7.000 s May 29 2007 MiTek Industries, Inc. Wed Jul 25 09:40:47 2007 Page 1			

0-1-8



Scale = 1:27.9

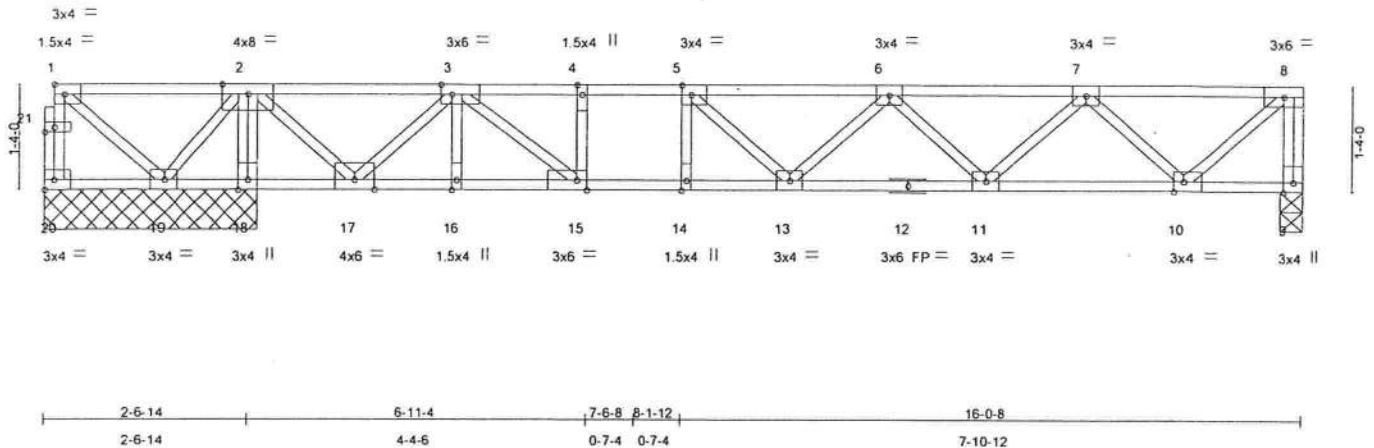


Plate Offsets (X,Y): [3:0-1-12,Edge], [5:0-1-8,Edge], [10:0-1-8,Edge], [15:0-1-8,Edge], [21:0-1-8,0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.52	Vert(LL)	-0.11 13-14	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.88	Vert(TL)	-0.17 13-14	>948	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.40	Horz(TL)	0.02 9	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 89 lb

LUMBER
TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 19-20,18-19,17-18.

REACTIONS (lb/size) 20=-263/2-8-6, 9=659/0-3-8, 18=1378/2-8-6, 19=-43/2-8-6
Max Uplift20=-338(LC 3), 19=-73(LC 3)
Max Grav9=660(LC 3), 18=1378(LC 1), 19=23(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 20-21=0/346, 1-21=0/345, 8-9=-655/0, 1-2=0/402, 2-3=-36/89, 3-4=-1378/0, 4-5=-1378/0, 5-6=-1593/0, 6-7=-1418/0, 7-8=-623/0
BOT CHORD 19-20=-18/0, 18-19=-773/0, 17-18=-767/0, 16-17=0/652, 15-16=0/652, 14-15=0/1378, 13-14=0/1378, 12-13=0/1660, 11-12=0/1660, 10-11=0/1163, 9-10=0/0
WEBS 2-18=-1351/0, 4-15=-291/0, 5-14=-275/0, 1-19=-527/0, 2-19=0/566, 2-17=0/992, 3-17=-918/0, 3-16=-102/0, 8-10=0/829, 7-10=-752/0, 7-11=0/354, 6-11=-336/0, 6-13=-135/69, 5-13=-1/347, 3-15=0/922

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 20 and 73 lb uplift at joint 19.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Manning
July 26,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

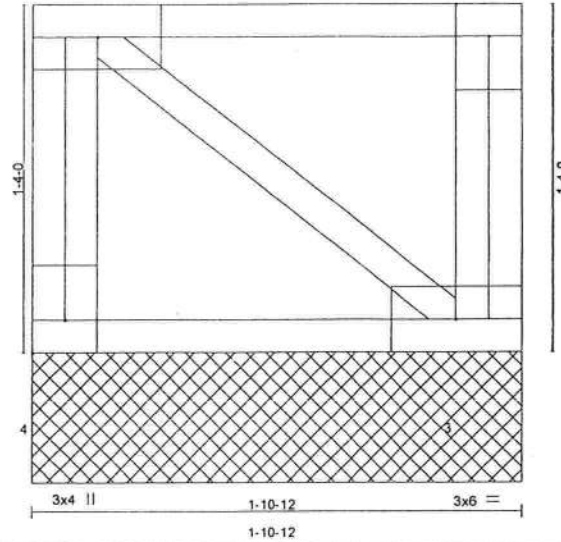
ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEORGE WASHINGTON FL	E4261213
ELEV_F	FF	FLOOR	2	1	ELEVATION F	
					Job Reference (optional)	

Maronda Homes, Inc, Pittsburgh, PA

7.000 s May 29 2007 Mitek Industries, Inc. Wed Jul 25 09:40:48 2007 Page 1

1 3x6 = 2 3x4 ||



Scale = 1/8" = 1'-0"

Plate Offsets (X,Y): [4:Edge,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.16	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.02	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 15 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-10-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=91/1-10-12, 3=91/1-10-12

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-4=-82/0, 2-3=-82/0, 1-2=0/0
BOT CHORD 3-4=-0/0
WEBS 1-3=0/0

NOTES

- 1) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 2) Gable requires continuous bottom chord bearing.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Maria A. Strupinski

July 26, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

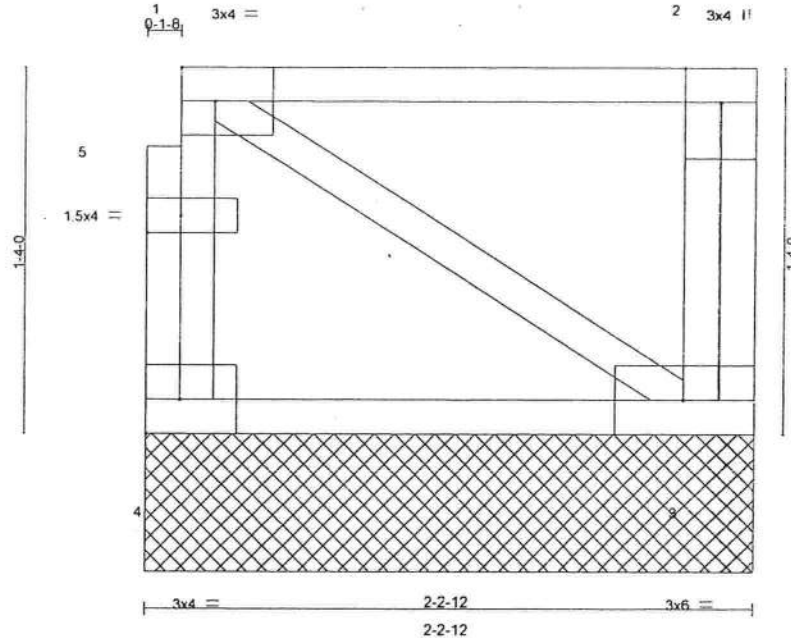
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GEORGE WASHINGTON FL	E4261214
ELEV_F	FG	FLOOR	1	1	ELEVATION F Job Reference (optional)	

Maronda Homes, Inc., Pittsburgh, PA

7.000 s May 29 2007 MiTek Industries, Inc. Wed Jul 25 15:26:59 2007 Page 1



Scale: 1/8"=1'

Plate Offsets (X,Y): [5:0-1-8,0-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.22	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.03	Vert(TL)	n/a	-	n/a		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 16 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=103/2-2-12, 3=109/2-2-12

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 4-5=-93/0, 1-5=-93/0, 2-3=-96/0, 1-2=0/0
BOT CHORD 3-4=0/5
WEBS 1-3=-6/0

NOTES

- 1) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 2) Gable requires continuous bottom chord bearing.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 4) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Marcus Stapp

July 26, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 BEFORE USE.

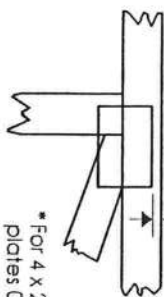
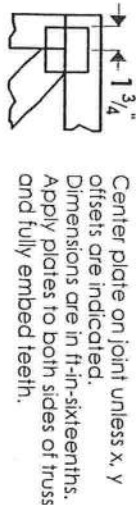
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ICSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate

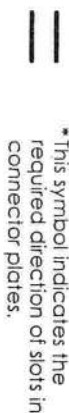
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



* For 4 x 2 orientation, locate plates 0- $\frac{1}{8}$ " from outside edge of truss.



* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

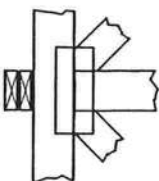
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use L, I or Eliminator bracing if indicated.

BEARING

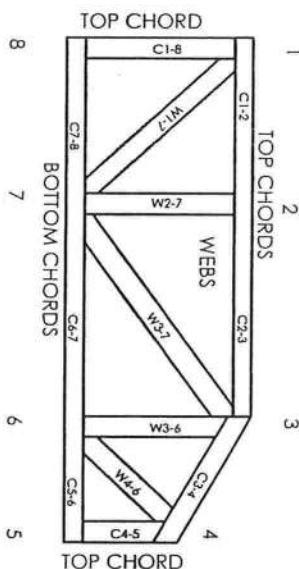


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/FP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR-5243, 9604B
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Mitek Engineering Reference Sheet: MI-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative L, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by ANSI/FP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/FP 1 Quality Criteria.

LEGAL DESCRIPTION:
LOT THIRTEEN (13) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

CERTIFIED TO:
1) MARONDA HOMES

BUILDING SETBACK NOTE:
BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

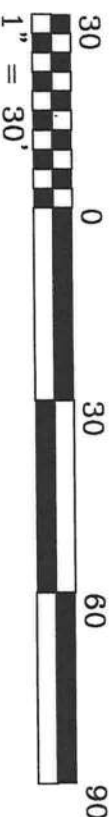
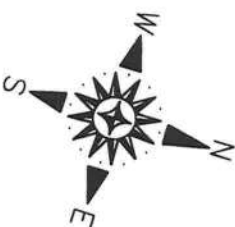
BENCHMARK NOTE:
ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN.

FLOOD NOTE:
IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

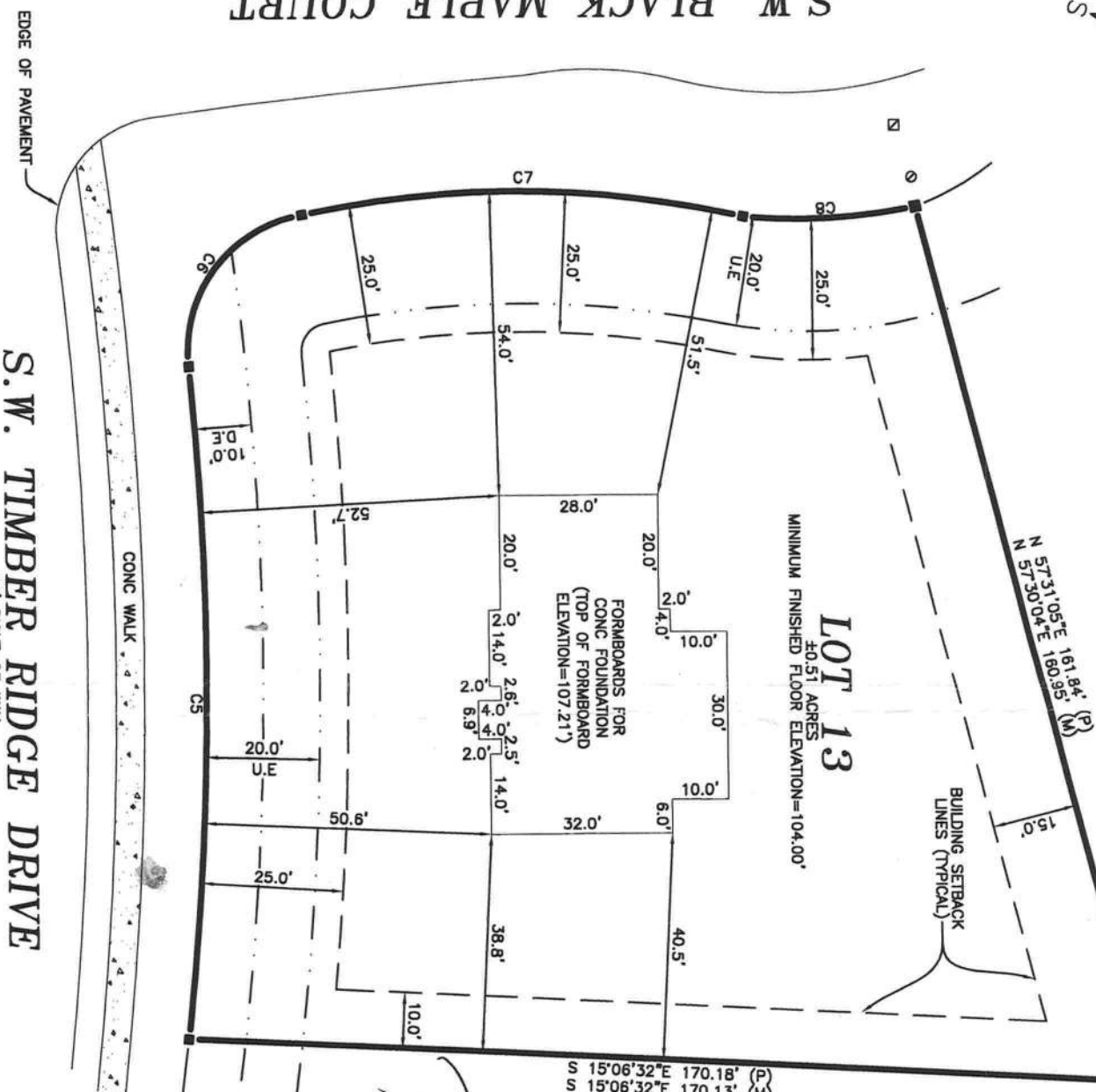
SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

BOUNDARY SURVEY
IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



S.W. BLACK MAPLE COURT



CURVE TABLE:

CURVE	RADIUS	TANGENT	LENGTH	DELTA	CHORD	CHORD BEARING
C5(P)	560.00'	60.14'	119.83'	127°15'36"	119.60'	S 72°33'16" W
C5(M)	560.00'	60.10'	119.75'	127°15'08"	119.52'	S 72°30'02" W
C6(P)	25.00'	22.75'	36.92'	84°36'19"	33.66'	N 71°16'22" W
C6(M)	25.00'	22.53'	36.68'	84°03'19"	33.47'	N 71°01'09" W
C7(P)	200.00'	39.54'	78.07'	227°21'53"	77.58'	N 17°47'16" W
C7(M)	200.00'	39.71'	78.40'	227°27'39"	77.90'	N 17°28'27" W
C8(P)	60.00'	13.88'	31.04'	29°38'26"	30.70'	N 21°25'32" W

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
 - = FOUND 1/2" REBAR & CAP
 - = SET 1/2" REBAR & CAP
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 - = FOUND 4" X 4" CONC. MON.
 - = NO IDENTIFICATION
 - = SET 4" X 4" CONC. MON.
 - = SET NAIL & DISK P.S.M. 5582
 - X = FOUND NAIL & DISK
 - X = FOUND 6" X 6" S.R.D.
 - R/W MON.
 - E = CAVY RISER
 - ⊠ = TELEPHONE PEDESTAL
 - ⊠ = WOOD POWER POLE
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CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 690.15-6, FLORIDA ADMINISTRATIVE CODE.

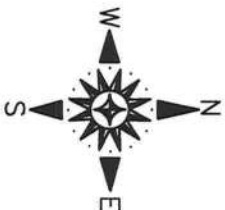
BY: *James E. Brinkman*
JAMES E. BRINKMAN, PSM - FLA. CERT# 5582
DATE: 10/9/08



BRINKMAN SURVEYING & MAPPING INC.
4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'	"THE BENCHMARK IN QUALITY SERVICE"	DRAWN BY: ZL
DATE: 10/9/2008		CHECKED BY: J.B.
FIELD WORK COMPLETED ON 10/7/2008	FIELDBOOK 97, PAGE 67	
PREPARED FOR: MARONDA		DRAWING NUMBER 163-08

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PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:

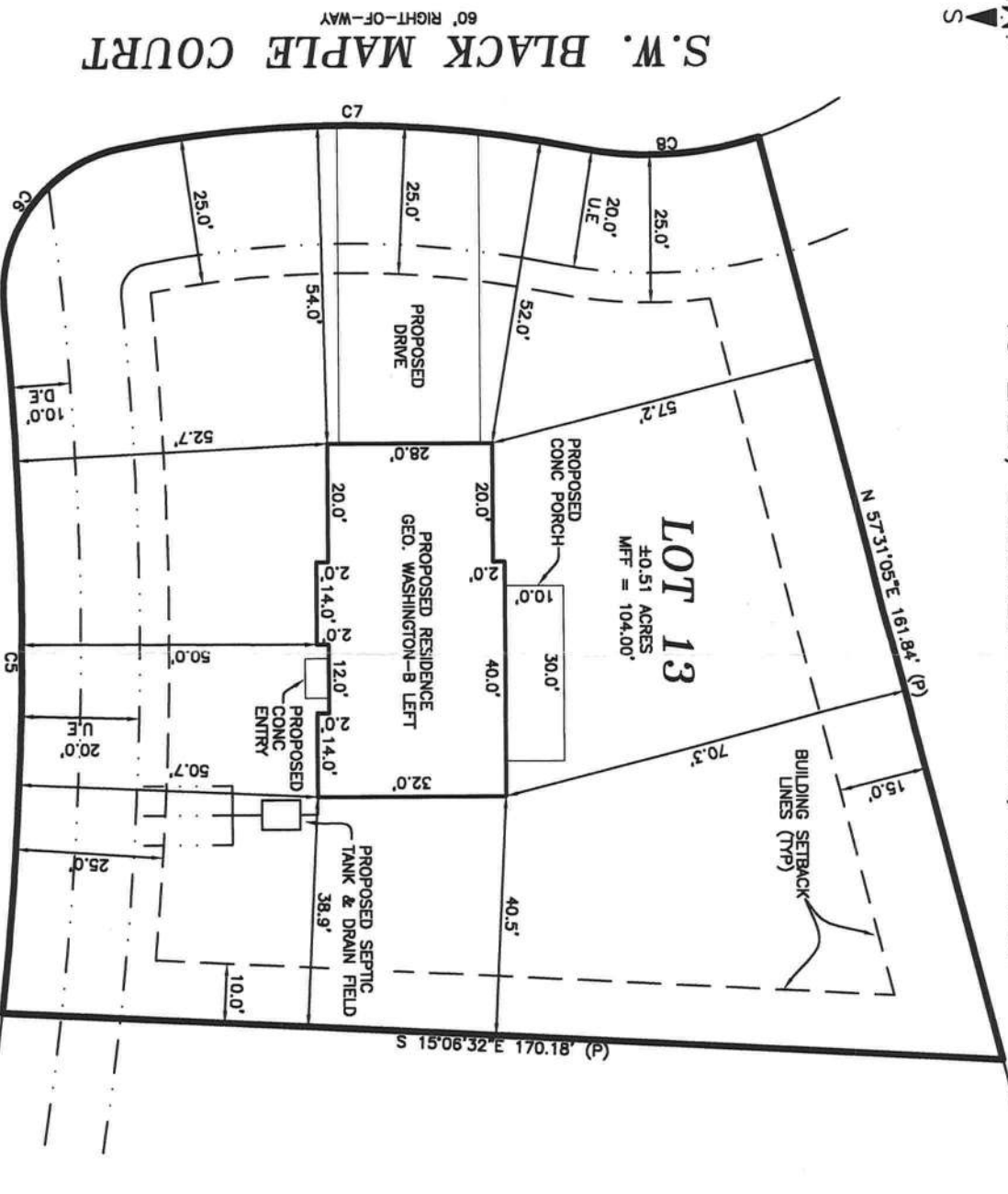
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 - ✕ = SET NAIL & DISK P.S.M. 5582
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BY: 
JAMES E. BRINKMAN, PSM - FLA. CERT# 5582
DATE: 8/14/08



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PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

DATE: 8/14/08

FIELD WORK COMPLETED ON **** FIELDBOOK **, PAGE **

PREPARED FOR: MARONDA DRAWING NUMBER 163-08