JOE GARAGE SHOWN ON BLUEPRINTS W/ STR

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0711-11 Date Received 11/2 By JW Permit # 26401

Application Approved by - Zoning Official _____ Date _____ Plans Examiner OK JTH Date 11-6-07

Flood Zone _____ Development Permit _____ Zoning _____ Land Use Plan Map Category _____

Comments ☑ E.N ☐ NOC ☑ TOWN & H. White Letter

Applicants Name John R Teele Phone 386-497-3360

Address 165 sw Blue Jay CT Fort white FL 32038

Owners Name John R Teele Phone 386-497-3360

911 Address 165 sw Blue Jay CT. Ford white FL. 32038

Contractors Name Owner Builder Phone 386-497-3360

Address cell 352-215-3999

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address Marty J. Humphries #51976 7932 240 St OAkley FL. 32071

Mortgage Lenders Name & Address Millennium Bank 14480 NW 152 Lane Alachua FL. 32615

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progressive Energy

Property ID Number 14330-111 Estimated Cost of Construction 20,000

Subdivision Name Fort White Manor Lot 16 Block ____ Unit ____ Phase ____

Driving Directions From 27 in Fort White Turn on to CR 18 .3 mile on Left 6129

Type of Construction 2X4 Detached Garage Number of Existing Dwellings on Property None

Total Acreage 1.01 Lot Size 1.01 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive H. White

Actual Distance of Structure from Property Lines - Front 185 Side 22 Side 65 Rear 183

Total Building Height 16' Number of Stories 1 Heated Floor Area None 26x30 Roof Pitch 5/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

John R Teele

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number _____

Competency Card Number _____

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 15th day of November 2007.

Personally known ✓ or Produced Identification _____

Kelly R Redding

Notary Signature

Notary Public State of Florida
Kelly R Redding
My Commission DD524763
Expires 03/02/2010

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under a exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, whic means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

( ) Single Family Dwelling
( ) Farm Outbuilding
( ) Two-Family Residence
(✓) Other Detached Garage

NEW CONSTRUCTION OR IMPROVEMENT

(✓) New Construction
( ) Addition, Alteration, Modification or other Improvement

I John R Teele, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

John R Teele 11-1-07
Owner Builder Signature Date

Notary Public State of Florida
Kelly R Redding
My Commission DD524763
Expires 03/02/2010

The above signer is personally known to me or produced identification _____

Notary Signature Kelly R Redding Date 11/1/07 (Stamp / Seal)

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date_____ Building Official/Representative_____

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 10/9/2007 **DATE ISSUED:** 10/10/2007

ENHANCED 9-1-1 ADDRESS:

6129 SW COUNTY ROAD 18

FORT WHITE FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

00-00-00-14330-111

Remarks:

PARENT PARCEL(LOT 16 FORT WHITE MANOR S/D.)

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

986

Approved Address

OCT 10 2007

911Addressing/GIS Dept

This Instrument Prepared by & return to:
Name: ***KIM WATSON, an employee of TITLE OFFICES, LLC***
Address: ***343 NW COLE TERRACE, SUITE 101 LAKE CITY, FLORIDA 32055***
File No. 07Y-08063KW

Parcel I.D. #: ***14330-111***

SPACE ABOVE THIS LINE FOR PROCESSING DATA | *SPACE ABOVE THIS LINE FOR RECORDING DATA*

THIS WARRANTY DEED

Made the ***28th*** *day of* ***September****, A.D.* ***2007****, by* ***RANDY T. BORCHARDT****, A SINGLE PERSON, hereinafter called the grantor, to* ***JOHN R. TEELE*** *and* ***KIMBERLY TEELE****, HIS WIFE, whose post office address is* ***P.O. BOX 176, FORT WHITE, FLORIDA 32038****, hereinafter called the grantees:*

(Wherever used herein the terms "grantor" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantor, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in* ***Columbia County, State of Florida****, viz:*

Lot 16, FORT WHITE MANOR, according to the map or plat thereof as recorded in Plat Book 6, Page 30, of the Public Records of Columbia County, Florida.

Together *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.*

To Have and to Hold *the same in fee simple forever.*

And *the grantor hereby covenants with said grantees that he is lawfully seized of said land in fee simple; that he has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007.*

In Witness Whereof, *the said grantor has signed and sealed these presents, the day and year first above written.*

Signed, sealed and delivered in the presence of:

Witness Signature
MARTHA BRYAN
Printed Name

Witness Signature
PATRICIA LAMB
Printed Name

L.S.
RANDY T. BORCHARDT
Address:
15175 SW TUSTENUGGEE AVENUE, FT. WHITE, FLORIDA 32038

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this ***28th*** *day of* ***September****, 2007, by* ***RANDY T. BORCHARDT****, who is known to me or who has produced* DL *as identification.*

Notary Public

MARTHA BRYAN

Town of Fort White

Post Office Box 129 Fort White, Florida 32038-0129
Town Hall - (386) 497-2321 • Public Works - (386) 497-3345 • Fax (386) 497-4946
Email: townofftwhite@alltel.net • Web site: Townoffortwhitefl.com

CERTIFICATE OF COMPLIANCE & REQUEST FOR ISSUANCE OF BUILDING PERMIT

The undersigned hereby certify the following property is in compliance with the Town of Fort White's Comprehensive Plan and Land Development Regulations for the stated development purposes:

FILE No._____

OWNER'S NAME: John R. Teele

ADDRESS: 165 SW Bluejay Ct. Fort White, FL 32038

PROPERTY DESCRIPTION: 1.01 ac lot #16 Fort White Manor
w/ parcel number parcel #14330-111

DEVELOPMENT: Single family dwelling RSF 1

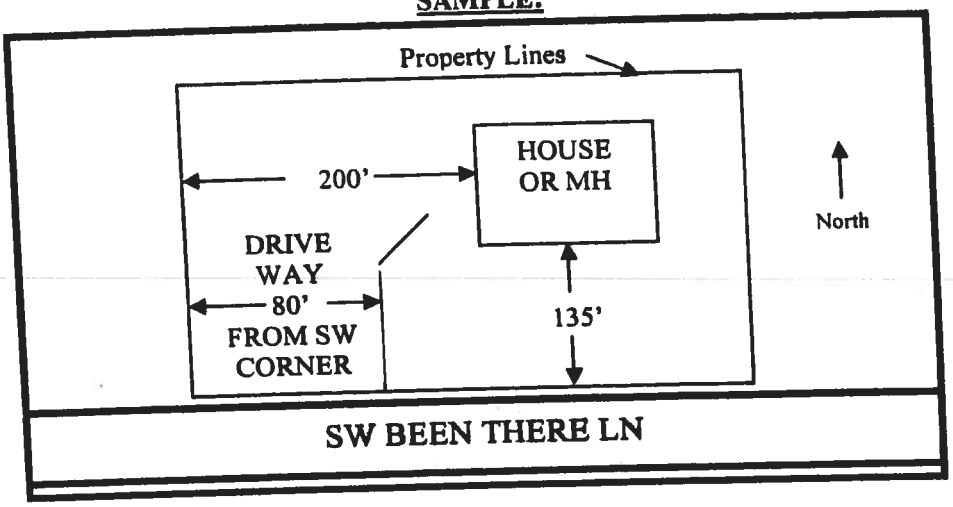
You are hereby authorized to issue the appropriate permits

11/01/07
DATE

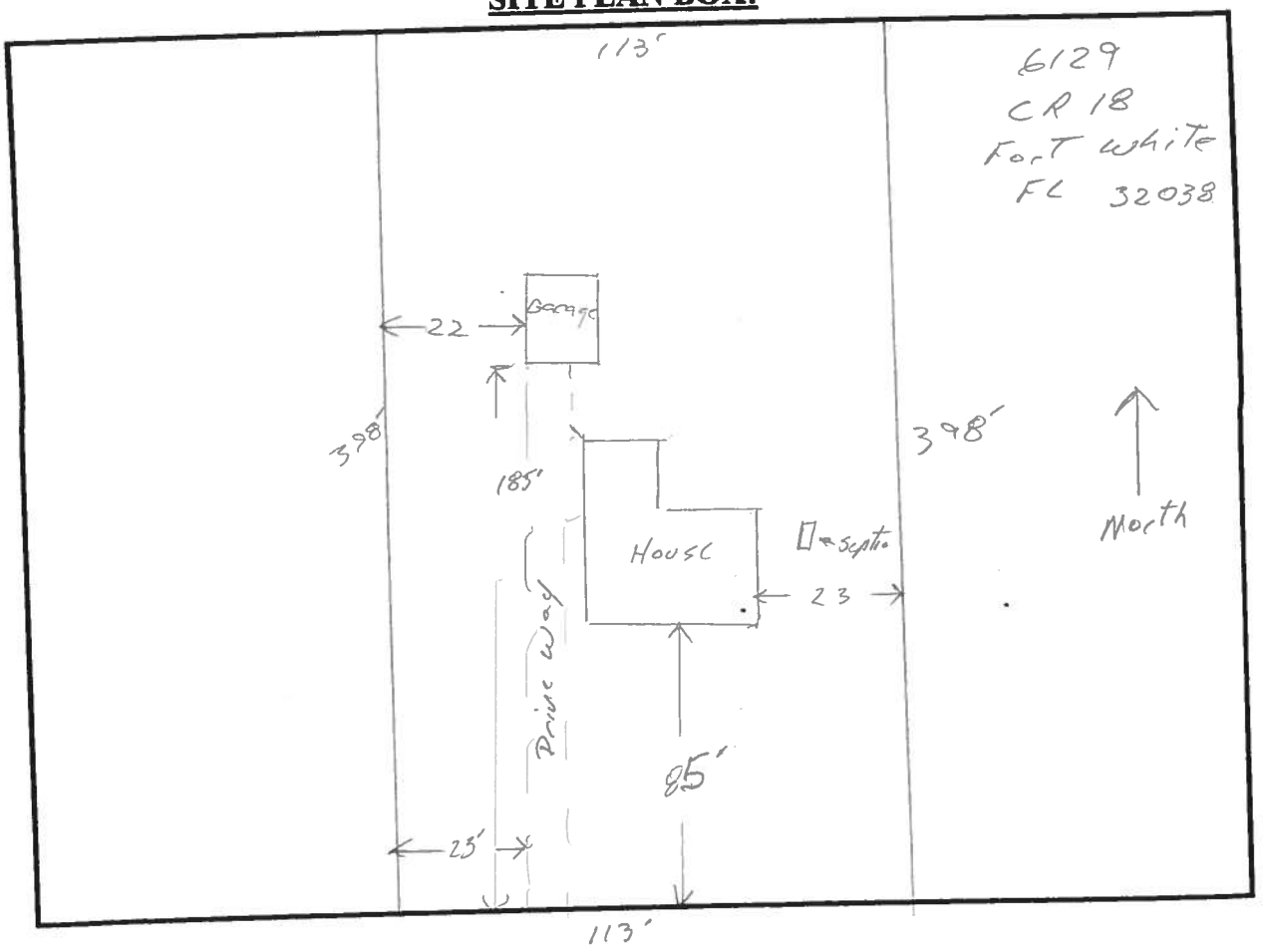
Jessie Reveles
LDR ADMINISTRATOR
Town of Fort White

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:



CR 18

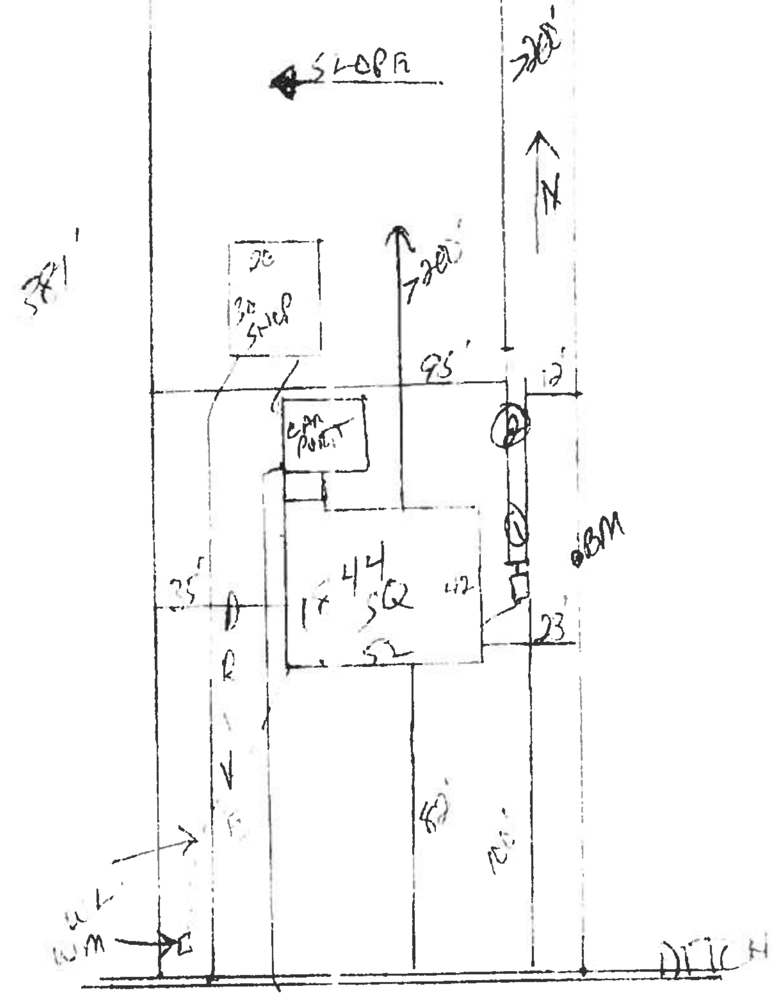
113

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0792

PART II - SITEPLAN

Scale: 1 inch = 50 feet.



Notes: ____

Site Plan submitted by: Rock [signature] — MASTER CONTRACTOR

Plan Approved ✓ Not Approved ____ Date 10/12/...

By [signature] Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4016 which may be used)
(Stock Number: 5744-002-4015-6)

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ———110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. *The following information must be shown as per section 1603.1.4 FBC* a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including:
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

☑ ☐ d) Location, size and height above roof of chimneys.
☐ ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☑ ☐ b) Shear walls identified.
☑ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
☑ ☐ d) Show safety glazing of glass, where required by code.
☑ ☐ e) Identify egress windows in bedrooms, and size.
☑ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
☐ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☑ ☐ b) All posts and/or column footing including size and reinforcing
☑ ☐ c) Any special support required by soil analysis such as piling
☑ ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☑ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☑ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☑ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☑ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☑ ☐ b) Floor joist size and spacing

☑ ☐ c) Girder size and spacing

☑ ☐ d) Attachment of joist to girder

☑ ☐ e) Wind load requirements where applicable

☑ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

☑ ☐ h) Exhaust fans in bathroom

HVAC information

☑ ☐ a) Energy Calculations (dimensions shall match plans)

☑ ☐ b) Manual J sizing equipment or equivalent computation

☑ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment

☑ ☐ **Disclosure Statement for Owner Builders**

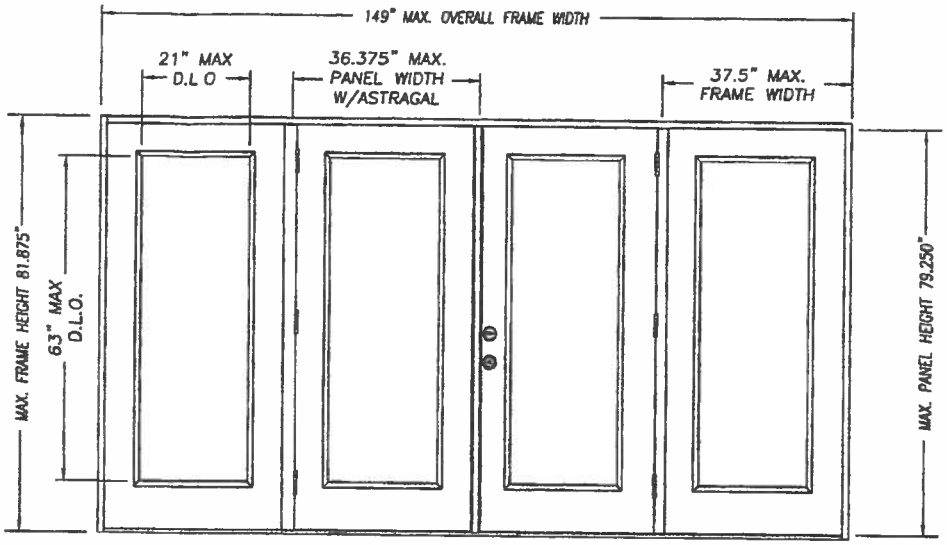
☐ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☐ **Private Potable Water**

FL # 4668.9

Masonite

SIDE-HINGED FIBERGLASS DOOR UNIT
6'-8" GLAZED DOUBLE DOOR WITH / WITHOUT SIDELITES



DOUBLE INSWING UNIT W/SIDELITES

GENERAL NOTES

1. EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLORIDA BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED
2. HURRICANE PROTECTIVE SYSTEM (SHUTTERS) IS REQUIRED.
3. POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 60 PER ASTM E84 POLYSTYRENE CORE FLAME SPREAD INDEX OF 15 AND SMOKE DEVELOPED INDEX OF 115 PER ASTM E84.
4. PLASTICS TESTING OF FIBERGLASS FACING·

TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	803 °F > 650 °F
RATE OF BURNING	ASTM D635	0.79 IN/MIN
SMOKE DENSITY	ASTM D2843	48.9%
TENSILE STRENGTH*	ASTM D638	-7.3% DIFF

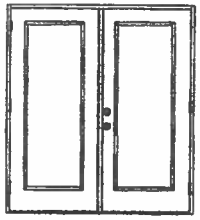
5. PLASTICS TESTING OF LITE FRAME MATERIAL:

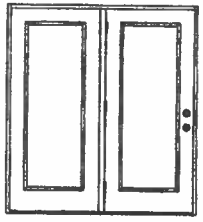
TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	680 °F > 650 °F
RATE OF BURNING	ASTM D635	1.10 IN/MIN
SMOKE DENSITY	ASTM D2843	69.6%
TENSILE STRENGTH*	ASTM D638	-7.48% DIFF

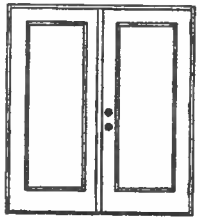
* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1

Addendum to NAMI
Certification No.: NJ-006063R
Reviewed By:
Date Reviewed: 7/18/05


SINGLE DOOR UNIT


DOUBLE DOOR UNIT


SINGLE DOOR UNIT WITH SIDELITE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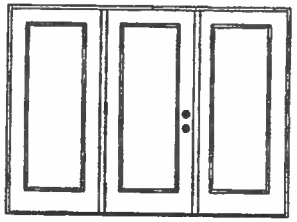
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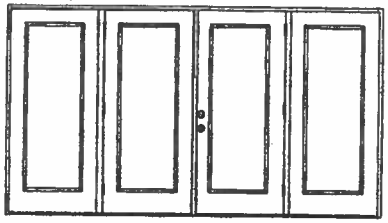
DOUBLE DOOR UNIT W/SIDELITES

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CONFIG	MAX WIDTH	DESIGN PRESSURE RATING				WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE					
		INSWING		OUTSWING		INSWING		OUTSWING		OUTSWING*	
X	37.5"	+52.0	-52.0	+55.0	-55.0	+19.0	-19.0	+40.0	-40.0	+55.0	-55.0
XX	74"	+52.0	-52.0	+55.0	-55.0	+19.0	-19.0	+40.0	-40.0	+55.0	-55.0
OX or XO	75"	+52.0	-52.0	+55.0	-55.0	+19.0	-19.0	+40.0	-40.0	+55.0	-55.0
OXO	112.5"	+52.0	-52.0	+55.0	-55.0	+19.0	-19.0	+40.0	-40.0	+55.0	-55.0
OXXO	149"	+52.0	-52.0	+55.0	-55.0	+19.0	-19.0	+40.0	-40.0	+55.0	-55.0

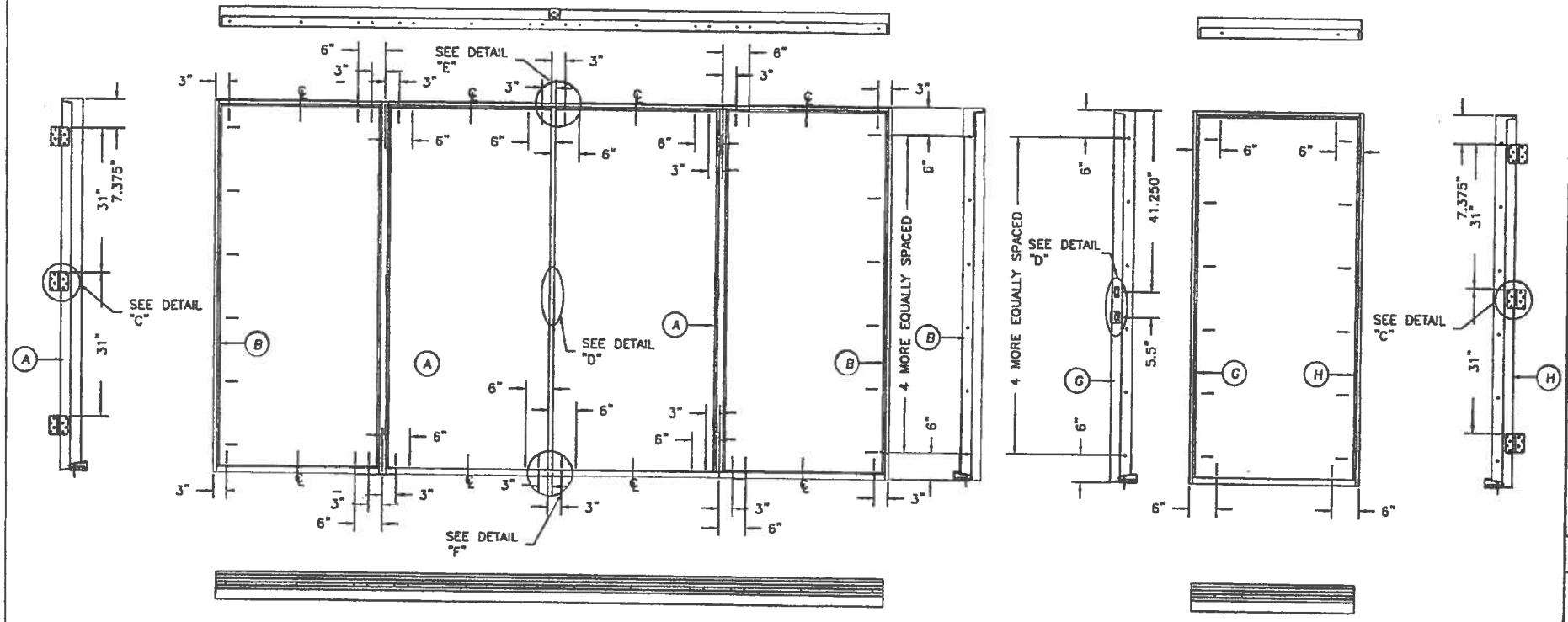
* High Dam Threshold Design

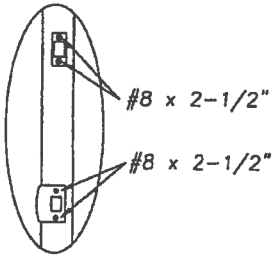
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" DOUBLE 6'8" GLAZED FIBERGLASS DOOR
PART OR ASSEMBLY: TYPICAL ELEVATIONS & GENERAL NOTES

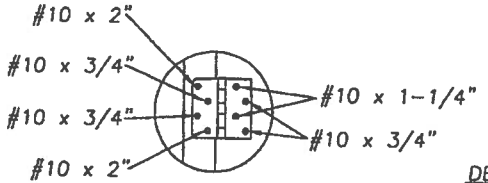
BY	DATE	NO

DATE: 5/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0126-
SHEET 1 OF 3





DETAIL "D"



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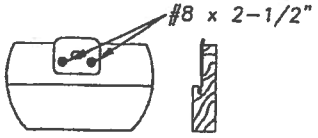
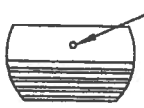


DETAIL "E" ASTRAGAL

ATTACH ASTRAGAL RETAINER BOLT STRIKE PLATE TO FRAME AS SHOWN.



ASTRAGAL RETAINER BOLT HOLE MUST BE DRILLED THROUGH THE THRESHOLD & INTO THE STRUCTURE DEEP ENOUGH FOR A 1.375" THROW

DETAIL "F" ASTRAGAL

INSWING THRESHOLD

OUTSWING THRESHOLD

HIGH DAM O/S THRESHOLD

Addendum to NAMI

Certification No: NJ.00160638

Reviewed By:

Date Reviewed: 7/18/05

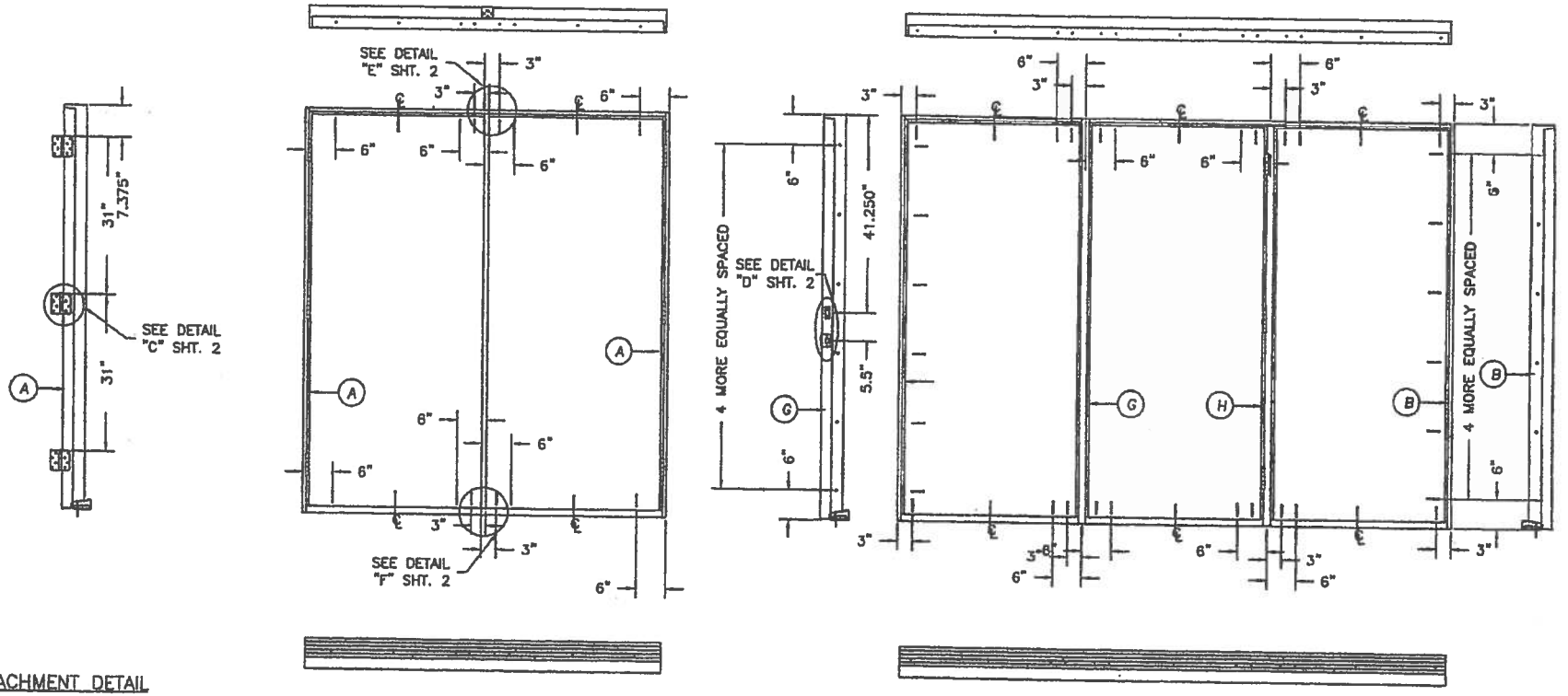
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-0" FIBERGLASS GLAZED DOUBLE DOOR UNIT

PART OR ASSEMBLY: ANCHORING LOCATIONS

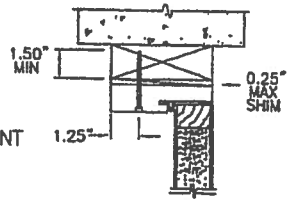
NO.	DATE	BY

DATE: 5/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0126-
SHEET 2 OF 3



ATTACHMENT DETAIL

1. ANCHOR ANALYSIS FOR LOADING CONDITIONS PREPARED, SIGNED AND SEALED BY HAROLD E. RUPP, PE (FLORIDA #15935) WITH THE LOWEST (LEAST) FASTENER RATING FROM THE DIFFERENT FASTENERS BEING CONSIDERED FOR USE. JAMB, HEAD, AND THRESHOLD FASTENERS ANALYZED FOR THIS UNIT INCLUDE #10 WOOD SCREWS OR 3/16" TAPCONS. A PHYSICAL SHIM MUST BE PLACED IN SHIM SPACE AT EACH ANCHOR LOCATION.
2. THE WOOD SCREW SINGLE SHEAR DESIGN VALUES COME FROM ANSI/AF&PA NDA FOR SOUTHERN PINE LUMBER AND ACHEIVEMENT OF 1-1/2" MINIMUM EMBEDMENT. THE TAPCON MUST ACHIEVE MINIMUM EMBEDMENT OF 1-1/4".
3. WOOD BUCKS BY OTHERS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE.
4. MINIMUM DESIGN VALUE STRENGTH OF ANCHORS 171 LBS.



TYPICAL ANCHOR INSTALLATION

HARDWARE SCHEDULE

1.	KWIKSET OR SCHLEGE ANSI/BHMA GRADE 3 OR BETTER CYLINDRICAL AND DEADLOCK HARDWARE TO BE INSTALLED AT 5-1/2" CENTERLINE.
2.	4" X 4" FULL MORTISE BUTT HINGES.

Addendum to NA88
Certification No.: NTOO1065R
Reviewed By:
Date Reviewed: 7/18/05

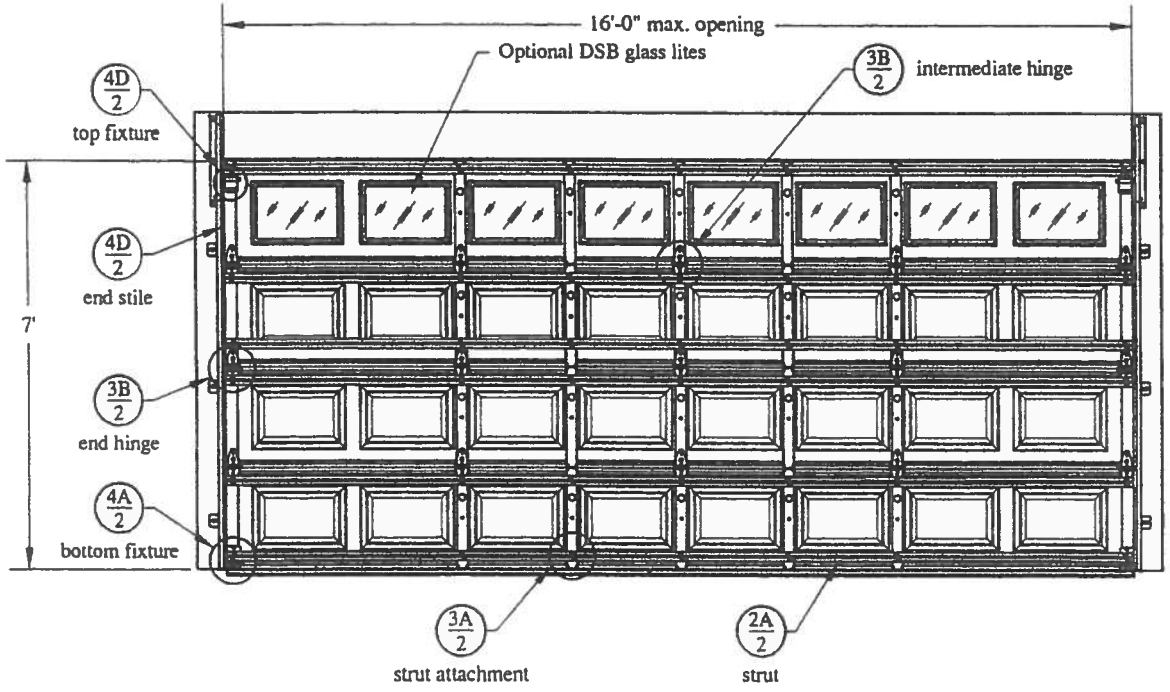
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-8" FIBERGLASS GLAZED DOUBLE DOOR UNIT
PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS

NO.	DATE	BY

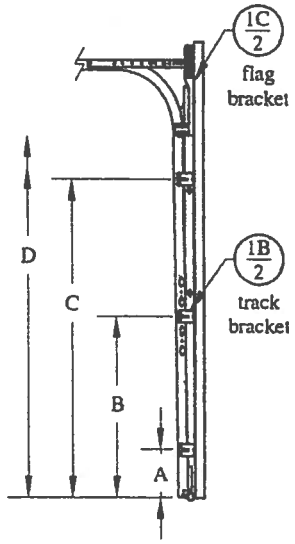
DATE: 5/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0126-
SHEET 3 OF 3

Door Model	Gauge	Decimal
2250/2251	25	.0185
2240/2241	24	.0225



door height	section quantity	strut quantity	trk brkt per side
6'-6" to 7'-0"	4	6	3
7'-6" to 8'-0"	5	7	4
8'-3" to 8'-9"	5	8	4
9'-0" to 10'-6"	6	10	5
10'-9" to 12'-3"	7	12	6
12'-6" to 14'-0"	8	14	7

Refer to Supplemental Instructions for strut placement on doors over 7'-0" high



Track Bracket Chart

		door height								
		6'-6"	6'-9"	7'-0"	7'-6"	7'-9"	8'-0"	8'-3"	8'-6"	8'-9"
track brackets	D	n/a	n/a	n/a	72"	69"	72"	81"	84"	87"
	C	60"	63"	66"	58"	55"	58"	60"	63"	66"
	B	35"	35"	38"	34"	31"	34"	32"	35"	38"
	A	10"	7"	10"	10"	7"	10"	4"	7"	10"

Track bracket locations shown above are for doors up to five sections high. Additional door sections may be added for a maximum door height of 14'-0". One track bracket (per track) must be added for each section and spaced at a distance not greater than the corresponding section height.

This door has been tested in accordance with ANSI/DASMA 108-2002
Design Pressure (DP): 22.0 pos / 24.5 neg
Test Pressure (TP): 33.0 pos / 36.8 neg

Per 2004 FBC Table 1609.6E, DP meets or exceeds basic wind speed of;
V = 120 MPH for Exposure B and mean roof height of 30' or less
V = 102 MPH for Exposure C and mean roof height of 30' or less

Maximum door size: 16'-0" wide by 14'-0" tall

Glazing and door have not been tested for windborne debris.

Wood buck and supporting structural elements shall be designed by a registered professional engineer for wind loads shown on this drawing.

If door is not electrically operated, a lock must be installed.

Professional Engineer's seal provided only for verification of windload construction details

John E. Scates, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

FL 5519

C.H.I. SCALE nts DATE 10-25-200
Model 2250/51 (16'-0" wide
C.H.I. Drawing: Z4-1607-011

...ge (.070) galvanized steel
...ture manufactured by
... Each fixture attached
...our 1/4" x 3/4" screws.
...nut

...auge (.036) end
... manufactured by
...I.I

Strut, if applicable, not shown for clarity.

The 2x6 vertical wood jambs are to be grade 2 or better Southern Pine. Fasteners may be countersunk to provide a flush mounting surface.

2" x 7/16" (nominal) Stop molding required (not supplied by C.H.I.)

12 gauge (.095) galvanized steel track bracket fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.

2" x .051 min. galvanized steel track fastened to track brackets. Each track bracket attached with one 1/4" x 5/8" track bolt and nut.

20 gauge (.036) center stile manufactured by C.H.I.

2" steel roller

End Hinge
16 gauge (.058) galvanized steel end hinge fastened to section with four 1/4" x 3/4" screws.

push nut

2" steel track roller.

2 3/4"

3-1/2" (min.) stem3 1/2"

Intermediate Hinge
18 gauge (.047) galvanized steel intermediate hinge fastened to section with four 1/4" x 3/4" screws.

2 3/4"

3"

1 7/8"

1/2"

20 gauge (.034) 50 ksi galvanized steel 3" strut attached with two 1/4" x 3/4" screws per stile or hinge plate.

Vinyl weatherstrip

12 gauge (.102) galvanized steel bottom bracket manufactured by C.H.I. Each bracket attached with four red 1/4" x 3/4" screws.

push nut

Aluminum extrusion

Details on some views may have been omitted for clarity.

12 gauge (.086) galvanized steel flag bracket fastened to wood jamb with three 5/16" x 1-5/8" wood lag screws.

Flag bracket attached to horizontal track with two 1/4" x 5/8" track bolts and nuts.

Flag bracket attached to vertical track with two 1/4" x 5/8" track bolts and nuts.

12 gauge (.095) galvanized steel track bracket fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.

Each track bracket attached with one 1/4" x 5/8" track bolt and nut. Or 1/4" x 11/32" rivets.

Design Load: 22 pos / 24.5 neg
Test Load: 33 pos / 36.8 neg
page 2 of 2

Professional Engineer's seal provided only for verification of windload construction details

John E. Scates, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

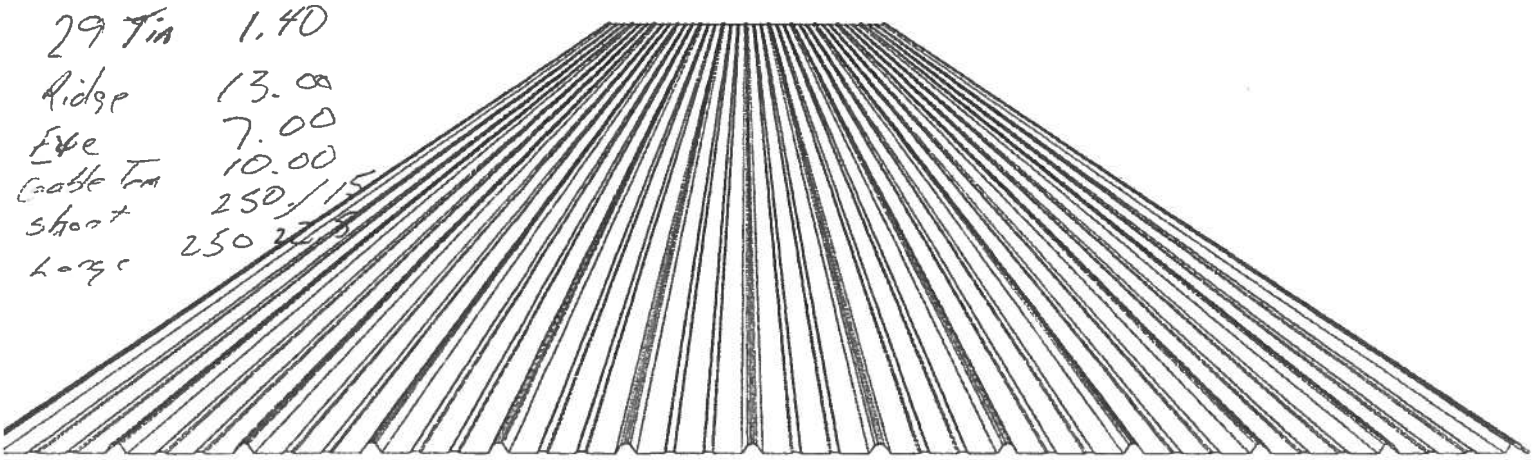
C.H.I.

SCALE nts
DATE 10-25-2005
Model 2250/51 (16'-0" wide)
C.H.I. Drawing: Z4-1607-0110

Gulf Coast Supply & Mfg. Inc.

DETAIL MANUAL

and guide to Gulf Coast products



Tuff-Rib Roofing Panels & Accessories

Rt. 1 Box 112 • Horseshoe Beach, FL 32648
(352) 498-0778 • Toll Free (888) 393-0335 • FAX (352) 498-7852

linear feet of panels in your order	Screw (purlin) Spacing 12 inch	18 inch	24 inch	30 inch
50	270	180	135	108
100	540	360	270	216
200	1080	720	540	432
300	1620	1080	810	648
400	2160	1440	1080	864
500	2700	1800	1350	1080
600	3240	2160	1620	1296
700	3780	2520	1890	1512
800	4320	2880	2160	1728
900	4860	3240	2430	1944
1000	5400	3600	2700	2160
1100	5940	3960	2970	2376
1200	6480	4320	3240	2592

Figure 6 Tuff-rib panel screw calculation chart

Panel lap detail

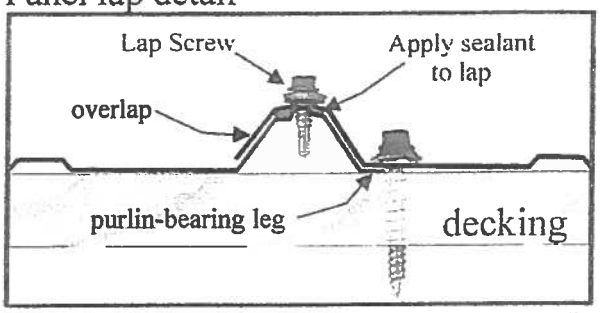


Figure 7 On low-pitched roofs butyl tape or caulk should be applied at the panel lap to keep water from overflowing the lap. Note that the *underlap* side of the panel has a short purlin-bearing leg that rests on the roof decking.

How to figure screws:

For 2-foot spacing between rows of screws, multiply the total linear feet of metal times 2.7

Example: your order is 1250 feet of Tuff-rib roofing. 1250 x 2.7 = 3375 screws

See table above for other spacings, or contact your Gulf Coast representative for a free estimate.

Gulf Coast Supply carries screws in 3 different lengths: 1 inch, 1½ inch, and 2 ½ inch. 1-inch screws will barely penetrate a 1x4, but the 1½ inch are the best all-purpose size. 1½- or 2½-inch screws are necessary for attaching ridge caps.

If care is taken, metal roofing application can be aided by pre-drilling panels, allowing screws to go quickly and accurately into the desired spacing. Pre-drilling will work provided that pilot holes *<u>are placed accurately</u>* in the proper locations on panels. Purlin spacing must be uniform and carefully measured.

To apply metal roofing over existing shingles, we recommend first overlaying the shingles with properly attached 1x4 purlins. If pressure treated purlins are used, felt paper should be applied over them in strips to prevent chemical interaction with the roofing panels. For solid decking, at least ½-inch plywood or its equivalent is required. For minimum penetration (such as might be desired over porches), 1-inch screws are recommended.

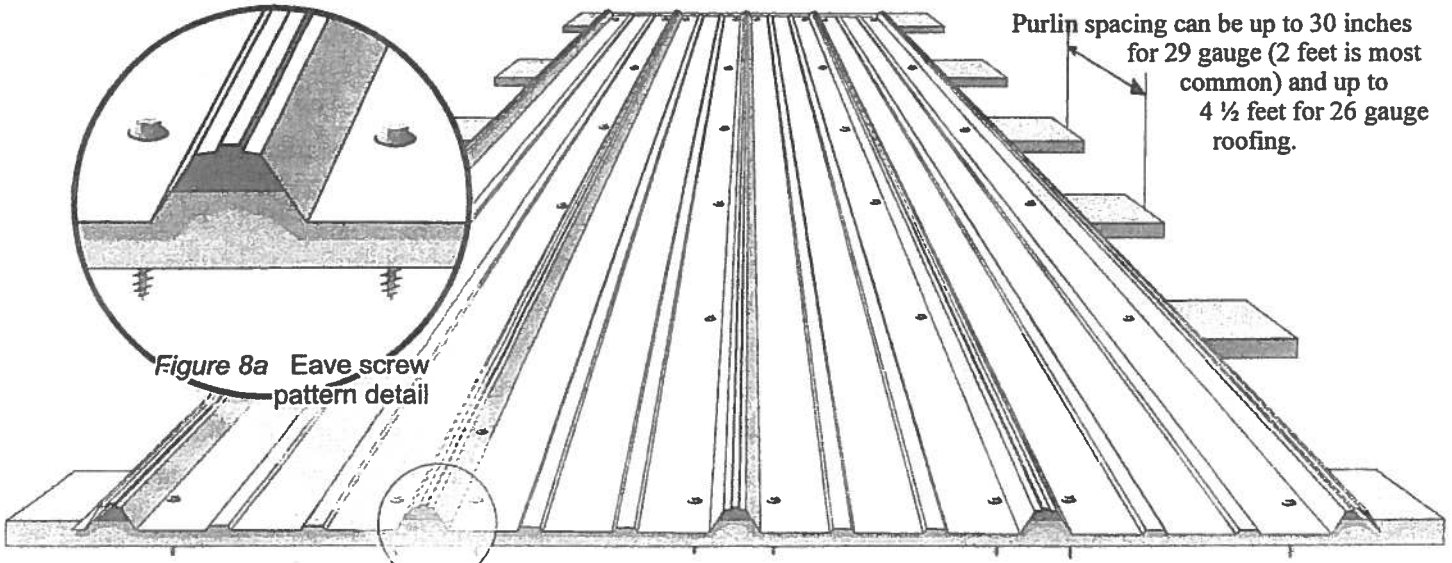


Figure 8a Eave screw pattern detail

Figure 8 Screws should be placed on both sides of the ribs on the eave

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS	Masonite	Fiber glass	FL 4668.1
A. SWINGING	Masonite	Fiber glass	4668.9
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	Capitol/MI/AA	Vinyl - Low E	FL# 5438.9
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	Hardee Lap		889.2
B. SOFFITS	Owens-Corning	Facia + soffit	FL 6869-R1
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL	Gulf Coast	TUFF-Rib 29 Ga.	7099.2
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER	Warrior	30 # Felt	FL 2346
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS	Simpson	Straps + Truss Anchors	FL 474-FL555
B. WOOD ANCHORS	Simpson	Anchors	FL 474-FL5550
C. TRUSS PLATES	Mitek Plates		FL 2197-R1
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

[signature] APPLICANT SIGNATURE

10-31-07 DATE

R-1305 01-04

FL # 5438.9

MI WINDOWS and DOORS

CAPITOL | BetterBilt | CAPITOL ARCHITECTURAL PRODUCTS

INSTALLATION INSTRUCTIONS FOR NEW CONSTRUCTION VINYL FIN WINDOWS

READ THESE INSTRUCTIONS COMPLETELY BEFORE BEGINNING. Please inspect your MI Windows and Doors, Inc. product thoroughly before beginning installation. Inspect the opening and the product, and do not install if there is any observable damage or other irregularity. The product specification sheet and warranty include important information regarding your product and may include product-specific installation requirements (for example, types of fasteners to be used with impact resistant windows and limitations on the height at which the product may be installed); if you did not obtain copies please contact MI Windows and Doors, Inc. Local building codes may impose additional requirements, and those codes supercede these instructions.

FAILURE TO FOLLOW THESE INSTRUCTIONS, AND BUILDING CODE REQUIREMENTS, MAY AFFECT THE REMEDIES AVAILABLE UNDER YOUR WARRANTY.

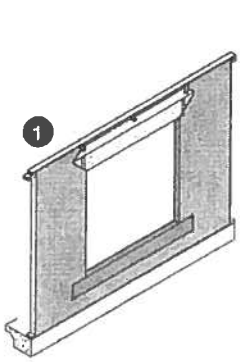
1. IF THE BUILDING HAS A WEATHER RESISTANT BARRIER (WRB) I.E. HOUSE WRAP, PREPARE THE OPENING ACCORDING TO WRB MANUFACTURER'S INSTRUCTIONS. AT EACH TOP CORNER MAKE A 45° CUT IN THE WRB. FOLD UP THE WRB SO THAT THE TOP NAIL FIN OF THE UNIT CAN BE INSTALLED UNDERNEATH IT. (See Figure 1 below) FLASHING OF THE WINDOW OPENING IS RECOMMENDED AND MAY BE REQUIRED BY SOME BUILDING CODES.
2. MAKE SURE THE ROUGH OPENING IS PLUMB, SQUARE AND THE SILL PLATE IS LEVEL. ROUGH OPENINGS SHOULD BE 1/2" LARGER THAN WINDOW FRAME IN WIDTH & HEIGHT. (See Figure 2 below)
3. CLOSE & LOCK THE SASH THROUGHOUT INSTALLATION. KEEP THE SIDE JAMBS PLUMB & SQUARE WITH HEAD AND SILL. BE CAREFUL NOT TO "CROWN UP" OR "BOW DOWN" THE SILL OR HEAD. CONSTANTLY CHECK WIDTH AT THE MEETING RAILS OF SINGLE AND DOUBLE HUNGS (CENTER POINT ON CASEMENTS) TO AVOID A "BOWED OUT" INSTALLATION. WHEN USING FLASHING APPLY THE BOTTOM PIECE BEFORE INSTALLING THE WINDOW. (See Figure 1 below) FLASHING MUST BE RATED TO MEET ASTM D-779. 24 HOUR WATER RESISTANCE TEST.
4. APPLY A CONTINUOUS 3/8" BEAD OF PREMIUM GRADE, COMPATIBLE EXTERIOR SEALANT TO THE INTERIOR (BACKSIDE) OF THE NAIL FIN NEAR THE OUTSIDE EDGE IN LINE WITH THE PRE-PUNCHED HOLES ON **ALL SIDES PRIOR** TO SETTING THE WINDOW INTO THE ROUGH OPENING. (See Figure 3 below)
5. PLACE 1/4" FLAT SHIMS ON THE ROUGH OPENING SILL PLATE UNDER THE BOTTOM CORNERS OF THE WINDOW (See Figure 4 below). THESE SHIMS SHOULD BE REMOVED WHEN INSTALLATION IS COMPLETE. DO NOT PLACE SHIMS OR BLOCKS UNDER THE SILL EXCEPT AT THE FRAME CORNERS. SET THE WINDOW ONTO THE SHIMS CENTERING THE WINDOW IN THE OPENING ALLOWING EQUAL SPACE ON EITHER SIDE. FOR WINDOWS WITH INTERMEDIATE JAMBS AND ALL SLIDER WINDOWS, CONTINUOUS SHIM OR HORIZONTAL SHIMS ARE RECOMMENDED UNDER EACH INTERMEDIATE JAMB AND MEETING RAIL TO ENSURE SILL IS LEVEL). THESE SILL SHIMS SHOULD REMAIN AFTER INSTALLATION IS COMPLETE. APPLY ADDITIONAL SHIMS AS NECESSARY TO MAINTAIN A LEVEL SILL THROUGHOUT INSTALLATION.
6. PLACE A TEMPORARY FASTENER IN THE SLOT PROVIDED IN THE NAIL FIN ON EACH TOP CORNER, CHECK LEVEL AND SQUARE OF THE WINDOW BY MEASURING THE DIAGONALS. OPEN BOTTOM SASH, CHECK THE "REVEAL" (SPACE) BETWEEN THE BOTTOM OF THE SASH AND THE WINDOW SILL. CLOSE AND RELOCK THE SASH, ADJUST IF NECESSARY. PLACE ADDITIONAL FASTENERS IN THE BOTTOM CORNERS CHECKING WINDOW AGAIN FOR LEVEL, PLUMB AND SQUARE.
7. SECURE THE WINDOW WITH FASTENERS THAT PENETRATE THE FRAMING BY A MINIMUM OF 1", CARE SHOULD BE TAKEN TO INSTALL FASTENERS STRAIGHT, NOT ANGLED. KEEP THE SASH LOCKED UNTIL ALL SIDES ARE SECURE. PRIOR TO FASTENING THE SILL AND HEAD BE SURE THEY ARE STRAIGHT AND LEVEL. FASTENERS SHOULD BE APPLIED SECURELY INTO EVERY OTHER SLOT ON ALL SIDES, DO NOT DISTORT THE NAIL FIN WITH THE FASTENERS.
8. APPLY SEALANT OVER EXPOSED FASTENER HEADS, ANY UNUSED SLOTS AND THE OUTSIDE EDGE OF THE NAIL FIN WHERE IT COMES IN CONTACT WITH THE WRB/SHEATING. **OR IF FLASHING (WINDOW TAPE) IS BEING USED** - NOTE: SILL FLASHING SHOULD HAVE BEEN APPLIED PRIOR TO INSTALLING THE WINDOW. APPLY THE SIDE FLASHING ON TOP OF THE NAIL FIN, OVERLAPPING THE SILL FLASHING AND EXTENDING UP PAST THE TOP NAIL FIN APPROXIMATELY 2". THEN APPLY THE TOP FLASHING ALSO OVER THE NAIL FIN, OVERLAPPING THE SIDE PIECES AND EXTENDING PAST THE SIDE FLASHING BY APPROXIMATELY 1". LASTLY FOLD DOWN THE WRB FLAP OVER THE FLASHING, TAPE THE DIAGONAL CUTS ABOVE EACH CORNER. (SEE FIGURE #5 BELOW)
9. PLACE SHIMS AT THE MEETING RAIL/CHECK RAIL ON THE SIDE JAMBS TO PREVENT BOWING, THESE SHIMS SHOULD REMAIN AFTER INSTALLATION. CAUTION SHOULD BE TAKEN AS TO NOT OVER SHIM, CAUSING DEFLECTION OF THE FRAME AND HINDER SASH OPERATION. CHECK THE FRAME WIDTH AT TOP, MIDDLE AND BOTTOM, IF NOT THE SAME, SHIM ACCORDINGLY. UNLOCK AND OPERATE THE SASH(S). VISUALLY INSPECT ALL SIGHT LINES. ADJUST OR SHIM AS REQUIRED TO ASSURE CONSISTENT SASH REVEAL AND EASE OF OPERATION.
10. INSULATE BETWEEN THE WINDOW FRAME & ROUGH OPENING WITH FIBERGLASS INSULATION OR EQUAL. THE SPACE MAY BE EFFECTIVELY FILLED WITH MEASURED USE OF LOW EXPANSION FOAM BUT ONLY AFTER DETERMINING THAT FOAM WILL NOT EXERT PRESSURE AGAINST THE FRAME, WHICH CAN IMPAIR OPERATION. DISTORTION OF THE FRAME WILL AFFECT THE USER'S RIGHTS UNDER THE WARRANTY.
11. ALLOW A 1/4" GAP BETWEEN THE EXTERIOR CLADDING, SIDING, BRICK, STUCCO OR STONE AND THE WINDOW FRAME ON ALL SIDES (EXCEPT VINYL J CHANNEL). THE GAP (EXPANSION JOINT) SHOULD BE FILLED WITH CORRECT SIZE BACKER ROD, THEN SEALED WITH A HIGH GRADE EXTERIOR SEALANT AND WILL NEED TO BE MAINTAINED.

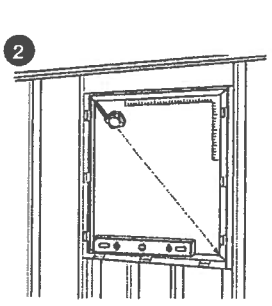
CAUTION:

- USE OF SOLVENTS OR ACIDS WILL DAMAGE COMPONENTS OF THIS PRODUCT AND WILL LIMIT RIGHTS UNDER THE WARRANTY
- VINYL WINDOWS HAVE PRE-PUNCHED SLOTS FOR INSTALLATION - FASTENING IN ANY OTHER PORTION MAY PERMANENTLY DAMAGE UNIT WHICH WILL LIMIT RIGHTS UNDER THE WARRANTY.
- IT IS THE SOLE RESPONSIBILITY OF THE OWNER, ARCHITECT, AND/OR BUILDER TO SELECT CORRECT PRODUCTS TO BE IN COMPLIANCE WITH APPLICABLE LAWS, SITE REQUIREMENTS AND BUILDING CODES AND TO ENSURE THAT INSTALLATION IS IN COMPLIANCE WITH APPLICABLE LAWS, SITE REQUIREMENTS AND BUILDING CODES.
- DO NOT STORE IN THE SUN OR LAY FLAT BEFORE OR DURING INSTALLATION.
- ANY PENETRATIONS (e.g. ALARM SENSORS) MADE THROUGH ANY PORTION OF ANY M.I., BETTERBILT OR CAPITOL PRODUCT MAY AFFECT RIGHTS UNDER THE MANUFACTURER'S WARRANTY.
- SOME LAWS AND BUILDING CODES REQUIRE SAFETY GLASS. THE ORDERING PARTY IS RESPONSIBLE TO SPECIFY SAFETY GLASS AND ENSURE COMPLIANCE WITH LOCAL LAWS AND BUILDING CODES.

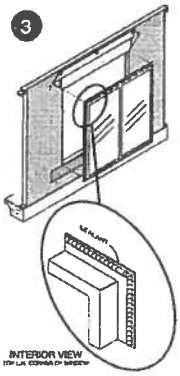
THESE INSTRUCTIONS ARE MINIMUM REQUIREMENTS ONLY, CHECK STATE AND LOCAL CODE RESTRICTIONS FOR ADDITIONAL COMPLIANCE ON INSTALLATION AND/OR FASTENING. IF UNIT HAS EXTERIOR TRIM (BRICKMOULD/J CHANNEL, ETC.) THE UNIT MUST BE SEALED BEHIND THE NAIL FIN, THE TRIM IS PROVIDED FOR AESTHETIC PURPOSES ONLY, AND NOT DESIGNED TO BE WATER TIGHT. INSTALLATION INTO MASONRY OR REPLACEMENT OPENINGS MUST BE SEALED TO THE OPENINGS USING AN APPROVED, PROPER METHOD. REFER TO AAMA 2400 AND/OR ASTM 2112 STANDARDS.

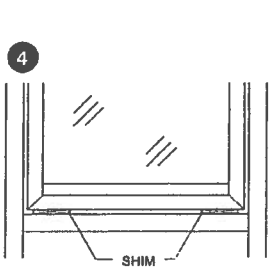
These installation instructions are provided for information only; no representation and warranty is made that these instructions set forth all of the information necessary for proper installation of the product. Given the variety of field conditions, primary responsibility for product installation rests with the installer. Do not proceed unless you have addressed the factors necessary to achieve weather-tight installation of a properly functioning product. MI Windows and Doors, Inc. assumes no liability for any personal injury or property damage incurred in installation. These instructions, together with the product specifications and warranty set forth the entire liability of MI Windows and Doors, Inc. with regard to the product.



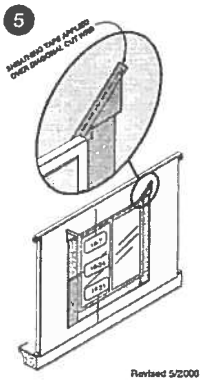


Check for level, plumb and square.





"DRAWINGS HAVE BEEN REPRINTED WITH PERMISSION FROM THE INSTALLATION MASTERS™ #INSTITUTE. THE INSTALLATION MASTERS™ ACCEPTS NO RESPONSIBILITY FOR THE USE OR APPLICATION OF THIS INFORMATION."

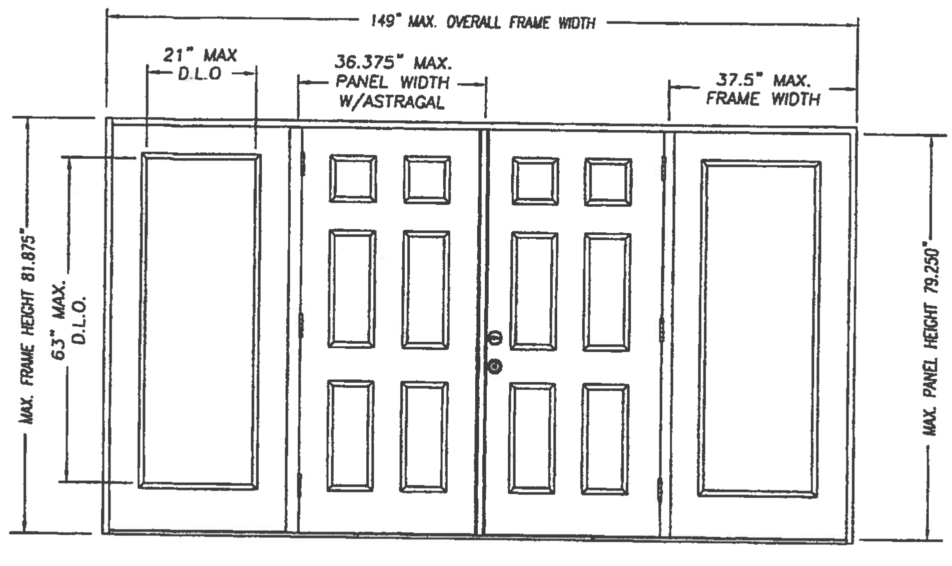


Revised 5/2008

FL# 4668.1

Masonite

SIDE-HINGED FIBERGLASS DOOR UNIT
6'-8" DOUBLE DOOR WITH / WITHOUT SIDELITES



DOUBLE INSWING UNIT W/SIDELITES

GENERAL NOTES

1. EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLORIDA BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED
2. HURRICANE PROTECTIVE SYSTEM (SHUTTERS) IS REQUIRED.
3. POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 60 PER ASTM E84. POLYSTYRENE CORE FLAME SPREAD INDEX OF 15 AND SMOKE DEVELOPED INDEX OF 115 PER ASTM E84
4. PLASTICS TESTING OF FIBERGLASS FACING:

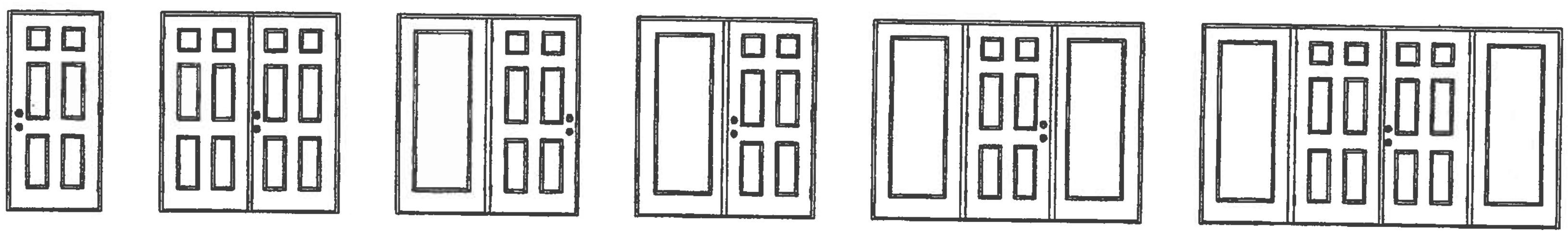
TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	803 °F > 650 °F
RATE OF BURNING	ASTM D635	0.79 IN/MIN
SMOKE DENSITY	ASTM D2843	48.9%
TENSILE STRENGTH*	ASTM D638	-7.3% DIFF

5. PLASTICS TESTING OF LITE FRAME MATERIAL:

TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	680 °F > 650 °F
RATE OF BURNING	ASTM D635	1.10 IN/MIN
SMOKE DENSITY	ASTM D2843	69.6%
TENSILE STRENGTH*	ASTM D638	-7.48% DIFF

* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1

Addendum to NAMI
Certification No.: NI00663R
Reviewed By:
Date Reviewed: 7/18/05



SINGLE DOOR UNIT | DOUBLE DOOR UNIT | SINGLE DOOR UNIT WITH SIDELITE | SINGLE DOOR UNIT WITH SIDELITE | SINGLE DOOR UNIT W/SIDELITES | DOUBLE DOOR UNIT W/SIDELITES

TABLE OF CONTENTS

		DESIGN PRESSURE RATING		WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE		
CONFIG	MAX WIDTH	INSWING	OUTSWING	INSWING	OUTSWING	OUTSWING*
X	37.5"	+70.0 / -70.0	+70.0 / -70.0	+19.0 / -19.0	+70.0 / -70.0	+70.0 / -70.0
XX	74"	+50.5 / -50.5	+55.0 / -50.5	+19.0 / -19.0	+40.0 / -40.0	+55.0 / -50.5
OX or XO	75"	+50.5 / -50.5	+55.0 / -50.5	+19.0 / -19.0	+40.0 / -40.0	+55.0 / -50.5
OXO	112.5"	+50.5 / -50.5	+55.0 / -50.5	+19.0 / -19.0	+40.0 / -40.0	+55.0 / -50.5
OXXO	149"	+50.5 / -50.5	+55.0 / -50.5	+19.0 / -19.0	+40.0 / -40.0	+55.0 / -50.5

* High Dom Threshold Design

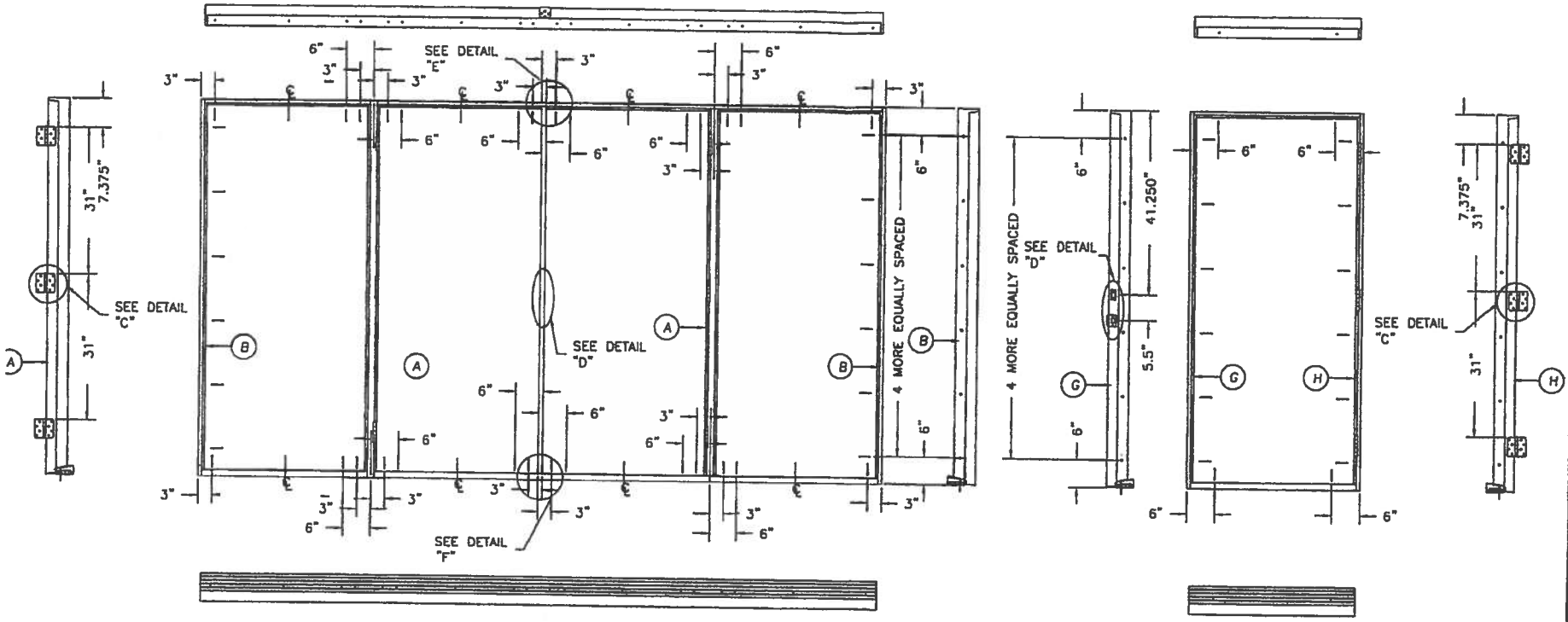
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

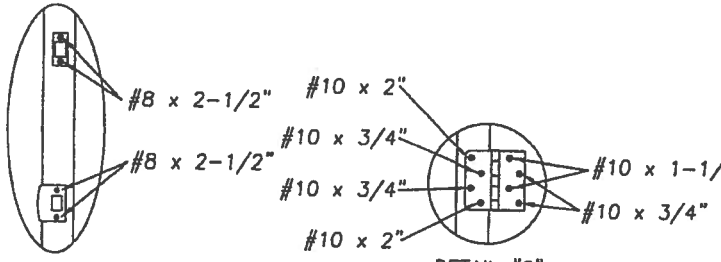
PRODUCT: "EXTERIOR DOOR PRODUCT" DOUBLE 6'8" OPAQUE FIBERGLASS DOOR

PART OR ASSEMBLY: TYPICAL ELEVATIONS & GENERAL NOTES

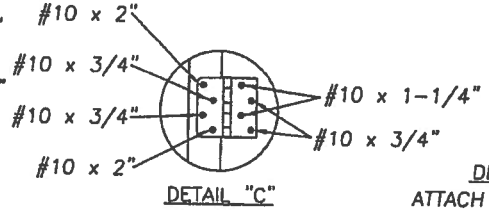
NO | DATE | BY

DATE: 5/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0124-
SHEET 1 OF 3

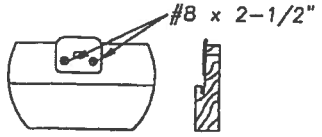




DETAIL "D"

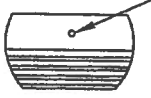


DETAIL "C"



DETAIL "E" ASTRAGAL

ATTACH ASTRAGAL RETAINER BOLT STRIKE PLATE TO FRAME AS SHOWN.



ASTRAGAL RETAINER BOLT HOLE MUST BE DRILLED THROUGH THE THRESHOLD & INTO THE STRUCTURE DEEP ENOUGH FOR A 1.375" THROW

DETAIL "F" ASTRAGAL

INSWING THRESHOLD

OUTSWING THRESHOLD

HIGH DAM O/S THRESHOLD

Addendum to NAMI

Certification No: NT.001063R

Reviewed By:

Date Reviewed: 7/18/05

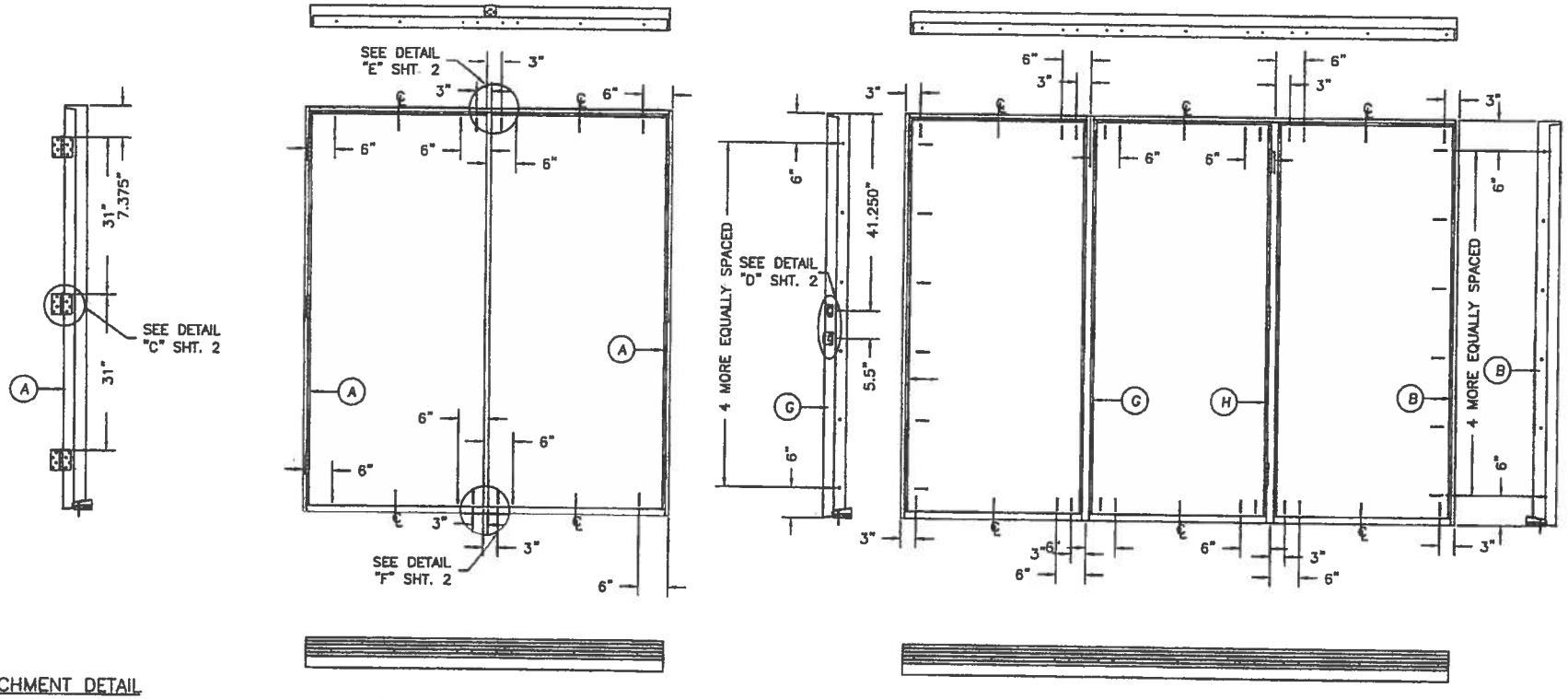
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-8" FIBERGLASS OPAQUE DOUBLE DOOR UNIT

PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS

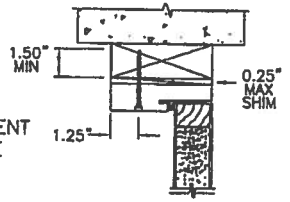
NO.	DATE	REVISIONS	BY

DATE: 5/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0124-05
SHEET 2 OF 3



ATTACHMENT DETAIL

1. ANCHOR ANALYSIS FOR LOADING CONDITIONS PREPARED, SIGNED AND SEALED BY HAROLD E. RUPP, PE (FLORIDA #15935) WITH THE LOWEST (LEAST) FASTENER RATING FROM THE DIFFERENT FASTENERS BEING CONSIDERED FOR USE. JAMB, HEAD, AND THRESHOLD FASTENERS ANALYZED FOR THIS UNIT INCLUDE #10 WOOD SCREWS OR 3/16" TAPCONS. A PHYSICAL SHIM MUST BE PLACED IN SHIM SPACE AT EACH ANCHOR LOCATION.
2. THE WOOD SCREW SINGLE SHEAR DESIGN VALUES COME FROM ANSI/AF&PA NDA FOR SOUTHERN PINE LUMBER AND ACHEIVEMENT OF 1-1/2" MINIMUM EMBEDMENT. THE TAPCON MUST ACHIEVE MINIMUM EMBEDMENT OF 1-1/4".
3. WOOD BUCKS BY OTHERS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE.
4. MINIMUM DESIGN VALUE STRENGTH OF ANCHORS 171 LBS.



TYPICAL ANCHOR INSTALLATION

HARDWARE SCHEDULE

1.	KWIKSET OR SCHLEGE ANSI/BHMA GRADE 3 OR BETTER CYLINDRICAL AND DEADLOCK HARDWARE TO BE INSTALLED AT 5-1/2" CENTERLINE.
2.	4" X 4" FULL MORTISE BUTT HINGES.

Addendum to NAMI
Certification No.: NTL00606R
Reviewed By:
Date Reviewed: 7/18/05

MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-8" FIBERGLASS OPAQUE DOUBLE DOOR UNIT
PART OR ASSEMBLY: ANCHORING LOCATIONS

BY	DATE	NO

DATE: 5/25/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0124-
SHEET 3 OF 3

26401

NOTICE OF COMMENCEMENT

This document prepared by and to be returned to:
Kyle E. Petteway
Grunder & Petteway, P. A.
23349 NW CR 236, Suite 10
High Springs, Florida, 32643

PERMIT NO.:_____
TAX PARCEL NUMBER R14330-111

STATE OF FLORIDA
COUNTY OF ALACHUA

Inst:200712027161 Date:12/10/2007 Time:3:09 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 2

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. **Description of Property**:

Lot 16, FORT WHITE MANOR, according to the map or plat thereof as recorded in Plat Book 6, Page 30, of the Public Records of Columbia County, Florida.

Address:

2. General description of improvement: Residential Home Construction

3. Owner information:
   a. Name and address:

      John R. Teele and Kimberly Teele
      PO Box 176
      Ft. White, Fl 32038

   b. Phone number: 386-497-3360
   c. Fax number:
   d. Interest in property: Fee simple title holder
   e. Name and address of fee simple titleholder (if other than owner): N/A

4. Contractor: Owner/Builder

   a. Phone number:

5. Surety on any payment bond: None

6. Lender: Millennium Bank
   4340 Newberry Road
   Gainesville, Fl 32607
   a. Phone number: (352) 352-335-4035

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided for by Section 713.13 (1) (a) 7, Florida Statutes: N/A

   a. Phone number:

8. In addition to himself, Owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes:
   None

   a. Phone number:
   b. Fax number:

9. Expiration Date of Notice of Commencement (the expiration is one year from date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

John R. Teele

Kimberly Teele

STATE OF FLORIDA
COUNTY OF ALACHUA

SWORN TO and subscribed before me this 6th day of December, 2007 by John R. Teele and Kimberly Teele who

(√) are personally known to me
( ) produced a valid Florida driver's license as identification
( ) produced _____ as identification

Signature of Notary
(SEAL)

Notary Public State of Florida
Kyle E Petteway
My Commission DD680638
Expires 08/01/2011

Verification pursuant to Section 95.525, Florida Statutes.

Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

John R. Teele

Kimberly Teele

8838

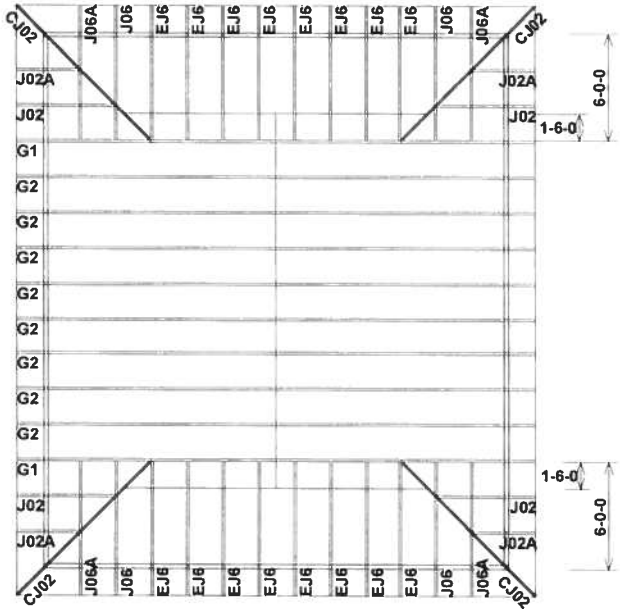
STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By _____ Deputy Clerk
Date Dec 11 2007

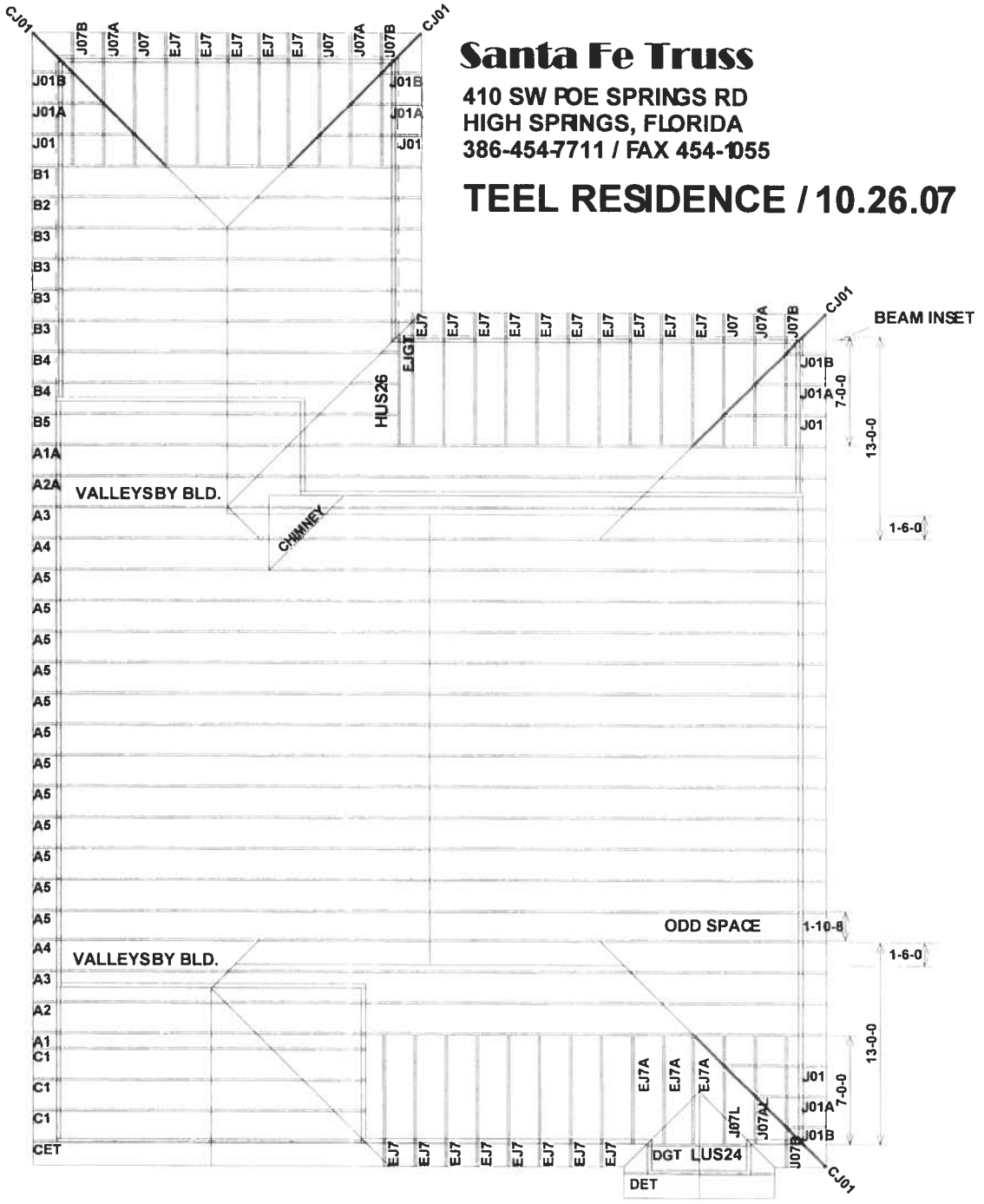
CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

Santa Fe Truss

410 SW POE SPRINGS RD
HIGH SPRINGS, FLORIDA
386-454-7711 / FAX 454-1055

TEEL RESIDENCE / 10.26.07





ENGINEERING BY
TRENCO™
A MiTek Affiliate

RE: TEEL - TEELE RES AND GARAGE

TRENCO
818 Soundside Road
Edenton, NC 27932
Telephone 252/482-7000
Fax 252/482-7115

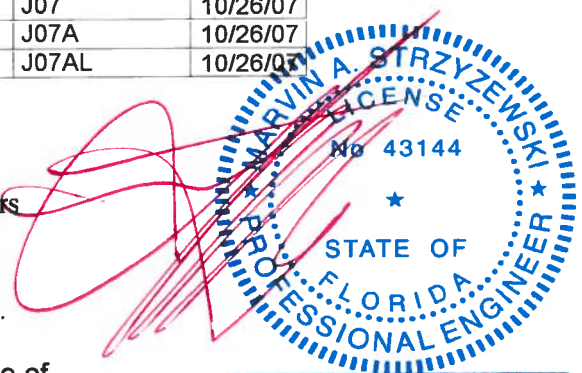
Site Information:
Project Customer: Project Name:
Lot/Block: Subdivision:
Address: 6129 SW CR18
City: FORT WHITE State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 6.5
Wind Code: ASCE 7-02 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 36 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E4483116	A1	10/26/07	18	E4483133	DGT	10/26/07
2	E4483117	A1A	10/26/07	19	E4483134	EJ6	10/26/07
3	E4483118	A2	10/26/07	20	E4483135	EJ7	10/26/07
4	E4483119	A2A	10/26/07	21	E4483136	EJ7A	10/26/07
5	E4483120	A3	10/26/07	22	E4483137	EJGT	10/26/07
6	E4483121	A4	10/26/07	23	E4483138	G1	10/26/07
7	E4483122	A5	10/26/07	24	E4483139	G2	10/26/07
8	E4483123	B1	10/26/07	25	E4483140	J01	10/26/07
9	E4483124	B2	10/26/07	26	E4483141	J01A	10/26/07
10	E4483125	B3	10/26/07	27	E4483142	J01B	10/26/07
11	E4483126	B4	10/26/07	28	E4483143	J02	10/26/07
12	E4483127	B5	10/26/07	29	E4483144	J02A	10/26/07
13	E4483128	C1	10/26/07	30	E4483145	J06	10/26/07
14	E4483129	CET	10/26/07	31	E4483146	J06A	10/26/07
15	E4483130	CJ01	10/26/07	32	E4483147	J07	10/26/07
16	E4483131	CJ02	10/26/07	33	E4483148	J07A	10/26/07
17	E4483132	DET	10/26/07	34	E4483149	J07AL	10/26/07



The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.
Truss Design Engineer's Name: Strzyzewski, Marvin
My license renewal date for the state of is February 28, 2009.

Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co
818 Soundside Road
Edenton, NC 27932
FL COA #7239

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

October 26,2007
Strzyzewski, Marvin

ENGINEERING BY
TRENCO™
A MiTek Affiliate

RE: TEEL - TEELE RES AND GARAGE

TRENCO
818 Soundside Road
Edenton, NC 27932
Telephone 252/482-7000
Fax 252/482-7115

Site Information:
Project Customer: Project Name:
Lot/Block: Subdivision:
Address: 6129 SW CR18
City: FORT WHITE State: FLORIDA

No.	Seal#	Truss Name	Date
35	E4483150	J07B	10/26/07
36	E4483151	J07L	10/26/07

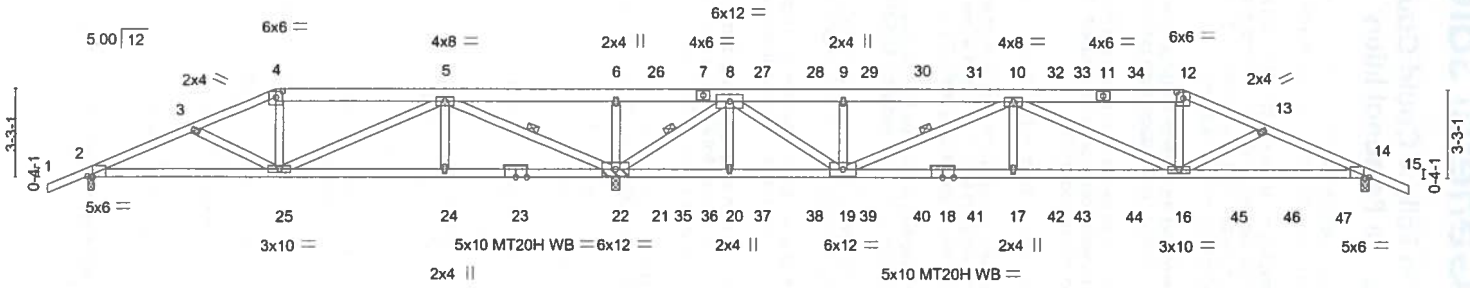
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483116
TEEL	A1	HIP	1	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:01 2007 Page 1

1-6-0	4-0-8	7-0-0	13-4-6	19-8-12	24-0-0	28-3-4	34-7-10	41-0-0	43-11-8	48-0-0	49-6-0
1-6-0	4-0-8	2-11-8	6-4-6	6-4-6	4-3-4	4-3-4	6-4-6	6-4-6	2-11-8	4-0-8	1-6-0

Scale = 1:86.2

Hanger nails that extend 1/2" or more beyond back of girder shall be clinched.



7-0-0	13-4-6	19-8-12	24-0-0	28-3-4	34-7-10	41-0-0	48-0-0
7-0-0	6-4-6	6-4-6	4-3-4	4-3-4	6-4-6	6-4-6	7-0-0

Plate Offsets (X,Y): [2:0-2-5,Edge], [4:0-3-0,0-2-9], [12:0-3-0,0-2-9], [14:0-2-5,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.19	17	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.50	17-19	>675	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.10	14	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 269 lb	

LUMBER
TOP CHORD 2 X 6 SYP 2400F 2.0E *Except*
1-4 2 X 4 SYP No.2D, 12-15 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

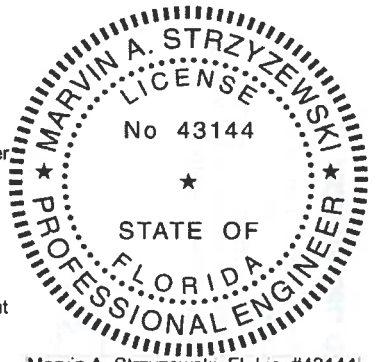
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-22, 8-22, 10-19

REACTIONS (lb/size) 2=369/0-3-0, 22=4009/0-3-8, 14=1931/0-3-8
Max Horz 2=56(LC 5)
Max Uplift 2=-36(LC 5), 22=-909(LC 3), 14=-471(LC 6)
Max Grav 2=378(LC 7), 22=4009(LC 1), 14=1937(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-315/116, 3-4=-232/200, 4-5=-203/186, 5-6=-737/3316, 6-26=-737/3316, 7-26=-737/3316, 7-8=-737/3316, 8-27=-2660/767, 27-28=-2660/767, 9-28=-2660/767, 9-29=-2660/767, 29-30=-2660/767, 30-31=-2660/767, 10-31=-2660/767, 10-32=-3553/922, 32-33=-3553/922, 11-33=-3553/922, 11-34=-3553/923, 12-34=-3550/922, 12-13=-3800/972, 13-14=-3900/943, 14-15=0/33
BOT CHORD 2-25=-87/268, 24-25=-1192/517, 23-24=-1192/517, 22-23=-1192/517, 21-22=-70/104, 21-35=-70/104, 35-36=-70/104, 20-36=-70/104, 20-37=-70/104, 37-38=-70/104, 19-38=-70/104, 19-39=-1109/4332, 39-40=-1109/4332, 18-40=-1109/4332, 18-41=-1109/4332, 17-41=-1109/4332, 17-42=-1109/4332, 42-43=-1109/4332, 43-44=-1109/4332, 16-44=-1109/4332, 16-45=-820/3535, 45-46=-820/3535, 46-47=-820/3535, 14-47=-820/3535
WEBS 3-25=-248/100, 4-25=-282/150, 5-25=-262/1249, 5-24=0/229, 5-22=-2389/427, 6-22=-565/200, 8-22=-4057/1044, 8-20=0/313, 8-19=-766/3095, 9-19=-704/290, 10-19=-1852/465, 10-17=0/539, 10-16=-919/300, 12-16=-46/875, 13-16=-39/132

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 2, 909 lb uplift at joint 22 and 471 lb uplift at joint 14.

Continued on page 2



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FL COA #7239

October 26,2007

NOTES

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 130 lb down and 75 lb up at 21-0-12, 130 lb down and 75 lb up at 23-0-12, 130 lb down and 75 lb up at 25-0-12, 130 lb down and 75 lb up at 27-0-12, 130 lb down and 75 lb up at 29-0-12, 130 lb down and 75 lb up at 31-0-12, 130 lb down and 75 lb up at 33-0-12, 130 lb down and 75 lb up at 35-0-12, 141 lb down and 84 lb up at 37-0-12, and 141 lb down and 84 lb up at 39-0-12, and 295 lb down and 155 lb up at 41-0-0 on top chord, and 96 lb down at 21-0-12, 96 lb down at 23-0-12, 96 lb down at 25-0-12, 96 lb down at 27-0-12, 96 lb down at 29-0-12, 96 lb down at 31-0-12, 96 lb down at 33-0-12, 96 lb down at 35-0-12, 96 lb down at 37-0-12, 96 lb down at 39-0-12, 96 lb down at 40-11-4, 98 lb down at 42-11-4, and 114 lb down at 44-11-4, and 98 lb down at 46-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-12=-60, 12-15=-60, 2-14=-20

Concentrated Loads (lb)

Vert: 7=-130(F) 12=-255(F) 16=-48(F) 26=-130(F) 27=-130(F) 28=-130(F) 29=-130(F) 30=-130(F) 31=-130(F) 32=-130(F) 33=-141(F) 34=-141(F) 35=-48(F) 36=-48(F) 37=-48(F) 38=-48(F) 39=-48(F) 40=-48(F) 41=-48(F) 42=-48(F) 43=-48(F) 44=-48(F) 45=-49(F) 46=-88(F) 47=-49(F)

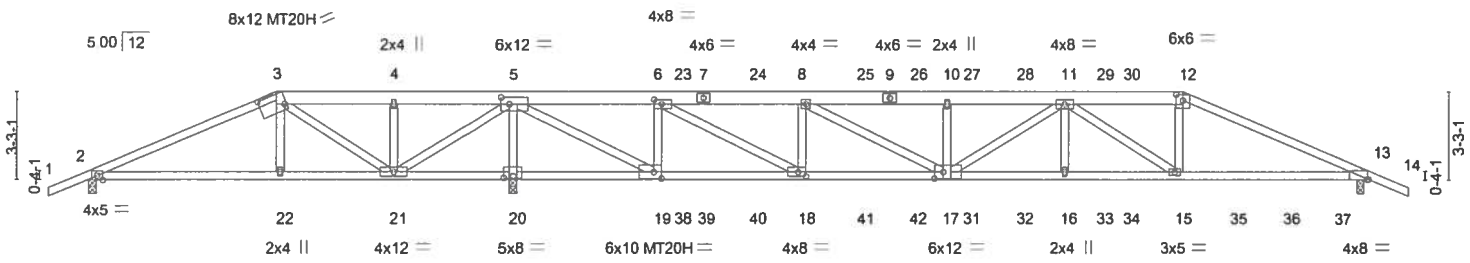
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483117
TEEL	A1A	HIP	1	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL. 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:03 2007 Page 1

1-6-0	7-0-0	11-5-2	15-10-4	21-3-7	26-8-9	32-1-12	36-6-14	41-0-0	48-0-0	49-6-0
1-6-0	7-0-0	4-5-2	4-5-2	5-5-3	5-5-3	5-5-3	4-5-2	4-5-2	7-0-0	1-6-0

Scale = 1:86.3

Hanger nails that extend 1/2" or more beyond back of girder shall be clinched.



7-0-0	11-5-2	15-10-4	21-3-7	26-8-9	32-1-12	36-6-14	41-0-0	47-10-0	48-0-0
7-0-0	4-5-2	4-5-2	5-5-3	5-5-3	5-5-3	4-5-2	4-5-2	6-10-0	0-2-0

Plate Offsets (X,Y): [3:0-11-0,0-5-8], [5:0-3-8,0-3-0], [6:0-3-8,0-2-0], [12:0-3-0,0-2-9], [13:0-0-6,0-0-2], [17:0-3-12,0-3-0], [18:0-3-8,0-2-0], [19:0-3-8,0-3-0], [20:0-4-0,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	-0.28	16-17	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.73	16-17	>528	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.08	13	n/a	n/a		
BCDL	10.0	Code	FBC2004/TPI2002	(Matrix)							Weight: 262 lb	

LUMBER
TOP CHORD 2 X 6 SYP 2400F 2.0E *Except*
1-3 2 X 4 SYP No.2D, 12-14 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
5-19 2 X 4 SYP 2400F 2.0E, 6-18 2 X 4 SYP 2400F 2.0E
8-17 2 X 4 SYP 2400F 2.0E

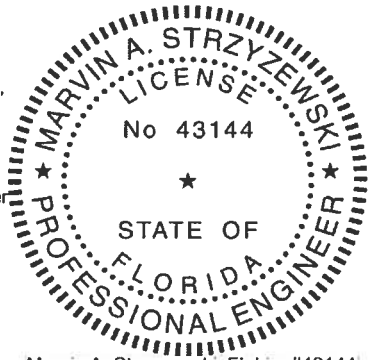
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-7-8 oc bracing.

REACTIONS (lb/size) 2=-96/0-3-8, 20=4677/0-3-8, 13=2239/0-3-0
Max Horz 2=56(LC 5)
Max Uplift2=-370(LC 8), 20=-971(LC 3), 13=-518(LC 6)
Max Grav 2=141(LC 6), 20=4677(LC 1), 13=2244(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-383/1347, 3-4=-603/2597, 4-5=-603/2596, 5-6=-1321/340, 6-23=-4615/1169, 7-23=-4615/1169, 7-24=-4615/1169, 8-24=-4615/1169, 8-25=-5857/1482, 9-25=-5857/1482, 9-26=-5857/1482, 10-26=-5857/1482, 10-27=-5857/1482, 27-28=-5857/1482, 11-28=-5857/1482, 11-29=-4383/1074, 29-30=-4382/1074, 12-30=-4380/1074, 12-13=-4767/1133, 13-14=0/33
BOT CHORD 2-22=-1209/434, 21-22=-1202/436, 20-21=-4318/1006, 17-31=-1342/5584, 31-32=-1342/5584, 16-32=-1342/5584, 16-33=-1342/5584, 33-34=-1342/5584, 15-34=-1342/5584, 15-35=-986/4325, 35-36=-986/4325, 36-37=-986/4325, 13-37=-986/4325, 19-20=-4318/1006, 19-38=-254/1321, 38-39=-254/1321, 39-40=-254/1321, 18-40=-254/1321, 18-41=-1088/4615, 41-42=-1088/4615, 17-42=-1088/4615
WEBS 3-22=0/274, 3-21=-1875/363, 4-21=-129/61, 5-21=-386/2096, 5-20=-4578/1014, 5-19=-1425/6332, 6-19=-2242/673, 6-18=-943/3740, 8-18=-1408/487, 8-17=-357/1415, 10-17=-632/261, 11-17=-76/340, 11-16=0/301, 11-15=-1571/432, 12-15=-176/1167

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2, 971 lb uplift at joint 20 and 518 lb uplift at joint 13.

Continued on page 2



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October 26,2007

NOTES

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 142 lb down and 82 lb up at 22-0-12, 130 lb down and 75 lb up at 22-11-4, 130 lb down and 75 lb up at 24-11-4, 130 lb down and 75 lb up at 26-11-4, 130 lb down and 75 lb up at 28-11-4, 130 lb down and 75 lb up at 30-11-4, 130 lb down and 75 lb up at 32-11-4, 130 lb down and 75 lb up at 34-11-4, 130 lb down and 75 lb up at 36-11-4, and 130 lb down and 75 lb up at 38-11-4, and 284 lb down and 146 lb up at 41-0-0 on top chord, and 623 lb down and 59 lb up at 22-0-12, 96 lb down at 22-11-4, 96 lb down at 24-11-4, 96 lb down at 26-11-4, 96 lb down at 28-11-4, 96 lb down at 30-11-4, 96 lb down at 32-11-4, 96 lb down at 34-11-4, 96 lb down at 36-11-4, 96 lb down at 38-11-4, 96 lb down at 40-11-4, 96 lb down at 42-11-4, and 96 lb down at 44-11-4, and 98 lb down at 46-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

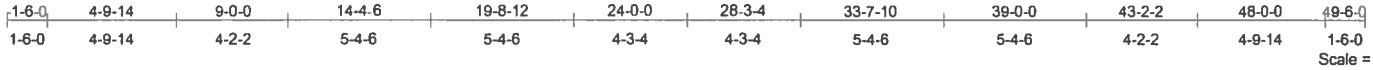
Vert: 1-3=-60, 3-12=-60, 12-14=-60, 2-17=-20, 13-17=-20

Concentrated Loads (lb)

Vert: 7=-130(B) 12=-244(B) 18=-48(B) 8=-130(B) 15=-48(B) 23=-142(B) 24=-130(B) 25=-130(B) 26=-130(B) 27=-130(B) 28=-130(B) 29=-130(B) 30=-130(B) 31=-48(B) 32=-48(B) 33=-48(B) 34=-48(B) 35=-48(B) 36=-48(B) 37=-49(B) 38=-623(B) 39=-48(B) 40=-48(B) 41=-48(B) 42=-48(B)

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483118
TEEL	A2	HIP	1	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Apr 2 2007 MiTek Industries, Inc Fri Oct 26 14:54:05 2007 Page 1



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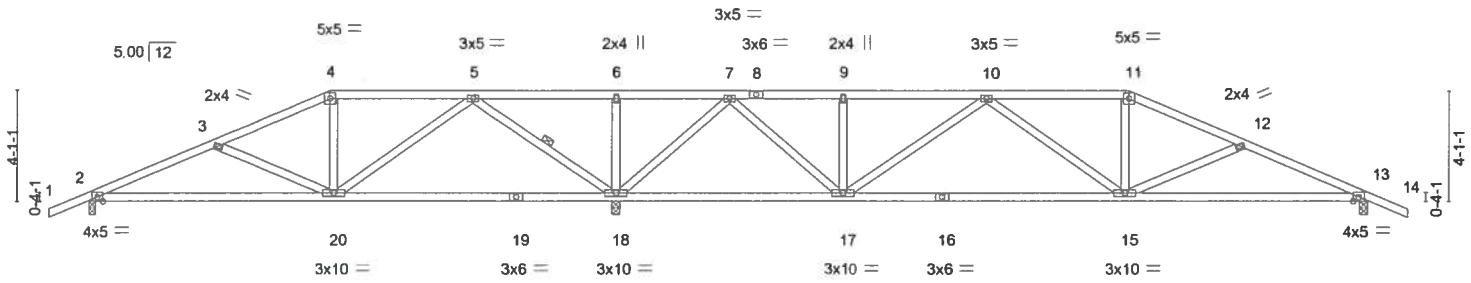

9-0-0 19-8-12 28-3-4 39-0-0 48-0-0
9-0-0 10-8-12 8-6-8 10-8-12 9-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.21	15-17	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.56	15-17	>599	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.05	13	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 238 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-18

REACTIONS (lb/size) 2=582/0-3-0, 18=2417/0-3-8, 13=1015/0-3-8
Max Horz 2=-67(LC 6)
Max Uplift2=-142(LC 5), 18=-316(LC 3), 13=-159(LC 6)
Max Grav 2=606(LC 7), 18=2417(LC 1), 13=1029(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-824/142, 3-4=-490/66, 4-5=-402/80, 5-6=-142/1485, 6-7=-142/1485, 7-8=-805/153, 8-9=-805/153, 9-10=-805/153, 10-11=-1406/187, 11-12=-1559/186, 12-13=-1874/222, 13-14=0/33
BOT CHORD 2-20=-122/723, 19-20=-360/85, 18-19=-360/85, 17-18=-248/78, 16-17=-154/1341, 15-16=-154/1341, 13-15=-158/1671
WEBS 3-20=-347/136, 4-20=-141/84, 5-20=-9/832, 5-18=-1418/222, 6-18=-306/110, 7-18=-1656/260, 7-17=-131/1311, 9-17=-293/108, 10-17=-681/128, 10-15=-29/205, 11-15=0/356, 12-15=-303/134

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 2, 316 lb uplift at joint 18 and 159 lb uplift at joint 13.

LOAD CASE(S) Standard

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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483119
TEEL	A2A	HIP	1	1	Job Reference (optional)	

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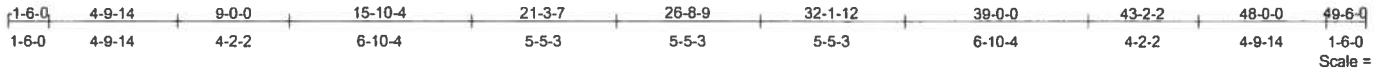

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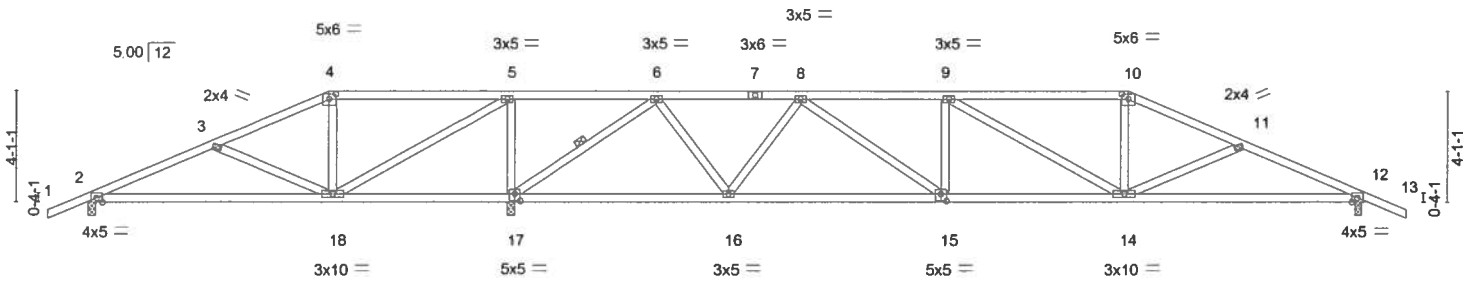


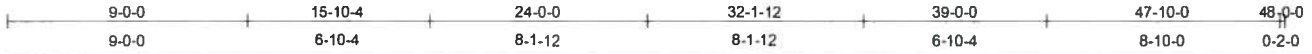


Plate Offsets (X,Y): [4:0-3-0,0-2-4], [10:0-3-0,0-2-4], [15:0-2-8,0-3-0], [17:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.15	12-14	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.42	12-14	>913	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.05	12	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 239 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

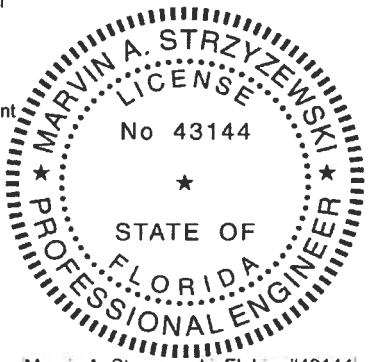
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-6 oc bracing.
WEBS 1 Row at midpt 6-17

REACTIONS (lb/size) 2=316/0-3-8, 17=2522/0-3-8, 12=1176/0-3-0
Max Horz 2=-67(LC 6)
Max Uplift 2=-138(LC 5), 17=-327(LC 3), 12=-161(LC 6)
Max Grav 2=346(LC 7), 17=2522(LC 1), 12=1186(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-157/355, 3-4=-53/470, 4-5=-21/426, 5-6=-147/1631, 6-7=-824/159, 7-8=-824/159, 8-9=-1953/317, 9-10=-1783/255, 10-11=-1956/257, 11-12=-2241/281, 12-13=0/33
BOT CHORD 2-18=-300/114, 17-18=-1631/250, 16-17=-8/175, 15-16=-163/1352, 14-15=-216/1953, 12-14=-210/2019
WEBS 3-18=-353/126, 4-18=-455/133, 5-18=-148/1534, 5-17=-1159/217, 6-17=-2123/311, 6-16=-80/1126, 8-16=-918/188, 8-15=-65/750, 9-15=-292/106, 9-14=-313/78, 10-14=0/442, 11-14=-274/125

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2, 327 lb uplift at joint 17 and 161 lb uplift at joint 12.

LOAD CASE(S) Standard



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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483120
TEEL	A3	HIP	2	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc Fri Oct 26 14:54:07 2007 Page 1

Scale = 1:86.2

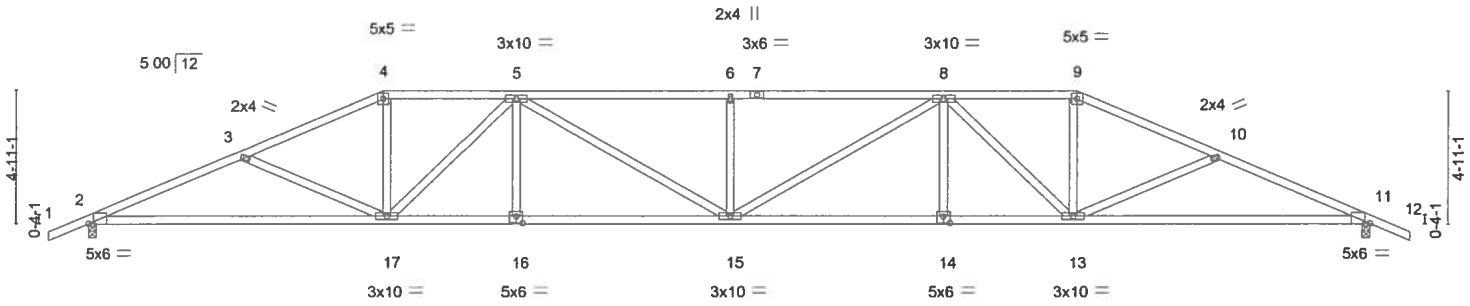


Plate Offsets (X,Y): [2:0-2-5,Edge], [11:0-2-5,Edge], [14:0-3-0,0-3-0], [16:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.44	15	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-1.14	15-16	>503	180		
BCLL	0.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.28	11	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 245 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

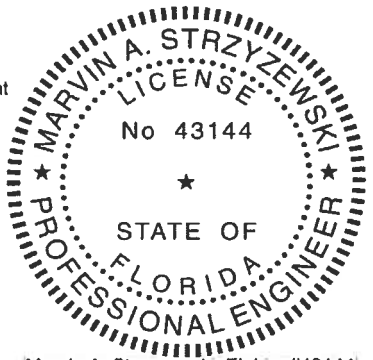
BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=2007/0-3-8, 11=2007/0-3-8
Max Horz 2=76(LC 5)
Max Uplift2=-248(LC 3), 11=-248(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-4233/523, 3-4=-3851/477, 4-5=-3531/462, 5-6=-4997/696, 6-7=-4997/696, 7-8=-4997/696, 8-9=-3531/462, 9-10=-3851/477, 10-11=-4233/523, 11-12=0/33
BOT CHORD 2-17=-414/3846, 16-17=-490/4402, 15-16=-490/4402, 14-15=-495/4402, 13-14=-495/4402, 11-13=-426/3846
WEBS 3-17=-387/171, 4-17=-94/1174, 5-17=-1294/219, 5-16=0/186, 5-15=-99/780, 6-15=-510/188, 8-15=-98/780, 8-14=0/186, 8-13=-1294/219, 9-13=-94/1174, 10-13=-387/171

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 2 and 248 lb uplift at joint 11.

LOAD CASE(S) Standard



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818 Soundside Road
Edenton, NC 27932
FL COA #7239

October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	
TEEL	A4	HIP	2	1	Job Reference (optional)	E4483121

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:08 2007 Page 1

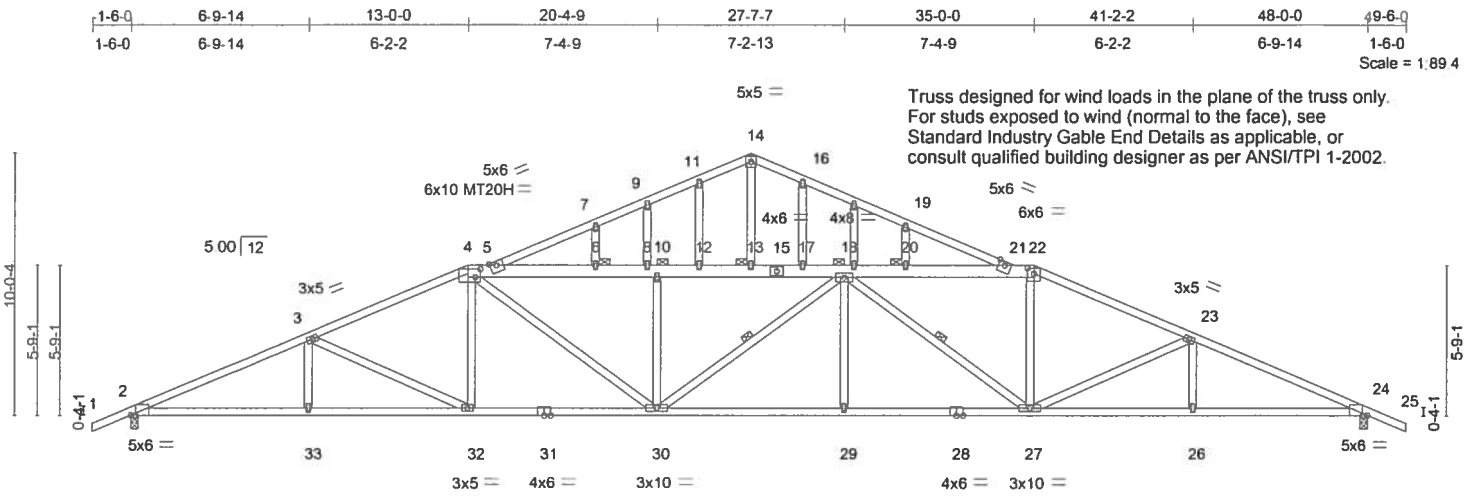


Plate Offsets (X,Y): [2:0-2-5,Edge], [4:0-2-8,0-3-12], [22:0-3-0,0-2-9], [24:0-2-5,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.31	29-30	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.83	29-30	>693	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.30	24	n/a	n/a		
BCDL	10.0	Code	FBC2004/TPI2002	(Matrix)							Weight: 325 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D *Except* 4-15 2 X 6 SYP 2400F 2.0E, 15-22 2 X 6 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

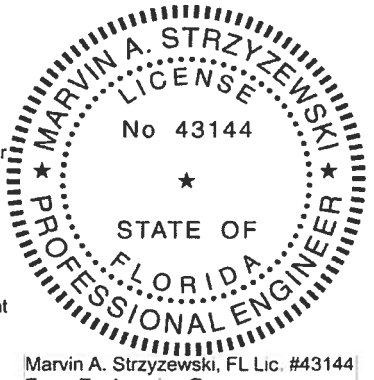
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-11 oc purlins. Except:
5-7-0 oc bracing: 10-13, 13-18
5-8-0 oc bracing: 5-6, 6-10
6-0-0 oc bracing: 18-20, 20-21
BOT CHORD Rigid ceiling directly applied or 9-9-2 oc bracing.
WEBS 1 Row at midpt 18-30, 18-27
JOINTS 1 Brace at Jt(s): 10, 18, 6, 13, 20

REACTIONS (lb/size) 2=2007/0-3-8, 24=2007/0-3-8
Max Horz 2=-137(LC 6)
Max Uplift 2=-274(LC 5), 24=-274(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-4277/432, 3-4=-3686/393, 4-5=-3736/389, 5-6=-3056/426, 6-8=-3056/426, 8-10=-3056/426, 10-12=-3056/426, 12-13=-3056/426, 13-15=-3056/426, 15-17=-3056/426, 17-18=-3056/426, 18-20=-2666/431, 20-21=-2666/431, 21-22=-3346/389, 22-23=-3689/393, 23-24=-4275/432, 24-25=0/33, 5-7=-826/42, 7-9=-782/66, 9-11=-775/84, 11-14=-772/104, 14-16=-778/110, 16-19=-796/86, 19-21=-822/49
BOT CHORD 2-33=-444/3857, 32-33=-444/3857, 31-32=-326/3360, 30-31=-326/3360, 29-30=-226/3746, 28-29=-226/3746, 27-28=-226/3746, 26-27=-308/3855, 24-26=-308/3855
WEBS 3-33=0/268, 3-32=-581/129, 4-32=0/447, 4-30=-110/498, 10-30=-213/148, 18-30=-125/384, 18-29=0/289, 18-27=-501/104, 22-27=-5/659, 23-27=-612/130, 23-26=0/267, 6-7=-161/78, 8-9=-107/55, 11-12=-105/55, 13-14=-51/399, 16-17=-168/97, 19-20=-215/110

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 2x4 MT20 unless otherwise indicated.
7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 274 lb uplift at joint 24.
9) Design requires purlins at oc spacing indicated.

LOAD CASE(S) Standard



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October 26,2007

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483122
TEEL	A5	COMMON	12	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:09 2007 Page 1

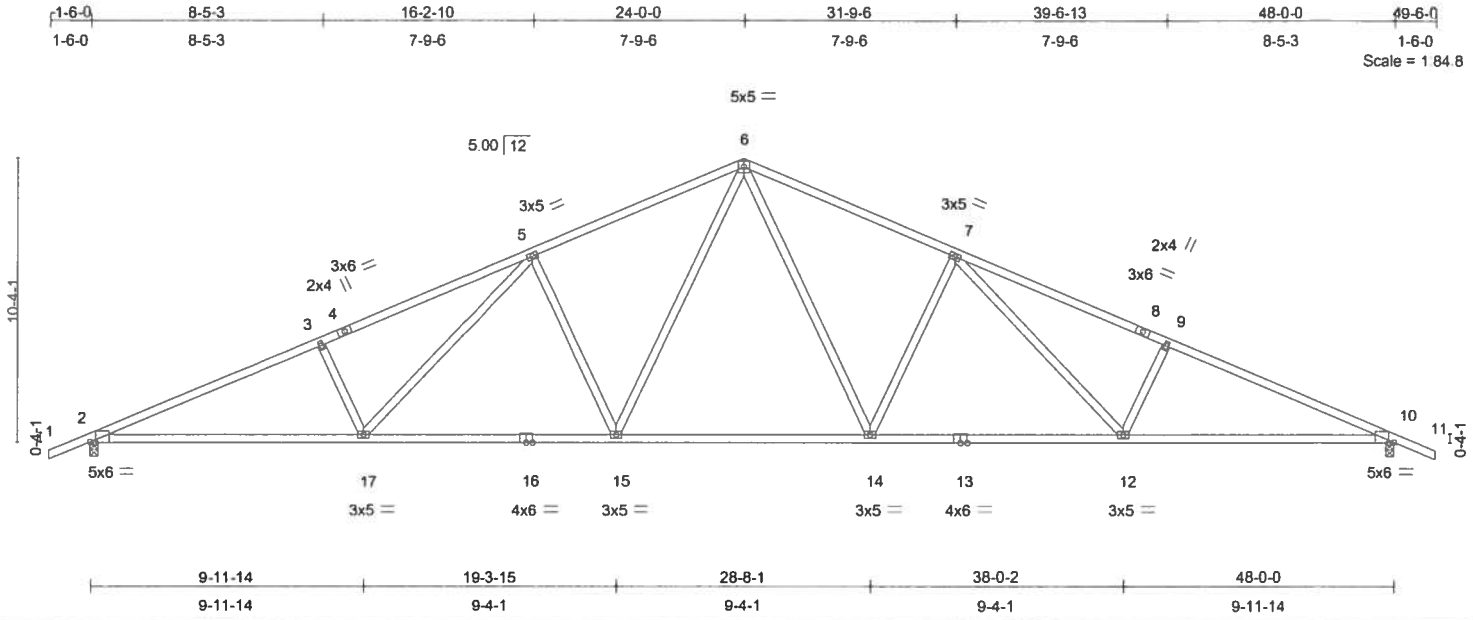


Plate Offsets (X,Y): [2:0-2-5,Edge], [10:0-2-5,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.29	14-15	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.86	14-15	>664	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.25	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 245 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

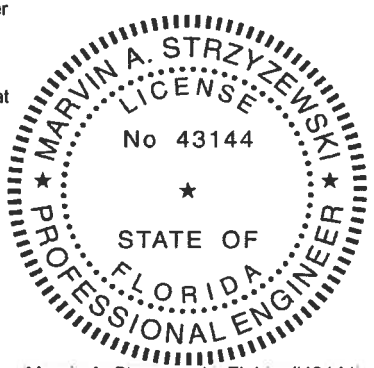
BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 9-8-7 oc bracing.

REACTIONS (lb/size) 2=2007/0-3-8, 10=2007/0-3-8
Max Horz 2=-140(LC 6)
Max Uplift2=-274(LC 5), 10=-274(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-4183/437, 3-4=-3975/433, 4-5=-3882/461, 5-6=-3008/375, 6-7=-3008/375, 7-8=-3882/461, 8-9=-3975/434, 9-10=-4183/437, 10-11=0/33
BOT CHORD 2-17=-440/3766, 16-17=-277/3018, 15-16=-277/3018, 14-15=-106/2284, 13-14=-138/3018, 12-13=-138/3018, 10-12=-300/3766
WEBS 3-17=-441/201, 5-17=-118/841, 5-15=-794/254, 6-15=-153/995, 6-14=-153/995, 7-14=-794/254, 7-12=-118/841, 9-12=-441/201

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 274 lb uplift at joint 10.

LOAD CASE(S) Standard



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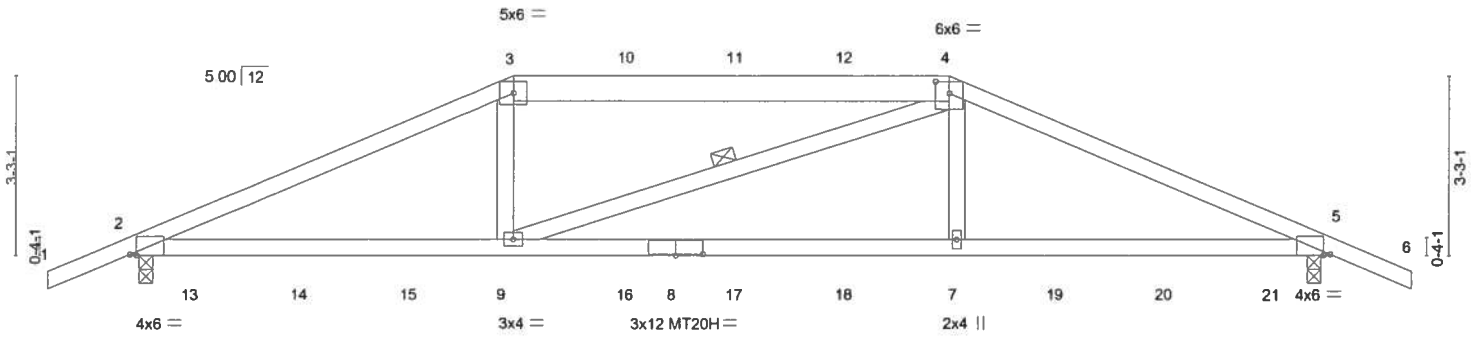
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483123
TEEL	B1	HIP	1	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:11 2007 Page 1

Scale = 1:42.3

Hanger nails that extend 1/2" or more beyond back of girder shall be clinched.



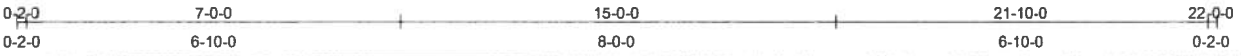


Plate Offsets (X,Y): [2:0-1-6,0-0-2], [4:0-3-0,0-2-9], [5:0-1-6,0-0-2], [8:0-5-15,0-0-5]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.16	7-9	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.48	7-9	>539	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.10	5	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP 2400F 2.0E *Except* 3-4 2 X 6 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-9

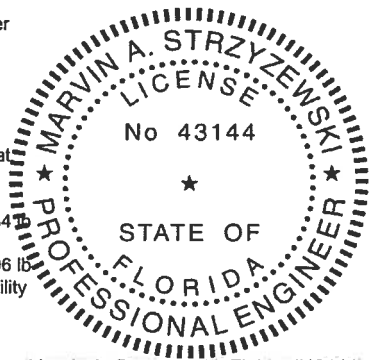
REACTIONS (lb/size) 2=1671/0-3-0, 5=1674/0-3-0
Max Horz 2=-56(LC 6)
Max Uplift2=-395(LC 5), 5=-388(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-3360/755, 3-10=-3034/737, 10-11=-3034/737, 11-12=-3034/737, 4-12=-3034/737, 4-5=-3364/752, 5-6=0/33
BOT CHORD 2-13=-665/3052, 13-14=-665/3052, 14-15=-665/3052, 9-15=-665/3052, 9-16=-644/3036, 8-16=-644/3036, 8-17=-644/3036, 17-18=-644/3036, 7-18=-644/3036, 7-19=-640/3054, 19-20=-640/3054, 20-21=-640/3054, 5-21=-640/3054
WEBS 3-9=0/714, 4-9=-185/143, 4-7=0/717

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 395 lb uplift at joint 2 and 388 lb uplift at joint 5.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 244 lb down and 146 lb up at 7-0-0, 130 lb down and 75 lb up at 9-0-12, 130 lb down and 75 lb up at 11-0-12, and 130 lb down and 75 lb up at 13-0-12, and 284 lb down and 146 lb up at 15-0-0 on top chord, and 98 lb down at 1-0-12, 96 lb down at 3-0-12, 96 lb down at 5-0-12, 96 lb down at 7-0-12, 96 lb down at 9-0-12, 96 lb down at 11-0-12, 96 lb down at 13-0-12, 96 lb down at 14-11-4, 96 lb down at 16-11-4, and 96 lb down at 18-11-4, and 98 lb down at 20-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2



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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-6=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 3=-244(B) 4=-244(B) 9=-48(B) 7=-48(B) 10=-130(B) 11=-130(B) 12=-130(B) 13=-49(B) 14=-48(B) 15=-48(B) 16=-48(B) 17=-48(B) 18=-48(B) 19=-48(B) 20=-48(B) 21=-49(B)

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483124
TEEL	B2	HIP	1	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc Fri Oct 26 14:54:11 2007 Page 1

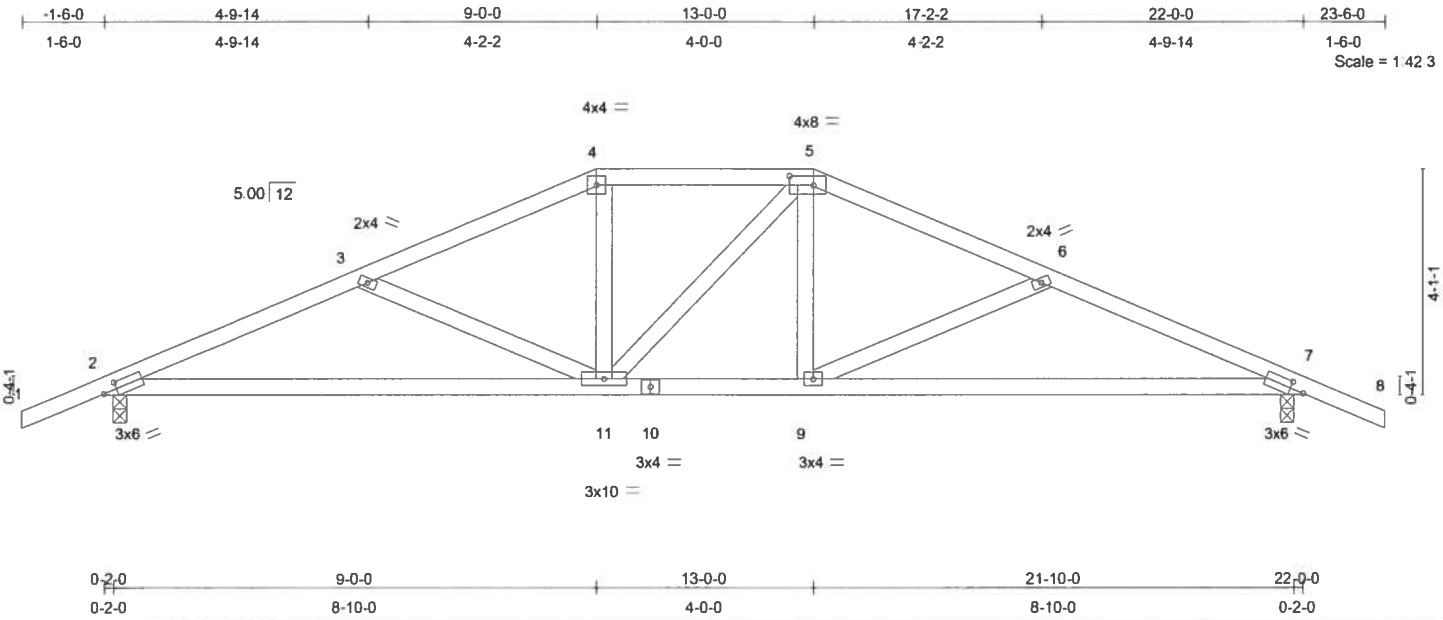

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-5-4,0-2-0], [7:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.15	7-9	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.40	7-9	>654	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.05	7	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 104 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=968/0-3-0, 7=968/0-3-0
Max Horz 2=-67(LC 6)
Max Uplift2=-151(LC 5), 7=-151(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-1710/171, 3-4=-1379/123, 4-5=-1234/127, 5-6=-1378/123, 6-7=-1710/172, 7-8=0/33
BOT CHORD 2-11=-147/1536, 10-11=-27/1233, 9-10=-27/1233, 7-9=-101/1536
WEBS 3-11=-343/131, 4-11=0/323, 5-11=-114/116, 5-9=0/323, 6-9=-344/131

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 2 and 151 lb uplift at joint 7.

LOAD CASE(S) Standard

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FL COA #7239

October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483125
TEEL	B3	COMMON	4	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:12 2007 Page 1

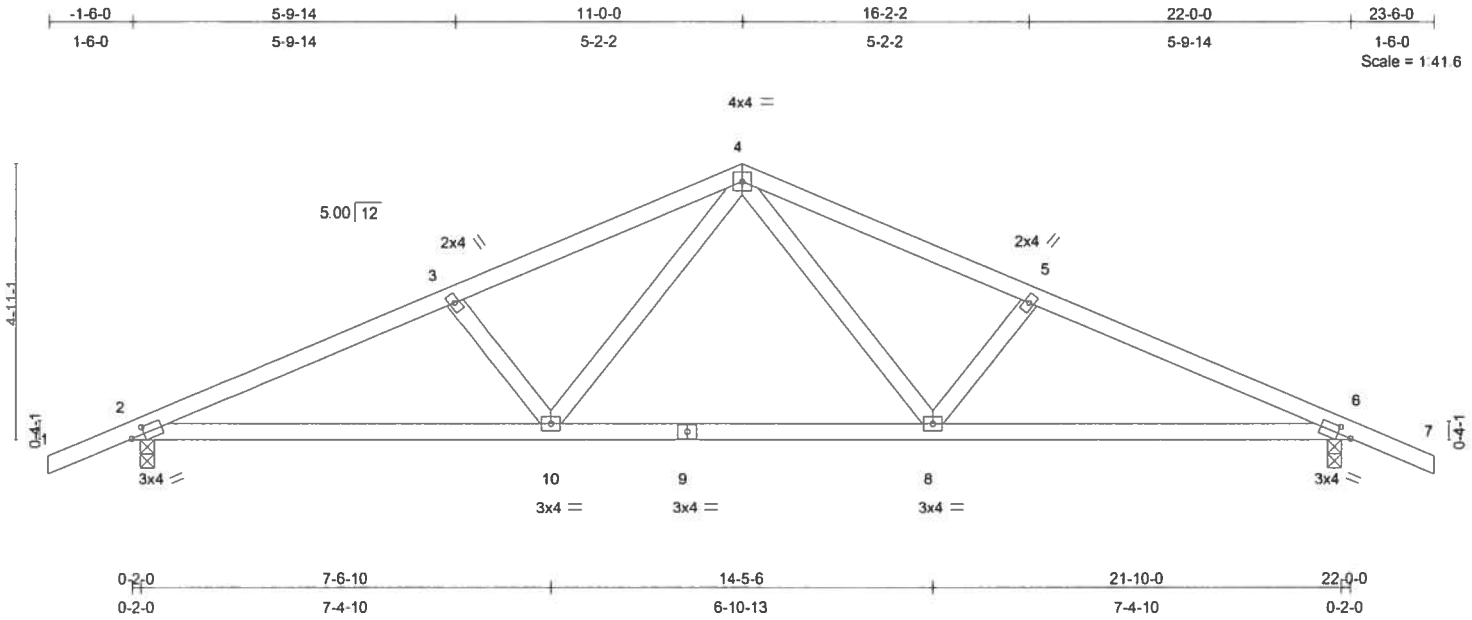


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [6:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.07	6-8	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.21	6-8	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 98 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

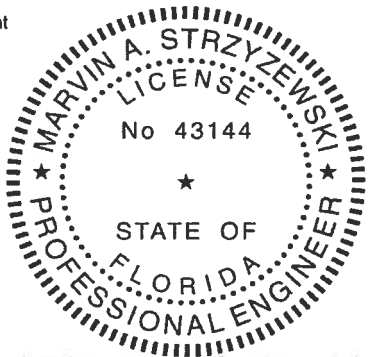
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=968/0-3-0, 6=968/0-3-0
Max Horz 2=76(LC 5)
Max Uplift2=-162(LC 5), 6=-162(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-1707/175, 3-4=-1500/161, 4-5=-1500/162, 5-6=-1707/175, 6-7=0/33
BOT CHORD 2-10=-156/1512, 9-10=-44/1029, 8-9=-44/1029, 6-8=-80/1512
WEBS 3-10=-324/139, 4-10=-46/515, 4-8=-46/515, 5-8=-324/139

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 2 and 162 lb uplift at joint 6.

LOAD CASE(S) Standard



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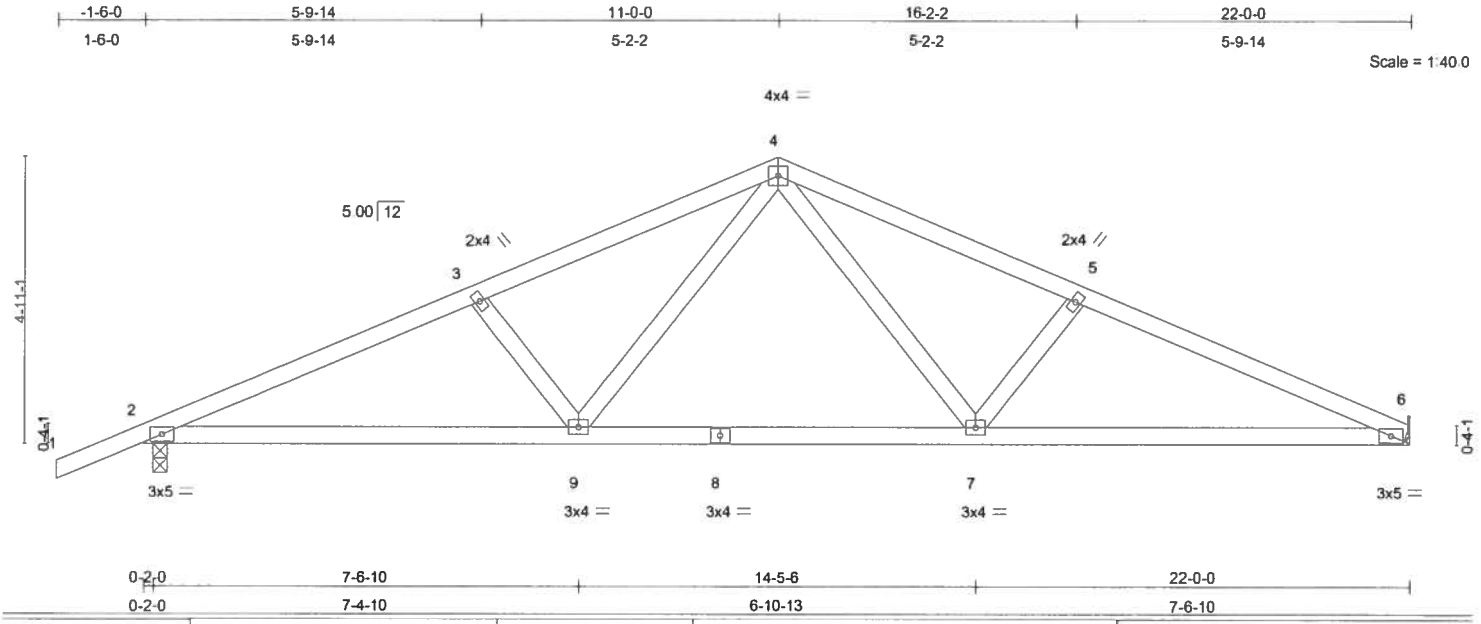
October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483126
TEEL	B4	COMMON	2	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL. 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:13 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.23	6-7	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

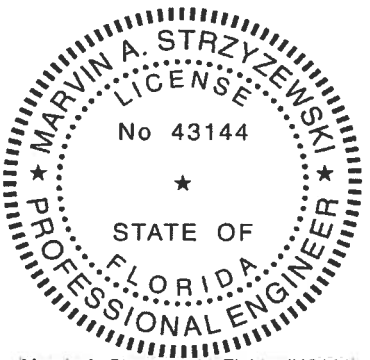
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=869/Mechanical, 2=974/0-3-0
Max Horz 2=84(LC 5)
Max Uplift6=-94(LC 6), 2=-163(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-1722/176, 3-4=-1515/162, 4-5=-1538/182, 5-6=-1737/198
BOT CHORD 2-9=-165/1525, 8-9=-53/1044, 7-8=-53/1044, 6-7=-134/1558
WEBS 3-9=-324/139, 4-9=-46/514, 4-7=-64/546, 5-7=-344/150

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 6 and 163 lb uplift at joint 2.

LOAD CASE(S) Standard



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October 26,2007

ENGINEERING BY TRENCO
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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483127
TEEL	B5	COMMON	1	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc Fri Oct 26 14:54:14 2007 Page 1

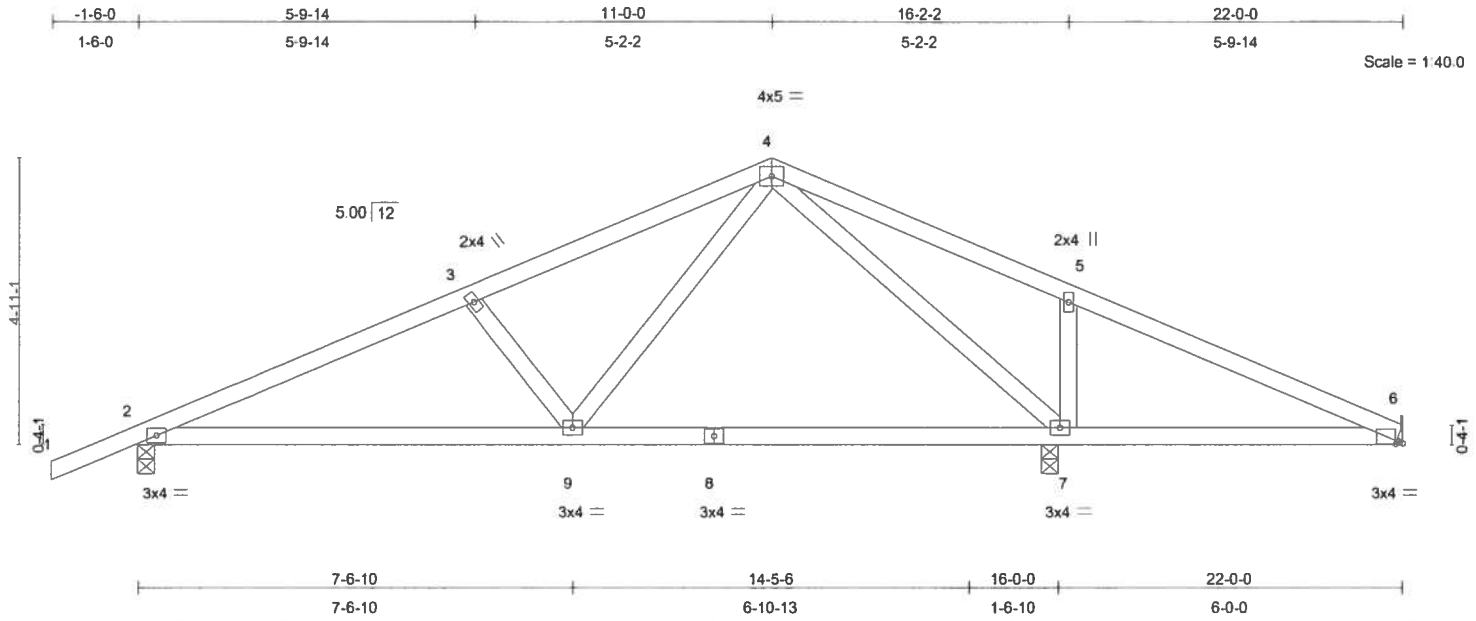

Plate Offsets (X,Y): [6:0-1-6,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.08	7-9	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.20	7-9	>984	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.02	7	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 6=121/Mechanical, 2=706/0-3-8, 7=1016/0-3-8
Max Horz 2=84(LC 5)
Max Uplift6=-23(LC 6), 2=-147(LC 5), 7=-96(LC 6)
Max Grav6=146(LC 8), 2=706(LC 1), 7=1016(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-1055/130, 3-4=-845/116, 4-5=0/193, 5-6=-35/215
BOT CHORD 2-9=-123/912, 8-9=-19/401, 7-8=-19/401, 6-7=-129/51
WEBS 3-9=-330/142, 4-9=-32/550, 4-7=-708/87, 5-7=-374/165

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 6, 147 lb uplift at joint 2 and 96 lb uplift at joint 7.

LOAD CASE(S) Standard

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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483128
TEEL	C1	COMMON	3	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:14 2007 Page 1

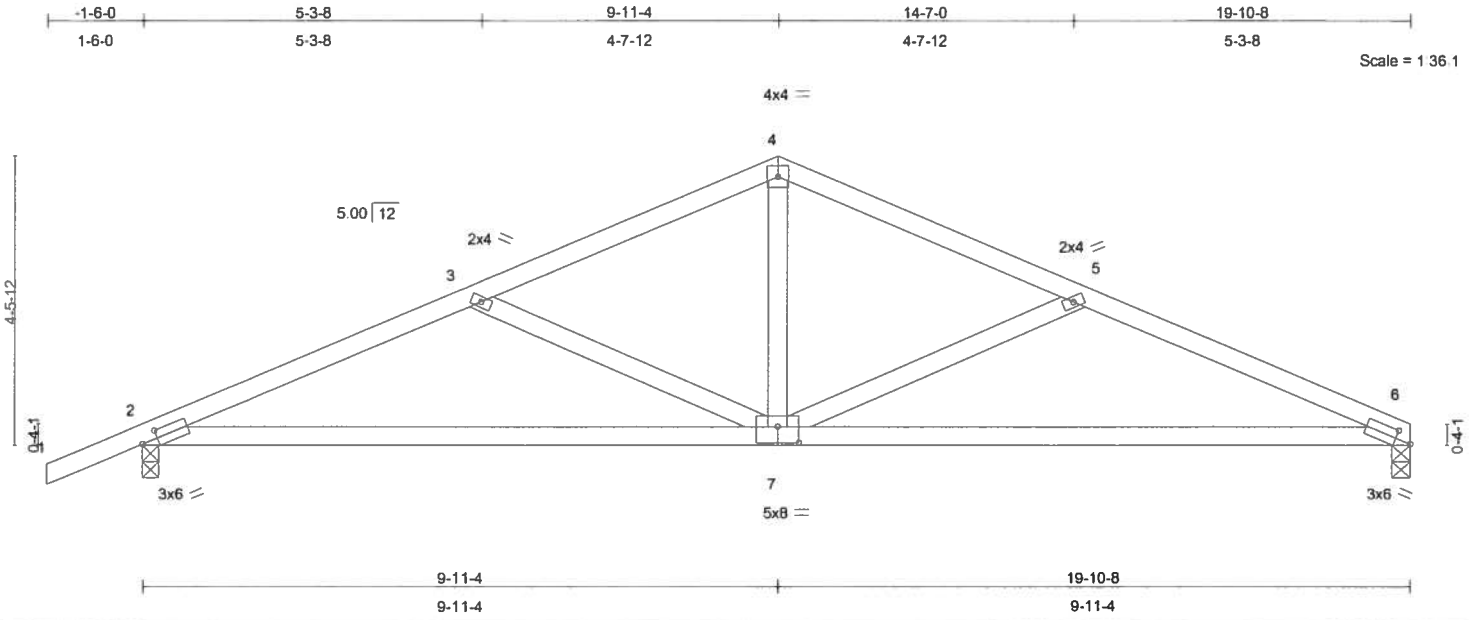


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [6:0-3-0,0-1-8], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.14	2-7	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.40	6-7	>592	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.04	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

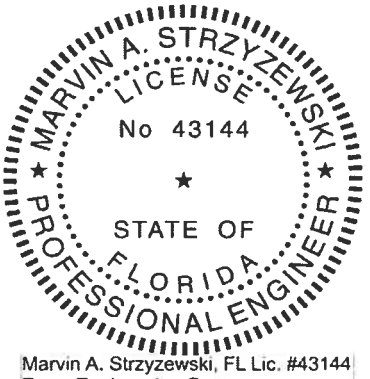
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=780/0-3-8, 2=886/0-3-0
Max Horz 2=79(LC 5)
Max Uplift6=-84(LC 6), 2=-154(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-1489/173, 3-4=-1124/109, 4-5=-1125/101, 5-6=-1501/193
BOT CHORD 2-7=-159/1331, 6-7=-131/1346
WEBS 3-7=-404/150, 4-7=0/582, 5-7=-420/165

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 6 and 154 lb uplift at joint 2.

LOAD CASE(S) Standard



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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	CET	GABLE	1	1	E4483129
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL. 6 500 s Apr 2 2007 MiTek Industries, Inc Fri Oct 26 14 54 15 2007 Page 1

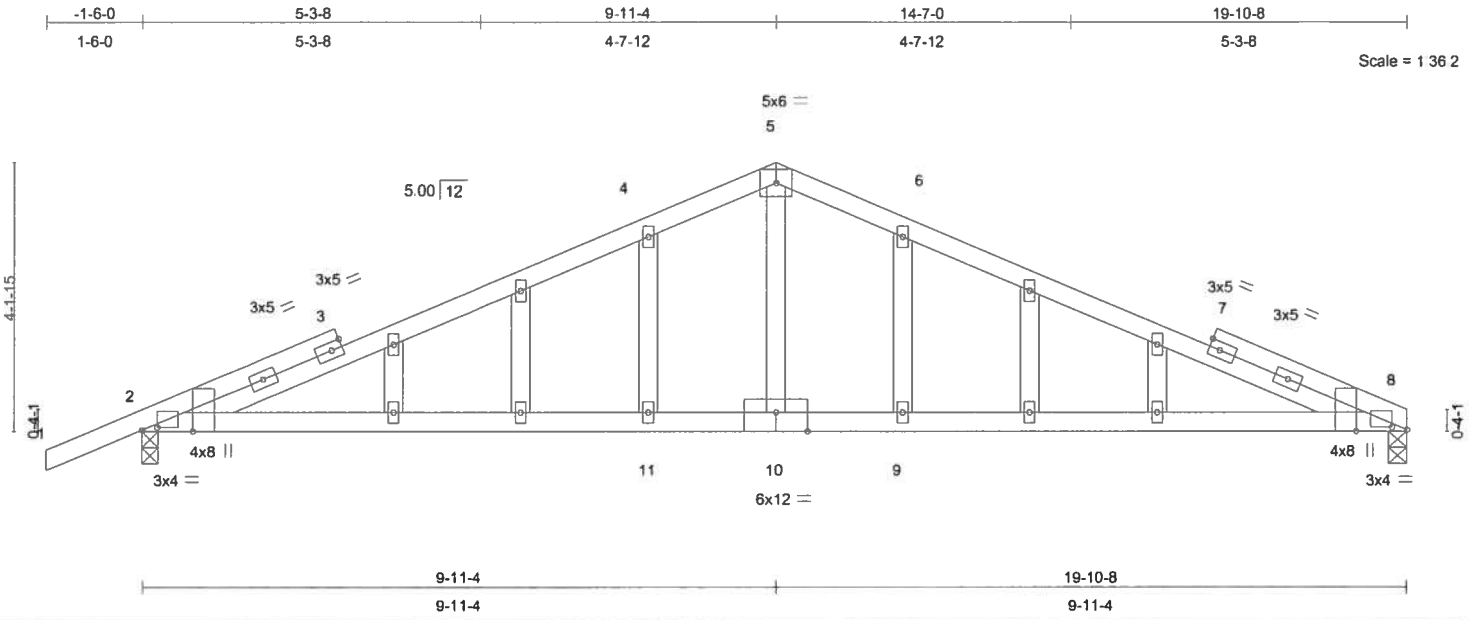


Plate Offsets (X,Y): [2:0-2-13,0-0-8], [2:0-0-4,Edge], [8:0-2-13,0-0-8], [8:0-0-4,Edge], [10:0-6-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.28	8-9	>833	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.62	8-9	>379	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.03	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

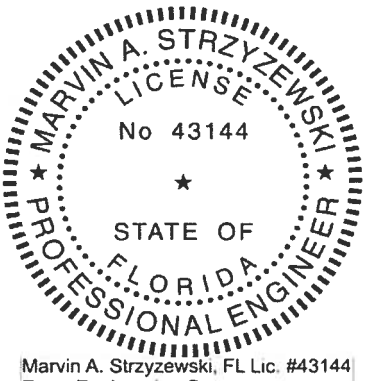
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=886/0-3-0, 8=780/0-3-8
Max Horz 2=75(LC 5)
Max Uplift2=-154(LC 5), 8=-85(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-1404/106, 3-4=-1343/132, 4-5=-1321/173, 5-6=-1321/166, 6-7=-1343/121, 7-8=-1402/95
BOT CHORD 2-11=-70/1240, 10-11=-70/1240, 9-10=-70/1240, 8-9=-70/1240
WEBS 5-10=-123/703, 4-11=-227/137, 6-9=-232/142

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 2 and 85 lb uplift at joint 8.

LOAD CASE(S) Standard



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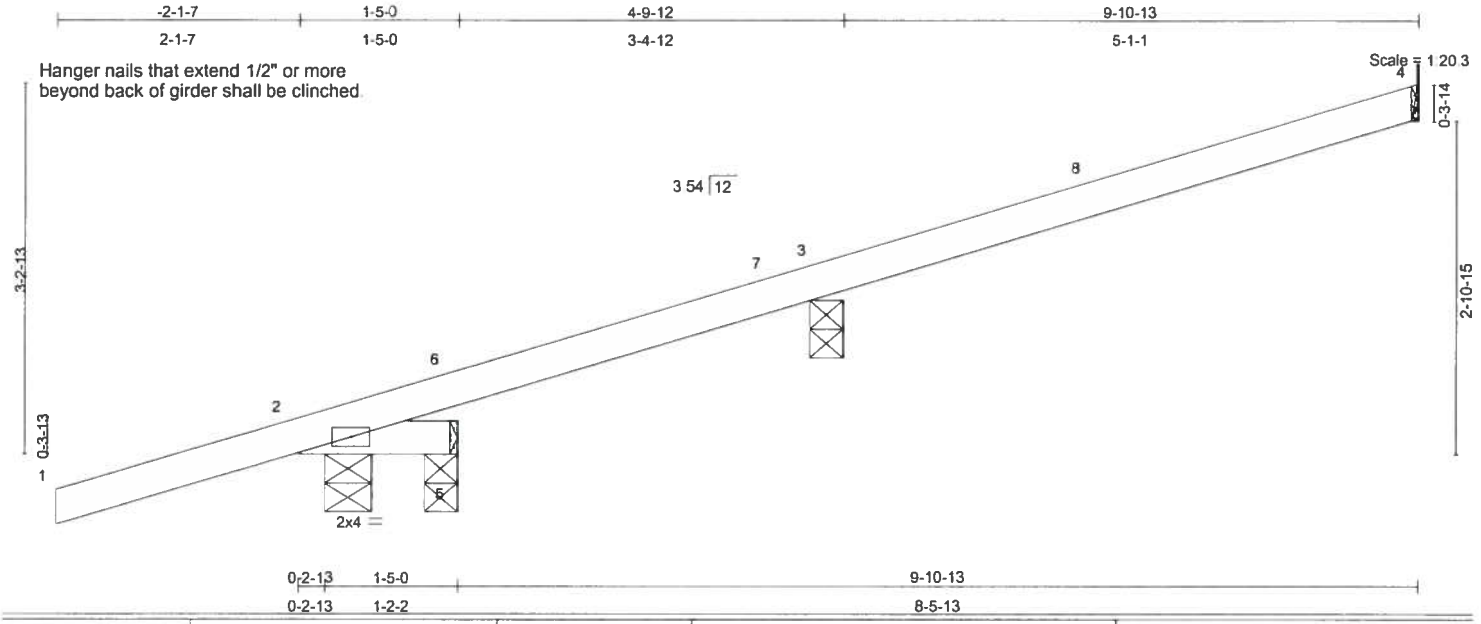
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483130
TEEL	CJ01	MONO TRUSS	4	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:16 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.00	2	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

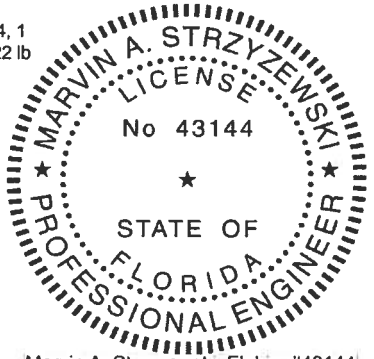
BRACING
TOP CHORD Structural wood sheathing directly applied or 1-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=174/Mechanical, 5=23/0-3-8, 2=265/0-5-0, 3=416/0-3-8
Max Horz 2=125(LC 3)
Max Uplift 4=-81(LC 3), 2=-87(LC 3), 3=-163(LC 4)
Max Grav 4=174(LC 1), 5=46(LC 2), 2=265(LC 1), 3=416(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-6=-88/0, 6-7=-72/38, 3-7=-64/44, 3-8=-75/1, 4-8=-31/40
BOT CHORD 2-5=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 4, 87 lb uplift at joint 2 and 163 lb uplift at joint 3.
6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 3.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1 lb down and 18 lb up at 4-3-4, 1 lb down and 11 lb up at 4-3-4, 61 lb down and 46 lb up at 7-1-3, 66 lb down and 44 lb up at 7-1-3, and 22 lb down at 1-5-4, and 22 lb down at 1-5-4 on top chord, and 19 lb down at 1-4-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 2-5=-20
Concentrated Loads (lb)
Vert: 5=-10(F) 7=5(F=-1, B=6) 8=-127(F=-61, B=-66)



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October 26,2007

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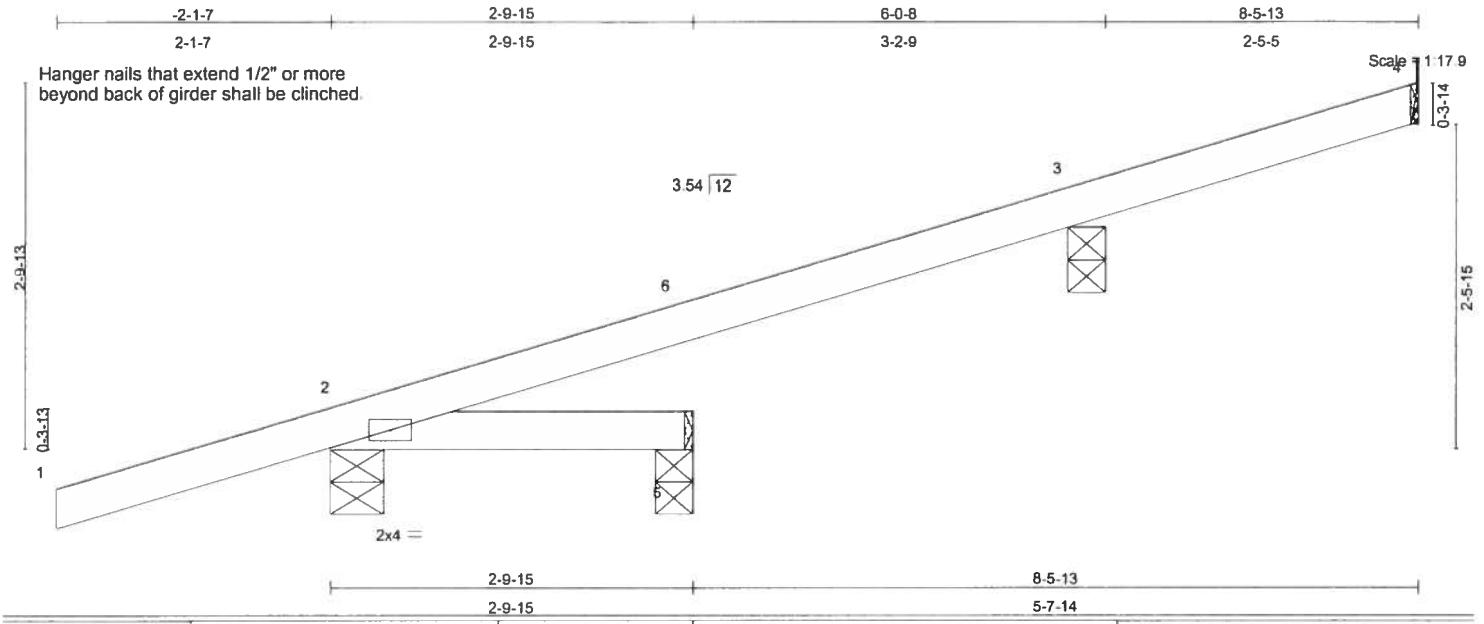
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483131
TEEL	CJ02	MONO TRUSS	4	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:17 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	2-5	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	2-5	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

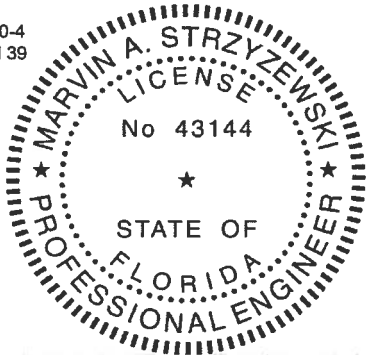
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=52/Mechanical, 2=306/0-5-0, 5=44/0-3-8, 3=479/0-3-8
Max Horz 2=111(LC 3)
Max Uplift 4=-24(LC 3), 2=-134(LC 3), 3=-177(LC 4)
Max Grav 4=52(LC 1), 2=306(LC 1), 5=89(LC 2), 3=479(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-6=-62/0, 3-6=-43/33, 3-4=-29/10
BOT CHORD 2-5=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 4, 134 lb uplift at joint 2 and 177 lb uplift at joint 3.
6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 3.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 10 lb down and 36 lb up at 2-10-4 , 12 lb down and 52 lb up at 2-10-4, and 1 lb down and 18 lb up at 5-8-3, and 264 lb down and 86 lb up at 5-8-3 on top chord, and 39 lb down at 2-8-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 2-5=-20
Concentrated Loads (lb)
Vert: 5=-20(F) 3=-265(F=-1, B=-264) 6=88(F=36, B=52)



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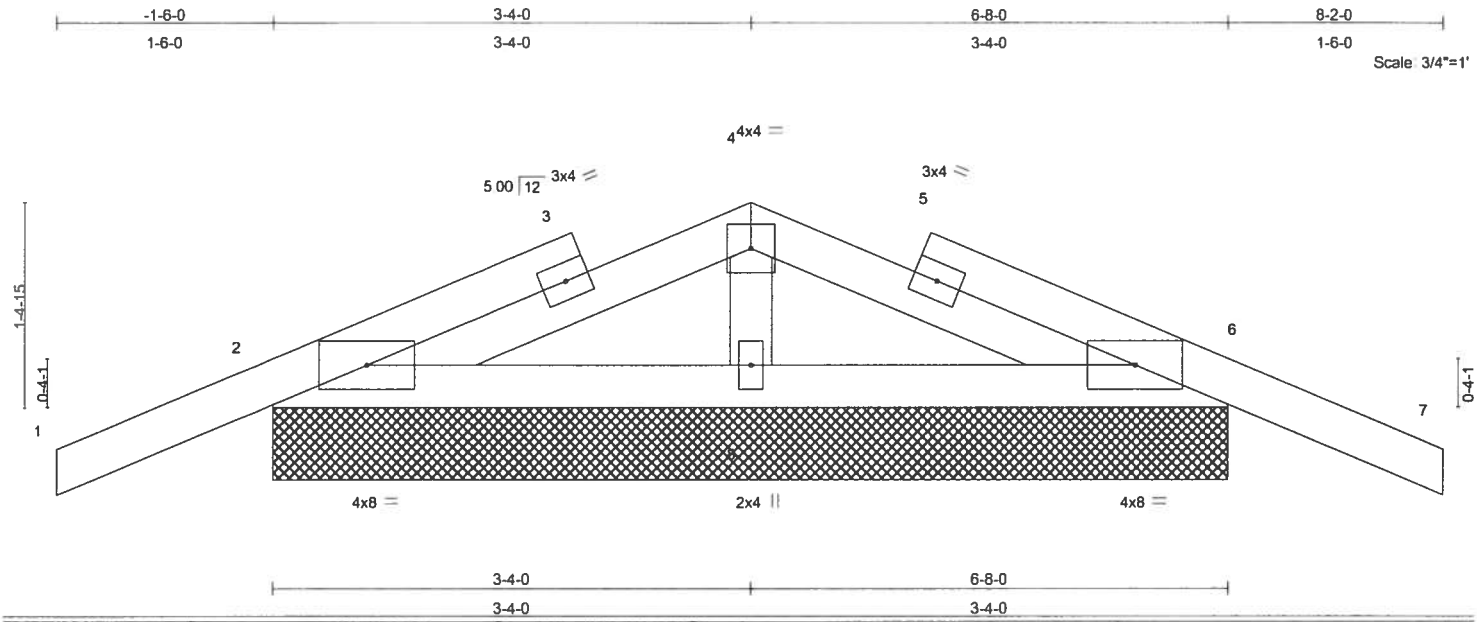
October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	DET	GABLE	1	1	E4483132
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:18 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	7	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	0.00	6	n/r	120		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

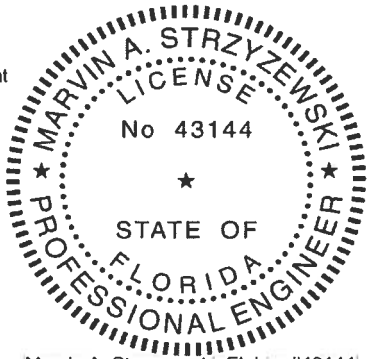
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=315/6-8-0, 6=315/6-8-0, 8=83/6-8-0
Max Horz 2=35(LC 5)
Max Uplift2=-118(LC 5), 6=-118(LC 6)
Max Grav 2=315(LC 1), 6=315(LC 1), 8=167(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-287/60, 3-4=-247/63, 4-5=-247/63, 5-6=-287/60, 6-7=0/33
BOT CHORD 2-8=-38/240, 6-8=-38/240

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 2 and 118 lb uplift at joint 6.
9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.

LOAD CASE(S) Standard



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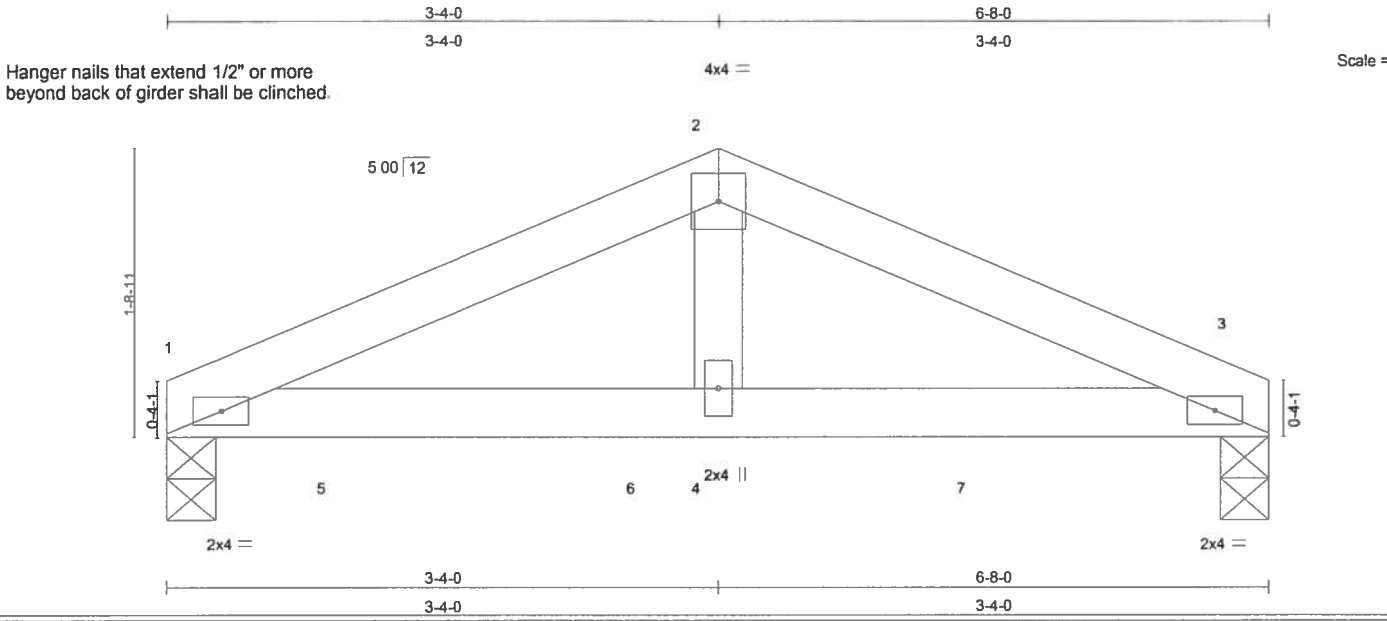
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483133
TEEL	DGT	COMMON	1	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:18 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.02	1-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.04	1-4	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.17	Horz(TL)	0.01	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

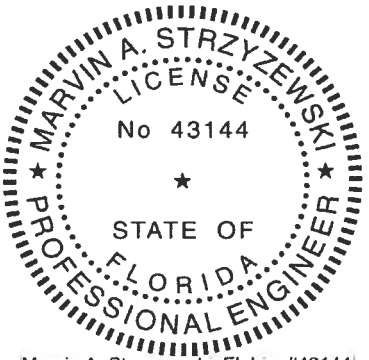
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=656/0-3-8, 3=546/0-3-8
Max Horz 1=17(LC 5)
Max Uplift1=-74(LC 5), 3=-56(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-930/87, 2-3=-930/87
BOT CHORD 1-5=-60/821, 5-6=-60/821, 4-6=-60/821, 4-7=-60/821, 3-7=-60/821
WEBS 2-4=-11/543

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 1 and 56 lb uplift at joint 3.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 249 lb down and 30 lb up at 1-0-12, and 249 lb down and 30 lb up at 2-11-4, and 194 lb down and 15 lb up at 4-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-60, 2-3=-60, 1-3=-20
Concentrated Loads (lb)
Vert: 5=-249(B) 6=-249(B) 7=-194(B)



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October 26,2007

ENGINEERING BY TRENCO
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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483134
TEEL	EJ6	JACK	16	1	Job Reference (optional)	

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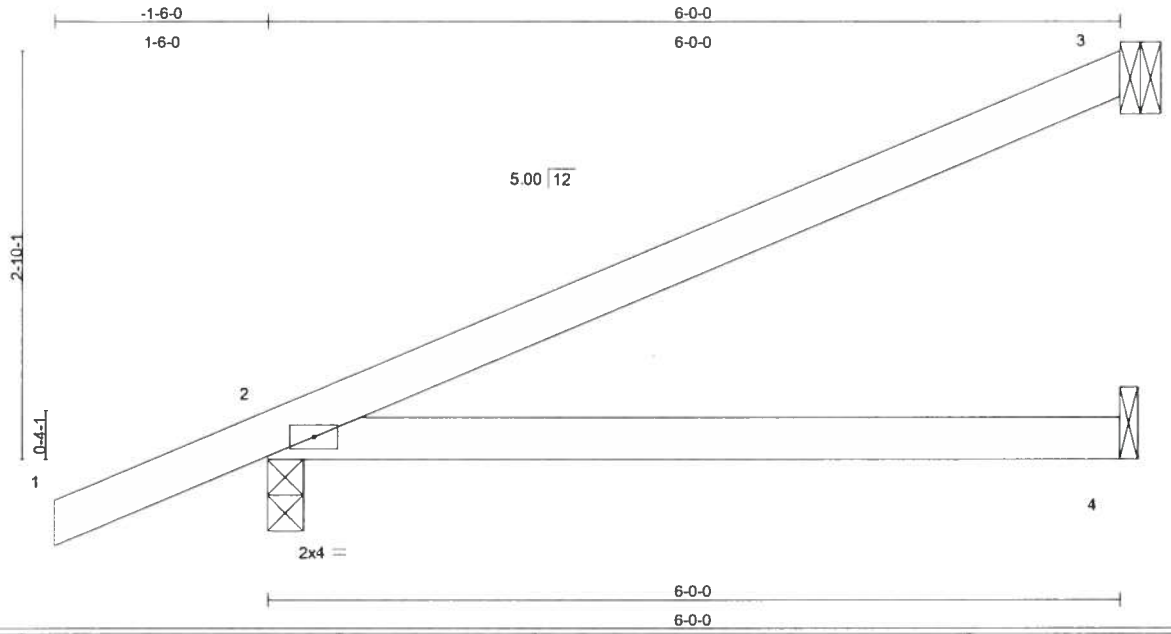


Scale = 1:16.2

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.06	2-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.14	2-4	>495	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

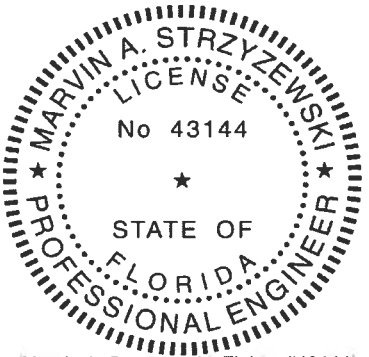
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=158/Mechanical, 2=341/0-3-0, 4=58/Mechanical
Max Horz 2=109(LC 5)
Max Uplift 3=-70(LC 5), 2=-94(LC 5)
Max Grav 3=158(LC 1), 2=341(LC 1), 4=116(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-74/49
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 3 and 94 lb uplift at joint 2.

LOAD CASE(S) Standard



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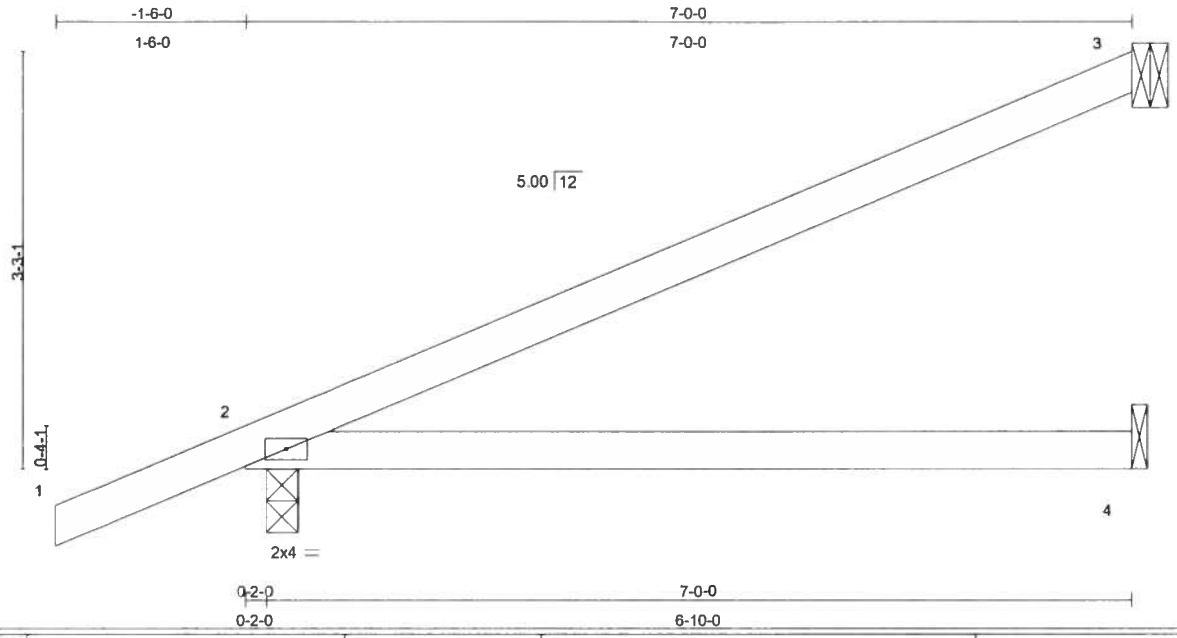
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483135
TEEL	EJ7	JACK	23	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Oct 26 16:25:04 2007 Page 1



Scale = 1:18.2

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.11	2-4	>768	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.27	2-4	>307	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

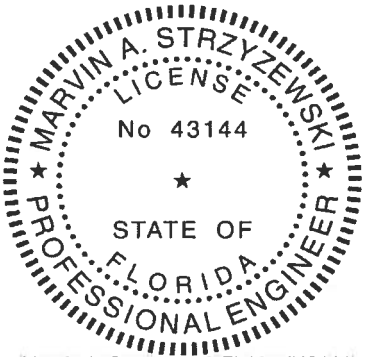
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=190/Mechanical, 4=68/Mechanical, 2=379/0-3-0
Max Horz 2=123(LC 5)
Max Uplift 3=-85(LC 5), 2=-96(LC 5)
Max Grav 3=190(LC 1), 4=136(LC 2), 2=379(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-84/59
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 3 and 96 lb uplift at joint 2.

LOAD CASE(S) Standard



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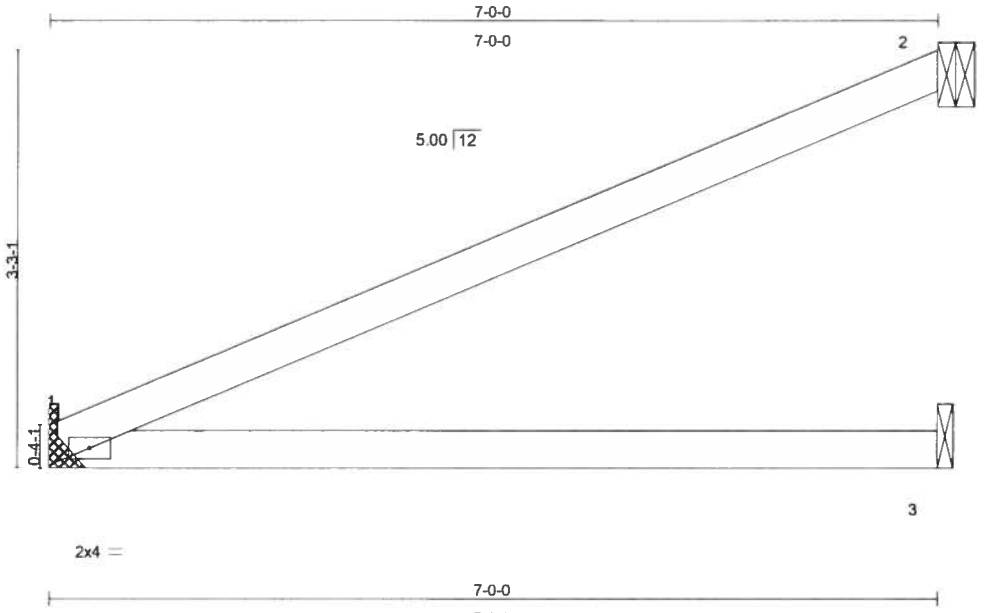
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	EJ7A	JACK	3	1	E4483136
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Oct 26 16:25:46 2007 Page 1



Scale = 1:18.1

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.11	1-3	>776	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.26	1-3	>310	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

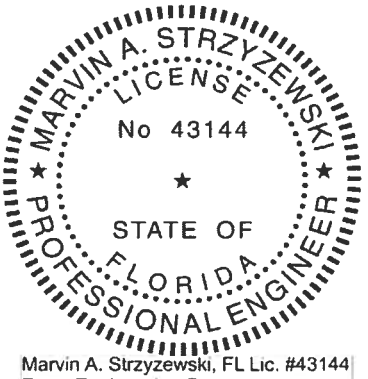
BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=269/Mechanical, 2=201/Mechanical, 3=68/Mechanical
Max Horz 1=93(LC 5)
Max Uplift 1=-20(LC 5), 2=-94(LC 5)
Max Grav 1=269(LC 1), 2=201(LC 1), 3=136(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-64/64
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1 and 94 lb uplift at joint 2.

LOAD CASE(S) Standard



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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	EJGT	MONO TRUSS	1	1	E4483137
					Job Reference (optional)

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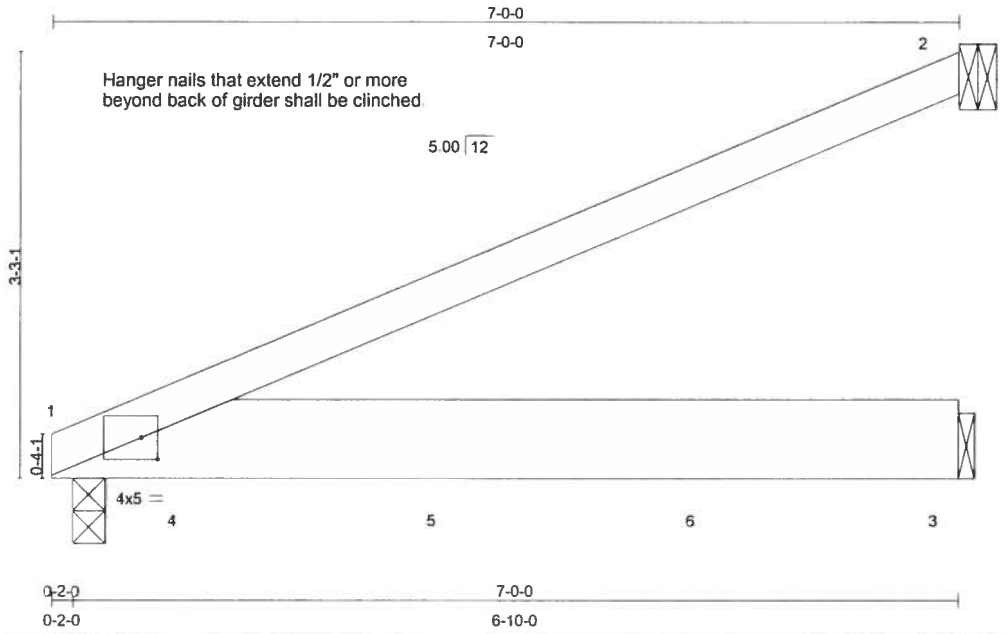


Scale = 1:17.8

Plate Offsets (X,Y): [1:0-1-8,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.08	1-3	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.20	1-3	>404	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP 2400F 2.0E

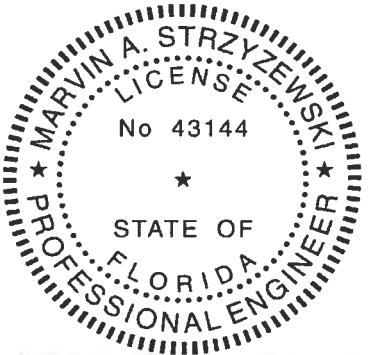
BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-6 oc bracing.

REACTIONS (lb/size) 2=202/Mechanical, 3=643/Mechanical, 1=1520/0-3-0
Max Horz 1=88(LC 5)
Max Uplift 2=-92(LC 5), 3=-49(LC 3), 1=-180(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-61/61
BOT CHORD 1-4=0/0, 4-5=0/0, 5-6=0/0, 3-6=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II, Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 2, 49 lb uplift at joint 3 and 180 lb uplift at joint 1.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 849 lb down and 104 lb up at 1-0-12, and 849 lb down and 104 lb up at 3-0-12, and 126 lb down and 33 lb up at 5-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-60, 1-3=-20
Concentrated Loads (lb)
Vert: 4=-849(F) 5=-849(F) 6=-126(F)



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Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	G1	HIP	2	1	E4483138
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc Fri Oct 26 14:54:22 2007 Page 1

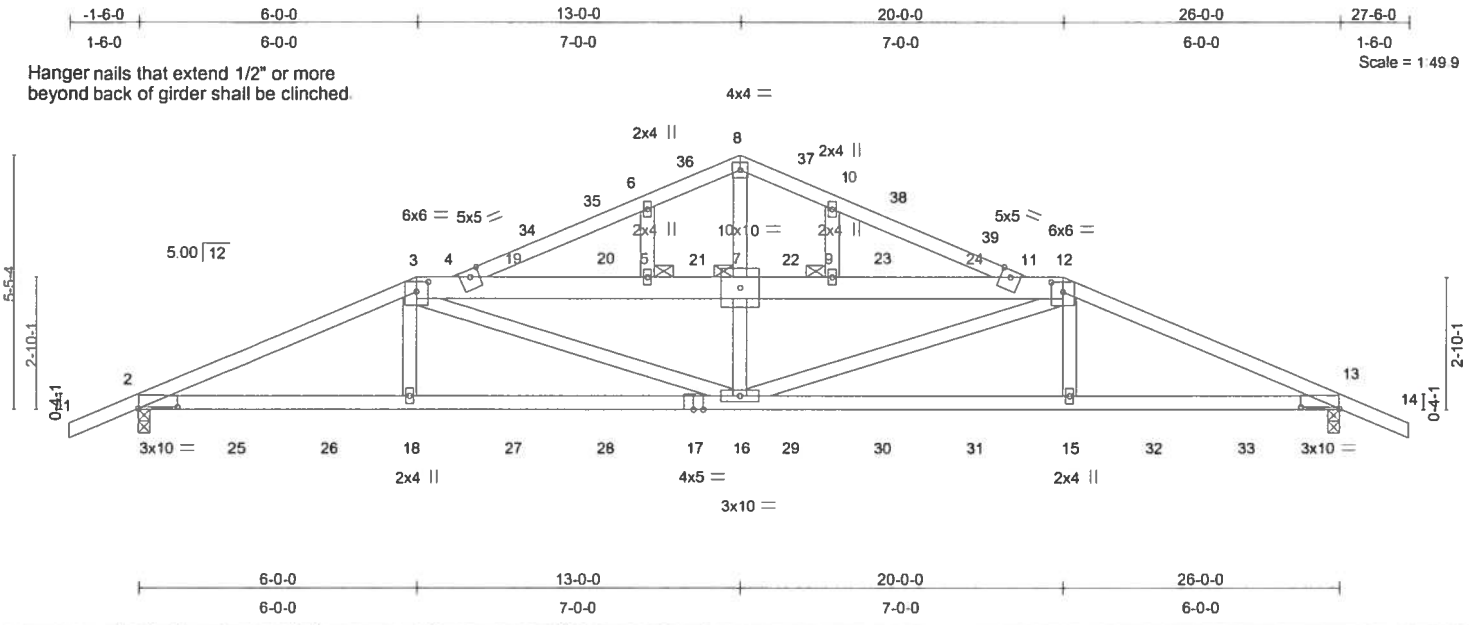


Plate Offsets (X,Y): [2:0-10-2,0-0-6], [3:0-3-0,0-2-9], [12:0-3-0,0-2-9], [13:0-10-2,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.15	16	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.45	15-16	>682	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.15	13	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D *Except* 3-12 2 X 6 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

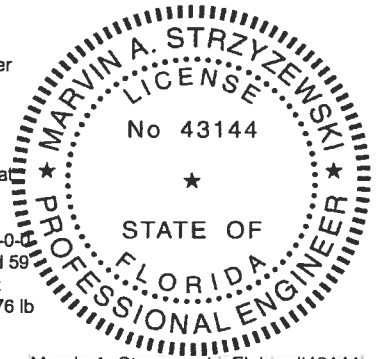
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-1 oc purlins. Except: 6-0-0 oc bracing: 4-5, 5-7, 7-9, 9-11
BOT CHORD Rigid ceiling directly applied or 7-11-8 oc bracing.
JOINTS 1 Brace at Jt(s): 7, 9, 5

REACTIONS (lb/size) 2=1807/0-3-0, 13=1811/0-3-0
Max Horz 2=-83(LC 6)
Max Uplift2=-383(LC 5), 13=-384(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-3800/716, 3-4=-3647/777, 4-19=-2325/515, 19-20=-2325/515, 5-20=-2325/515, 5-21=-2325/515, 7-21=-2325/515, 7-22=-2325/515, 9-22=-2325/515, 9-23=-2325/515, 23-24=-2325/515, 11-24=-2325/515, 11-12=-3647/777, 12-13=-3810/721, 13-14=0/33, 4-34=-1520/313, 34-35=-1482/316, 6-35=-1452/323, 6-36=-1470/335, 8-36=-1433/339, 8-37=-1433/338, 10-37=-1469/334, 10-38=-1455/322, 38-39=-1482/315, 11-39=-1520/312
BOT CHORD 2-25=-663/3460, 25-26=-663/3460, 18-26=-663/3460, 18-27=-666/3442, 27-28=-666/3442, 17-28=-666/3442, 16-17=-666/3442, 16-29=-615/3451, 29-30=-615/3451, 30-31=-615/3451, 15-31=-615/3451, 15-32=-611/3469, 32-33=-611/3469, 13-33=-611/3469
WEBS 3-18=0/542, 3-16=-123/300, 7-16=0/487, 12-16=-120/357, 12-15=0/546, 7-8=-187/898, 9-10=-113/61, 5-6=-115/61

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 383 lb uplift at joint 2 and 384 lb uplift at joint 13.
7) Design requires purlins at oc spacing indicated.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 170 lb down and 73 lb up at 6-0-0, 138 lb down and 59 lb up at 8-0-12, 138 lb down and 59 lb up at 10-0-12, 138 lb down and 59 lb up at 12-0-12, 138 lb down and 59 lb up at 14-0-12, 138 lb down and 59 lb up at 16-0-12, and 138 lb down and 59 lb up at 18-0-12, and 170 lb down and 73 lb up at 20-0-0 on top chord, and 76 lb down at 2-0-12, 107 lb down and 10 lb up at 4-0-12, 76 lb down at 6-0-12, 76 lb down at 8-0-12, 76 lb down at 10-0-12, 76 lb down at 12-0-12, 76 lb down at 14-0-12, 76 lb down at 16-0-12, 76 lb down at 18-0-12, 76 lb down at 19-11-4, and 107 lb down and 10 lb up at 21-11-4, and 76 lb down at 23-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2
LOAD CASE(S) Standard



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October 26,2007

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 11-12=-60, 12-14=-60, 2-13=-20, 4-8=-60, 8-11=-60

Concentrated Loads (lb)

Vert: 3=-90(B) 12=-90(B) 17=-38(B) 18=-38(B) 15=-38(B) 19=-98(B) 20=-98(B) 21=-98(B) 22=-98(B) 23=-98(B) 24=-98(B) 25=-38(B) 26=-107(B) 27=-38(B) 28=-38(B) 29=-38(B) 30=-38(B) 31=-38(B) 32=-107(B) 33=-38(B)

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483139
TEEL	G2	COMMON	8	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL — 6 500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14 54 23 2007 Page 1

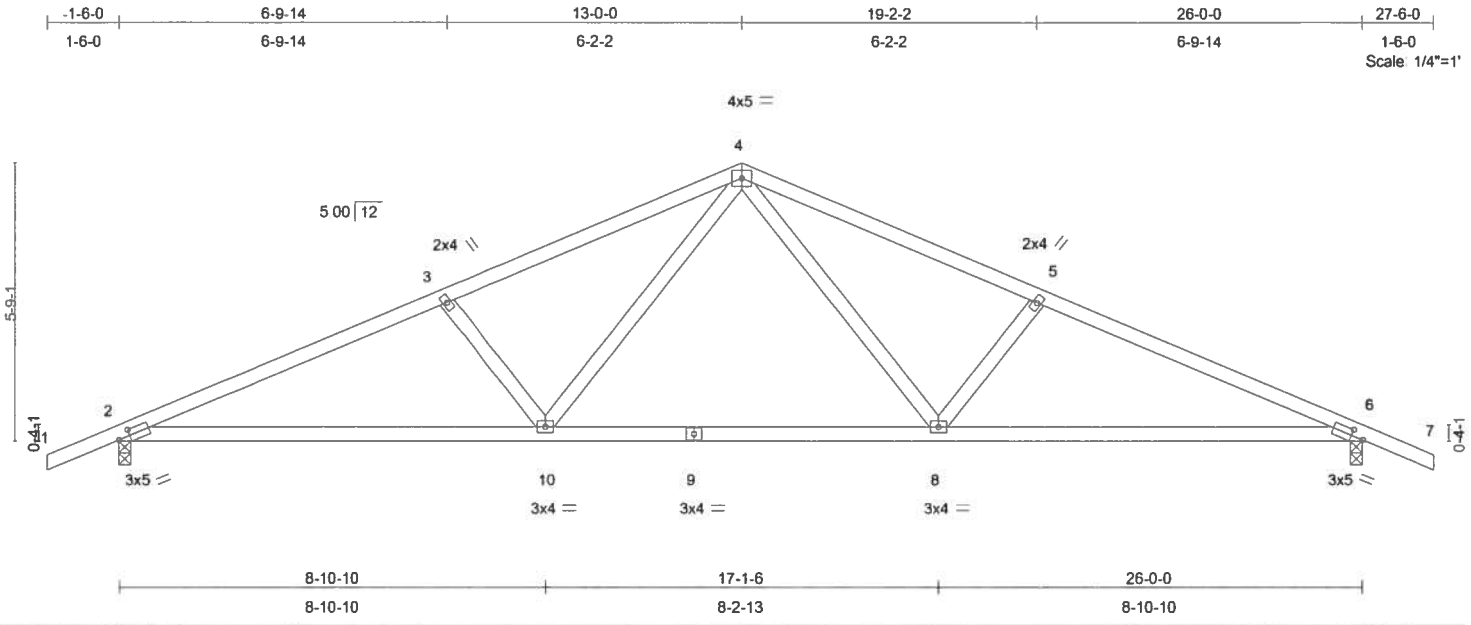


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [6:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.13	2-10	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.38	2-10	>812	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.07	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

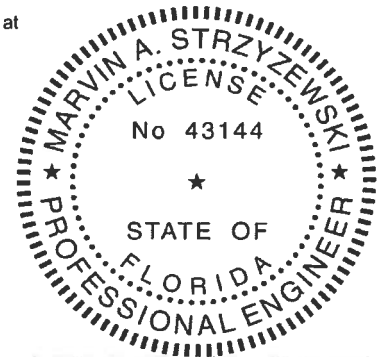
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1128/0-3-0, 6=1128/0-3-0
Max Horz 2=86(LC 5)
Max Uplift2=-179(LC 5), 6=-179(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-2049/216, 3-4=-1801/198, 4-5=-1801/199, 5-6=-2049/216, 6-7=0/33
BOT CHORD 2-10=-196/1820, 9-10=-57/1228, 8-9=-57/1228, 6-8=-110/1820
WEBS 3-10=-394/169, 4-10=-61/625, 4-8=-61/625, 5-8=-394/169

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 2 and 179 lb uplift at joint 6.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
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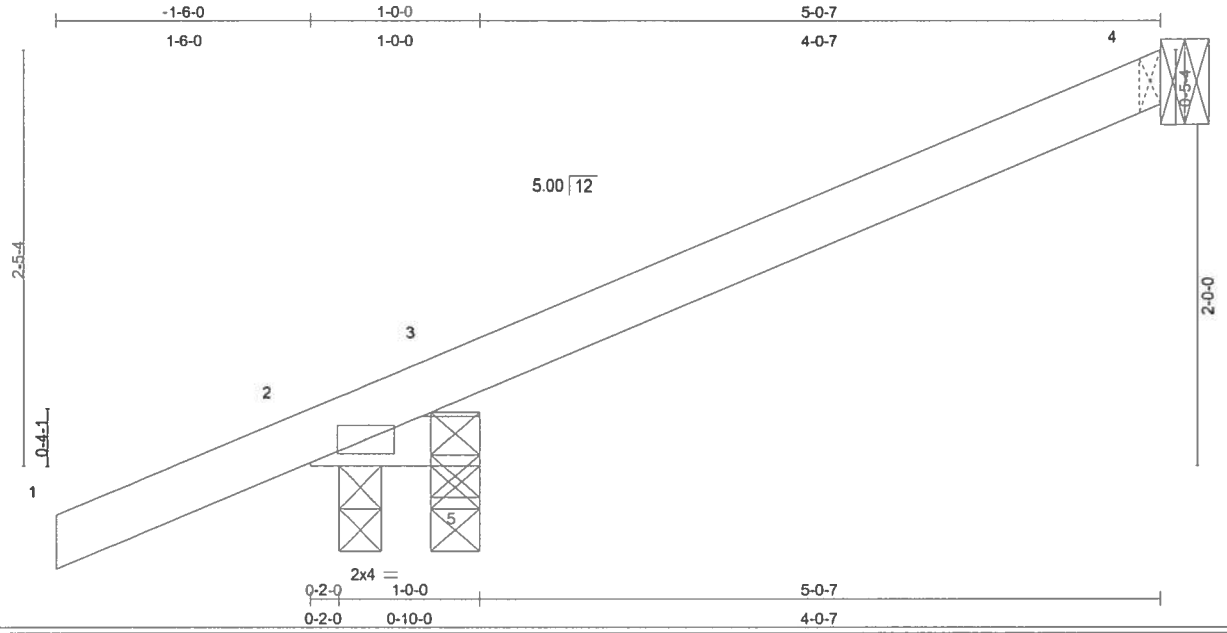
FL COA #7239

October 26,2007

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	J01	JACK	4	1	E4483140
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Oct 26 16:27:56 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

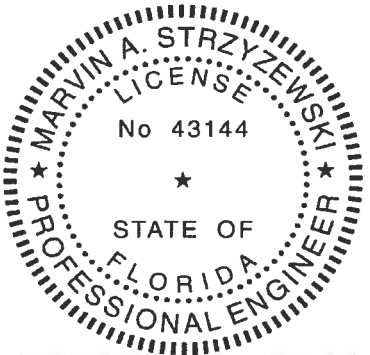
BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=121/Mechanical, 5=9/0-3-8, 2=210/0-3-0, 3=62/0-3-8
Max Horz 2=96(LC 5)
Max Uplift 4=-56(LC 5), 2=-106(LC 5), 3=-33(LC 4)
Max Grav 4=121(LC 1), 5=19(LC 2), 2=210(LC 1), 3=62(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-46/0, 3-4=-37/37
BOT CHORD 2-5=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 4, 106 lb uplift at joint 2 and 33 lb uplift at joint 3.
6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 3.

LOAD CASE(S) Standard



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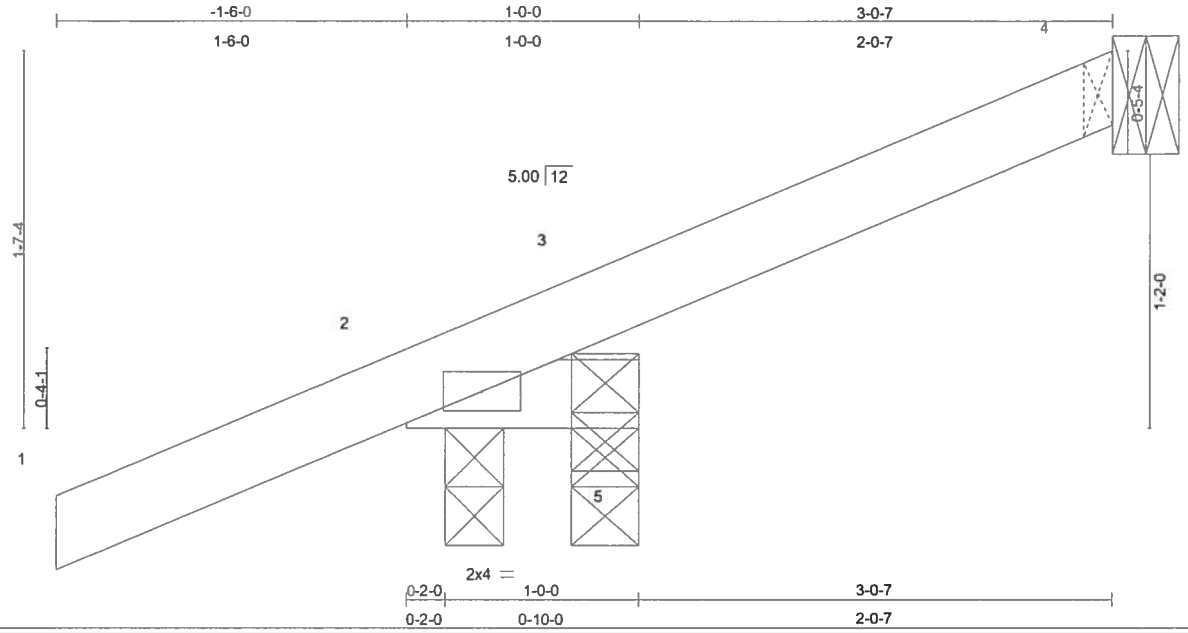
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483141
TEEL	J01A	JACK	4	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL. 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Oct 26 16:28:17 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 9 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

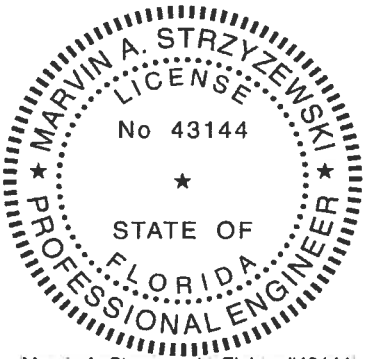
BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=61/Mechanical, 5=9/0-3-8, 2=210/0-3-0, 3=2/0-3-8
Max Horz 2=70(LC 5)
Max Uplift 4=-28(LC 5), 2=-118(LC 5), 3=-14(LC 4)
Max Grav 4=61(LC 1), 5=19(LC 2), 2=210(LC 1), 3=27(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-46/0, 3-4=-19/19
BOT CHORD 2-5=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 4, 118 lb uplift at joint 2 and 14 lb uplift at joint 3.
6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 3.

LOAD CASE(S) Standard



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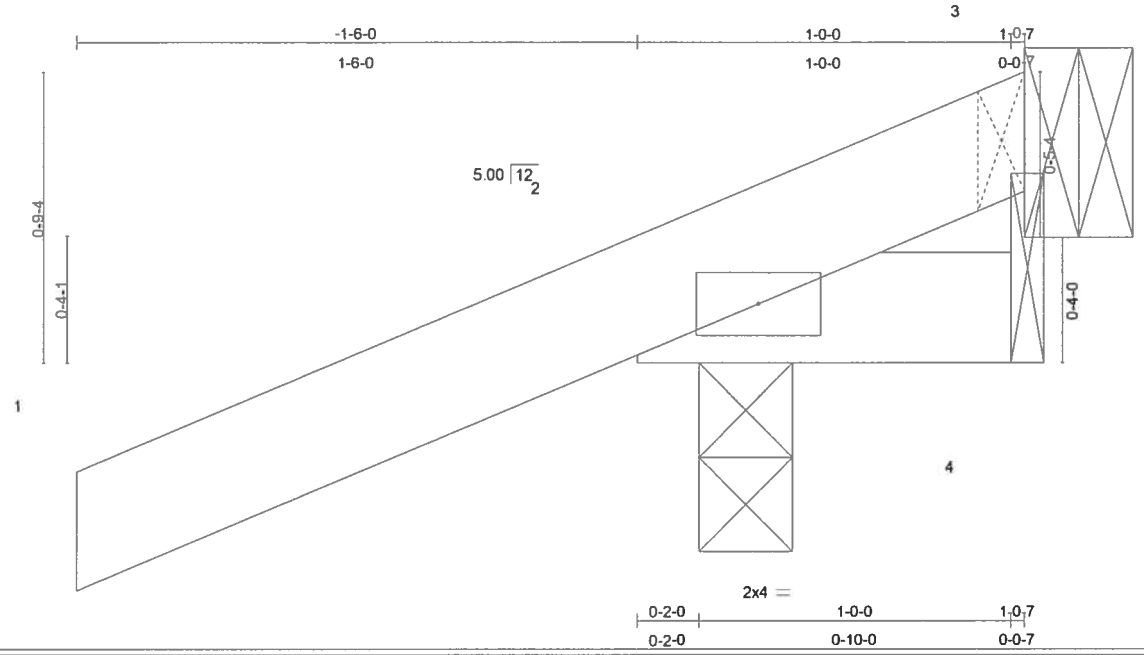
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483142
TEEL	J01B	JACK	4	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Oct 26 16:28:41 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

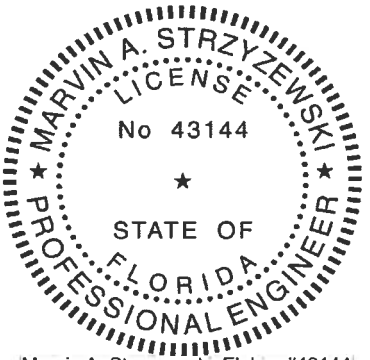
BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=10/Mechanical, 2=202/0-3-0, 3=-42/Mechanical
Max Horz 2=44(LC 5)
Max Uplift 2=-121(LC 5), 3=-42(LC 1)
Max Grav 4=19(LC 2), 2=202(LC 1), 3=43(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-42/17
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 2 and 42 lb uplift at joint 3.

LOAD CASE(S) Standard



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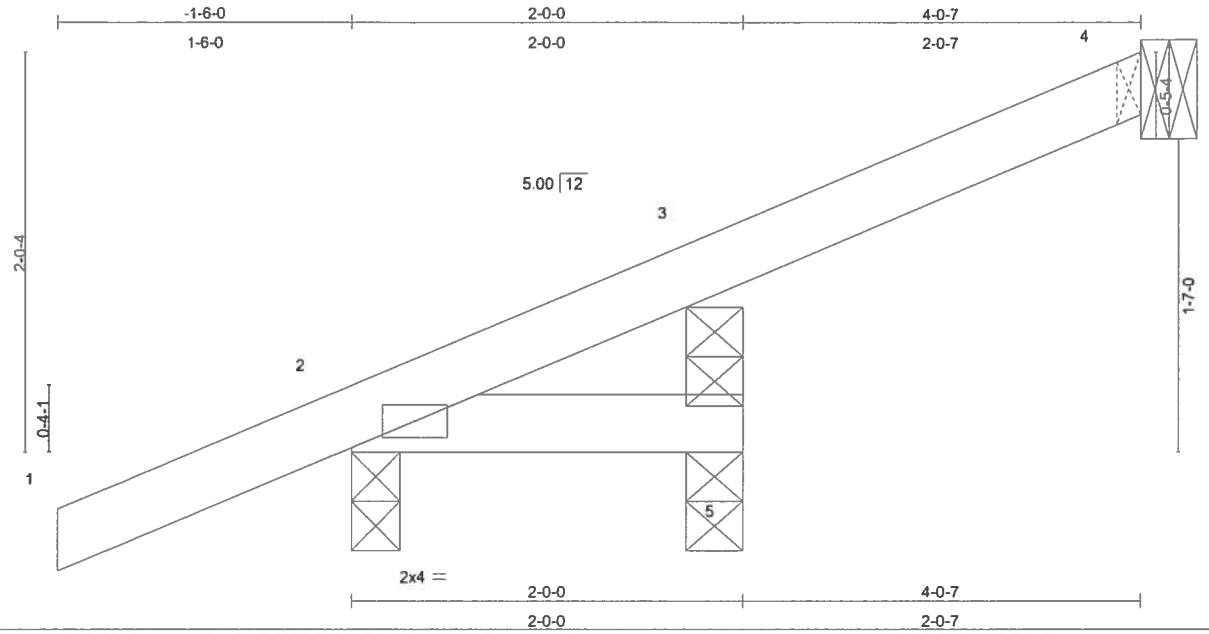
October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	J02	JACK	4	1	E4483143
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Oct 26 16:29:03 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-5	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

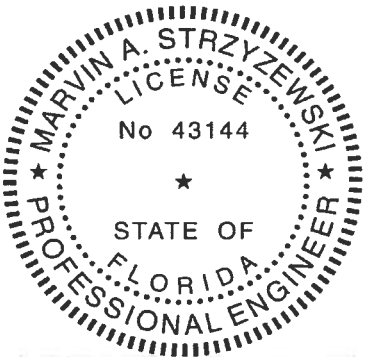
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=61/Mechanical, 2=204/0-3-0, 5=19/0-3-8, 3=78/0-3-8
Max Horz 2=83(LC 5)
Max Uplift 4=-28(LC 5), 2=-85(LC 5), 3=-29(LC 4)
Max Grav 4=61(LC 1), 2=204(LC 1), 5=39(LC 2), 3=78(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-37/2, 3-4=-19/19
BOT CHORD 2-5=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 4, 85 lb uplift at joint 2 and 29 lb uplift at joint 3.
6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 3.

LOAD CASE(S) Standard



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October 26,2007

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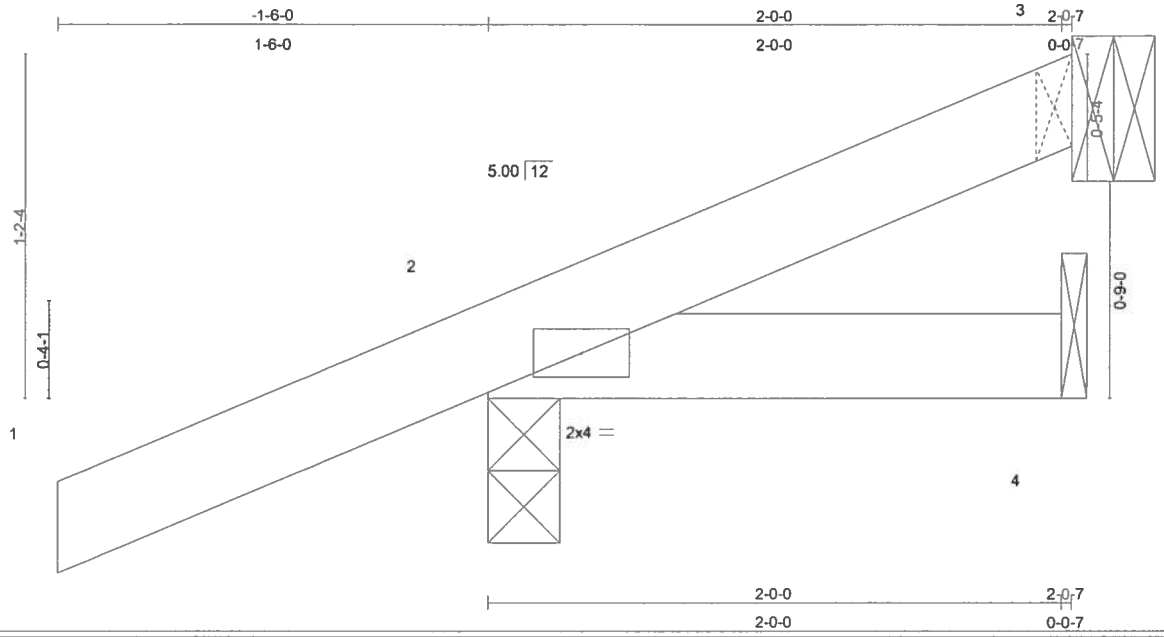
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	J02A	JACK	4	1	E4483144
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Oct 26 16:29:27 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 9 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=206/0-3-0, 4=20/Mechanical, 3=24/Mechanical
Max Horz 2=58(LC 5)
Max Uplift 2=-96(LC 5), 3=-13(LC 4)
Max Grav 2=206(LC 1), 4=39(LC 2), 3=24(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-38/5
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 2 and 13 lb uplift at joint 3.

LOAD CASE(S) Standard

MARVIN A. STRZYZEWSKI LICENSE No 43144 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Edenton, NC 27932
FL COA #7239

October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483145
TEEL	J06	JACK	4	1	Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL 6.500 s Apr 2 2007 MiTek Industries, Inc. Fri Oct 26 14:54:26 2007 Page 1

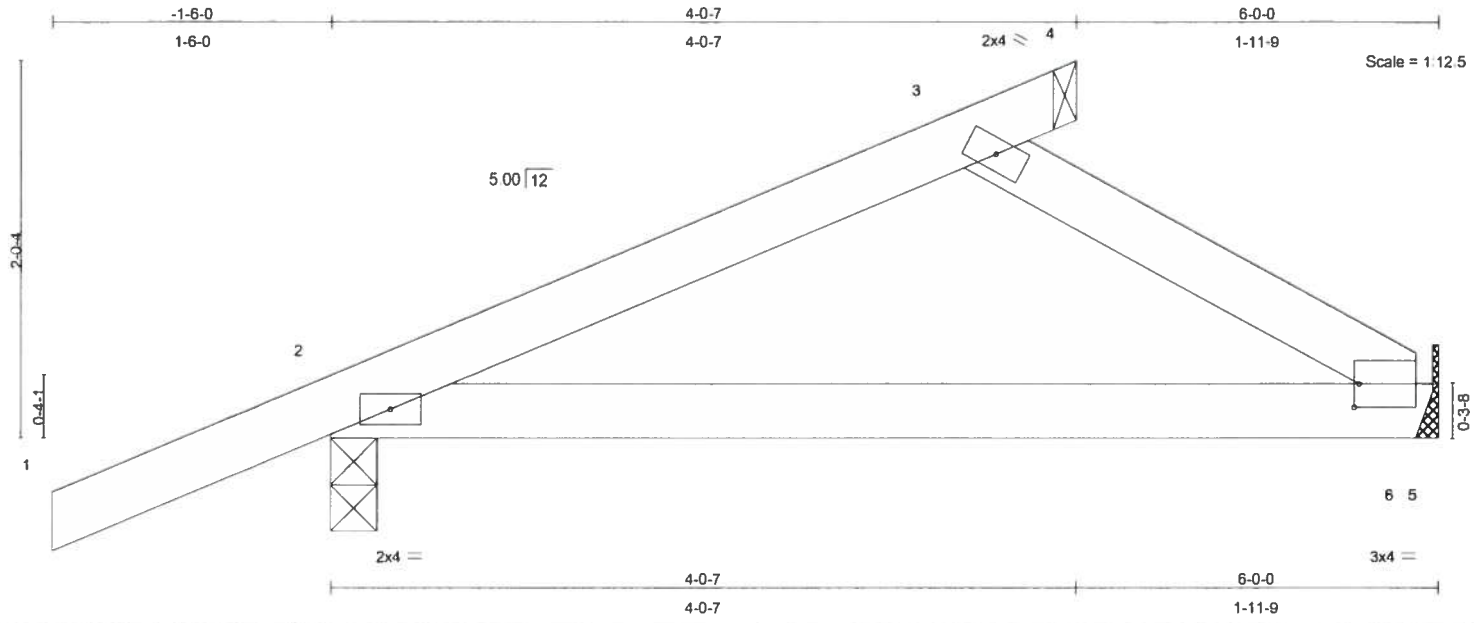


Plate Offsets (X,Y): [6:0-0-5,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.05	2-6	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.13	2-6	>512	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

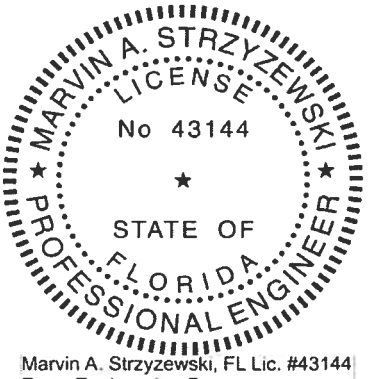
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=324/0-3-0, 6=127/Mechanical
Max Horz 2=83(LC 5)
Max Uplift 2=-96(LC 5)
Max Grav 2=324(LC 1), 6=144(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-169/32, 3-4=-14/0
BOT CHORD 2-6=-51/110, 5-6=0/0
WEBS 3-6=-128/60

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 2.

LOAD CASE(S) Standard



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October 26,2007

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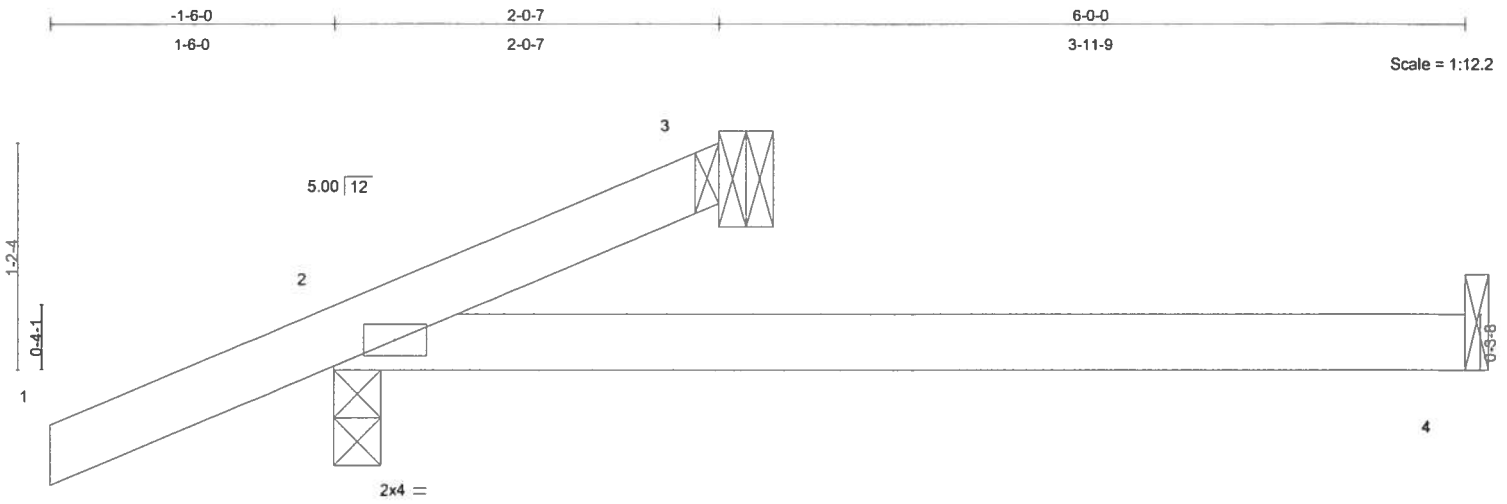
Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	J06A	JACK	4	1	E4483146
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL — 6.500 s Mar 8 2007 MiTek Industries, Inc Fri Oct 26 16:29:55 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.06	2-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.14	2-4	>495	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

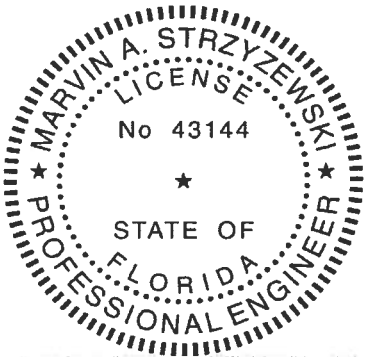
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=8/Mechanical, 2=253/0-3-0, 4=58/Mechanical
Max Horz 2=56(LC 5)
Max Uplift 3=-10(LC 4), 2=-86(LC 5)
Max Grav 3=12(LC 3), 2=253(LC 1), 4=116(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-41/4
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 3 and 86 lb uplift at joint 2.

LOAD CASE(S) Standard



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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	J07	JACK	3	1	E4483147
					Job Reference (optional)

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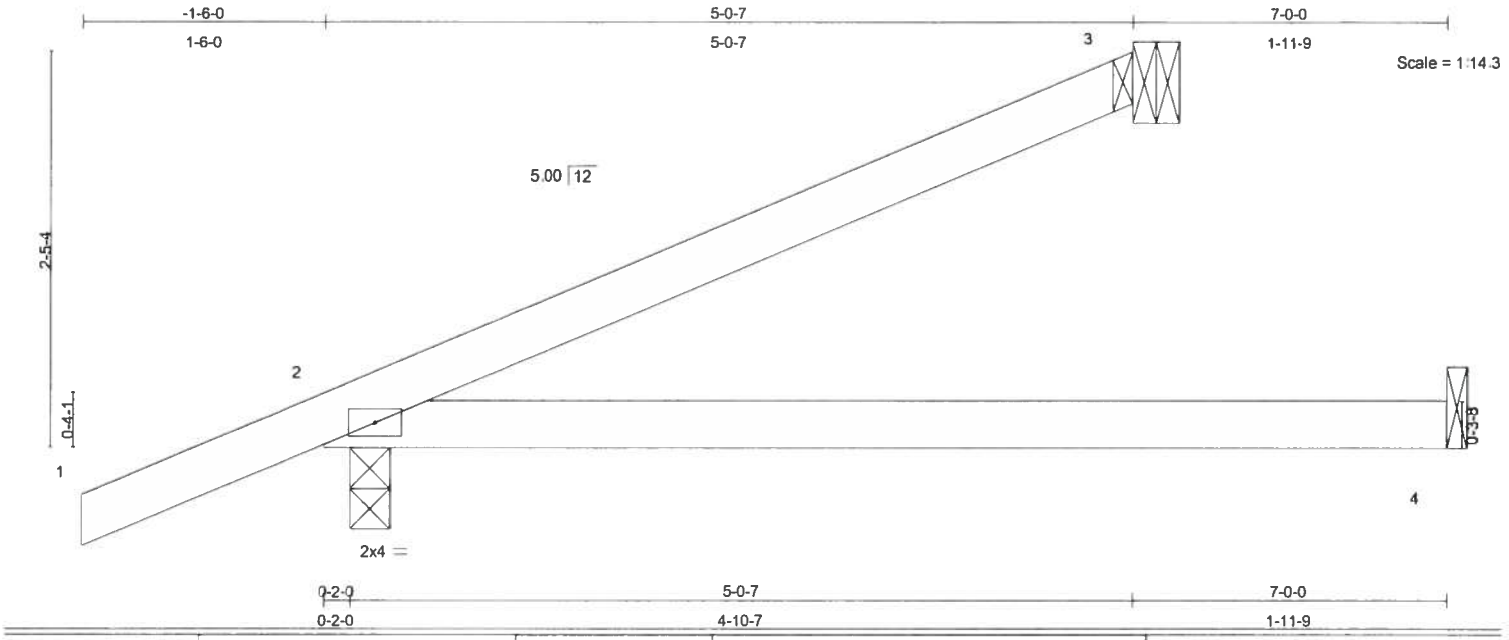


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.11	2-4	>768	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.27	2-4	>307	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

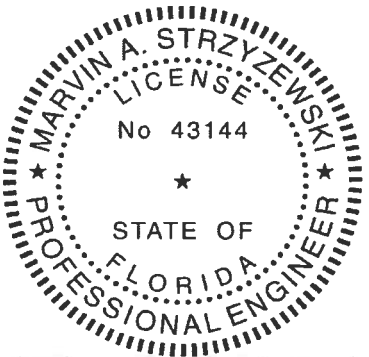
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=126/Mechanical, 4=68/Mechanical, 2=325/0-3-0
Max Horz 2=96(LC 5)
Max Uplift 3=-54(LC 5), 2=-83(LC 5)
Max Grav 3=126(LC 1), 4=136(LC 2), 2=325(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-64/39
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 3 and 83 lb uplift at joint 2.

LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483148
TEEL	J07A	JACK	3	1	Job Reference (optional)	

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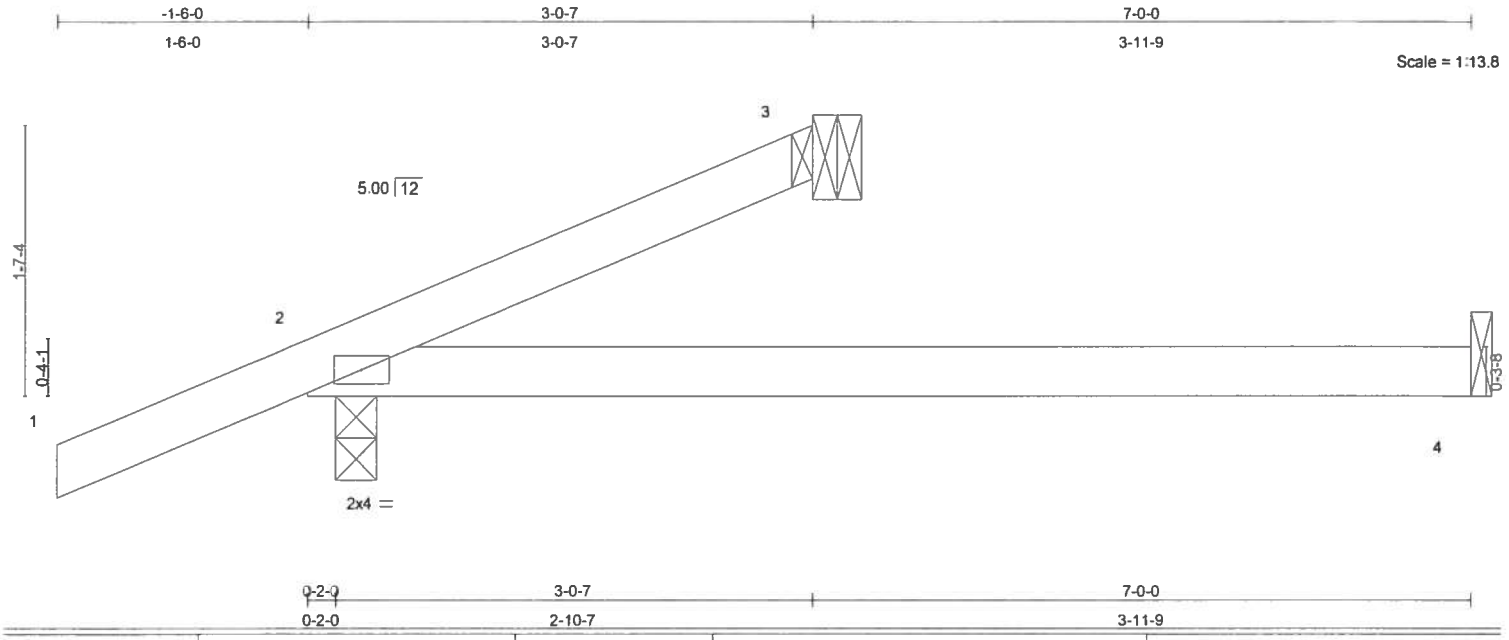

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.11	2-4	>768	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.27	2-4	>307	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=54/Mechanical, 4=68/Mechanical, 2=277/0-3-0
Max Horz 2=70(LC 5)
Max Uplift 3=-21(LC 4), 2=-76(LC 5)
Max Grav 3=54(LC 1), 4=136(LC 2), 2=277(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-46/15
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 3 and 76 lb uplift at joint 2.

LOAD CASE(S) Standard

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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE
TEEL	J07AL	JACK	1	1	E4483149
					Job Reference (optional)

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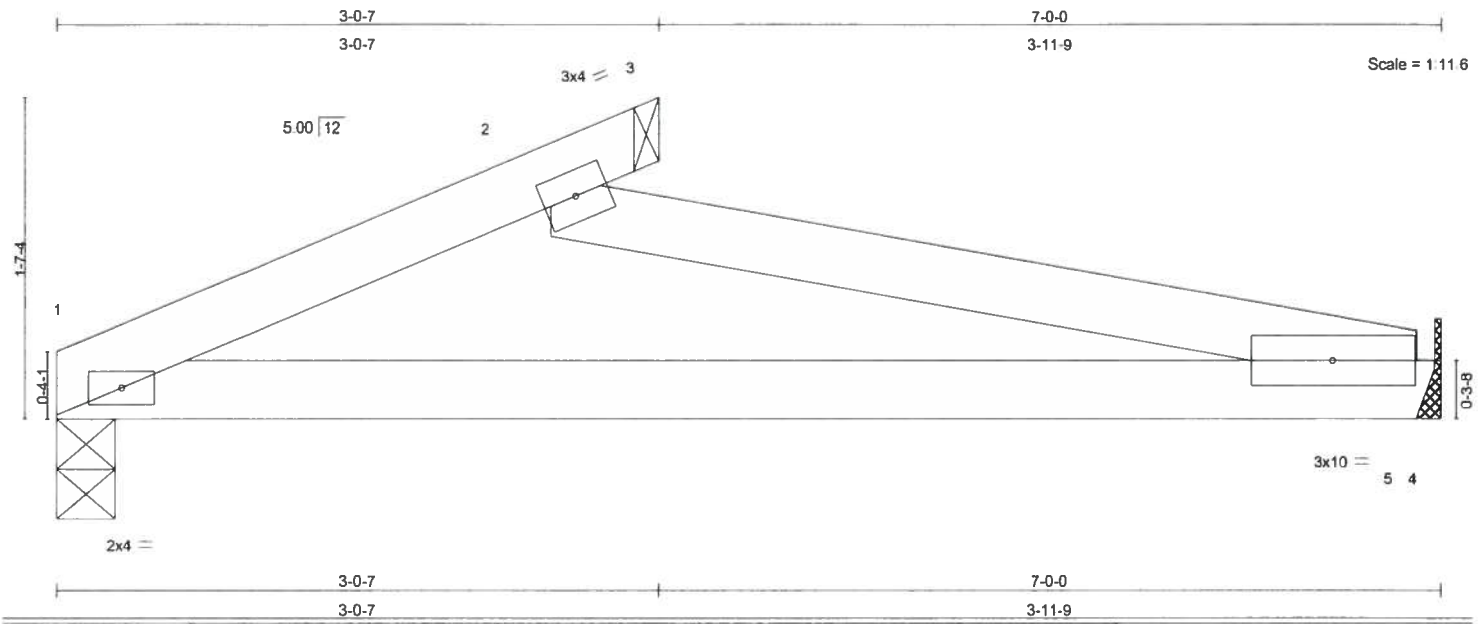


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(LL)	-0.10	1-5	>799	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.25	1-5	>319	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	5	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

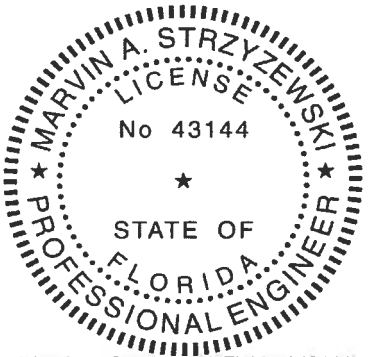
BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=203/0-3-8, 5=108/Mechanical
Max Horz 1=40(LC 5)
Max Uplift1=-10(LC 5)
Max Grav 1=203(LC 1), 5=154(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-194/56, 2-3=-14/0
BOT CHORD 1-5=-70/158, 4-5=0/0
WEBS 2-5=-162/72

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1.

LOAD CASE(S) Standard



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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483150
TEEL	J07B	JACK	4	1	Job Reference (optional)	

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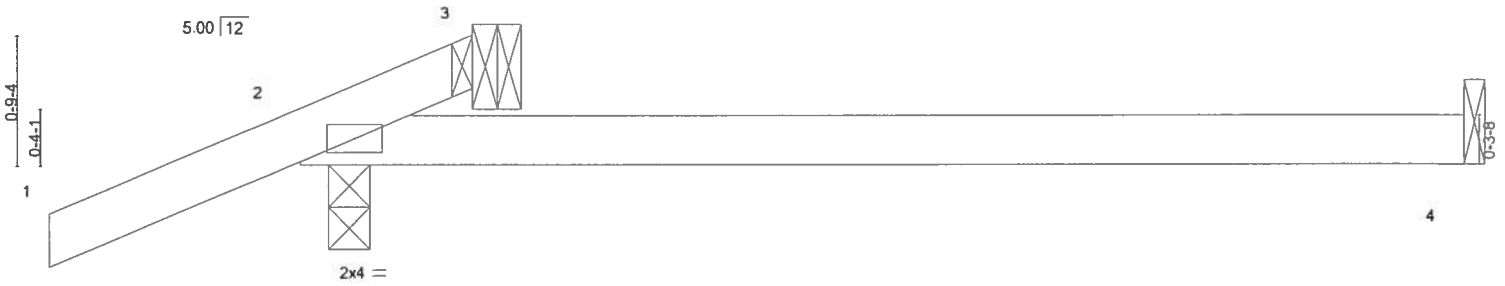
-1-6-0 / 1-6-0 | 1-0-7 / 1-0-7 | 7-0-0 / 5-11-9

Scale = 1:13.8



0-2-0 / 0-2-0 | 1-0-7 / 0-10-7 | 7-0-0 / 5-11-9

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.11	2-4	>736	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.28	2-4	>294	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

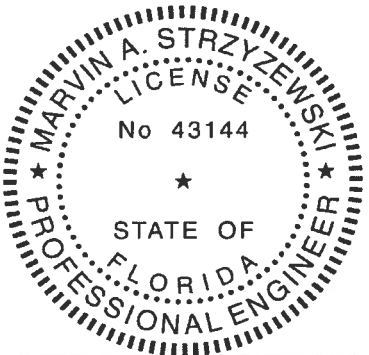
BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=69/Mechanical, 2=261/0-3-0, 3=-42/Mechanical
Max Horz 2=44(LC 5)
Max Uplift 2=-92(LC 5), 3=-42(LC 1)
Max Grav 4=138(LC 2), 2=261(LC 1), 3=43(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-42/17
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 2 and 42 lb uplift at joint 3.

LOAD CASE(S) Standard



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October 26,2007

Job	Truss	Truss Type	Qty	Ply	TEELE RES AND GARAGE	E4483151
TEEL	J07L	JACK	1	1	Job Reference (optional)	

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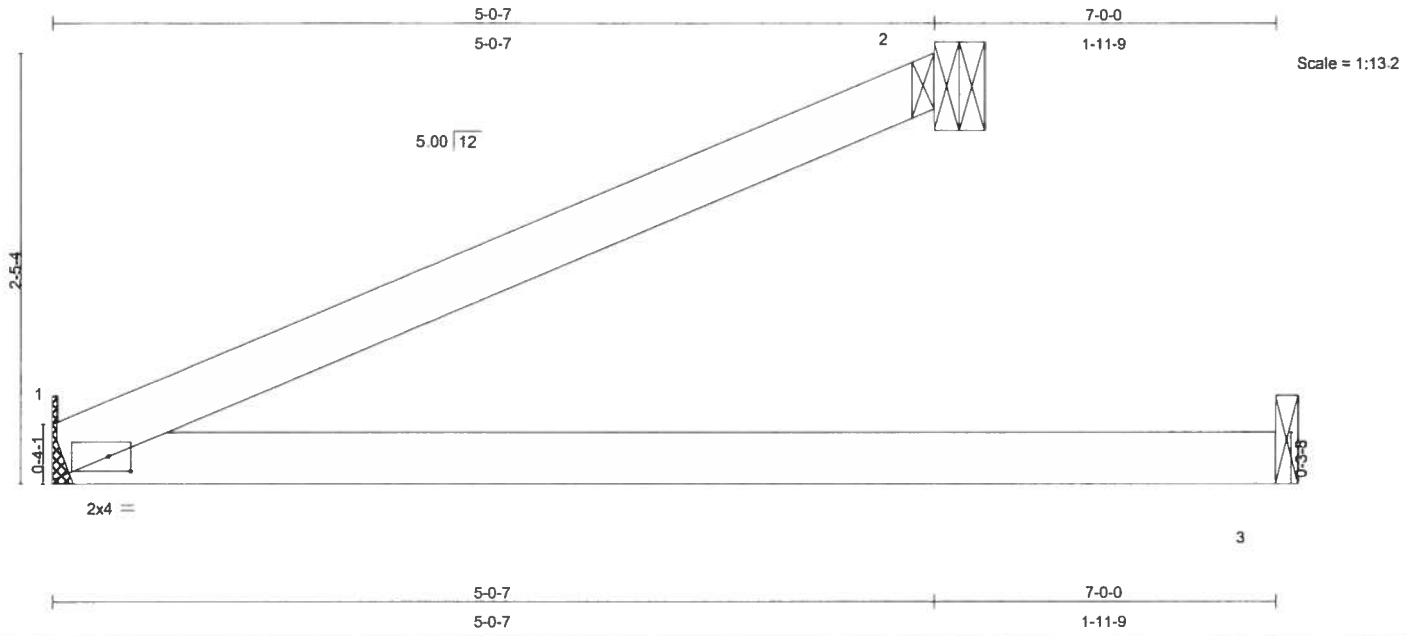


Plate Offsets (X,Y): [1:0-1-8,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.11	1-3	>748	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.28	1-3	>299	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

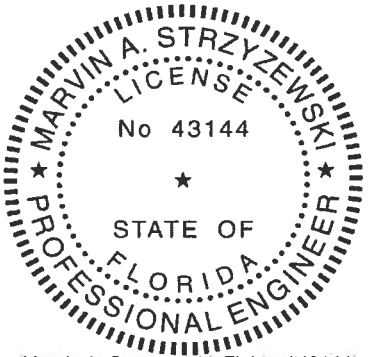
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=214/Mechanical, 2=145/Mechanical, 3=69/Mechanical
Max Horz 1=66(LC 5)
Max Uplift 1=-5(LC 5), 2=-68(LC 5)
Max Grav 1=214(LC 1), 2=145(LC 1), 3=138(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-46/46
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 1 and 68 lb uplift at joint 2.

LOAD CASE(S) Standard



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