DATE 07/02/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000025976

APPLICANT MIKE ROBERTS PHONE 755-9476

ADDRESS 657 SW CATHERINE LANE LAKE CITY FL 32025

OWNER MIKE ROBERTS PHONE 755-9476

ADDRESS 118 SE WILDERNESS DRIVE LAKE CITY FL 32025

CONTRACTOR MIKE ROBERTS PHONE 755-9476

LOCATION OF PROPERTY BAYA, TR ON SR 100, TR ON CR 245, TR ON PLANT ST, TR ON WILDERNESS, 1ST LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 74750.00

HEATED FLOOR AREA 1495.00 TOTAL AREA 2137.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X PS DEVELOPMENT PERMIT NO.

PARCEL ID 03-4S-17-07570-117 SUBDIVISION SUZANNE

LOT 11 BLOCK PHASE UNIT 5 TOTAL ACRES

000001409 | | | Applicant/Owner/Contractor

Culvert Permit No. | Culvert Waiver | Contractor's License Number

WAIVER | 07-429 | BK | JH | Y

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 2230

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Final date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 375.00 CERTIFICATION FEE $ 10.69 SURCHARGE FEE $ 10.69

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 471.38

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

(2)

This Instrument Prepared by & return to:
Name: *Joyce Kirpach, an employee of TITLE OFFICES, LLC*
Address: *1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025*
04Y-01084JK
Parcel I.D. #: *07570-117*

Inst:2004002990 Date:02/11/2004 Time:08:44
Doc Stamp-Deed : 77.00
MK DC,P.DeWitt Cason,Columbia County B:1006 P:2347

SPACE ABOVE THIS LINE FOR PROCESSING DATA | *SPACE ABOVE THIS LINE FOR RECORDING DATA*

THIS WARRANTY DEED *Made the 3 day of* ***February****, A.D.* ***2004****, by*

MARIAN L. JEWELL, a widow, hereinafter called the grantor, to

MIKE W. ROBERTS, single whose post office address is

RT 3 BOX 14862, LAKE CITY, FL 32025, hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantor, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in* ***Columbia County, State of FLORIDA****, viz:*

X Lot 11, SUZANNE SUBDIVISION, UNIT 5, according to the map or plat thereof as recorded in Plat Book 4, Page 100, of the Public Records of Columbia County, FLORIDA.

Restrictions, conditions, reservations, easements, and other matters common to the subdivision or shown on the map or plat thereof recorded in Plat Book 4, Page 100, but omitting any covenant or restriction based on race, color, religion, sex, handicap, familial status or national origin.

Subject to declaration of covenants, conditions and restrictions as recorded in Official Records Book 463 Page 622, but omitting any covenant or restrictions as to race, color, religion, sex, handicap, familial status or national origin.

Together *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.*

To Have and to Hold *the same in fee simple forever.*

And the grantor hereby covenants with said grantee that she is lawfully seized of said land in fee simple; that she has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2003.

In Witness Whereof, *the said grantor has signed and sealed these presents, the day and year first above written.*

Signed, sealed and delivered in the presence of:

Becky Worley
Witness Signature

Becky Worley
Printed Name

Libby Caudell
Witness Signature

Libby Caudell
Printed Name

Marian L. Jewell *L.S.*
MARIAN L. JEWELL
Address:
445 SHOAL CREEK CROSSING, LAVONIA, GA 30553

STATE OF Ga
COUNTY OF Hart

The foregoing instrument was acknowledged before me this 3 *day of Febr* 2004, *by MARIAN L. JEWELL, who is known to me or who has produced* Drivers License *as identification.*

DEBBIE JONES NOTARY PUBLIC Exp. May 06, 2005 HART COUNTY, GA

Notary Public
My commission expires

CK# 2229 CK# 2230

Columbia County Building Permit Application

For Office Use Only Application # 0706-21 Date Received 6/7/07 By LG Permit # 1409/25976

Application Approved by - Zoning Official BLK Date 21.06.07 Plans Examiner OKJTH Date 6-29-07

Flood Zone X per surveyors Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res Low Dev.

Comments

☑ NOC ☑ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Permit

Name Authorized Person Signing Permit Mike Roberts Phone 755-9476 Fax

Address 657 S.W. Catherine Lane L.C. 32025

Owners Name M. Roberts Phone 755-9476

911 Address 118 SE Wilderness Dr, Lake City Fla. 32025

Contractors Name none Phone

Address

Fee Simple Owner Name & Address

Bonding Co. Name & Address none

Architect/Engineer Name & Address Schaffer Eng. 7104 N.W. 42nd Lane Gainsville 32606

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 03-4S-17-07590-117 Estimated Cost of Construction 120,000.00

Subdivision Name SUZANNE S/D Lot 11 Block Unit 5 Phase

Driving Directions S.E. Baya Right on 100 Right on C. 245 Right on S.E Plant St. Right S.E. Wilderness Drive First hot on left

Type of Construction Wood Frame - SFD Number of Existing Dwellings on Property 0

Total Acreage 1/3 Lot Size Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 45' Side 55' Side 36' Rear 60'

Total Building Height 18'2 Number of Stories 1 Heated Floor Area 1495 TOTAL 2137 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Mike Roberts

Owner Builder or Authorized Person by Notarized Letter

Contractor Signature

Contractors License Number

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL

DALE TEDDER
MY COMMISSION # DD 333586
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 7th day of June 2007.

Personally known ✓ or Produced Identification

Dale Tedder

Notary Signature

(Revised Sept. 2006)

ATN: WEGGIE

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000001409

DATE: 07/02/2007 BUILDING PERMIT NO. 25976

APPLICANT MIKE ROBERTS PHONE 755-9476

ADDRESS 657 SW CATHERINE LANE LAKE CITY FL 32025

OWNER MIKE ROBERTS PHONE 755-9476

ADDRESS 118 SE WILDERNESS DRIVE LAKE CITY FL 32025

CONTRACTOR MIKE ROBERTS PHONE

LOCATION OF PROPERTY BAYA, TR ON SR 100, TR ON CR 245, TR ON PLANT ST, TR ON WILDERNESS, 1ST LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT SUZANNE 11 5

PARCEL ID # 03-4S-17-07570-117

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Mike Roberts

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED _____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: _____

SIGNED: _____ DATE: 7-10-07

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

RECEIVED
JUL 05 2007
By

COLUMBIA COUNTY GATEWAY TO FLORIDA

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

(X) Single Family Dwelling
( ) Farm Outbuilding
( ) Two-Family Residence
( ) Other_____

NEW CONSTRUCTION OR IMPROVEMENT

(X) New Construction
( ) Addition, Alteration, Modification or other Improvement

I Mike Roberts, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

Mike Roberts 6/7/07
Owner Builder Signature Date

GALE TEDDER
MY COMMISSION # DD 333586
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

The above signer is personally known to me or produced identification _____

Notary Signature Gale Tedder Date 6/7/07 (Stamp / Seal)

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

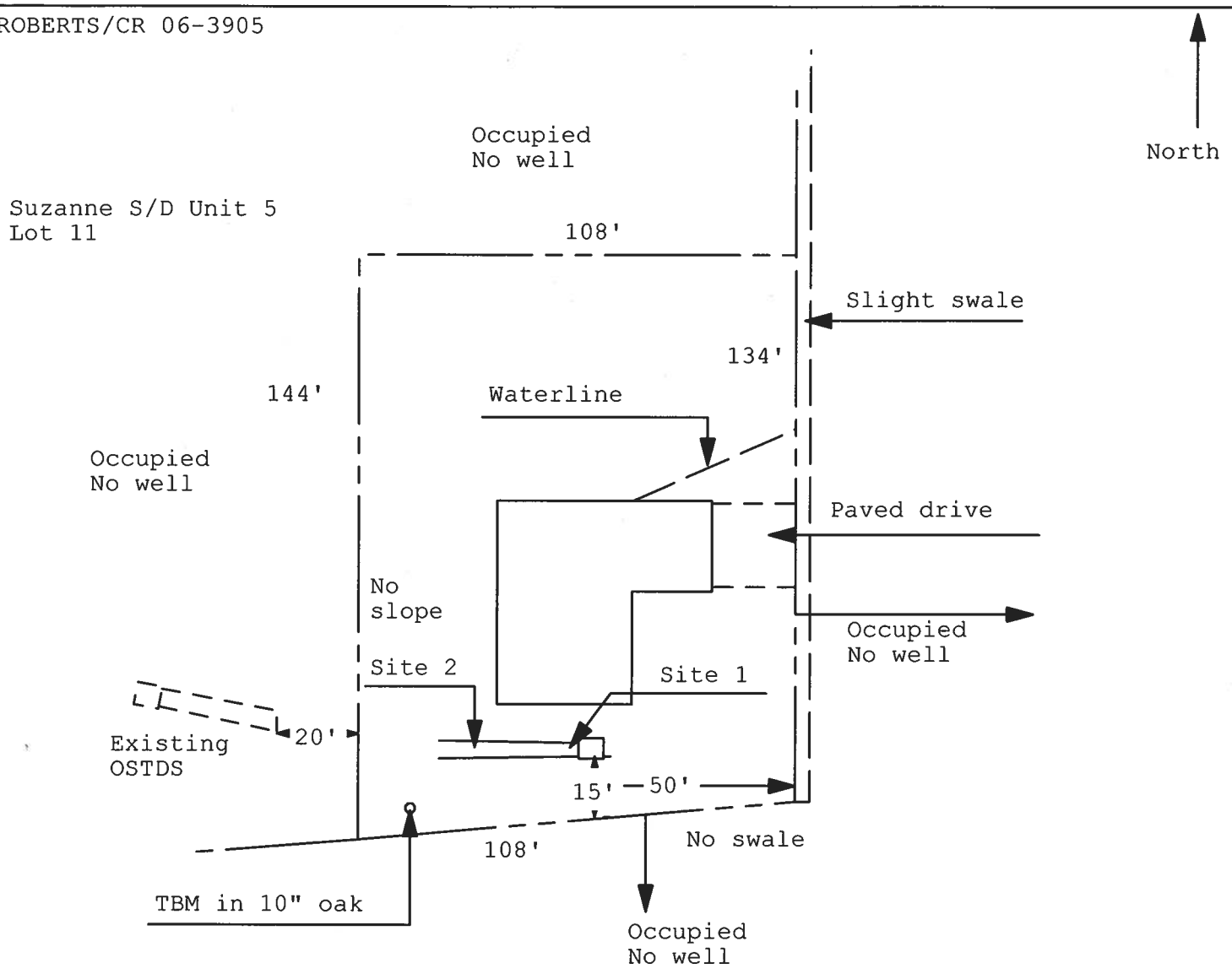
Date_____ Building Official/Representative_____

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07-429

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

ROBERTS/CR 06-3905



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd **Date** 3/2/07

Plan Approved ✓ **Not Approved** ____ **Date** 5/21/07

By [illegible] Columbia **CPHU**

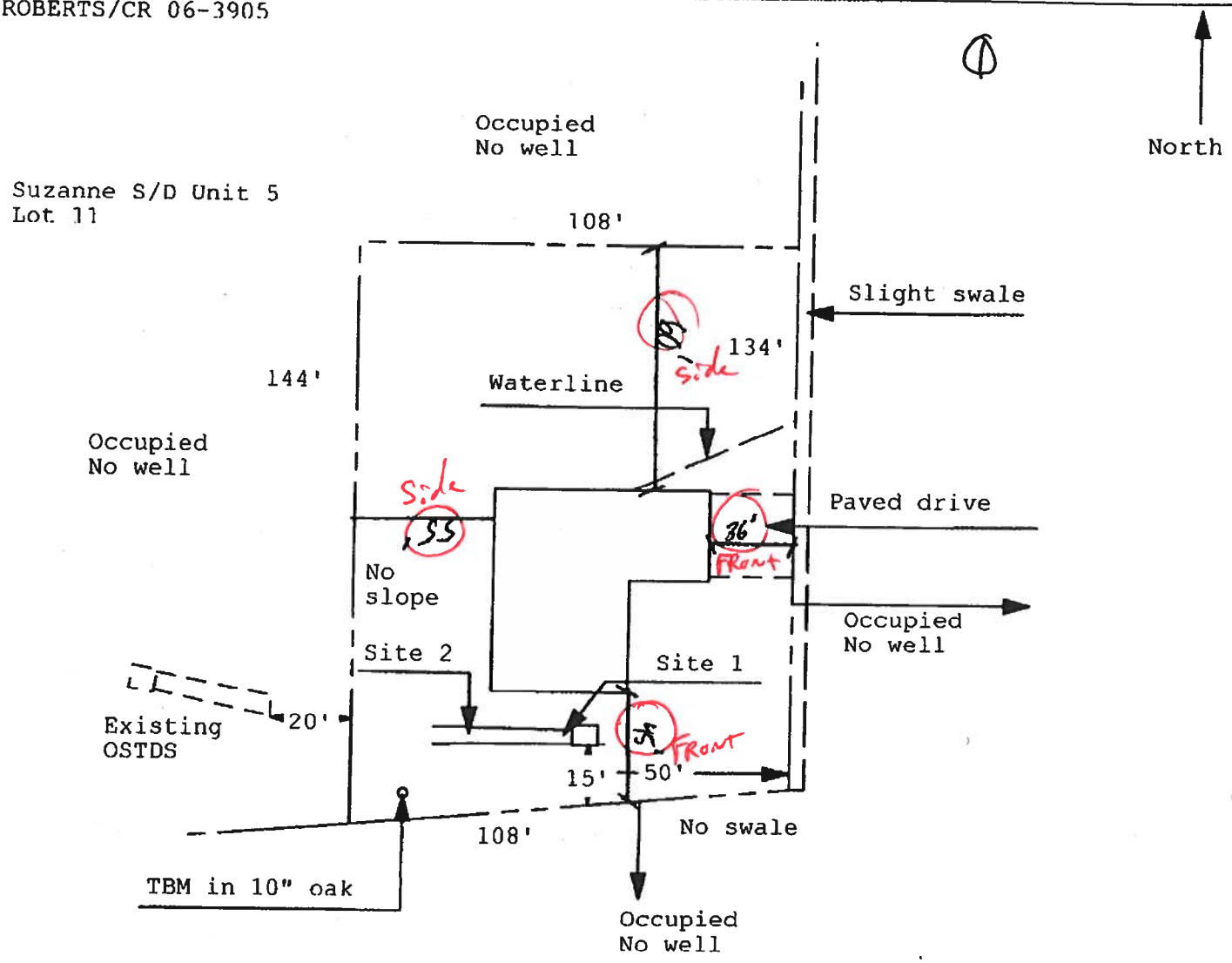
Notes: ____

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: _____

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

ROBERTS/CR 06-3905



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd **Date** 3/2/07

Plan Approved_____ **Not Approved**_____ **Date**_____

By_____ _____**CPHU**

Notes:_____

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 3/1/2007 **DATE ISSUED:** 3/2/2007

ENHANCED 9-1-1 ADDRESS:

118 SE WILDERNESS DR

LAKE CITY FL 32025

PROPERTY APPRAISER PARCEL NUMBER:

03-4S-17-07570-117

Remarks:

LOT 11 SUZANNE S/D UNIT 5

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

646

Approved Address

MAR 0 2 2007

911Addressing/GIS Dept

(1)

Lenvil Dicks Water Systems

1286 W. US HWY 90
PO BOX 1
LAKE CITY, FL 32056

1-800-545-3501
FAX(386)758-6760

June 6, 2007

To Whom It May Concern:

The purpose of this letter is to verify that Suzanne Subdivision Units I through 5, recorded subdivisions in Columbia County, Florida, are served by the Clayton Smith community water system. This system, identified by I.D. number 2124213, is regulated by the Florida Public Service Commission and the Department of Environmental Protection. The Columbia County Environmental Health office receives our monthly bacteriological testing. We would be happy to provide copies of recent test results. Further inquiries may be directed to one of these official agencies.

Sincerely,

Andy J. Dicks

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and inaccordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.
IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Tax Parcel ID Number 03-4S-17-07570-117 **Permit Number** _____

1. Description of property: (legal description of the property and street address or 911 address)
Lot 11 SUZANNE S/D Unit 5
118 S.E Wilderness Dr.
Lake City Fla 32025

2. General description of improvement: NEW Home

3. Owner Name & Address Mike Roberts 657 S.W. CAtherine Lane L.C. FLA. 32025 **Interest in Property** Owner

4. Name & Address of Fee Simple Owner (if other than owner): _____

5. Contractor Name Mike Roberts **Phone Number** 755-9476
Address _____

6. Surety Holders Name none
Address _____
Amount of Bond _____

Inst:2007011788 Date:05/29/2007 Time:12:57
DC,P.DeWitt Cason,Columbia County B:1120 P:843

7. Lender Name none
Address _____

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name N. A. **Phone Number** _____
Address _____

9. In addition to himself/herself the owner designates N. A **of** _____ **to receive a copy of the Lien Notice as provided in Section 713.13 (1) –**
(a) 7. Phone Number of the designee N. A

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) _____

THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN IN HIS/HER STEAD.

Mike Roberts
Signature of Owner

CASEY ROBERTS
MY COMMISSION # DD583642
EXPIRES: August 10, 2010
Fl. Notary Discount Assoc. Co.
1-800-3-NOTARY

Sworn to (or affirmed) and subscribed before day of 29th May, 2007.

Casey Roberts
Signature of Notary

NOTARY STAMP/SEAL
As to signature Only

FORM 600A-2004R

(1)

EnergyGauge® 4.5

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Mike Roberts**	Builder:	**owner**
Address:		Permitting Office:	Columbia
City, State:	,	Permit Number:	25976
Owner:		Jurisdiction Number:	24000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 28.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft²)	1404 ft²	13. Heating systems	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		a. Electric Heat Pump	Cap: 30.0 kBtu/hr
a. U-factor:	Description Area		HSPF: 8.00
(or Single or Double DEFAULT)	7a. (Dble Default) 146.0 ft²	b. N/A	
b. SHGC:		c. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 146.0 ft²	14. Hot water systems	
8. Floor types		a. Electric Resistance	Cap: 50.0 gallons
a. Slab-On-Grade Edge Insulation	R=0.0, 180.0(p) ft		EF: 0.90
b. N/A		b. N/A	
c. N/A		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
9. Wall types		15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
a. Frame, Wood, Adjacent	R=13.0, 290.0 ft²		
b. Frame, Wood, Exterior	R=13.0, 1100.0 ft²		
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 1404.0 ft²		
b. Under Attic	R=19.0, 200.0 ft²		
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 123.0 ft		
b. N/A			

Glass/Floor Area: 0.10

Total as-built points: 20876

Total base points: 21356

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Suncoast Insulators

DATE: 5/31/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1404.0	18.59	4698.0	1.Double, Clear	E	2.0	5.0	59.0	42.06	0.80	1977.0
				2.Double, Clear	W	2.0	5.0	77.0	38.52	0.80	2371.0
				3.Double, Clear	S	2.0	5.0	4.0	35.87	0.72	103.0
				4.Double, Clear	N	2.0	5.0	6.0	19.20	0.87	100.0
				As-Built Total:				**146.0**			**4551.0**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	290.0	0.70	203.0	1. Frame, Wood, Adjacent			13.0	290.0	0.60		174.0
Exterior	1100.0	1.70	1870.0	2. Frame, Wood, Exterior			13.0	1100.0	1.50		1650.0
Base Total:	**1390.0**		**2073.0**	**As-Built Total:**				**1390.0**			**1824.0**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	18.0	2.40	43.2	1.Exterior Insulated				18.0	4.10		73.8
Exterior	18.0	6.10	109.8	2.Adjacent Insulated				18.0	1.60		28.8
Base Total:	**36.0**		**153.0**	**As-Built Total:**				**36.0**			**102.6**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1404.0	1.73	2428.9	1. Under Attic			30.0	1404.0	1.73 X 1.00		2428.9
				2. Under Attic			19.0	200.0	2.34 X 1.00		468.0
Base Total:	**1404.0**		**2428.9**	**As-Built Total:**				**1604.0**			**2896.9**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	180.0(p)	-37.0	-6660.0	1. Slab-On-Grade Edge Insulation			0.0	180.0(p	-41.20		-7416.0
Raised	0.0	0.00	0.0								
Base Total:			**-6660.0**	**As-Built Total:**				**180.0**			**-7416.0**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1404.0	10.21	14334.8					1404.0	10.21		14334.8

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 17027.8			**Summer As-Built Points:**					**16293.4**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 28000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
17027.8	0.3250	5534.0	16293 **16293.4**	1.00 **1.00**	(1.09 x 1.147 x 1.00) **1.250**	0.260 **0.260**	1.000 **1.000**	5296.3 **5296.3**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1404.0	20.17	5097.0	1.Double, Clear	E	2.0	5.0	59.0	18.79	1.08	1201.0
				2.Double, Clear	W	2.0	5.0	77.0	20.73	1.06	1690.0
				3.Double, Clear	S	2.0	5.0	4.0	13.30	1.40	74.0
				4.Double, Clear	N	2.0	5.0	6.0	24.58	1.01	148.0
				As-Built Total:				**146.0**			**3113.0**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	290.0	3.60	1044.0	1. Frame, Wood, Adjacent			13.0	290.0	3.30		957.0
Exterior	1100.0	3.70	4070.0	2. Frame, Wood, Exterior			13.0	1100.0	3.40		3740.0
Base Total:	**1390.0**		**5114.0**	**As-Built Total:**				**1390.0**			**4697.0**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	18.0	11.50	207.0	1.Exterior Insulated				18.0	8.40		151.2
Exterior	18.0	12.30	221.4	2.Adjacent Insulated				18.0	8.00		144.0
Base Total:	**36.0**		**428.4**	**As-Built Total:**				**36.0**			**295.2**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points
Under Attic	1404.0	2.05	2878.2	1. Under Attic			30.0	1404.0	2.05 X 1.00		2878.2
				2. Under Attic			19.0	200.0	2.70 X 1.00		540.0
Base Total:	**1404.0**		**2878.2**	**As-Built Total:**				**1604.0**			**3418.2**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	180.0(p)	8.9	1602.0	1. Slab-On-Grade Edge Insulation			0.0	180.0(p	18.80		3384.0
Raised	0.0	0.00	0.0								
Base Total:			**1602.0**	**As-Built Total:**				**180.0**			**3384.0**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1404.0	-0.59	-828.4					1404.0	-0.59		-828.4

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**14291.2**	**Winter As-Built Points:**					**14079.0**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 30000 btuh ,EFF(8.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
14291.2	**0.5540**	**7917.3**	14079.0 **14079.0**	1.000 **1.00**	(1.069 x 1.169 x 1.00) **1.250**	0.426 **0.426**	1.000 **1.000**	7499.5 **7499.5**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X Multiplier	= Total		Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3	2635.00	7905.0		50.0	0.90	3	1.00	2693.56	1.00	8080.7
				As-Built Total:						8080.7

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
5534	7917	7905	21356	5296	7499	8081	20876

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.9

The higher the score, the more efficient the home.

1.	New construction or existing	New		12.	Cooling systems
2.	Single family or multi-family	Single family		a.	Central Unit	Cap: 28.0 kBtu/hr
3.	Number of units, if multi-family	1				SEER: 13.00
4.	Number of Bedrooms	3		b.	N/A	
5.	Is this a worst case?	Yes				
6.	Conditioned floor area (ft²)	1404 ft²		c.	N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)					
a.	U-factor:	Description Area		13.	Heating systems	
	(or Single or Double DEFAULT)	7a. (Dble Default) 146.0 ft²		a.	Electric Heat Pump	Cap: 30.0 kBtu/hr
b.	SHGC:					HSPF: 8.00
	(or Clear or Tint DEFAULT)	7b. (Clear) 146.0 ft²		b.	N/A	
8.	Floor types					
a.	Slab-On-Grade Edge Insulation	R=0.0, 180.0(p) ft		c.	N/A	
b.	N/A					
c.	N/A			14.	Hot water systems	
9.	Wall types			a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Adjacent	R=13.0, 290.0 ft²				EF: 0.90
b.	Frame, Wood, Exterior	R=13.0, 1100.0 ft²		b.	N/A	
c.	N/A					
d.	N/A			c.	Conservation credits	
e.	N/A				(HR-Heat recovery, Solar	
10.	Ceiling types				DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1404.0 ft²		15.	HVAC credits	
b.	Under Attic	R=19.0, 200.0 ft²			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A				HF-Whole house fan,	
11.	Ducts				PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 123.0 ft			MZ-C-Multizone cooling,	
b.	N/A				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SCHAFER ENGINEERING, LLC

7104 N. W. 42ND LANE

GAINESVILLE, FLORIDA 32606

THE ROBERTS RESIDENCE \ LOT 11 SUZANNE SUBDIVISION

SCHAFER ENGINEERING, LLC

NO COPIES TO BE PERMITTED

Florida Building Code \ Latest Edition

386-462-1340 / 352-375-6329

SCHAFER ENGINEERING LLC

Trusses: Pre-engineered with manufacturer's required bracing system installed.

Roof sheathing: Type OSB Size 7/16 Fastener type Nails Size 8d/.113 Ring Shank
R803.2.3.1

Interior zone spacing: Interior 6 in. Periphery 4 in.
Edge and end zone spacing: Interior 6 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 10 in.

Studs: Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4
Interior stud spacing 16 in. Composite (yes or no) Y
End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB Thickness 7/16 in.
Trans: Fastener 8d / 131 Spacing: Int 8 in. Edge 4 in.
Long: Fastener 8d / 131 Spacing: Int 8 in. Edge 4 in.

Allowable unit shear on shearwalls: 322 **pounds per linear foot**
Unit shear transferred from diaphragm: Trans: 65 Long: 62

Wall tension transferred by: Siding nails 8d/131 @ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi
Size 1/2 in. Shape L Washer 2" Embedment 7 in.
Location of first anchor bolt from corner 8 in.

Anchor Bolts @ 48" O.C. Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. #5 at 72 in.
Monolithic footing: Depth 20 in. Bottom width 12 in.

Footing: Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars
Interior Footings: 16" W X 10" D

Porch Columns: 4X4X9 Syp #2 PT @ 84" o.c MAX

Porch Column Fasteners: Simpson CBSQ/CCQ OR EQUIL

Install OSB ceiling diaphragm on open rear porch with same GRADE MATERIAL, NAIL SIZE AND SPACING AS ROOF SHEATHING

NOTE:

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

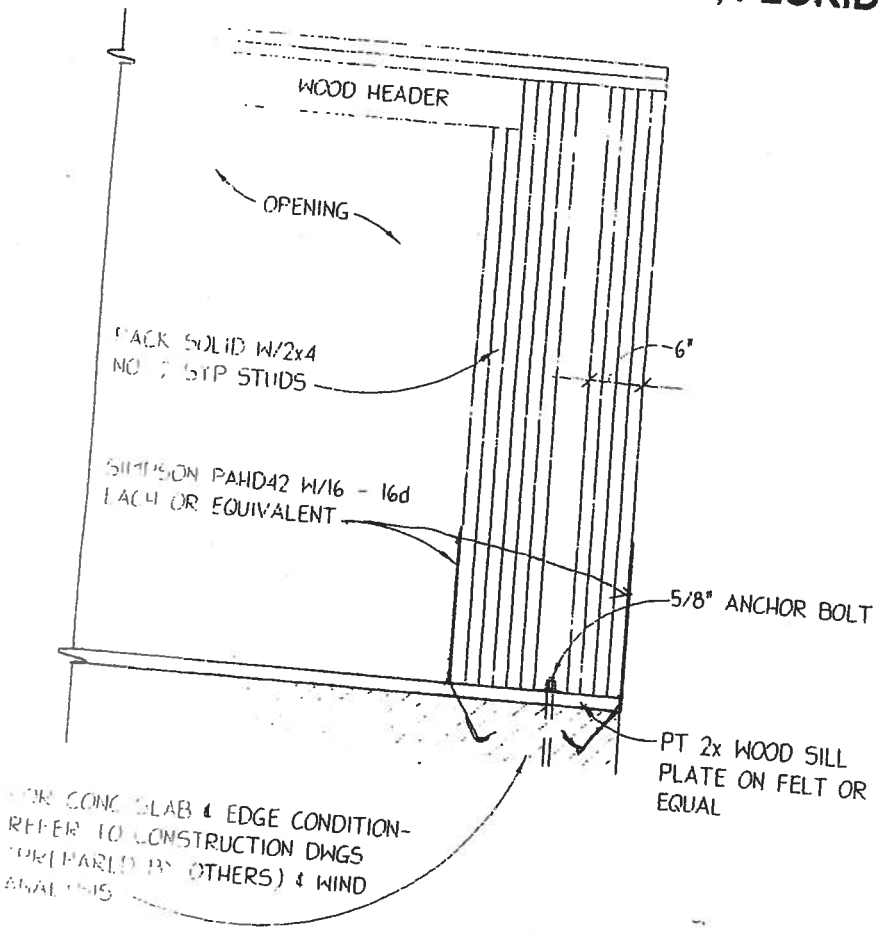
3-8-07

48984
7104 NW 42nd Ln
Gainesville, Fl

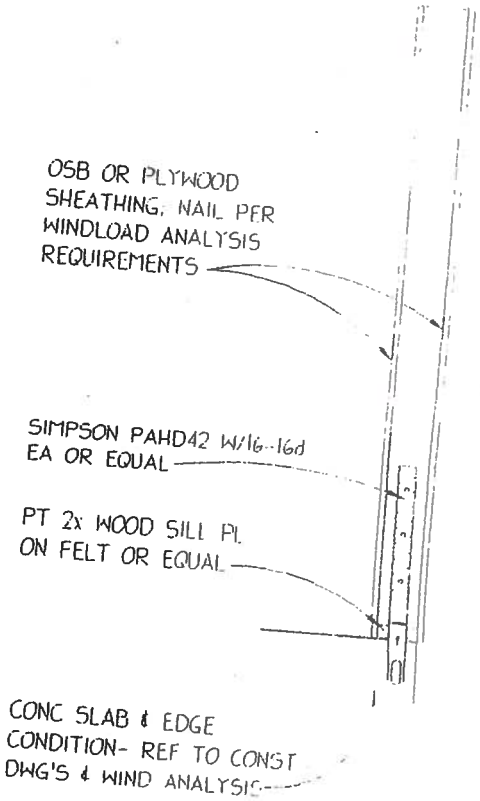
SCHAFER ENGINEERING, LLC

7104 N. W. 42ND LANE

GAINESVILLE, FLORIDA 32606



FRONT VIEW



SIDE VIEW

EQUIVALENT 3'-0" SHEAR WALL SEGMENT

3-8-07

48984
7104 NW 42nd Ln
Gainesville, Fl

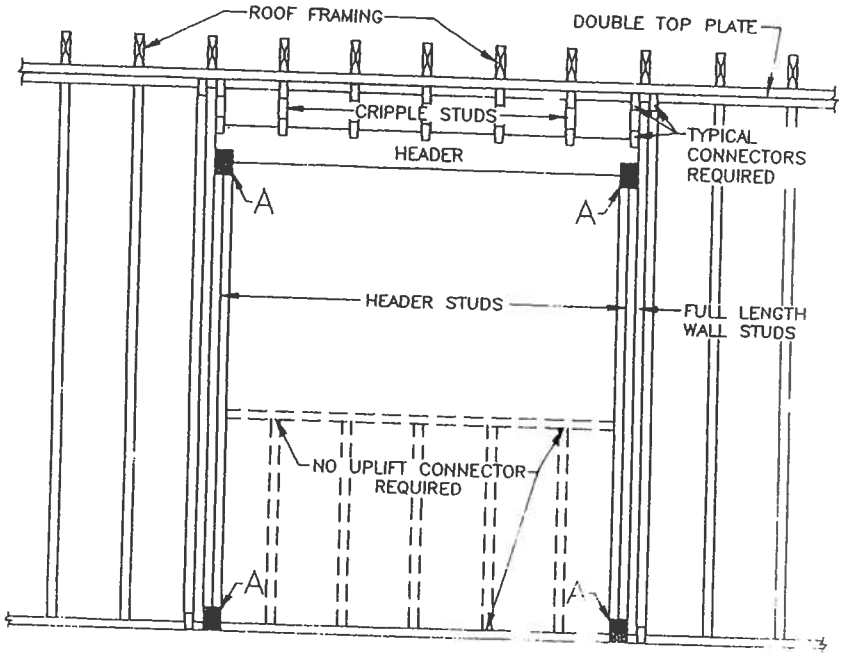
STATE OF FLORIDA

Unsupported Wall Height	Stud Spacing	Maximum Header Span (ft.)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting End of Header					
		1[1]	1	2	2	2	2
		Number of Full-Length Studs at Each End of Header					
10' or less	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
greater than 10'	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

1. The header stud shall not be required if the header is supported by a suitable framing anchor.

Uplift connection requirement at points A (top and bottom of header studs). Uplift load per framing member above the header from Table 307F1 or 307A, as appropriate, multiplied by the number of framing members displaced divided by two

NOTE. Uplift connection is required at each end of header and at bottom of header studs in addition to connectors at wall studs and at top and bottom of cripples.



TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force.
Use proper bolt anchors sufficient to support required load.

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate.
Use proper bolt anchors.
Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.
Strap ridge beam at each end with minimum uplift resistance of 1000#.
It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300

*or per truss engineering
Use proper bolt anchors
All beams to be sheathed or strapped to Double Top Plate when applicable.

CRIPPLES	Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS
Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
Use SP1 & SP2 @ 32" O.C. on all bearing walls.
Interior anchor bolts to be ½" x 8" A307 or ½" x 7" wedge anchor or equivalent.

** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE:

1. For nailing into SPF members, multiply table values by .86
2. See truss engineering for anchor tie-down values.

ASCE 7-02

3/7/07

Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	9.50	ft
Ridge Height (RHt)	18.67	ft
Mean Roof Height (Ht)	14.02	ft
Width Perp. to Wind (B)	55.00	ft
Width Parallel to Wind (L)	50.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.28	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8964	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8639	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	0.91	
Nb	4.6*NatFreq*B/Vzm	3.57	
Nd	15.4*NatFreq*Depth/Vzm	10.86	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.5930	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.2410	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0878	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.6712	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.05	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

ASCE 7-02

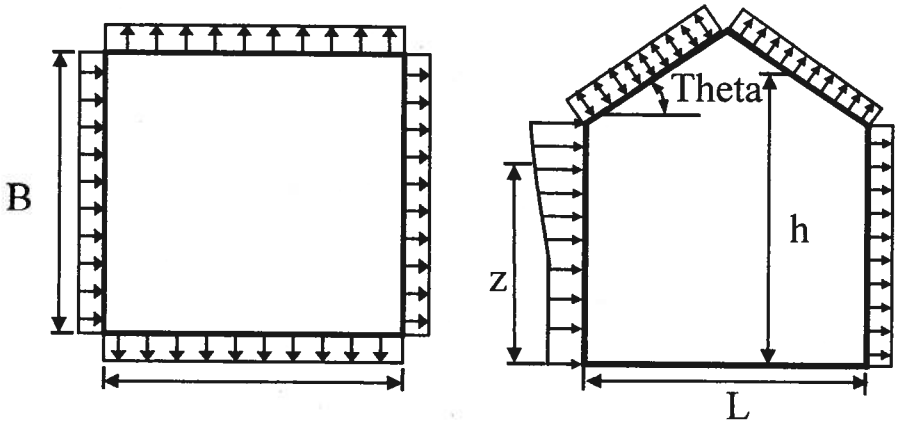
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
18.67	0.70	1.00	1.00	21.70	11.79	18.20
15	0.70	1.00	1.00	21.70	11.79	18.20

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 55 ft wall)	-0.50	-10.89	-4.48
Leeward Walls (Wind Dir Parallel to 50 ft wall)	-0.48	-10.59	-4.18
Side Walls	-0.70	-13.97	-7.56
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.21	-6.41	0.00
Windward - Max Positive	0.29	1.22	7.63
Leeward Normal to Ridge	-0.60	-12.43	-6.02
Overhang Top	-0.21	-3.20	-3.20
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 7.01 ft	-0.90	-17.05	-10.64
Dist from Windward Edge: 7.01 ft to 14.02 ft	-0.90	-17.05	-10.64
Dist from Windward Edge: 14.02 ft to 28.04 ft	-0.50	-10.89	-4.48

ASCE 7-02

3/7/07

Wind Load Design per ASCE 7-02

Dist from Windward Edge: > 28.04 ft	-0.30	-7.82	-1.41

* Horizontal distance from windward edge

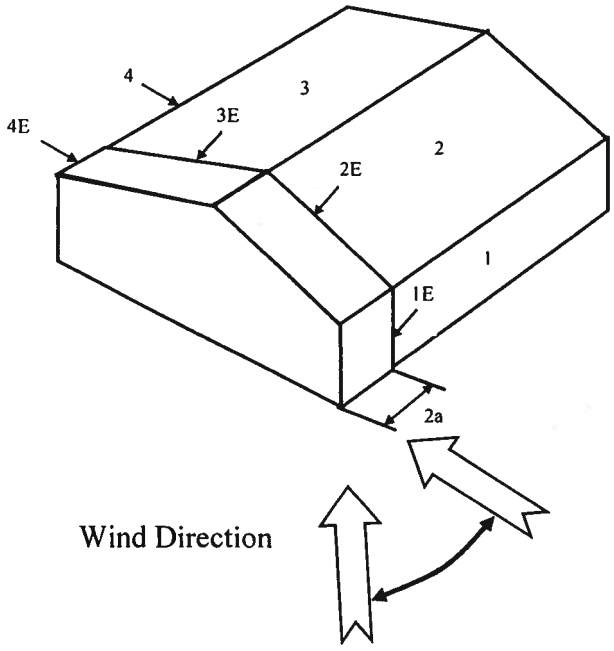
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-02

Wind Load Design per ASCE 7-02

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

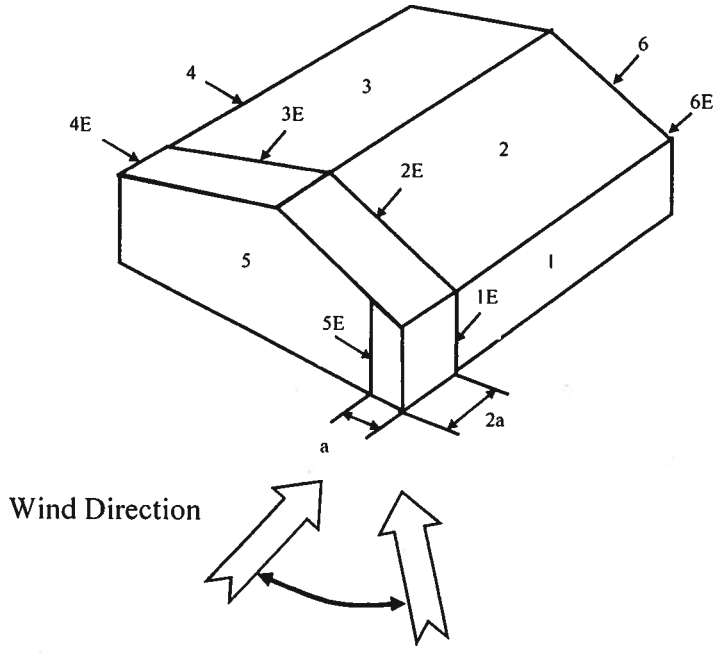
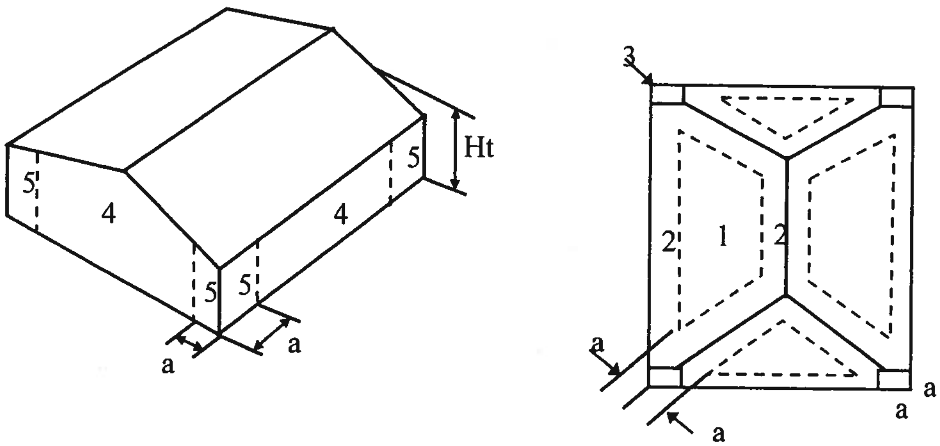


Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft

ASCE 7-02

Wind Load Design per ASCE 7-02



Hipped Roof
10 < Theta <= 30

a = 5 ==> 5.00 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp Max	GCp Min	Wind Press (lb/ft^2) Max	Wind Press (lb/ft^2) Min
	16	7	112.00	5	0.81	-1.03	17.71	-21.53
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi Max +	Gcpi Max -

Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.28	-18.58	-10.89
	P (-GCpi) -psf	5.13	-12.17	-4.48
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.25	-18.58	-10.89
	P (-GCpi) -psf	3.25	-18.58	-10.89

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	50.00	ft
B	Roof dimension parallel to wind direction	55.00	ft
L/B	Ratio of L to B	0.909	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.19	
X	Distance to center of pressure from windward edge	0.42	ft

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0706-21

Mike Roberts Owner/Builder Property ID# 03-4s-17-07590-117 on the date of June 13, 2007 application 0706-21 and plans for construction of a single family dwelling was reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0706-21 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

1. The energy effiency which was submitted form 600A-2004R line number six (condition floor space) has the total square of condition floor space as 1404 square feet. The submitted plan notes within the area summary that the living area is 1495. Line six of the form should equal the living area. Please have the preparer of this Florida Energy Efficiency Code For Building Construction recalculate this for using the correct total condition (living) area.

2. The floor plan shows a 16 foot garage door opening. The truss plan shows that the roof trusses will bear over this opening. Please have your windload engineer design the proper header which will span this opening. Show the header size, type, number of king jack studs. Also show the required fastening device which will be required to connect this header to the foundation.

3. The electrical plans show no detail as to the size and location of the electrical service. On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Also show the total amperage rating for the electrical service.

4. The garage area shows an attic pull down access door. This door is considered by the residential building section R309.1 as an opening between the garage and residence this door shall have a 20-minute fire-rated door. The entry door into the living area from the garage shall also be a 20-minute fire-rated door.

Please correct the above requirements and submit a written addendum which corrects these requirements.

Joe Haltiwanger

Plan Examiner
Columbia County Building Department

iLevel™ TRUS JOIST®
engineered lumber products

Southeast Division
6001 Jackson Square, Suite 600
LaVergne, TN 37086
Phone: (615) 793-7788
Toll Free: (800) 854-5647
Fax: (615) 793-7721

Transmittal Cover Sheet

To:	**Kim Halloway**
Company:	**Builders First Source**
	2525 East Duval Street
	Lake City, FL 32055
Phone:	**386-755-6894**
FAX:	**386-755-7973**

Date: **6/22/07**

From: **Chad Wall**

Number of pages (including cover) **2**

3 Copies to follow by:

☐ Mail ☐ Electronic ☐ Next Day ☑ 2nd Day

☐ Original will not follow

Project Information:

Lot 11 Suzanne

Columbia County, FL

Comments:

Thank you for specifying Trus Joist products.

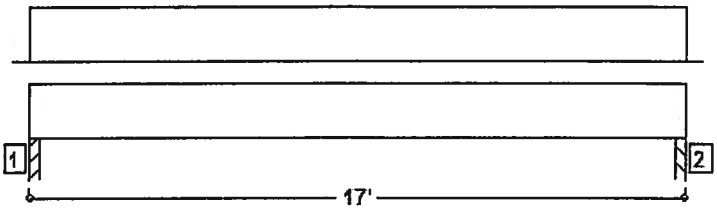
TJ-Beam® 6.25 Serial Number: 7005101720
User: 4 6/22/2007 1:38:02 PM
Page 1 Engine Version: 6.25.71

Garage Door Header

2 Pcs of 1 3/4" x 11 7/8" 1.9E Microllam® LVL

THIS PRODUCT MEETS OR EXCEEDS THE SET DESIGN CONTROLS FOR THE APPLICATION AND LOADS LISTED

Member Slope: 0/12 Roof Slope0/12



All dimensions are horizontal.

Product Diagram is Conceptual.

LOADS:

Analysis is for a Drop Beam Member. Tributary Load Width: 1'
Primary Load Group - Roof (psf): 20.0 Live at 125 % duration, 15.0 Dead
Vertical Loads:

Type	Class	Live	Dead	Location	Application	Comment
Uniform(plf)	Roof(1.25)	99.0	40.0	0 To 17'	Adds To	EJ3 Loads

SUPPORTS:

		Input Width	Bearing Length	Vertical Reactions (lbs) Live/Dead/Uplift/Total	Detail	Other
1	Trimmers	3.00"	1.50"	1012 / 565 / 0 / 1577	L2	None
2	Trimmers	3.00"	1.50"	1012 / 565 / 0 / 1577	L2	None

DESIGN CONTROLS:

	Maximum	Design	Control	Control	Location
Shear (lbs)	1553	-1347	9871	Passed (14%)	Rt. end Span 1 under Roof loading
Moment (Ft-Lbs)	6505	6505	22310	Passed (29%)	MID Span 1 under Roof loading
Live Load Defl (in)		0.239	0.558	Passed (L/840)	MID Span 1 under Roof loading
Total Load Defl (in)		0.373	0.837	Passed (L/539)	MID Span 1 under Roof loading

-Deflection Criteria: MINIMUM(LL:L/360,TL:L/240).
-Bracing(Lu): All compression edges (top and bottom) must be braced at 17' o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

ADDITIONAL NOTES:

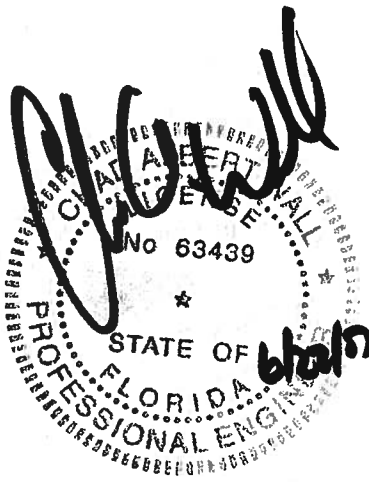
-IMPORTANT! The analysis presented is output from software developed by Trus Joist (TJ). Allowable product values shown are in accordance with current TJ materials and code accepted design values. TJ Engineering has verified the analysis. The input loads and dimensions have been provided by others (KIM HOLLOWAY : BFS) and must be verified and approved for the specific application by the design professional for the project.
-THIS ANALYSIS FOR TRUS JOIST PRODUCTS ONLY! PRODUCT SUBSTITUTION VOIDS THIS ANALYSIS.
-Allowable Stress Design methodology was used for Building Code IBC analyzing the TJ Custom product listed above.
-Note: See TJ SPECIFIER'S / BUILDER'S GUIDES for multiple ply connection.

PROJECT INFORMATION:

L229281
Lot 11 Suzanne
Columbia County, FL

OPERATOR INFORMATION:

Chad A. Wall, P.E.
iLevel by Weyerhaeuser:
South Central Division
6001 Jackson Square Suite 600
LaVergne, TN 37086
Phone : 615-793-7788



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TJ-Beam® 6.25 Serial Number: 7005101720
User: 4 6/22/2007 1:38:02 PM
Page 1 Engine Version: 6.25.71

Garage Door Header

2 Pcs of 1 3/4" x 11 7/8" 1.9E Microllam® LVL

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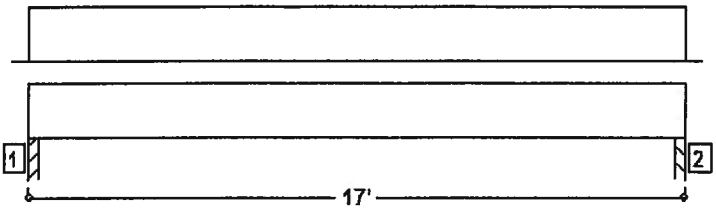
Member Slope: 0/12 Roof Slope0/12



All dimensions are horizontal.

Product Diagram is Conceptual.

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Uniform(plf)	Roof(1.25)	99.0	40.0	0 To 17'	Adds To	EJ3 Loads

SUPPORTS:

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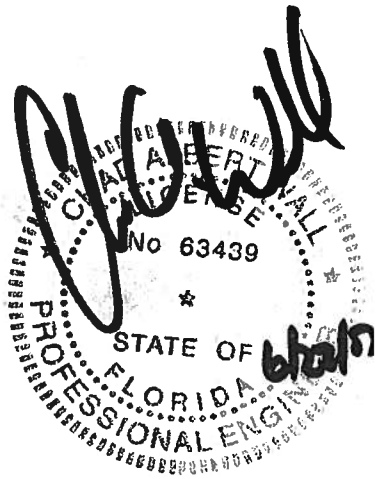

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Trus Joist
A Weyerhaeuser Business

TJ-Beam® 6.25 Serial Number: 7005101720
User: 4 6/22/2007 1:38:02 PM
Page 1 Engine Version: 6.25.71

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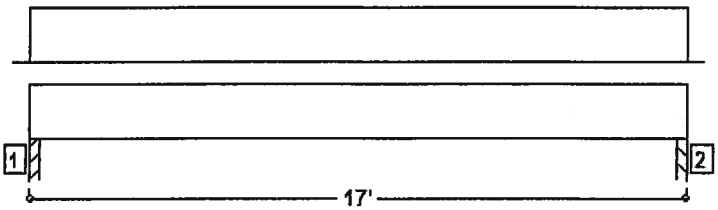

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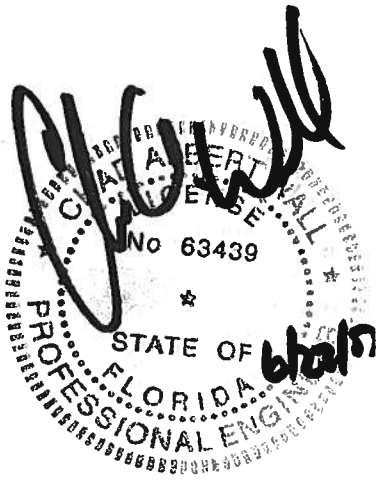

6/20/07

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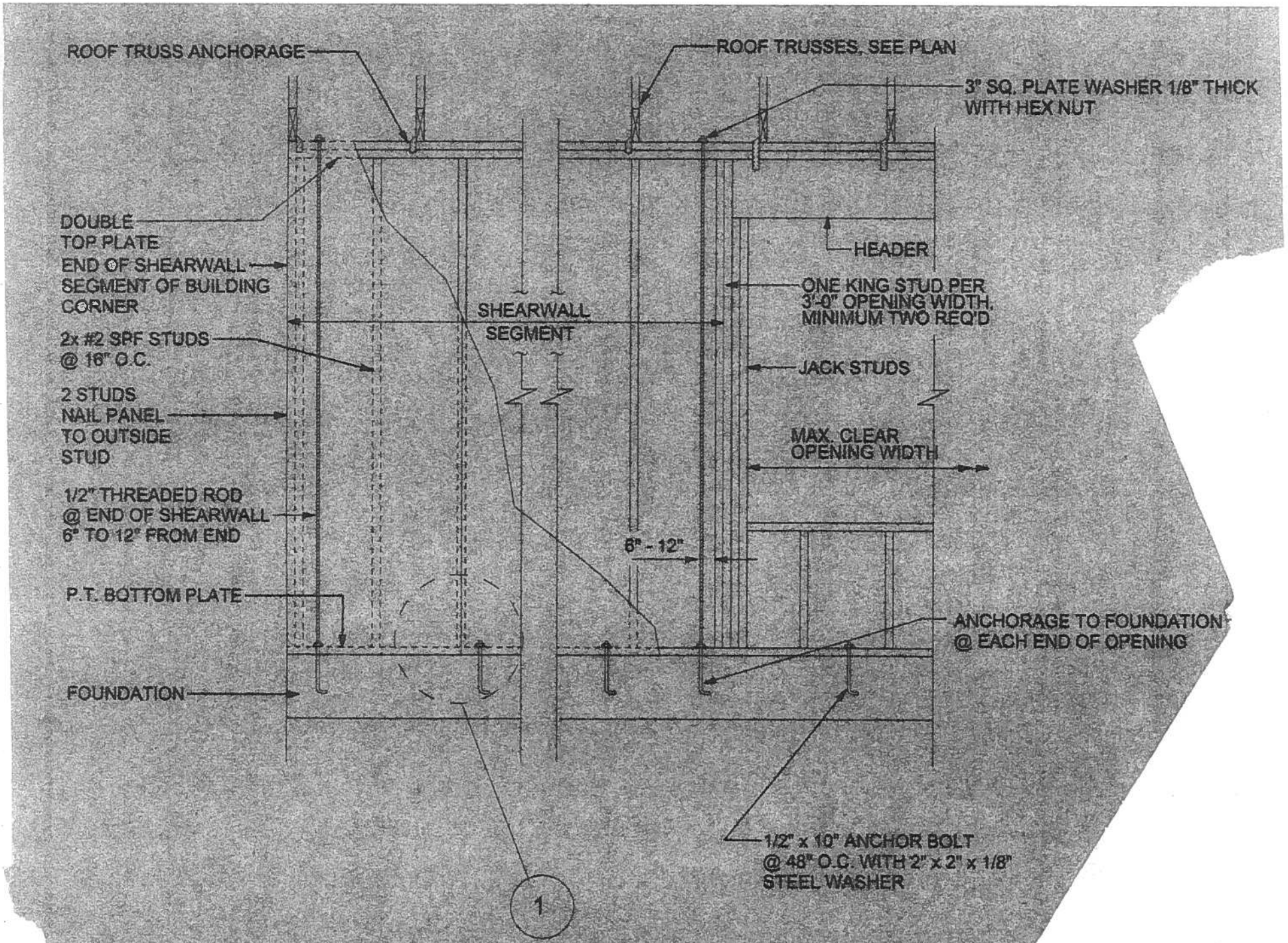
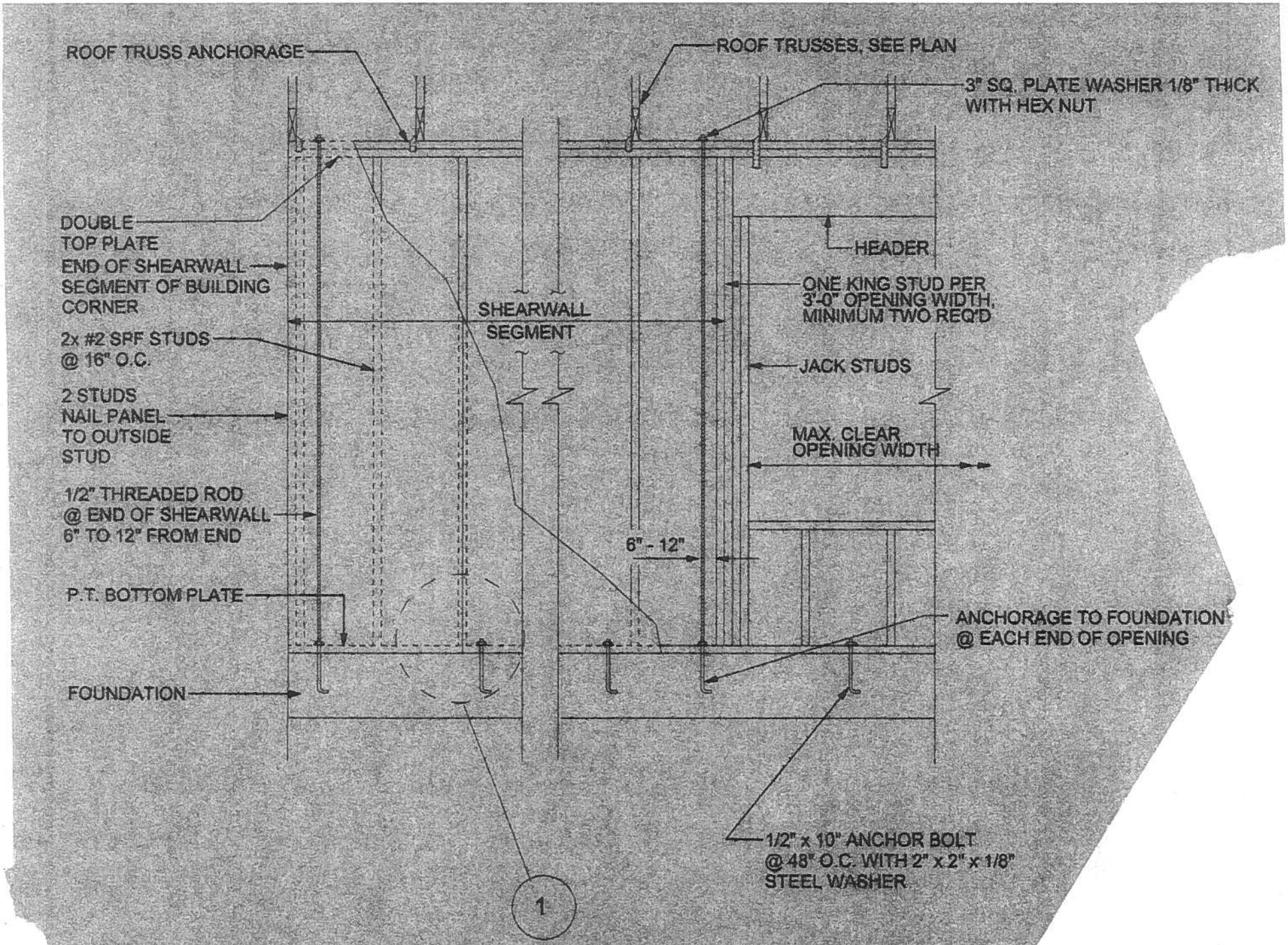


SHEARWALL DETAILS

SCALE: 1/2" = 1'-0"



SHEARWALL DETAILS

SCALE: 1/2" = 1'-0"

Project Information for:	L229281		
Builder:	MIKE ROBERTS (O/B)	Date:	3/7/2007
Lot:	N/A	Start Number:	**1084**
Subdivision:	118 SE WILDERNESS DR	SEI Ref:	L229281
County or City:	COUNTY 32025		
Truss Page Count:	29		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

Owner Builder
Address: N/A
N/A
Designer: **135**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0307071084	3/7/2007				
2	CJ3	0307071085	3/7/2007				
3	CJ5	0307071086	3/7/2007				
4	EJ5	0307071087	3/7/2007				
5	EJ7	0307071088	3/7/2007				
6	EJ7A	0307071089	3/7/2007				
7	EJ7B	0307071090	3/7/2007				
8	HJ7	0307071091	3/7/2007				
9	HJ9	0307071092	3/7/2007				
10	T01	0307071093	3/7/2007				
11	T02	0307071094	3/7/2007				
12	T03	0307071095	3/7/2007				
13	T04	0307071096	3/7/2007				
14	T05	0307071097	3/7/2007				
15	T06	0307071098	3/7/2007				
16	T07	0307071099	3/7/2007				
17	T08	0307071100	3/7/2007				
18	T09	0307071101	3/7/2007				
19	T10	0307071102	3/7/2007				
20	T11	0307071103	3/7/2007				
21	T12	0307071104	3/7/2007				
22	T13	0307071105	3/7/2007				
23	T14	0307071106	3/7/2007				
24	T15	0307071107	3/7/2007				
25	T16	0307071108	3/7/2007				
26	T17	0307071109	3/7/2007				
27	T18	0307071110	3/7/2007				
28	T19	0307071111	3/7/2007				
29	T20	0307071112	3/7/2007				

MAR 07 2007

FILE COPY

LATERAL TOE-NAIL DETAIL

ST-TOENAIL

MITek Industries, Chesterfield, MO

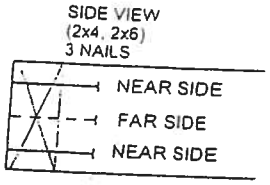
NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

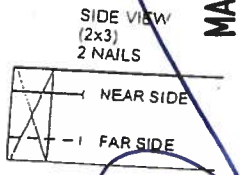
TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

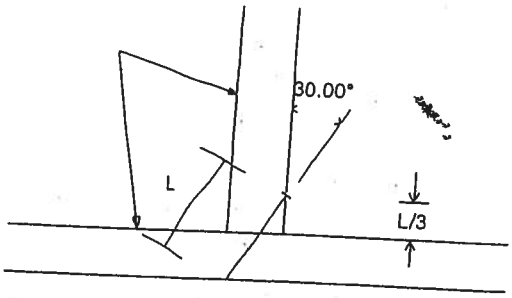
	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

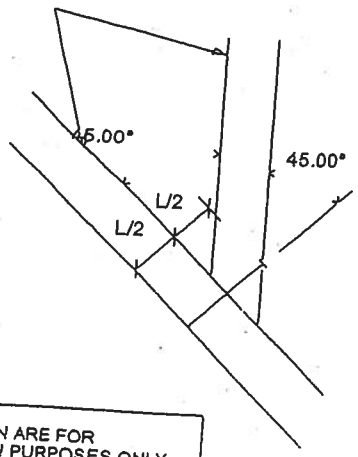
SQUARE CUT

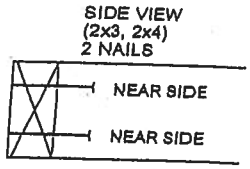


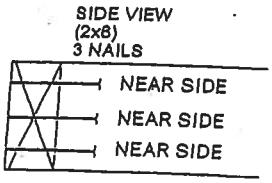




45 DEGREE ANGLE BEVEL CUT







This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

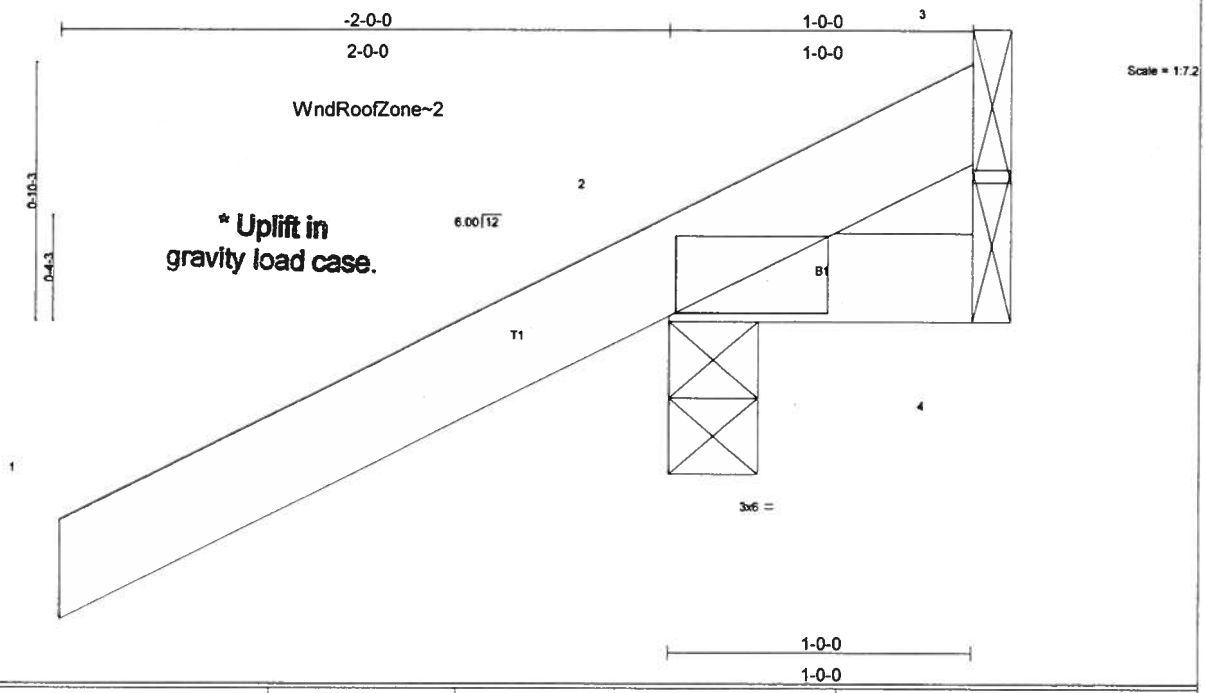
Byron K. Anderson, FL PE 60987 EB #9196

MAR 07 2007

Dwg.#0307071084

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	CJ1	JACK	16	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 10:57:18 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

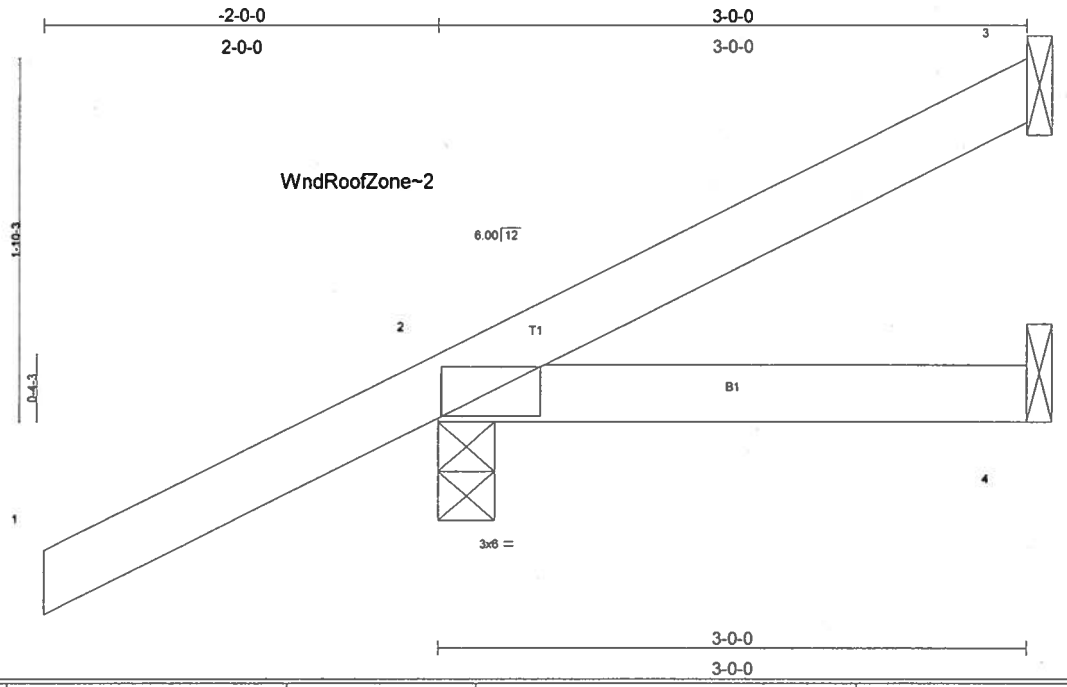
LOAD CASE(S) Standard

MARCH 7.2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071085

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	CJ3	JACK	16	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 10:57:40 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

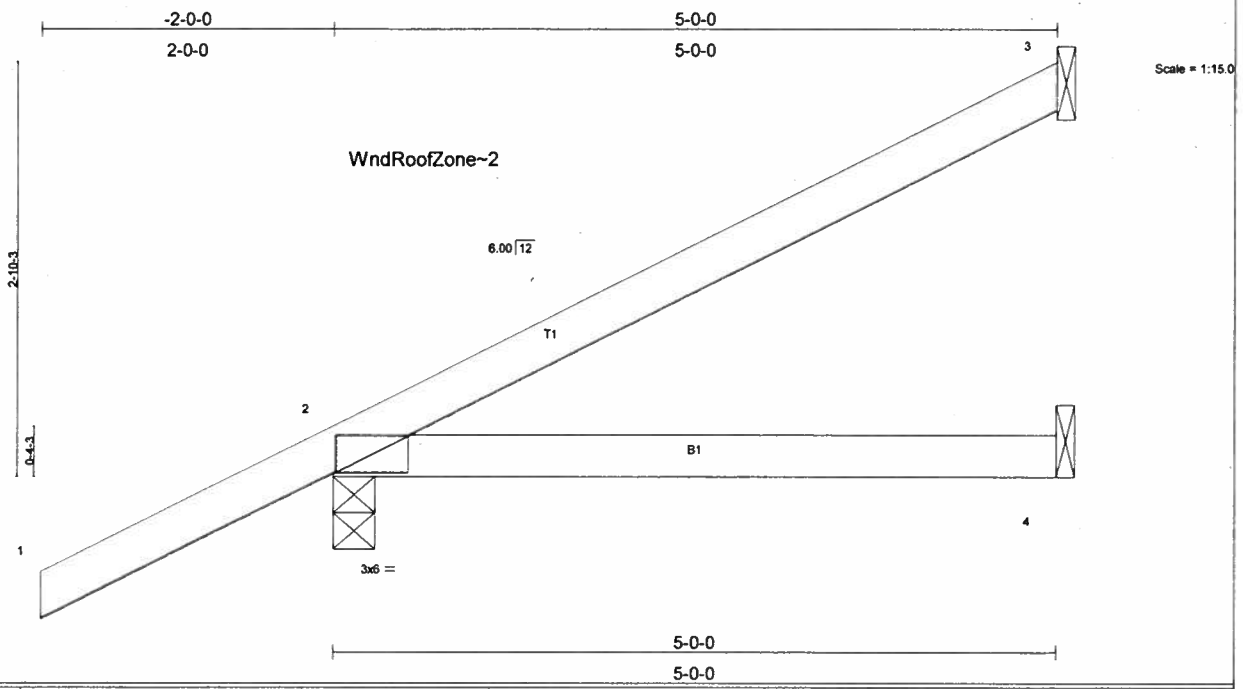
LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E LUTZ, FL 33549

Dwg.#0307071086

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	CJ5	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 10:58:01 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

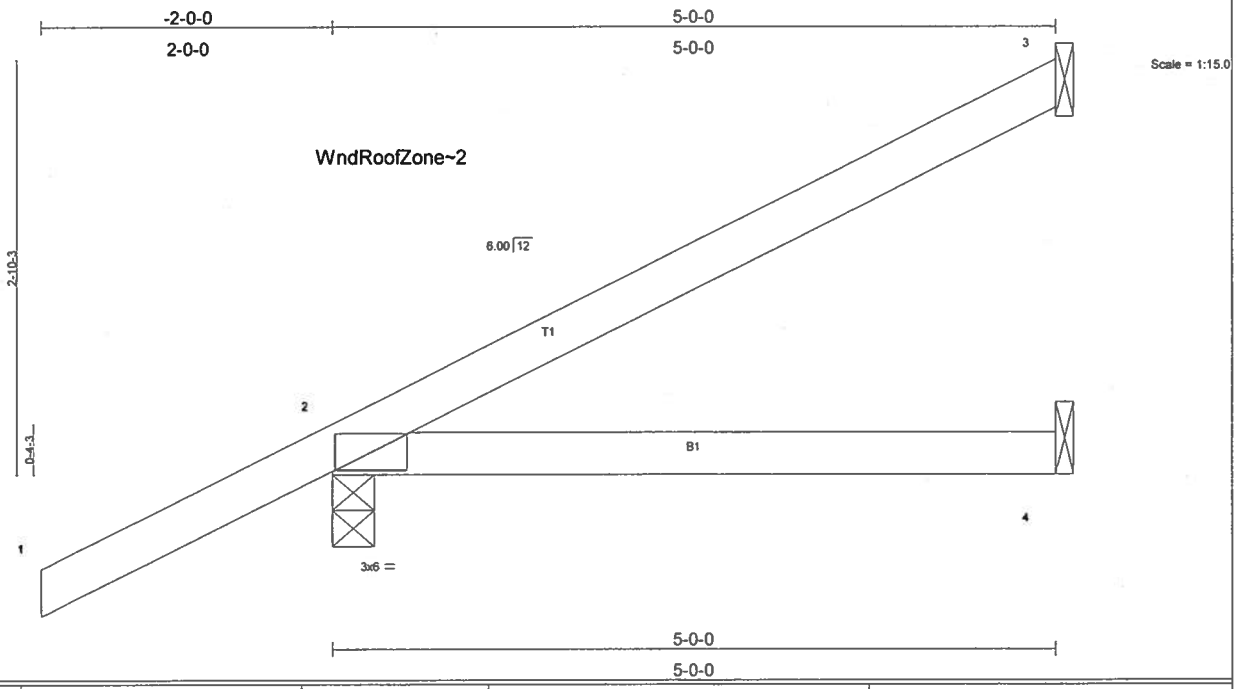
LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071087

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	EJ5	JACK	3	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 10:58:32 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

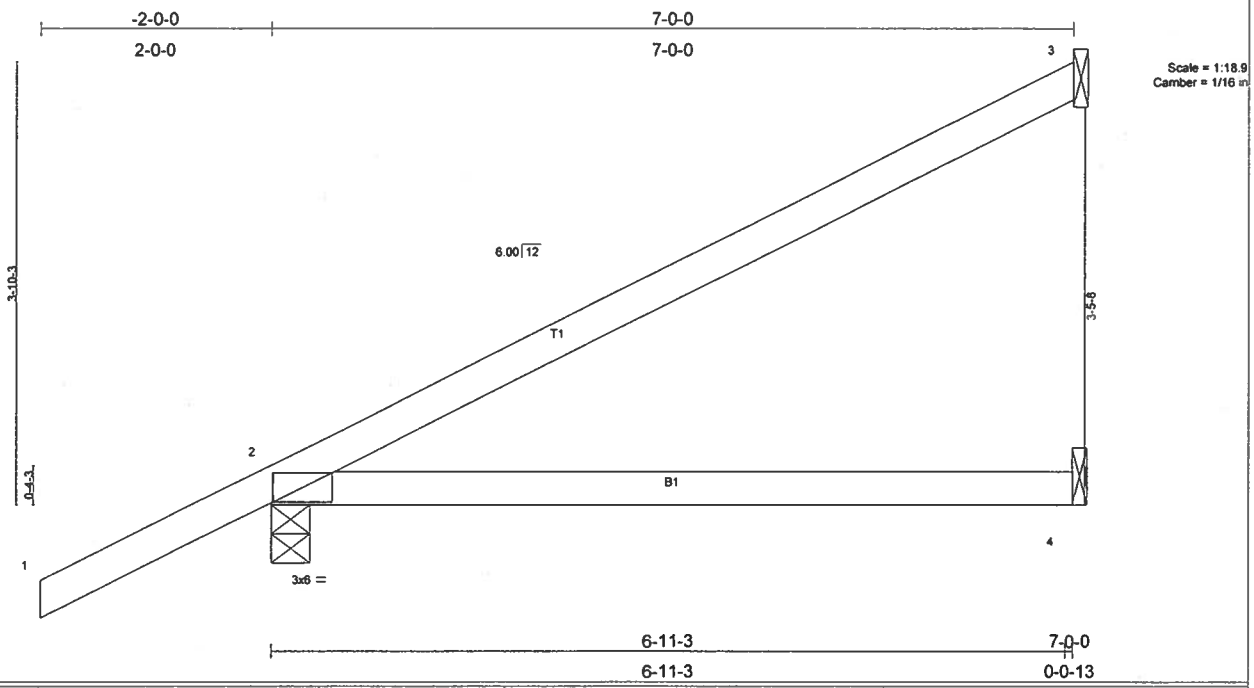
LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0307071088

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	EJ7	JACK	27	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 10:58:59 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.12	2-4	>674	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.20	2-4	>403	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-133(load case 5), 2=-211(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 3 and 211 lb uplift at joint 2.

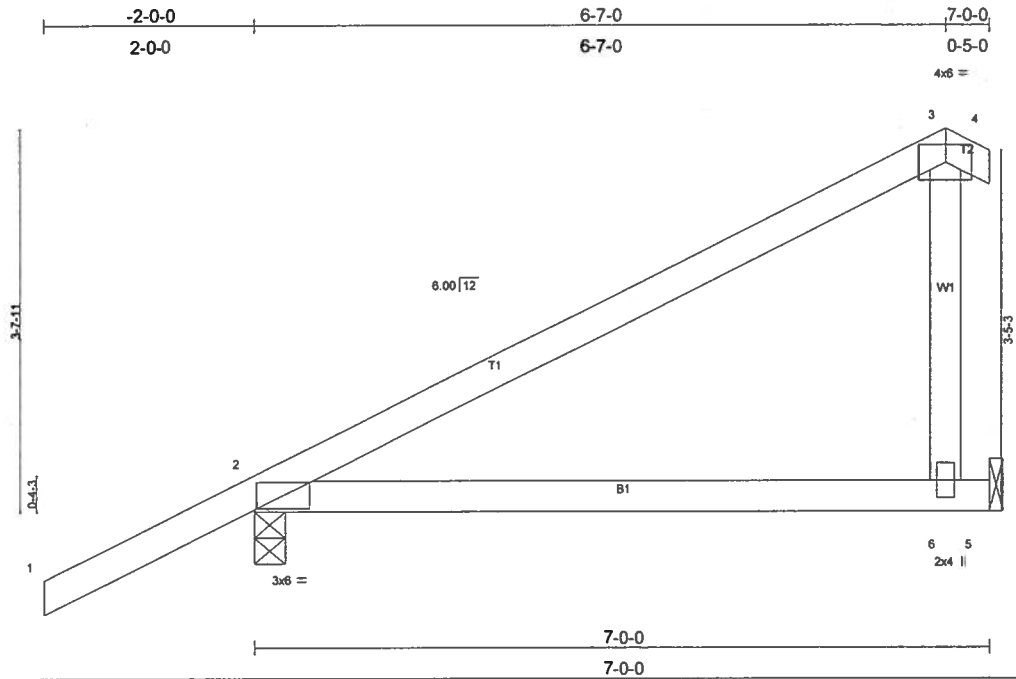
LOAD CASE(S) Standard

MARCH 7.2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E LUTZ, FL 33549

Dwg.#0307071089

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	EJ7A	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 10:59:49 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.14	2-6	>597	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.23	2-6	>360	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=419/0-3-8, 5=270/Mechanical
Max Horz 2=205(load case 5)
Max Uplift2=-219(load case 5), 5=-121(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/56, 3-4=0/10
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=-180/140

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 2 and 121 lb uplift at joint 5.

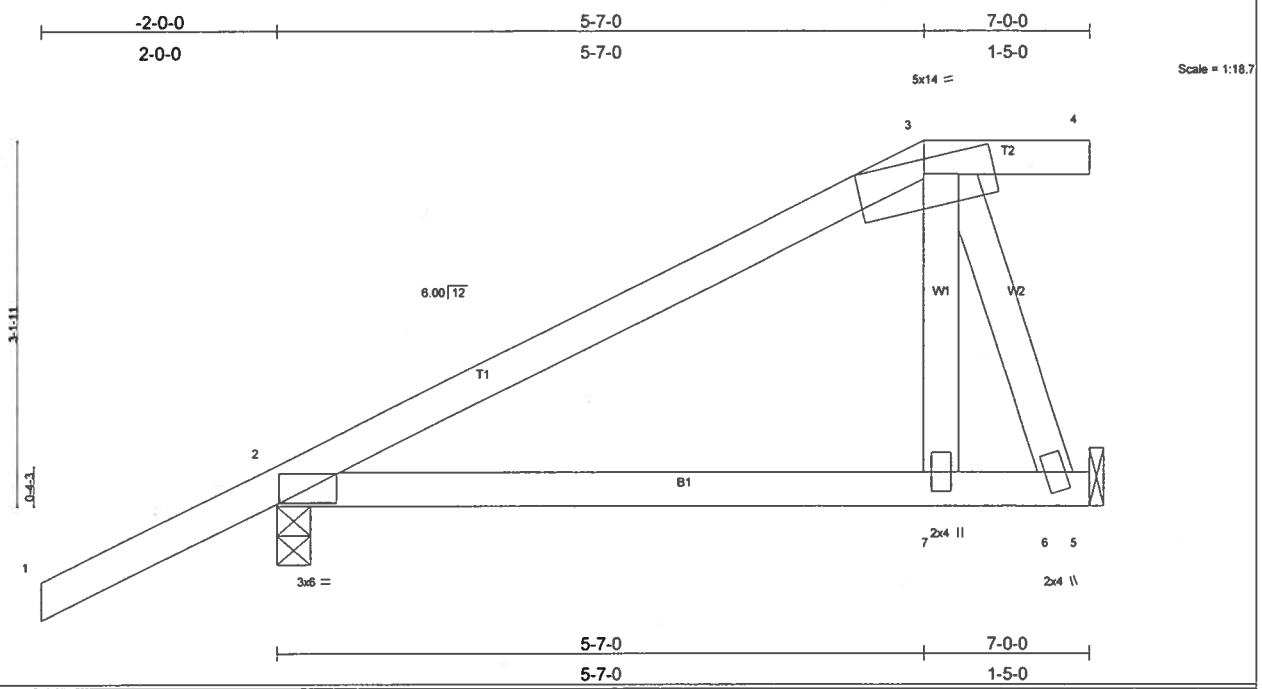
LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071090

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	EJ7B	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:00:34 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.05	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=412/0-3-8, 6=280/Mechanical
Max Horz 2=193(load case 5)
Max Uplift2=-221(load case 5), 6=-123(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-220/15, 3-4=-0/0
BOT CHORD 2-7=-48/130, 6-7=-46/140, 5-6=0/0
WEBS 3-7=0/196, 3-6=-368/120

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2 and 123 lb uplift at joint 6.

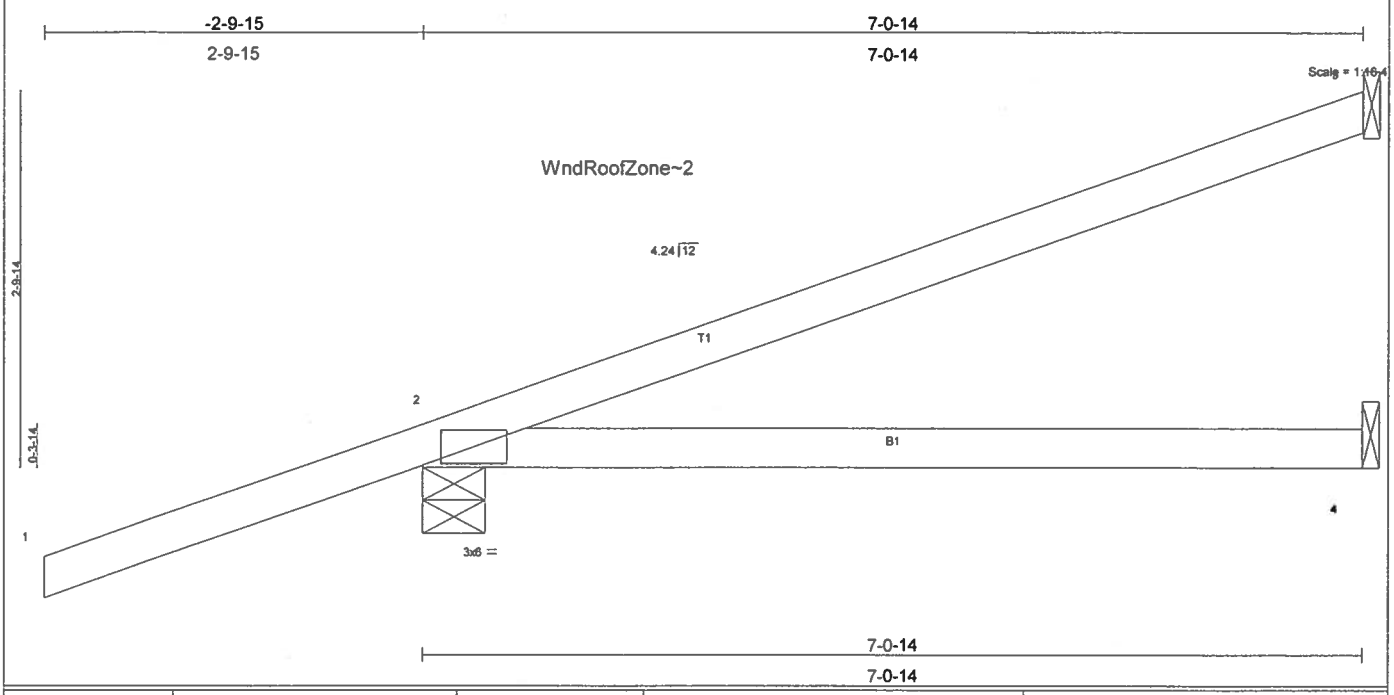
LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0307071091

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	HJ7	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:01:16 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.13	2-4	>627	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=191/Mechanical, 2=378/0-5-11, 4=110/Mechanical
Max Horz 2=167(load case 2)
Max Uplift3=-144(load case 2), 2=-251(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-68/45
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3 and 251 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=0(F=15, B=15)-to-4=-53(F=-12, B=-12)

MARCH 7.2007 TRUSS DESIGN ENGINEER:
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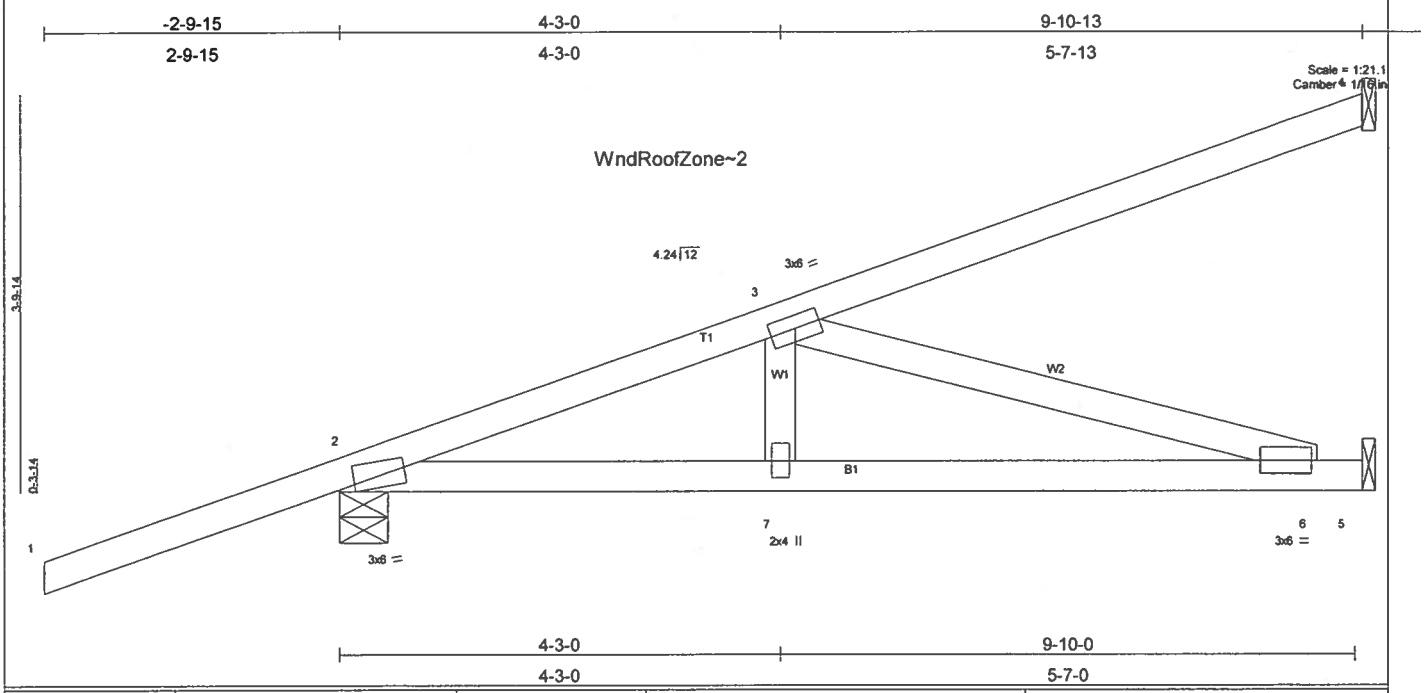
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071092

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	HJ9	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:02:28 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.19	6-7	>623	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-9 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-233(load case 2), 2=-401(load case 2), 5=-181(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-883/363, 3-4=-105/66
BOT CHORD 2-7=-534/816, 6-7=-534/816, 5-6=0/0
WEBS 3-7=-94/188, 3-6=-851/557

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 401 lb uplift at joint 2 and 181 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

MARCH 7.2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0307071093

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T01	HIP	1	2	Job Reference (optional)

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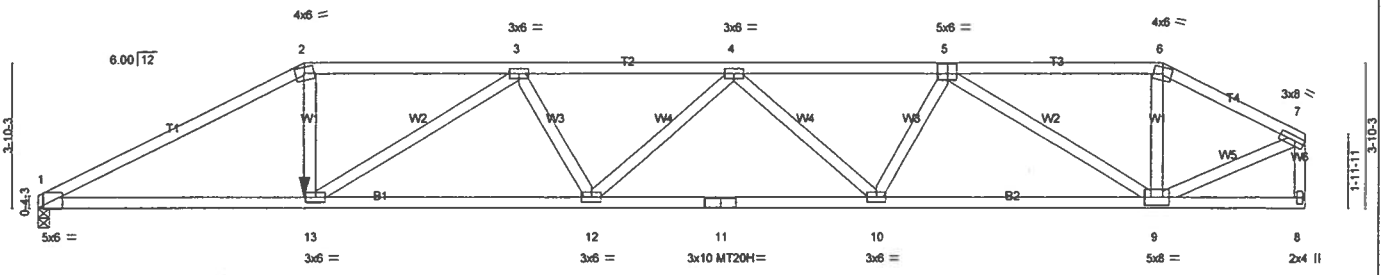
7-0-0	12-8-2	18-4-4	24-0-6	29-8-8	33-5-8
7-0-0	5-8-2	5-8-2	5-8-2	5-8-2	3-9-0

Scale = 1:57.6
Camber = 3/16 in



7-0-0	14-6-13	22-1-11	29-8-8	33-5-8
7-0-0	7-6-13	7-6-13	7-6-13	3-9-0

Plate Offsets (X,Y): [1:0-1-10,Edge], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.34	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.55	10-12	>725	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.61	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 328 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-5 oc bracing.

REACTIONS (lb/size) 1=2848/0-3-8, 8=3096/Mechanical
Max Horz 1=106(load case 4)
Max Uplift1=-1156(load case 3), 8=-1272(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5786/2454, 2-3=-5161/2258, 3-4=-7222/3149, 4-5=-6505/2817, 5-6=-3018/1305, 6-7=-3351/1404, 7-8=-2881/1185
BOT CHORD 1-13=-2201/5076, 12-13=-3118/6959, 11-12=-3240/7216, 10-11=-3240/7216, 9-10=-2623/5886, 8-9=-51/86
WEBS 2-13=-801/2108, 3-13=-2244/1111, 3-12=-30/589, 4-12=-22/182, 4-10=-986/616, 5-10=-358/1324, 5-9=-3433/1632, 6-9=-315/1061, 7-9=-1340/3184

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1156 lb uplift at joint 1 and 1272 lb uplift at joint 8.
9) Girder carries tie-in span(s): 7-0-0 from 29-8-8 to 33-5-8
10) Girder carries hip end with 3-9-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-6=-118(F=-64), 6-7=-54, 1-13=-30, 9-13=-65(F=-35), 8-9=-132(F=-102)
Concentrated Loads (lb)
Vert: 13=-539(F)

MARCH 7.2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
1616 N. FLORIDA AVE. STE E. LUTZ, FL 33549

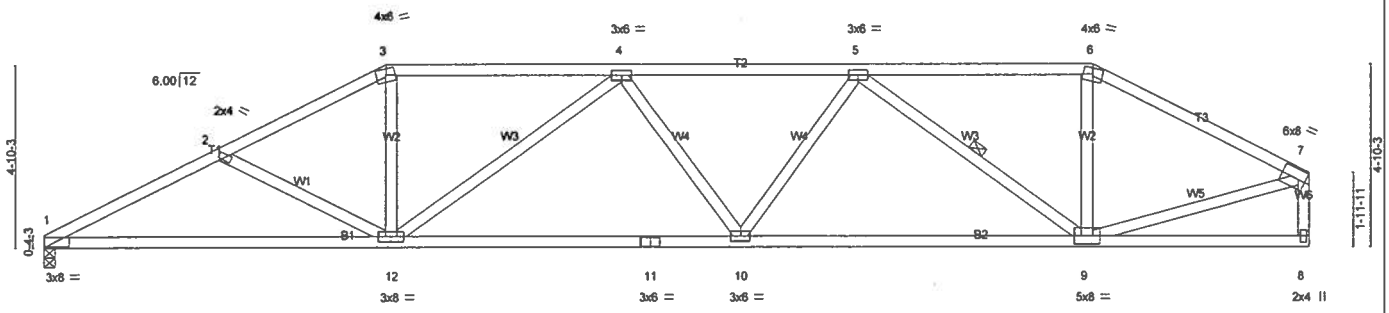
Dwg.#0307071094

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:18:27 2007 Page 1

4-9-4	9-0-0	15-2-13	21-5-11	27-8-8	33-5-8
4-9-4	4-2-12	6-2-13	6-2-13	6-2-13	5-9-0

Scale = 1:57.6
Camber = 3/16 in



9-0-0	18-4-4	27-8-8	33-5-8
9-0-0	9-4-4	9-4-4	5-9-0

Plate Offsets (X,Y): [1:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.26	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.44	9-10	>908	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-5 oc bracing.
WEBS 1 Row at midpt 5-9

REACTIONS (lb/size) 1=1393/0-3-8, 8=1393/Mechanical
Max Horz 1=119(load case 5)
Max Uplift1=-395(load case 5), 8=-393(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2589/773, 2-3=-2352/718, 3-4=-2082/677, 4-5=-2536/838, 5-6=-1521/511, 6-7=-1750/532, 7-8=-1322/409
BOT CHORD 1-12=-721/2268, 11-12=-853/2537, 10-11=-853/2537, 9-10=-785/2344, 8-9=-70/107
WEBS 2-12=-237/229, 3-12=-149/739, 4-12=-658/327, 4-10=-21/97, 5-10=-28/360, 5-9=-1078/443, 6-9=-86/480, 7-9=-445/1462

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 395 lb uplift at joint 1 and 393 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ FL 33549

Dwg.#0307071095

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T03	HIP	1	1	Job Reference (optional)

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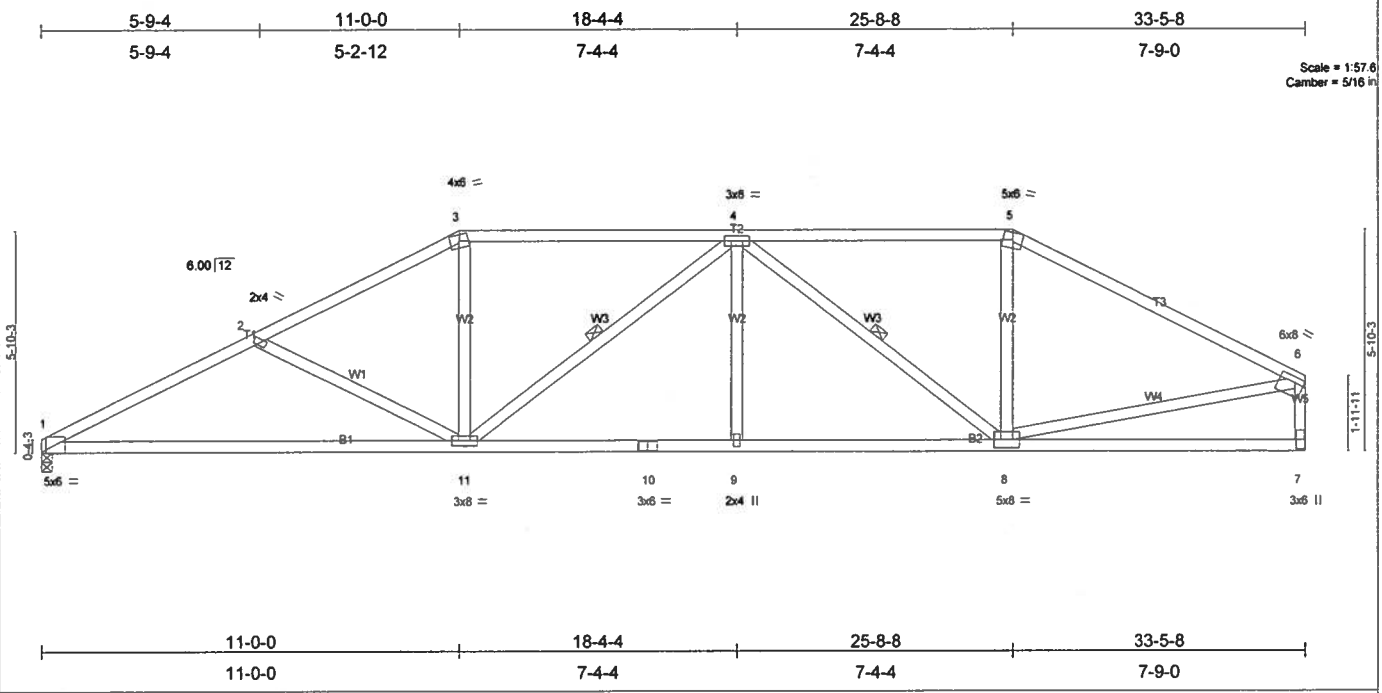

Plate Offsets (X,Y): [1:0-1-11,Edge], [6:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.41	1-11	>964	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.70	1-11	>569	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 175 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-2-0 oc bracing.
WEBS 1 Row at midpt 4-11, 4-8

REACTIONS (lb/size) 1=1393/0-3-8, 7=1393/Mechanical
Max Horz 1=133(load case 5)
Max Uplift1=-413(load case 5), 7=-383(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2519/772, 2-3=-2198/659, 3-4=-1924/635, 4-5=-1557/541, 5-6=-1822/541, 6-7=-1274/411
BOT CHORD 1-11=-746/2210, 10-11=-632/2117, 9-10=-632/2117, 8-9=-632/2117, 7-8=-126/232
WEBS 2-11=-344/291, 3-11=-95/616, 4-11=-375/210, 4-9=0/182, 4-8=-794/305, 5-8=-50/438, 6-8=-401/1349

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 413 lb uplift at joint 1 and 383 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
18105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0307071096

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T04	SPECIAL	1	1	Job Reference (optional)

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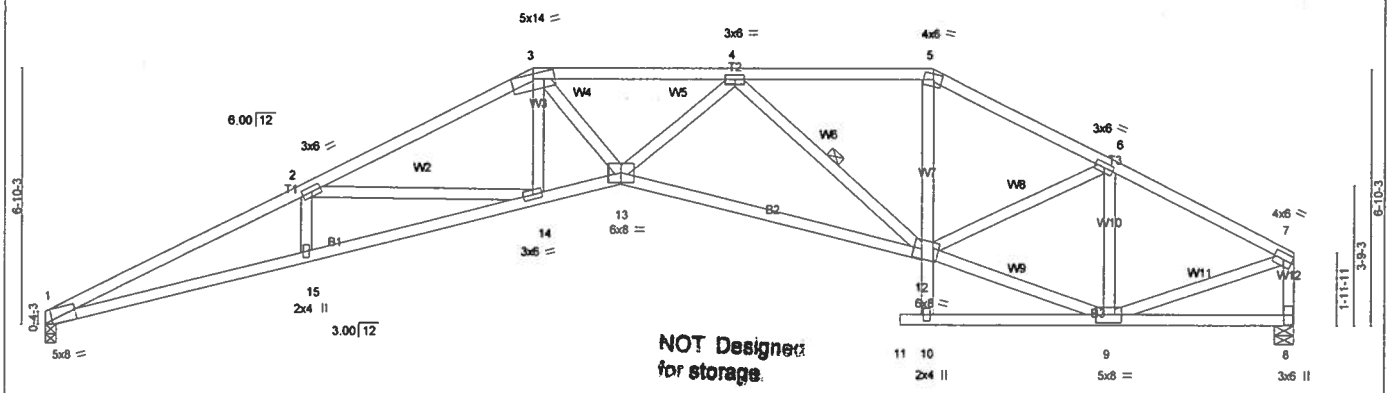
6-10-15 | 13-0-0 | 18-4-4 | 23-8-8 | 28-5-4 | 33-5-8
6-10-15 | 6-1-1 | 5-4-4 | 5-4-4 | 4-8-12 | 5-0-4

Scale = 1:58.3
Camber = 5/16 in



6-10-15 | 13-0-0 | 15-4-4 | 22-10-0 | 28-5-4 | 33-5-8
6-10-15 | 6-1-1 | 2-4-4 | 7-5-12 | 5-7-4 | 5-0-4

Plate Offsets (X,Y): [1:0-2-7,Edge], [12:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	-0.54	12-13	>740	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.88	12-13	>453	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.50	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 182 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-8 oc bracing.
WEBS 1 Row at midpt 4-12

REACTIONS (lb/size) 1=1398/0-3-8, 8=1405/0-6-8
Max Horz 1=149(load case 5)
Max Uplift1=-427(load case 5), 8=-399(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4845/1498, 2-3=-3789/1083, 3-4=-4184/1161, 4-5=-2063/663, 5-6=-2305/690, 6-7=-1666/474, 7-8=-1328/407
BOT CHORD 1-15=-1438/4386, 14-15=-1436/4379, 13-14=-899/3458, 12-13=-852/3385, 10-11=0/0, 9-10=-3/23, 8-9=-49/108
WEBS 2-15=0/217, 2-14=-920/520, 3-14=-148/344, 3-13=-268/1310, 4-13=-307/1272, 4-12=-1639/528, 9-12=-388/1495, 6-9=-814/285, 10-12=0/87, 5-12=-151/719, 6-12=-203/659, 7-9=-351/1409

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 1 and 399 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

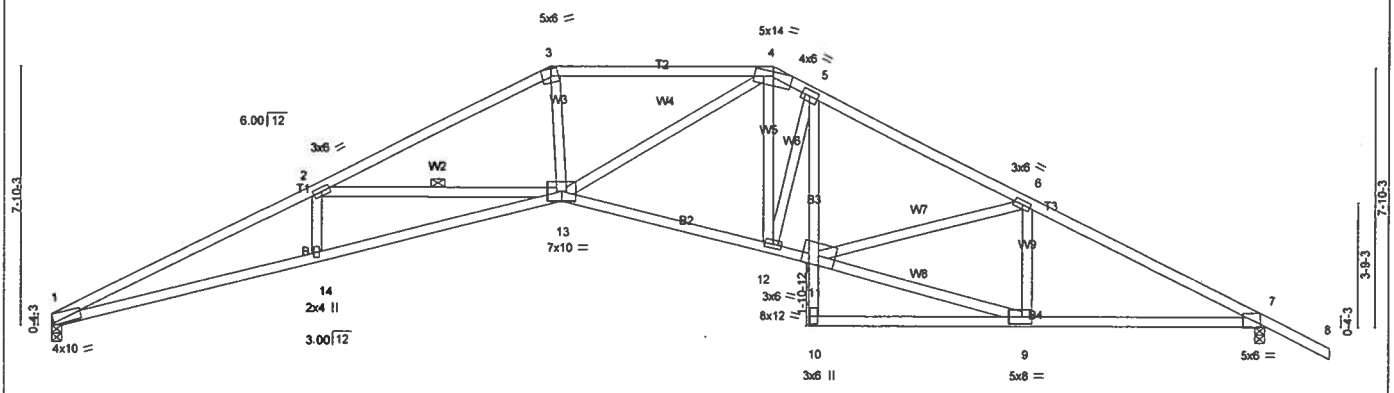
Dwg.#0307071097

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:29:51 2007 Page 1

7-10-15	15-0-0	21-8-8	22-10-0	29-6-0	36-8-8	38-8-8
7-10-15	7-1-1	6-8-8	1-1-8	6-8-0	7-2-8	2-0-0

Scale = 1:66.1
Camber = 3/8 in



7-10-15	15-4-4	21-8-8	22-10-0	29-6-0	36-8-8
7-10-15	7-5-5	6-4-4	1-1-8	6-8-0	7-2-8

Plate Offsets (X,Y): [1:0-0-14,Edge], [7:0-1-11,Edge], [9:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.56	13-14	>782	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.90	13-14	>486	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.52	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 197 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-4-7 oc bracing.
WEBS 1 Row at midpt 2-13

REACTIONS (lb/size) 1=1526/0-3-8, 7=1649/0-3-8
Max Horz 1=-167(load case 6)
Max Uplift1=-476(load case 5), 7=-602(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5308/1595, 2-3=-4010/1073, 3-4=-3637/1047, 4-5=-2820/819, 5-6=-2956/810, 6-7=-2846/822, 7-8=0/47
BOT CHORD 1-14=-1450/4811, 13-14=-1446/4804, 12-13=-491/2644, 11-12=-522/2679, 10-11=0/98, 5-11=-97/49, 9-10=-9/138, 7-9=-572/2459
WEBS 2-14=0/257, 2-13=-1148/642, 3-13=-260/1387, 4-13=-358/1274, 4-12=-250/358, 5-12=-98/243, 9-11=-586/2414, 6-11=-87/238, 6-9=-445/205

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 1 and 602 lb uplift at joint 7.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071098

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:30:27 2007 Page 1

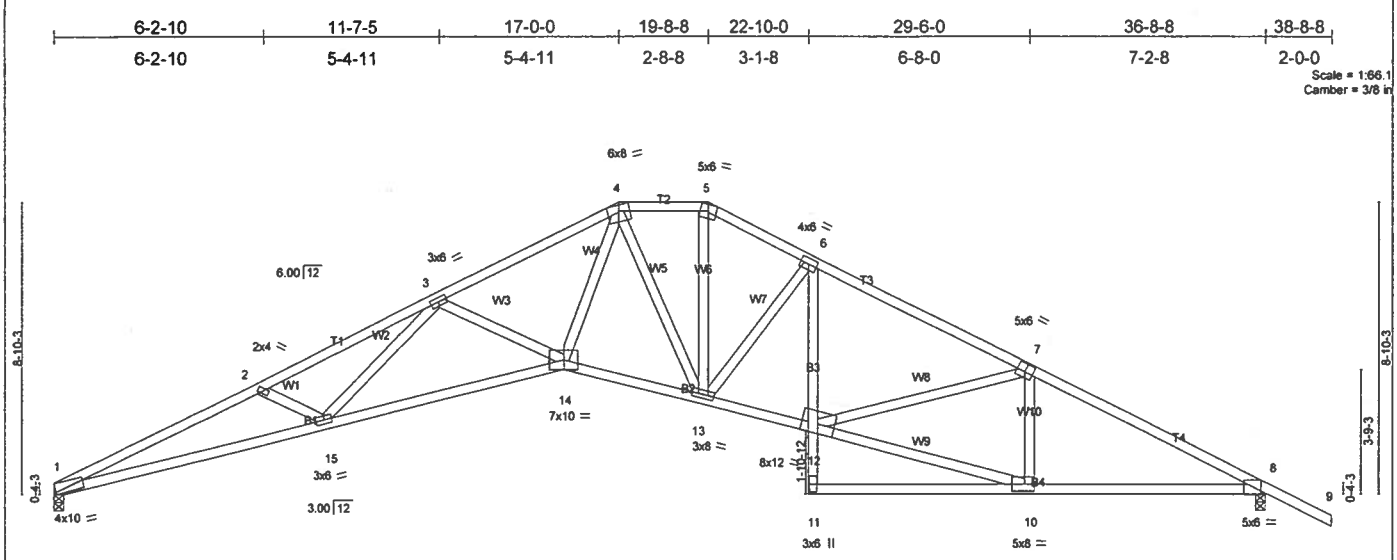

Plate Offsets (X,Y): [1:0-0-14,Edge], [7:0-3-0,0-3-0], [8:0-1-11,Edge], [10:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.58	14-15	>752	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.93	14-15	>467	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.52	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-9 oc bracing.

REACTIONS (lb/size) 1=1526/0-3-8, 8=1649/0-3-8
Max Horz 1=-181(load case 6)
Max Uplift1=-488(load case 5), 8=-614(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5313/1730, 2-3=-5054/1597, 3-4=-3887/1138, 4-5=-2382/760, 5-6=-2683/825, 6-7=-2965/850, 7-8=-2844/847, 8-9=0/47
BOT CHORD 1-15=-1604/4835, 14-15=-1176/4133, 13-14=-584/2771, 12-13=-567/2675, 11-12=0/96, 6-12=-73/169, 10-11=-8/110, 8-10=-593/2456
WEBS 2-15=-242/273, 3-15=-260/736, 3-14=-662/419, 4-14=-623/2271, 4-13=-764/260, 5-13=-338/1054, 6-13=-396/298, 10-12=-615/2450, 7-12=-48/206, 7-10=-453/212

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 488 lb uplift at joint 1 and 614 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE STE B, LUTZ, FL 33549

Dwg.#0307071099

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T07	SPECIAL	3	1	Job Reference (optional)

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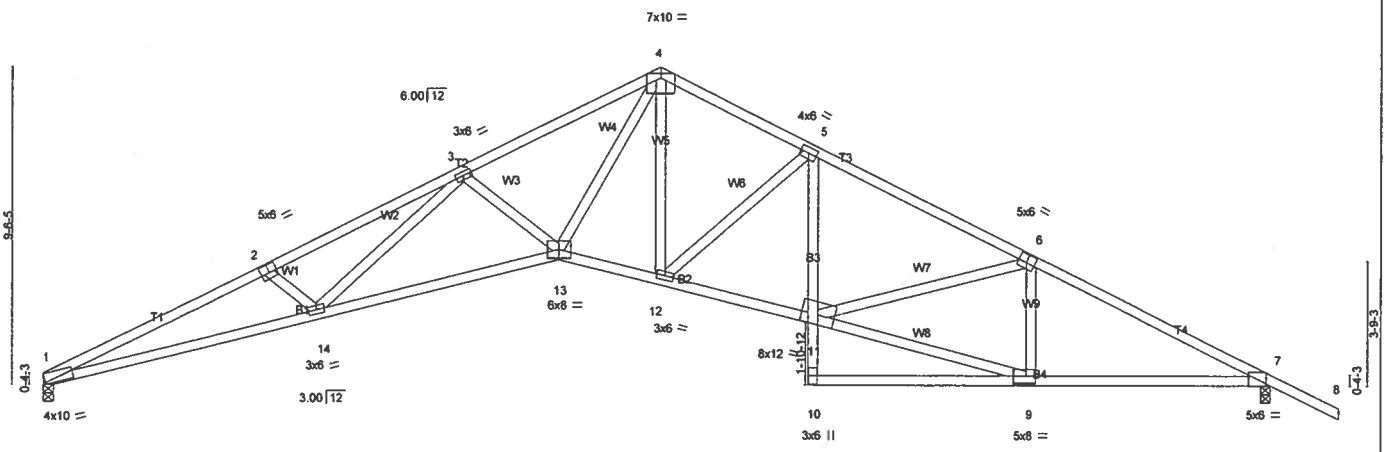
6-8-1	12-6-2	18-4-4	22-10-0	29-6-0	36-8-8	38-8-8
6-8-1	5-10-2	5-10-2	4-5-12	6-8-0	7-2-8	2-0-0

Scale = 1:65.2
Camber = 3/8 in



8-1-1	15-4-4	18-4-4	22-10-0	29-6-0	36-8-8
8-1-1	7-3-3	3-0-0	4-5-12	6-8-0	7-2-8

Plate Offsets (X,Y): [1:0-0-14,Edge], [2:0-3-0,0-3-0], [6:0-3-0,0-3-0], [7:0-1-11,Edge], [9:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	-0.59	13-14	>736	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.95	13-14	>458	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.52	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-12 oc bracing.

REACTIONS (lb/size) 1=1526/0-3-8, 7=1649/0-3-8
Max Horz 1=-190(load case 6)
Max Uplift1=-496(load case 5), 7=-622(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5307/1748, 2-3=-5083/1680, 3-4=-3883/1202, 4-5=-2590/841, 5-6=-2971/882, 6-7=-2844/863, 7-8=0/47
BOT CHORD 1-14=-1625/4830, 13-14=-1153/4000, 12-13=-480/2349, 11-12=-597/2687, 10-11=0/96, 5-11=-67/209, 9-10=-9/95, 7-9=-608/2456
WEBS 2-14=-248/265, 3-14=-367/906, 3-13=-615/406, 4-13=-713/2364, 4-12=-260/540, 5-12=-496/323, 9-11=-627/2466, 6-11=-22/188, 6-9=-456/216

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 496 lb uplift at joint 1 and 622 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS. INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ. FL 33549

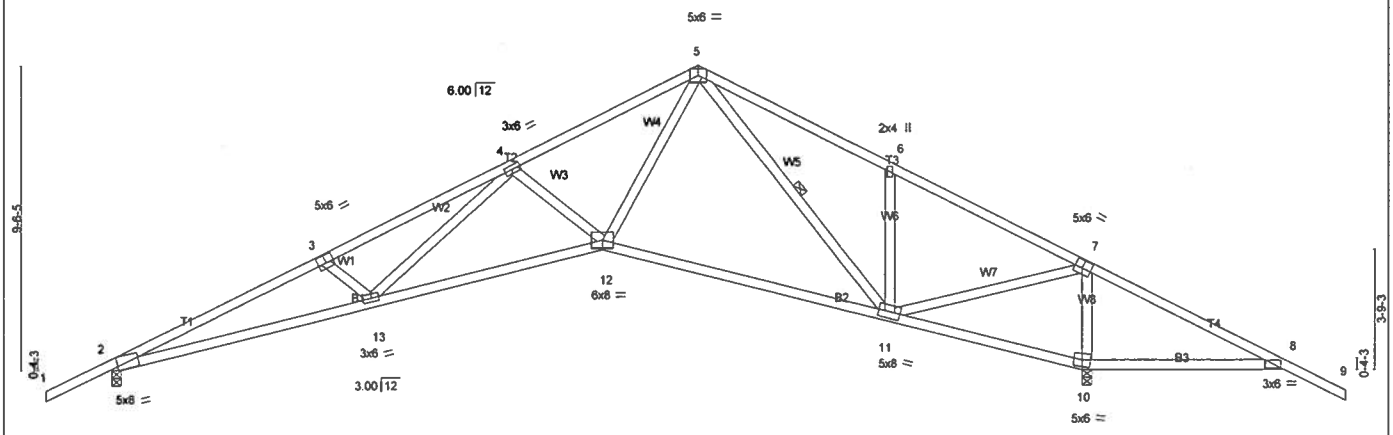
Dwg.#0307071100

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T08	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:32:49 2007 Page 1

-2-0-0	6-8-1	12-6-2	18-4-4	24-4-10	30-5-0	36-8-8	38-8-8
2-0-0	6-8-1	5-10-2	5-10-2	6-0-6	6-0-6	6-3-8	2-0-0

Scale = 1:68.2
Camber = 1/4 in



8-1-1	15-4-4	24-4-10	30-5-0	36-8-8
8-1-1	7-3-3	9-0-6	6-0-6	6-3-8

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-3-0,0-3-0], [7:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.35	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.57	11-12	>637	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.33	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-13 oc bracing.
WEBS 1 Row at midpt 5-11

REACTIONS (lb/size) 2=1310/0-3-8, 10=1985/0-3-8, 10=1985/0-3-8
Max Horz 2=-167(load case 6)
Max Uplift2=-532(load case 5), 10=-917(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3878/1300, 3-4=-3630/1234, 4-5=-2421/784, 5-6=-1134/464, 6-7=-1140/360, 7-8=-769/830, 8-9=0/47
BOT CHORD 2-13=-1207/3505, 12-13=-764/2657, 11-12=-211/1326, 10-11=-739/855, 8-10=-666/807
WEBS 3-13=-258/258, 4-13=-338/884, 4-12=-612/398, 5-12=-505/1800, 5-11=-492/183, 6-11=-330/298, 7-11=-525/1706, 7-10=-1615/681

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 2 and 917 lb uplift at joint 10.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16[illegible] FLORIDA AVE. STE E. LUTZ. FL 33549

Dwg.#0307071101

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T09	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:38:34 2007 Page 1

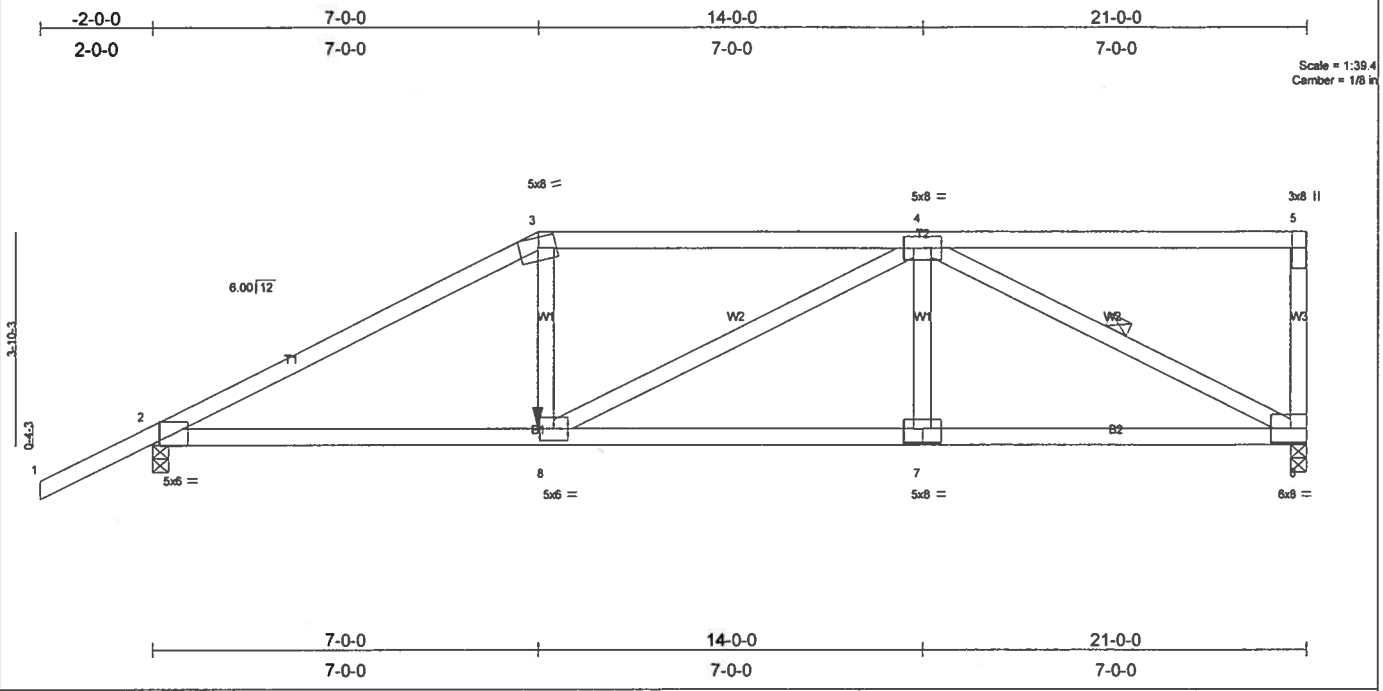


Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.22	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.36	7-8	>689	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.89	Horz(TL)	0.09	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 104 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-12 oc bracing.
WEBS 1 Row at midpt 4-6

REACTIONS (lb/size) 6=1956/0-3-8, 2=1803/0-3-8
Max Horz 2=227(load case 4)
Max Uplift6=-878(load case 3), 2=-798(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3235/1337, 3-4=-2847/1246, 4-5=-167/73, 5-6=-380/297
BOT CHORD 2-8=-1238/2812, 7-8=-1249/2773, 6-7=-1249/2773
WEBS 3-8=-233/843, 4-8=-104/83, 4-7=0/457, 4-6=-2936/1324

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 878 lb uplift at joint 6 and 798 lb uplift at joint 2.
4) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-118(F=-64), 2-8=-30, 6-8=-65(F=-35)
Concentrated Loads (lb)
Vert: 8=-539(F)

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E, LUTZ, FL 33549

Dwg.#0307071102

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T10	MONO HIP	1	1	Job Reference (optional)

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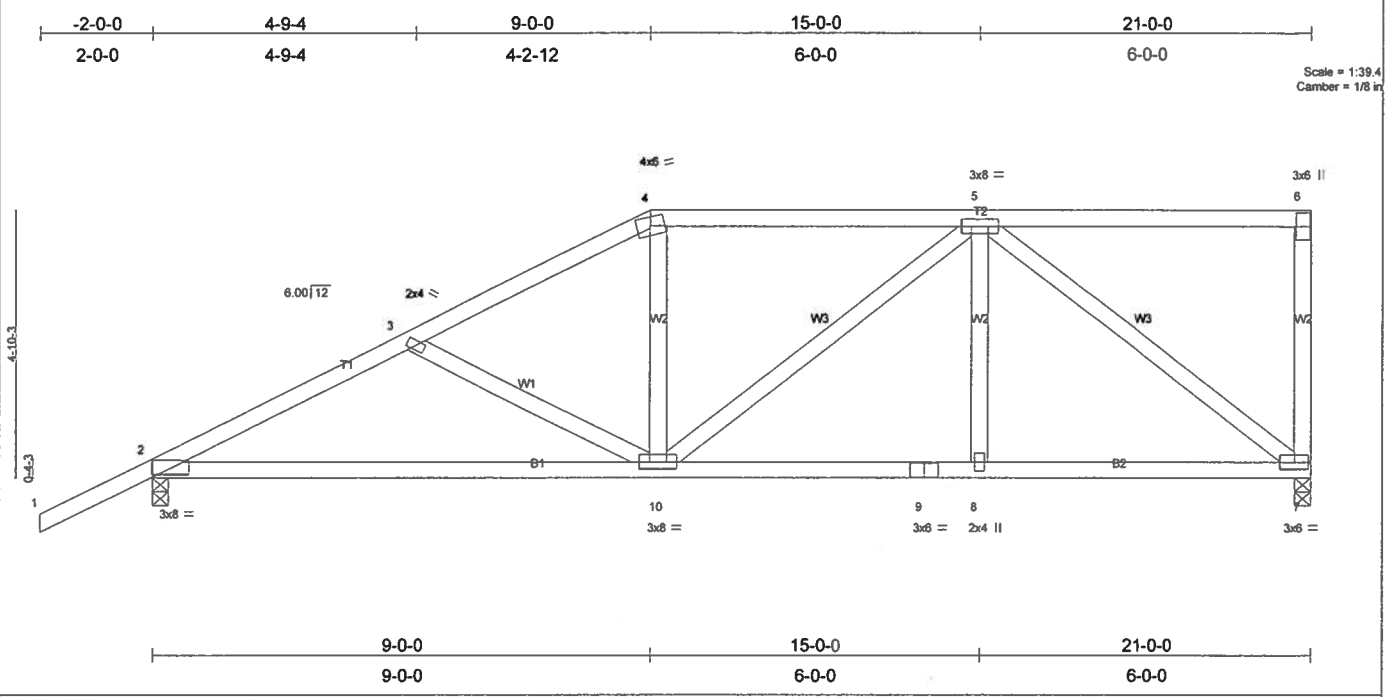

Plate Offsets (X,Y): [2:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.15	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.27	2-10	>933	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-10-14 oc bracing.

REACTIONS (lb/size) 7=864/0-3-8, 2=992/0-3-8
Max Horz 2=272(load case 5)
Max Uplift7=-307(load case 4), 2=-386(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1427/406, 3-4=-1179/324, 4-5=-1017/318, 5-6=-40/13, 6-7=-145/98
BOT CHORD 2-10=-497/1234, 9-10=-299/845, 8-9=-299/845, 7-8=-299/845
WEBS 3-10=-253/201, 4-10=0/227, 5-10=-105/217, 5-8=0/153, 5-7=-1021/361

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 7 and 386 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E, LUTZ, FL 33549

Dwg.#0307071103

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T11	MONO HIP	1	1	Job Reference (optional)

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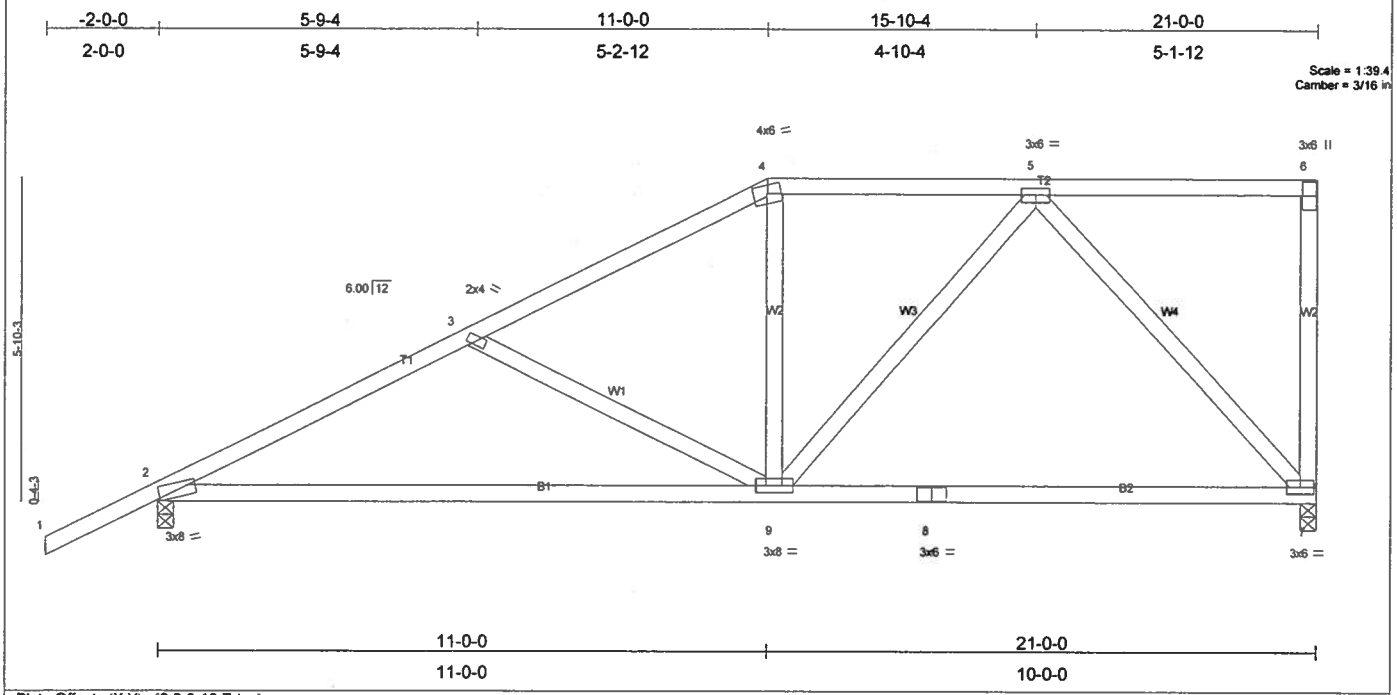


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.28	2-9	>894	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.48	2-9	>517	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS (lb/size) 7=864/0-3-8, 2=992/0-3-8
Max Horz 2=318(load case 5)
Max Uplift7=-297(load case 4), 2=-389(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1374/408, 3-4=-1040/265, 4-5=-871/288, 5-6=-41/2, 6-7=-131/82
BOT CHORD 2-9=-537/1189, 8-9=-217/586, 7-8=-217/586
WEBS 3-9=-363/278, 4-9=0/171, 5-9=-140/440, 5-7=-815/321

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 7 and 389 lb uplift at joint 2.

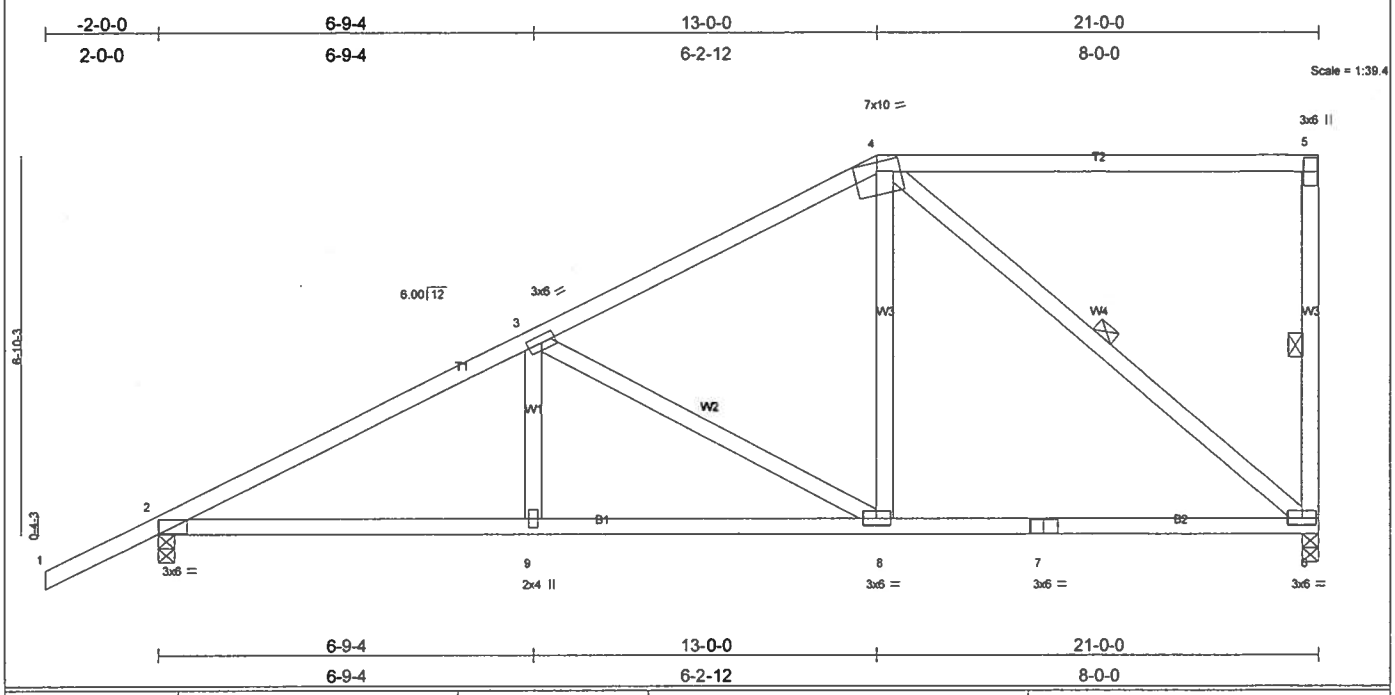
LOAD CASE(S) Standard

MARCH 7.2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0307071104

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T12	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:39:15 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.09	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.14	2-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 116 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-7-8 oc bracing.
WEBS 1 Row at midpt 5-6, 4-6

REACTIONS (lb/size) 6=864/0-3-8, 2=992/0-3-8
Max Horz 2=364(load case 5)
Max Uplift6=-284(load case 4), 2=-388(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1432/363, 3-4=-841/231, 4-5=-48/20, 5-6=-203/144
BOT CHORD 2-9=-533/1203, 8-9=-533/1203, 7-8=-269/701, 6-7=-269/701
WEBS 3-9=0/208, 3-8=-583/302, 4-8=-94/489, 4-6=-850/339

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 6 and 388 lb uplift at joint 2.

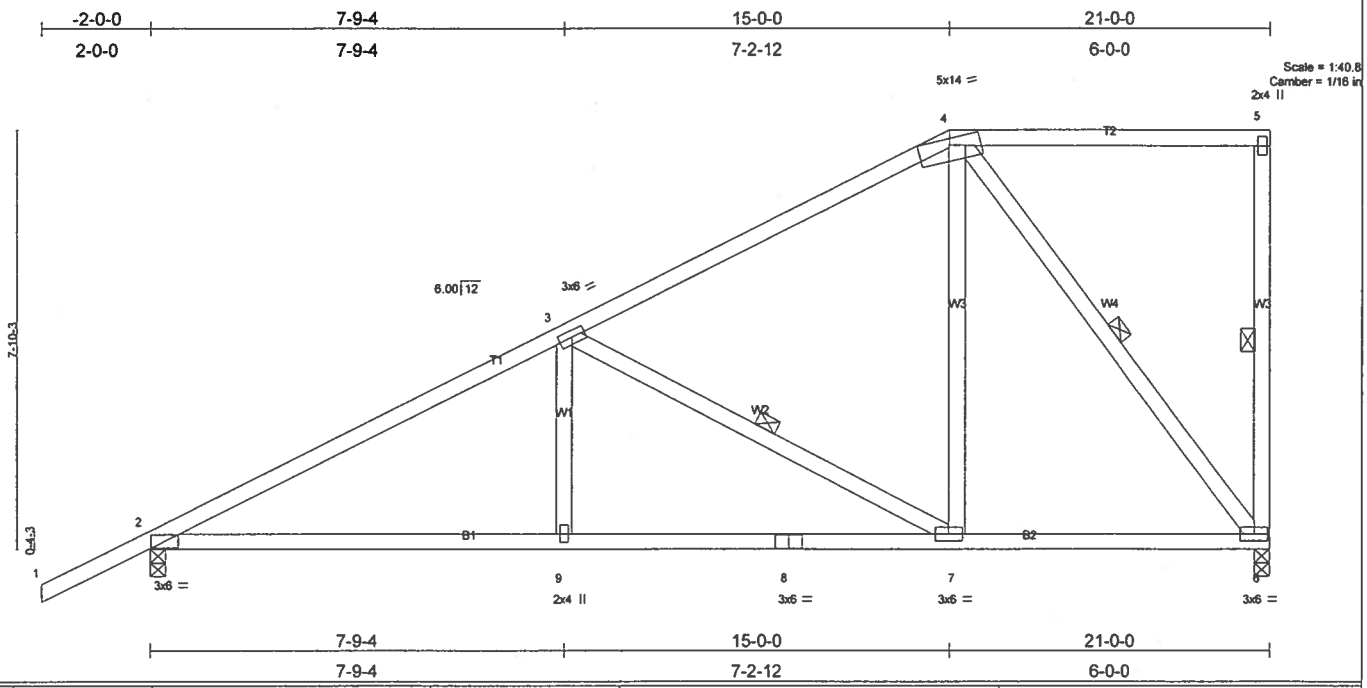
LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0307071105

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T13	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:39:44 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.13	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.21	2-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 121 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-5-9 oc bracing.
WEBS 1 Row at midpt 5-6, 3-7, 4-6

REACTIONS (lb/size) 6=864/0-3-8, 2=992/0-3-8
Max Horz 2=410(load case 5)
Max Uplift6=-314(load case 5), 2=-381(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1393/336, 3-4=-664/168, 4-5=-17/8, 5-6=-135/105
BOT CHORD 2-9=-545/1163, 8-9=-545/1163, 7-8=-545/1163, 6-7=-220/522
WEBS 3-9=0/258, 3-7=-738/371, 4-7=-130/542, 4-6=-825/356

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 6 and 381 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0307071106

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:40:19 2007 Page 1

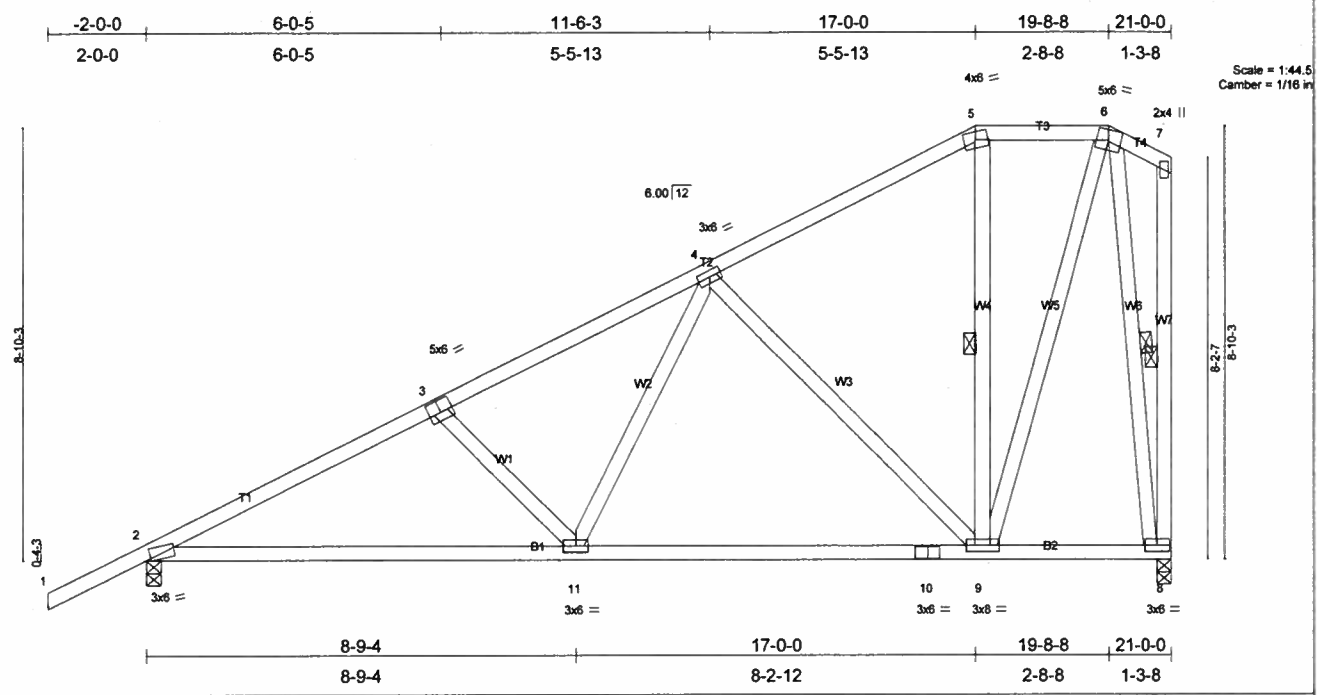


Plate Offsets (X,Y): [2:0-1-1,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.13	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.23	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 145 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-0-13 oc bracing.
WEBS 1 Row at midpt 5-9, 7-8, 6-8

REACTIONS (lb/size) 2=992/0-3-8, 8=864/0-3-8
Max Horz 2=438(load case 5)
Max Uplift2=-378(load case 5), 8=-346(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1419/363, 3-4=-1195/316, 4-5=-441/124, 5-6=-331/159, 6-7=-34/26, 7-8=-68/44
BOT CHORD 2-11=-614/1204, 10-11=-389/774, 9-10=-389/774, 8-9=-43/108
WEBS 3-11=-289/244, 4-11=-135/544, 4-9=-635/353, 5-9=-93/117, 6-9=-341/778, 6-8=-775/306

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 378 lb uplift at joint 2 and 346 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 7.2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071107

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:40:45 2007 Page 1

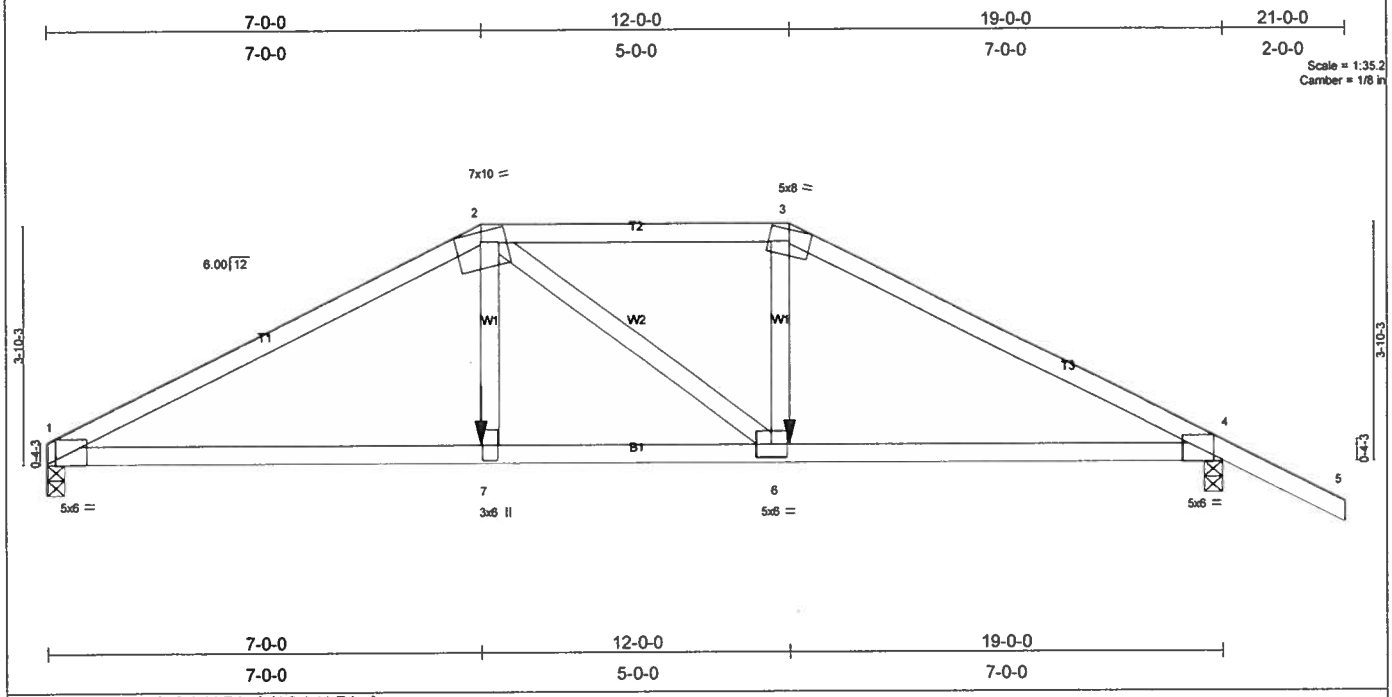


Plate Offsets (X,Y): [1:0-1-11,Edge], [4:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.15	1-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.24	1-7	>924	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.28	Horz(TL)	0.08	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 81 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-1 oc bracing.

REACTIONS (lb/size) 1=1560/0-3-8, 4=1689/0-3-8
Max Horz 1=-112(load case 5)
Max Uplift1=-642(load case 4), 4=-774(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2942/1232, 2-3=-2578/1141, 3-4=-2941/1197, 4-5=0/47
BOT CHORD 1-7=-1031/2565, 6-7=-1041/2599, 4-6=-970/2544
WEBS 2-7=-246/832, 2-6=-149/119, 3-6=-267/884

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 642 lb uplift at joint 1 and 774 lb uplift at joint 4.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 12-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-118(F=-64), 3-5=-54, 1-7=-30, 6-7=-65(F=-35), 4-6=-30
Concentrated Loads (lb)
Vert: 7=-539(F) 6=-539(F)

MARCH 7,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071108

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:43:53 2007 Page 1

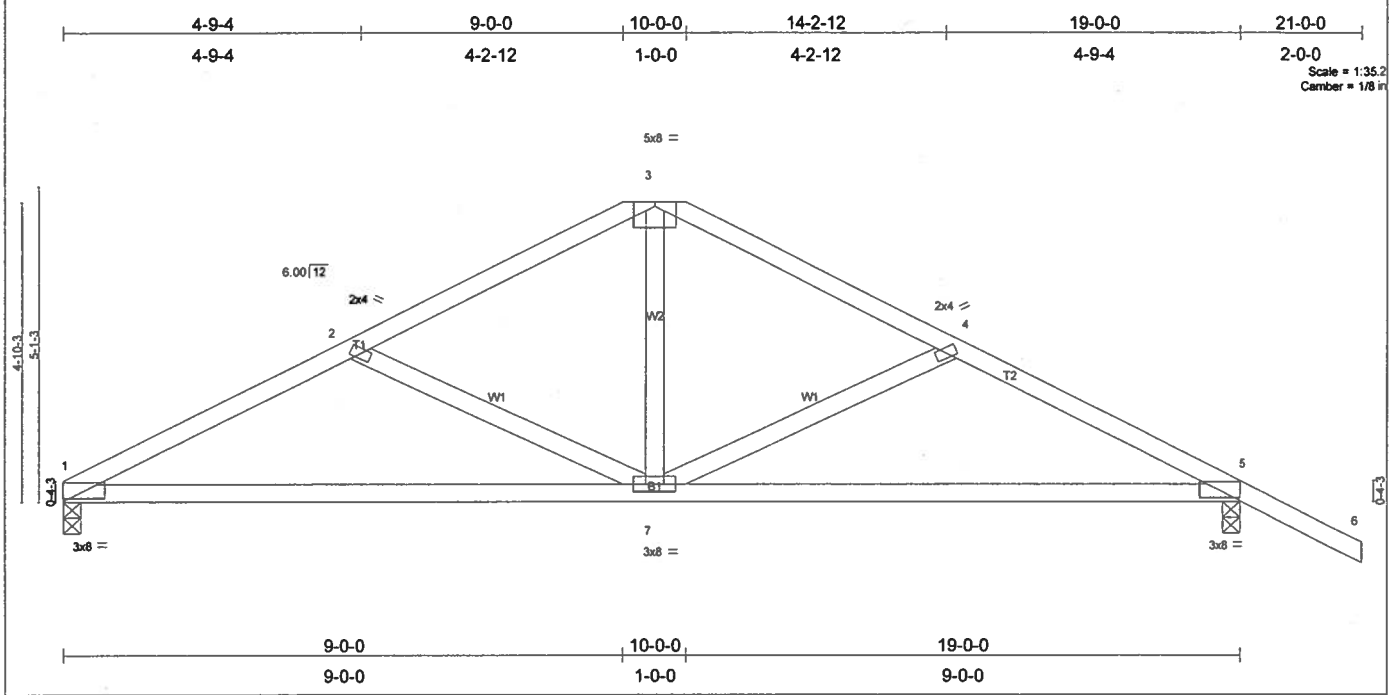

Plate Offsets (X,Y): [1:0-8-0,0-0-10], [5:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.17	1-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.28	1-7	>795	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=779/0-3-8, 5=908/0-3-8
Max Horz 1=-129(load case 6)
Max Uplift1=-253(load case 5), 5=-384(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1283/448, 2-3=-989/308, 3-4=-987/322, 4-5=-1263/408, 5-6=0/47
BOT CHORD 1-7=-381/1117, 5-7=-269/1090
WEBS 2-7=-341/262, 4-7=-311/224, 3-7=-116/571

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 1 and 384 lb uplift at joint 5.

LOAD CASE(S) Standard

MARCH 7.2007 TRUSS DESIGN ENGINEER:
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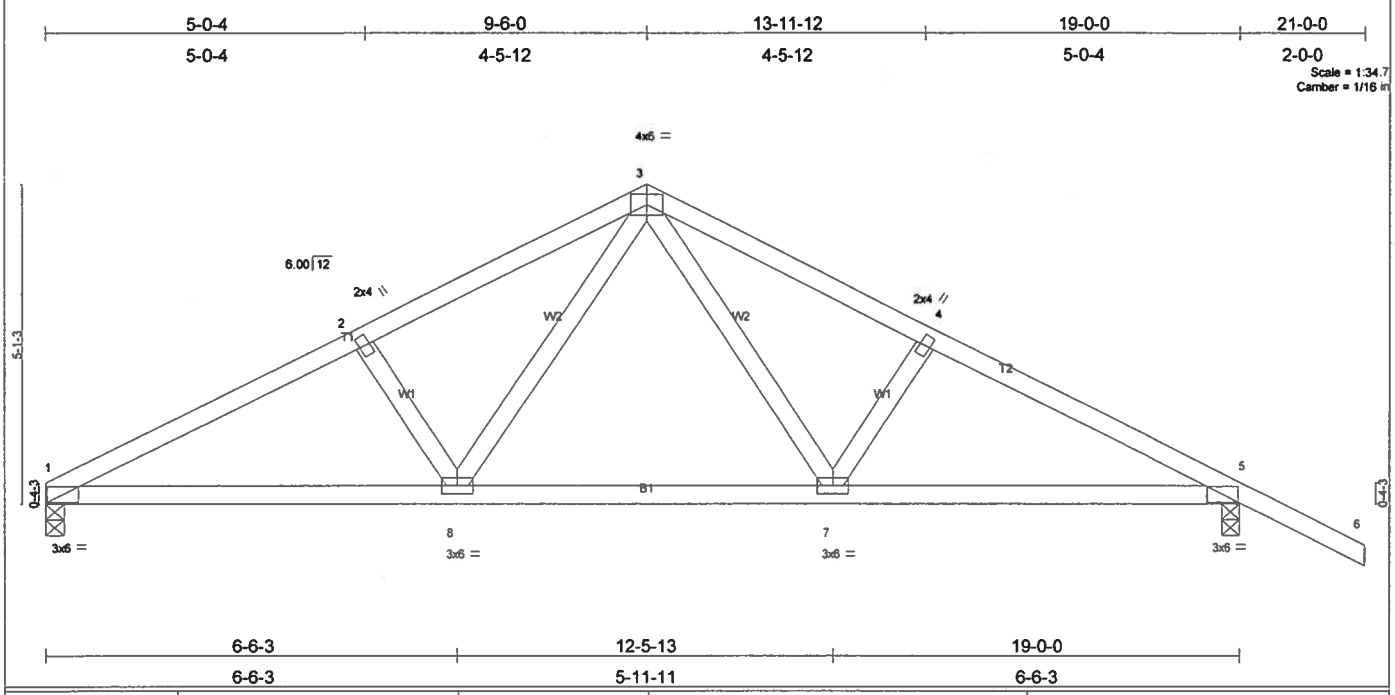
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

Dwg.#0307071109

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T17	COMMON	5	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 11:44:13 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.13	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.21	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 88 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-11-14 oc bracing.

REACTIONS (lb/size) 1=928/0-3-8, 5=1058/0-3-8
Max Horz 1=-129(load case 6)
Max Uplift1=-309(load case 5), 5=-441(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1666/561, 2-3=-1540/560, 3-4=-1515/517, 4-5=-1651/516, 5-6=0/47
BOT CHORD 1-8=-476/1440, 7-8=-235/985, 5-7=-369/1409
WEBS 2-8=-227/207, 3-8=-248/662, 3-7=-200/625, 4-7=-208/183

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 1 and 441 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-8=-30, 7-8=-80(F=-50), 5-7=-30

MARCH 7,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E. LUTZ, FL 33549

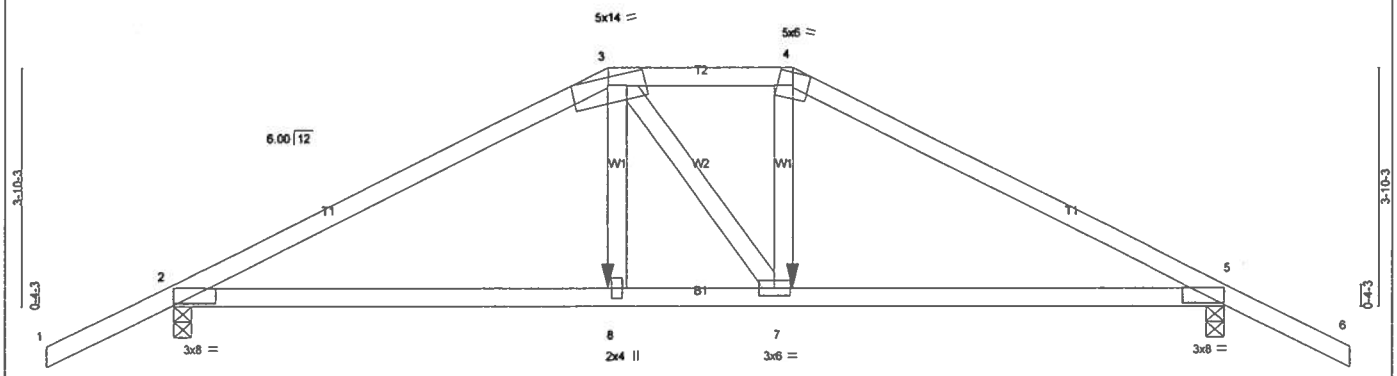
Dwg.#0307071110

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 12:11:11 2007 Page 1

-2-0-0 | 7-0-0 | 10-0-0 | 17-0-0 | 19-0-0
2-0-0 | 7-0-0 | 3-0-0 | 7-0-0 | 2-0-0

Scale = 1:35.2
Camber = 1/16 in



7-0-0 | 10-0-0 | 17-0-0
7-0-0 | 3-0-0 | 7-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.15	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.21	2-8	>942	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.06	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 76 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-7 oc bracing.

REACTIONS (lb/size) 2=1500/0-3-8, 5=1500/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-910(load case 4), 5=-910(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2509/1322, 3-4=-2190/1260, 4-5=-2512/1323, 5-6=0/47
BOT CHORD 2-8=-1106/2157, 7-8=-1121/2187, 5-7=-1080/2159
WEBS 3-8=-376/749, 3-7=-126/136, 4-7=-410/833

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 910 lb uplift at joint 2 and 910 lb uplift at joint 5.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 10-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-30, 7-8=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

MARCH 7.2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE F. LUTZ, FL 33549

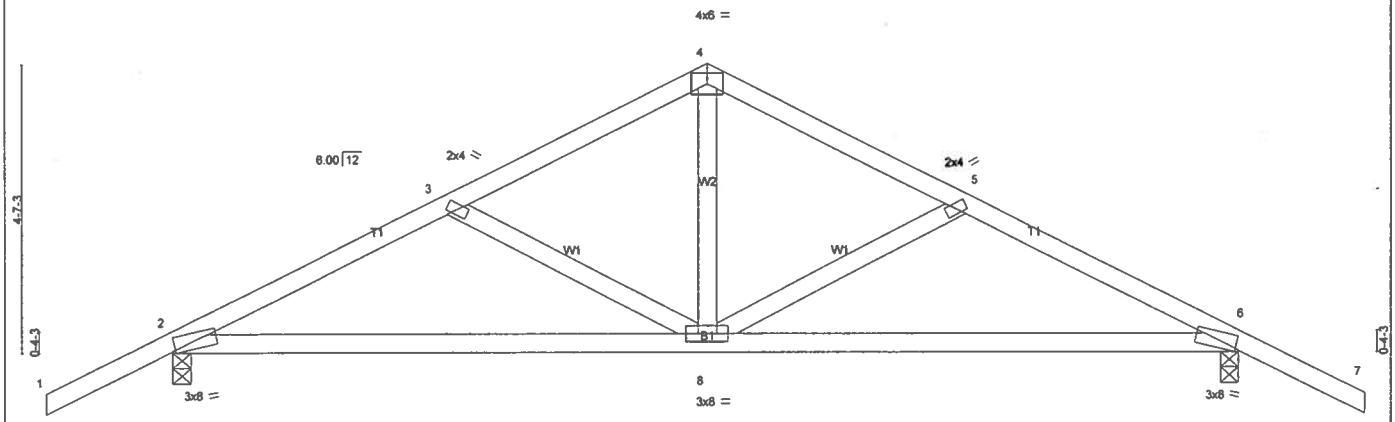
Dwg.#0307071111

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T19	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 12:11:21 2007 Page 1

-2-0-0 | 4-6-4 | 8-6-0 | 12-5-12 | 17-0-0 | 19-0-0
2-0-0 | 4-6-4 | 3-11-12 | 3-11-12 | 4-6-4 | 2-0-0

Scale = 1:34.7
Camber = 1/16 in



8-6-0 | 17-0-0
8-6-0 | 8-6-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.22	2-8	>898	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	0.18	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 80 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-3 oc bracing.

REACTIONS (lb/size) 2=818/0-3-8, 6=818/0-3-8
Max Horz 2=-98(load case 6)
Max Uplift2=-565(load case 5), 6=-565(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1085/922, 3-4=-852/836, 4-5=-852/836, 5-6=-1085/922, 6-7=0/47
BOT CHORD 2-8=-700/930, 6-8=-700/930
WEBS 3-8=-259/215, 4-8=-633/496, 5-8=-259/216

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 565 lb uplift at joint 2 and 565 lb uplift at joint 6.

LOAD CASE(S) Standard

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0307071112

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - MIKE ROBERTS
L229281	T20	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 02 12:12:27 2007 Page 1

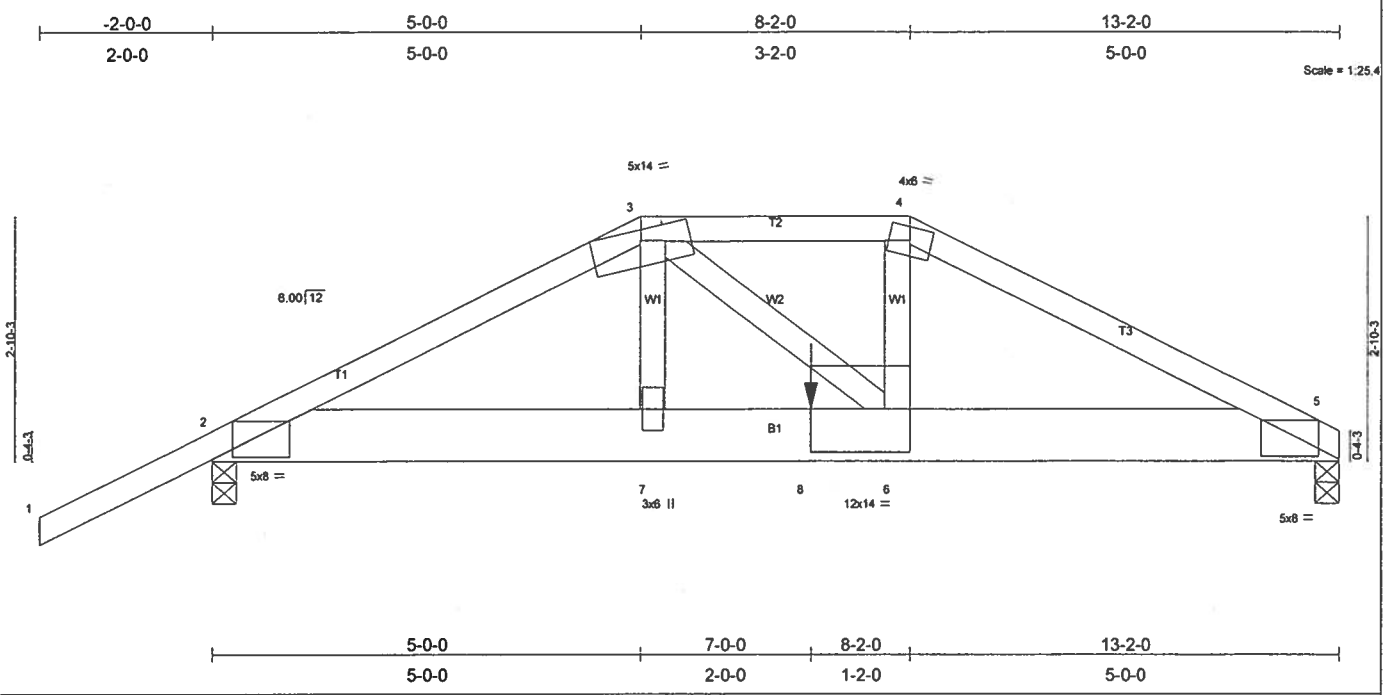


Plate Offsets (X,Y): [2:0-4-0,0-1-15], [5:0-4-0,0-1-15], [6:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.12	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.48	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=4816/0-3-8, 2=2750/0-3-8
Max Horz 2=106(load case 4)
Max Uplift5=-1780(load case 5), 2=-1086(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-5496/1971, 3-4=-6592/2471, 4-5=-7189/2633
BOT CHORD 2-7=-1723/4872, 7-8=-1739/4926, 6-8=-1739/4926, 5-6=-2298/6414
WEBS 3-7=-268/895, 3-6=-850/2190, 4-6=-1055/2966

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1780 lb uplift at joint 5 and 1086 lb uplift at joint 2.
7) Girder carries tie-in span(s): 33-5-8 from 8-0-0 to 13-2-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3096 lb down and 1169 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

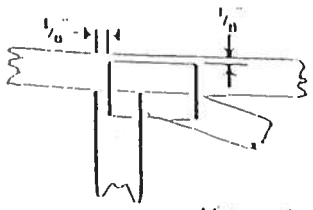
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-6=-30, 5-6=-685(F=-655)
Concentrated Loads (lb)
Vert: 8=-3096(F)

MARCH 7,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE E, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web).

* This symbol indicates the required direction of slots in connector plates

PLATE SIZE

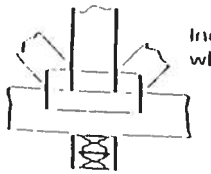
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

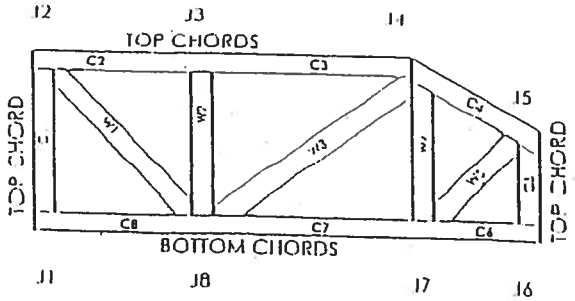
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

FLORIDA DEPARTMENT OF Community Affairs

Community Affairs

Product Approval

USER: Public User

FL #	FL7674
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	Alenco
Address/Phone/Email	615 Carson Bryan, TX 77802 (979) 779-7770 ext 343 mkoppers@alenco.com
Authorized Signature	Martin Koppers mkoppers@alenco.com
Technical Representative	Martin Koppers
Address/Phone/Email	615 Carson St. Bryan, TX 77802 mkoppers@alenco.com
Quality Assurance Representative	
Address/Phone/Email	
Category	Windows
Subcategory	Single Hung
Compliance Method	Certification Mark or Listing
Certification Agency	National Accreditation & Management Institute,
Referenced Standard and Year (of	**Standard**

Standard)	AAMA/NWWDA 101/IS2 1

Equivalence of Product Standards
Certified By

Product Approval Method	Method 1 Option A
Date Submitted	10/06/2006
Date Validated	11/17/2006
Date Pending FBC Approval	11/20/2006
Date Approved	12/06/2006

Summary of Products

FL #	Model, Number or Name	Description
7674.1	3753	Single Hung
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** +40 /-40 **Other:** 3753 - 4'0" X 6'0" H-R40 – 1/8" annealed glass - smaller units to comply with ASTM E-1300-02		**Certification Agency Certificate** FL7674_R0_C_CAC_3753 H R-40.pdf **Installation Instructions** FL7674_R0_II_3753 Installation Instructions.pdf Verified By: National Accreditation & Management Institute,
7674.2	3753FL	Single Hung
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** No **Design Pressure:** +35 /-35 **Other:** 3753FL - 4'5" X 5'3" H-R35 – 1/8" annealed glass - smaller units to comply with ASTM E-1300-02		**Certification Agency Certificate** FL7674_R0_C_CAC_3753FL H R35.pdf **Installation Instructions** FL7674_R0_II_3753FL - 4710FL Install Instructions.pdf Verified By: National Accreditation & Management Institute,
7674.3	3753FL	Single Hung
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** No **Design Pressure:** +30 /-30 **Other:** 3753FL - 4'4" X 5'11" H-R30 – 1/8" annealed glass - smaller units to comply with ASTM E-1300-02		**Certification Agency Certificate** FL7674_R0_C_CAC_3753FL H R30.pdf **Installation Instructions** FL7674_R0_II_3753FL - 4710FL Install Instructions.pdf Verified By: National Accreditation & Management Institute,
7674.4	4710/4710F	Single Hung
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** No		**Certification Agency Certificate** FL7674_R0_C_CAC_4710_4710F H-R4(DP50.pdf **Installation Instructions**

Design Pressure: +40 /-50 **Other:** 4710/4710F - 4'0" X 6'0" H-R40 -DP 50 – DSB annealed glass - smaller units to comply with ASTM E-1300-02	FL7674_R0_II_4710F Installation Instructions.pdf Verified By: National Accreditation & Management Institute,

7674.5	4710FL	Single Hung
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** No **Design Pressure:** +40 /-40 **Other:** 4710FL - 3'8" X 6'6" H-R40 – 1/8" annealed glass - smaller units to comply with ASTM E-1300-02		**Certification Agency Certificate** FL7674_R0_C_CAC_4710FL H R-40.pdf **Installation Instructions** FL7674_R0_II_3753FL - 4710FL Install Instructions.pdf Verified By: National Accreditation & Management Institute,

Back Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

(1)

Ahlbrandt Ref. Inc.
Cooler By Design

Short Form
Entire House
Ahlbrandt Ref. Inc.

Job:
Date: May 16, 2007
By:

P O Box 1945, Alachua, Fl 32616 Phone: 352-225-1308 Fax: 386-418-0549

Project Information

For: Mike Roberts

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	1 (Average)

HEATING EQUIPMENT

Make York
Trade Guarden
Model HP030X1321

Efficiency	8 HSPF	
Heating input		
Heating output	30000	Btuh @ 47°F
Temperature rise	29	°F
Actual air flow	933	cfm
Air flow factor	0.035	cfm/Btuh
Static pressure	0.10	in H2O
Space thermostat		

COOLING EQUIPMENT

Make York
Trade Guarden
Cond HP030X1321
Coil G2FD036S17+1TV0701

Efficiency	13 SEER	
Sensible cooling	19600	Btuh
Latent cooling	8400	Btuh
Total cooling	28000	Btuh
Actual air flow	933	cfm
Air flow factor	0.048	cfm/Btuh
Static pressure	0.10	in H2O
Load sensible heat ratio	0.69	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
Dining rm	131	3391	1820	119	87
Kitchen	108	1408	2284	50	109
Bath	52	975	495	34	24
Bedrm 2	182	5143	3325	181	159
Bedrm 3	186	3195	2343	112	112
Fam rm	357	4398	4219	155	202
Master bedrm	219	3379	3010	119	144
Master bath	78	2926	1128	103	54
W.I.c.	48	1531	569	54	27
core	132	189	314	7	15

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House Other equip loads Equip. @ 0.97 RSM Latent cooling	1494	26536 5651	19507 2596 21440 9979	933	933
TOTALS	1494	32187	31419	933	933

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

GARAGE HEADER (84) $26F_b$ - 1.9E

Anthony Power Header®

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8885	12015	15996	20145	24792	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1557.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

	810 / 340	3-1/2" x 8-3/4"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
	990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-3/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
	640 / 400	3-1/2" x 12-1/8"	3-1/2" x 13-1/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"
	765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"
	750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"
	900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	—

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

WOODFORD PLYWOOD, INC.
"Structural Wood Products"

11960 West Beaver Street
Jacksonville, Florida 32220

(904) 695-9060
(800) 447-6566
FAX (904) 695-9160

Anthony POWER HEADER®

26F$_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896	1218		1373							
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975	1322									
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	659	693	

862	976	888	1056	1343	1367	1582							
107	153	169	245	260	380	369	540	501	715	664	864	840	
76	107	120	171	185	267	261	350	356	521	471	684	809	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

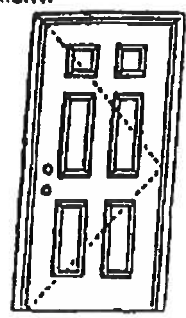
X

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door

Design Pressure

+66.0/-66.0

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED

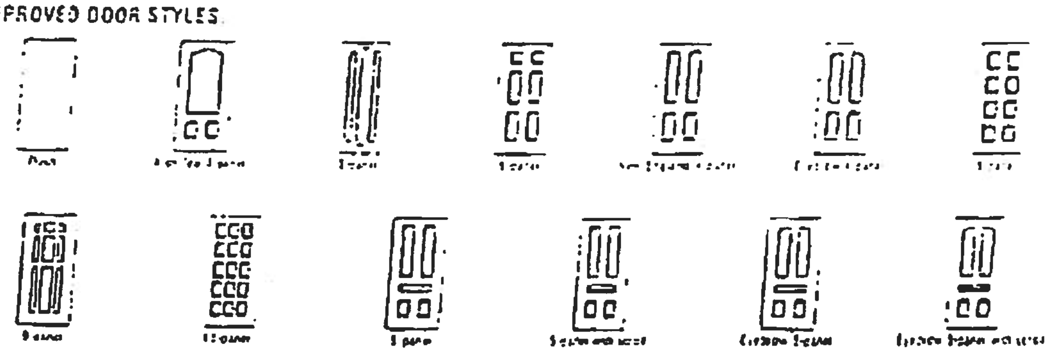
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed

APPROVED DOOR STYLES:



Johnson EntrySystems

Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

[signature]

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Johnson EntrySystems

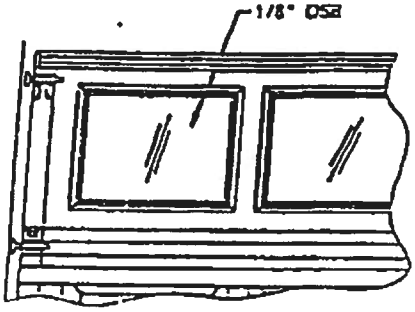
Masonite International Corporation

GARAGE DOORS

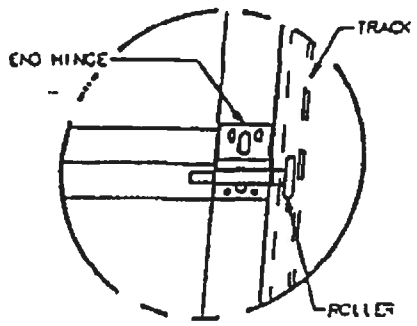
REVISIONS			
LETTER	DESCRIPTION	DATE	APPROVED
A	REV PER EN 10136	3/06/98	CK
B	REV PER EN 10141	3/13/98	CK
C	REV PER EN 10137	7/7/98	CK

ECTED WITH 3 OR MORE SECTIONS MUST BE
E SPLICE FOR THE ADDITIONAL INTERMEDIATE
ON THE THIRD SECTION.
.L SECTIONS LESS THAN 20.812" MUST BE
THE ACTUAL SECTION HEIGHT & 20.812".
1 x 5/8" LONG TYPE AB HEX HEAD METAL SCREW
4 WIND WARNINGS ISSUED.

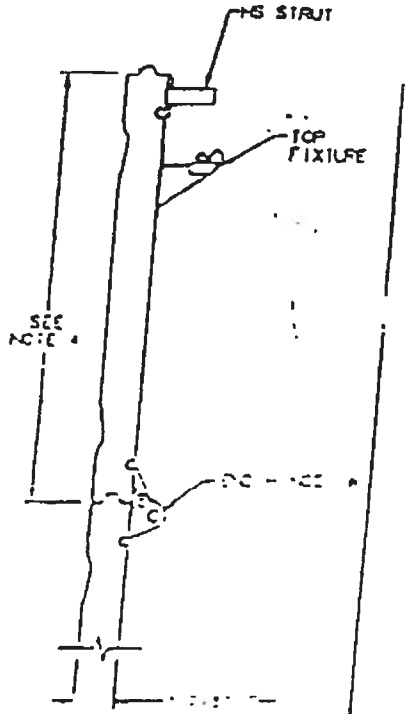
3.



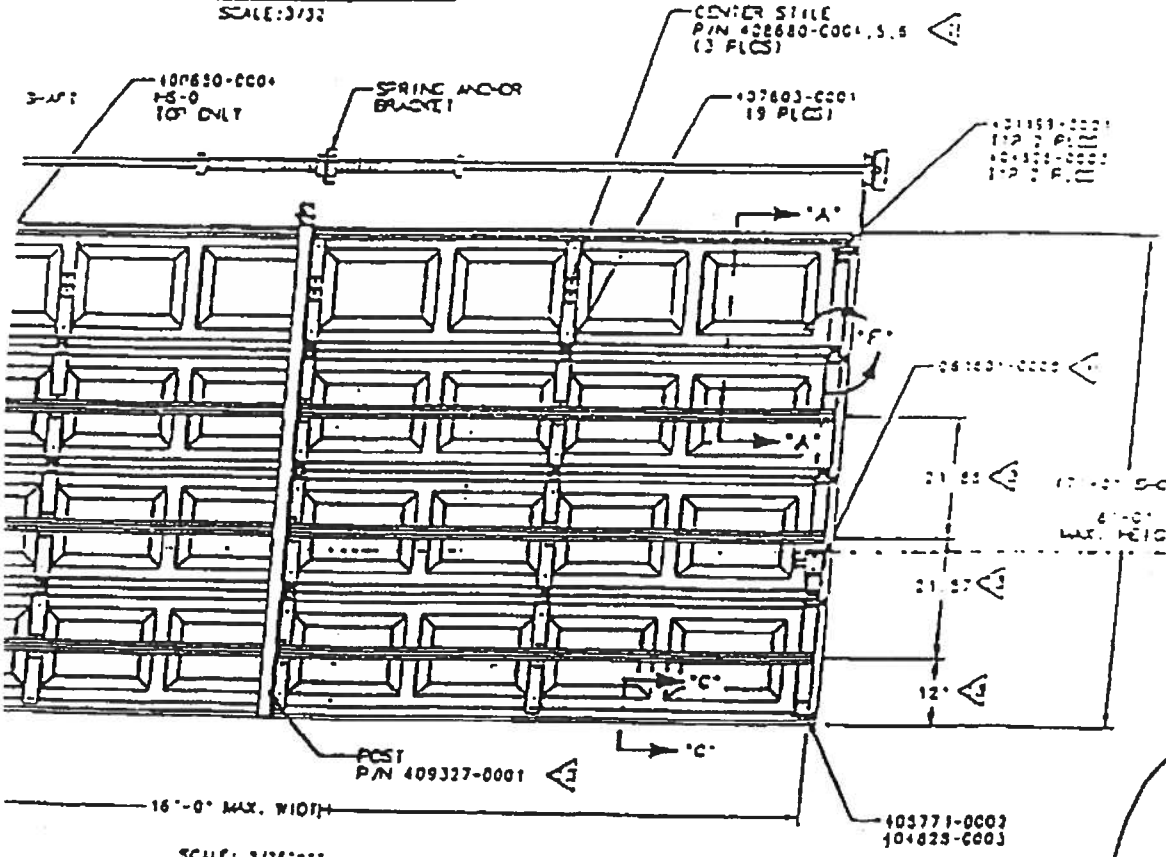
OPTIONAL WINDOW DETAIL
SCALE: 3/32



DETAIL "F"
SCALE: 1/4
(16" WIDE)



SECTION "A-A"

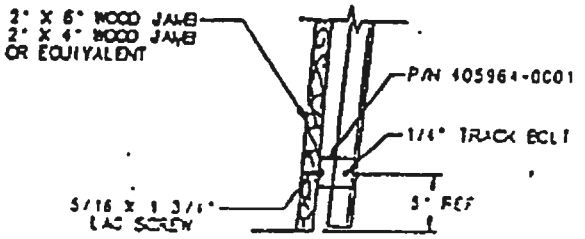


DESIGN LOAD
25 PSF =
TEST LOAD
37.5 PSF =

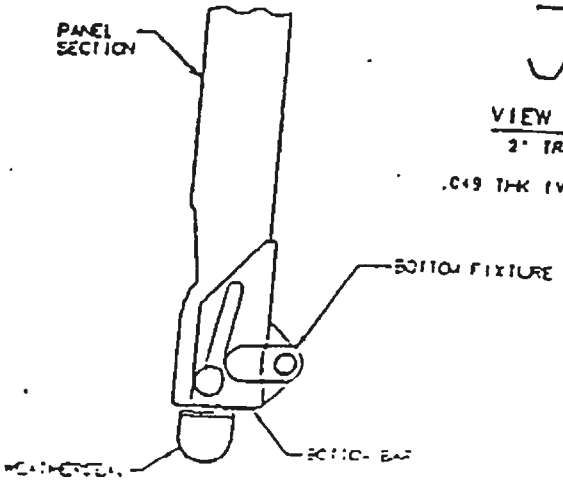
SCALE: 3/16"=1'
INTERIOR ELEVATION

	M. TOLNIS	01/13/98	SERIES 380 & 390 RESI STL DR. 16'-0" MAX WIDTH, WINDLOAD
N/A	DAVID FAN	02/19/98	D-409335
NONE EACH	DAVID FAN	02/19/98	

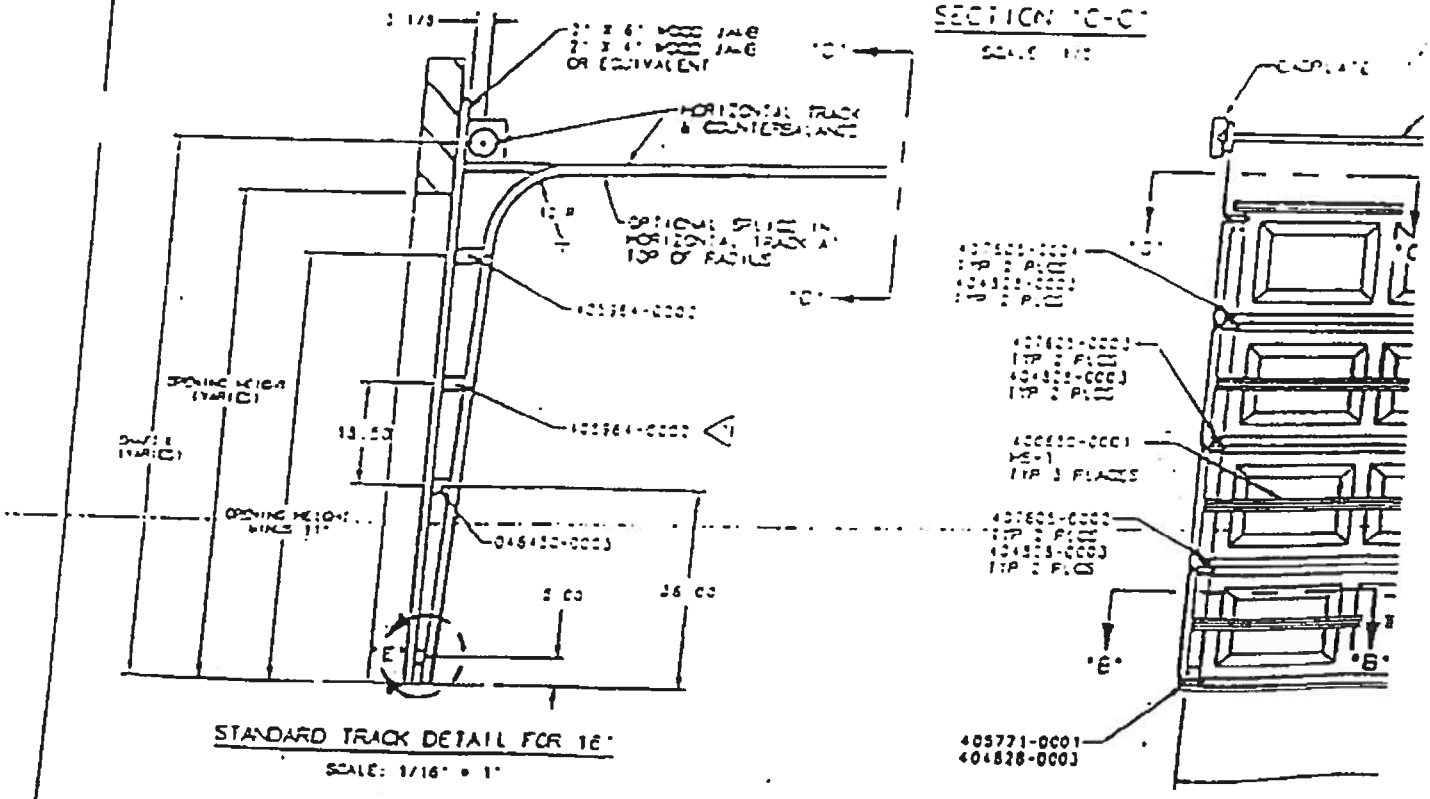
1. TESTED IN ACCORDANCE WITH STANDARD BUILDING CODE, CHAPTER 17, TO A POSITIVE AND NEGATIVE 37.5 PSF.
2. DASH NUMBERS REPRESENT VARIOUS SECTION HEIGHTS.
3. FOUR SECTION 7' HIGH DOOR SHOWN. 8' HIGH DOORS HAVE FIVE SECTIONS.
4. SECTION HEIGHT OF 20.812, 19.00 & 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS HEIGHT DOORS.
5. EMBOSSMENT PATTERN OF 16.50 X 20.375 SHOWN. ALTERNATE PATTERNS OF 12.50 X 43.375 AND 12.50 X 20.375 MAY BE USED.
6. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
7. USE THIS BRACKET, REF. P/N 405964-0002, ON 8' HIGH DOORS ONLY.
8. WINDOW MAY BE INSTALLED IN THE TOP SECTION OR THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
9. THE STRUT PLACEMENT ON E CONSISTENT WITH THE DOOR SECTIONS ARE TO BE PLACED
10. THE STRUT PLACEMENT DIMEN REDUCED BY THE DIFFERENCE
11. SCREW P/N 803911-0001 IS
12. POST TO BE INSTALLED ONLY
13. STRUT PLACEMENTS CAN VARY
14. QUANTITY FOR LOCKS CAN BE



DETAIL "E"
SCALE: 1/3



VIEW "D-
2" TRACK
.049 THK (VERT



STANDARD TRACK DETAIL FOR 16'
SCALE: 1/16" = 1"

SECTION "C-C"
SCALE: 1/2

SERIES 280 THRU 289 ARE EQUIVALENT CONSTRUCTION 25GA STEEL
SERIES 180 ARE SAME CONSTRUCTION AS SERIES 280 ONLY 24GA STEEL AND END CAPS

DOOR TESTED WAS 281 SERIES.

DOOR WIDTH	CENTER STILE	END STILE	ROLLER SHAFT BRACKET	STRUTS/SECT.	ROLLER	VERTICAL TRACK GAGE	JAMB LOAD (# PER FT-HT.)	HARDWARE
16'	3	SINGLE		HS1 HS2	2" X 7/16"	.049"	100	STD.

(CONTINUED from Pg. 2)

TAMKO ROOFING PRODUCTS

• Glass-Seal • Glass-Seal AR • Elite Glass-Seal® • Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a 3.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Mansard Fastening Pattern.

8. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

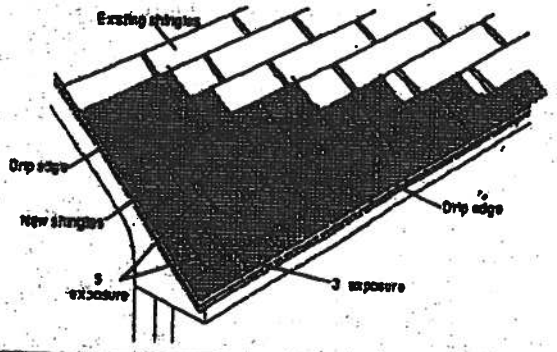
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nesting procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Succeeding Courses: According to the off-set application method you choose to use, remove the appropriate length from the rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.



9. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 50 lb. roll roofing in the valley. Nail the felt only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

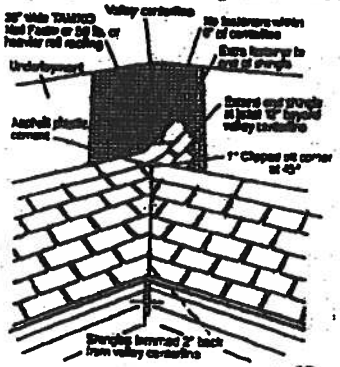
Note: For a neater installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

- ***CAUTION:*** *Adhesive must be applied in smooth, thin, even layers.*

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

TAMKO®
ROOFING PRODUCTS

FEB - 4 RECD

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

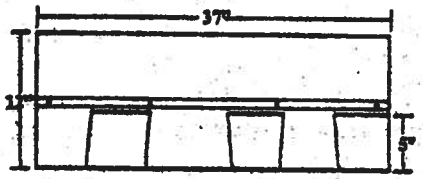
TAMKO Roofing Products, Inc.

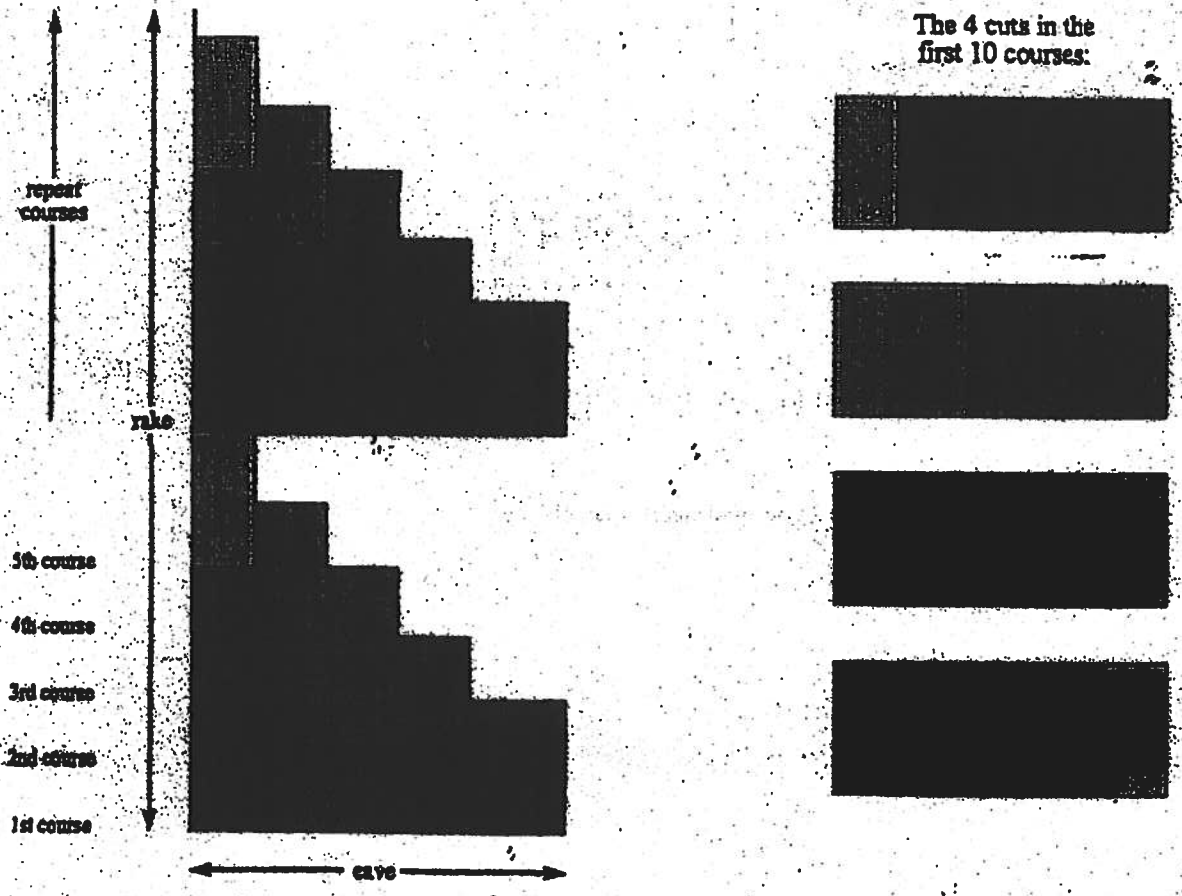
CORPORATE HEADQUARTERS
220 W. FOURTH STREET P.O. BOX 1404 JOPLIN, MO 64802-1404 800-641-4691 FAX 800-641-1925

TAMKO
ROOFING PRODUCTS

Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"





In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.

TAMKO ROOFING PRODUCTS

Application Instructions for

• Glass-Seal
• Glass-Seal AR
• Elite Glass-Seal®
• Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.
THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.
IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be ≥ 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.
FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

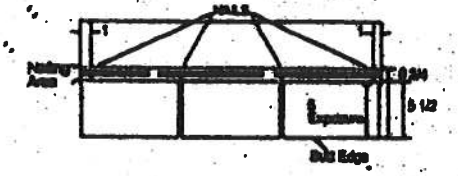
3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

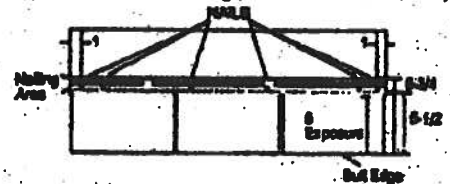
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below).



2) Mansard or High Wind Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fastener per shingle. (See Mansard fastening pattern illustrated below.)



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

Visit Our Web Site at
www.tamko.com

Central District	220 West 4th St., Joplin, MO 64801	800-641-4691
Northeast District	4500 Tamko Dr., Frederick, MD 21701	800-368-2065
Southeast District	2300 35th St., Tuscaloosa, AL 35401	800-228-2586
Southwest District	7910 S. Central Exp., Dallas, TX 75216	800-443-1834
Western District	6300 East 43rd Ave., Denver, CO 80216	800-530-8668

FF 25976

Cal-Tech Testing, Inc.

• Engineering
• Geotechnical
• Environmental
Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 07-350
DATE TESTED: 7/30/07
DATE REPORTED: 7/30/07

PROJECT:	Roberts Residence, Lake City, FL
CLIENT:	Mike Roberts, 657 SW Catherine Lane, Lake City, FL 32025
GENERAL CONTRACTOR:	Mike Roberts
EARTHWORK CONTRACTOR:	Mike Roberts
INSPECTOR:	Joe Janosh

ASTM METHOD	SOIL USE
(D-2922) Nuclear	BUILDING FILL

SPECIFICATION REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1		8' South x 8' East of NW Corner	12"	116.7	7.3	108.8	1	111.0	98%
2		7' South x 12' West of NE Corner	12"	115.0	7.7	106.8	1	111.0	96%
3		10' North x 12' West of SE Corner	12"	115.1	6.4	108.2	1	111.0	97%
4		9' North x 11' East of SW Corner	12"	117.2	7.5	109.0	1	111.0	98%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
1		Light Brown Fine Sand (Register Pit)	111.0	11.5	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

Date: 7/31/07
Licensed, Florida No: 57842

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

25976

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 321 N.W. Cole Terrace, Suite 107 City Lake City State FL Zip 32055
Company Business License No. JB109978 Company Phone No. 386-755-3611 • 352-494-5751
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Mike Roberts Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 1148 SE Wildwood Rd Lake City FL

Type of Construction (More than one box may be checked) ☑ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside Inside Type of Fill

Section 4: Treatment Information

Date(s) of Treatment(s) 9-19-07
Brand Name of Product(s) Used Bora-Care
EPA Registration No. 64405-1
Approximate Final Mix Solution % .23
Approximate Size of Treatment Area: Sq. ft. 2137 Linear ft. Linear ft. of Masonry Voids
Approximate Total Gallons of Solution Applied 4 ½
Was treatment completed on exterior? ☑ Yes ☐ No
Service Agreement Available? ☑ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) Steve Brannon Certification No. (if required by State law)

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Date 9-19-07

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 03-4S-17-07570-117 **Building permit No.** 000025976

Use Classification SFD,UTILITY **Fire:** 64.20

Permit Holder MIKE ROBERTS **Waste:** 167.50

Owner of Building MIKE ROBERTS **Total:** 231.70

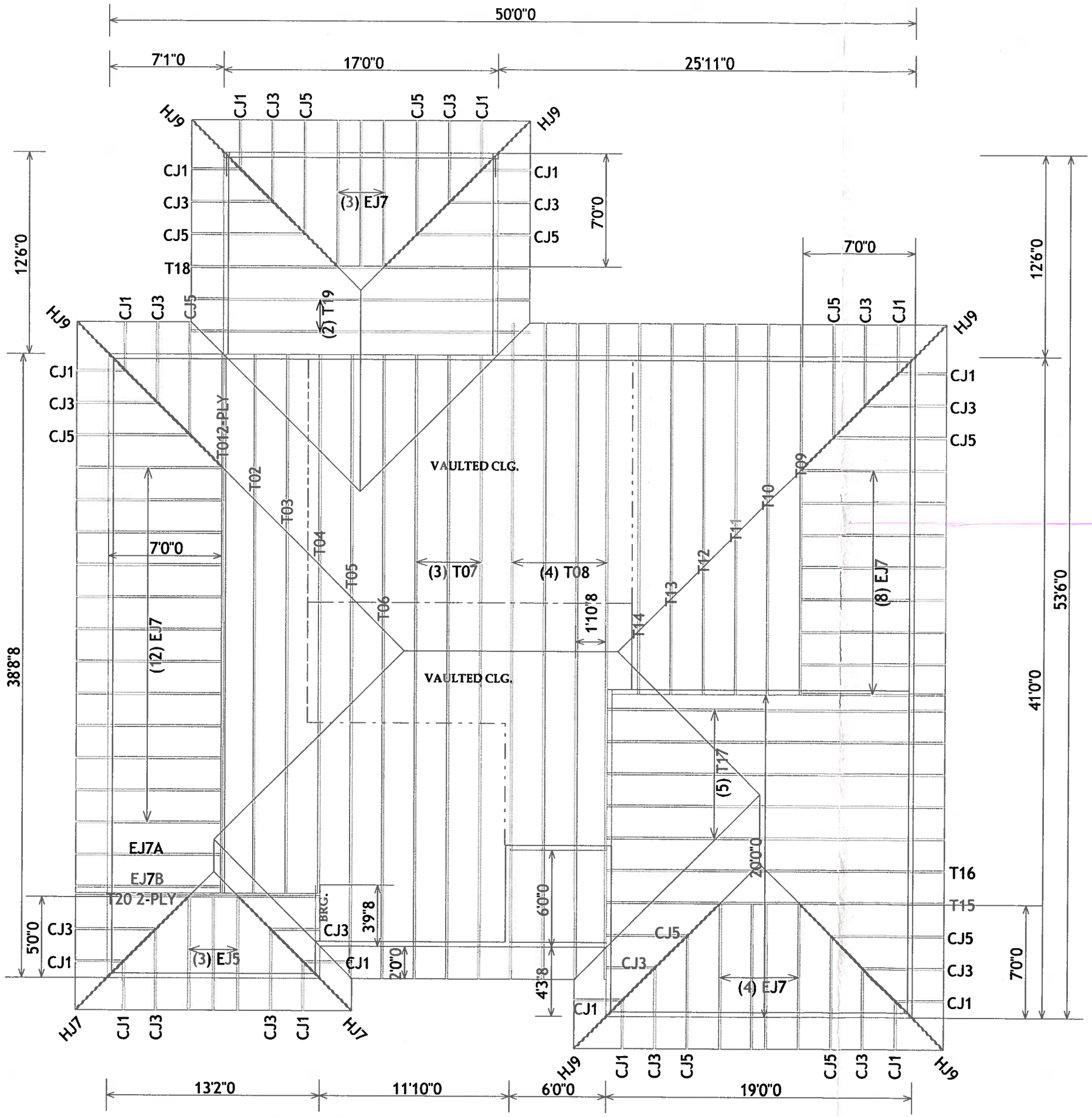
Location: 118 SE WILDERNESS DR, LAKE CITY, FL

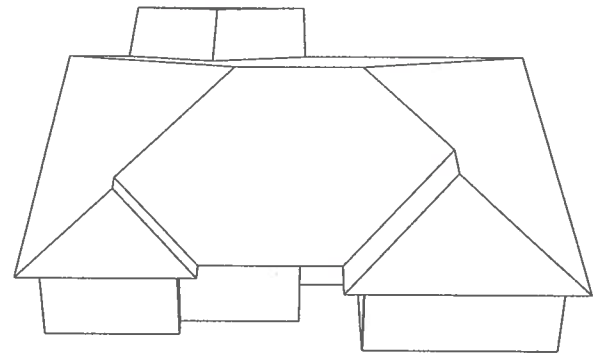
Date: 12/19/2007

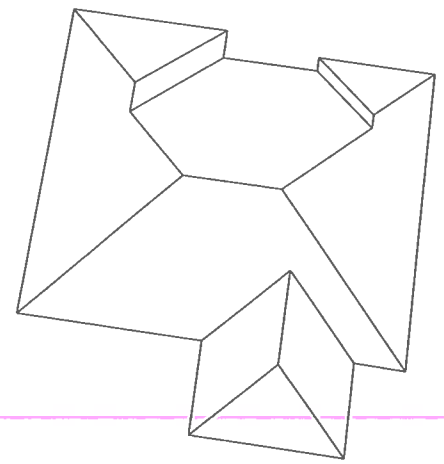
Randy Jones by E

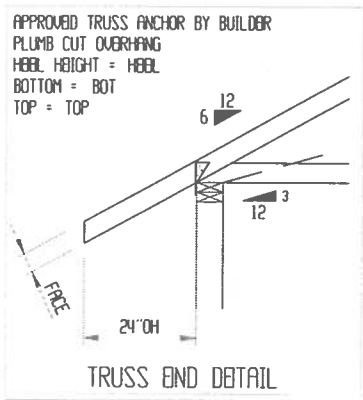
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



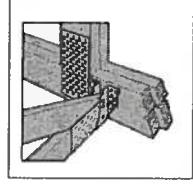
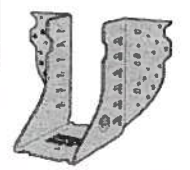






TRUSS END DETAIL

HANGER SCHEDULE

TRUSS HANGER INFORMATION Check TRUSS ENGINEERING For gravity and uplift values if the value exceeds the capacity of a hanger.	
(2)HTU26	(1)HGUS26-2
	

BEARING HEIGHT SCHEDULE

9'-0"

OVERHANG
2'-0"

ROOF PITCH(S)
6/12

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER:
CASH ACCOUNT - MIKE ROBERTS

LEGAL ADDRESS:
COLUMBIA, FL

MODEL:	REVISION:
CUSTOM	SCALE: NTS
DATE: **03/02/07**	DRAWN BY: **A MONDRAGON**
JOB #: **L229281**	