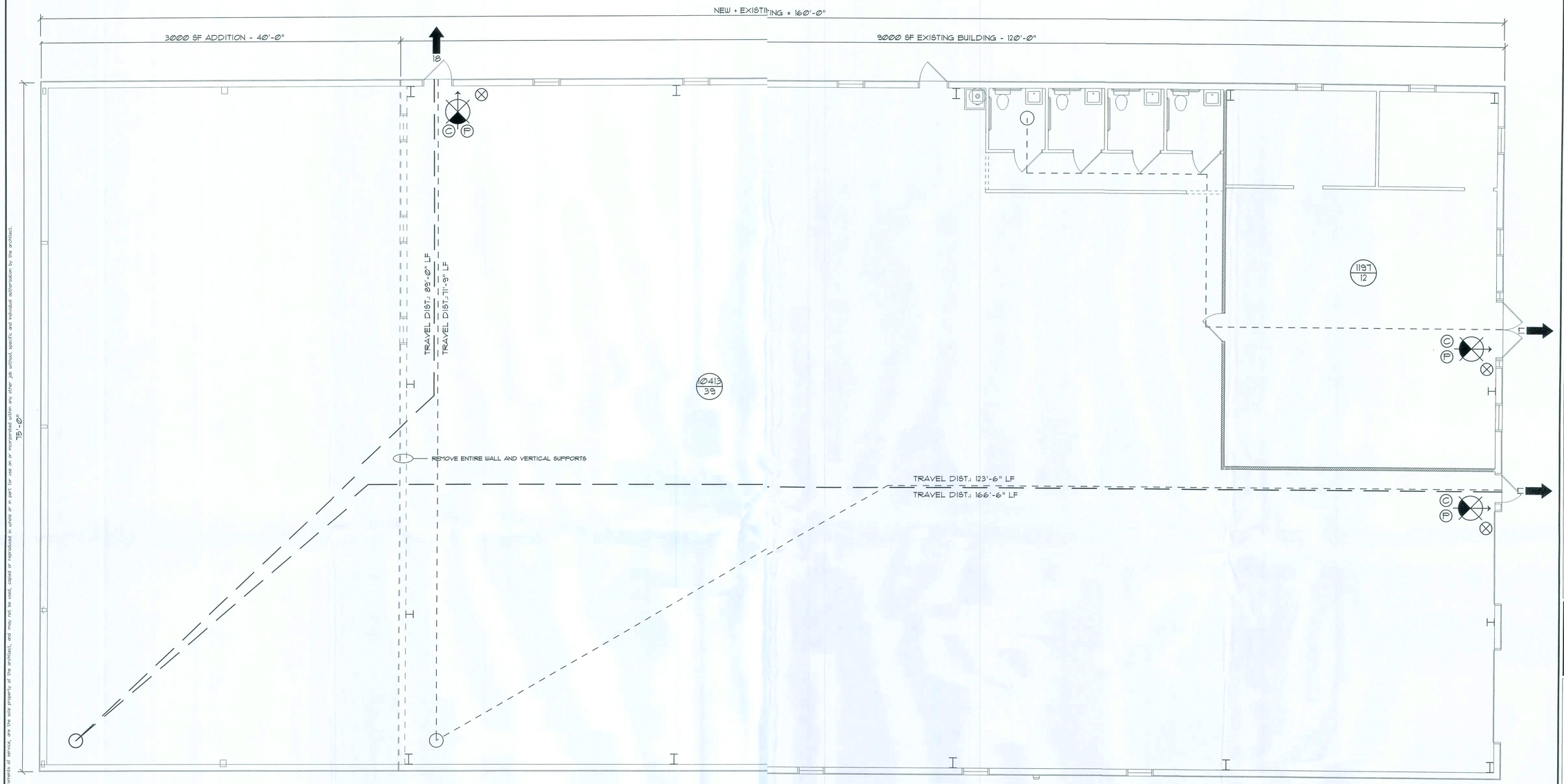

LIFE SAFETY PLAN

SCALE: 3/16" = 1'-0"

NOTE!

PLACE FIRE DEPARTMENT APPROVED SIGNAGE, NEAR ENTRY INDICATING " LIGHT WEIGHT TRUSS ATTIC FRAMING" REFER TO FIRE MARSHAL FOR APPROVAL

WET-PIPE SPRINKLER SYSTEM

THIS BUILDING SALL BE EQUIPPED WITH A WET-PIPE SPRINKLER SYSTM, DESIGNED BY AICENSED FIRE PROTECTION ENGINEER OR FIRE PROTECTION SYSTEM DESSIGN CERTIFIED BY "NICET" TO A LEVEL THREE MINIMUM AND ENGAGED IN DESN OF FIRE PROTECTION SYSTEMS.

NOTE!

EMERGENCY LIGING AND EXIT SIGNS, SHALL BE PROVIDED AS DIRECTED BYHE FIRE MARSHAL, AND SHALL BE WIRED PER NEC '700-12F

NOTE!

SMOKE DETECTO SHALL BE MOUNTED NOT LESS THAN 90" ABOVEINISHED FLOOR AND SHALL BE THE IONIZATION TYPEINTERLOCKED TOGETHER, POWERED FROM EACH STOR PANEL W/BATTERY BACKUP

OCCUPANCY LOAD - BUSINESS

AREA	TOTAL SF.	ALLOWABLE	TOTAL OCCUP.
SHOWROOM:	892.5	1/100	9
OFFICE	304.0	1/100	3
PRODUCTION	10148.8	1/300	34
RESTROOMS	160.5	1/50	4
PASSAGEWAY	104.0	1/100	1
BUILDING	392.2	1/500	1
BLDG. TOTAL	12000		52

LEGEND

- EXIT LIGHT - ARROW REPRESENTS DIRECTION OF EXIT - PER 2020 FBC 1006.3.1, 1006.3.2 & 1006.3.3.1
- WALL HUNG "ABC" FIRE EXTINGUISHER
- Ⓒ DOOR /CLOSER FOR EXITING OR RATING REASONS
- Ⓟ PANIC DEVICE
- DUAL HEAD EMERGENCY LIGHTING W/ BAT. PAK PER 2020 FBC 1006.1
- 999 / 30 — ROOM SQUARE FOOTAGE / ROOM OCCUPANCY LOAD
- 107 — PRIMARY EGRESS W/EXIT CAPACITY
- 24 — ALTERNATE EGRESS W/EXIT CAPACITY

NOTE!
MAXIMUM COMMON PATH OF EGRESS IS LESS THAN 40'-0" FROM ALL LOCATIONS PER 2020 FBC 1014.3

NOTE!
ALL DOOR NUMBERS AND ROOM SIGNAGE SHALL BE EQUIPTED W/ TACTIAL CHARACTERS IN COMPLIANCE W/ 2020 FAC 1006.3.4

EXIT ACCESS TRAVEL DISTANCE PER FBC 1015, TABLE 1015.1

OCCUPANCY - BUSINESS
150 FT. (W/O SPRINKLER SYSTEM)
250 FT/ (W/SPRINKLER SYSTEM)

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REVISION:

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METAL BUILDING FOUNDATION for:
DAVID & IMAGE CONSTRUCTION
DIY LETTERING, LAKE CITY, FLORIDA
LIFE SAFETY PLAN

Celebrating 49 Years of Service
1972 - 2021
N.P. Gisler, Architect
AR0007005

NICHOLAS PAUL GEISLER ARCHITECT
N.C.A.R.B. Certified
1758 NW Brown Rd.
Lake City, FL 32055
386-752-4352

DATE: 19 NOV 2021

COMM: 2K2152

SHEET: LSP.1
1 OF 1

Digitally signed by: N. P. GEISLER
DN: CN = N. P. GEISLER C = US
O = AR0007005 OU =
ARCHITECT
Date: 2021-12-01 17:11:05 -05'00'

AR0007005

GENERAL STRUCTURAL NOTES

GENERAL:

1. THE DRAWINGS ARE INTENDED TO SHOW THE GENERAL ARRANGEMENT, DESIGN AND EXTENT OF THE WORK AND ARE PARTIALLY DIAGRAMMATIC. THEY ARE NOT INTENDED TO BE SCALED FOR ROUGH-IN MEASUREMENTS, OR TO SERVE AS SHOP DRAWINGS OR PORTIONS THEREOF.

2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.

3. PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR AND ALL THE SUBCONTRACTORS SHALL VERIFY ALL GRADES, LINES, LEVELS, DIMENSIONS AND COORDINATE EXISTING CONDITIONS AT THE JOB SITE WITH THE PLANS AND SPECIFICATIONS. THEY SHALL REPORT ANY INCONSISTENCIES OR ERRORS IN THE ABOVE TO THE ARCHITECT/ENGINEER BEFORE COMMENCING WORK. THE CONTRACTOR AND HIS SUBCONTRACTORS SHALL LAY OUT THEIR WORK FROM ESTABLISHED REFERENCE POINTS AND BE RESPONSIBLE FOR ALL LINES, ELEVATIONS AND MEASUREMENTS IN CONNECTION WITH THEIR WORK.

4. IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS, GENERAL NOTES OR OTHER DOCUMENTS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IN WRITING OF SUCH OMISSION OR ERROR PRIOR TO PROCEEDING WITH ANY WORK WHICH APPEARS IN QUESTION. IN THE EVENT OF THE CONTRACTOR'S FAILING TO GIVE SUCH AN ADVANCED NOTICE, HE SHALL BE HELD RESPONSIBLE FOR THE RESULTS OF ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME.

5. THE CONTRACTOR SHALL USE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS TOGETHER WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS AND SHOP DRAWINGS, TO LOCATE DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, BOLT SETTING, SLEEVES, DIMENSIONS, ETC. NOTIFY ARCHITECT/ENGINEER, IN WRITING, OF ANY POTENTIAL CONFLICTS BEFORE PROCEEDING WITH THE WORK.

SHOP DRAWINGS AND DELEGATED ENGINEERING:

1. ALL SHOP DRAWINGS SHALL BE SUBMITTED FOR ARCHITECT'S REVIEW ONLY AFTER THEY HAVE BEEN THOROUGHLY REVIEWED BY THE CONTRACTOR FOR CONSTRUCTION METHODS, DIMENSIONS AND OTHER TRADE REQUIREMENTS, AND STAMPED WITH THE CONTRACTOR'S APPROVAL STAMP. THE ARCHITECT ASSUMES NO RESPONSIBILITY FOR DIMENSIONS, QUANTITIES, ENGINEERING DESIGN BY DELEGATED ENGINEERS, ERRORS OR OMISSIONS AS A RESULT OF REVIEWING ANY SHOP DRAWINGS. ANY ERRORS OR OMISSIONS MUST BE MADE GOOD BY THE CONTRACTOR, IRRESPECTIVE OF RECEIPT, CHECKING OR REVIEW OF DRAWINGS BY THE ENGINEER AND EVEN THOUGH WORK IS DONE IN ACCORDANCE WITH SUCH DRAWINGS.

2. BEFORE STRUCTURAL INSPECTIONS CAN BE MADE ON A PORTION OF THE STRUCTURE, ALL RELATED SHOP DRAWINGS, DELEGATED ENGINEERING, PRODUCT APPROVAL, MANUFACTURER'S DATA AND OTHER RELATED INFORMATION, MUST BE REVIEWED AND ACCEPTED BY THE ARCHITECT-OF-RECORD AND APPROVED BY THE BUILDING DEPARTMENT.

3. SHOP DRAWINGS SHALL CONTAIN ALL INFORMATION SHOWN ON THE STRUCTURAL PLANS (RELATED TO THE DELEGATED DESIGN) INCLUDING ALL DESIGN LOADS, IN ADDITION TO THE INFORMATION REQUIRED BY THE DELEGATED ENGINEER'S DESIGN.

4. ARCHITECT WILL REVIEW ALL SUBMITTED SHOP DRAWINGS, PREPARED AND SIGNED AND SEALED BY THE CONTRACTOR'S DELEGATED ENGINEER, ONLY FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT, REQUIRED LOADING AND COORDINATION WITH THE STRUCTURAL DESIGN.

5. CONTRACTOR SHALL SUBMIT TO THE ARCHITECT TWO SETS OF BLUE PRINTS OF THE STRUCTURAL SHOP DRAWINGS FOR ARCHITECT REVIEW, BEFORE STARTING FABRICATION. THE ARCHITECT WILL RETURN ONE MARKED UP AND STAMPED COPY TO THE CONTRACTOR. THE MARKED-UP COPY SHALL BE USED TO MAKE THE PRINTS REQUIRED FOR SHOP DRAWING DISTRIBUTION.

CONSTRUCTION MEANS AND METHODS:

1. THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCE OR PROCEDURES, SAFETY PRECAUTIONS, SHORES, RESHORES, LATERAL BRACING AND PROGRAMS IN CONNECTION WITH THE PROJECT, ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. OUR SERVICES DO NOT GUARANTEE NOR ASSURE LIABILITY FOR THE JOB SAFETY, TEMPORARY SHORING AND BRACING AND THE PERFORMANCE OF THE CONTRACTOR.

2. THE CONTRACTOR IS RESPONSIBLE AND SHALL COMPLY WITH THE SAFETY REQUIREMENTS OF THE 2010 FLORIDA BUILDING CODE AND APPLICABLE LOCAL, STATE AND FEDERAL LAWS.

3. PROVIDE ALL SHORING, BRACING AND SHEETING AS REQUIRED FOR SAFETY, STRUCTURAL STABILITY AND FOR THE PROPER EXECUTION OF THE WORK. REMOVE WHEN WORK IS COMPLETED.

4. PROVIDE AND MAINTAIN GUARD LIGHTS AT ALL BARRICADES, RAILINGS, OBSTRUCTIONS IN THE STREETS, ROADS OR SIDEWALKS AND ALL TRENCHES OR PITS ADJACENT TO PUBLIC WALKS OR ROADS.

5. AT ALL TIMES, PROVIDE PROTECTION AGAINST WEATHER (RAIN, WIND, STORMS OR THE SUN), SO AS TO MAINTAIN ALL WORK, MATERIALS, APPARATUS AND FIXTURES FREE FROM INJURY OR DAMAGE.

6. AT THE END OF THE DAYS WORK, COVER ALL WORK LIKELY TO BE DAMAGED. ANY WORK DAMAGED BY FAILURE TO PROVIDE PROTECTION SHALL BE REMOVED AND REPLACED WITH NEW WORK AT THE CONTRACTOR'S EXPENSE.

7. THE CONTRACTOR SHALL PAY FOR ALL DAMAGES TO ADJACENT STRUCTURES, SIDEWALKS AND TO STREETS OR OTHER PUBLIC PROPERTY OR PUBLIC UTILITIES.

UNDATIONS: (SPREAD FOOTINGS)

I FOUNDATIONS ARE DESIGNED TO BEAR ON WELL COMPACTED ADE OR CLEAN FILL OF AN ALLOWABLE BEARING CAPACITY OF 1,000 PSF NIMUM. FOR REQUIRED SOIL BEARING CAPASITIES GREATER THAN 1,000 F, A CERTIFIED TESTING LABORATORY SHALL BE ENGAGED BY THE NER TO VERIFY THAT THE REQUIRED BEARING CAPACITY WAS OBTAINED. ED SOIL CAPACITY SHALL BE CERTIFIED AND TESTED BY A FLORIDA GISTERED FOUNDATION ENGINEER, PRIOR TO CASTING OF CONCRETE IN E FOOTINGS.

: NATURAL GRADE (OR FILL) BELOW FOOTINGS SHALL BE MPACTED TO 98 % MODIFIED PROCTOR (ASTM D-1557).

: TOP OF WALL FOOTINGS TO BE AT THE SAME ELEVATION AS TOP OF LUMN PAD FOOTINGS. STEP WALL FOOTING FROM HIGHER COLUMN FOOTING THE LOWER ONE (AS DETAILED ON THE PLANS).

, BOTTOM OF ALL FOOTINGS TO BE A MINIMUM 1'-6" BELOW THE TOP OF NCRETE SLAB ON GRADE (UNLESS OTHERWISE NOTED) OR MINIMUM 1'-0" LOW FINISHED GRADE, WHICHEVER IS LOWER. IN THE EVENT THAT THE SLAB EPS ON EACH SIDE OF THE FOOTING, THE FOOTING SHALL BE 1'-6" BELOW TOP THE LOWER SLAB.

! REINFORCING IN THE CONTINUOUS WALL FOOTINGS (MONOLITHIC D NON-MONOLITHIC) SHALL BE SPLICED 40 BAR DIAMETERS MINIMUM AND ALL EXTEND CONTINUOUSLY THRU ALL FOOTING PADS.

(ALL LONGITUDINAL REBARS IN THE CONTINUOUS WALL FOOTINGS, ALL BE CONTINUED AT BENTS AND CORNERS BY BENDING THE REBARS 48 IR DIAMETERS AROUND THE CORNERS OR ADDING MATCHING CORNER BARS, ITENDING 48 BAR-DIAMETERS INTO FOOTING EACH SIDE OF CORNER OR BENT.

' ALL FOOTINGS SHALL BE 12" MINIMUM THICKNESS.

! WHEN GEO-TECHNICAL REPORTS ARE PROVIDED, ALL RECOMENDATIONS THE SOILS ENGINEER SHALL BE FOLLOWED AND THE DESIGN SOIL BEARING ESSURE SHALL BE AS RECOMMENDED IN SUCH REPORTS, AND SUPERCEEDS ESSURES INDICATED HEREIN.

NCRETE SLABS ON GRADE:

I ALL INTERIOR AND EXTERIOR SLABS AND WALKWAYS AS SHOWN THE STRUCTURAL OR ARCHITECTURAL PLANS, SHALL BE FOUR INCHES ICK MINIMUM REINFORCED WITH 6 X 6 - W1.4 X W1.4 WELDED WIRE FABRIC NLESS OTHERWISE NOTED).

: ALL SLABS ON GRADE TO BE CONSTRUCTED IN ACCORDANCE WITH ITEST A.C.I - "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION" (A.C.I. 302.1R)

: JOINTS SHALL BE PROVIDED IN ALL INTERIOR SLABS ON GRADE AT C. INDICATED ON THE PLANS DIVIDING THE SLAB INTO SQUARE PANELS NOT TO CEED 20 X 20 FT. IN SIZE. CAST SLAB IN LONG ALTERNATE STRIPS. PROVIDE CONTRACTION JOINT BETWEEN EACH STRIP. SEE PLAN FOR SAW-CUT, NTRACTION AND ISOLATION JOINT DETAILS.

. PROVIDE SAW-CUT JOINTS AT ALL SIDEWALKS AT A MAXIMUM 'ACING OF FIVE FEET ON CENTERS AND ISOLATION JOINTS AT 20 FEET O.C. .O.N.).

! FILL MATERIAL SHALL BE PLACED IN LIFTS NOT EXCEEDING 12" D COMPACTED TO 98 % MODIFIED PROCTOR (ASTM D-1557) WITHIN A STANCE OF 3 FEET BEYOND ALL FOOTING EDGES. TAKE AT LEAST ONE NSITY TEST FOR EACH 1,600 SQ.FT. OF AREA AND 12" BELOW SURFACE. SEND ISULTS OF THE TEST TO OWNER, ARCHITECT AND ENGINEER.

NCRETE AND REINFORCING:

I CONCRETE DESIGN AND REINFORCEMENT IN ACCORDANCE WITH 'ILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE' (A.C.I. 318 - TEST EDITION) AND WITH 'DETAILS AND DETAILING OF CONCRETE INFORCEMENT' - (A.C.I. 315 - LATEST EDITION).

: ALL CONCRETE WORK IN ACCORDANCE WITH "SPECIFICATIONS FOR 'RUCTURAL CONCRETE FOR BUILDING" (A.C.I. 301 - LATEST EDITION). ODUCTION OF CONCRETE, DELIVERY, PLACING AND CURING TO BE IN CORDANCE WITH "HOT WEATHER CONCRETING" (A.C.I. 305R - LATEST ITION).

: ALL CONCRETE TO BE REGULAR WEIGHT WITH A DESIGN STRENGTH 3,000 P.S.I. AT 28 DAYS. MAXIMUM SLUMP 5".

. ALL REINFORCING TO BE NEW BILLET STEEL CONFORMING TO THE TEST A.S.T.M. A-615 GRADE 60, FABRICATED IN ACCORDANCE WITH C.R.S.I. NUAL OF STANDARD PRACTICE AND PLACED IN ACCORDANCE WITH A.C.I. 315 D C.R.S.I. MANUAL OF STANDARD PRACTICE.

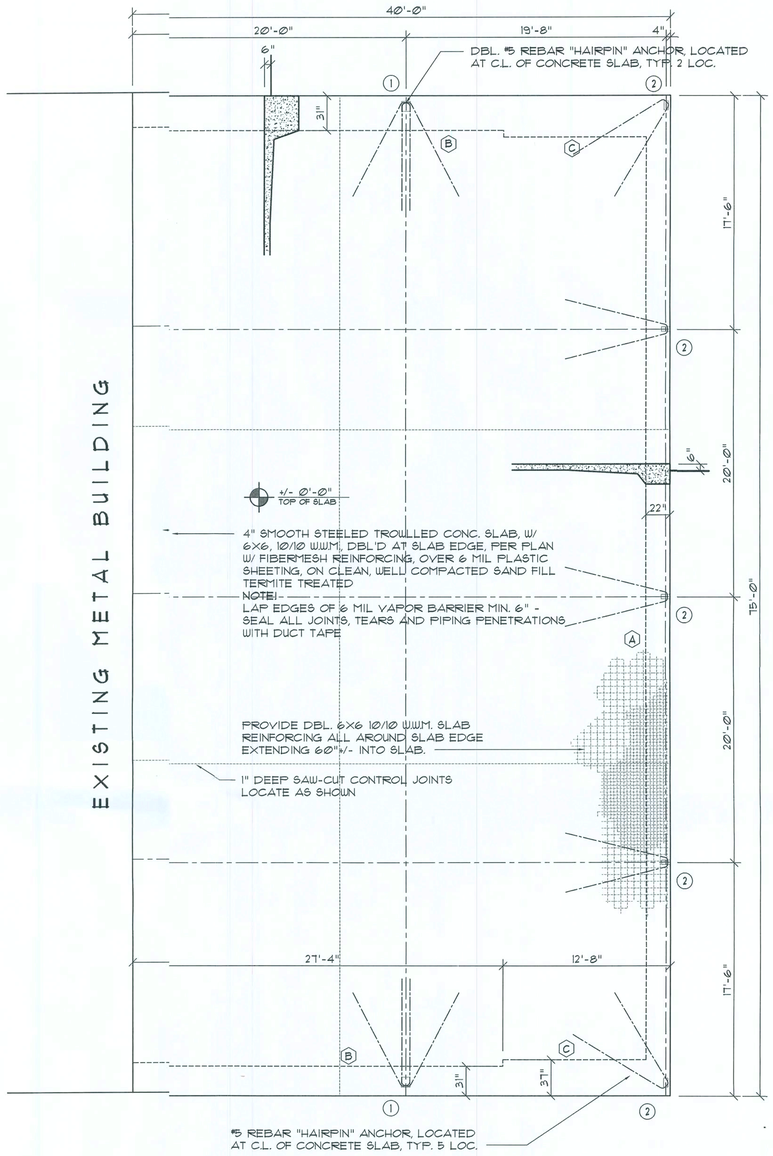
! CONCRETE COVER UNLESS OTHERWISE DETAILED ON DRAWINGS:

OTINGS:	(BOTTOM). . . . . 3"
	(TOP & SIDES) . . . . . 2"
ABS ON GRADE:	CENTERED W/SLAB

BEAM REINFORCEMENT: LAPPED 36 BAR DIAMETER OR MINIMUM 18 CHES. BOTTOM BARS SPLICED ONLY AT SUPPORTS, TOP BARS SPLICED LY AT MID-SPAN. ALL TOP BARS HOOKED AT NONCONTINUOUS EDGES .O.N.). ALL HOOKS TO BE STANDARD 90 DEGREE HOOKS AS REQUIRED .O.N.).

' ADDED REINFORCEMENT: PROVIDE ADDITIONAL CORNER BARS INT 36 INCHES MINIMUM EACH WAY AT "L" AND "T" CORNERS IN OUTER FACES (ALL BEAMS TO MATCH ALL HORIZONTAL BAR (TOP, BOTTOM AND 'ERMEDIATE REBARS).

E SEE PLAN FOR MINIMUM SIZE CONCRETE TIE BEAM REQUIREMENTS.



ANCHOR BOLT // FOUNDATION SIZING:

THE ANCHOR BOLT DIAMETERS A AND DEVELOPED LENGTHS INDICATED IN THIS DRAWING WERE DETERMIN NED USING SHEAR FRICTION THEORY AS DESCRIBED IN AISC DESIGN GUI IDE NO.1, SECTION 9.2, ASSUMING AN ANCHOR BOLT MATERIAL OF AS TM A307 OR A36. THE COMBINED FORCES ACTING AT THE BASE OF THE ST EEL FRAME RESULTING IN A VERTICAL REACTION ACTING UPON THE FO UNDATION WERE DEVELOPED AS FOLLOWS:

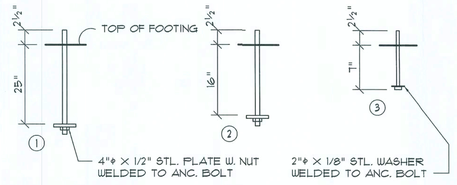
T = Td + Tsf

WHERE
T = TOTAL TENSILE FORCE PE ER BOLT
Td = TENSILE FORCE PER BOL LT DUE TO DIRECTLY APPLIED LOAD = P/N
Tsf = TENSILE FORCE PER BOL LT DUE TO SHEAR FRICTION = V / (n X u)

WHERE
P = P = TOTAL UPLIFT TO BE R RESISTED BY ANCHOR BOLT GROUP
V = V = TOTAL SHEAR FORCE E TO BE RESISTED BY ANCHOR BOLT GROUP
n = n = NUMBER OF ANCHOR B BOLTS
u = u = COEFFICIENT OF FRICT TION (TAKEN AS Ø.1 FOR UNGROUTED BASE PLATES OR Ø.3 FOR GROUTED BASE PLATES)

Foundation PLAN

SCALE: 3/16" = 1'-0"



NOTE!
ALL ANCHOR BOLTS ARE ASTM GRADE A36 STEEL ROD, THREADED 3", OR GRADE A307, BLACK, AND FREE FROM RUST AND SCALE

NOTE!
ALL DOOR/ENTRY OPENINGS INDICATED ARE NET SIZE, AND REQUIRE 2 #5 ANCHOR BOLTS AT EACH SIDE OF THE OPENING REFER TO METAL BUILDING SHOP DRAWINGS FOR DETAIL

Anchor Bolt DETAILS

SCALE: NONE

1 2

STRUCTURAL DESIGN CRITERIA:

1. THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE 2020 FLORIDA BUILDING CODE - SECTION 1609 AND OTHER REFERENCED CODES AND SPECIFICATIONS. ALL CODES AND SPECIFICATIONS SHALL BE LATEST EDITION AT TIME OF PERMIT.

2. WIND LOAD CRITERIA: RISK CATAGORY: 2, EXPOSURE "C"

BASED ON ANSI/ASCE 7-10. 2020 FBC 1609-A WIND VELOCITY: V_{ULT} = 130 MPH, V_{ASD} = 102 MPH

3. ROOF DESIGN LOADS:
SUPERIMPOSED DEAD LOADS: . . . . . 20 PSF
SUPERIMPOSED LIVE LOADS: . . . . . 20 PSF

4. FLOOR DESIGN LOADS:
SUPERIMPOSED DEAD LOADS: . . . . . 25 PSF
SUPERIMPOSED LIVE LOADS:
COMMERCIAL . . . . . 100 PSF
BALCONIES/CORRIDORS . . . . . 80 PSF

5. WIND NET UPLIFT: ARE AS INDICATED ON PLANS

BUILDING COMPONENTS & CLADDING LOADS
MEAN BUILDING HEIGHT = 30.0', EXPOSURE "B"

	ZONE	AREA	Vult 110 MPH	Vult 120 MPH	Vult 130 MPH	Vult 140 MPH
ROOF 7° TO 27°	1	10	12.0 / -19.9	14.9 / -23.7	17.5 / -27.8	20.3 / -32.3
	1	20	11.4 / -19.4	13.6 / -23.0	16.0 / -27.0	18.5 / -31.4
	1	50	10.0 / -18.6	11.9 / -22.2	13.9 / -26.0	16.1 / -30.2
	2	10	12.5 / -34.7	14.9 / -41.3	17.5 / -48.4	20.3 / -56.2
	2	20	11.4 / -31.9	13.6 / -38.0	16.0 / -44.6	18.5 / -51.7
	2	50	10.0 / -28.2	11.9 / -33.6	13.9 / -39.4	16.1 / -45.7
	3	10	12.5 / -51.3	14.9 / -61.0	17.5 / -71.6	20.3 / -83.1
	3	20	11.4 / -47.9	13.6 / -57.1	16.0 / -67.0	18.5 / -77.7
	3	50	10.0 / -43.5	11.9 / -51.8	13.9 / -60.8	16.1 / -70.5
WALL	4	10	21.8 / -23.6	25.9 / -34.7	30.4 / -33.0	35.3 / -38.2
	4	20	20.8 / -22.6	24.7 / -26.9	29.0 / -31.6	33.7 / -36.7
	4	50	19.5 / -21.3	23.2 / -25.4	27.2 / -29.8	31.6 / -34.6
	5	10	21.8 / -29.1	25.9 / -34.7	30.4 / -40.7	35.3 / -47.2
	5	20	20.8 / -27.2	24.7 / -32.4	29.0 / -38.0	33.7 / -44.0
	5	50	19.5 / -24.6	23.2 / -29.3	27.2 / -34.3	31.6 / -39.8

HEIGHT & EXPOSURE ADJUSTMENT COEFFICIENTS FOR BUILDING COMPONENTS & CLADDING

BLDG HEIGHT	EXPOSURE "B"	EXPOSURE "C"	EXPOSURE "D"
15	1.00	1.21	1.47
20	1.00	1.29	1.55
25	1.00	1.35	1.61
30	1.00	1.40	1.66

NOTE!
REFER TO THE METAL BUILDING SHOP DRAWINGS AS PER "METALLIC BUILDING SYSTEMS, INC.", FOR EXACT LOCATION OF ALL EMBEDDED ANCHOR BOLTS.

NOTE!
ADDED FILL SHALL BE APPLIED IN 12" LIFTS - EA. LIFT SHALL BE COMPACTED TO 98% DRY COMPACTION PER THE "MODIFIED PROCTOR" METHOD.

NOTE!
THE DESIGN WIND SPEED FOR THIS PROJECT IS 130 MPH PER 2020 FBC 7th ED. AND LOCAL JURISDICTION REQUIREMENTS

NOTE!
ALL ANCHOR BOLTS ARE ASTM GRADE A36 STEEL ROD, THREADED 3 1/2", BLACK AND FREE FROM RUST AND SCALE

NOTE!
THIS PROJECT IS TYPE 5 UNPROTECTED CONSTRUCTION PER 2014 FBC TABLE 503 AND TABLE 602

COLUMBIA COUNTY BUILDING DEPARTMENT
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PLANS EXAMINER

FOOTING SCHEDULE

(A) 22" X 18" X CONTINUOUS, FOOTING, W/ 2 #5 REBAR, TOP & BOTTOM, CONT., LAP SPLICE ALL REBAR A MINIMUM OF 30 BAR DIAMETERS - TYPICAL

(B) 31" X 31" X CONTINUOUS, FOOTING, W/ 5 #5 REBAR, TOP & BOTTOM, CONT., LAP SPLICE ALL REBAR A MINIMUM OF 30 BAR DIAMETERS - BATTER SIDES OF FOOTING MIN. 15° - SEE SLAB DETAIL, BELOW

(C) 37" X 37" X 12'-6", FOOTING, W/ 6 #5 REBAR, TOP & BOTTOM, CONT., LAP SPLICE ALL REBAR A MINIMUM OF 30 BAR DIAMETERS - BATTER SIDES OF FOOTING MIN. 15° - SEE SLAB DETAIL, BELOW

REVISION:

DRAWN:
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METAL BUILDING FOUNDATION for:
DAVID SMQUE CONSTRUCTION
DIY LETTERING, LAKE CITY, FLORIDA
FOUNDATION PLAN

Celebrating 49 Years of Architecture
1972 - 2021
N.P. Geisler, Architect
AR0007005

NICHOLAS GEISLER ARCHITECT
N.C.A.R.B. Certified
1758 NW Brown Rd.
Lake City, FL 32055
386-365-4355

DATE:
:7 OCT 2021
COMM:
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1 OF 1

Digitally signed by: N. P. GEISLER
DN: CN = N. P. GEISLER O = US
O = AR0007005 OU = ARCHITECT
Date: 2021.11.07 10:49:45 -05'00'

AR0007005

Builder/Contractor Responsibilities

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (April 2010 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (April 2010 Section 7.10.3).

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (April 2010 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design

The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

METALLIC BUILDING SYSTEMS

7301 FAIRVIEW • HOUSTON, TEXAS • P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

For questions regarding the interpretation of the drawings, materials provided, or assembly of the parts:
- Call 1-844-840-4603 and ask for the "Field Service" department.
- Before or after normal hours, you may send an email to fieldservices@ncigroup.com, Please include the order no., brief description of the question, & contact name and phone number.

ENGINEERING DESIGN CRITERIA

Building Code	2020 FLORIDA BUILDING CODE 7TH EDITION / 2018 IBC
Building Risk Category	Normal (Risk Category II)
Roof Dead Load	
Superimposed	2.33 psf
Collateral (5.00 psf Other)	5.00 psf
Roof Live Load	20.00 psf reduction allowed

Wind

Ultimate Wind Speed (Vult)	120.00 mph
Nominal Wind Speed (Vasd)	92 mph (IBC section 1609.3.1)
Serviceability Wind Speed	73 mph
Ground Elevation Factor	1.00 (96 ft ASL)
Wind Exposure Category	B
Exposure Coefficient (MWFRS)	0.701
Exposure Coefficient (C & C)	0.585
Enclosure Classification	Enclosed Building
Internal Pressure Coef (GCpi)	0.18/-0.18

Wall Loads for components not provided by building manufacturer
Zone 5 Areas (within 4.00' of corner) 19.74 psf pressure -26.32 psf suction
Zone 4 Areas (away from corners) 19.74 psf pressure -21.38 psf suction
These values are the maximum values required based on a 10 sq ft area.
Components with larger areas may have lower wind loads.

Seismic

Seismic Importance Factor (Ie)	1.00		
Seismic Design Category	B		
Soil Site Class	D Stiff Soil		
Ss	0.086 g	Sds	0.092 g
S1	0.051 g	Sd1	0.082 g
Analysis Procedure	Equivalent Lateral Force		
Column Line	ALL	SWA/SWC	
Basic Force Resisting System	S.EWD	G	
Response Modification Coefficient (R)	3.00	1.25	
Seismic Response Coefficient (Cs)	0.03	0.074	
Design Base Shear in kips (V)	0.92	0.772	

Basic Structural System (From ASCE 7-16 Table 12.2-1)
H - Steel System not Specifically Detailed for Seismic Resistance
G - Cantilevered Column Systems

DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS..... BLDG-B

Roof Limits		Rafters	Purlins	Panels
Live	L/	180	150	60
Serviceability Wind	L/	180	180	60
Total Gravity	L/	120	120	60
Total Uplift	L/	N/A	N/A	60

Frame Limits		Sidesway	FBCC Sidesway
Live	H/	60	
Serviceability Wind	H/	60	
Seismic Drift	H/	40	40
FBCC Serviceability Wind	H/	N/A	100
Total Gravity	H/	60	
Service Seismic	H/	40	100

Wall Limits		Limit
Total Wind Panels	L/	60
Total Wind Girts	L/	90
Total Wind EW Columns	L/	120

The Service Seismic limit as shown here is at service level loads.

FLORIDA APPROVAL #

FL11819.5 PBR ROOF PANEL	FL11917.2 AVP WALL PANEL
FL22211 WALK DOORS	FL6964.3 DBCI DOOR 5000 SERIES

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the most recent edition of the RCSC Specification For Structural Joints Using ASTM A325 or A490 Bolts. Pre-tensioning methods, including turn-of-nut, calibrated wrench, twist-off-type tension-control bolts or direct-tension-indicator are NOT required. Installation inspection requirements for Snug Tight Bolts (Specification for Structural Joints Section 9.1) is suggested.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

This metal building system is designed as an Enclosed Building. Exterior and/or operable components including, but not limited to, doors, windows, vents, etc. ("Components") must be designed to withstand the required component and cladding wind pressures specified by the building code. In order to maintain the metal building system's Enclosed Building condition, all Components shall be closed when wind velocities reach half the designed wind load for the metal building system as shown on the drawings and design criteria documentation. Failure to maintain the metal building system's Enclosed Building condition will violate and void all warranties and certifications applicable to the material supplied by the metal building manufacturer.

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

Roof and wall panels have been designed in accordance with section 2222.4 of the Florida Building Code, 7th Edition (2020) Product approval numbers for the State of Florida, Department of Community Affairs per Product Rule 9B-72:
1. Panel Walls
   FL11917.2 AVP 26 gauge walls
2. Roofing Products
   FL11819.5 PBR 26 gauge roofs

Wall accesories have been designed in accordance with section 2222.4 of the Florida Building Code, 7th Edition (2020) Product approval numbers for the State of Florida, Department of Community Affairs per Product Rule 9B-72:
1. Doors
   FL17900.2 Telstar 3070, Wind-rated to +/- 50 psf, Non-Impact-rated
2. DBCI Overhead doors
   FL6964.3 Series 5000 Commercial Roll Up Door with wind locks

This project is designed using manufacture's standard serviceability standards. Generally this means that all deflections are within typical performance limits for normal occupancy and standard metal building products.

Using 5 x 5 eave gutter with 4 x 5 downspouts, the roof drainage system has been designed using the method outlined in the MBMA Metal Building Systems Manual. Downspout locations have not been located on these drawings. The downspouts are to be placed on the building sidewalls at a spacing not to exceed 28 feet with the first downspout from both ends of the gutter run within 14 feet of the end. Downspout spacing that does not exceed the maximum spacing will be in compliance with the building code. The gutter and downspout system as provided by the manufacturer is designed to accommodate 10 in/hr rainfall intensity.

The common wall at the existing building is to remain sheeted.

Drawing Index

Revision	Date	Description	By	Ck'd
A	08/23/21	FOR CONSTRUCTION PERMIT	BSL	KSK

METALLIC BUILDING SYSTEMS
7301 FAIRVIEW • HOUSTON, TEXAS • P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY, FL 32024-0693 US

Customer:
SIMQUE CONSTRUCTION
PO BOX 2962
LAKE CITY, FL 32056-2962 US
DAVID SIMQUE

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	BSL 8/23/21
Checked by:	KSK 8/23/21
Project Engineer:	MJ
Job Number:	18-B-30506
Sheet Number:	E1 of 9

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

Luke KE HU LICENSE No.88271 STATE OF FLORIDA PROFESSIONAL ENGINEER

Download panel installation manuals from:
www.CBBmanuals.com

Descargue los manuales de instalación del panel desde:
www.CBBmanuals.com

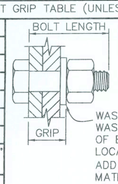
BUILDING DESCRIPTIONS

Building ID	Width	Length	Height	Slope
Building B	75'-0	40'-0	16'-0	1:12

1" ø A325 BOLT GRIP TABLE (UNLESS NOTED)

GRIP	LENGTH
0 TO 9/16"	1 1/4" F.T.
over 9/16" TO 1 1/16"	1 3/4" F.T.
over 1 1/16" TO 1 5/16"	2"
over 1 5/16" TO 1 9/16"	2 1/4"
over 1 9/16" TO 1 13/16"	2 1/2"
over 1 13/16" TO 2 1/16"	2 3/4"

LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS
F.T. DENOTES FULLY THREADED



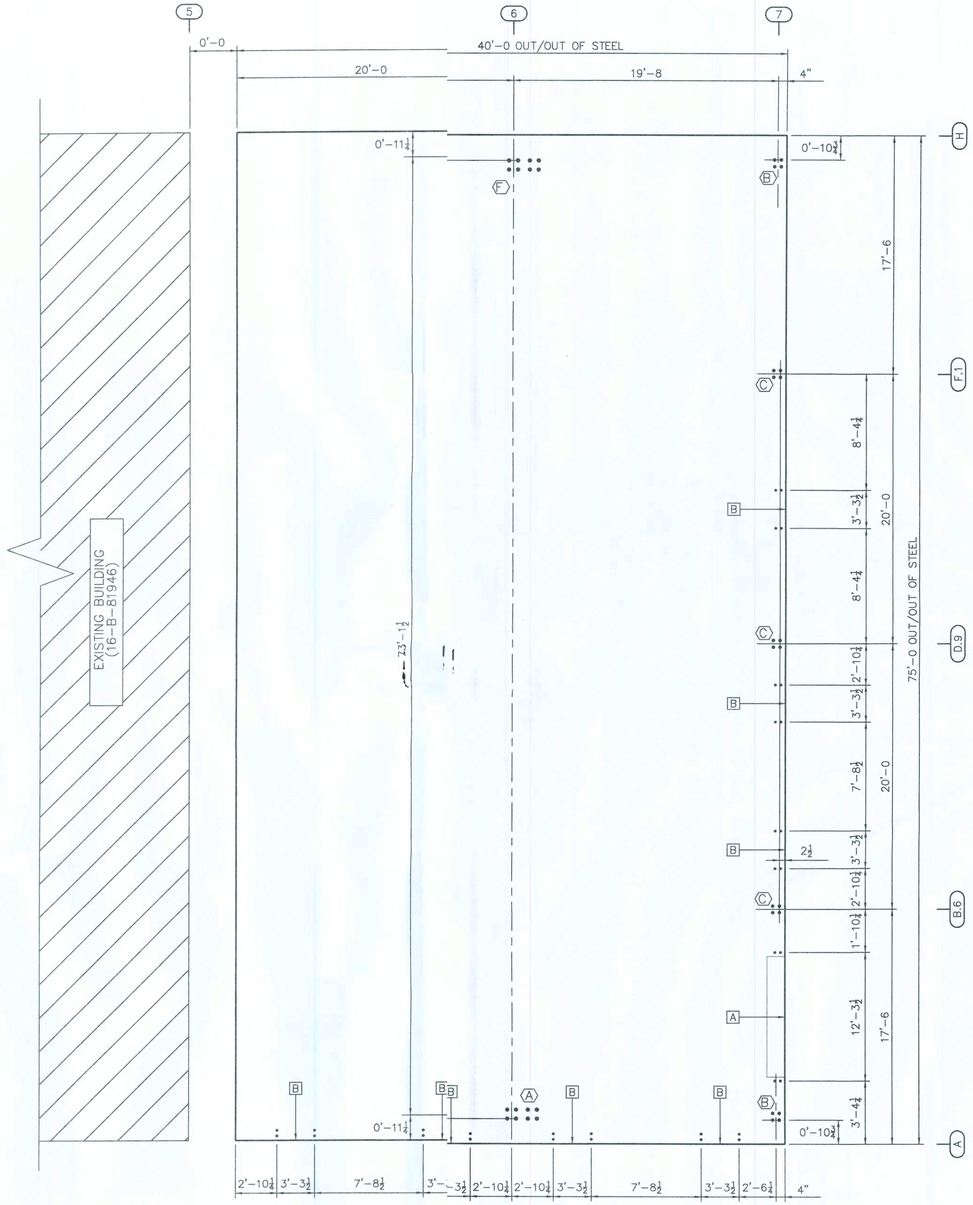
NOTE: FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

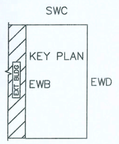
WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

Anchor Rod Drawings

1) This drawing is for anchor rod placement only and is not foundation design.
2) Foundation must be square and level with all anchor rods true in size, location, and projection.
3) Projection shown must be held to keep threads clear of finished concrete.
4) This structural design data includes magnitude and location of design loads and support conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
5) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer, and are not provided by the manufacturer.
6) Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise.
7) 3000 psi concrete compressive strength (f'c) is assumed for the purpose of column base plate design unless otherwise noted.

FINISH FLOOR AT ELEVATION 100'-0





ACCESSORY SCHEDULE			
MARK	DESCRIPTION	DETAIL	QUAN.
A	12'-0 X 12'-0 FRAMED OPENINGS	E	1
B	3'-0 X 5'-0 FRAMED OPENINGS	D	7

ANCHOR BOLTS TO BE DESIGNED BY FOUNDATION ENGINEER USING DIAMETERS SHOWN IN THIS TABLE.

ANCHOR ROD DESCRIPTION	QUANTITY
⅞" ø DIAMETER X	52
¾" ø DIAMETER X	16

ANCHOR ROD SETTING PLAN

Revision	Date	Description	By	Ck'd
0	08/23/21	FOR ERECTOR INSTALLATION	BSL	KSK

METALLIC BUILDING SYSTEMS

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Customer:
SIMOJE CONSTRUCTION
P.O. BOX 2062
LAKE CITY, FL 32056-2962 US
DAVID SIMOJE

Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY, FL 32024-0693 US

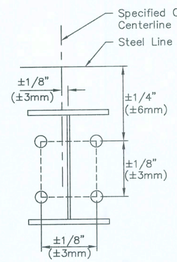
Drawing Status:
☐ Preliminary, Not For Construction
☐ For Approval, Not For Construction
☐ For Construction Permit
☒ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	BSL 8/23/21
Checked by:	KSK 8/23/21
Project Engineer:	MJ
Job Number:	18-B-30506
Sheet Number:	F1 of 3

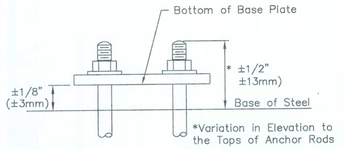
KE HU LICENSE No 88271 STATE OF FLORIDA PROFESSIONAL ENGINEER

Ke Hu

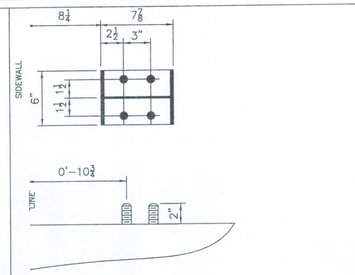
AISC CODE OF STANDARD PRACTICE TOLERANCES FOR SETTING ANCHOR RODS



ANCHOR ROD SETTING TOLERANCES

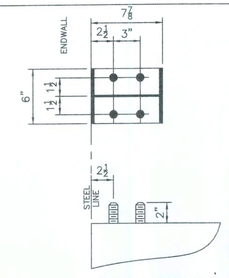


BASE PLATE AND ANCHOR ROD TOLERANCES



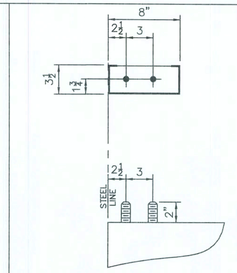
TYPICAL SECTION
⅝"ø ANCHOR RODS

(B) BASE OF STEEL AT ELEV. 100'–0



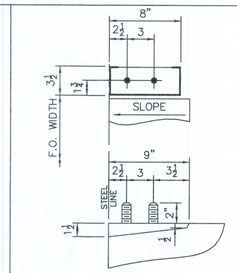
TYPICAL SECTION
⅝"ø ANCHOR RODS

(C) BASE OF STEEL AT ELEV. 100'–0



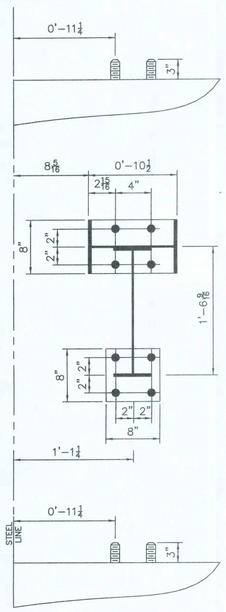
TYPICAL SECTION
⅝"ø ANCHOR RODS

(D) BASE OF STEEL AT ELEV. 100'–0



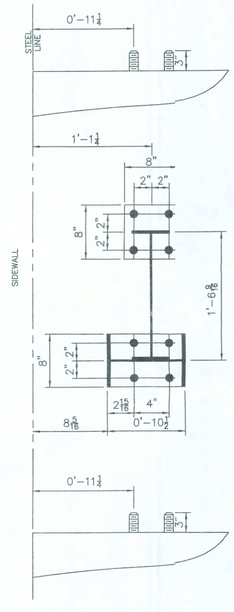
TYPICAL SECTION
⅝"ø ANCHOR RODS

(E) BASE OF STEEL AT ELEV. 100'–0



TYPICAL SECTION
¾"ø ANCHOR RODS

(A) BASE OF STEEL AT ELEV. 100'–0



TYPICAL SECTION
¾"ø ANCHOR RODS

(F) BASE OF STEEL AT ELEV. 10'–0

Revision	Date	Description	By	Ck'd
0	08/23/21	FOR ERECTOR INSTALLATION	BSL	KSK

METALLIC BUILDING SYSTEMS

7301 FAIRVIEW • HOUSTON, TEXAS • P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Customer:
SIMQUE CONSTRUCTION
P.O. BOX 2962
LAKE CITY, FL 32056-2962 US
DAVID SIMQUE

Project Name & Location:
215 LETTERING WIDTH SWEPT GLN
LAKE CITY, FL 32024-0693 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☐ For Construction Permit
☒ For Erector Installation

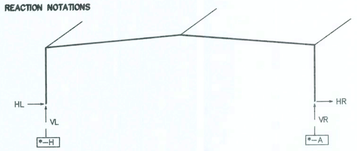
Scale:	NOT TO SCALE	
Drawn by:	BSL	8/23/21
Checked by:	KSK	8/23/21
Project Engineer:	MJ	
Job Number:	18-B-30506	
Sheet Number:	F2 of 3	

KE HU
LICENSE
No 88271
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

FRAME ID #1 | USER NAME: MJimenez | DATE: 8/16/21 | PAGE: 1-3
cs 75./16./24.792 20./120./ | JOB NAME: 30506B | FILE: frame_8.fra

SUPPORT REACTIONS FOR EACH LOAD GROUP
*LOCATION: Gridlines: 6 | TIME: 07:19:07
NOTES: (1) All reactions are in kips and kip-ft.
(2) Primary wind load cases are not concurrent.
(3) X-bracing reactions (RBPULW and RBUPEQ) are combined withLWL and LEQ groups only.

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = 6

COLUMN	*–H			*–A		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	2.6	3.4	0.0	−2.6	3.4	0.0
LL	9.6	11.2	0.0	−9.6	11.2	0.0
COLL	4.0	4.6	0.0	−4.0	4.6	0.0
EQ	−0.3	−0.1	0.0	−0.4	0.1	0.0
WL1	−15.2	−17.0	0.0	8.4	−11.9	0.0
WL2	−10.5	−9.7	0.0	3.7	−4.6	0.0
LWL1	−9.3	−16.0	0.0	9.8	−12.9	0.0
LWL2	−9.8	−12.9	0.0	9.3	−16.0	0.0
LWL3	−4.6	−8.7	0.0	5.2	−5.6	0.0
LWL4	−5.2	−5.6	0.0	4.6	−8.7	0.0
WL3	−8.4	−11.9	0.0	15.3	−17.0	0.0
WL4	−3.8	−4.6	0.0	10.6	−9.7	0.0

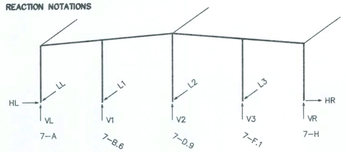
LOAD GROUP DESCRIPTION

DL	:	Roof Dead Load
LL	:	Roof Live Load
COLL	:	Roof Collateral Load
EQ	:	Lateral Seismic Load [parallel to plane of frame]
WL1	:	Wind from Left to Right with +GCpi
WL2	:	Wind from Left to Right with −GCpi
LWL1	:	Windward Corner Left with +GCpi
LWL2	:	Windward Corner Right with +GCpi
LWL3	:	Windward Corner Left with −GCpi
LWL4	:	Windward Corner Right with −GCpi
WL3	:	Wind from Right to Left with +GCpi
WL4	:	Wind from Right to Left with −GCpi

FRAME DESCRIPTION: Endwall END | USER NAME: MJimenez | DATE: 8/16/21 | PAGE: EW-1
PATH: R:\Jobs\Active\Eng\18-B-30506\ver02-mjimenez\BLDG-B\run01\ | JOB NAME: 30506B | FILE: REWABLD01

SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft. | TIME: 07:20:55

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

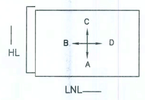
COLUMN	7–A			7–B.6			7–D.9			7–F.1			7–H		
LOAD GROUP	HL	VL	LL	H1	V1	L1	H2	V2	L2	H3	V3	L3	HR	VR	LR
D	0.0	0.4	0.	0.	0.9	0.	0.	0.7	0.	0.	0.9	0.	0.0	0.4	0.
C	0.0	0.3	0.	0.	1.2	0.	0.	0.8	0.	0.	1.2	0.	0.0	0.3	0.
L	0.0	1.4	0.	0.	4.6	0.0	0.	3.2	0.0	0.	4.6	0.0	0.0	1.4	0.
W+	−0.1	−2.5	1.1	0.	−7.3	2.4	0.	−4.9	2.8	0.	−7.3	2.4	0.1	−2.5	0.
W−	−0.1	−2.5	−1.3	0.	−7.3	−2.7	0.	−4.9	−3.1	0.	−7.3	−2.7	0.1	−2.5	0.
WR	1.4	−2.5	0.	0.	−7.3	0.0	0.	−4.9	0.0	0.	−7.3	0.0	0.1	−2.5	0.
WL	−1.2	−2.5	0.	0.	−7.3	0.0	0.	−4.9	0.0	0.	−7.3	0.0	0.1	−2.5	0.
W16+	0.	0.	0.	0.	0.	2.5	0.	0.	3.0	0.	0.	2.5	0.	0.	0.
E+	0.	0.	0.	0.	0.	0.0	0.	0.	0.1	0.	0.	0.0	0.	0.	0.
E−	0.	0.	0.	0.	0.	0.0	0.	0.	−0.1	0.	0.	0.0	0.	0.	0.

LOAD GROUP DESCRIPTION

D	:	Dead load
C	:	Collateral load
L	:	Live load
W+	:	Wind load as an inward acting pressure
W−	:	Wind load as an outward acting suction
WR	:	Wind force from the right
WL	:	Wind force from the left
W16+	:	Min. 16 psf wind as an inward acting pressure
E+	:	Seismic force acting inward
E−	:	Seismic force acting outward

SUPPORT REACTIONS FOR EACH LOAD Case
NOTES: (1) All reactions are in kips and kip-ft. | JOB NAME: 18-B-30506

REACTION NOTATIONS



Note:
VL reactions will create a couple resulting in a MOMENT (ML) to be transferred by the foundation. The foundation design professional shall consider these forces in the design of the foundation.

LOAD CASE REACTION TABLE

COLUMN	2		
LOAD Case	LNL	VL	ML
LEQ	1.10	10.48	15.99
LWL	4.53	43.07	65.73

Note: Reactions are provided based on longitudinal forces acting left to right only. The e value of the reactions are the same but opposite when forces act right to left thus is uplift is applicable at both sides of the fixed base cantilever column.

Primary Load Cases
- LEQ : Longitudinal Earthquake Load Case
- LWL : Longitudinal Wind Load Case

NOTES

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
- a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
- b) RIGID FRAMES
  - (1) GABLED BUILDINGS
    - (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
    - (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
  - (2) SINGLE SLOPE BUILDINGS
    - (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
    - (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
    - (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
- c) ENDWALLS
  - (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
  - (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
- d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
- e) ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
- f) X–BRACING
  - (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
  - (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X–BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Ω_0.
  - (3) FOR CANADA BUILDING CODE (NBC), WHEN X–BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, Rd, WHEN SPECIFIED SHORT–PERIOD SPECTRAL ACCELERATION RATIO $I_E F_a S_a(0.2)$ IS GREATER THAN 0.45.

3) REACTIONS ARE PROVIDED AS UN–FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
- a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
- b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.
- c) FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE R_d*R_o FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

Revision	Date	Description	By	Ck'd
0	08/23/21	FOR ERECTOR INSTALLATION	BSL	KSK

METALLIC BUILDING SYSTEMS
7301 FAIRVIEW • HOUSTON, TEXAS • P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

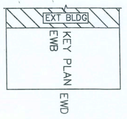
Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY , FL 32024-0693 US

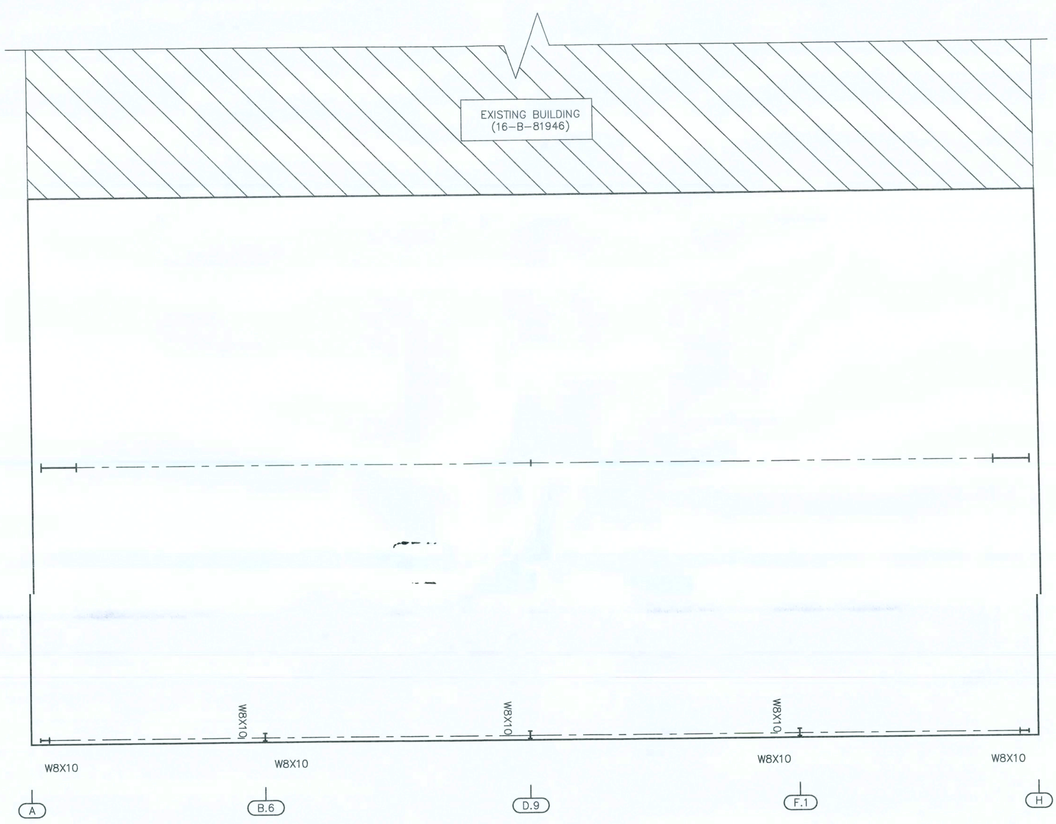
Customer:
SIMQUE CONSTRUCTION
PO BOX 2962
LAKE CITY , FL 32056-2962 US
DAVID SIMQUE

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☐ For Construction Permit
☒ For Erector Installation

Scale: NOT TO SCALE
Drawn by: BSL 8/23/21
Checked by: KSK 8/23/21
Project Engineer: MJ
Job Number: 18-B-30506
Sheet Number: F3 of 3

KE HU LICENSE No 88271 STATE OF FLORIDA PROFESSIONAL ENGINEER





PRIMARY STEEL LOCATION PLAN

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Sheet Number: E2 of 9

Job Number: 18-B-30506

Project Engineer: MJ

Checked by: KSK 8/23/21

Drawn by: BSL 8/23/21

Scale: NOT TO SCALE

METALLIC BUILDING SYSTEMS
7301 FAIRVIEW • HOUSTON, TEXAS • P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Customer:
SIMQUE CONSTRUCTION
P.O BOX 2962
LAKE CITY ,FL 32056-2962 US
DAVID SIMQUE

Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY ,FL 32024-0693 US

Drawing Status:
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Revision	Date	Description	By	Ck'd
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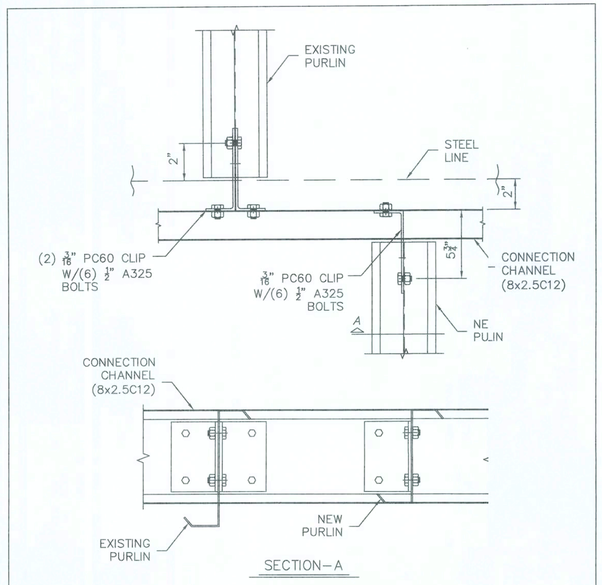
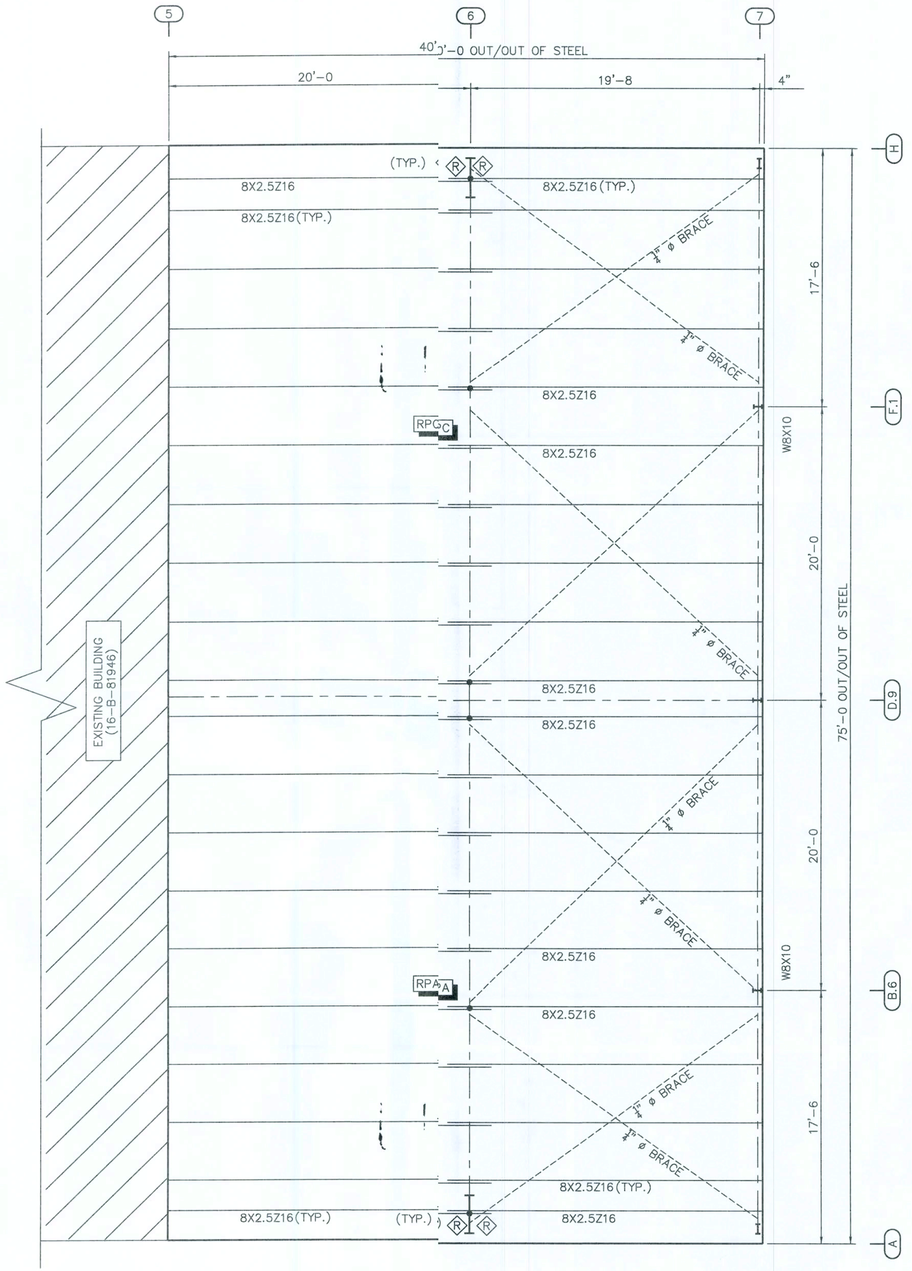
● – DENOTES: CLIP LOCATION
SC90 AT 8" PURLINS
SC92 AT 10" PURLINS
SC94 AT 12" PURLINS



NEW BUILDING TO EXISTING BUILDING PURLIN TIE IN CONNECTN WITH TRANSVERSE CHANNEL



ROOF FRAMING PLAN

SWC
KEY PLAN
EXT BLDG
EWB
EWD
SWA

ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
G	–0'–0¼"	T	2'–5¾"
S	0'–3¾"	V	3'–1¾"
R	1'–5¾"	REFER TO CF01122	

Revision	Date	Description	By	Ck'd
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METALLIC BUILDING SYSTEMS
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ZIP 77041 (713) 466–7788 ZIP 77240

Project Name & Location:
215 LETTERING
215 SW WINDSWEPT GLN
LAKE CITY, FL 32024–0693 US

Customer:
SIMQUE CONSTRUCTION
PO BOX 2962
LAKE CITY, FL 32056–2962 US
DAVID SIMQUE

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☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	BSL 8/23/21
Checked by:	KSK 8/23/21
Project Engineer:	MJ
Job Number:	18-B-30506
Sheet Number:	E3 of 9

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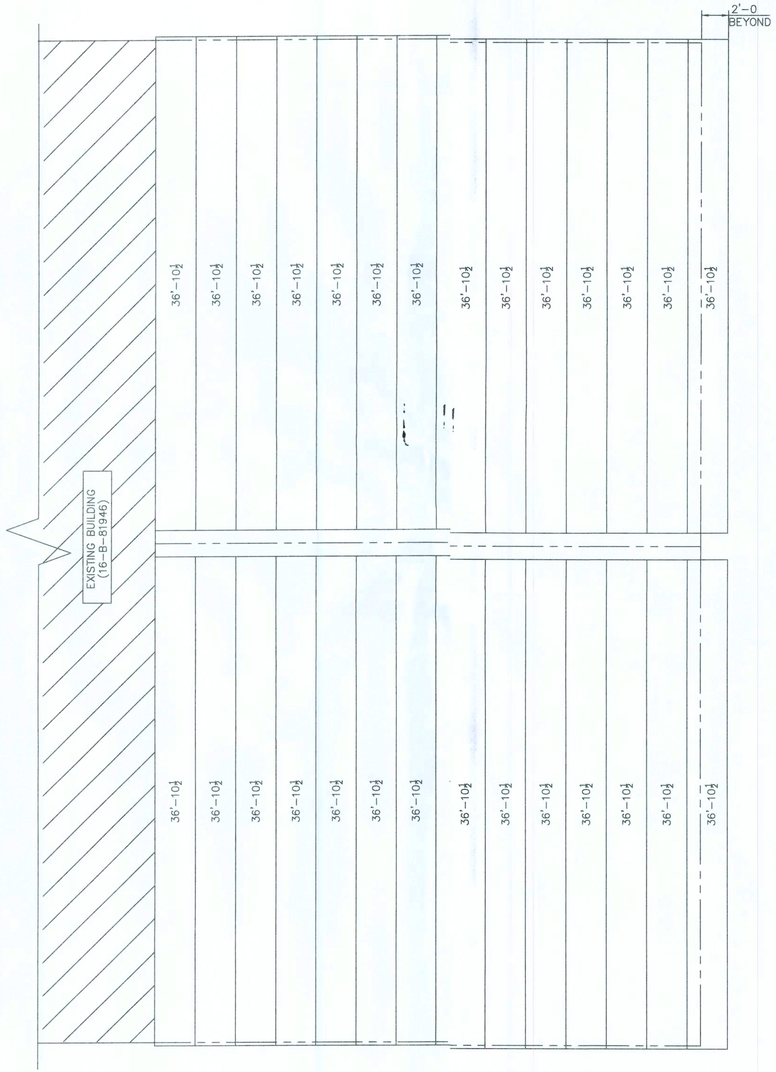
Non-Standard PBR Roof Panel Fasteners

#1E member fasteners are to be used for panel to secondary attachment in lieu of #3 shown on the R Drawings

PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.

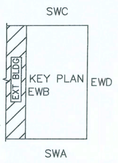
ROOF SHEETING PLANE 2
PANEL TYPE = PBR (POLAR WHITE)
PANEL OVERHANG = 3"
FROM OUTER STEEL



ROOF SHEETING PLAN

ROOF SHEETING PLANE 1
PANEL TYPE = PBR (POLAR WHITE)
PANEL OVERHANG = 3"
FROM OUTER STEEL

Note; The downspouts are to be placeon the building sidewalls at a spacing not to exceed28 feet with the first downspout from both ends of the gutt run within 14 feet of the end.



DOWNSPOUT LAYOUT
[4 REQ'D]

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Scale:	NOT TO SCALE
Drawn by:	BSL 8/23/21
Checked by:	KSK 8/23/21
Project Engineer:	MJ
Job Number:	18-B-30506
Sheet Number:	E4 of 9

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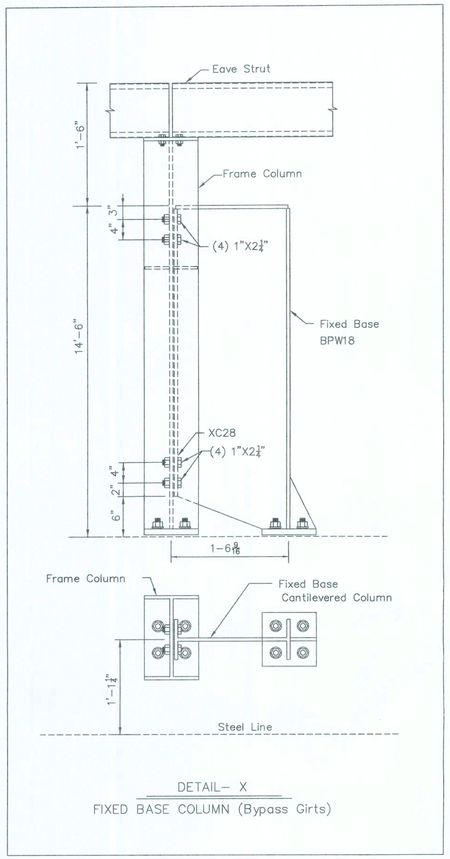
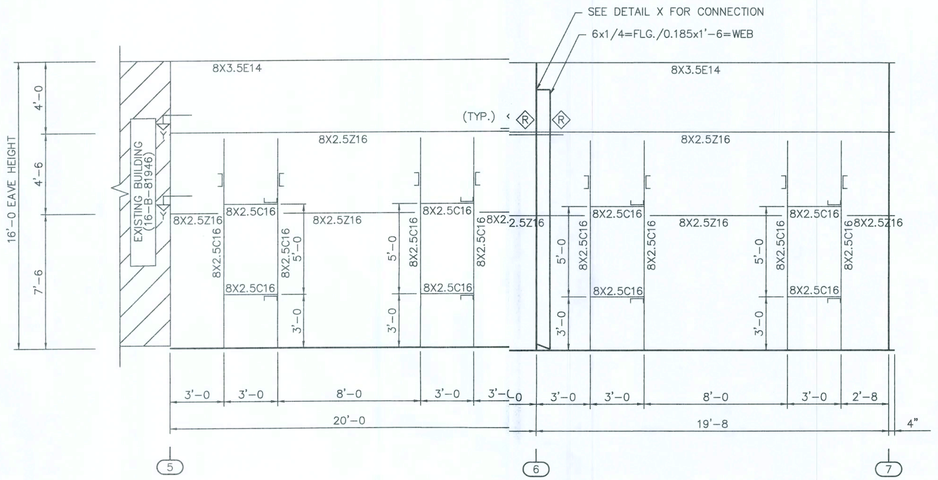
SCHEDULE OF ACCESSORIES

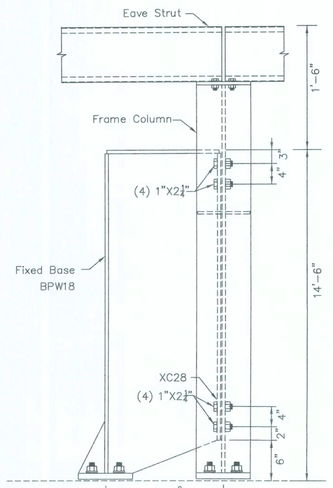
NO. REQD	DESCRIPTION
1	12'-0 X 12'-0 FACTORY LOCATED FRAMED OPENING
7	3'-0 X 5'-0 FACTORY LOCATED FRAMED OPENINGS
1	3070 KNOCK-DOWN WALK DOOR

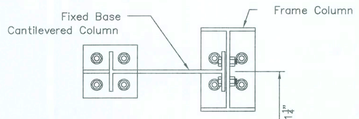
REFER TO DETAILS ON INSTALLATION OF WALK DOOR.
REFER TO DETAILS ON INSTALLATION OF FRAMED OPENINGS.
USE STANDARD WALL PROCEDURES TO ERECT THE SIDEWALL AND ENDWALL PANELS.



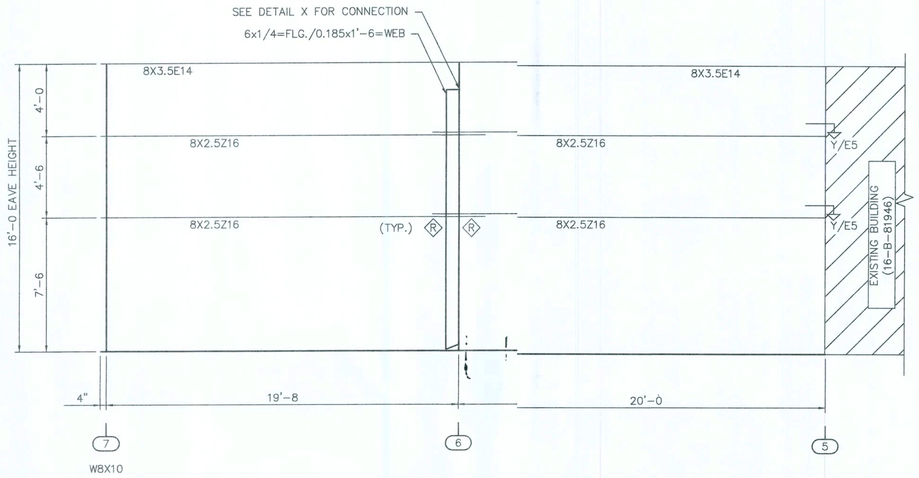
DETAIL- X
FIXED BASE COLUMN (Bypass Girts)


SEE DETAIL X FOR CONNECTION
6x1/4=FLG./0.185x1'-6=WEB
16'-0 EAVE HEIGHT
4'-0
4'-6
7'-6
EXISTING BUILDING (16-B-81946)
8X3.5E14
8X3.5E14
(TYP.)
8X2.5Z16
8X2.5Z16
8X2.5C16
5'-0
3'-0
3'-0
3



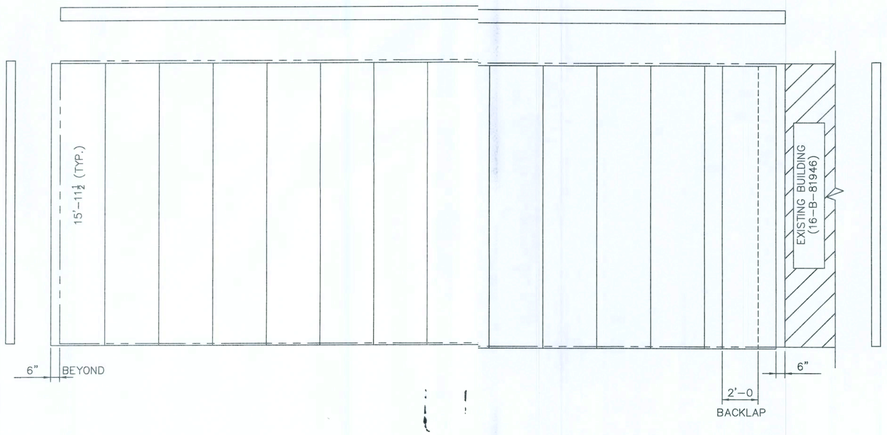


DETAIL– X
FIXED BASE COLUMN (Bypass Girts)



SIDEWALL ELEVATION "SWC" AT GRID LINE "H"

AVPVALL PANELS
PANEL COVTAGE = 3'-0
COLOR= ASH GRAY
PANEL PKG. RED. = AVS-3
Field Cut Pancand Trim as
required per Constction Details



WALL SHEETING ELEVATION "SWC"
BLDG "B"

ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
G	-0'-0½"	T	2'-5¾"
S	0'-3¾"	V	3'-1¾"
R	1'-5¾"	REFER TO CF01122	

Revision	Date	Description	By	Ck'd
A	08/23/21	FOR CONSTRUCTION PERMIT	BSL	KSK

METALLIC BUILDING SYSTEMS

7301 FARMVIEW • HOUSTON, TEXAS • P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Customer:
SIMQUE CONSTRUCTION
PO BOX 2962
LAKE CITY ,FL 32056-2962 US
DAVID SIMQUE

Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY ,FL 32024-0693 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: BSL 8/23/21
Checked by: KSK 8/23/21
Project Engineer: MJ
Job Number: 18-B-30506
Sheet Number: E6 of 9

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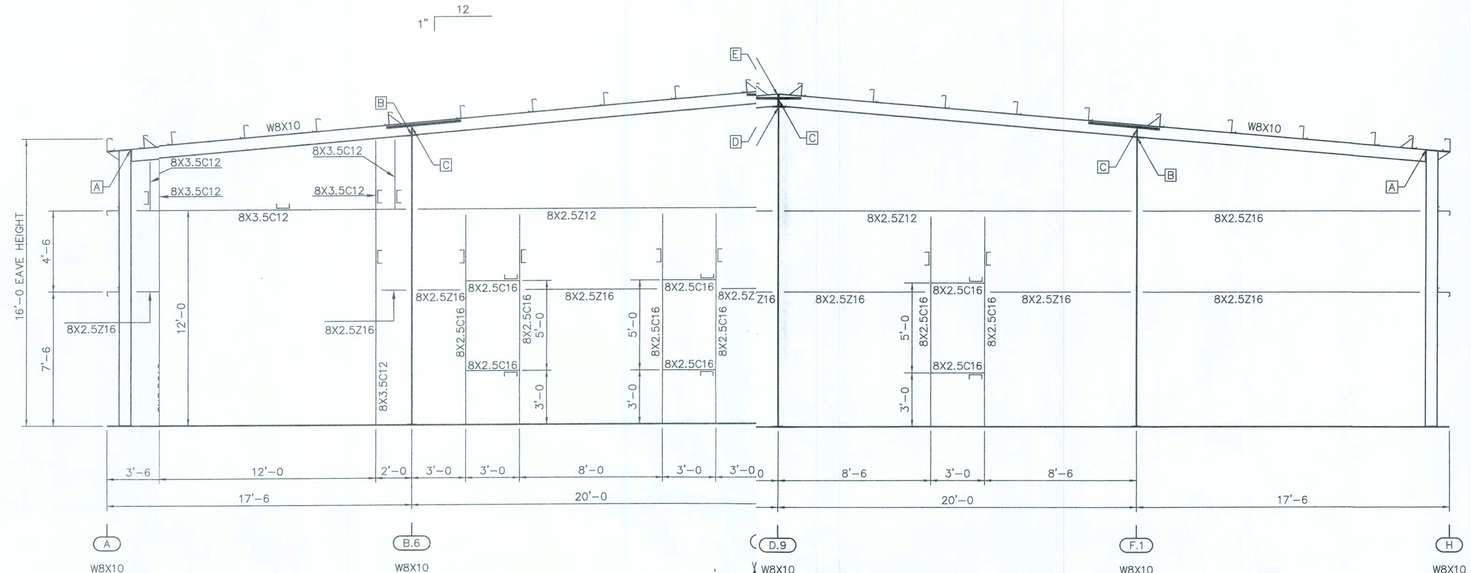
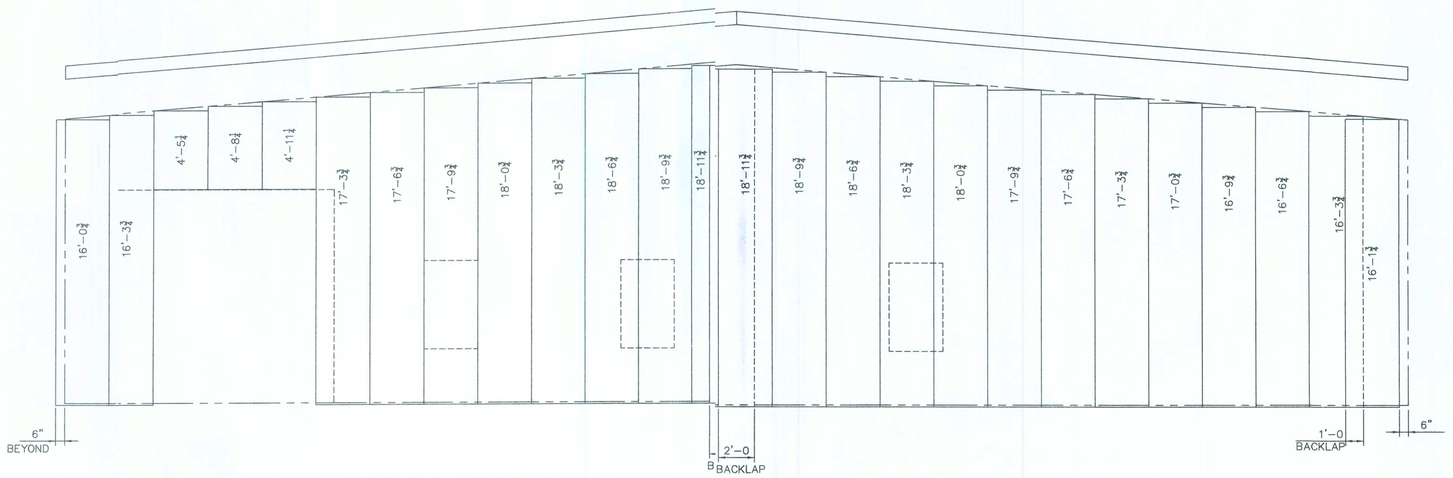
PROFESSIONAL ENGINEER

SPLICE BOLT TABLE				
CONN.	QTY. SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	(2) ½ X 1¾	A325 B&N	0	0
B	(4) ½ X 1¾	A325 B&N	4	0
C	(4) ½ X 1¾	A325 B&N	0	0
D	(4) ½ X 1¾	A325 B&N	2	0
E	(4) ½ X 1¾	A325 B&N	0	0

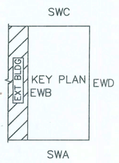


ENDWALL ELEVATION "EWD" AT GRID LINE "7"



WALL SHEETING ELEVATION "EWD"
BLDG "B"

AVP WALL PANELS
PANEL COVERAGE = 3'-0
COLOR = ASH GRAY
PANEL PKG. REQ'D. = AVS-1
Field Cut Panel and Trim as required per Construction Details



Revision	Date	Description	By	Ck'd
A	08/23/21	FOR CONSTRUCTION PERMIT	BSL	KSK

METALLIC BUILDING SYSTEMS
7301 FAIRVIEW • HOUSTON, TEXAS • P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY, FL 32024-0693 US

Customer:
SIMQUE CONSTRUCTION
PO BOX 2962
LAKE CITY, FL 32056-2962 US
DAVID SIMQUE

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE	
Drawn by:	BSL	8/23/21
Checked by:	KSK	8/23/21
Project Engineer:	MJ	
Job Number:	18-B-30506	
Sheet Number:	E7 of 9	

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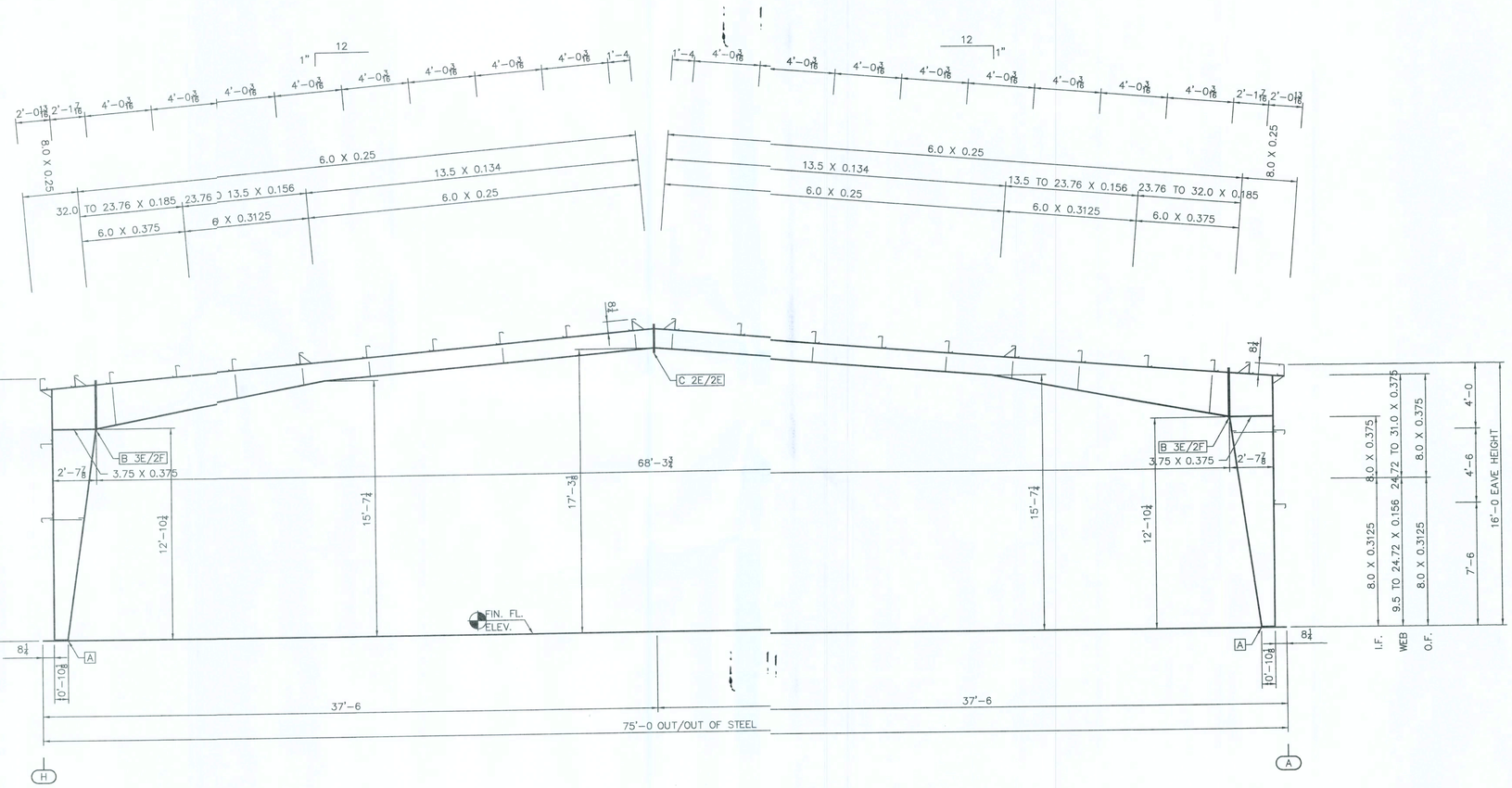
Y/PROJECTS\XDS-V8-17-01 FRAME = Active/Eng/18-B-30506/ver01-mjimenez/BLDG-B/Drftg/x01L Active/Eng/18-B-30506/ver01-mjimenez/BLDG-B/Drftg/x01R 8/16/21 07:19:07 8/16/21 07:19:07
Y/PROJECTS\XDS-V8-17-01 FB SET = Eng/18-B-30506/ver01-mjimenez/BLDG-B/Drftg/x01L Eng/-B-30506/ver01-mjimenez/BLDG-B/Drftg/x01R

GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND MAY VARY DUE TO CONDITIONS (DEFLECTION).

VERTICAL CLEARANCE DIMENSIONS ARE FROM FINISHED FLOOR REFERENCE ELEVATION.

** DENOTES: CLIPS AT FLANGE BRACE
CL196 & CL199 AT 8" PURLINS/GIRTS
CL197 & CL199 AT 10" PURLINS/GIRTS
CL198 & CL199 AT 12" PURLINS/GIRTS



CROSS SECTION AT FRAME LINE "6"

CONN.	PLATE SIZE TABLE		SPLICE BOLT TABLE			
	LOW SIDE	HIGH SIDE	QTY. SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	8 X 0.75 X 0'-10 1/8					
B	8 X 0.5 X 3'-0 15/16	6 X 0.5 X 3'-0 7/16	(10) 3/4 X 2"	A325 B&N	0	0
C	6 X 0.375 X 1'-8 3/16	6 X 0.375 X 1'-8 3/16	(8) 3/4 X 1 3/4	A325 B&N	0	0

Revision	Date	Description	By	Ck'd
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METALLIC BUILDING SYSTEMS
7301 FAIRVIEW • HOUSTON, TEXAS • P.O. BOX 40338
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Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY, FL 32024-0693 US

Customer:
SIMQUE CONSTRUCTION
PO BOX 2982
LAKE CITY, FL 32056-2962 US
DAVID SIMQUE

Drawing Status:
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☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	BSL 8/23/21
Checked by:	KSK 8/23/21
Project Engineer:	MJ
Job Number:	18-B-30506
Sheet Number:	E8 of 9

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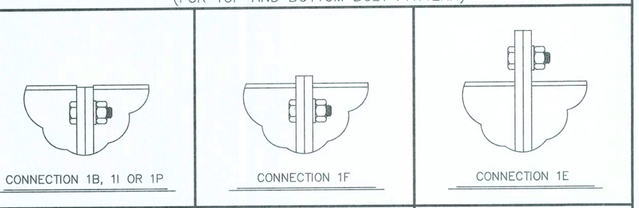
Frame Documentation
A325 Connection Bolt Details

Page 05-12-10
Date Jun '18 Rev. 04

CONNECTION CODES
(FOR TOP AND BOTTOM BOLT PATTERN)

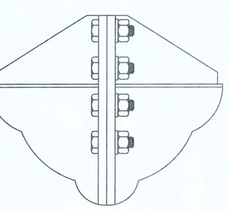


CONNECTION 1B, 1I OR 1P | CONNECTION 1F | CONNECTION 1E

CONNECTION 2B, 2I OR 2P | CONNECTION 2F | CONNECTION 2E

CONNECTION 3B, 3I OR 3P | CONNECTION 3F | CONNECTION 3E

CONNECTION 4B, 4I OR 4P | CONNECTION 4F | CONNECTION 4E



CONNECTION 4X

4 E / 2 E H
- CONNECTION DESIGNATION
  - BLANK = STANDARD CONNECTION
  - H = HEAVY CONNECTION
- BOTTOM CONNECTION CODE
- BOTTOM QUANTITY OF BOLT ROWS
- CONNECTION DESIGNATION
  - BLANK = STANDARD CONNECTION
  - H = HEAVY CONNECTION
- TOP CONNECTION CODE
- TOP QUANTITY OF BOLT ROWS

CONNECTION CODE FORMAT

CONNECTION CODE DESCRIPTION

= THIS DESCRIPTION CODE IS USED TO DEFINE SHEAR CONNECTIONS. BOLTS ARE LOCATED INSIDE E TOP FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP FLANGE. CONNECTION ATE LENGTH MUST BE A MINIMUM OF HALF THE RAFTER WEB DEPTH AND SHALL NOT EXCEED THE FTER TOTAL DEPTH.

= THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED WITH E SET OUTSIDE THE TOP OR BOTTOM FLANGE AND THE REMAINING SETS ARE LOCATED INSIDE THE P OR BOTTOM FLANGE.

= THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED INSIDE E TOP OR BOTTOM FLANGE AND CONNECTION PLATE PROJECTS 1/2" BEYOND THE TOP OR BOTTOM ANGE.

= THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED INSIDE E TOP OR BOTTOM FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP OR TTOM FLANGE.

= THIS DESCRIPTION CODE IS USED TO DEFINE SHEAR CONNECTIONS. BOLTS ARE LOCATED INSIDE E TOP FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP FLANGE. CONNECTION ATE LENGTH MUST BE A MINIMUM OF HALF THE RAFTER WEB DEPTH AND SHALL NOT EXCEED THE FTER TOTAL DEPTH.

= THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED WITH O SETS EACH SIDE OF THE TOP OR BOTTOM FLANGE WITH A GUSSET PLATE OUTSIDE THE TOP AND TTOM FLANGE OR COLUMN CAP PLATE.

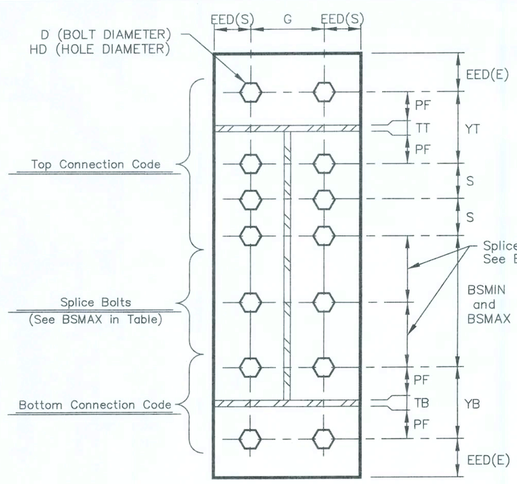
CONNECTION BOLT DATA

NAME	DESCRIPTION FOR A325 BOLT DIMENSIONS		A325 CONNECTION BOLT DIMENSIONS					
D	DIAMETER OF THE BOLT		1/2"	3/4"	7/8"	1"	1 1/4"	1 1/2"
HD	BOLT HOLE DIAMETER		9/16"	13/16"	15/16"	1 1/16"	1 5/16"	1 9/16"
G	BOLT GAUGE		2 1/2"	3"	4"	3 1/2"	4"	5 1/2"
	MAX. WEB THICKNESS (Max. 5/16" Fillet Weld) WITHOUT WASHER		1"	1 1/8"	1 7/8"	1 1/4"	1 3/8"	2 1/8"
	MAX. WEB THICKNESS (Max. 5/16" Fillet Weld) WITH WASHER		3/4"	7/8"	1 5/8"	7/8"	7/8"	1 7/8"
HG	HEAVY CONN. BOLT GAUGE		N/A	2 1/4"	2 5/8"	3"	3 3/4"	4"
S	NORMAL BOLT SPACING		2 1/2"	3"	3 1/4"	3 1/2"	4"	4 1/2"
BSMIN	MINIMUM SPACING BETWEEN TOP & BOTTOM SETS OF BOLTS		1 1/2"	2 1/4"	2 5/8"	3"	3 3/4"	4"
BSMAX	MAXIMUM BOLT SPACING BETWEEN TOP AND BOTTOM SETS OF BOLTS ON CONNECTION PLATES LESS THAN OR EQUAL TO 3/4" THICK		2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
			SPLICE BOLT SPACING (NOT TO EXCEED 2'-0)	1/2 BSMAX (±1/16") WHEN BSMAX = 2'-0 1/16" TO 4'-0 1/3 BSMAX (±1/16") WHEN BSMAX = 4'-0 1/16" TO 6'-0 1/4 BSMAX (±1/16") WHEN BSMAX = 6'-0 1/16" TO 8'-0				
BFCD	MINIMUM BOLT-TO-FLANGE CLEARANCE AT OUT OF NUT SEE BOLT AT FLANGE DETAIL		1 1/2"	1 3/4"	1 7/8"	2 1/4"	2 1/2"	2 3/4"
PF	MINIMUM BOLT-TO-FLANGE CLEARANCE AT CONNECTION PLATE SEE BOLT AT FLANGE DETAIL		(BFCD + RNWT) PF FOR CONNECTION B, F, I AND P ARE THE SAME AS USED ON CONNECTION E					
NWT	NUT AND WASHER THICKNESS		SEE BOLT AT FLANGE DETAIL. NUT THICKNESS IS EQUAL TO THE BOLT DIAMETER AND .1562 5" WASHER THICKNESS IS USED EVEN IF A WASHER IS NOT REQUIRED.					
RNWT	RISE ON NUT AND WASHER THICKNESS							
TT	THICKNESS TOP FLANGE		REFER TO FRAME CROSS SECTION DRAWING FOR LARGEST FLANGE THICKNESS EITHER SIDE OF THE CONNECTION.					
TB	THICKNESS BOTTOM FLANGE							
YT	BOLT SPACING TOP (ROUND UP TO NEXT 1/2", MIN = S)		3" + TT	3 1/2" + TT	3 3/4" + TT	4 1/2" + TT	5" + TT	5 1/2" + TT
YB	BOLT SPACING BOTTOM (ROUND UP TO NEXT 1/2", MIN = S)		or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped
EED(E)	MINIMUM END EDGE DIMENSION		1 1/4"	1 1/4"	1 1/2"	1 3/4"	2 1/4"	2 5/8"
EED(S)	MINIMUM SIDE EDGE DIMENSION		3/4"	1"	1 1/8"	1 1/4"	1 5/8"	2 1/4"
EEDK	END EDGE DIMENSION AT KNEE CONNECTION		1 3/8"	1 3/8"	1 5/8"	1 7/8"	2 3/8"	2 3/4"
BCWM	MINIMUM BOLT CLEARANCE FROM A FLANGE OR WEB WELD	WITHOUT WASHER	7/16"	5/8"	3/4"	13/16"	1"	1 3/8"
		WITH HARDENED WASHER	9/16"	3/4"	7/8"	1"	1 1/4"	1 1/2"
WCSM	MINIMUM WIDTH OF CONNECTION PLATE (Standard Connection)		5"	6"	8"	8"	10"	12"
WCHM	MINIMUM WIDTH OF CONNECTION PLATE (Heavy Connection)		N/A	10"	12"	12"	16"	18"
TCMIN	MINIMUM THICKNESS OF CONNECTION PLATE		1/4"	3/8"	7/16"	1/2"	5/8"	1"

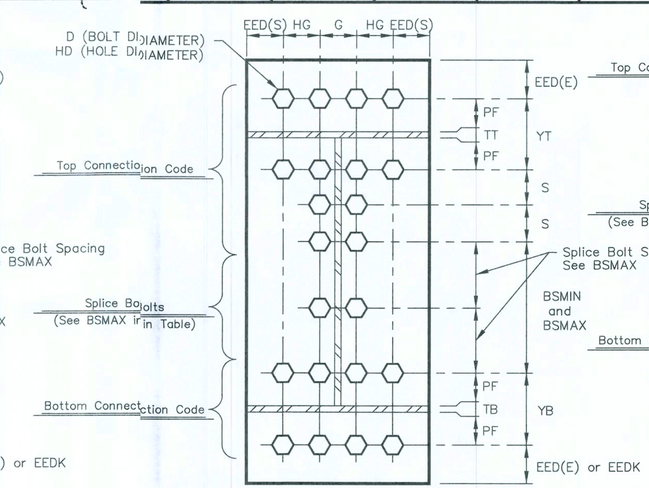
B 4E/2EH
- Connection Code (See "Connection Code Format" on this drawing)
- Connection Location

CROSS SECTION CONNECTION CODE KEY
(AS SHOWN AT CONNECTIONS ON FRAME CROSS SECTION DRAWINGS)

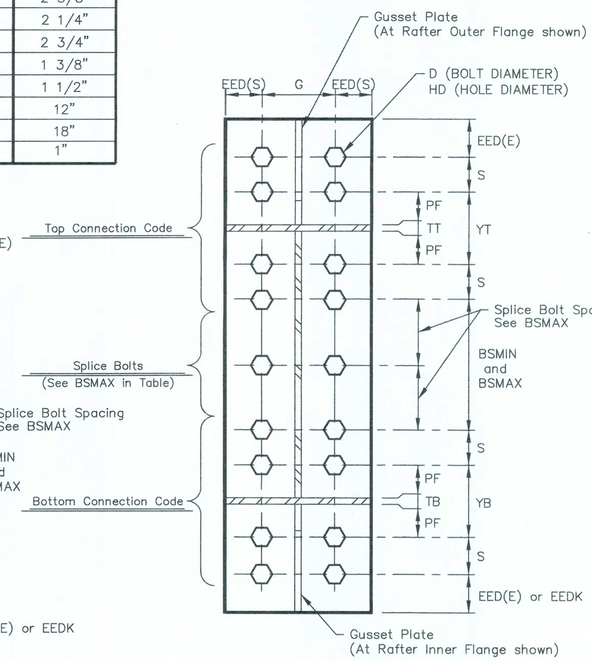
Flange Brace Material Schedule	
Part Mark	Material
FB4_	L 2" x 2" x 14 Ga.
FB5_	L 2" x 2" x 14 Ga.
FB6_	L 2" x 2" x 1/8"
FB7_	L 2 1/2" x 2 1/2" x 3/16"



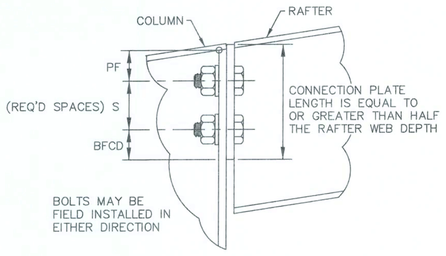
STANDARD CONNECTION DESIGNATION
(CODE 4E/2E SHOWN)



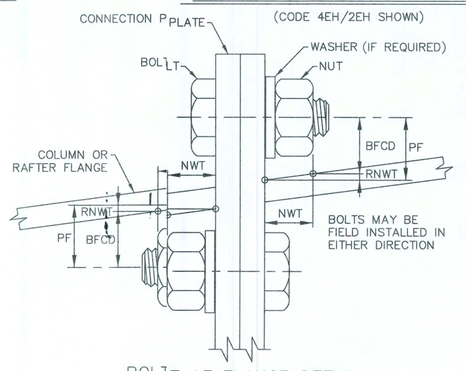
HEAVY CONNECTION DESIGNATION
(CODE 4EH/2EH SHOWN)



4X CONNECTION DESIGNATION
(CODE 4X/4X SHOWN)



CONNECTION B & P
(Low Side Shown, High Side Similar)



BOLT AT FLANGE DETAIL
(Top Flange Shown, Bottom Flange Similar)

Revision	Date	By	Description
A	08/23/21	BSL	FOR CONSTRUCTION PERMIT
		KSK	Ck'd

METALLIC BUILDING SYSTEMS
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Project Name & Location:
DIY LETTERING
215 SW WINDSWEPT GLN
LAKE CITY FL 32024-0693 US

Customer:
SIMQUE CONSTRUCTION
P.O. BOX 2962 32056-2962 US
LAKE CITY FL
DAVID SIMQUE

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: BSL 8/23/21
Checked by: KSK 8/23/21
Project Engineer: MJ
Job Number: 18-B-30506
Sheet Number: E9 of 9

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

KE HU
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PROFESSIONAL ENGINEER