

GENERAL NOTES

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140 MPH, EXPOSURE B

USING THE FOLLOWING CRITERIA AA ASM 35, ASTM ALUMINUM AND ALUMINUM ALLOY SPECIFICATIONS, ALUMINUM STANDARDS AND DAQTA, AND ASCE 7-10 THE DESIGN OF THIS STRUCTURE HAS BEEN REVIEWED FOR COMPLIANCE WITH THE FLORIDA BUILDING CODE 6TH EDITION 2017, BUILDING (FBCB) SECTION 2002.1

BUILDING RISK CATEGORY = I NOMINAL DESIGN WIND SPEED, Vasd = 108 M.P.H. ULTIMATE DESIGN WIND SPEED, Vult = 140 M.PH.(3 SECOND GUST)

EXPOSURE CATEGORY = B (WORSE CASE, ALL DIRECTIONS) +- 0.18 FOR ENCLOSURE STRUCTURES

+- 0.55 FOR PARTIALLY ENCLOSED STRUCTURES

+- 0.00 FOR OPEN STRUCTURES

DESIGN PRESSURE TAKEN FROM TABLE 2002.4 FBC 2017 6TH EDITION

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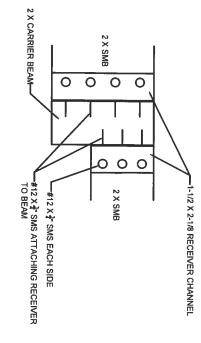
BOLT CENTERS SHALL BE 2.5 TIMES BOLT DIAMETER. UNLESS NOTED SHALL BE LESS THAN ROCKWELL C35. MINIMUM SPACING BETWEEN HOT DIPPED TO A490 BOLTS SHALL NOT BE USED. BOLT HARDNESS GALVANIZED TO ASTM A153 OR ELECTRO-GALVANIZED TO ASTM B633. ALL BOLTS, NUTS, AND WASHER SHALL BE CARBON STEEL HOT DIPPED

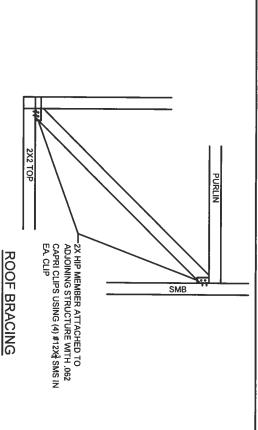
- Ņ SMS SHALL BE COATED OR PLATED CARBON STEEL WITH A ROCKWELL A123, A641, OR B633 OR NICKEL/CHROMIUM PLATED PER ASTM B456, HARDNESS LESS THAN C35. SCREWS SHALL BE ZINC COATED PER ASTM,
- 7 9 9 4 9 ALL FASTENERS TO BE CARBON STEEL WITH A CORROSION RESISTANT COATING.
 - ALL CONCRETE ANCHORS SHALL BE A MINIMUM OF 2" FROM EDGE OF CONCRETE. NO ALUMINUM SHALL BE WELDED OR EXPOSED TO EXTREME TEMPERATURE.
- ALL TAPCONS SHALL HAVE A MINIMUM OF 1-1/4" EMBEDMENT INTO CONCRETE.
- SELF MATING BEAMS (SMB) SHALL BE STITCHED AT EACH END, AT EACH PURLIN OR CHAIR RAIL CONNECTION AND NO MORE THAN 18" O.C. WITH #12X3/4 STS ON BOTH
- œ OTHERWISE. THIS INCLUDES ATTACHMENT OF FRONT WALL TO SIDE WALLS. #12X2" SMS AT 24" O.C. MAX. AND WITHIN 2" OF EACH END UNLESS NOTED 1X2 EXTRUSION (LACING) SHALL BE FASTENED WITH EITHER TAPCONS OR

TYP CARRIER BEAM CONNECTION

- 9 GIRTS 88" AND UNDER ARE TO BE 2X2'S AND GIRTS OVER 88" ARE TO BE 2X3'S. USE 2X4'S BETWEEN 2X10 BEAMS OR POSTS
- 10. 2000 PSF PER FBCB TABLE 1806.2. COMPACT FILL TO 95% MODIFIED ORGANIC MATERIAL WITH A MINIMUM ALLOWABLE BEARING PRESSURE OF CONDITIONS: LOOSE GRANULAR MATERIAL WITH NO APPRECIABLE CLAY OR FOOTINGS ARE DESIGNED FOR THE FOLLOWING ASSUMED SOIL BEARING BEEN PROVIDED TO THE ENGINEER. THEREFORE FOUNDATIONS AND CHAPTER 18, FBCB. SUBSURFACE GEO-TECHNICAL INFORMATION HAS NOT CONCRETE FOUNDATIONS SHALL COMPLY WITH THE REQUIREMENTS OF

- MASONRY CONSTRUCTION SHALL CONFORM TO REQUIREMENTS OF CHAPTER 21, GLASS UNIT MASONRY SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION ONE-FOURTH THE UNIT LENGTH. THICKNESS OF BED JOINTS SHALL NOT EXCEED 5/8". WITH HEAD JOINTS IN SUCCESSIVE COURSES OFFSET BY NOT LESS THAN MORTAR SHALL BE USED. ALL MASONRY SHALL BE LAID IN RUNNING BOND PATTERN FBCB. NET AREA COMPRESSIVE STRENGTH OF MASONRY IS 1500 PSI. TYPE M, OR S
- 12. COMPRESSIVE STRENGTH IS 2000 PSI AT 28 DAYS UNLESS OTHERWISE NOTED REQUIREMENTS OF ASTM C476 AND CHAPTER 21, FBCB. REQUIRED MINIMUM GROUT USED TO FILL CELLS, LINTELS AND BOND BEAMS SHALL CONFORM TO
- <u>1</u>3. CONCRETE SHALL CONFORM TO REQUIREMENTS OF CHAPTER 19, FBCB, AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS UNLESS OTHERWISE
- REINFORCING BARS SHALL BE GRADE 40 OR 60 MINIMUM IN FOUNDATIONS, MASONRY CONFORMING TO ASTM A82 REQUIREMENTS. WELDED WIRE FABRIC SHALL CONFORM TO REQUIREMENTS OF SECTION 1910, FBCB. REQUIRED IN SECTION 1910, FBCB. SYNTHETIC FIBER REINFORCEMENT SHALL CONFORM TO ASTM A 185 REQUIREMENTS. WIRE FABRIC SHALL BE SUPPORTED AS REQUIREMENTS. JOINT REINFORCING IF USED, SHALL BE 9 GAGE, GALVANIZED STEEL BARS SHALL BE DEFORMED BILLET STEEL BARS AND COMPLY WITH ASTM A 615 FOUNDATION WALLS, AND CMU WALLS UNLESS OTHERWISE NOTED. REINFORCING







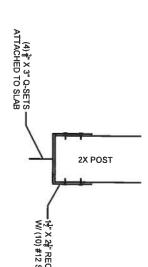
2X10X0.092X0.389 SMB	2X9X0.082X0.306 SMB	2X8X0.072X0.224 SMB	2X7X0.050X0.120 SMB	2X6X0.050X0.120 SMB	2X4X0.044X0.100 SMB	BEAM	IABLE	BEAM SPLICE DETAILS
1/4"	3/16"	3/16"	1/8"	1/8"	1/8"	THICKNESS LENGTH	SPLICE PLATE DIMENSIONS	
20"	18"	18"	16"	16"	16.	LENGTH	ATE DIME	
φį	8.25"	7.25"	6.25"	5.25"	4.25"	HEIGHT	SNOIS	
22	18	16	14	10	7		PER END	#12 SMS
88	72	64	56	40	24		TOTAL #12 SMS PER SPLICE	
4	4	4	3	3	3	HORIZONTAL	NUMBER OF SPACES	FASTNER SPACES
D.	4	3	3	3	3	VERTICAL		

SMS EA. HALF / SEE TABLE BELOW

- X 16" INTERNAL GUSSET PLATE

MANSERED BEAM CONNECTION

2X PURLIN



} CABLE PLATE ATTACHED W/(7) #10 SMS (MIN)

개 x 2급" RECIEVER CHANEL W/ (10) #12 SMS EA. SIDE

CONNECTION MADE W/(2) #10 X 4"
SMS THRU TOP PLATE INTO SCREW
BOSS OF PURLIN

TYP PURLIN TO TOP PLATE CONNECTION

TYP 2X POST TO CONCRETE DETAIL

14" X 64" X 8" STRAP ATTACHED
W/(2) 4" DIA. X 1-1/2" LG. Q-SETANCHORS INTO FOOTER OR EDGE

BTTM.PLATE

2" CLR.

CABLE BRACING

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAVE BEEN REVIEWED COMPLIANOF WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE EDITION BOOK USING ASCE/SEI 7-10 140 M.P.H. 3 SECOND GUST.

Prelim: Project: SHEET $\stackrel{\smile}{\Pi}$ BW-8309 STRUC STRUC STRUC STRUC STRUC INDI STRUC S

Final:

Date:

STRUCTURAL ENGINEERING, NC. MICHAEL A ROBINSON P.E.

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CONSTRUCTION 'ORDAN'S

THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERLTYING ALL DIMENSIONS SHOWN, AND THAT ALL CONSTRUCTION COMPLEX WITH LOCAL BUILDING CODE & ORDINANCES.

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INC.

THE MCKENZIE RESIDENCE

A CUSTOM PLAN FOR:

