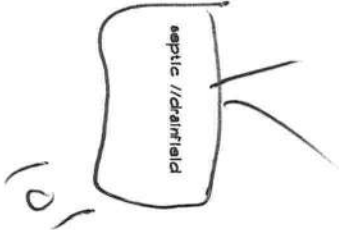
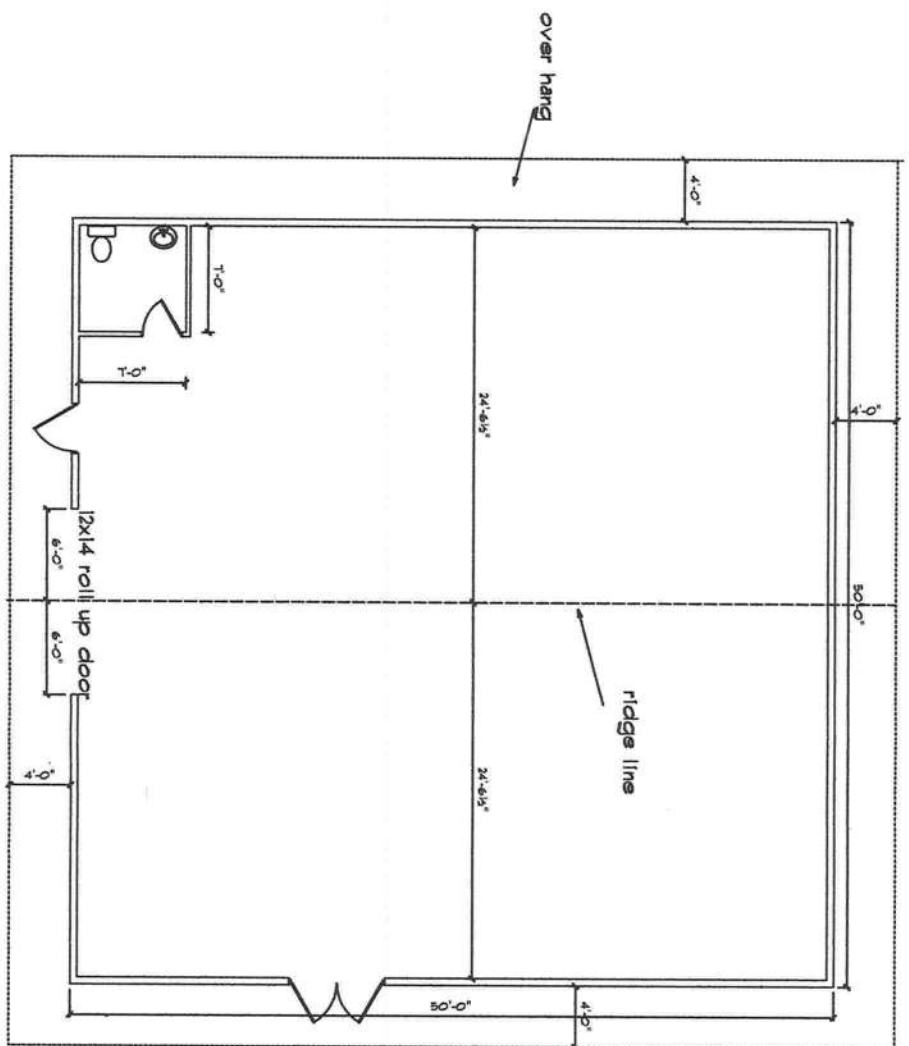


new bldg for
BAIRD TREE CO.



1000



100

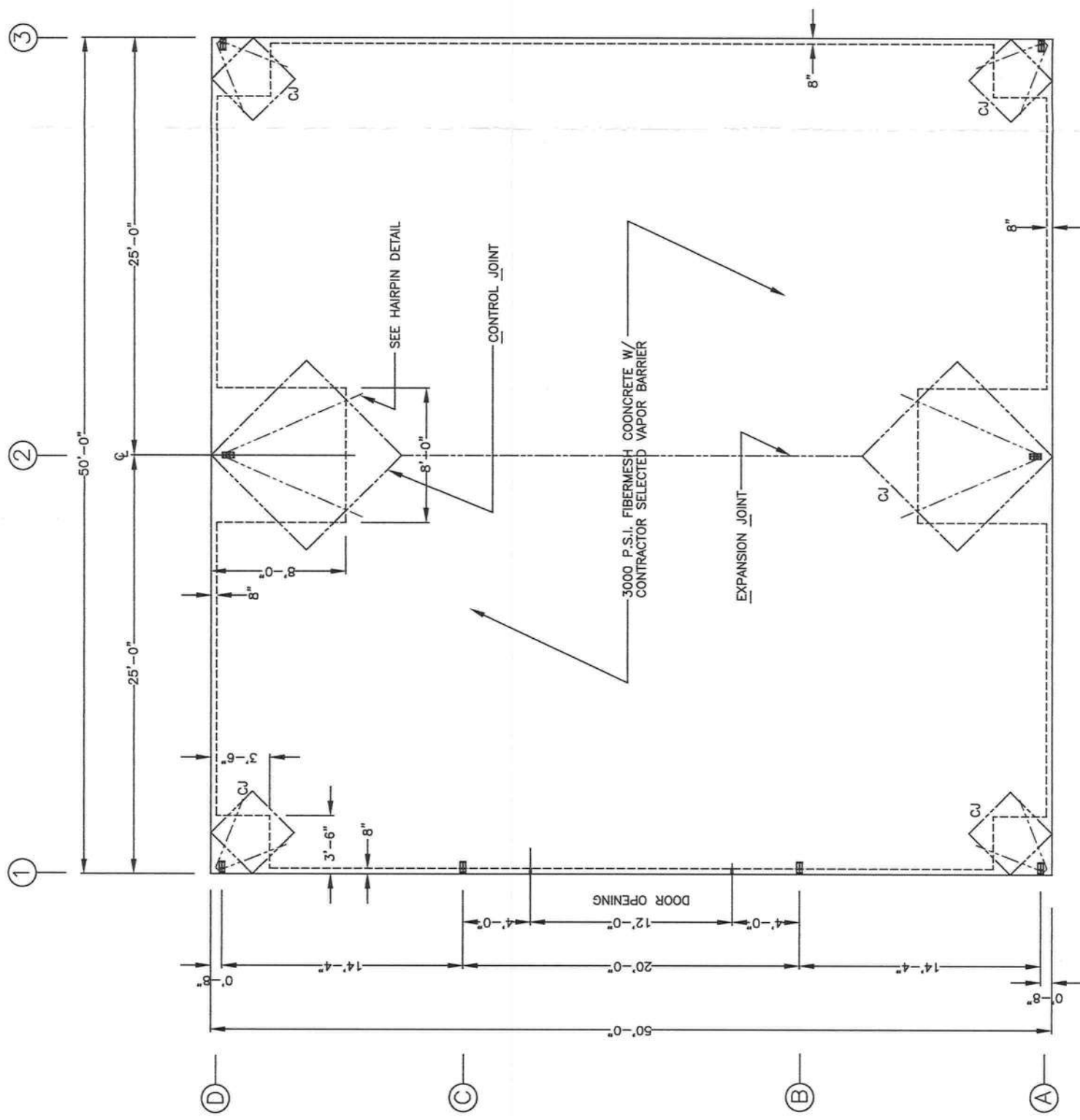
1000

SCALE NOTE:
PLAN VIEW 1/16" = 1'-0"

1204-13
FOOTER
TRANSITION
REVISED



ANCHOR BOLT IMBEDMENTS		
BOLT DIA. INCHES	PROJECTION INCHES	MIN. IMBEDMENT INCHES
5/8"	2-1/2"	
3/4"	2 1/2"	10"
7/8"	3"	
1"	3"	



1 DIMENSIONED FOUNDATION PLAN VIEW
S1.0.0 SCALE: 1/16" = 1'-0"

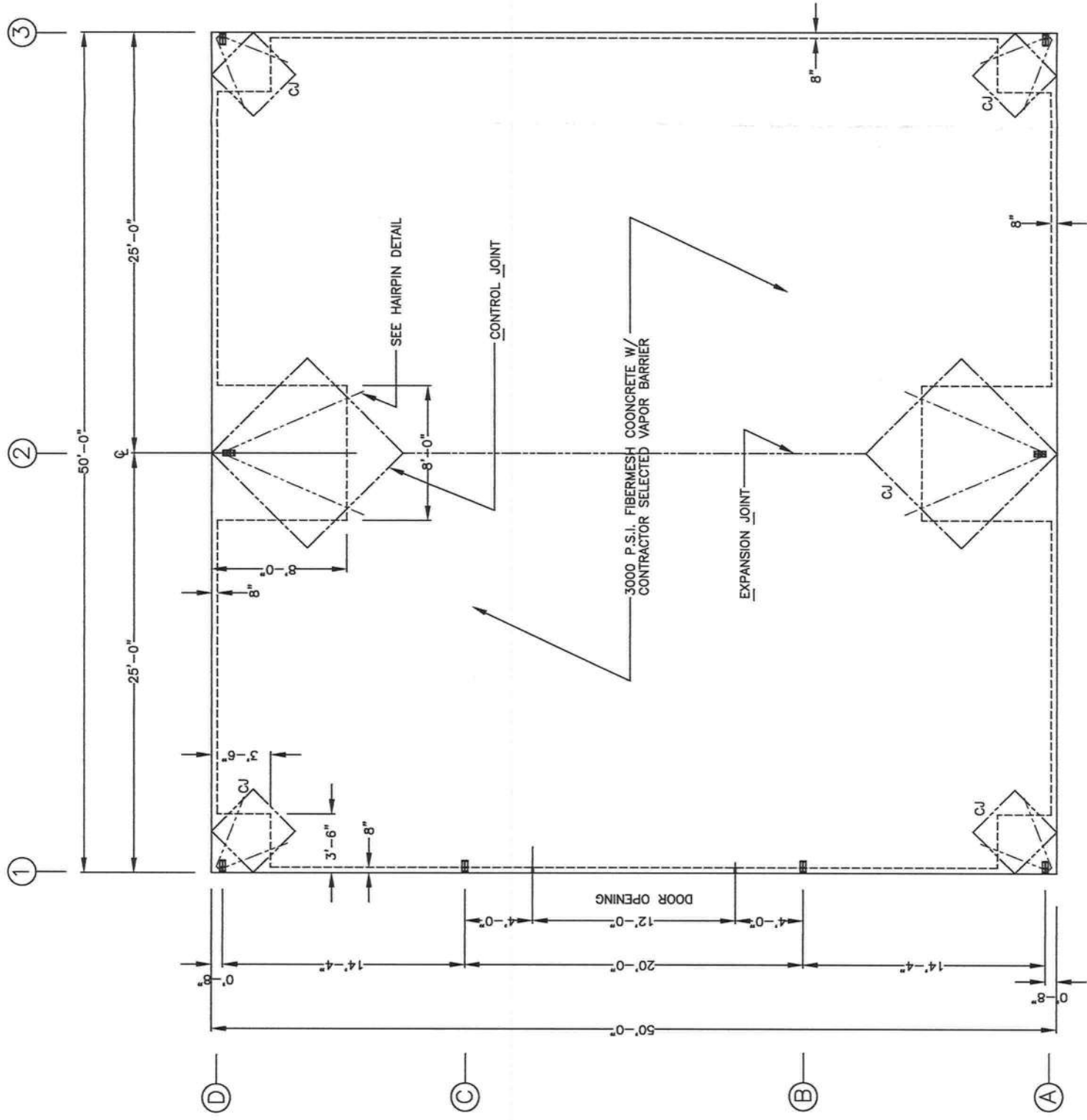
CERTIFICATION:
THESE FOUNDATION PLANS FOR THE BAIRD BUILDING WILL COMPLY WITH SECTION 1600 OF THE 2010 FLORIDA BUILDING CODE FOR A 120 MPH WIND LOAD, 3 SECOND GUST, EXPOSURE B, WITH THE INTERNAL PRESSURE OF + 0.18 AND - 0.18 INCLUDED IN THESE LOADS BASED ON THE REACTIONS BY A S BUILDING SYSTEMS, JOB# 3210-2258274.

Curtis E. Keen 4/18/12
CURTIS E. KEEN, PE #23836, CA #3761

NOTE: CONTRACTOR SHALL REFER TO STEEL BUILDING MANUFACTURER'S COLUMN LOCATIONS, RELATED DIMENSIONS & LIST(S) OF REQUIRED COMPONENTS FOR PROPER COORDINATION W/ THESE PLANS

PROJECT No. BAIRD1-S1.0.0.DWG	DATE 04/05/12	DIMENSIONED PLAN VIEW MISC. NOTES, REFERENCES & INSTRUCTIONS	BAIRD TREE COMPANY COLUMBIA COUNTY, FLORIDA	9263 CR 417 LIVE OAK, FLORIDA 32060 386-362-4787 ENG. LIC. EB 3761
SHEET No. S1.0.0		Curtis E. Keen, PE #23836 Certification of Authorization #3761 DATE:		
© 2012 KEEN ENGINEERING & SURVEYING, INC.				

SCALE NOTE:
PLAN VIEW 1/16" = 1'-0"



1 DIMENSIONED FOUNDATION PLAN VIEW
SCALE: 1/16" = 1'-0"

CERTIFICATION:

THESE FOUNDATION PLANS FOR THE BAIRD BUILDING WILL COMPLY WITH SECTION 1600 OF THE 2010 FLORIDA BUILDING CODE FOR A 115 MPH WIND LOAD, 3 SECOND GUST.

EXPOSURE B, WITH THE INTERNAL PRESSURE OF + 0.18 AND - 0.18 INCLUDED IN THESE LOADS BASED ON THE REACTIONS BY A_1 & S_1 INCLUDING SYSTEMS, JOB# 3210-2258274.

Curtis E. Keen 9/6/12

NOTE: CONTRACTOR SHALL REFER TO STEEL BUILDING MANUFACTURER'S COLUMN LOCATIONS, RELATED DIMENSIONS & LIST(S) OF REQUIRED COMPONENTS FOR PROPER COORDINATION W/ THESE PLANS.

PROJECT No. BAIRD1-S1.0.0.DWG	SHEET No. S1.0.0	DATE 04/05/12
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DIMENSIONED PLAN VIEW
MISC. NOTES, REFERENCES & INSTRUCTIONS

Curtis E. Keen, PE #23836
Certification of Authorization #3761
DATE:

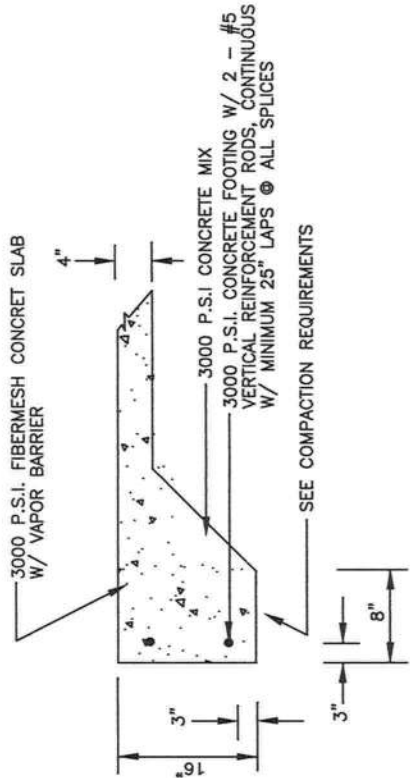
BAIRD TREE COMPANY
COLUMBIA COUNTY, FLORIDA

**KEEN ENGINEERING
& SURVEYING, INC.**

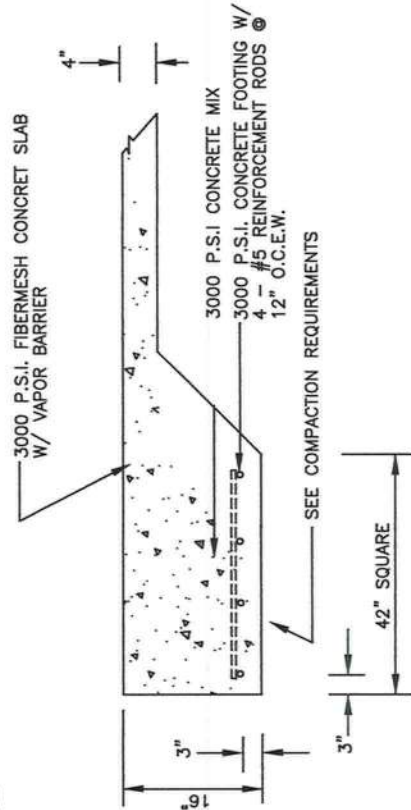
SCALE NOTE:
SECTIONS/DETAILS: N.T.S.

NOTE: CONTRACTOR SHALL REFER TO STEEL BUILDING MANUFACTURER'S COLUMN LOCATIONS, RELATED DIMENSIONS
& LIST(S) OF REQUIRED COMPONENTS FOR PROPER COORDINATION W/ THESE PLANS

#4 copper wire
w/ Approved
concrete encased
Anchor grouted
to footer rebar

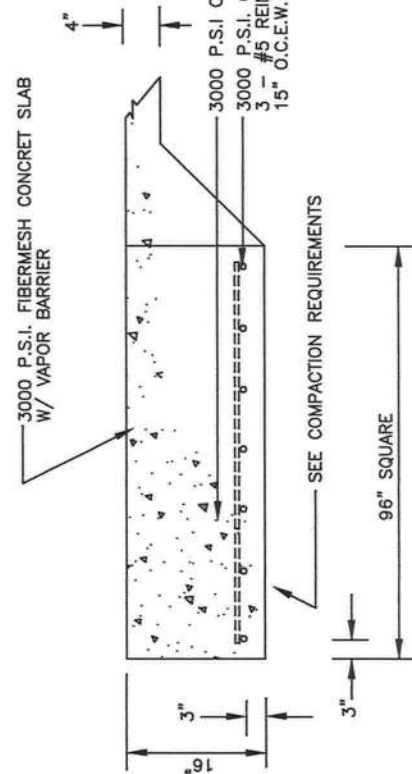


1 SECTION @ PERIMETER FOOTING, TYPICAL
SCALE: N.T.S.



COLUMNS A1, A3, D1 & D3

2 SECTION @ ENDWALL COLUMN FOOTING
SCALE: N.T.S.



COLUMNS A2 & D2

3 SECTION @ SIDEWALL COLUMN FOOTING
SCALE: N.T.S.

CONCRETE & RELATED REQUIREMENTS

- CONCRETE
- CONCRETE CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 318," & "MANUAL CONCRETE PRACTICE, PART 1 ACI 305 & ACI 306," & MANUAL OF CONCRETE PRACTICE, PART 1 ACI 305 & 306" LATEST EDITION
 - CEMENT FOR CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C 150
 - AGGREGATES FOR CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C 33
 - WATER FOR CONCRETE SHALL BE POTABLE WATER FROM MUNICIPAL MAINS
 - OPTIONAL: TEST CONCRETE FOR COMPRESSION WITH 1 SET OF 3 CYLINDERS FOR EACH 50 CUBIC YARDS OF CONCRETE PLACED ON A GIVEN DAY. BREAK 1 CYLINDER @ 7 DAYS AND THE OTHERS @ 28 DAYS. TESTING WILL BE PAID FOR BY OWNER.
 - CONCRETE SHALL HAVE STRENGTHS AND CHARACTERISTICS AS INDICATED ELSEWHERE THESE PLANS
 - SAWED JOINTS MUST BE SAWED WITHIN 24 HOURS OF PLACEMENT OF CONCRETE
 - REINFORCING STEEL SHALL MEET THE REQUIREMENTS OF ASTM A615 OR 60 UNLESS OTHERWISE NOTED
 - NOT USED
 - NOT USED
 - SLAB REINFORCING SHALL BE IN TOP 1/2 OF SLAB OR AS ILLUSTRATED
 - VIBRATE OR SCREEN ALL CONCRETE THOROUGHLY INTO PLACE
 - MINIMUM COVER OF REINFORCEMENT SHALL BE AS REQUIRED BY CODE
 - MOIST CURE CONCRETE FOR 7 DAYS AFTER PLACING
 - PROVIDE VAPOR BARRIER OF POLYETHYLENE UNDER SLAB(S)
 - PLACE CONTROL JOINTS IN SLAB TO PROVIDE MAXIMUM SLAB SIZE OF 600 SQUARE FEET
 - CONCRETE TEMPERATURE SHALL NOT EXCEED 90 DEGREES F DURING PLACEMENT
 - CONCRETE SHALL BE PLACED IN A MANNER TO PREVENT SEGREGATION
 - CONCRETE SHALL NOT BE ALLOWED TO FREE FALL MORE THAN 60 INCHES
 - AREAS TO RECEIVE CONCRETE SHALL BE CLEAR OF ANY DEBRIS AND SHALL HAVE REINFORCING STEEL PROPERLY POSITIONED PRIOR TO CONSTRUCTION JOINTS OTHER THAN THOSE ILLUSTRATED VERIFY W/ ENGINEER
 - FOR LOCATION OF CONTROL OR CONSTRUCTION JOINTS
 - NOT USED
 - ANCHOR BOLTS AND DOWELS SHALL BE SET IN SUCH A MANNER THAT THEIR FULL EMBEDDED LENGTH SHALL BE COVERED WITH CONCRETE
 - LAP SPLICES SHALL BE 40 BAR DIAMETERS OR AS SHOWN OR NOTED ELSEWHERE THESE PLANS
 - DETAILING, FABRICATION AND PLACEMENT OF REINFORCEMENT STEEL SHALL CONFORM TO CURRENT CRSI AND ACI SPECIFICATIONS
 - REINFORCING STEEL SHALL BE FREE OF LOOSE RUST, MIL SCALE AND COATINGS THAT WOULD REDUCE OR DESTROY BOND
 - REINFORCING BARS SHALL NOT BE REDUCED IN SECTION, KINKED OR BENT OTHER THAN INDICATED
 - NOT USED
 - SUPPORT REINFORCING STEEL IN CHAIRS
 - KEEP ONE SET OF CONCRETE CYLINDERS ON SITE AT ALL TIMES TO MAKE SAMPLES IN CASE CONCRETE CHARACTER CHANGES
- REINFORCING STEEL
- REINFORCING STEEL SHALL BE #5 UNLESS OTHERWISE NOTED
 - ALL REINFORCING STEEL SHALL BE A MINIMUM OF GRADE 40 AND IDENTIFIED IN ACCORDANCE W/ ASTM A615, A616, A617 OR A 706
 - SPLICES SHALL BE LAP SPLICES W/ A MINIMUM OF 25" FOR #5 BARS
 - FOR MINIMUM COVER OVER REINFORCEMENT - SEE DETAILS & SECTIONS ELSEWHERE THESE PLANS
 - ALL REINFORCEMENT IN CMU'S SHALL EXTEND A MINIMUM OF 6" INTO ALL FOOTINGS W/ A 6" STANDARD BEND
- METAL ACCESSORIES
- ALL JOINT REINFORCEMENT & ANCHOR TIES SHALL CONFORM TO ASTM A36 & A366 AS REQUIRED
 - LONGITUDINAL WIRES OF JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR OR GROUT W/ A MINIMUM COVER OF 5/8" WHEN EXPOSED TO EARTH OR WEATHER AND A MINIMUM OF 1/2" WHEN NOT EXPOSED TO EARTH OR WEATHER
 - METAL ACCESSORIES USED IN EXTERIOR WALL CONSTRUCTION SHALL BE GALVANIZED IN ACCORDANCE W/ ASTM A153 CLASS B-2
 - METAL ACCESSORIES USED IN INTERIOR WALL CONSTRUCTION SHALL BE MILL GALVANIZED IN ACCORDANCE W/ ASTM A641, CLASS 1
- FILL COMPACTION
- PRIOR TO GRADING OPERATIONS ALL SOIL, ORGANIC LITTER AND FILL SHALL BE STRIPPED FROM BUILDING AREA
 - COMPACTION SHALL NOT BE LESS THAN 98% OF THE STANDARD PROCTOR DENSITY
 - ALL FILL MATERIAL SHALL BE INORGANIC W/ NOT MORE THAN 30% BY WEIGHT FINER THAN 200 U.S. STANDARD SIEVE CONFORMING TO A. LIQUID LIMIT, LW.....30 MAXIMUM
B. ELASTICITY, LW.....15, MAXIMUM
C. DRY UNIT WEIGHT.....100 LBS. PER CU. FT.
 - ALL FILL MATERIAL SHALL BE UNIFORMLY PLACED @ OPTIMUM MOISTURE CONTENT IN 6" UNIFORM LAYERS AND COMPACTED TO A DENSITY OF 98% OF THE STANDARD PROCTOR IN ACCORDANCE W/ ASTM D698T
 - FOOTINGS EXCAVATIONS SHALL BE INSPECTED PRIOR TO PLACING ANY CONCRETE TO ENSURE THAT FOOTINGS REST UPON SOUND EARTH
 - ALL SUBGRADES MUST BE LEVEL, SMOOTH AND UNIFORMLY COMPACTED
 - SUB GRADE MUST BE ACCURATE WITHIN 1/4" OF THE DESIGNATED LEVEL
 - ANY WALL WHICH IS TO RECEIVE BACK FILL ON BOTH SIDES SHALL HAVE THE BACK FILL PLACED SIMULTANEOUSLY ON BOTH SIDES IN EVEN LAYERS AS PREVIOUSLY DESCRIBED SO AS NOT TO APPLY UNEVEN LOADS
- GENERAL
- FOOTINGS SHALL BE LEVEL OR STEPPED AS INDICATED ON PLAN VIEWS & DETAILS OR SECTIONS
 - SOIL, WASTE PIPES OR BUILDING DRAINS PASSING UNDER A FOOTING OR THROUGH A FOUNDATION SHALL BE PROVIDED W/ A RELIEVING ARCH OR AN IRON PIPE SLEEVE A MINIMUM OF 2 - PIPE SIZES GREATER THAN THE PIPE PASSING THROUGH
 - STEM WALLS SHALL EXTEND NO GREATER THAN 3 FEET ABOVE THE FINISH GRADE AND CONSTRUCTED W/ THE PREVIOUSLY DESCRIBED MASONRY UNITS
 - ALL STATE AND LOCAL CODES SHALL BE COMPLIED WITH BY THE CONTRACTOR
 - 2,000 P.S.F. SOIL BEARING PRESSURE SHALL BE OBTAINED UNDER ALL FOOTINGS & SLABS

BUILDER/CONTRACTOR RESPONSIBILITIES

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirm that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (ASC code of standard practice Sept 86 Section 4.2.1)(Mar 05 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Builder is responsible for State, Federal and OSHA safety compliance – The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (ASC Code of Standard Practice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3)

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (ASC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturer's assumptions will govern.

Correction of Errors – Normal erection operations include the correction of minor misfits by moderate amounts of reaming, chipping, welding or cutting and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in the member configuration should be reported immediately to the owner and fabricator by the erector, to enable whoever is responsible either to correct the error or to approve the most efficient and economical method of correction to be used by others. (ASC Code of Standard Practice Sept 86 Section 7.12)(Mar 05 Section 7.14)

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Safety Commitment

The Metal Building Manufacturer has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of the building manufacturer. It is strongly recommended that safe working conditions and accident prevention is the top priority of any job site. Local, State and Federal safety and health standards, whether standard statutory or customary, should always be followed to help ensure worker safety. Make certain all employees know the safest and most productive way to erect a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended. For purposes of determining lift requirements, no bundles supplied by the manufacturer will exceed 4000 lbs. For further information also reference the bill of materials for individual member weights of other structural members. If additional information is required contact the customer service department.

Foundation Design

The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items are embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

Dissimilar Materials

Never allow your roof to come in contact with, or water runoff from, any dissimilar metal including but not limited to: Copper and Arsenic Sulfates used in treated lumber, Calcium used in concrete, mortar and grout.

Debris Removal

Any foreign debris such as sawdust, dirt, animal droppings, etc. will cause corrosion of the roof, gutters, trim, etc. If left on building surfaces for a long enough time. The roof should be periodically inspected for such conditions and if found, they should be removed.

DESIGN LOADING

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY:

FBC 10

THE BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

FRAME / ROOF DEAD LOAD

2,500 PSF

COLLATERAL (LIGHTS)

1 PSF

FRAME / ROOF LIVE LOAD

12 /20.00 PSF

SNOW LOAD

0.0000 PSF

GROUND SNOW LOAD (Pg)

1.0000

SNOW LOAD IMPORTANCE FACTOR (Ia)

0

FLAT ROOF SNOW LOAD (Pn)

1.00

SNOW EXPOSURE FACTOR (Ce)

1.00

THERMAL FACTOR (Ct)

120 MPH

WIND LOAD

120 MPH

ULTIMATE WIND SPEED

120 MPH

WIND EXPOSURE CATEGORY

B

RISK CATEGORY

II

RAIN INTENSITY

10 IN/HOUR

5-MINUTE DURATION, 5-YEAR

11.0000 IN/HOUR

RECURRENT (I1)

11.0000 IN/HOUR

RECURRENT (I2)

11.0000 IN/HOUR

SEISMIC LOAD

II - Normal

OCCUPANCY CATEGORY

1.00

SEISMIC IMPORTANCE FACTOR (Ia)

0.1483

Ss 0.1380

0.0960

S1 0.0600

0.0960

SITE CLASS

D

SEISMIC DESIGN CATEGORY

B

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

TRANSVERSE

LONGITUDINAL

BASIC FORCE RESISTING SYSTEM*

3

RESPONSE MODIFICATION COEFFICIENT(R)

3.0000

SYSTEM OVER-STRENGTH FACTOR(Os)

0.049

SEISMIC RESPONSE COEFFICIENT(Cs)

0.049

BLDG DESIGN BASE SHEAR (V)

1.12 (K)

THE TRANSVERSE DIRECTION IS PARALLEL TO THE RIGID FRAMES

0.97 (K)

THE LONGITUDINAL DIRECTION IS PERPENDICULAR TO THE RIGID FRAMES

0.97 (K)

BASIC FORCE RESISTING SYSTEM*

CA. STEEL ORINARY MOMENT FRAME

BA. STEEL ORINARY CONCENTRIC BRACED FRAMES

HA. STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY

GA. INVERTED PENDULUM SYSTEMS

GB. COMBINED COLUMN SYSTEMS

GC. COMBINED COLUMN SYSTEMS

GD. COMBINED COLUMN SYSTEMS

GE. COMBINED COLUMN SYSTEMS

DRAWING INDEX

ISSUE	PAGE	DESCRIPTION
0	C1	COVER SHEET
0	F1	ANCHOR BOLT PLAN
0	F2	ANCHOR BOLT REACTIONS
0	F3	ANCHOR BOLT DETAILS
0	E1	ROOF FRAMING PLAN
0	E2	FRONT SIDEWALL
0	E3	BACK SIDEWALL
0	E4	LEFT ENDWALL
0	E5	RIGHT ENDWALL
0	E6	FRAME CROSS SECTION
0	DET-3	STANDARD DETAILS

DRAWING STATUS

FOR APPROVAL

THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

FOR CONSTRUCTION PERMIT

THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL, ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

FOR ERECTOR INSTALLATION

FINAL DRAWINGS FOR CONSTRUCTION.

FOR QUESTIONS OR ASSISTANCE
CONCERNING ERECTION CALL
800-274-2100
MONDAY - FRIDAY 7:30AM TO 5:00PM

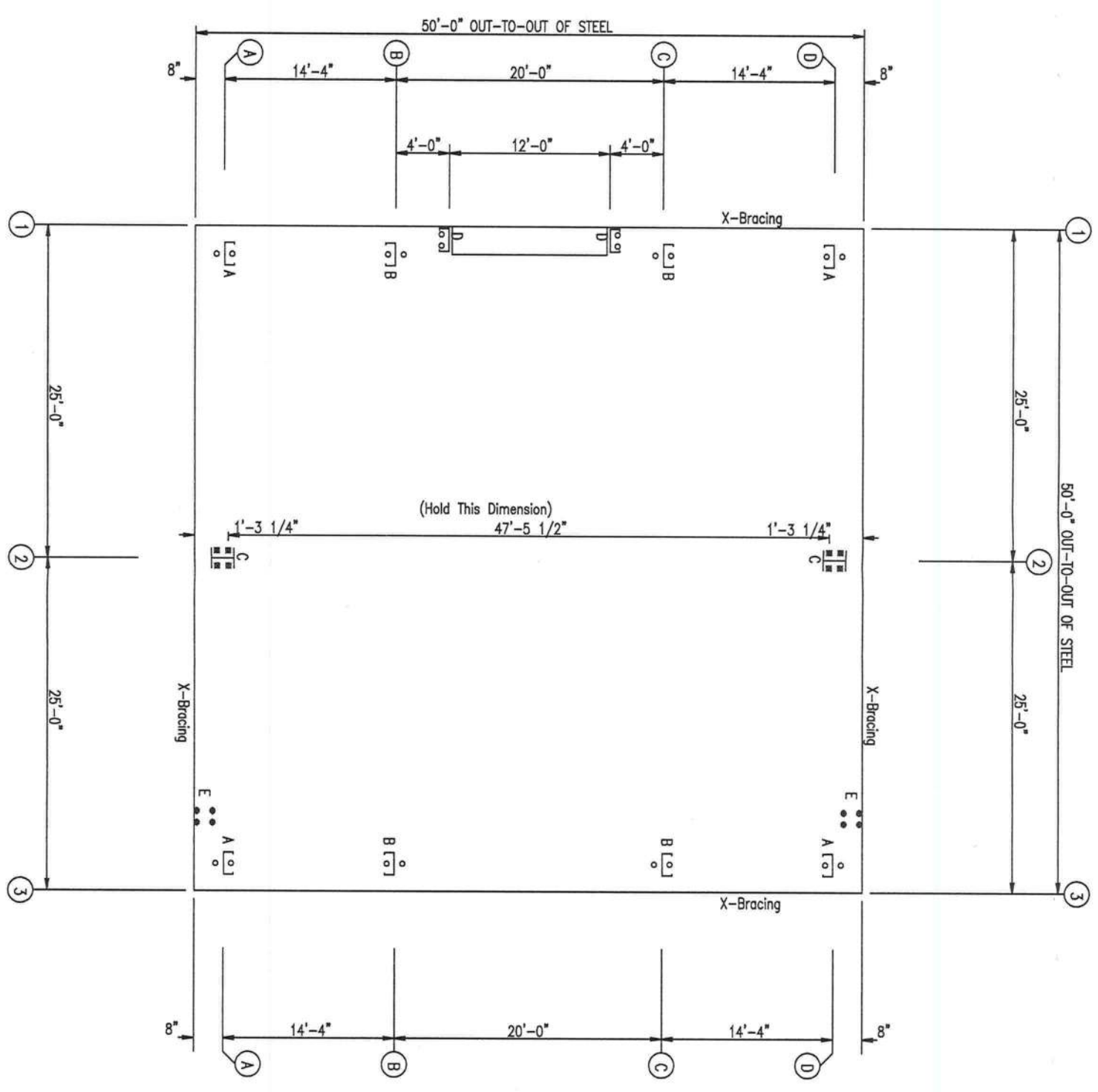
ENGINEERING SEAL

THIS CERTIFICATION COVERS PARTS MANUFACTURED AND DELIVERED BY THE MANUFACTURER ONLY AND EXCLUDES PARTS SUCH AS DOORS, WINDOWS, FOUNDATION DESIGN AND ERECTION OF THE BUILDING. THESE DRAWINGS AND THE METAL BUILDING SYSTEM THEY REPRESENT ARE THE PRODUCT OF AN AFFILIATE OF NCI GROUP, INC. - 10943 N. SAM HOUSTON PARKWAY W., HOUSTON, TX 77064. THE PROFESSIONAL ENGINEER WHOSE SEAL APPEARS HEREON IS EMPLOYED BY AN AFFILIATE OF NCI GROUP, INC. AND IS NOT THE ENGINEER-OF-RECORD FOR THE OVERALL PROJECT.

BAH 330/12
APR 02 2012



○ Dia= 5/8"
⊗ Dia= 3/4"
⊕ Dia= 1"



ANCHOR BOLT PLAN

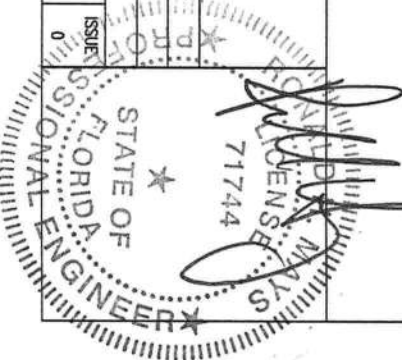
ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	3/26/12	FOR ERECTOR INSTALLATION	SG	JN	BH



A&S BUILDING SYSTEMS
1890 HWY. 116
CARYVILLE, TENNESSEE 37714
1-800-274-2100

PROJECT: BAIRD TREE CO.							
CUSTOMER: GAUBLE & ASSOCIATES	OWNER: BOBBY BAIRD						
LOCATION: LAKE CITY, FL 32065							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	3/26/12	N.T.S.	1	A	3210-2258274	F1	0

APR 02 2012



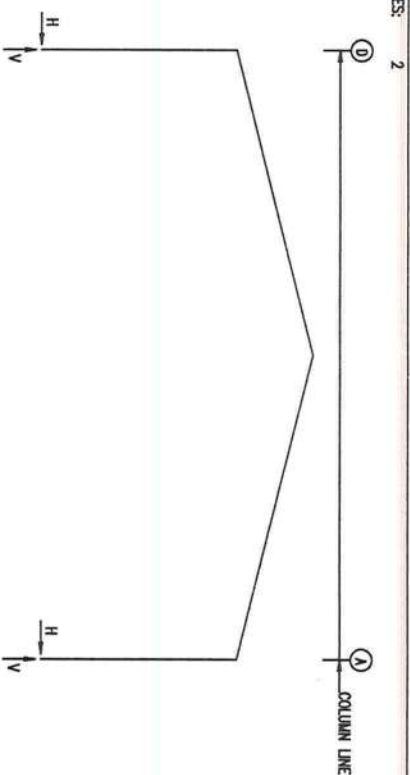
1. THE REACTIONS PROVIDED ARE BASED ON THE OTHER OCCUPANTS AT THE TIME OF TALLING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE TALLING.
2. REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
3. THE FOUNDATION ENGINEER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMIC FOUNDATION DESIGN.
4. THE METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLT DIAMETER ONLY TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE FOUNDATION. THE ANCHOR BOLT SIZE, EMBEDMENT AND DESIGN IS NOT THE RESPONSIBILITY OF THE MANUFACTURER. THE MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLT THROUGH THE TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL, AND CONSTRUCTION OF THE FOUNDATION EMBLEMENTS. THE END USER CUSTOMER SHOULD ASSUME HIS/HERSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL. AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (SECTION 45 MIAL 2006 METAL BUILDING SYSTEMS MANUAL).
5. BOTTOM OF ALL BASE PLATES ARE AT THE SAME ELEVATION. (UNLESS NOTED)
6. ANCHOR RODS ARE ASTM F1554 GRADE 36 MATERIAL, UNLESS NOTED OTHERWISE.

Well	Col	Reactions (K)		Panel Shear (lb/ft)
		Wind Horz	Seismic Vert	
LEW 1	DC	2.2	2.6	0.4
ESW 2	2.6	1.5	0.5	0.3
REW 3	CD	2.2	2.6	0.3
B.SW 4	5.2	2.6	1.5	0.4
B.SW 5	5.2	2.6	1.5	0.3

ENDWALL COLUMN:			ANCHOR BOLTS & BASE PLATES				
Firm Line	Col Line	Anchor Bolt Dia	Base Plate Width	Base Plate Length	Thick	Grout (in)	
1	D	2	7.000	10.00	0.250	0.0	
1	C	2	0.625	7.000	10.00	0.250	0.0
1	B	2	0.625	7.000	10.00	0.250	0.0
1	A	2	0.625	7.000	10.00	0.250	0.0
3	A	2	0.625	7.000	10.00	0.250	0.0
3	B	2	0.625	7.000	10.00	0.250	0.0
3	C	2	0.625	7.000	10.00	0.250	0.0
3	D	2	0.625	7.000	10.00	0.250	0.0

	Dy	Locate	Dia (in)	Type	Proj (in)
○ 4		Jonh	5/8"	F1554	2.00
○ 16		Endrell	5/8"	F1554	2.00
⊗ 8		Frame	3/4"	F1554	2.50
⊕ 8		Windrest		F1554	3.00

RIGID FRAME:				ANCHOR BOLTS & BASE PLATES	
Firm Line	Col Line	Arc, Bolt Qty Dia	Base Plate (in) Width Length	Thick	Grout (in)



BASIC COLUMN REACTIONS (k)

Forme	Column	Doof	Calender	Live	Wind 1	Wind R1	Wind L2
Line	Horiz	Vert	Horiz	Horiz	Vert	Horiz	Vert
2	D	1	2.8	0.4	0.9	1.1	-8.8
2	A	-1.1	2.8	-0.4	0.9	-4.3	10.6
Forme	Column	Wind R2	Wind R3	Wind R4	Seasonic 1	Seasonic 8	Loose
Line	Horiz	Vert	Horiz	Horiz	Vert	Horiz	Vert
2	D	1.0	-5.1	-1.8	-10.4	-1.4	-6.7
2	A	6.9	-8.5	1.8	-10.4	1.4	-6.7
Forme	Column	Wind R2	Wind R3	Wind R4	Seasonic 1	Seasonic 8	Loose
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Line	Horiz	Vert	Horiz</				

BASIC COLUMN REACTIONS (k)

[illegible][illegible]

A&S BUILDING SYSTEMS

1880 HWY. 116
CARYVILLE, TENNESSEE 37714
1-800-274-2100

BAIRD TREE CO.

CUSTOMER: GAMBLE & ASSOCIATES

WINNER: BOBBY BAIRD

LOCATION: LAKE CITY, FL 32055

CAD

DATE	
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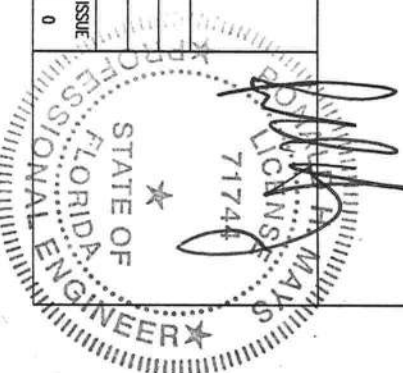
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BUILDING IT

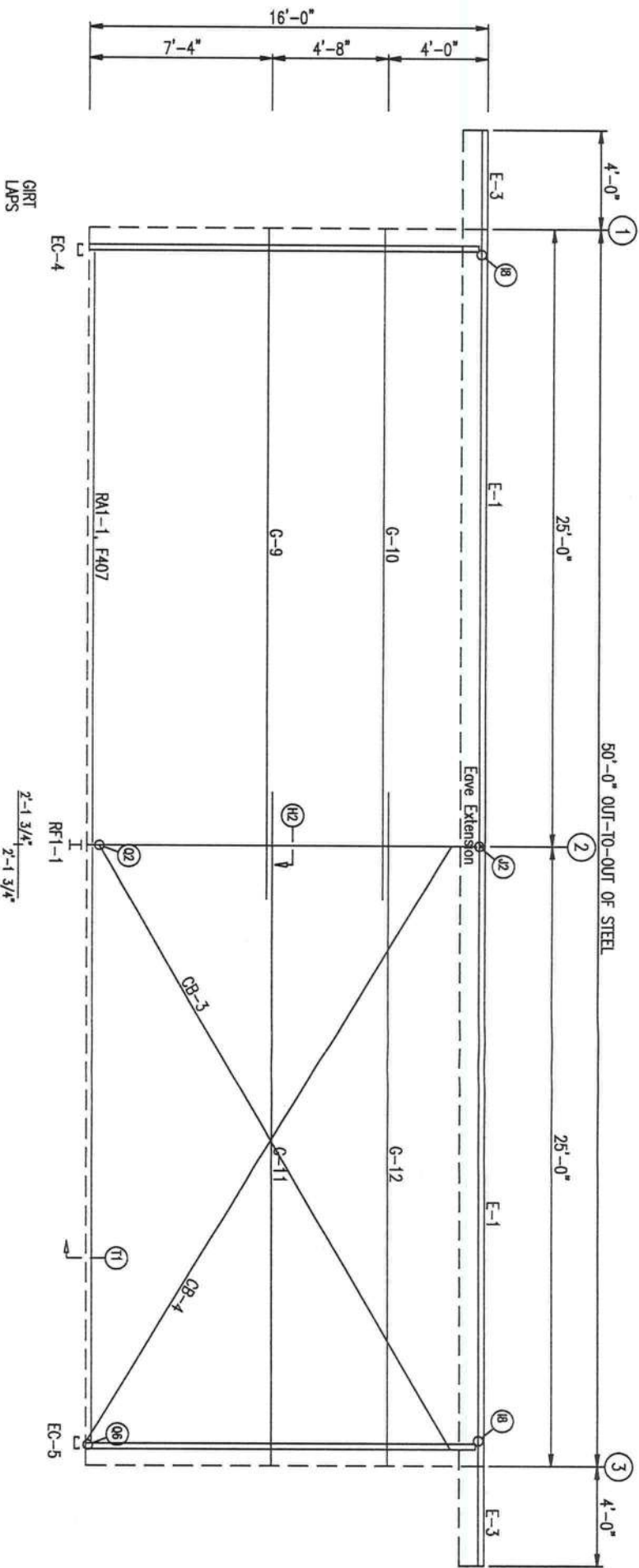
JOB NUMBER

SHEET NUMBER	ISSUE
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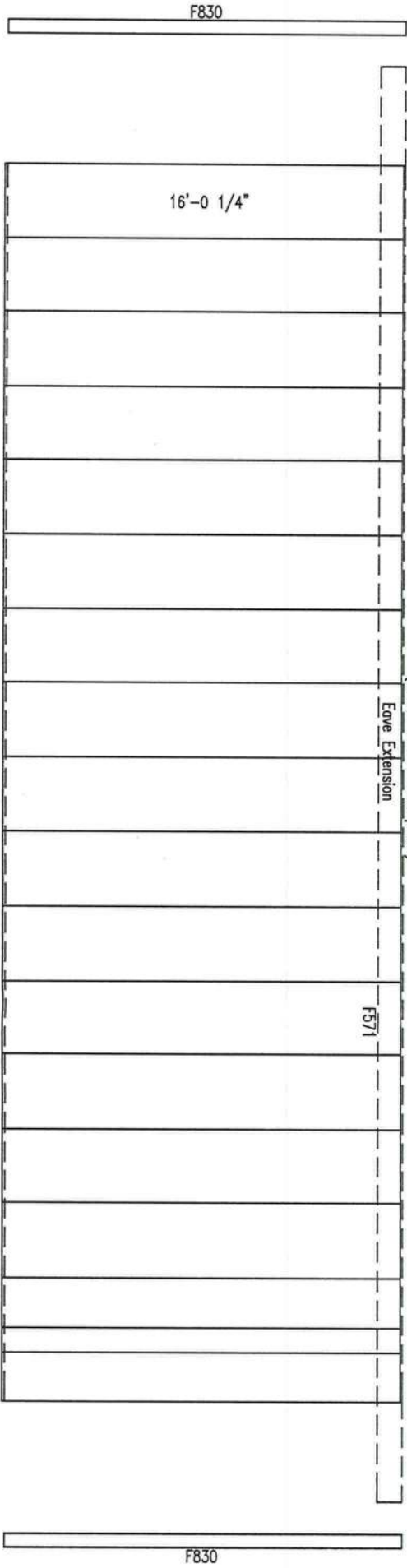


MEMBER TABLE			
FRAME LINE A			
MARK	PART	LENGTH	
E-1	8ES3L14	23'-3 3/4"	
E-3	8ES3L14	4'-8"	1 1/2"
G-9	8X25Z14	27'-1 1/2"	
G-10	8X25Z16	27'-1 1/2"	
G-11	8X25Z14	27'-1 1/2"	
G-12	8X25Z16	27'-1 1/2"	
CB-3	1 1/4" DIA.	28'-8"	
CB-4	1 1/4" DIA.	27'-3"	



SIDEWALL FRAMING: FRAME LINE A

F2001-DL F2002-DL 1
F2955-D, F2001-D 2
F2001-DR F2002-DR 3



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. PR - Polar White

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	3/26/12	FOR ERECTOR INSTALLATION	SG	JN	BAH



A&S BUILDING SYSTEMS
1880 HWY. 116
CARYVILLE, TENNESSEE 37714
1-800-274-2100

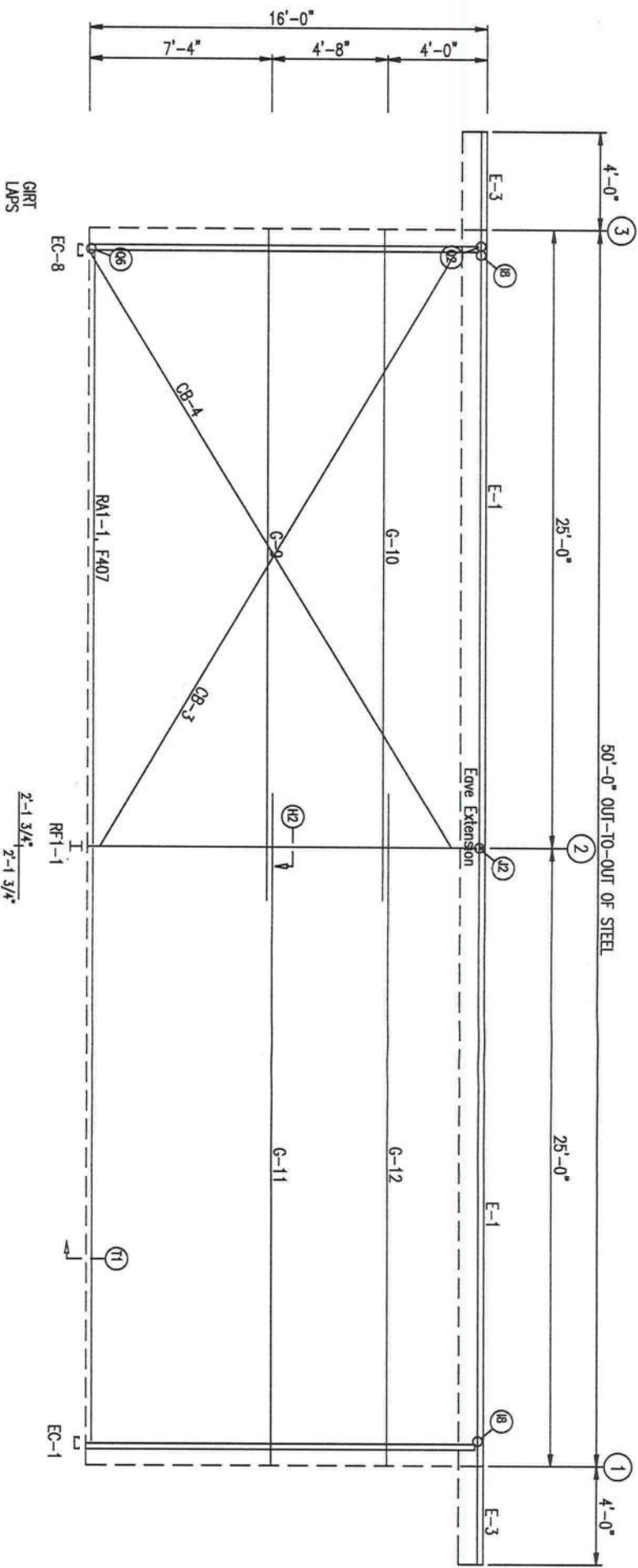
GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHIMMINGS CAUSED BY DRILLING.



APR 02 2012

MEMBER TABLE		
FRAME LINE D		
MARK	PART	LENGTH
E-1	BECS114	23'-3 3/4"
E-3	BECS114	4'-8"
G-9	8X25214	27'-1 1/2"
G-10	8X25216	27'-1 1/2"
G-11	8X25214	27'-1 1/2"
G-12	8X25216	27'-1 1/2"
CR-3	1/4" DIA.	28'-8"
CR-4	1/4" DIA.	27'-3"



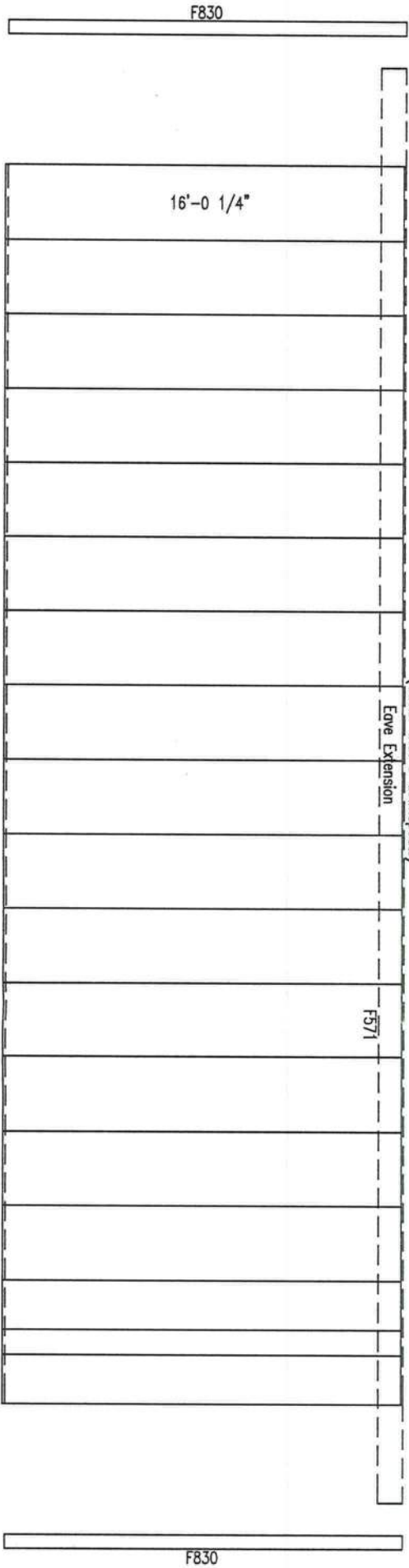
SIDEWALL FRAMING: FRAME LINE D

F2001-DL
F2002-DL

F2955-D, F2001-D

F2001-DR
F2002-DR

(Gutter with 3 downspouts)



SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Ga. PR - Polar White

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	3/26/12	FOR ERECTOR INSTALLATION	SG	JN	BH1



A&S BUILDING SYSTEMS

1890 HWY. 116
CARYVILLE, TENNESSEE 37714
1-800-274-2100

GENERAL NOTES:

1. INSTALL ALL GRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

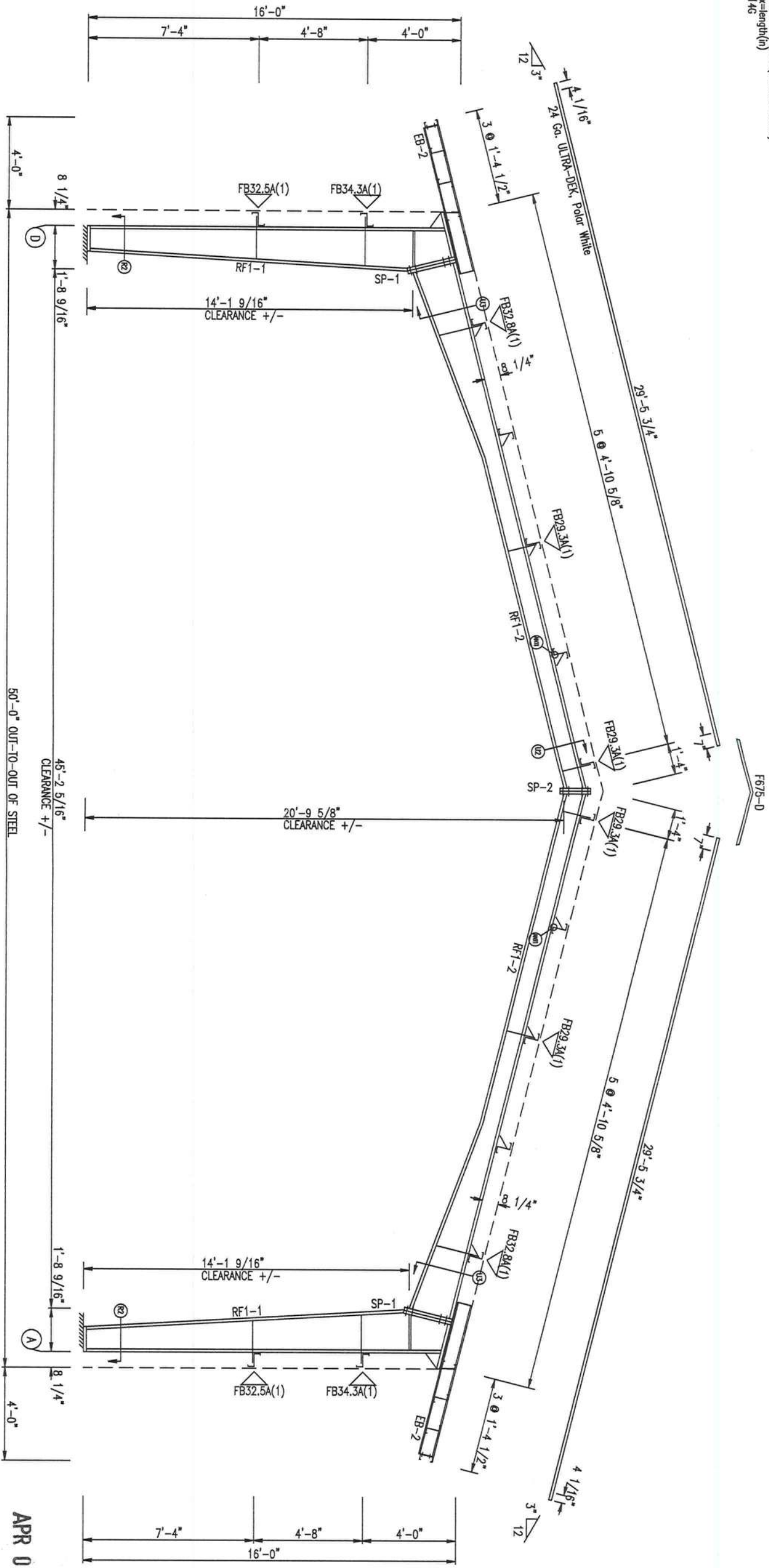
APR 02 2012



SPURCE BOLT TABLE					
Mark	Qty	Bot	Int	Type	Length
SP-1	4	4	0	A325	3/4"
SP-2	4	4	0	A325	3/4"

FLANGE BRACES: BOTH SIDES (UNLESS NOTED)
FB32.5A(1): xx=length(in)
A - L2X2X1/4

MEMBER TABLE					
Mark	Web Depth	Web Thickness	Web Plate	Outside Flange	Inside Flange
RF1-1	10.0/20.1	0.134	165.5	5 x 1/4" x 184.9	5 x 1/4" x 165.8
RF1-2	20.1/14.3	0.156	23.1	5 x 1/4" x 23.5	5 x 1/4" x 240.0
	20.0/ 8.0	0.134	104.3	5 x 1/4" x 44.3	5 x 1/4" x 177.9



RIGID FRAME ELEVATION: FRAME LINE 2

GENERAL NOTES:

1. ALL BOLTED JOINTS WITH A325/A-490 TYPE 1 BOLTS ARE SPECIFIED AS SNUG TIGHTED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, JUNE 30, 2004. PRETENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST OFF TYPE TENSION BOLTS OR DIRECT TENSION INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
2. ALL FIELD WELDED CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 MACHINE BOLTS.
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

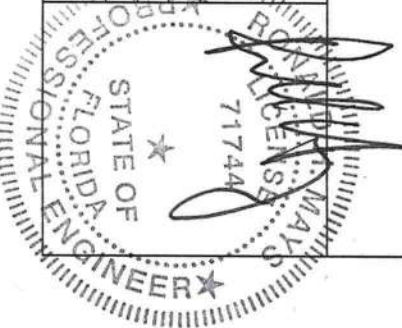
ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	3/26/12	FOR ERECTOR INSTALLATION	SG	JN	BAH

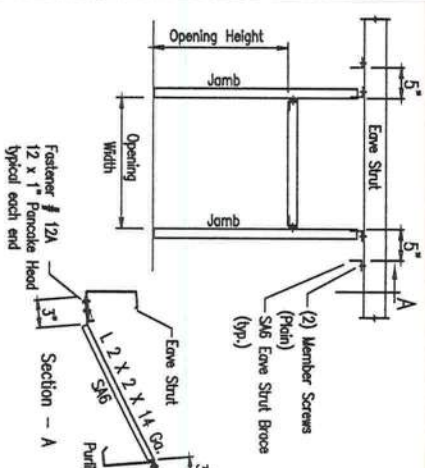
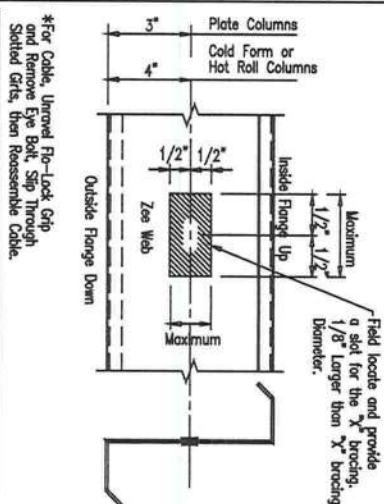
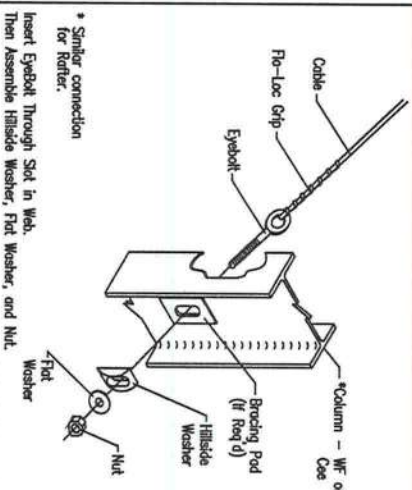


A&S BUILDING SYSTEMS
1880 HWY. 116
CARMINE, TENNESSEE 37714
1-800-274-2100

PROJECT: BAIRD TREE CO.		OWNER: BOBBY BAIRD	
CUSTOMER: GAMBLE & ASSOCIATES			
LOCATION: LAKE CITY, FL 32055			
CAD	DATE	SCALE	PHASE
	3/26/12	N.T.S.	1
		BUILDING ID	JOB NUMBER
		A	3210-2258274
		SHEET NUMBER	ISSUE
		E6	0

APR 02 2012





Standard Grade		
Description	Fastener Number	Application
1/4" - 14 x 7/8"	4A	Slit & Firm Screw
12-14 x 1 1/4"	17A	Member Screw
12-14 x 1 1/2"	17B	Member Screw
12-14 x 2"	2B	Member Screw

Note:
Standard details call for 1 1/4" fasteners as member screws by default.

Description	Fastener Number	Application
1/4" - 14 x 7/8"	4	Stitch & Trim Screws
12-14 x 1 1/4"	3	Member Screw
12-14 x 1 1/2"	3A	Member Screw
12-14 x 2"	68	Member Screw

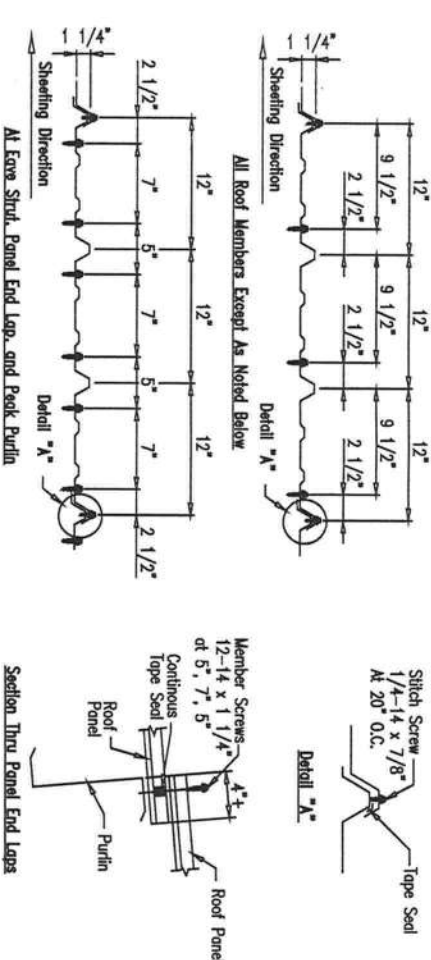
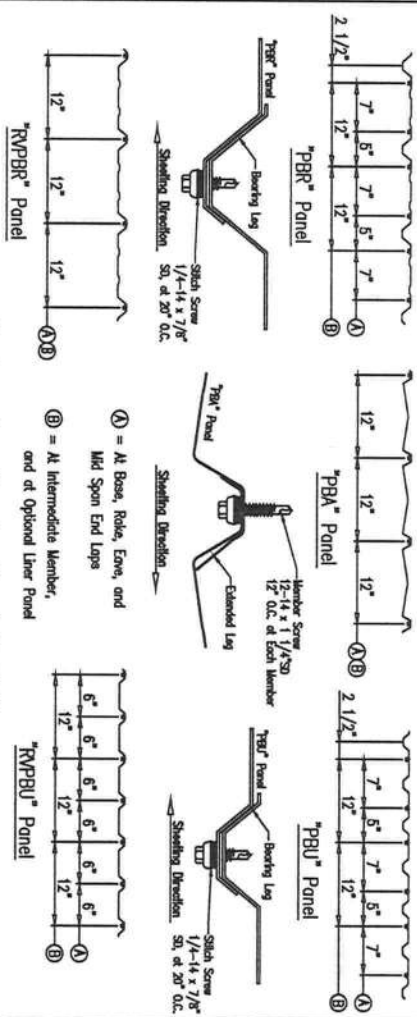
Member screws may be 1 1/4", 1 1/2", or 2" depending on insulation, application, or customer request.

Q2 DIAGONAL CABLE, EYEBOLT END

CABLE AT FLUSH WALL. GIRT

Sidewall Framed Opening Jamb Brace Detail

Self-Drilling Screw Application



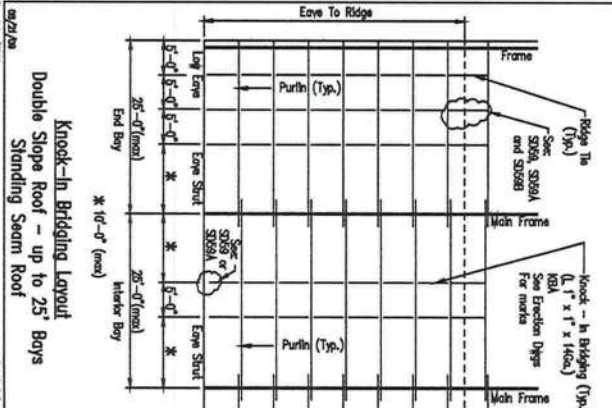
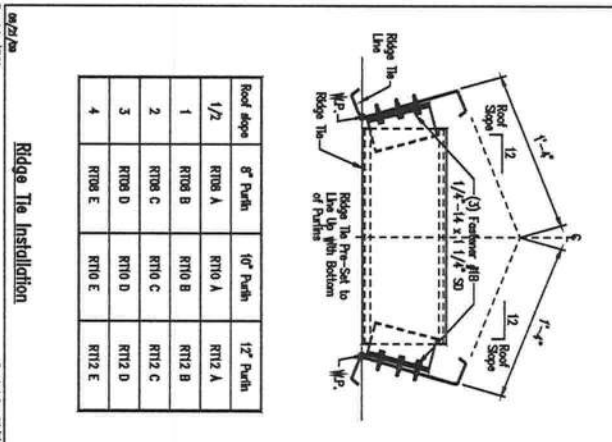
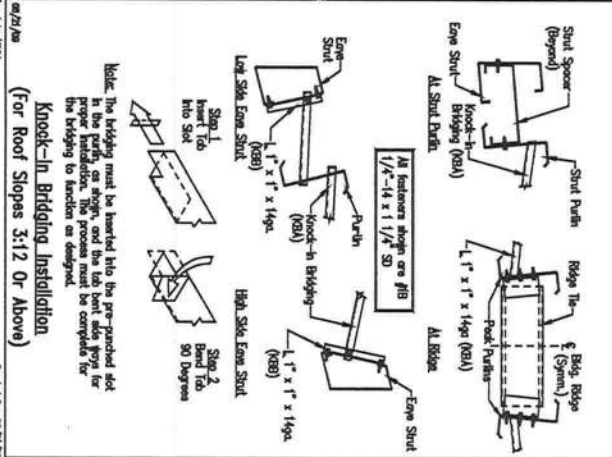
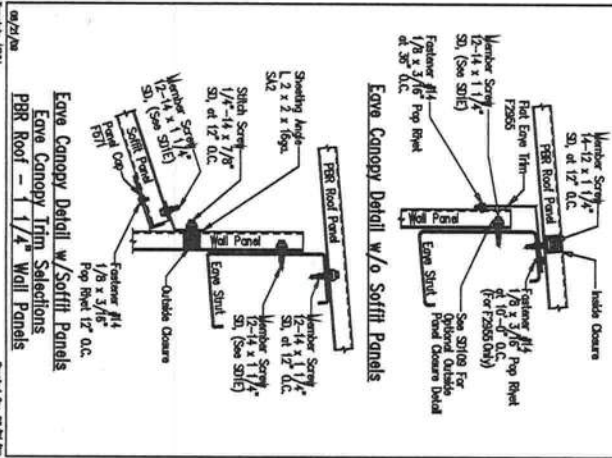
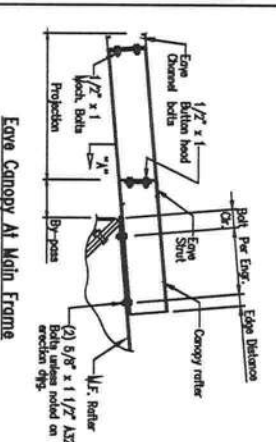
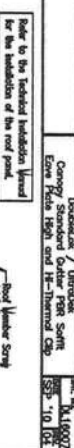
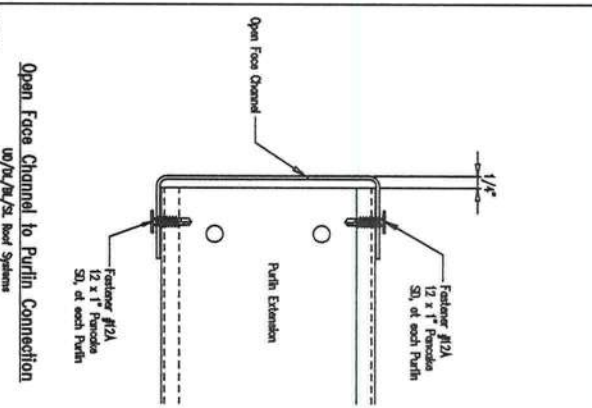
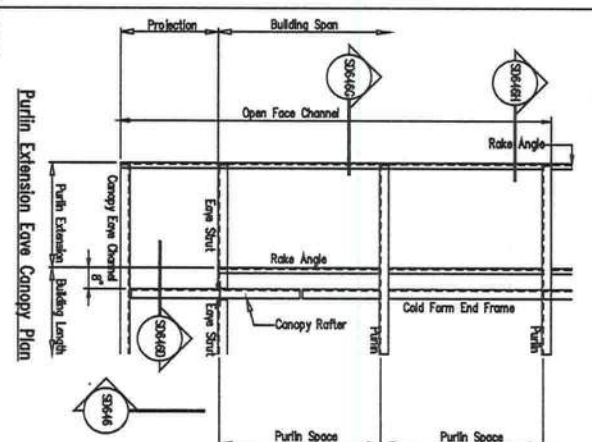
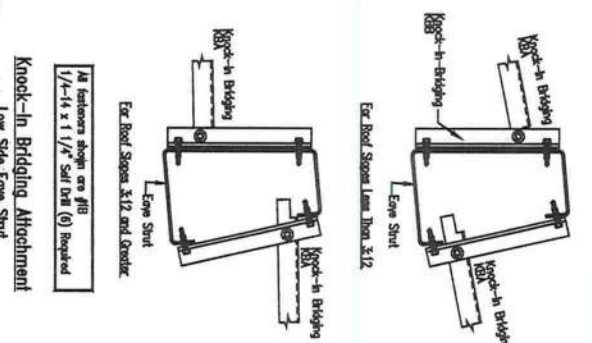
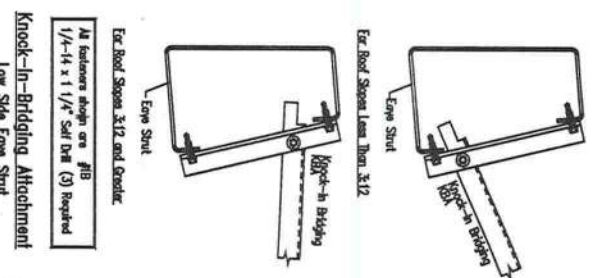
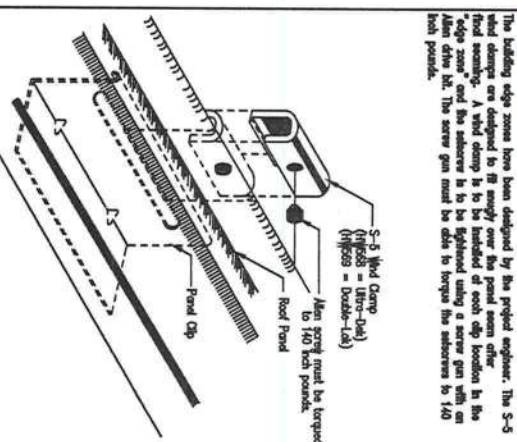
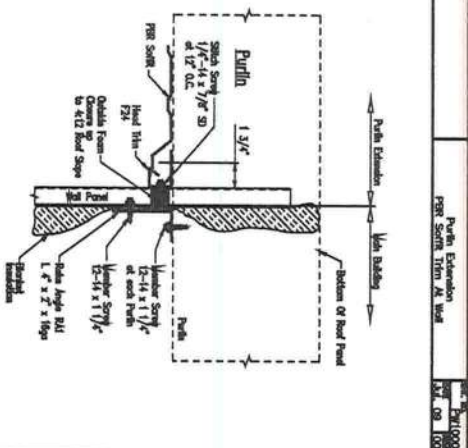
Fastener Location for Panel At Wall

Fastener Location for "PBR" Roof Panel

[illegible]

A&S BUILDING SYSTEMS
1880 HWY. 116
CARVILLE, TENNESSEE 37714
1-800-274-2100

PROJECT: BARD TREE CO.		OWNER: BOBBY BARD	
CUSTOMER: GAMBLE & ASSOCIATES			
LOCATION: LAKE CITY, FL 32066			
CAD	DATE	SCALE	PHASE
	3/26/12	N.T.S.	1
			A
		BUILDING ID	JOB NUMBER
			3210-2258274
		SHEET NUMBER	ISSUE
		DET3	0

[illegible]

A&S BUILDING SYSTEMS
1880 HWY. 116
CARYVILLE, TENNESSEE 37714
1-800-274-2100

PROJECT: BAIRD TREE CO.		OWNER: BOBBY BAIRD	
CUSTOMER: GAMBLE & ASSOCIATES			
LOCATION: LAKE CITY, FL 32055			
CAD	DATE	SCALE	PHASE
	3/26/12	N.T.S.	1
			A
		BUILDING ID	JOB NUMBER
			3210-2256274
		SHEET NUMBER	ISSUE
		DET 4	0