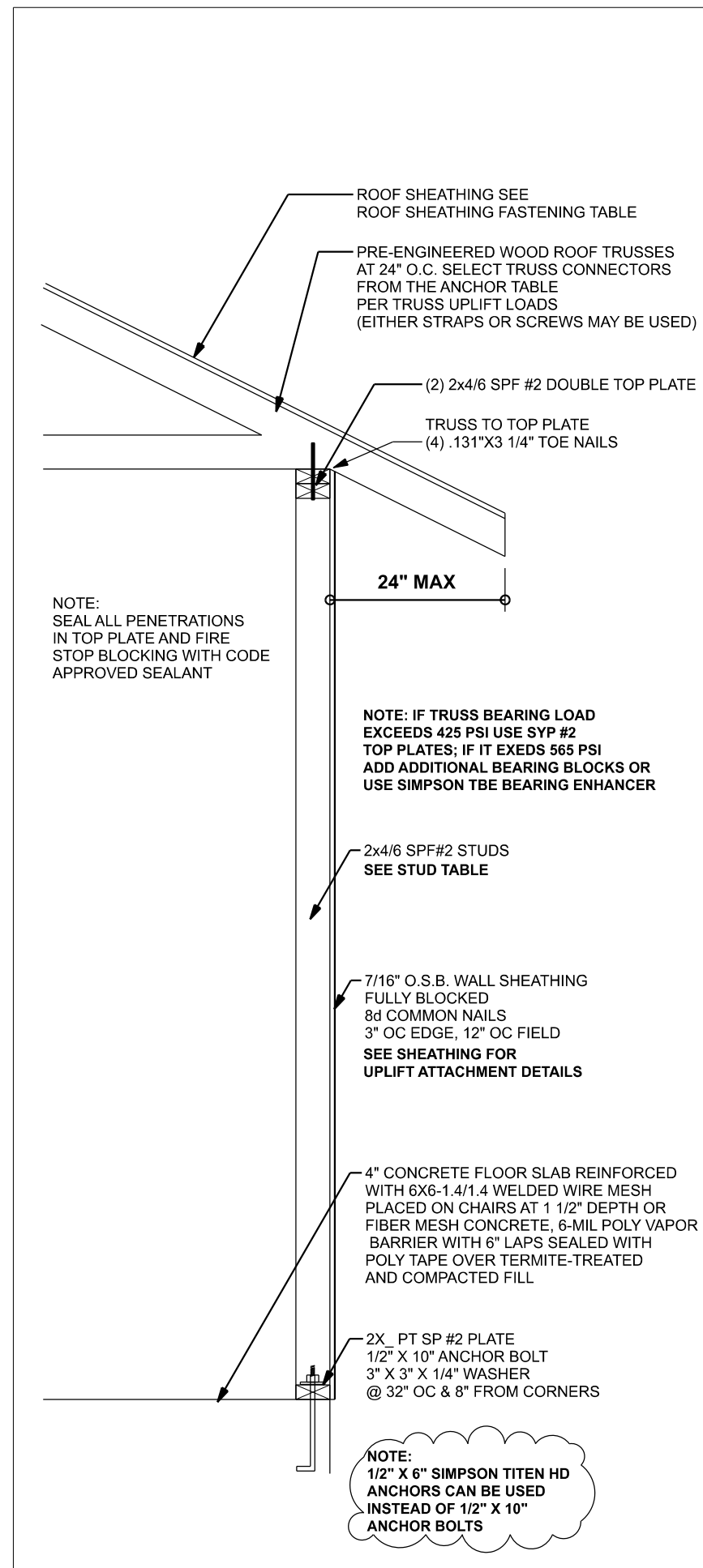
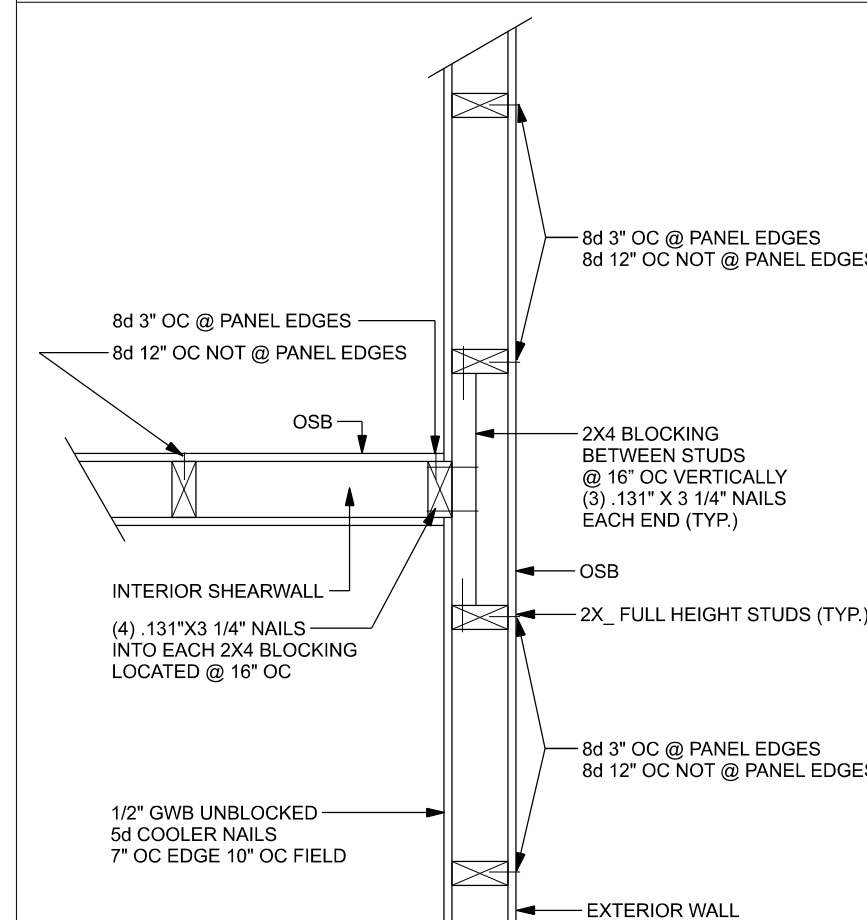
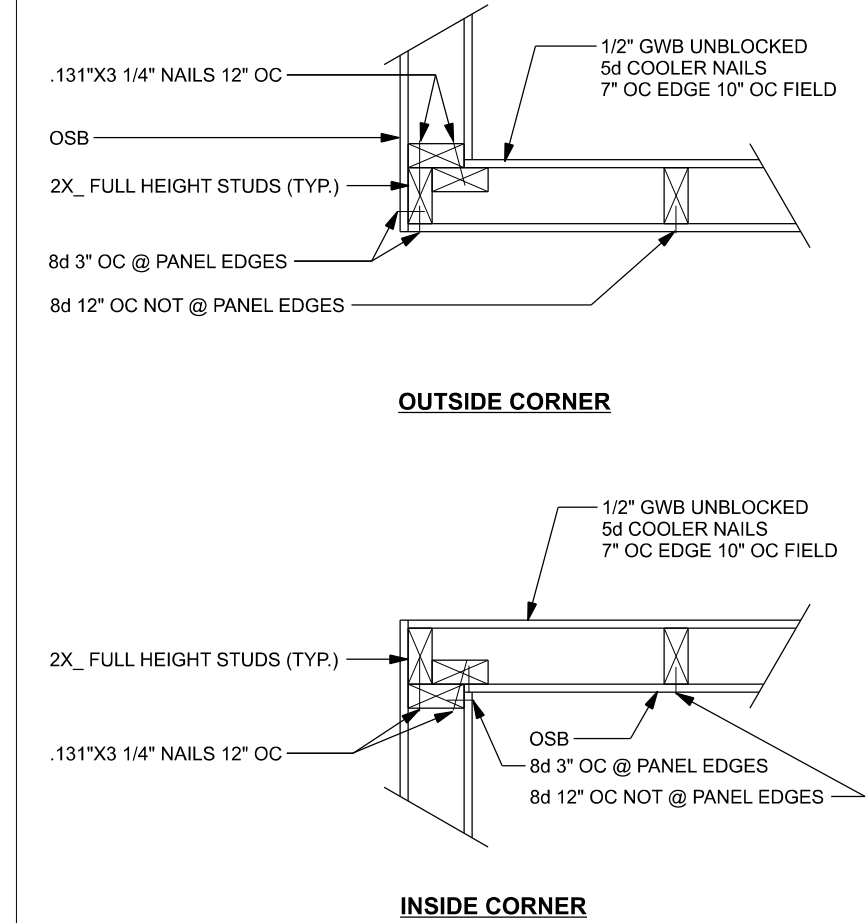




ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

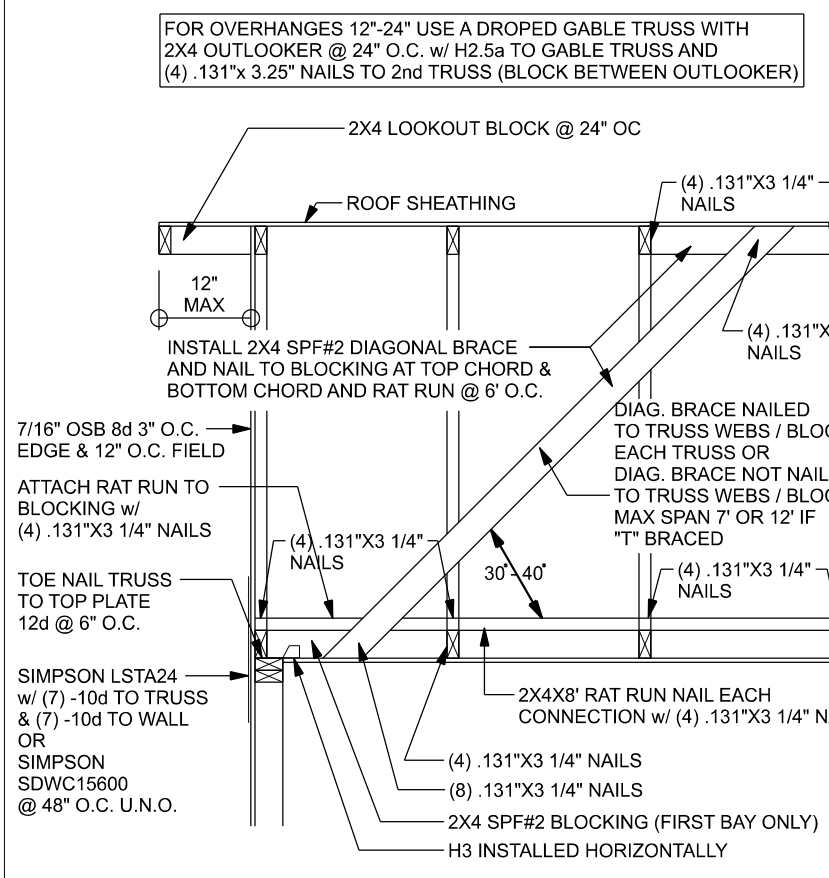


(TYP.) CORNER FRAMING
WOOD FRAME

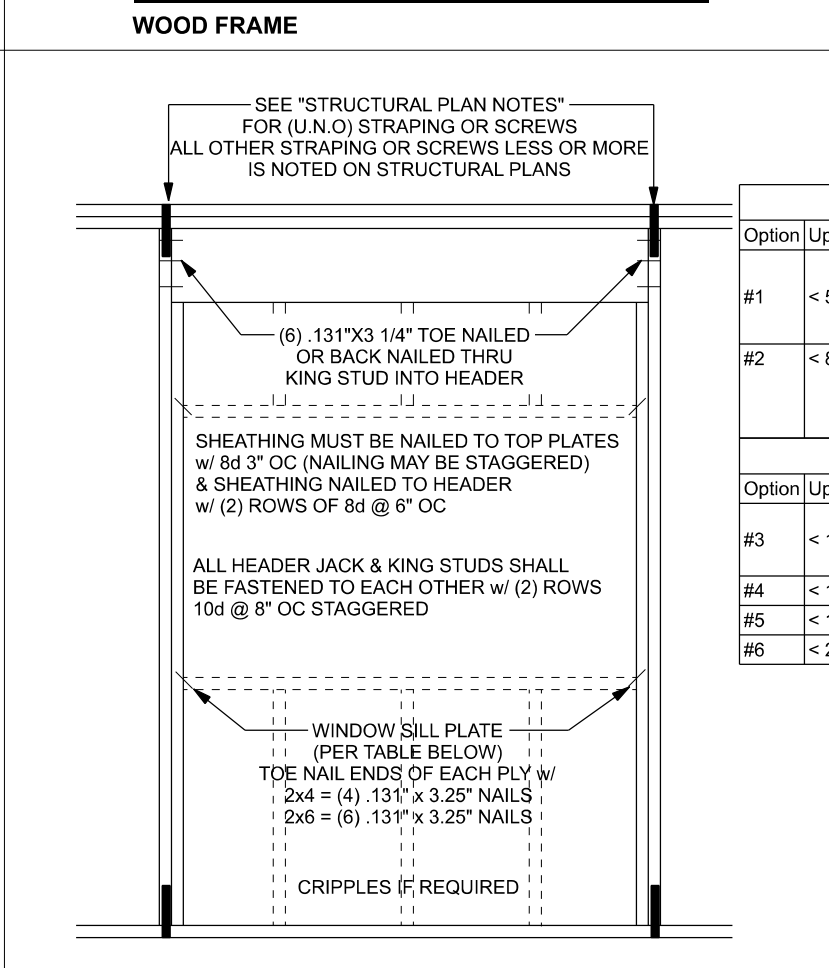
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSRs-01 (2 3/8" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. D	15/32"	ASTM F1667 RSRs-01 (2 3/8" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	4" oc	4" oc

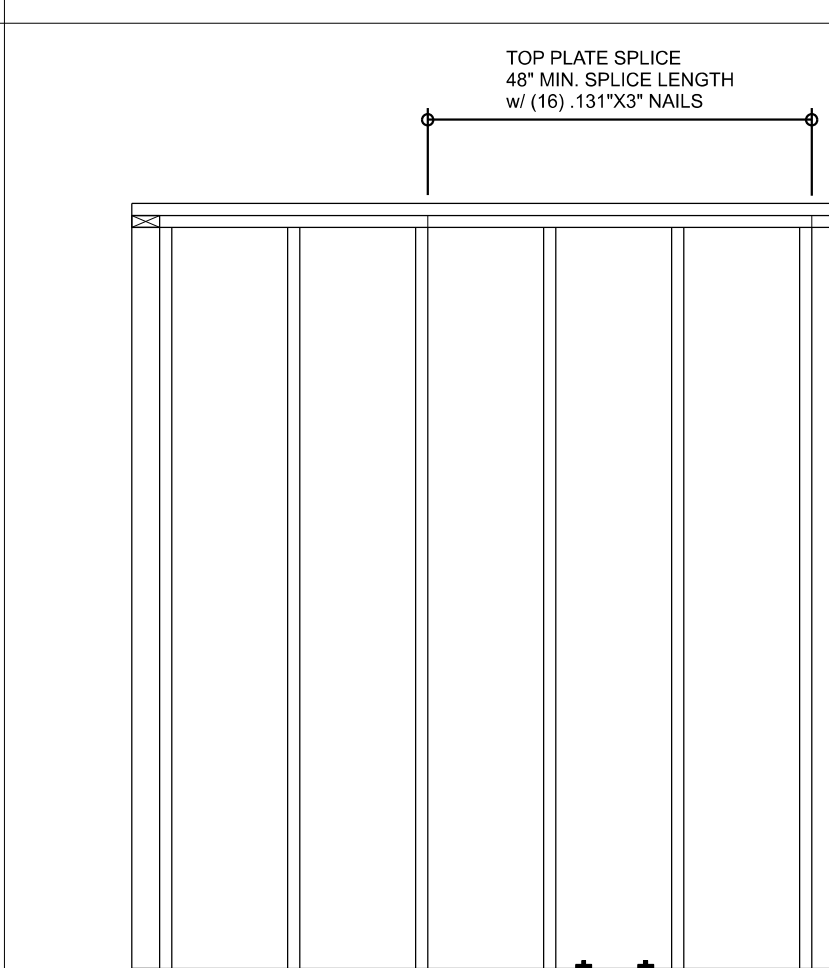
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 8 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



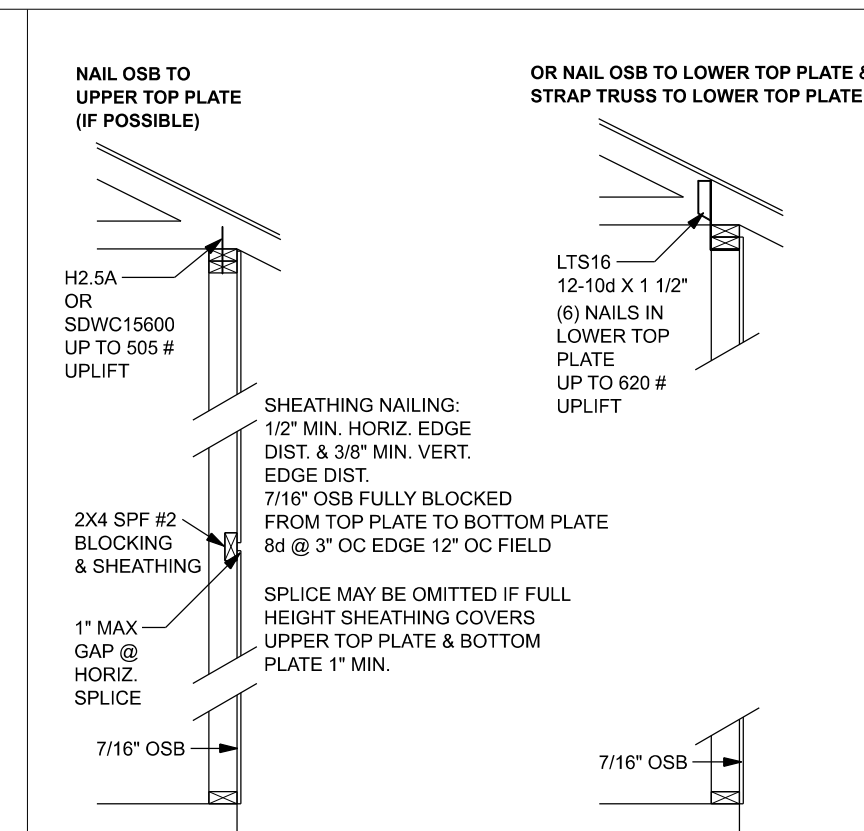
(TYP.) GABLE BRACING DETAIL
WOOD FRAME



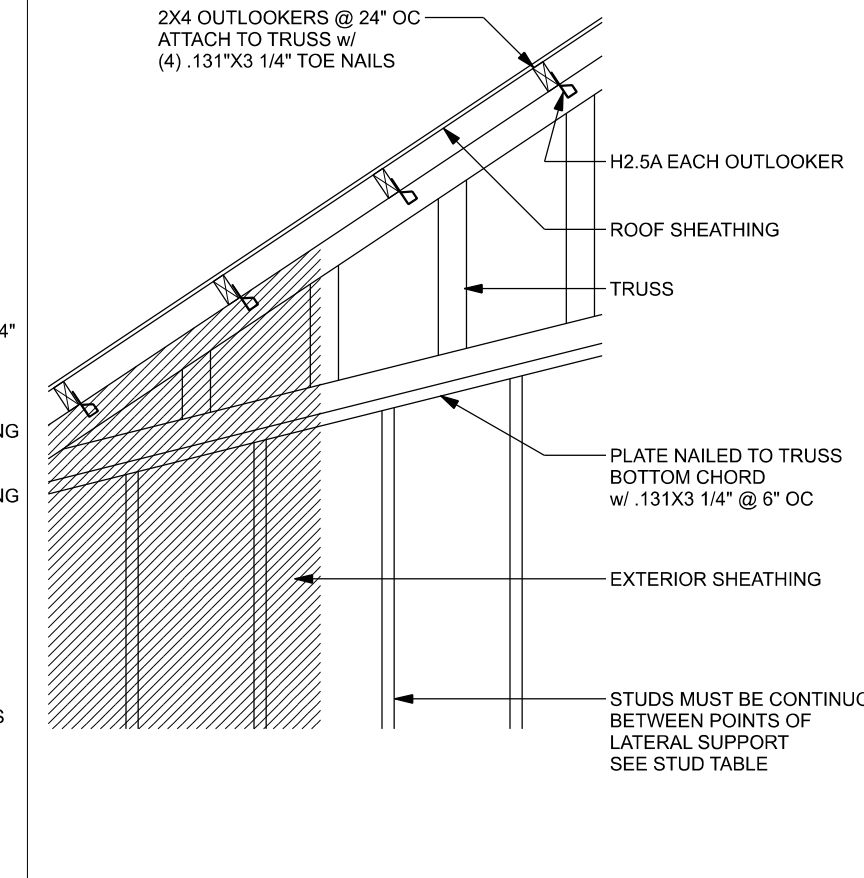
TYPICAL HEADER STRAPING OR SCREWS DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

HEADER SCREWS TABLE

Option/Uplift	Top Connection	Bottom Connection
#1 < 510	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#2 < 895	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations

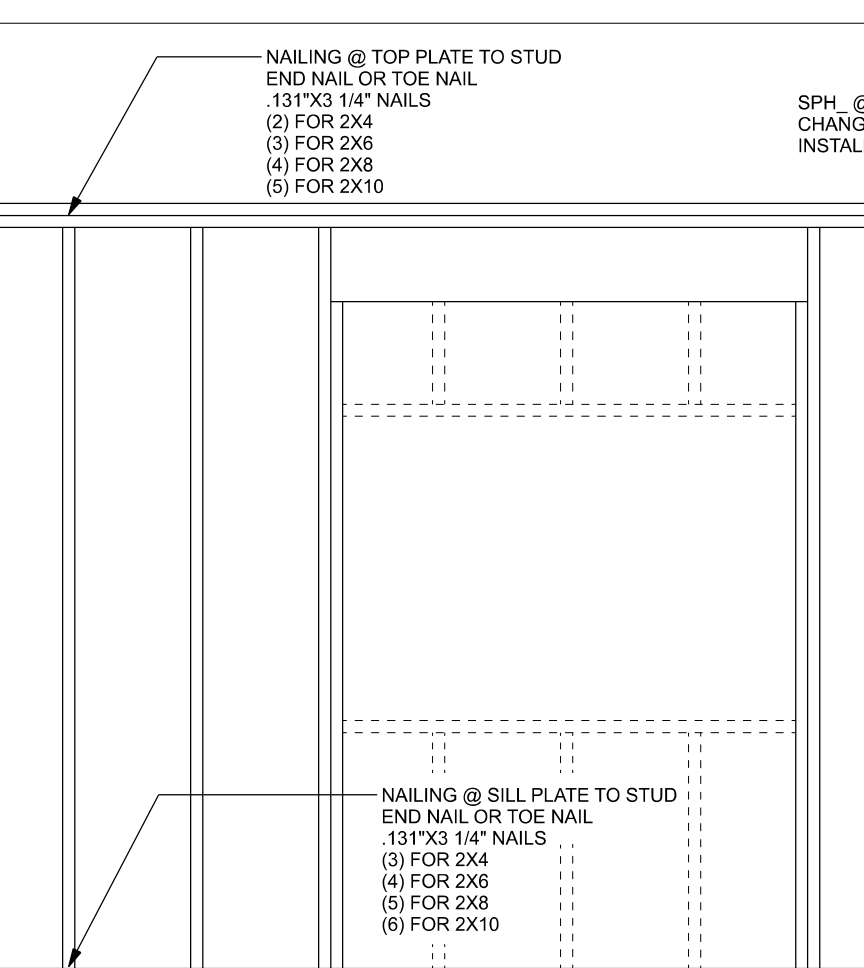
HEADER STRAP TABLE

Option/Uplift	Top Connection	Bottom Connection
#3 < 1235	LSTA24, (14), 148" x 1 1/2" wrap over plate	LSTA24, (14), 148" x 1 1/2" wrap under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#4 < 1455	MSTA24, 16-148"x1 1/2" header to jacks	DTT22
#5 < 1800	MSTA24, 16-148"x1 1/2" header to jacks	DTT22
#6 < 2910	MSTA24, 16-148"x1 1/2" header to jacks	HTT4

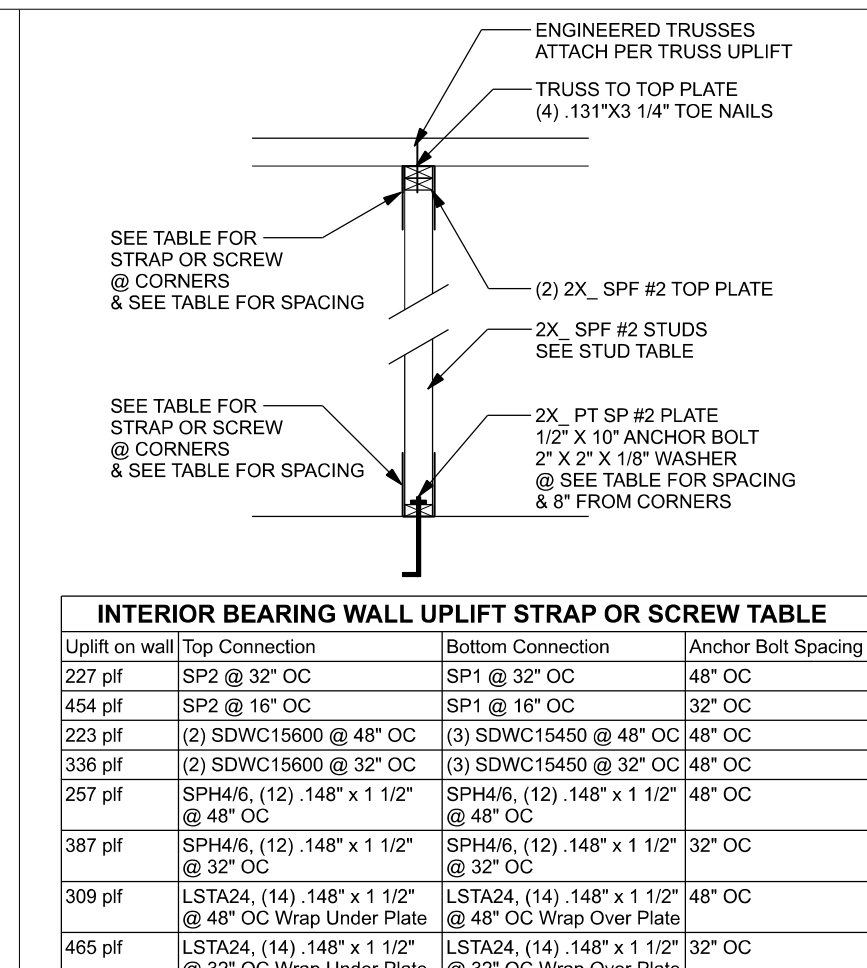
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM
(1) 2x4	(2) 2x4	(1) 2x6
(2) 2x6	(3) 2x6	(2) 2x6
(3) 2x8	(4) 2x8	(3) 2x8
(4) 2x10	(5) 2x10	(4) 2x10

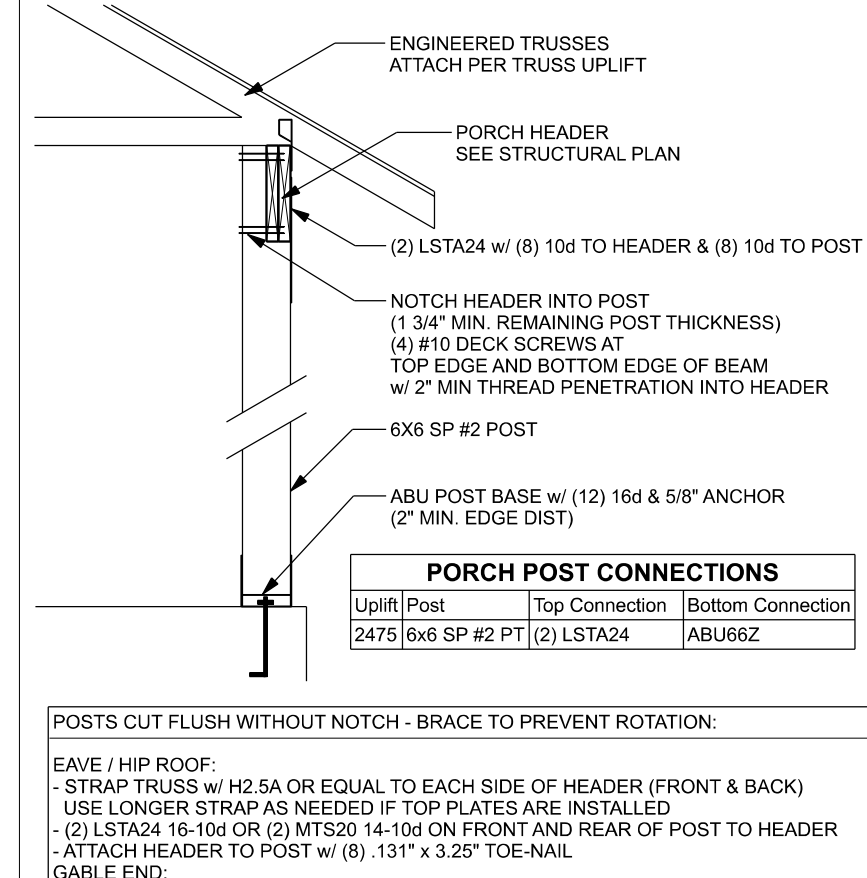
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



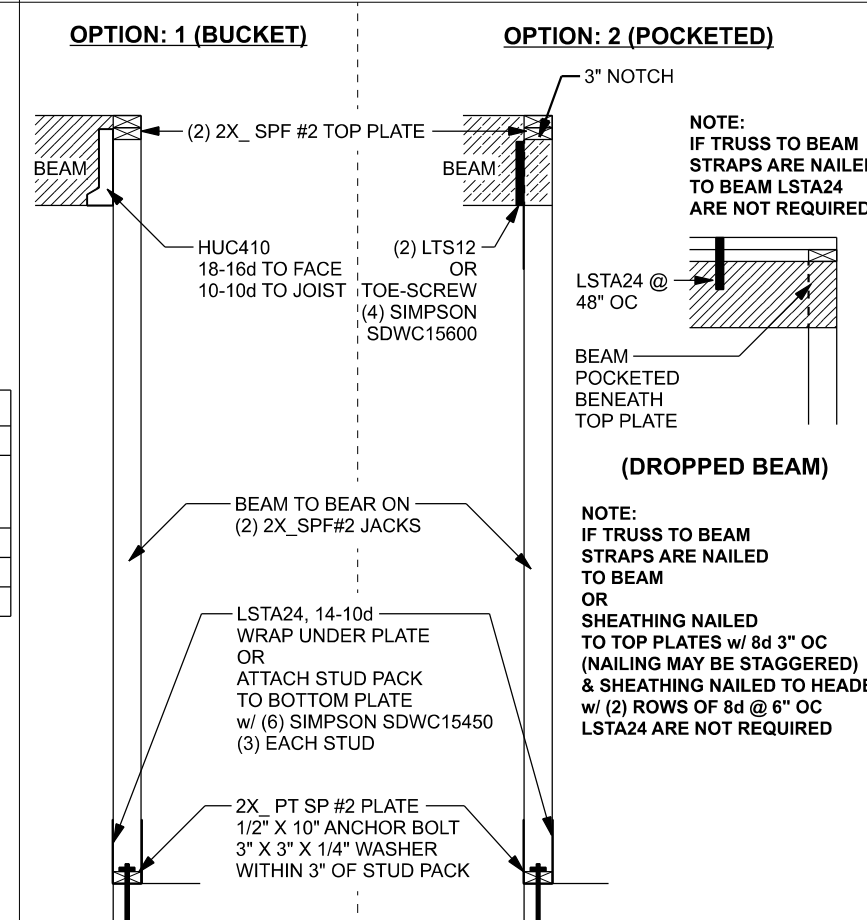
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



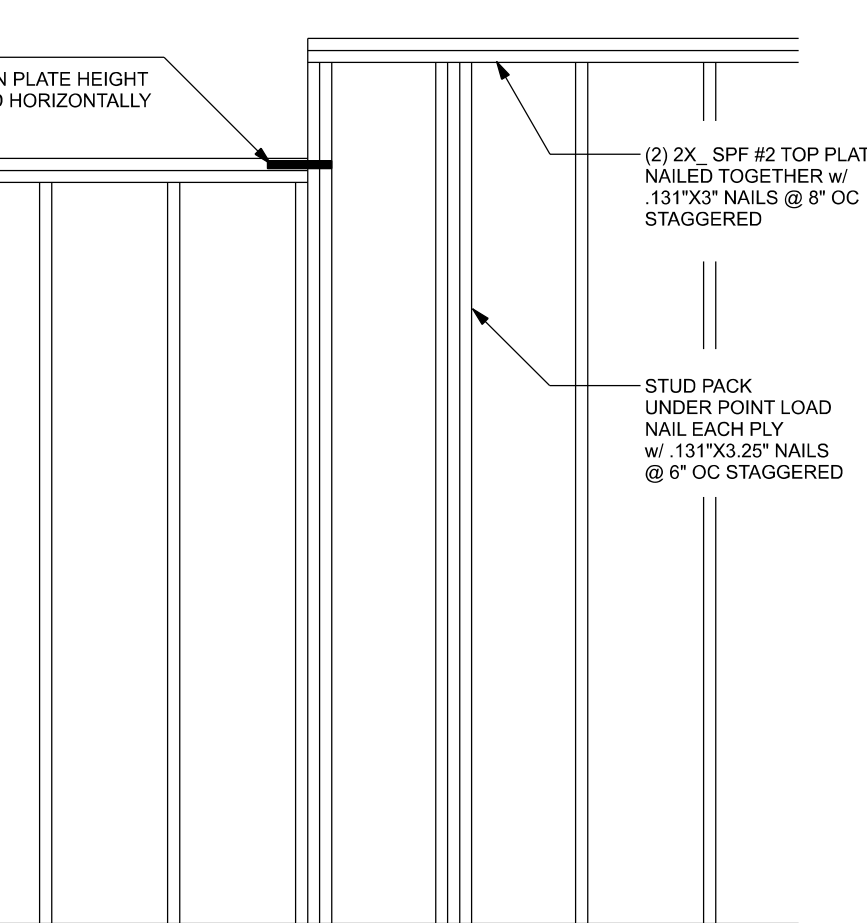
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



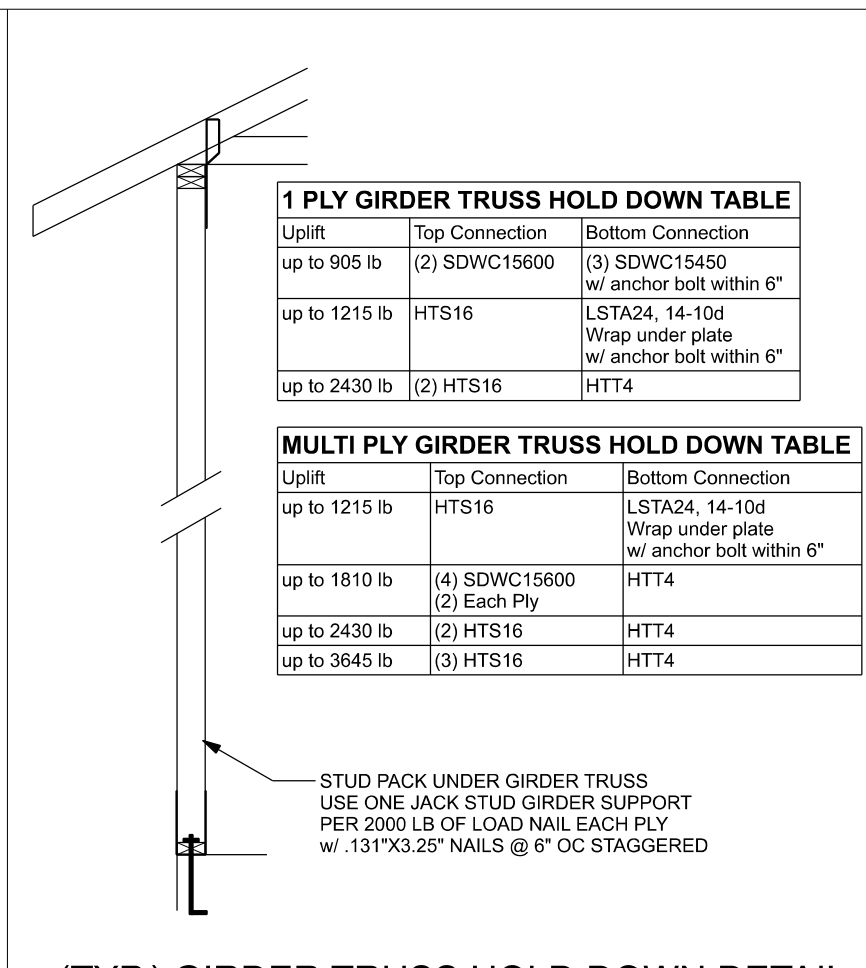
(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



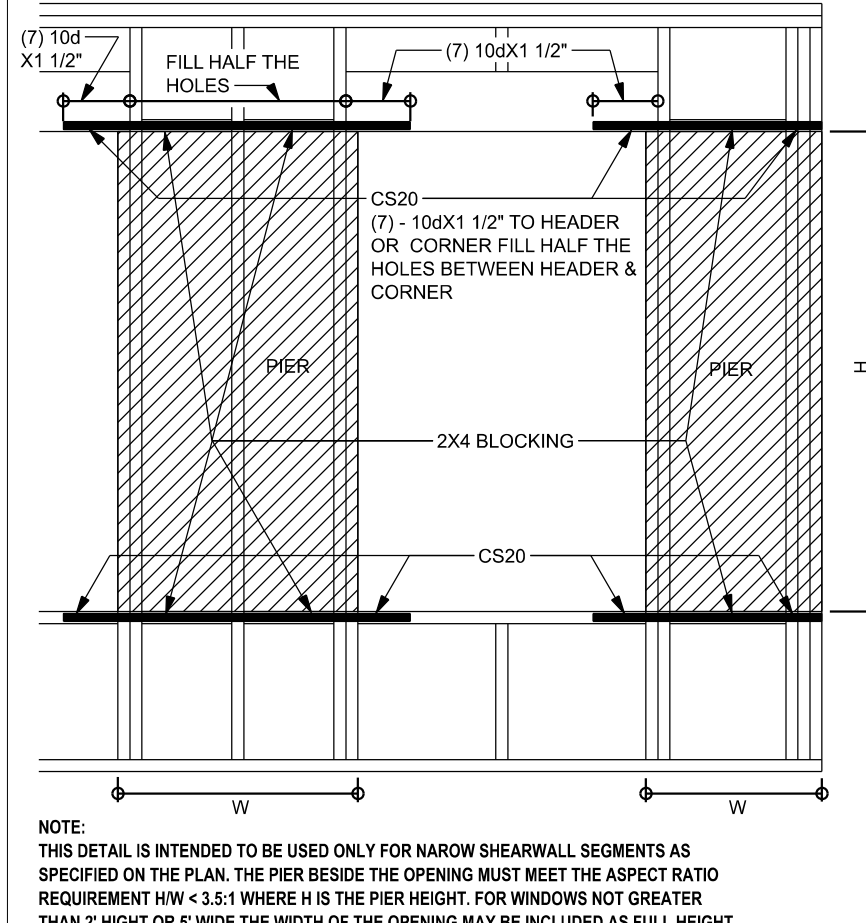
(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

CONNECTOR TABLE

Uplift SP/Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505 SDWC15600	-	-
400	290 H3	4-131"x1 1/2"	4-131"x1 1/2"
625	540 H2.5A	5-131"x1 1/2"	5-131"x1 1/2"
1040	1015 H10A	9-148"x1 1/2"	9-148"x1 1/2"
845	515 LTS12-20	6-148"x1 1/2"	6-148"x1 1/2"
990	850 MTS12-30	7-148"x1 1/2"	7-148"x1 1/2"
1415	1215 HTS20-30	8-148"x1 1/2"	8-148"x1 1/2"
1235	1235 LSTA21	8-148"x1 1/2"	8-148"x1 1/2"
1640	1460 MSTA24	9-148"x1 1/2"	9-148"x1 1/2"
1030	1030 CS20	7-148"x1 1/2"	7-148"x1 1/2"
555	535 SP1	4-148"x3"	4-148"x3"
1010	605 SP2	6-148"x3"	6-148"x3"
1280	1100 SPH48	12-148"x1 1/2"	wrap under or over plate w/ anchor bolt within 6"
771	771 LSTA24	10-148"x1 1/2"	wrap under or over plate
1235	1235 LSTA24	14-148"x1 1/2"	wrap under or over plate
Uplift SP/Uplift SPF	Holdowns @ Stemmwall	To Stud / Post	Anchor
2145	1835 DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640 HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP/Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
2145	1835 DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640 HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP/Uplift SPF	Post Bases @ Stemmwall	To Post	Anchor
1900	ABU44Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
2475	ABU6Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP/Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900	ABU44Z	12-162"x3 1/2"	5/8"x7" Drill & Epoxy
2475	ABU6Z	12-162"x3 1/2"	5/8"x7" Drill & Epoxy

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 10' O.C. x 0.8 = 12.8' O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND	1700	1.7
LSL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT AND TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE 10FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO ENGINEER'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS), UNO, ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 318-11, SECTION 17.4.2, E.T.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OBTAIN A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

DESIGN CRITERIA & LOADS:

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-16, 3S (UST))	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (F _{T2})	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6 (Vasd) -27.8 (Vasd)	+25.6 (Vasd) -34.2 (Vasd)
0 - 20	+42.6 (Vult) -46.2 (Vult)	+42.6 (Vult) -57 (Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

	+22.6 (Vasd) -25.5 (Vasd)
9x7 GARAGE DOOR	
16x7 GARAGE DOOR	+21.7 (Vasd) -24.1 (Vasd)

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

Netties Res.

PROJECT ADDRESS:
Columbia County, FL

FL PE 53915

This Item has been digitally signed and sealed by Mark Disoway P.E. on digital signature date Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01410C0000017E97DE07CA000746F0, CN=Mark d Disoway

2023-12-28 14:10:52

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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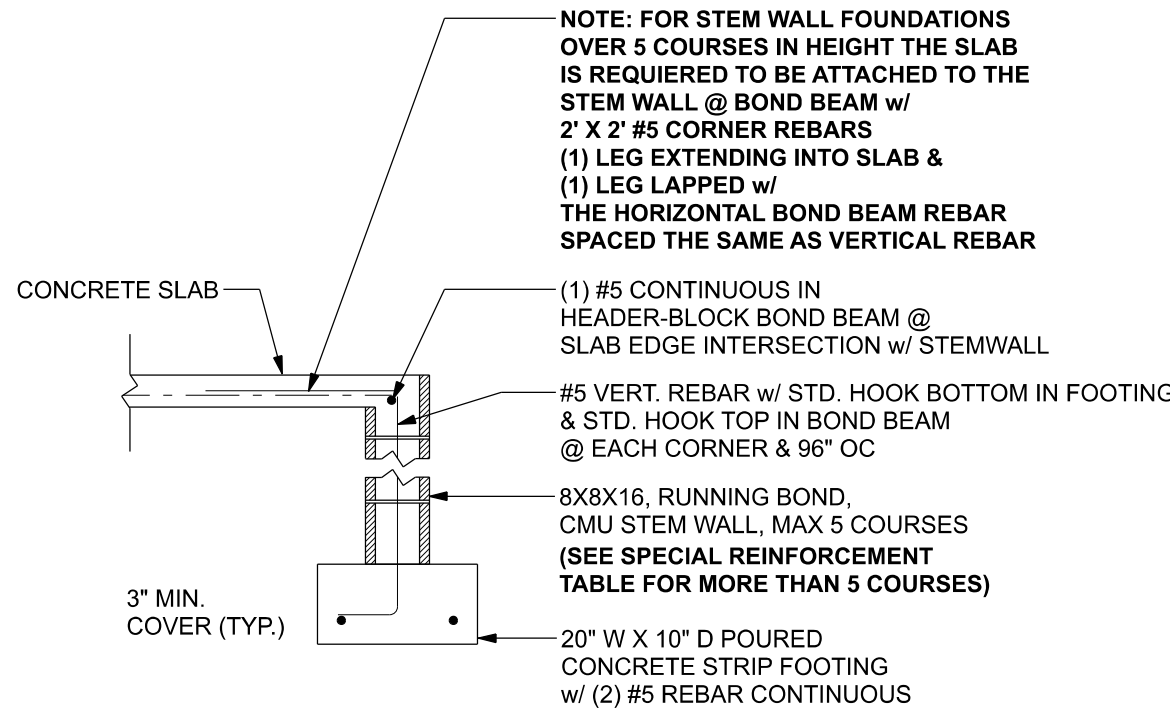
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
231515

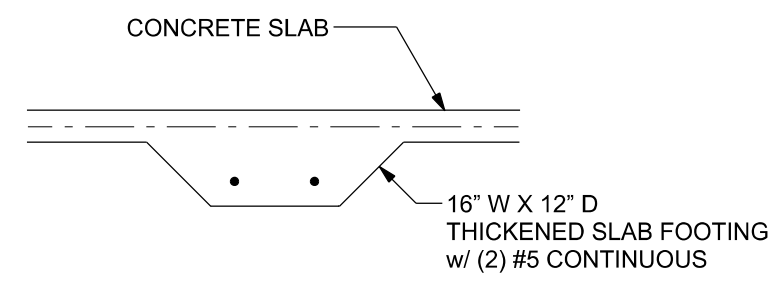
S-1
OF 3 SHEETS



F1
S-2

STEM WALL FOOTING

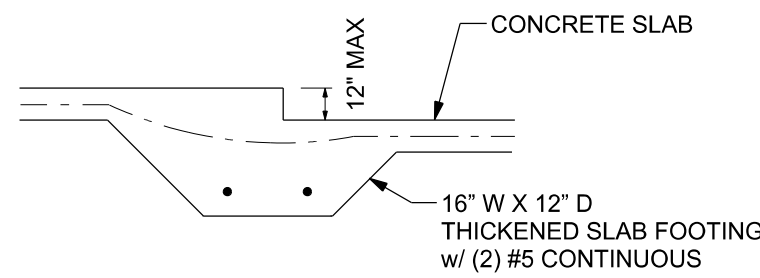
SCALE: 1/2" = 1'-0"



F2
S-2

INTERIOR BEARING FOOTING

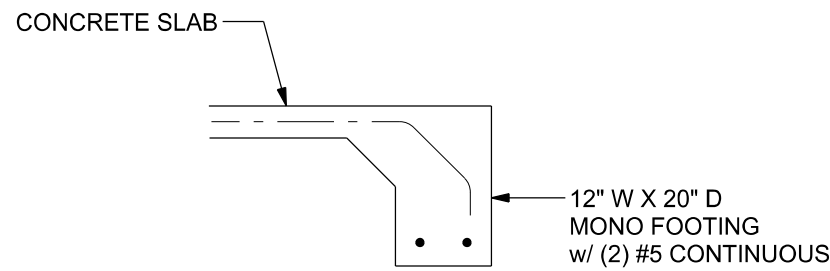
SCALE: 1/2" = 1'-0"



F3
S-2

INTERIOR BEARING STEP FOOTING

SCALE: 1/2" = 1'-0"



F4
S-2

MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE:						
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.						
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)	
		#5	#7	#8	#5	#7
3.3	3.0	96	96	96	96	96
4.0	3.7	96	96	96	96	96
4.7	4.3	88	96	96	96	96
5.3	5.0	56	96	96	96	96
6.0	5.7	40	80	96	80	96
6.7	6.3	32	56	80	56	96
7.3	7.0	24	40	56	40	80
8.0	7.7	16	32	48	32	64
8.7	8.3	8	24	32	24	48
9.3	9.0	8	16	24	16	40

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

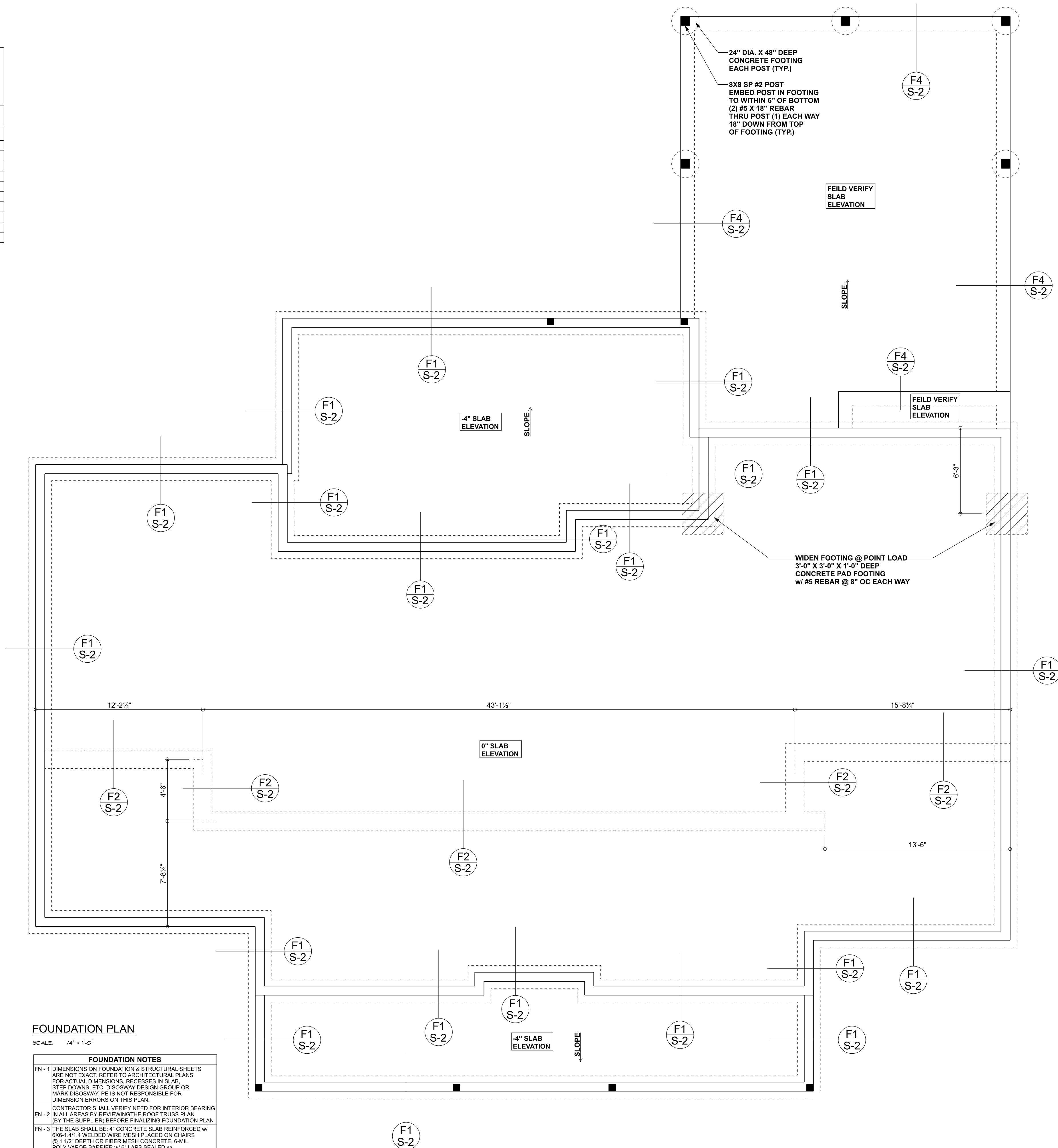
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
- FN - 2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN
- FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 8X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1-1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITTE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITTE-TREATMENT METHOD CAN BE USED INSTEAD)



Nettles Res.

PROJECT ADDRESS:
Columbia County, FL

FL PE 53915

This item has been digitally signed and sealed by Mark Disoway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01410C0000017E970, CN=Mark d Disoway
2023-12-28 14:11:15

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
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disowaydesign@gmail.com

JOB NUMBER:
231515

S-2
OF 3 SHEETS

