	1	2
	GENERAL NOTE	STRUCTURAL STEEL NOTES
	EXISTING STRUCTURE. EVERY EFFORT HAS BEEN MADE TO IDENTIFY DISCREPANCIES AND ENSURE THAT THE DETAILS ARE DEPICTED CORRECTLY. THESE DRAWINGS HAVE BEEN PREPARED BASED ON INFORMATION AVAILABLE HOWEVER SINCE THIS CONSTRUCTION IS WITHIN/ADJACENT TO AN EXISTING	 ALL STRUCTURAL STEEL WORK SHALL CONFORM TO 360-16 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" OF THE A.I.S.C.
	STRUCTURE, THE BUILDER CAN EXPECT AND SHOULD PLAN ON ENCOUNTERING VARIANCES AND DEVIATIONS BETWEEN THE INFORMATION FOUND IN THESE DRAWINGS AND THE EXISTING SITE CONDITIONS. ACTUAL FIELD CONDITIONS MAY REQUIRE MODIFICATIONS IN CONSTRUCTION DETAILS AND QUANTITIES. THE BUILDER IS RESPONSIBLE TO FIELD VERIFY ALL DETAILS INCLUDING GEOMETRY AND FLEVATIONS PRIOR TO THE INSTALLATION OF ANY MATERIAL	 FABRICATORS AND ERECTORS SHALL BE MEMBERS OF THE A.I.S.C. OR CERTIFIED FOR CATEGORY I, A.I.S.C. QUALITY CERTIFICATION PROGRAM OR HAVE AT LEAST 10 YEARS EXPERIENCE IN FABRICATION AND ERECTION OF SIMILAR STEEL STRUCTURES. SHOP DRAWINGS FOR ALL STRUCTURAL STEEL SHALL BE SUBMITTED AND APPROVED PRIOR TO ANY
	REINFORCED CONCRETE NOTES	 FABRICATION. 4. STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL BE A MINIMUM OF 3/4" DIAMETER ASTM A325 BOLTS UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS SHALL BE "SNUG TIGHT" UNLESS NOTED OTHERWISE. TC BOLTS MAY BE USED. ALL CONNECTIONS SHALL HAVE A MINIMUM OF A DOLTO UNLESS NOTED OTHERWISE. TO BOLY BE USED. ALL CONNECTIONS SHALL HAVE A
	 ALL CONCRETE WORK SHALL CONFORM TO ACI 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". ALL CONCRETE PLACED SHALL BE VIBRATED BY MECHANICAL VIBRATORS. 	 MINIMOM OF 2 BOLTS U.N.O. DESIGN CONNECTIONS TO PROVIDE SUITABLE ACCESS FOR FIELD INSTALLATION. 5. ALL WELDS SHALL CONFORM TO LATEST EDITION OF AWS D1.1, "STRUCTURAL WELDING CODE". ALL GR
D	3. COMPLETE FABRICATION AND PLACING DRAWINGS FOR REINFORCING STEEL SHALL BE SUBMITTED FOR APPROVAL. NO FABRICATION MAY BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND APPROVED.	 WELDS SHOWN ON CONTRACT DOCUMENTS SHALL BE FULL PENETRATION UNLESS NOTED OTHERWISE WELDING SHALL BE DONE WITH E-70XX ELECTRODES UNLESS NOTED OTHERWISE. 6. STRUCTURAL STEEL CONNECTIONS SHALL BE DESIGNED BY A QUALIFIED PROFESSIONAL ENGINEER
	 4. LAP SPLICES FOR REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 318. SPLICES SHALL BE AS CALLED FOR IN THE LATEST EDITION OF THE CRSI "MANUAL OF STANDARD PRACTICE". 5. REINFORCING OF ALL CONCRETE MEMBERS SHALL HAVE THE FOLLOWING CLEAR CONCRETE COVER: MEMBERS 	LICENSED IN THE STATE WHERE THE PROJECT IS BEING CONSTRUCTED. STRUCTURAL ANALYSIS DATA BE SIGNED/SEALED BY THE QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARAT SHALL BE PROVIDED WITH THE SHOP DRAWING SUBMITTALS. DESIGN CONNECTIONS TO PROVIDE SUIT. ACCESS FOR FIELD INSTALLATION WHERE APPLICABLE.
	CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 CONCRETE EXPOSED TO EARTH OR WEATHER:	7. UNLESS OTHERWISE SHOWN, ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED OR SEATED CONNECTIONS. UNLESS GREATER REACTIONS ARE INDICATED ON THE PLANS, CONNECTIONS SHALL DEVELOP AT LEAST ONE-HALF OF THE TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE TABLES OF THE MANUAL FOR THE GIVEN SHAPE AND SPAN OF THE BEAM IN QUESTION. IN NO CASE, HOWEVER, SH THE LENGTH OF THE FRAMED CONNECTIONS BE LESS THAN ONE-HALF OF THE "T" DISTANCE OF THE BE WEB.
	#6 BARS THROUGH #18 BARS2#5 BARS OR SMALLER1 1/2	8. WHERE PRACTICAL, UNLESS SHOWN DIFFERENTLY ON DRAWINGS, ALL BRACING CONNECTIONS SHALL DESIGNED AND DETAILED SO THAT ALL FORCE COMPONENTS CAN BE DELIVERED DIRECTLY TO THE CENTERLINE OF INTERSECTING MEMBERS. WHERE THIS IS NOT DONE, CONNECTIONS SHALL BE DESIGN TO ACCOUNT FOR PECH. THIS ECCENTRICITIES.
	CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH:	 SHOP OR FIELD SPLICES NOT SHOWN ON THE CONTRACT DOCUMENTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL.
	#14 AND #18 BARS 1 1/2 #11 BARS AND SMALLER 3/4	10. THE STRUCTURAL STEEL ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STRUCTURAL STEE FRAME WORK AGAINST LATERAL LOADING, SUCH AS WIND. THE BRACING SHALL REMAIN IN PLACE UNT THE FINAL SYSTEM FOR RESISTING LATERAL LOADS IS IN PLACE AND EFFECTIVE AS APPROVED BY THE
	6. THE CONTRACTOR SHALL PROVIDE ADEQUATE SUPPORT FOR REINFORCING TO ENSURE THE REINFORCEMENT REMAINS IN ITS INTENDED POSITION WHEN CASTING CONCRETE. THE SUPPORT SHALL BE CONSTRUCTED OF A TYPE THAT WILL NOT DAMAGE THE VAPOR BARRIER BELOW.	STRUCTURAL ENGINEER. 11. GUSSET PLATES SHALL BE 3/8" THICK MINIMUM.
	7. SPLICE WELDING OF REINFORCING STEEL SHALL BE DONE IN STRICT ACCORDANCE WITH THE AMERICAN WELDING SOCIETY "REINFORCING STEEL WELDING CODE". PREHEATING OF THE REINFORCING SHALL BE BASED ON THE CARBON EQUIVALENCY DETERMINED FROM REINFORCING MILL REPORTS. GRADE 40 REINFORCING SHALL BE WELDED WITH E90XX LOW HYDROGEN FLECTBODES	 BEAMS SHALL BE FABRICATED AND ERECTED WITH NATURAL CAMBER UP. THE CONTACT SURFACES WITHIN SLIP CRITICAL JOINTS SHALL BE FREE FROM OIL, PAINT, LACQUER OF GALVANIZING.
	8. MANUAL WELDING OF REINFORCING BARS FOR EMBEDS IS NOT ALLOWED.	14. STRUCTURAL STEEL FRAMING SHALL BE ERECTED TRUE AND PLUMB IN ACCORDANCE WITH A.I.S.C. CO OF STANDARD PRACTICE. ANY FRAMING EXCEEDING TOLERANCES OF THE CODE OF STANDARD PRACT SHALL BE CORRECTED AS DIRECTED BY THE STRUCTURAL ENGINEER.
	9. SAWN JOINTS IN SLABS-ON-GRADE SHALL SHALL BE COT AS SOON AS POSSIBLE AFTER THE CONCRETE HARDENS. JOINTS CAN BE CUT WITH A POWER BLADE WITHIN 4 TO 12 HOURS AFTER THE SLAB HAS BEEN CAST AND FINISHED. JOINTS MUST BE CUT AS SOON AS THE CONCRETE IS FIRM ENOUGH NOT TO BE TORN OR DAMAGED BY THE BLADE. THE CONCRETE IS HARD ENOUGH WHEN THE BLADE DOES NOT DISLODGE ANY AGGREGATE AND WHEN THE EDGES OF THE CUT DO NOT RAVEL.	15. ROOF DECKING SHALL BE INSTALLED IN ACCORDANCE WITH THE STEEL DECK INSTITUTE AND MANUFACTURER'S RECOMMENDATIONS. DECKING SHALL BE PLACED IN A THREE-SPAN CONTINUOUS CONDITION. SINGLE OR DOUBLE SPAN CONDITIONS REQUIRE PRIOR APPROVAL BY THE STRUCTURAL ENGINEER.
	10.CONTROL JOINTS IN ALL FOUNDATION AND RETAINING WALLS SHALL BE PLACED NOT MORE THAN 15'-0" APART AND SHALL BE 3/4" V-CHAMFERED ON BOTH SIDES, UNLESS NOTED OTHERWISE. EXPANSION JOINTS SHALL BE LOCATED AS NOTED ON THE PLANS. SEE TYP. DETAIL SHEET.	 STRUCTURAL STEEL EMBEDDED IN CONCRETE SHALL NOT BE PAINTED. GROUT USED IN GROUT BEDS UNDER COLUMN BASE PLATES SHALL BE CEMENT BASED, NON-SHRINK, NON METALLIC CROUT THE CROUT SHALL EXHIBIT NO SHRINKACE IN ACCORDANCE WITH ASTM (2027)
С	 11.CHAMFER EXPOSED CONCRETE EDGES 3/4". 12.CONCRETE WALLS SHALL BE REINFORCED AROUND ALL OPENINGS WITH 2~#5 BARS ON ALL SIDES AND EXTENDED 2'-0" BEYOND OPENING UNLESS NOTED OTHERWISE. 	"TEST METHOD FOR EARLY VOLUME CHANGE OF CEMENTITIOUS MIXTURES" AND SHALL HAVE A MINIMU 28-DAY COMPRESSIVE STRENGTH OF 5000 PSI WHEN TESTED IN ACCORDANCE WITH ASTMC-109-80, "TE METHOD FOR COMPRESSIVE STRENGTH OF HYDRAULIC CEMENT MORTARS".
	 13. THE LONGITUDINAL REINFORCING STEEL IN BOND BEAMS, WALLS AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS. SEE TYP. DETAIL SHEET. 14. THE CONCRETE MIX DESIGNS ARE THE RESPONSIBILITY OF THE SUPPLIER. MIX DESIGNS SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER PRIOR TO CONCRETE PLACEMENT. 	 18. K-JOISTS SHALL BE ATTACHED TO THEIR SUPPORT W/A MIN. OF 3/16 FILLET WELD x 2 LONG EACH SIDI JOIST SEAT OR EQUIVALENT. LH-JOISTS SHALL BE ATTACHED TO THEIR SUPPORT W/A MIN. OF 1/4" FILLET WELD x 2" LONG EACH SIDE OF JOIST SEAT. JOIST GIRDERS SHALL BE ATTACHED TO THEIR SUPPORT W MIN. OF 1/4" FILLET WELD x 4" LONG EACH SIDE OF GIRDER SEAT OR EQUIVALENT. 18. K-JOISTS SHALL BE ATTACHED TO THEIR SUPPORT W/A MIN. OF 3/16 FILLET WELD x 2 LONG EACH SIDI WELD x 2" LONG EACH SIDE OF JOIST SEAT. JOIST GIRDERS SHALL BE ATTACHED TO THEIR SUPPORT W MIN. OF 1/4" FILLET WELD x 4" LONG EACH SIDE OF GIRDER SEAT OR EQUIVALENT.
	15. A MIX DESIGN SHALL BE PROVIDED FOR EACH TYPE OF CONCRETE. THE MIX DESIGNS SHALL CLEARLY INDICATE ALL MATERIALS USED IN THE MIX, THE TYPE (USE) OF THE CONCRETE MIX, AND ALL MIX	 (+ OR C) INDICATES TENSION IN MEMBERS. (+ OR C) INDICATES COMPRESSION IN MEMBERS. 20. ALL BOTTOM CHORD BRACING, SWAY FRAMES, X-BRACING, LACE AND SIMILAR TYPE MEMBERS
	EACH-UP, DATA, TO INCLUDE BUT NOT LIMITED TO, THE COMPRESSIVE STRENGTH BREAK DATA FOR EACH MIX BEING SUBMITTED, FINE AND COARSE AGGREGATE GRADATIONS, AND ALL ADMIXTURES. 16. THE USE OF FLY ASH IS PROHIBITED IN ANY EXPOSED MEMBERS. FLY ASH SHALL BE ALLOWED IN	SHALL EITHER DEVELOP THE FORCE INDICATED ON THE DRAWINGS OR ONE-HALF THE ALLOWABLE TENSION FORCE IN THE MEMBER, WHICHEVER IS LARGER. 21.BAR JOISTS SHALL BE FABRICATED AND ERECTED, BRACED WITH RIGID BRIDGING AND ANCHORED TO
	FOUNDATIONS AND SUB-SLABS UPON ENGINEER'S APPROVAL. CONCRETE MIX DESIGNS SHALL BE SUBMITTED AND APPROVED PRIOR TO CASTING ANY CONCRETE. 17. CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C150. TYPE 1. MILL CERTIFICATE SHALL	SUPPORTING MEMBERS IN STRICT ACCORDANCE WITH THE STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE LATEST EDITION.
	SHOW TOTAL ALKALINE CONTENT NOT EXCEEDING 0.6%. 18. CONCRETE SHALL HAVE A MAXIMUM SLUMP = 4 ± 1 " UNLESS APPROVED BY ENGINEER.	REQUIRED WHEN EQUIPMENT IS HUNG FROM THE BOTTOM CHORD PAST LAST PANEL POINT. 23. JOISTS AND JOIST GIRDERS SHALL BE DESIGNED FOR LOADS INDICATED ON PLANS. SEE SPECIAL LOAD
	19. THE MIX SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.50. TROWEL FINISHED INTERIOR SLABS SUBJECTED TO VEHICULAR TRAFFIC SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.53.	TABLES FOR SPECIAL JOIST (SP) LOADING AND SPECIAL JOIST GIRDER (GSP) LOADING. 24.JOIST AND GIRDER FABRICATOR SHALL SUBMIT SIGNED AND SEALED CALCULATIONS BY A REGISTERED ENGINEER SHOWING ALL LOADS AND SPECIAL CONDITIONS TOGETHER WITH SHOP DRAWINGS AND
	 20. ALL INTERIOR TROWEL FINISHED SLABS, SUBJECTED TO VEHICULAR TRAFFIC, SHALL HAVE A MAXIMUM <u>ENTRAPPED</u> AIR CONTENT OF 3%. 21. WATER SHALL BE POTABLE. CLEAN AND EBEE FROM OUS ACIDS. SALTS AND OTHER DELETERIOUS. 	BILL OF MATERIAL PRIOR TO ERECTION OF JOISTS AND GIRDERS. 25. JOISTS AND GIRDERS SHALL BE DESIGNED FOR UPLIFT AS INDICATED ON PLANS. BOTTOM CHORD BRAC
	 SUBSTANCES. 22.ALL EXPOSED EXTERIOR CONCRETE, SUBJECTED TO FREEZE/THAW CYCLES, SHALL ACHIEVE AN AIR CONTENT OF 6% ± 1.5. 	 26. ALL JOIST GIRDERS SHALL HAVE BOTTOM CHORD BRACES AT MID SPAN AND AS DESIGNATED ON JOIST DRAWINGS. 27. WIND POSTS TERMINATE 11/2" BELOW, JOIST AND JOIST GIRDER BOTTOM CHORD.
	23. WATER REDUCING AGENTS SHALL CONFORM TO ASTM C494.1 24. AIR ENTRAINING AGENTS SHALL CONFORM TO ASTM C260.	28. ALL SAG RODS SHALL BE 5/8"Ø A36 SMOOTH BAR WITH THREADED ENDS. DOUBLE NUT EA. END OF SAG
	25. AGGREGATE SHALL CONFORM TO ASTM C33. COARSE AGGREGATE SHALL BE CRUSHED ROCK OR CRUSHED GRAVEL AND SHALL BE GRADED FROM 1 1/2" TO A NO. 4 SIEVE FOR WALLS AND SLABS. A LARGER AGGREGATE, GRADED FROM 2" TO NO. 4 SIEVE FOR FOUNDATIONS IS ACCEPTABLE. COMBINED AGGREGATE GRADATION SHALL BE 8% TO 18% RETAINED ON EACH SIEVE BELOW THE TOP SIZE AND ABOVE THE NO. 100 SIEVE.	 29.ALL STRUCTURAL STEEL, MISCELLANEOUS STEEL AND STEEL JOISTS U.N.O. SHALL BE GIVEN ONE SHOP COAT AND ONE FIELD TOUCH UP PAINT, GRAY COLOR. 30.THE STEEL FABRICATOR SHALL INSPECT ALL ERECTED STEEL INCLUDING, BUT NOT LIMITED TO, STRUCTURAL SHAPES, JOISTS, JOIST GIRDERS AND METAL DECK. THE STEEL FABRICATOR SHALL BE RESPONSIBLE FOR THE ACCURACY, SIZE AND QUALITY OF FIELD CONNECTIONS.
В	26. AGGREGATES SHALL BE TESTED FOR SOUNDNESS IN ACCORDANCE WITH ASTM C88 AND FOR POTENTIAL REACTIVITY IN ACCORDANCE WITH ASTM C289. AGGREGATES USED FOR SLABS SHALL BE CONSIDERED INNOCUOUS WHEN TESTED IN ACCORDANCE WITH ASTM C289.	31.ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED AND ALL VENT HOLES SHALL BE PLUG OR SEAL WELDED PRIOR TO ERECTION.
	27. ALL CONCRETE TESTING SHALL BE PERFORMED BY AN APPROVED TESTING AGENCY AND IN ACCORDANCE WITH ALL APPLICABLE ASTM SPECIFICATIONS.	32.FOR ALL HANGING LOADS, PROVIDE HANGER WITHIN 6" OF JOIST PANEL POINT OR PROVIDE ADDITIONA WEB MEMBER AS SHOWN IN "CONCENTRATED LOAD ON JOIST DETAIL" (REF. TYP. DETAIL SHEET).
	28. SAMPLE EACH TYPE/CLASS OF CONCRETE AT A RATE OF NOT LESS THAN ONE PER DAY NOR LESS THAN ONE PER 100 CU. YD. NOR LESS THAN ONE FOR EACH 5,000 SQ. FT. OF SURFACE AREA FOR SLABS AND WALLS (REF ACI 318, 5.6). IF TOTAL VOLUME OF CONCRETE IS SUCH THAT FREQUENCY OF TESTING REQUIRED WOULD PROVIDE LESS THAN 5 STRENGTH TESTS FOR A GIVEN CLASS OF CONCRETE, TESTS SHALL BE MADE FROM AT LEAST 5 RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN 5 BATCHES ARE USED.	33.SUPPORT HEAVY LINE LOADS (I.E. ANY SPRINKLER OR SIMILAR MAINS <u>OVER</u> 3" DIA.) FROM CENTER OF SUPPORT MEMBER (WF BEAM, JOIST, JOIST GIRDER) UTILIZING CENTER BEAM CLAMP OR TRAPEZE SYSTEM FROM CENTER OF MEMBER. SIDE MOUNTED CLAMPS, (I.E. C-CLAMP OR ECCENTRIC BEAM CLAMP) WILL NOT BE ACCEPTED. ECCENTRIC LOAD ON JOIST IS UNACCEPTABLE. SIDE MOUNTED CLAMPS SHALL NOT SUPPORT ANY LOAD OVER 100 LBS. WHEN SUPPORTING MEMBER IS A JOIST OR JOIST GIRDER. (REF. "PIPE MAIN SUPPORT DETAIL" ON TYP. DETAILS SHEET)
	29. EACH SAMPLE SHALL BE TESTED FOR, AS A MINIMUM, SLUMP, AIR CONTENT, TEMPERATURE, UNIT WEIGHT, AND COMPRESSIVE STRENGTH.	INSULATED METAL PANEL (IMP)
	30. COMPRESSIVE STRENGTH TESTS SHALL BE AS FOLLOWS: 1 CYLINDER SHALL BE TESTED @ 7 DAYS AFTER POUR, 2 CYLINDERS SHALL BE TESTED @ 28 DAYS AFTER POUR AND 1 CYLINDER SHALL BE HELD IN RESERVE.	 INSULATED METAL PANEL (IMP) MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR PANEL STRENGTH AND ATTACHMENT TO CONCRETE, METAL DECK, STEEL, ETC (FASTENING PATTERNS)
	31. FLOOR FLATNESS, F(F) AND FLOOR LEVELNESS, F(L) SHALL BE TESTED IN ACCORDANCE WITH ASTM E1155. ACTUAL OVERALL F-VALUES SHALL BE CALCULATED USING THE INFERIOR/SUPERIOR AREA METHOD. A.) DOCK SLABS OVERALL F(F) 35, F(L) 25 B.) RACK STORAGE (FREEZER):	 MAXIMUM DEFLECTION = L/180 (EXT. WALL), L/120 (INT. WALL). DESIGN WIND PRESSURE SHALL BE BASED ON PRESSURES RELATED TO A TRIBUTARY AREA OF 20 SQ FT. LISTED IN THE COMPONENTS AND CLADDING TABLE ON THIS DRAWING. WIND AND THERMAL LOADING NEED NOT BE COMBINED IF THE STEEL BUCKLING STRESS DUE TO THERMAL WARP IS LESS THAN 1/2 THE YIELD STRENGTH OF THE METAL SKIN. FOR THERMAL LOADING. THE MAXIMUM OUTSIDE AMBIENT TEMPERTURE FOR IMP SHALL BE 90 €
	FMIN100 C.)ELEVATED SLABS: OVERALL F(F) 25, F(L) 17 D.) SLABS SUPPORTING TANKS AND SPECIALTY EQUIPMENT	
))) Î	REFERENCE EQUIPMENT DWGS FOR SPECIAL REQUIRMENTS RELATED TO FLATNESS AND LEVELNESS. 32. PROVIDE A STANDARD TROWEL FINISH ON ALL SUB-SLABS UNLESS OTHERWISE INDICATED.	STRUCTURAL SHEET LIST
	33. ALL NEW SLABS SHALL PREFERABLY BE WET CURED FOR A MINIMUM OF 7 DAYS, AS APPLICABLE. REF. NOTES 36 AND 37 FOR ALTERNATE CURING METHODS.	S0001 STRUCTURAL GENERAL NOTES & INSPECTION SCHEDULE S1000 OVERALL FOUNDATION PLAN S1010 PARTIAL FOUNDATION PLAN - AREA 1
5	34. FLOOR JOINTS AS INDICTED ON DESIGN DRAWINGS OR PROPRIETARY SLAB DRAWINGS. ALL JOINTS SHALL BE AIR CLEANED AND FILLED WITH JOINT FILLER AS INDICATED BELOW: WEAR SLAB IN AMBIENT AREAS: SPAL-PRO RS 88 BY METZGER-MCGUIRE (33°F OR HIGHER) WEAR SLAB IN ERFECTED AND COOLER: SPAL PRO RSE BY METZGER MCGLURE (32°F OR LOWER)	S1101 OVERALL SLAB PLAN S1111 PARTIAL SLAB PLAN - AREA 1 & 3 S1201 OVERALL 2ND FLOOR FRAMING PLAN (AREA 2)
ן היי	WEAR SLAB IN AMBIENT, FREEZER AND COOLER AREAS: EUCO QUICKJOINT 200 BY EUCLID CHEMICAL CO. 35. USE DIAMOND DOWELS AT CONSTRUCTION JOINTS FOR SLABS ON GRADE AND LOAD PLATE BASKETS	S1401 OVERALL ROOF FRAMING PLAN S1411 PARTIAL ROOF FRAMING PLAN - AREA 1 S2001 STRUCTURAL FRAMING ELEVATIONS
	OF PD3 BASKETS AT CONTRACTION JOINTS AS NOTED ON PLANS. 36. DRESS AND SEAL 30 AS MANUFACTURED BY L&M CONSTRUCTION CHEMICALS OR SUPER DIAMOND CLEAR VOX AS MANUFACTURE BY EUCLID CHEMICAL COMPANY SHALL BE USED WITH NEW CONCRETE IN ALL	S2002STRUCTURAL TILT-UP ELEVATIONS & DETAILSS4001VESTIBULE PLANS AND STRUCTURAL ELEVATIONSS4211EMPTY PALLET STORAGE FRAMING PLANS
A	AREAS NOT RECEIVING A FLOOR TOPPING. 37. L&M CURE AS MANUFACTURED BY L&M CONSTRUCTION CHEMICALS OR KUREZ W VOX AS MANUFACTURED BY ELICURE OF MICH. COMPANY SUBJECT OF MICH.	S4212 MAINTENANCE CATWALK FRAMING PLAN - AREA 1 S4601 STAIR PLANS AND SECTIONS - AREA 1 S5001 TYPICAL CONCRETE DETAILS
	BRICK, EPOXY, ETC. 38. VAPOR RETARDER SHALL BE STEGO WRAP 10 MIL CLASS A OR APPROVED EQUAL. RETARDER SHALL BE	S5003 STRUCTURAL TYPICAL STEEL DETAILS S5005 BASE PLATE, ANCHOR BOLTS AND PIER DETAILS S5011 STRUCTURAL FOUNDATION DETAILS
	INSTALLED, LAPPED 6" MIN. AND TAPED PER MFG. RECOMMENDATIONS. 39. ALL ANCHOR RODS AND EMBEDDED ITEMS SHALL BE ACCURATELY LOCATED AND SECURELY SUPPORTED PRIM TO CONCRETE POUR. WET SETTING OF ANCHOR RODS AND EMBEDDED ITEMS IS PROBUBILED.	S5012 STRUCTURAL FOUNDATION DETAILS S5031 STRUCTURAL BRACING DETAILS S5041 STRUCTURAL ROOF FRAMING DETAILS
		S6001ROOF DECK ATTACHMENT PLANS6002ROOF JOIST UPLIFT DIAGRAM

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INSTITUTE AND EE-SPAN CONTINUOUS BY THE STRUCTURAL

T BASED, NON-SHRINK, NCE WITH ASTM C827-82, D SHALL HAVE A MINIMUM

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ELD x 2" LONG EACH SIDE OF RT w/ A MIN. OF 1/4" FILLET D TO THEIR SUPPORT w/ A

IG AND ANCHORED TO THE CATIONS OF THE STEEL

ANS. SEE SPECIAL LOAD DADING.

B. BOTTOM CHORD BRACES ER MANUFACTURER.

DESIGNATED ON JOIST SHOP

E NUT EA. END OF SAG RODS.

ALL BE GIVEN ONE SHOP NOT LIMITED TO,

OR PROVIDE ADDITIONAL P. DETAIL SHEET).

A.) FROM CENTER OF LAMP OR TRAPEZE ECCENTRIC BEAM E. SIDE MOUNTED EMBER IS A JOIST OR

BUTARY AREA OF 20 SQ. LEVEL 2 DISCIPLINE DESIGNATOR G STRESS DUE TO SHEET TYPE DESIGNATOR SHEET SUB-TYPE DESIGNATOR MP SHALL BE 90 °F. 2-DIGIT SEQUENCE NUMBER S##### SECTOR DESIGNATOR AREA DESIGNATOR * APPLICABLE TO PLANS ONLY **DISCIPLINE DESIGNATOR** S STRUCTURAL DRAWINGS SHEET TYPE DESIGNATOR 0 GENERAL - SYMBOLS, LEGENDS, NOTES, ETC. 1 PLANS 2 ELEVATIONS & PROFILES 3 SECTIONS 4 LARGE SCALE VIEWS • 5 DETAILS 6 SCHEDULES AND DIAGRAMS 7 USER DEFINED 8 USER DEFINED 9 3D REPRESENTATION SHEET SUB-TYPE DESIGNATOR 0 FOUNDATION 1 FIRST FLOOR / SLAB 2 ELEVATED FLOORS STRUCTURE & FRAMING, MEZZANINE 3 INTERSTITIAL SPACE 4 ROOF STRUCTURE / FRAMING 5 ROOF RAISED PLATFORM 6 FIRST FLOOR REFLECTED CEILING 7 ELEVATED FLOOR STRUCTURE & FRAMING, MEZZANINE REFLECTED CEILING

8 INTERSTITIAL SPACE REFLECTED CEILING (OPTIONAL)

STRUCTURAL SHEET IDENTIFIER

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DESIGN NOTES			SPE	CIAL INSPI	ECTION SCHEDULE		
				E	STABLISHED PER 201	8 IBC	SECTION 110 AND CHAPTER 17
2020 FLORIDA BUILDING CODE 7th ed.		ITEM	CONTINUOUS	PERIODIC	REFERENCE		COMMENTS
WIND DESIGN		PRE-FAB CONSTRUCTION			REFERENCE NOTES P1 &	P1.	SPECIAL INSPECTION IS NOT REQUIRED WHERE TH
RISK CATEGORY II	118 MPH				P2		DONE ON THE PREMISES OF A FABRICATOR REGIS APPROVED TO PERFORM WORK WITHOUT SPECIA
EXPOSURE ENCLOSURE CLASSIFICATION	C ENCLOSED BUILDING						PROVIDED THE FABRICATOR COMPLIES WITH IBC.
INTERNAL PRESSURE COEFFICIENT	±0.18					P2.	INSPECTION FOR PREFABRICATED CONSTRUCTION SAME AS IF THE MATERIAL USED IN THE CONSTRU
COMPONENTS & CLADDING	@ h = 58' (NOMINAL: 0.6 ULTIMATE WIND)						PLACE ON SITE. SPECIAL INSPECTION WILL NOT BE DURING PREFABRICATION IF THE APPROVED AGEN
AREA ROOF (GROSS, PSF)	AREA WALLS (GROSS, PSF)						THE CONSTRUCTION AND FURNISHES EVIDENCE C COMPLIANCE.
SQ.FT. ZONE 1 ZONE 2 ZONE 3 ZONE 3 10 -39.1 -51.2 -69.8 +9.9	S 1, 2, 3 & 1 SQ.F1. ZONE 4 ZONE 5 ZONES 4 & 5 9/+10.0 MIN 10 -24.6 -30.3 +22.7	STEEL CONSTRUCTION (IBC 1705.2)				S1.	WELDING INSPECTIONS SHALL BE IN COMPLIANCE
20 -36.6 -47.9 -63.2 +9.3 50 -33.3 -43.6 -54.5 +8.3	3/+10.0 MIN 20 -23.6 -28.3 +21.7 5/+10.0 MIN 50 -22.3 -25.6 +20.4	MATERIAL VERIFICATION OF STRUCTURAL STEEL		X	AISC 360	S2.	ALL WELDS SHALL BE VISUALLY INSPECTED. ALL M
100 -30.8 -40.3 -47.9 +7.5	9/+10.0 MIN 100 -21.3 -23.6 +19.4 MENSIONS NOTE: EDGE DISTANCE a = 25' LINO	IDENTIFICATION MARKINGS TO CONFORM WITH ASTM STANDARDS SPECIFIED IN THE DESIGN DRAWINGS					WELDING PROCEDURES AND QUALIFICATIONS OF SHALL BE VERIFIED PRIOR TO THE START OF WOR
		CERTIFIED MILL TESTS	EACH SU	JBMITTAL		S3.	ALL COMPLETE PENETRATION WELDS SHALL BE TI
COMPONENTS & CLADDING @ h	h = 25'-8 1/2" (NOMINAL: 0.6 ULTIMATE WIND)	SHOP AND FIELD WELDING			REFERENCE NOTE S1	_	ULTRASONICALLY OR BY USING ANOTHER APPROV (AISC 360).
SQ.FT. ZONE 1 ZONE 2 ZONE 3 ZONE	AREA WALLS (GHOSS, 1937) ES 1, 2, 3 & 1' SQ.FT. ZONE 4 ZONE 5 ZONES 4 & 5	SINGLE PASS FIELD WELD $\leq 5/16"$		X	REFERENCE NOTE S2	S4	PERIODIC SPECIAL INSPECTION IS ALLOWED FOR Y
10 -33.7 -44.1 -60.1 +8.5 20 -31.5 -41.3 -54.5 +8.0	5/+10.0 MIN 10 -20.8 -25.6 +19.2 0/+10.0 MIN 20 -20.0 -23.9 +18.4	FILLE I WELDS > 5/16"	X		REFERENCE NOTE S2	-	OF THE WELDABILITY IF REINFORCING STEEL OTHI A 706 IN ACCORDANCE WITH ANSI/AWS D1 4 CON
50 -28.7 -37.5 -47.0 +7.0 100 -26.5 -34.7 -41.3 +6.8	3/+10.0 MIN 50 -18.8 -21.7 +17.2 8/+10.0 MIN 100 -18.0 -20.0 +16.4	PARTIAL/COMPLETE PENETRATION WELDS	X		REFERENCE NOTE S3	-	SPECIAL INSPECTION IS REQUIRED FOR REINFORC
NOTE: REF. ROOF UPLIFT PLAN AND ZONE DI	MENSIONS. <u>NOTE:</u> EDGE DISTANCE, a = 6' UNO	LIGHT GAUGE METAL FRAMING WELDING		X	REFERENCE NOTE S2		AND SPECIAL MOMENT FRAMES, BOUNDARY ELEM
EARTHQUAKE DESIGN		FLOOR AND ROOF DECK WELDING		X	REFERENCE NOTE S2]	REINFORCEMENT. PERIODIC SPECIAL INSPECTION
OCCUPANCY CATEGORY SITE CLASS	II D (ASSUMED)	FLOOR AND ROOF DECK MECH. FASTENERS		X		-	NOT INCLUDED IN THE CONTINUOUS SPECIAL INSP
IMPORTANCE FACTOR SPECIAL RESPONSE ACCELERATION	1.0 Ss = 0.087	SIDE SEAM SCREWS, BUTTON PUNCHES ETC.		X		-	REQUIREMENTS NOTED ABOVE.
	S1 = 0.051 SDs = 0.093	EMBEDDED PLATES		X		_ S5.	SLIP-CRITICAL CONNECTIONS MAY HAVE PERIODIC INSPECTION PROVIDED THAT THE TURN-OF-THE-N
	SD1 = 0.081 B	HIGH STRENGTH BOLTING			AISC 360	-	WITH MATCH MARKING TECHNIQUES IS USED.
RESPONSE MODIFICATION FACTOR (R)	3.00: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC	VERIFY MATERIALS OF NUTS, BOLTS & WASHERS		X			
SOIL BEARING CAPACITY	qall. = 2,000 PSF	BEARING TYPE CONNECTIONS		X			
MODULUS OF SUBGRADE REACTION, K	100 PCI UNIVERSAL ENGINEERING SERVICES, LLC	SLIP-CRITICAL CONNECTIONS	X	X	REFERENCE NOTE S5	-	
	4475 SW 35TH TERRACE, GAINESVILLE, FL 32608 No. 1993279 (12/19/2022)	A. END CONNECTIONS - WELDED OR BOLTED		x	SJI SPECIFICATIONS SECTION 2207.1		
MIN. FOUNDATION BRG DEPTH	18" BELOW FINISHED GRADE	B. BRIDGING - HORIZONTAL AND DIAGONAL 1. STANDARD BRIDGING		x			
		2. BRIDGING THAT DIFFERS FROM SJI SPECIFICATIONS LISTED IN SECTION 2207.1		X			Digitally signed by
	20 PSF (WITH TRIBUTARY REDUCTIONS PER CODE)	VERIFY FIELD INSTALLED WEB MEMBERS LOCATED AT POINTS		X		-	Joseph A. Bove, P DN:
	4.3 IVIN 40 PSF	OF CONCENTRATED LOAD AS REQUIRED ON CONTRACT DRAWINGS.					E=jbove@stellar.n CN="Joseph A. Bo
STAIRS AND LANDINGS	100 PSF 100 PSF	VERIFY DECK, DECK ASSESSORIES AND MECHANICAL		X	SDI QA/QC, TABLE 1	-	Joseph A. Bove, P.E. Licensing, O-Stell
	LOCATED TO CAUSE MAXIMUM STRESS	PROFILES, MATERAIL PROPERTIES ADN BASE METAL THICKNESS					Group Incorporate
INTERIOR WALLS	5 PSF	VERIFY DECK, DECK ASSESSORIES AND CONNECTIONS		X	SDI QA/QC, TABLE 1		S=FL, C=US
GROUND SNOW LOAD, Pg FLAT ROOF SNOW LOAD, Pf	0 PSF 0 PSF	DOCUMENTS.				_	Date: 2023.05.28 22:24:25-04'00'
(WITH RAIN ON SNOW)		VERIFY TYPE, SIZE AND LOCATION OF SUPPORT AND SIDELAP CONNECTIONS.		X	SDI QA/QC, TABLE 1		
ROOF DEAD LOADS (PSF)					ACI 318		
WAREHOUSE	WELFARE					-	
(MAX.) (MIN.) (M	1AX.) (MIN.)	INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.		X			
ROOF MEMBRANE	0.5 0.5	INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR	X				
ROOF INSULATION 1.5 1.0 1.0 METAL ROOF DECK 2.0 <td>1.5 1.0 2.0 2.0</td> <td>TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED.</td> <td></td> <td></td> <td></td> <td></td> <td></td>	1.5 1.0 2.0 2.0	TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED.					
ROOF STEEL 5.0 3.0 5.0 COLLATERAL 8.0 2.5 6	5.0 <u>3.0</u> 6.0 <u>1.5</u>	VERIFYING USE OF REQUIRED DESIGN MIX.		x			
(REF. NOTES) SOLAR PANELS 8.0	2.0 1.0	AT THE TIME FRESH CONCRETE IS SAMPLED TO	X				
TOTAL 25.0 9.0 1	7.0 9.0	PERFORM SLUMP AND AIR CONTENT TESTS,					
1. MAXIMUM DL USED FOR GRAVITY LOAD DES	IGN AND MINIMUM DL USED FOR SEISMIC MASS AND WIND UPLIFT DESIGN.					-	
2. COLLATERAL LOAD INCLUDES LIGHT INCOME 3. EQUIPMENT GREATER THAN 150LBS ACCOUNTS 4. COLLATERAL LOAD OF ADOC ADOC MORE ACCOUNTS	NTED FOR INDEPENDENT OF COLLATERAL LOAD.	TEMPERATURE AND TECHNIQUES.		X			
4. COLLATERAL LOAD OF 8PSF ACCOUNTS FOR	R PIPE RUINS LESS I HAIN ZUPLE.	INSPECTION OF CONCRETE PLACEMENT FOR PROPER	x				
				v		-	
ELEVATED SLAB DEAD LOADS (PSF)				A V		-	
OFFICE		LOCATION AND DIMENSIONS.		×			
CONCRETE + DECK37.0FLOOR BEAM STEEL5.0		INSPECTION OF ANCHORS INSTALLED IN HARDENED		X	ACI 318, IBC 1912.1		
COLLATERAL 6.0 CEILING 2.0		INSPECTION OF PRESTRESSED CONCRETE:				-	
TOTAL 50.0		A. APPLICATION OF PRESTRESSING FORCES B. GROUTING OF BONDED PRESTRESSING	X				
MATERIALS		TENDONS IN THE SEISMIC-FORCE BESISTING SYSTEM (AS APPLICABLE)					
CHANNELS, ANGLES & PLATES	A36 (36 KSI) A500 CPADE D (46 KSI) OD CDADE C (50 KSI)	ERECTION OF PRECAST CONCRETE MEMBERS		x			
STANDARD PIPE	ASSO GRADE B (40 KSI) ON GRADE C (50 KSI)	VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO		x		-	GENERAL SPECIAL INSPECTION NO
C.I.P. ANCHOR RODS	HEAVY HEX HEAD F1554 GRADE 36 and GRADE 55 (REF. S5005)	STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS				1. TH	IE ITEMS MARKED WITH A "X" IN THE SPECIAL INSPE
POST-INSTALLED ANCHOR RODS	ASTM A193 GRADE B/ W/ COATING AS SPECIFIED IN ESR-2322 OR ESR-3187 OR ESR-3814. <u>POST-INSTALLED ANCHORS</u>	AND STRUCTURAL SLABS				17	OF THE INTERNATIONAL BUILDING CODE BY A CER
SAG RODS	REQUIRE ENGINEER'S APPROVAL PRIOR TO USE A36 BAR WITH THREADED ENDS	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF CONCRETE MEMBER(S) BEING FORMED.		X		M/	ATERIAL SAMPLING AND TESTING REQUIREMENTS,
STEEL JOISTS & JOIST GIRDERS METAL ROOF DECK	S.J.I. 1 1/2", TYPE B, Fy=40 ksi (MIN.), GALV. G60 (UNDERSIDE SHOP PAINTED WHITE)	SOILS (IBC 1705.6)				SF	ATERIAL SAMPLING AND TESTING SECTION, THE PR PECIFICATIONS, AND/OR THE SPECIFIC GENERAL N
METAL FLOOR DECK	(22 GA DECK UNLESS NOTED OTHERWISE, REF. S6001) 1.0C, 22GA., Fy=60 ksi			v		TH TE	IE TESTING AGENCY SHALL SEND COPIES OF ALL S ESTING AND INSPECTION REPORTS DIRECTLY TO TH
WELDING ELECTRODES RIGID UNDER FLOOR INSULATION	E70XX MIN. COMPRESSIVE STRENGTH = 25 PSI	ACHIEVE THE DESIGN BEARING CAPACITY.		^		EN WI	NGINEER ,CONTRACTOR AND BUILDING OFFICIAL. AI HICH FAIL TO COMPLY WITH THE APPROVED CONS ⁻
RIGID INSULATION BLOCK (LAST-A-FOAM R9330)	MIN. COMPRESSIVE STRENGTH = 1000 PSI	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		x		DC OF	DCUMENTS SHALL IMMEDIATELY BE BROUGHT TO T THE CONTRACTOR FOR CORRECTION. IF DISCREF
STÈEL GRATING	1 1/4"x3/16" GW (19W4) GRATING, GALV. STEEL, NAAMM 1 1/4"X3/16" CMW-4-125 (11W4) GRATING, GALV STEEL AS INDICATED ON ABOH DWG	PERFORM CLASSIFICATION AND TESTING OF				NC TF	OT CORRECTED, THEY SHALL BE BROUGHT TO THE IE BUILDING OFFICIAL, ARCHITECT AND ENGINEER I
RAISED PATTERN FLOOR PLATE	A786	CONTROLLED FILL MATERIALS.		X			DMPLETION OF THAT PHASE OF CONSTRUCTION.
CONCRETE (28 DAYS) FOOTINGS	3000 PSI	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF	x			SH 3 CC	
PIERS WEAR SLAB/SLAB-ON-GRADE	3000 PSI 4000 PSI	CONTROLLED FILL.					BSERVATION OF WORK REQUIRING SPECIAL INSPECTOR
ELEVATED SLABS SUB-SLAB	3000 PSI 3000 PSI	PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIEV THAT SITE HAS REEN DEEDADED		v		AP WI	HERE THE WORK IS BEING PERFORMED. PERIODIC
TILT-UP ALL OTHER CONCRETE	4000 PSI (3000 PSI @ 7 DAYS) 3000 PSI	PROPERLY.		^ 			SECTION MEANS THE PART-TIME OR INTERMITTEN SSERVATION OF WORK REQUIRING SPECIAL INSPEC
REINFORCING STEEL HEADED STUDS	A615 GRADE 60 A108	VERIFY SITE PREPARATION COMPLIES WITH APPROVED SOILS REPORT.	x			AP WI	HERE THE WORK HAS BEEN OR IS BEING PERFORM
WELDED WIRE FABRIC	A185 SIZE AS NOTED ON DRAWINGS MEETING AWSD1 1	VERIFY DRY-DENSITY OF COMPACTED FILL COMPLIES		v			DIVIPLE TION OF THE WORK (IBC SECTION 1704). SPE SPECTION TESTING REQUIREMENTS APPLY EQUALI
DEFORMED BAR ANCHORS (DBA) STRAND	A1064 UNCOATED, ASTM-416 GRADE 270	WITH APPROVED SOILS REPORT.		Å		BIC	UDER DESIGNED COMPONENTS.

EARTHQUAKE DESIGN

	WARE	HOUSE	WELFARE		
	(MAX.)	(MIN.)	(MAX.)	(MIN.)	
SINGLE PLY	0.5	0.5	0.5	0.5	
ROOF MEMBRANE					
ROOF INSULATION	1.5	1.0	1.5	1.0	
METAL ROOF DECK	2.0	2.0	2.0	2.0	
ROOF STEEL	5.0	3.0	5.0	3.0	
COLLATERAL	8.0	2.5	6.0	1.5	
(REF. NOTES)					
SOLAR PANELS	8.0	-	2.0	1.0	
TOTAL	25.0	9.0	17.0	9.0	
NOTEC					

HILTI HIT-RE 500 V3 ADHESIVE ANCHOR SYSTEM (ICC ESR-3814)

ADHESIVE ANCHORING ERECTION ALL STRUCTURAL COMPONENTS SHALL BE ERECTED WITHIN THE TOLERANCES SET FORTH BY AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

DEFERRED SUBMITTAL DOCUMENTS FABRICATION DRAWINGS AND CONNECTION DESIGN CALCULATIONS (SIGNED AND SEALED BY A PROFESSIONAL ENGINEER IN

RECORD FOR REVIEW FOLLOWED BY THE PERMITTING AGENCY PRIOR TO FABRICATION FOR THE FOLLOWING ITEMS. 1. STRUCTURAL STEEL (SIGNED AND SEALED CONNECTION CALCULATIONS) 2. STEEL JOISTS AND JOIST GIRDERS (SIGNED AND SEALED CALCULATIONS)

3

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HILTI HIT-RE 500-SD ADHESIVE ANCHOR SYSTEM (ICC ESR-2322) HILTI HIT-HY 200 ADHESIVE ANCHOR SYSTEM (ICC ESR-3187)

THE STATE THE PROJECT IS BEING CONSTRUCTED), AS NOTED BELOW, SHALL BE SUBMITTED FIRST TO THE ENGINEER OF

4





1

2

CONTINUOUS FOUNDATION SCHEDULE							
Mark		SIZE					
IVIAIK	Width	Depth	Bot (Long)	Bot (Short)	Top (Long)	Top (Short)	Remarks
CF2.0	2'-0"	1'-0"	3~#5 CONT.	#4x1'-6" @ 36" O.C.			
CF3.0	3'-0"	1'-0"	3~#5 CONT.	#4x2'-6" @ 24" O.C.			
CF3.5R	3'-6"	1'-0"	2~#5 CONT.		4~#5 CONT.	#4x3'-0" @ 18" O.C.	

	SIZE			REINFORCEMENT					
Mark	Length	Width	Depth	Bot (Long)	Bot (Short)	Top (Long)	Top (Short)	Type Comments	
F5.0	5'-0"	5'-0"	1'-0"	6~#5	6~#5				
F7.0A	7'-0"	7'-0"	1'-2"	7~#6	7~#6	7~#6	7~#6		
F8.0	8'-0"	8'-0"	1'-6"	8~#6	8~#6				
F9.0A	9'-0"	9'-0"	1'-10"	10~#6	10~#6	10~#6	10~#6		
RF5.0x3.0	5'-0"	3'-0"	2'-0"	5~#5	10~#5	5~#5	10~#5		
RF7.0x4.0	7'-0"	4'-0"	2'-0"	5~#5	7~#5	5~#5	7~#5		
RF7.0x4.0A	7'-0"	4'-0"	2'-6"	5~#5	7~#5	5~#5	7~#5		
RF9.0x6.5 XB	9'-0"	6'-6"	2'-0"	5~#5	12~#5	5~#5	12~#5		
RF9.0x6.5A XB	9'-0"	6'-6"	2'-10"	5~#5	12~#5	5~#5	12~#5		
RF9.5x6.0 XB	9'-6"	6'-1"	3'-0"	5~#5	12~#5	5~#5	12~#5		
RF10.0x6.0 XB	10'-0"	6'-0"	2'-6"	5~#5	12~#5	5~#5	12~#5		
RF11.5x9.0 XB	11'-6"	9'-0"	2'-10"	5~#5	12~#5	5~#5	12~#5		



SHEET ID

S1000

SHEET ID

S1101

END ION LAB	PROJEC	(R) (R) T INFORMATION	
	UNIT	ED STATES COLD S PHASE III EXPANSI LAKE CITY, FLORID	TORAGE ON
	Ĭ	A stel	lar
	2900 HART FLOF Fl SEAL	OPERATING AS: THE STELLAR GROUP ILEY ROAD, JACKSONVILLE, FL. 32: WWW.STELLAR.NET RIDA ARCHITECTURAL LICENSE NO LORIDA ENGINEERING LICENSE NO	257 (904) 260-2900). AR0013370). CA5930
by a, P.E. ar.net, Bove, porate tellar ated, S=FL, 28	CONSUL	NO. PE89830 STATE OF STATE OF	
	The S All rig repro comm prior v	Stellar Group, Inc. hts reserved. No part of this work may duced, distributed, displayed, or other nunicated in any form or by any means written consent of The Stellar Group, In	/ be used, wise s without the nc.
	NO.	DESCRIPTION	DATE
	B	DESIGN DEVELOPMENT 2 DESIGN DEVELOPMENT 1	05/26/2023
		SUED:	03/06/2023 SBS
		ED BY:	JFM .IAB
	PROJEC	T NUMBER:	07366
	Ри	ARTIAL SLAB P AREA 1 & 3	LAN -

OWNER

S1111

Group Incorporate L=Jacksonville, S= C=US Date: 2023.05.28 22:13:31-04/00

S1401

2					3			
(18.5)(R18.5)	(18.9) (R19) (R	19.5 (19.5) (19	.8) (R20)	(20.3) (R2	20.5 (20.9) (R21) (21.3) (F	21.5 (21.9) (R2
48' - 8"	- 11"	<u>48' - 8'</u> 34' - 7"	, 14' - 1"	29' - 3 1/2"	58' - 7"	29' - 3 1/2"	<u>48' - 8"</u> 24' - 4"	24' - 4"
(E))W14X26	(E)W14X26	(E)W14X26		(E)W14X26			(E)W14¥26
	A2							
24X76	4211	24X76	24X76	24X76	211	CU1001		9LX
(E)W		B5	M(g) 14' - 1"	= = (E)W(- [72] 5,000LB	(E)W24)
24K9 SP 24K9 [22.0K] - [28.5	SP K]	S5041 [29.0K]	24K9 S S504 [15.5K]	1) 24K9 SF [23.5K]		24K9 SP - [31.0K]	24K9 SP [26.0K]	24K9 SP [19.5K] B1 S4212 [13.0K]
X-BRACE		8.5K 30KCS5 30KCS5	(AU1203)		5X-BRACE <u></u>		0.0K 	
2840		4.0K 30KCS5		4.0K 30KCS	5 X4		3.5K 30KCS5	2840 LBS2.5K
<u>≥</u> 30K10	<u>≥I</u> • • • -	0.5K 30K10			30KCS5	<u>></u> I	0.5K 30K10	·
30K10		<u>1.5K</u> 30K10	U2 S4212 do	_1.5K	30K12		.5K30K10	<u></u>
30K10 ↔ 30K10	48G9N10	1.0K 30K10 30K10	48G9N1	1.5K	30K12 ↔	48G9N1	.5K <u>30K10</u> ↔ 30K10	48G9N1
30K10		30K10			30K12		30K10	
30K10		30K10	Ę]	30K12	Q	30K10	Q
30K10 30K10		30K10 30K10			30K12		30K10	
30K10		30K10			30K12		30K10	
30K10		30K10	F	<u></u> <u>S5041</u>	30K12		30K10	
30K10	9N10.0K	30K10	¶N11.0Қ		30K12	9N11.0K	30K10	\$N10.0K
30K10	- 48G	30K10	48G		30K12	48G	30K10	48G
30K10		30K10 30K10			30K12		30K10	
30K10		30K10	ć		30K12		30K10	ů
30K10		30K10			30K12	550	44 141 30K10	
30K10		30K10			30K12		30K10	
30K10		30K10			30K12		30K10	
30K10	G9N10.0	30K10	(G\$N11.0		30K12	(G\$N11.0	30K10	(G\$N10.0)
30K10		30K10			30K12		30K10	
30K10		30K10			30K12		30K10	
30K10		30K10			30K12		30K10	Ï
30K10		30K10			30K12		30K10	
30K10		30K10			30K12		30K10	
30K10		30K10			30K12		30K10	
30K10	48G\$N1C	30K10 30K10	48G\$N11		30K12	48G\$N11	30K10	48G\$N1C
30K10		30K10			30K12		30K10	
30K10		30K10	E	J	30K12		30K10	G
30K10		30K10			30K12		30K10	
30K10		30K10			30K12		30K10	
30K10		30K10			30K12		30K10	
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30K10		30K10			30K12		30K10	0
30K10		30K10		1.0K	30K12		1.5K 30K10	
30K10 30K10	с	30K10 30K10	P S		30KCS5	طى 	1.5K <u> </u>	₩ ₩ ₩ ₩ ₩ ₩ ₩
30KCS5	39N11.5K		29 11.0K		30KCS5	0" "0" "0" "	1.0K 30KC\$5	1.5K
30KCS5	480 WT7	2.0K 30KC	CS5 $\overset{\triangleleft}{\triangleleft}$	5.5K AU1210 8.5K 2840 LBS	X-BRACE 30	<u> </u>	4.0K 30KCS5	3.5K
X-BRACE 30KCS5 -	22K4 SP [28.5K]	22K4 30K0 [8.0K]	CS5		22K4_SP =30 [23.5K]	KC\$5 =22K4 SP	22K4 SP 30KC S5 [8.0K]	22K4 = 22K4 =X-BRACE 2 [19.0K]
					A3 \$5041	24G8.0K \$P		A2
				L3X3X	1/4 CONT. ANGLE	A2 1 1 1 1	TYP. UN	50 PLF SEF BEFBIG
						LF/99 PLF	LF/99 PLF	(EA. JOIST)
					24 X62 [15	L = 241 P L = 241 P L = 241 P L = 241 P	SP S	A SP
					(E)MC	SP TL/L SP TL/L SP TL/L SP TL/L	30) 30) 30) 30)	301 00 100 301 301 301 301 301 301 301 3
						30 30 30 30 30 30 30 30 30 30 30 30 30 3		F FRAME
			Y PALLET STORAGE		1.0K, TYP. EA. JOIST -			A3/S5003
							- 0.4K	
Ø		2	4 M	⊠ 	5' - 8 1/	2"5	- 8 1/2"	
						5' - 8"	5' - 4 1/2" 37' - 11" 7 SPA @ 5' - 5"	5' - 4 1/2" 5' - 6"
							48' - 8"	
(18.5)	(18.9)	19 (19.	4) (19	.8) (20)	(20.3) 20.5)	(20.6) (20.9) (21.3)(2	(21.9)

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ROOF FRAMING PLAN SHEET NOTES

OWNER

DN: E=jbove@stellar.net, CN= Joseph A. Bove, P.E.*, OU=Corporate Licensing, O=Stellar Group Incorporated, L=Jacksonville, S=FL C=US Date: 2023.05.28 22.14.47.04.00'

ABolt Grade	Remarks	Count
F1554 GR55		28
F1554 GR55		9
F1554 GR55		2
F1554 GR55	XBRACE	8
F1554 GR55	XBRACE	9
F1554 GR55	XBRACE	17
F1554 GR55	VESTIBULE	4
F1554 GR55		7
		8

2

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SHEET ID

(GRID)

BASE PLATE, ANCHOR BOLTS AND PIER DETAILS

NO.	DESCRIPTION	DATE
С	PERMIT ISSUE	05/26/2023
В	DESIGN DEVELOPMENT 2	04/03/2023
А	DESIGN DEVELOPMENT 1	03/06/2023
		_
		00/00/0000
		03/06/202
DESIGNE	ED BY:	SRS
REVIEW	ED BY:	JFN
A/EOR:		JAE
PROJEC	T NUMBER:	0736

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UNITED STATES COLD STORAGE PHASE III EXPANSION LAKE CITY, FLORIDA

PROJECT INFORMATION

D		
С		
В		

D		
С		
В		

	D4 BRACE DE
	C4 BRACE DE
	SCALE: NOT TO SCAL
	_
	\$/
	N.S. & F.S LOAF
	(B4) ANGLE BR
	JOUALE: NUT TO SCAL
	(A4) ANGLE BR

			Digitally signed				
			Jose	ph A.	Bove	e, P.E.	
				DN:			
				E=jbo	ove@	stella	ar.net,
				CN='	Jose	ph A.	1
				Bove	, P.E	.,	
Joseph	א A. E	Bove,	P.E.	OU=	Corpo	orate	
				Licer	nsing,	O=S	tellar
				Grou	p Inc	orpor	ated,
				L=Ja	cksor	nville,	
				S=FL	C=	JS	
				Date	202	3.05.2	28
				22.20)·04-(14'00'	
				(J.UT (5 7 00	

ROOF DECK ATTACHMENT SCHEDULE						
	ZONE	DECK	SUPPORT FASTENERS	NO. OF WELDS @ SUPPORTS	SIDELAP FASTENERS	NO. OF SI PER S
	А	TYPE B, 22 GA.	HILTI PINS	4	S-SLC HWH	2
	В	TYPE B, 22 GA	HILTI PINS	7	S-SLC HWH	2

DECK ATTACHMENT NOTES: 1. ATTACH DECK TO PERIMETER SUPPORTS WITH SAME FASTENER PATTERN AND TYPE SHOWN IN THE SCHEDULE FOR SUPPORTS. 2. END LAPS SHALL BE A MINIMUM OF 3" AND SHALL OCCUR OVER SUPPORTS.
 3. SIDE LAPS BETWEEN STRUCTURAL SUPPORTS SHALL BE FASTENED WITH CONNECTION NOTED ON SCHEDULE AS MANUFACTURED BY HILTI. 4. SUPPORTS SHALL BE MADE WITH THE APPLICABLE OF HILTI PINS X-HSN 24 OR X-ENP-19.

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SIDELAPS SPAN _____

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S6002

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