GENERAL NOTES

DESIGN CRITERIA

D1 ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARD OF THE FOLLOWING CODES:
2004 FLORIDA BUILDING CODE, BUILDING
2004 FLORIDA BUILDING CODE, FUEL GAS
2004 FLORIDA BUILDING CODE, MECHANICAL
2004 FLORIDA BUILDING CODE, PLUMBING
2004 FLORIDA FIRE PREVENTION CODE
2002 NATIONAL ELECTRIC CODE.

D2 DESIGN LOAD VALUES:

ROOF LIVE LOADS	20 PSF
ROOF DEAD LOADS	10 PSF
ASSUMED ALLOWABLE SOIL BEARING CAPACITY	2000 PSF

D3 THE STRUCTURAL PLANS AND WIND SPEED HAVE BEEN DESIGNED IN ACCORDANCE WITH SECTION 1609 OF THE FLORIDA BUILDING CODE 2004, EDITION.

GENERAL

G1 THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.

G2 THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONS, AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION & FOR ALL JOBSITE SAFETY.

G3 NO STRUCTURAL MEMBER SHALL BE CUT NOTCHED OR OTHERWISE REDUCED IN STRENGTH.

G4 THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER ON ANY DISCREPANCIES

G5 ANY SUBMITTALS RECEIVED BY A/E THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUBCONTRACTOR SHALL BE RETURNED WITHOUT REVIEW.

G6 ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.

G7 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING ALL EXISTING CONDITIONS, ELEVATIONS, AND DIMENSIONS PRIOR TO COMMENCING CONSTRUCTION, INCLUDING FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE A/E FOR RESOLUTION.

FOUNDATIONS

F1 A GEOTECHNICAL REPORT FOR THIS PROJECT WAS NOT PROVIDED BY THE OWNER. THESE PLANS ARE BASED UPON AN ASSUMED ALLOWABLE BEARING CAPACITY OF 2000 PSF. THE CONTRACTOR SHALL ENGAGE A QUALIFIED AND CERTIFIED GEOTECHNICAL ENGINEER TO DETERMINE THE ALLOWABLE SOIL BEARING CAPACITY. A COPY OF THE REPORT SHALL BE PROVIDED TO THE A/E. IF THE DETERMINED ALLOWABLE BEARING CAPACITY IS LESS THAN THE ASSUMED VALUE, MODIFICATIONS TO THE FOUNDATIONS MAY BE REQUIRED.

F2 THE GEOTECHNICAL ENGINEER SHALL MAKE A FIELD INVESTIGATION TO DETERMINE IF ANY SOIL CONDITIONS ARE PRESENT THAT MAY ADVERSELY AFFECT THE PROJECT. THE CONTRACTOR SHALL REMOVE ALL SUCH MATERIAL AND REPLACE IT WITH APPROVED FILL.

F3 SUBGRADE UNDER FOOTINGS AND SLABS SHALL BE COMPACTED TO AT LEAST 95% OF THE MODIFIED PROCTOR MAXMIMUM DRY DENSITY (ASTM D1557). SUBMIT DENSITY TESTS TO THE A/E.

SLABS ON GRADE

S1 PROVIDE 6 MIL POLYETHYLENE SHEETING UNDER ALL SLABS ON GRADE.

S2 PROVIDE CONTROL JOINTS (1/4" WIDE BY 3/8" DEEP) AS INDICATED ON PLAN. FILL JOINTS WITH POURED RUBBER. IF JOINTS ARE SAWCUT, SAWCUTTING MUST BE DONE THE SAME DAY THE CONCRETE IS PLACED.

S3 PROVIDE 1/2" EXPANSION JOINTS AT ALL LOCATIONS WHERE SLABS ABUT STRUCTURES (WALLS, COLUMNS, ETC.)

CONCRETE AND REINFORCING

C1 THE GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS. SUBMIT REPORTS TO A/E.

C2 CONCRETE WORK & MIX DESIGN SHALL CONFORM TO ACI 301 (LATEST EDITION) "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS". MIXING SHALL BE IN ACCORDANCE WITH ASTM C94

C3 MINIMUM 28-DAY COMPRESSIVE STRENGTH:

FOOTINGS AND TIE BEAMS	2,500 PSI
COLUMNS AND SLABS	2,500 PSI

C4 SPLICES OF REINFORCING (EXCEPT AS SHOWN ON PLANS)

#4 BARS	20 INCHES
#5 BARS	27 INCHES
WELDED WIRE MESH	6"

C5 AT ALL CORNERS OF TIE BEAMS AND WALL FOOTINGS, PROVIDE CORNER BARS (30 INCH MINIMUM LEGS) TO MATCH HORIZONTAL BARS.

C6 REINFORCING BARS SHALL CONFORM TO ASTM A615-96a GRADE 40. WELDED WIRE MESH SHALL CONFORM TO ASTM A-185. LAP WELDED WIRE MESH ON MESH + 2" WHERE SPLICED.

C7 MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHEWISE NOTED.

LOCATION	MIN. COVER
BOTTOM OF FOOTINGS	3"
SIDES OF FOOTINGS	3"
COLUMNS	1 1/2"
TIE BEAMS	1 1/2"
SLABS	AS NOTED

PRE-FABRICATED WOOD TRUSSES

WT1 WOOD TRUSSES SHALL BE DESIGNED, SIGNED & SEALED BY A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. TRUSSES SHALL BE FABRICATED IN CONFORMANCE WITH THE THE "QUALITY CONTROL MANUAL" BY TRUSS PLATE INSTITUTE (TPI).

WT2 HANDLING, ERECTION AND BRACING OF WOOD TRUSSES SHALL BE IN ACCORDANCE WITH "HANDLING AND ERECTING WOOD TRUSSES" (HET80) AND "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS" (BWT-76) BY THE TRUSS PLATE INSTITUTE (TPI).

WT3 PERMANENT BRACING SHALL BE INDICATED IN THE TRUSS LAYOUT DRAWINGS AND SHALL BE SUPPLIED AND INSTALLED BY THE FRAMING CONTRACTOR.

WT4 TRUSSES SHALL BE DESIGNED PER ASCE 7-98 FOR THE FOLLOWING LOADS:

DEAD LOAD	10 PSF
LIVE LOAD	20 PSF
WIND	110 MPH W/ 3 SECOND WIND GUST

WT5 PRE-FABRICATED WOOD TRUSSES SHALL BE FABRICATED FROM SOUTHERN PINE (SPIB) KILN DRIED #2 GRADE OR BETTER FOR CHORD AND #3 GRADE OR BETTER FOR WEBS.

WT6 TRUSS BEARING SHALL BE 4" NOMINAL UNLESS NOTED OTHERWISE. BEARING LOCATIONS MUST BE MARKED ON TRUSS BY FABRICATOR TO INSURE PROPER INSTALLATION.

WT7 SHOP DRAWINGS SHALL BE SUBMITTED WHICH INDICATE DESIGN LOADS, DURATION FACTOR TRUSS LAYOUT, TRUSS CONFIGURATION AND TRUSS TO TRUSS CONNECTION. SHOP DRAWINGS SHALL SHOW PIECE MARKS, MEMBER SIZE AND GRADE AND CONNECTION DETAILS.

WT8 NO WANE KNOTS, SKIPS OR OTHER DEFECTS SHALL OCCUR IN THE PLATE CONTACT AREA OR SCARFED AREA OF WEB MEMBERS. PLATES SHALL BE CENTERED WITH ONE REQUIRED EACH SIDE OR TRUSS.

WT9 DESIGN OF METAL CONNECTED WOOD ROOF TRUSSES TO COMPLY WITH STANDARD BLDG. CODE NFPA'S "NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADED LUBER AND ITS FASTRNINGS", AND TRUSS PLATE INSTITUTE'S "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES".

WT10 WOOD BLOCKING AT TRUSS BEARING SHALL BE LAP SPLICED 4'-0" MIN. AND NAILED WITH (20) 10d NAILS AT SPLICE, 10d NAILS @ 16" O.C, ELSEWHERE.

MASONRY

M1 MASONRY CONSTRUCTION SHALL CONFORM TO ACI STANDARD BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (ACI 530-88/ASCE 5-88) AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-88/ASCE 6-88)

M2 CONCRETE BLOCKS SHALL CONFORM TO ASTM C-90 (f'm = 1500 PSI) (1900 PSI ON THE NET AREA)

M3 MORTAR SHALL COMPLY WITH ASTM C270 TYPE M OR S (COMPRESSIVE STRENGTH = 2500 PSI AND 1800 PSI) RESPECTIVELY. SITE TESTED MORTAR CUBES SHALL ACHIEVE A MINIMUM OF 80% OF THE DESIGN COMPRESSIVE STRENGTH.

M4 BLOCK SHALL NOT BE MOISTENED BEFORE GROUTING.

M5 ALL MASONRY CROSS WEBS SHALL BE FULLY BEDDED IN MORTAR AROUND CELLS TO BE GROUTED.

M6 THE MINIMUM CONTINUOUS UNOBSTRUCTED CELL AREA IN CELL TO RECEIVE GROUT MUST BE NOT LESS THAN 2" x 3". MORTAR FINS MUST BE REMOVED AS BLOCK PLACEMENT PROCEEDS. MORTAR DROPPINGS MUST BE KEPT OUT OF CELLS WHICH ARE TO BE GROUTED.

M7 REINFORCE WALLS WITH LADDER TYPE (ASTM A-82, #9 GAGE WIRE) REINFORCEMENT EQUAL TO DURO-WALL IN BED JOINTS @ 16" O.C. MEASURED VERTICALLY. PLACE PER MFR. INSTR. LAP ALL HORIZONTAL JOINT REINFORCING 8" MIN.

M8 CORES OR BLOCK MASONRY SHALL BE FILLED WITH GROUT AT CORNERS, EACH SIDE OF OPENINGS, AND @ 48" O.C. WITH MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. GROUT SLUMP SHALL BE 8" TO 11" FOR MASONRY FILLED CELLS.

M9 GROUT FOR FILLED CELLS SHALL BE POURED OR PUMPED IN LIFTS NOT TO EXCEED EIGHT (8) FEET IN HEIGHT, AND SHALL BE CONSOLIDATED AT TIME OF POURING BY RODDING OR VIBRATING.

M10 PROVIDE KNOCK-OUT CMU AT BASE OF EACH FILLED CELL TO ALLOW VISUAL VERIFICATION OF COMPLETE GROUT PENETRATION (FOR LIFTS OF 5'-0" OR LESS. A KNOCK OUT AT BASE OF LIFT WILL NOT BE REQUIRED).

M11 VERTICAL REINFORCING MUST HAVE A MIMUM CLEARANCE OF 1/2" TO INSIDE FACE. VERTICAL REINFORCEMENT IN WALLS SHALL BE SECURED AND LATERALLY SUPPORTED AGAINST DISPLACEMENT AT INTERVALS NOT EXCEEDING 6 FT.

M12 GROUT PLACEMENT STOPPED FOR (1) HOUR OR MORE SHALL BE STOPPED (1 1/2") BELOW THE TOP OF THE MASONRY UNIT TO PROVIDE A KEY FOR SUBSEQUENT GROUTING.

M13 SEE FOUNDATION PLANS FOR ALL VERT. REINFORCING. TYP. VERTICAL REINFORCING SIZE & SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS.

M14 TEMPORARY BRACING AND SHORING OF WALLS TO PROVIDE STABILITY DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR.

M15 MASONRY CONSTRUCTION MATERIALS AND INSPECTIONS SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI-ASCE 530.1)" EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THESE DOCUMENTS.

M16 PROVIDE FILLED PRECAST U-LINTELS W/ (1) #5 CONT. AT ALL OPENINGS WHERE CONCRETE BEAMS ARE NOT SHOWN OR NOTED. MINIMUM UNFILLED LINTEL CAPACITY = 400 lb/LF FOR SPAN INIDICATED. SEE PLANS FOR LINTEL REQUIREMENTS AT DOOR OPENINGS.

M17 STOPPING AND RESUMING WORK: RACK BACK 1/2- UNIT LENGTH IN EACH COURSE. DO NOT TOOTH. CLEAN EXPOSED SURFACES OF SET MASONRY WET UNITS LIGHTLY (IF REQ'D.) AND REMOVE LOOSE MASONRY UNITS AND MORTAR PRIOR TO LAYING FRESH MASONRY.

M18 REINFORCE MASONRY OPENINGS GREATER THAN 1'-0" WIDE WITH HORIZ. JT. REINF. PLACED IN (2) HORIZ. JTS. APPROXIMATELY 8" APART. IMMEDIATELY ABOVE THE LINTEL AND IMMEDIATELY BELOW THE SILL. EXTEND REINFORCING A MINIMUM OF 2'-0" BEYOND JAMBS OF THE OPENING EXCEPT AT CONTROL JOINTS. SEE PLAN FOR ADDITIONAL REQUIREMENTS.

M19 DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS.

M20 DO NOT APPLY CONCENTRATED LOADS TO MASONRY WALLS FOR (7) DAYS.

M21 EXTEND ALL VERTICAL REINFORCEMENT TO WITHIN 2" OF TOP OF TIE BEAMS UNLESS OTHERWISE NOTED.