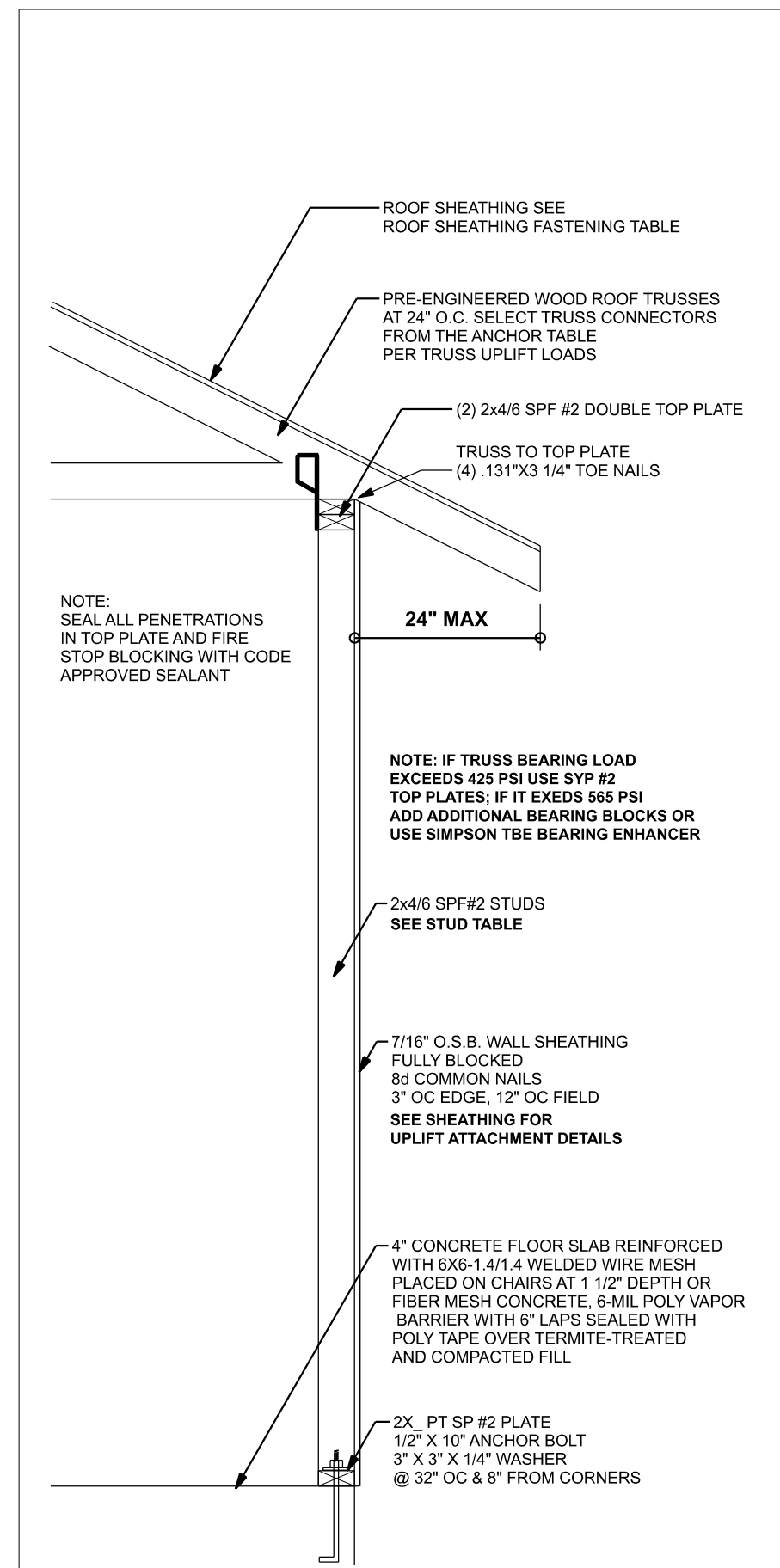
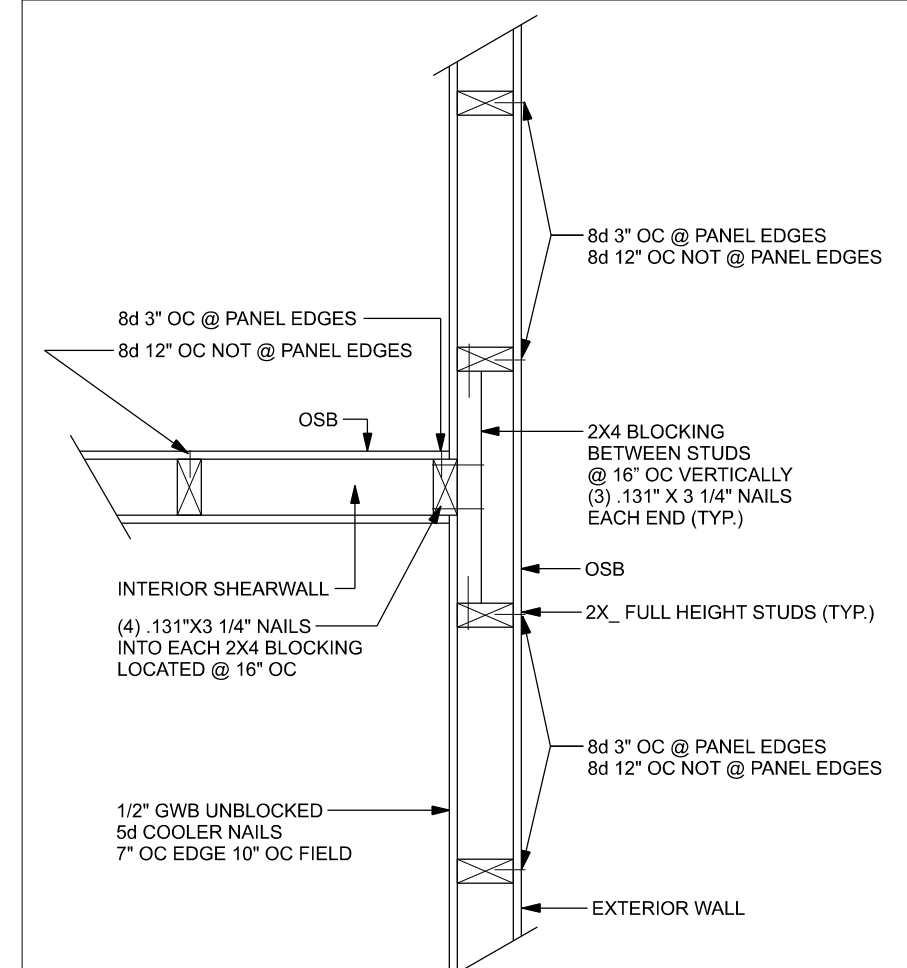
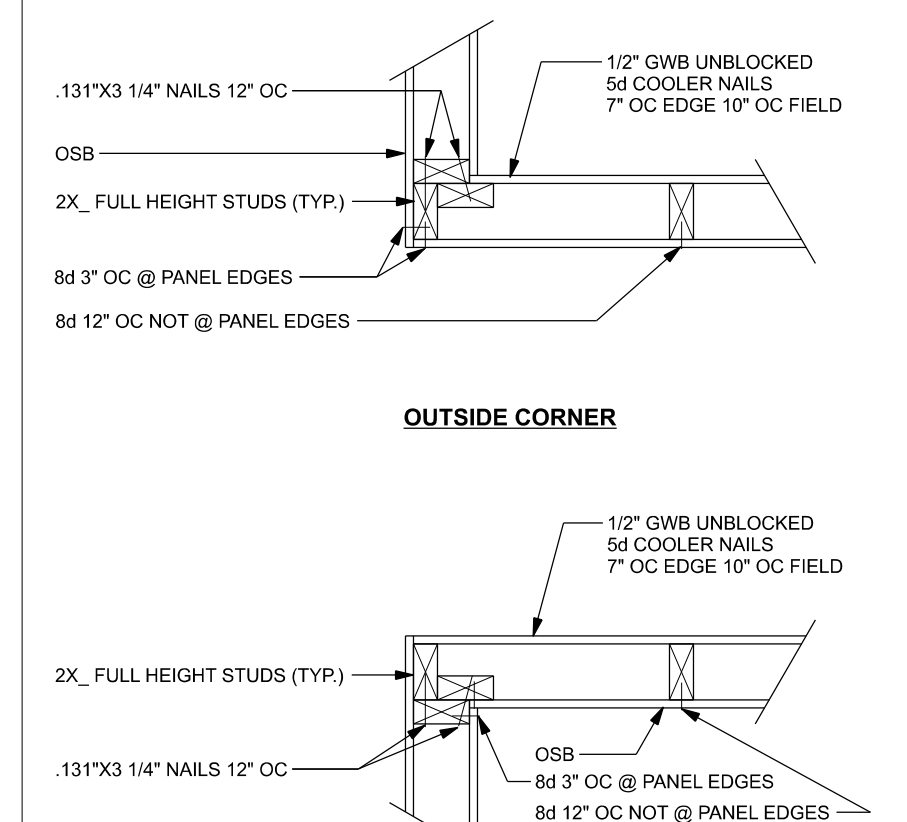




ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



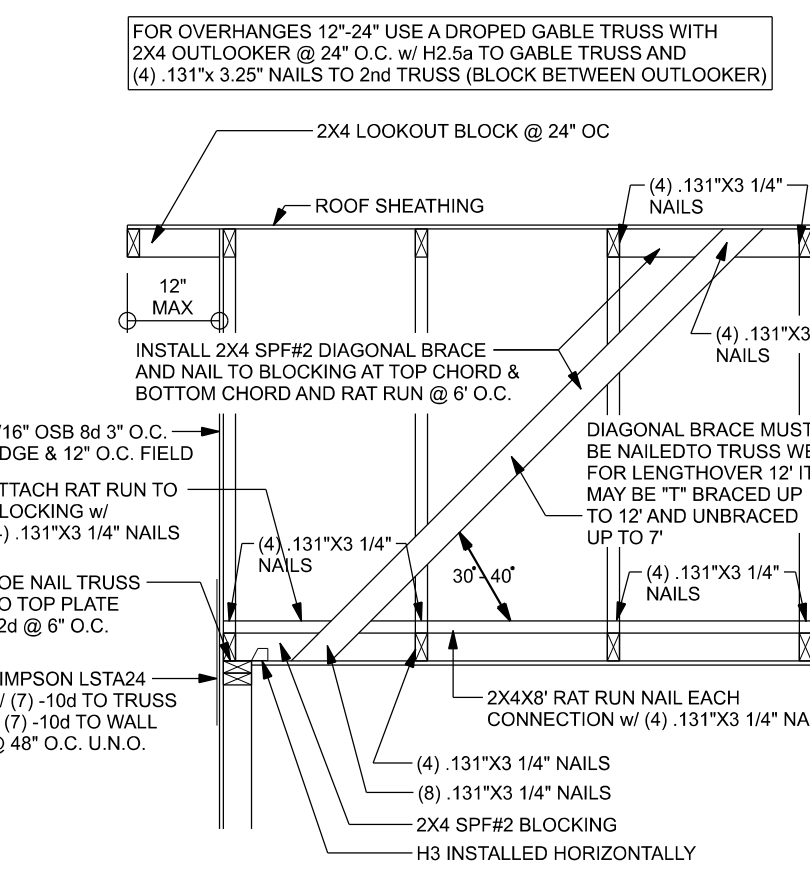
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (2" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (2" x 0.120")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (2" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (2" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (2" x 0.120")	6" oc	6" oc

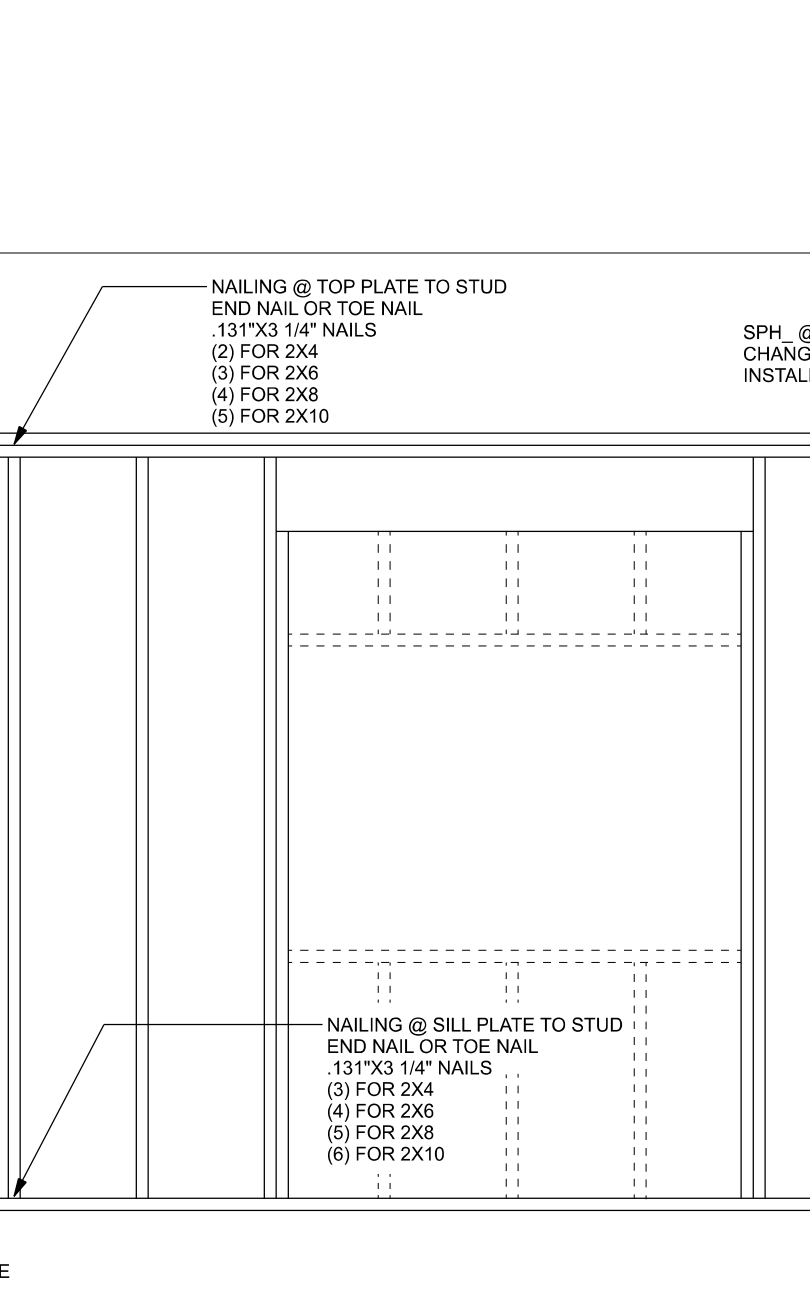
Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including a feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



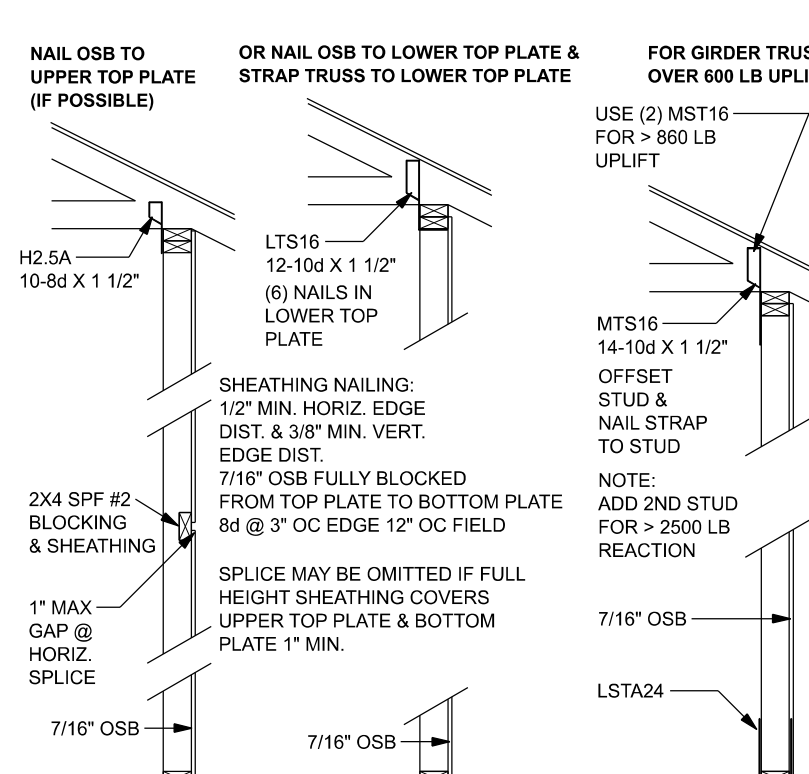
(TYP.) GABLE BRACING DETAIL
WOOD FRAME

Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 18-10d header to jacks	DTT22
< 1800	(2) MSTA24, 18-10d header to jacks	DTT22
< 2910	(2) MSTA24, 18-10d header to jacks	HTT4

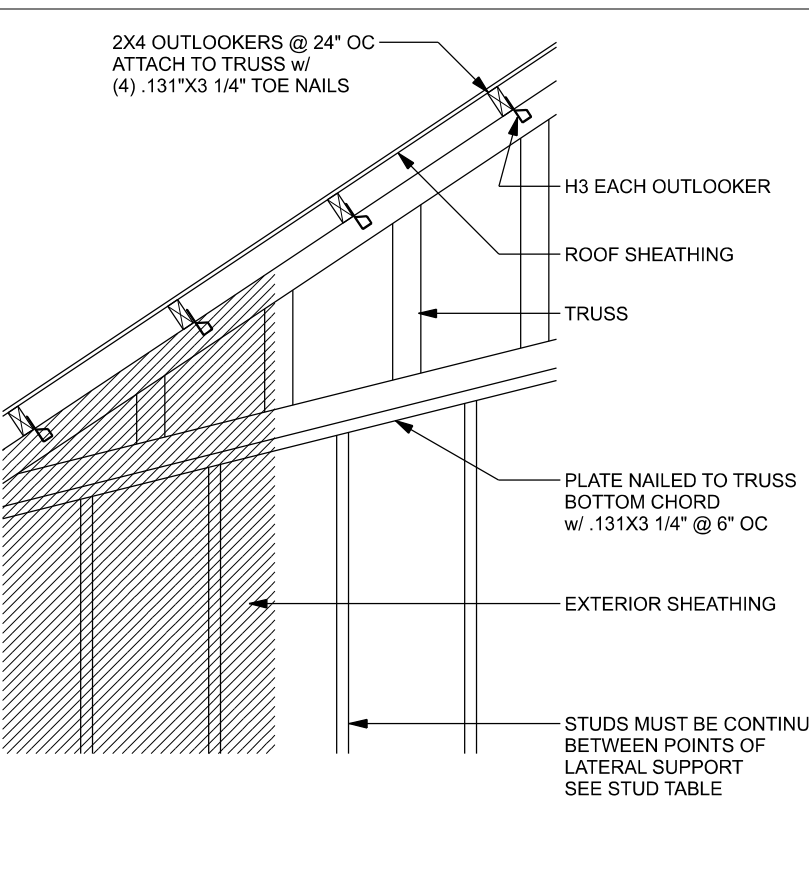
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2(8)
	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	
130 MPH EXP. C	5'-3" 7'-9" 7'-7" 11'-3"	FOR OTHER WALL HEIGHTS, SILL SPAN SHALL BE DIVIDED BY (H/10)



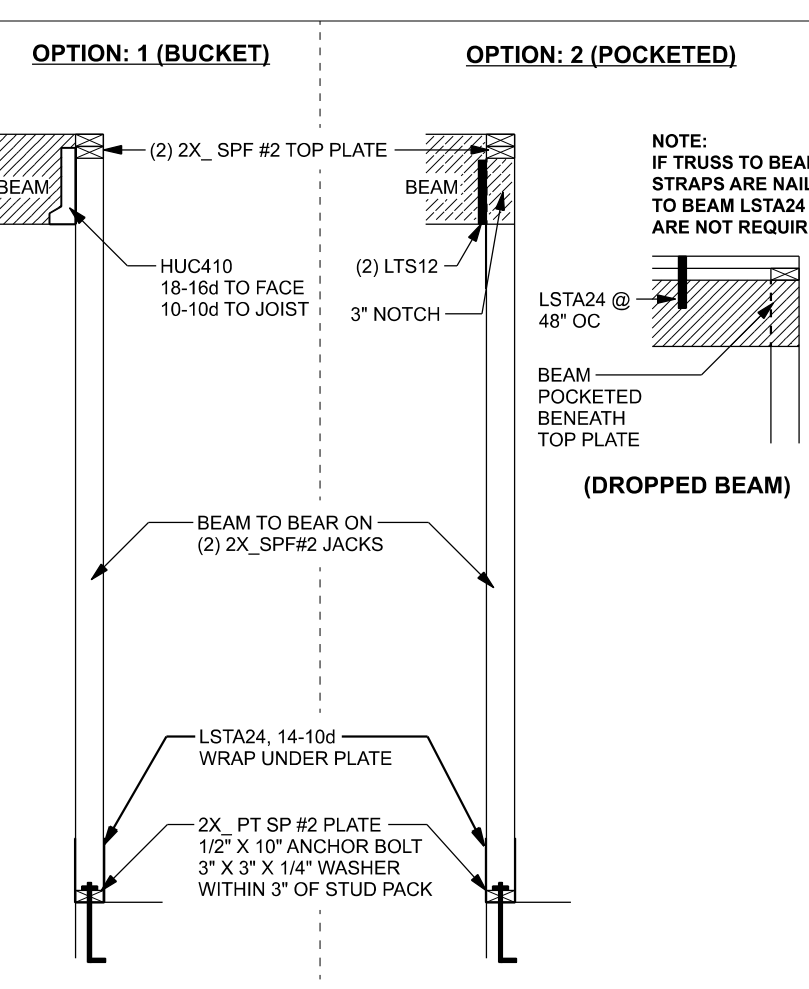
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



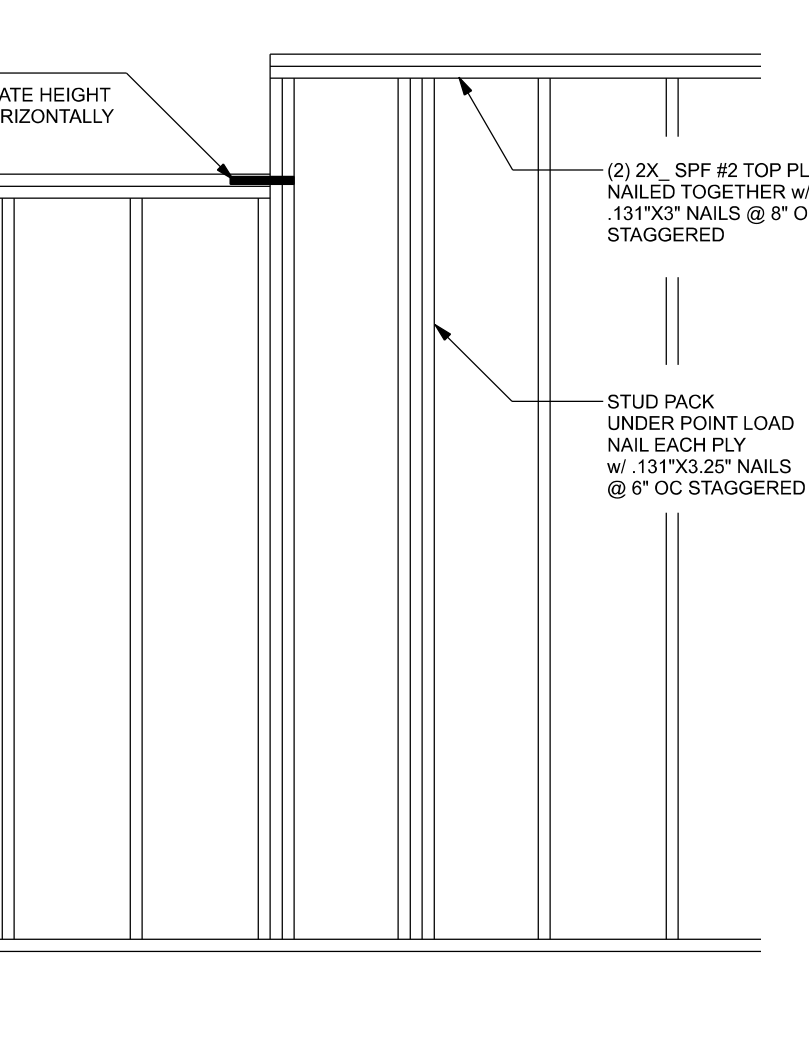
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



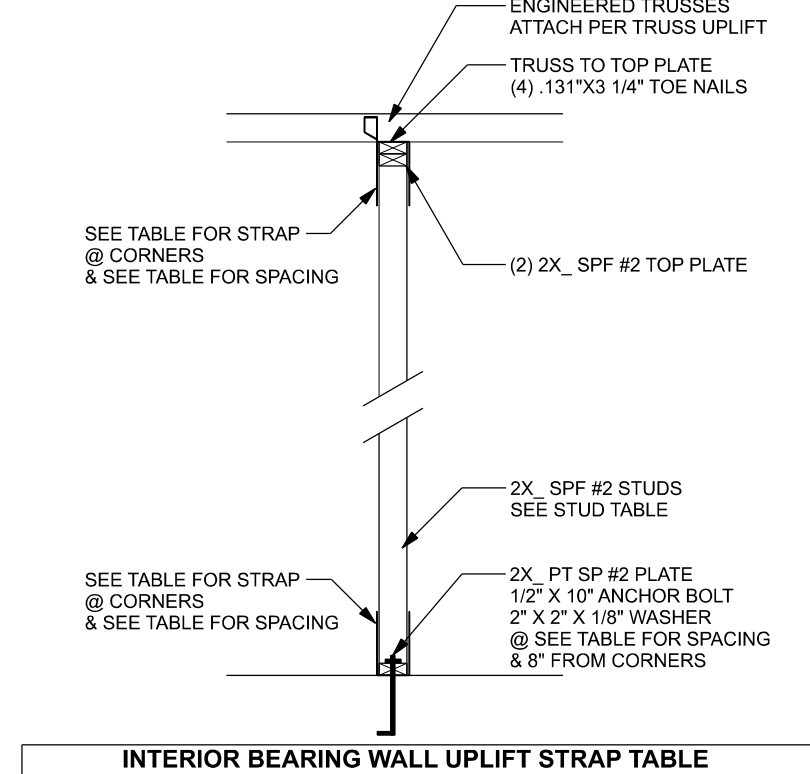
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



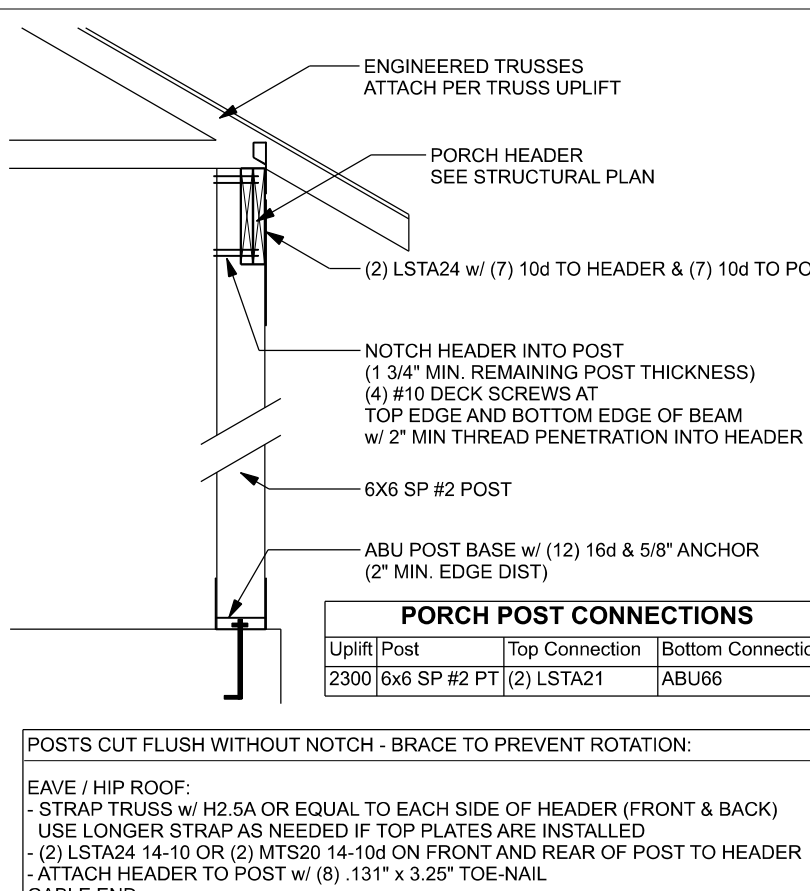
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



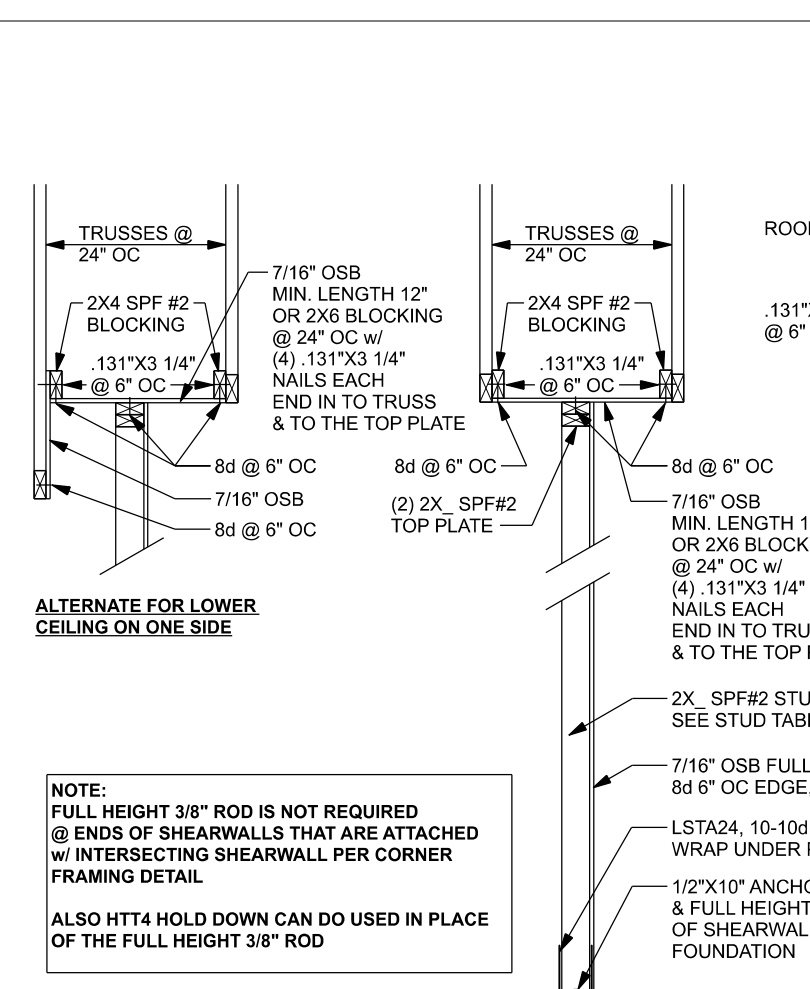
OPENING FORCE TRANSFER
WOOD FRAME



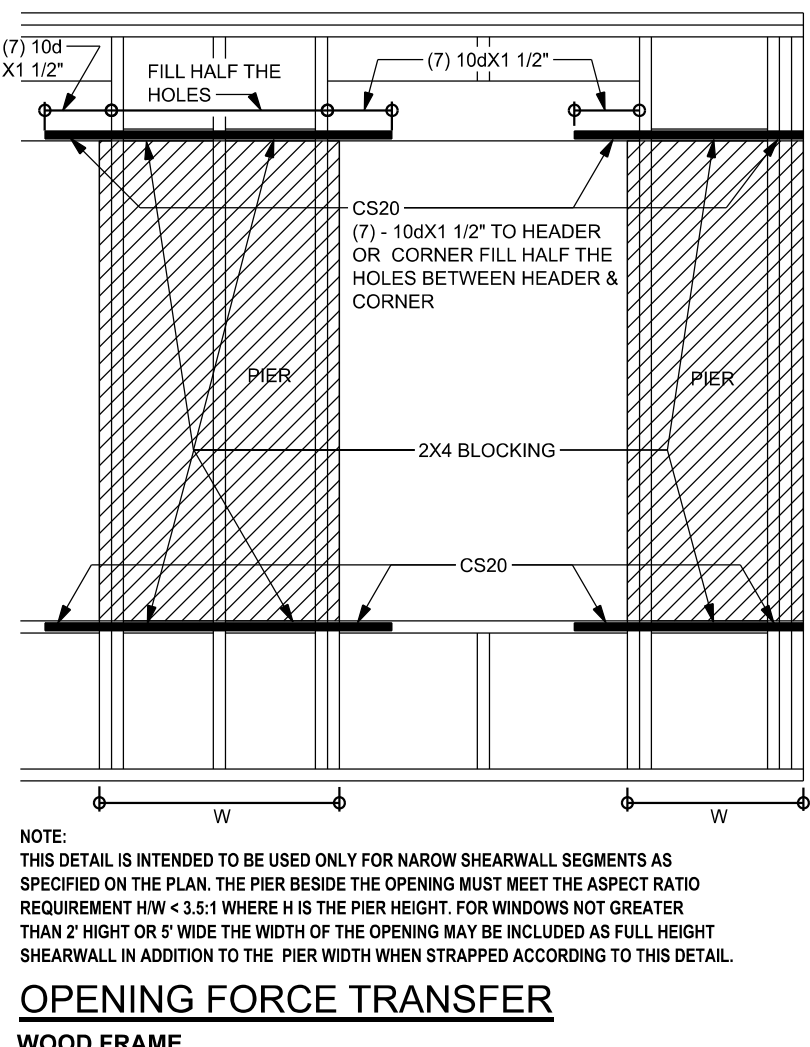
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST
ONE STORY WOOD



INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600		
415	290	H3	4-8dX1 1/2"	4-8dX1 1/2"
575	496	H2.5A	5-8dX1 1/2"	5-8dX1 1/2"
1340	1015	H10A	9-10dX1 1/2"	9-10dX1 1/2"
720	620	LTS12-20	6-10dX1 1/2"	6-10dX1 1/2"
1000	860	MTS12-30	7-10dX1 1/2"	7-10dX1 1/2"
1450	1245	HTS20-30	12-10dX1 1/2"	12-10dX1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	9-10d	9-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	9-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	16-16dX2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	16-16dX2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)		
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT	
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT	
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT	
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT	

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO TURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X6 RACON. WITH MIN. UPLIFT CONNECTION 415LB EACH END. 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1600 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL TEST PROVIDES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F_y = 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. BUILDERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED ONLY.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. IS BASED ON REACTION, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON DESIGN SHEETS.

Amira Custom Homes

Hicks Res.

PROJECT ADDRESS:
SW Marynuk Drive Parcel #16-75-17-10006-201
High Springs, FL 32608

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

MARK DISOWAY P.E. 53915

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Monday, August 2, 2021

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386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
211086

S-1
OF 3 SHEETS

(1) #5 CONTINUOUS IN HEADER-BLOCK BOND BEAM @ SLAB EDGE INTERSECTION w/ STEINWALL

#6 VERT. REBAR w/ STD. HOOK IN FOOTING @ EACH CORNER & 12" OC

8X8X16, RUNNING BOND

12" MIN. SPACING

SEE SPECIAL REINFORCEMENT TABLE FOR MORE THAN 5 COURSES

20" X 14" D POLISHED CONCRETE STRIP FOOTING

3" MIN. COVER (TYP.)

CONCRETE SLAB

)

Diagram illustrating the concrete slab and footing for a porch. The slab is labeled "CONCRETE SLAB" and is shown with a cross-section. The footing is labeled "12' W X 16' D FOR PORCH MONO FOOTING w/ (2) #5 CONTINUOUS". The slab is shown with a cross-section of 12' W X 20' D FOR HOUSE.

CONFIDENTIAL AND PROPRIETARY

INTERIOR DE

12"

ERIOR BEAR

(continued)

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OF THE

E: 1/2" = 1'-0"

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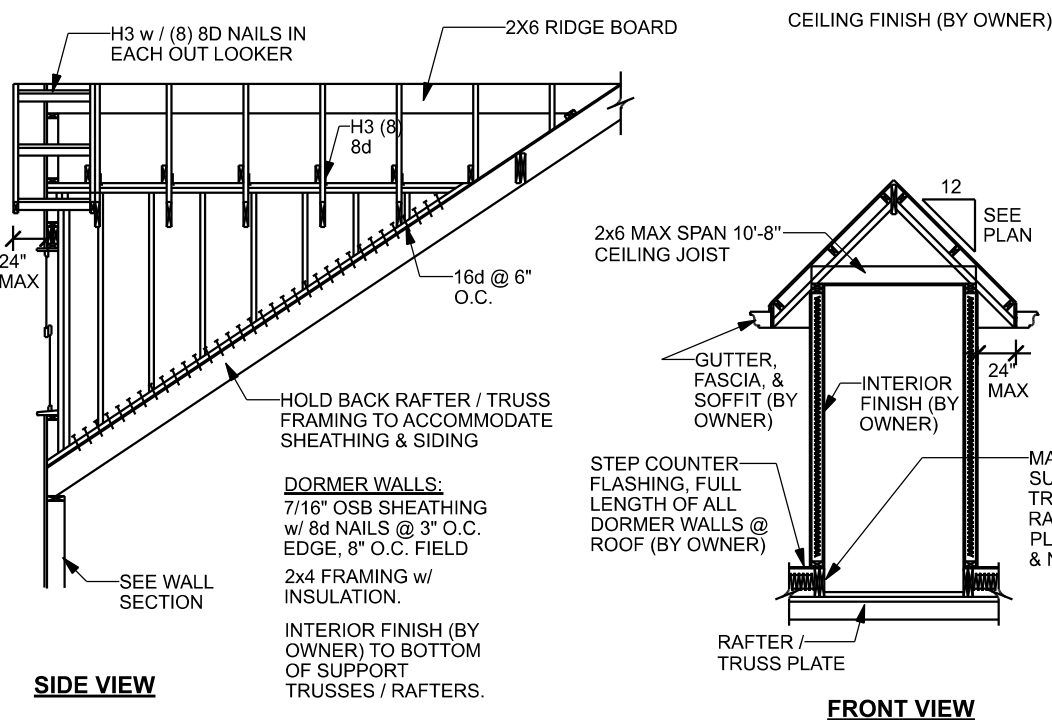
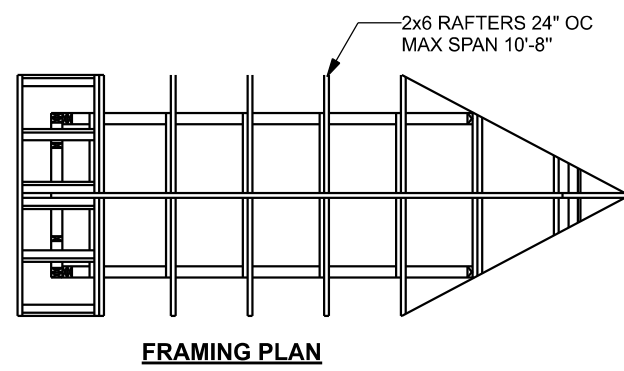
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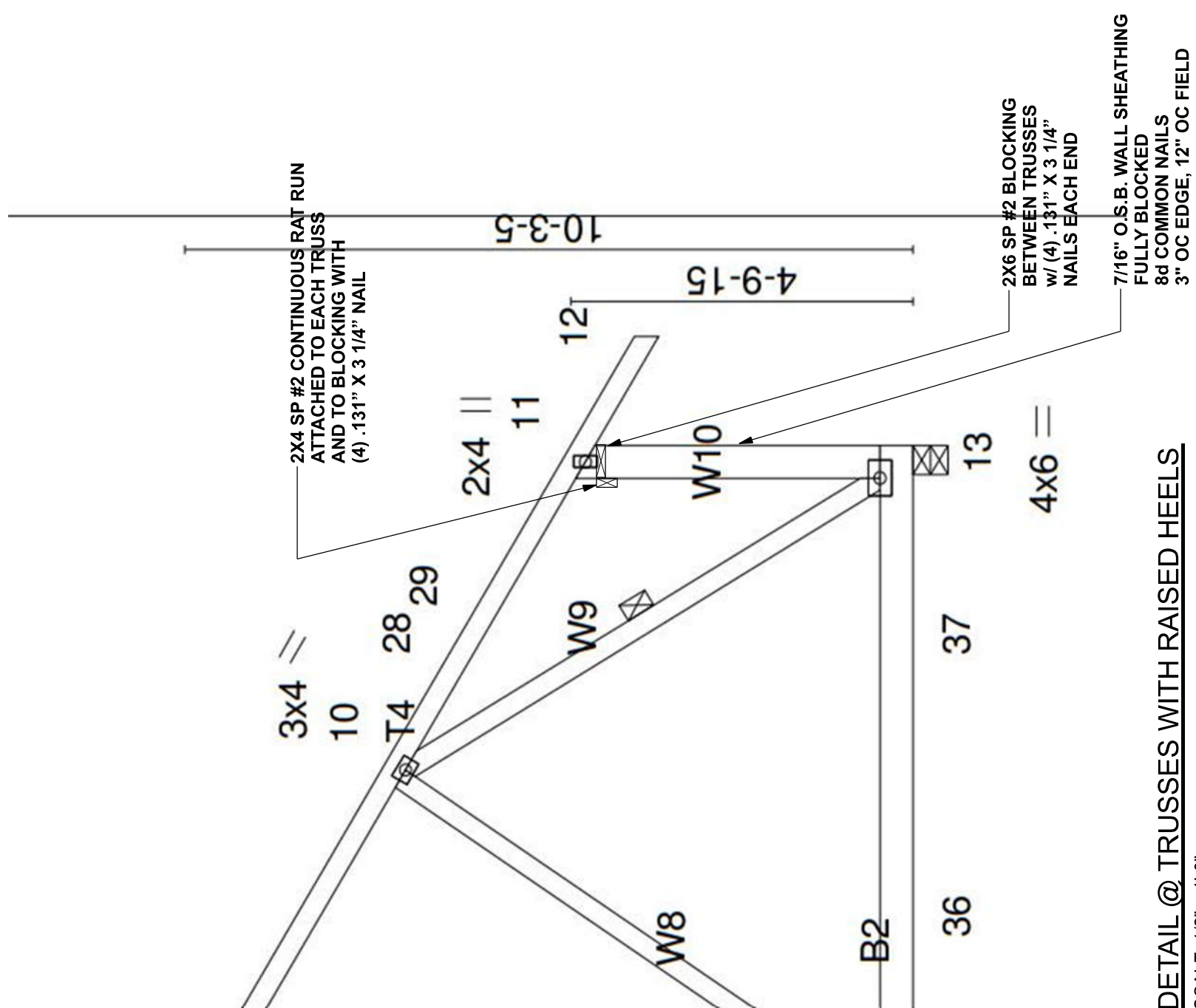
on

Hicks Res

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DORMER ANCHORING DETAIL (ON ROOF)
SCALE: N.T.S.



DETAIL @ TRUSSES WITH RAISED HEELS
SCALE: 1/2" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1** ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2** ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3** ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE. THE STRAP SHALL BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4** USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5** DIMENSIONS ON STRUCTURAL SHEETS SHALL BE USED FOR ACTUAL DIMENSIONS
- SN-6** PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS SHOWN ON THE SEALED TRUSS DRAWINGS. BRACING SHALL BE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

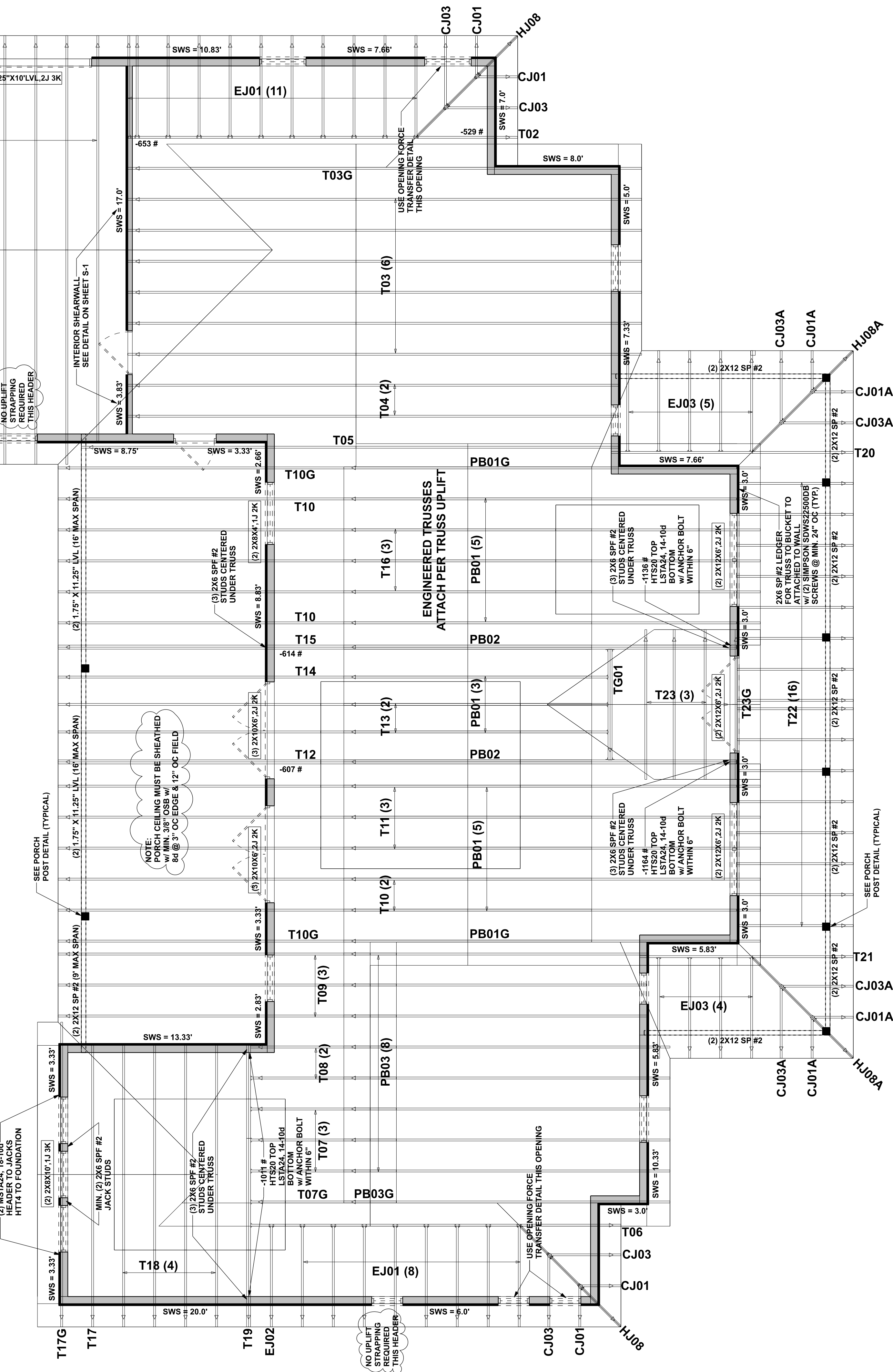
HEADER LEGEND

- (2) 2X6X10' 1/1" K ← HEADER/BEAM CALL-OUT (U.N.O.)
- ← NUMBER OF KING STUDS (FULL LENGTH)
- ← NUMBER OF JACK STUDS (UNDER HEADER)
- ← SPAN OF HEADER
- ← SIZE OF HEADER MATERIAL
- ← NUMBER OF PILES IN HEADER

ACTUAL vs REQUIRED SHEARWALL	
TRANSVERSE	LONGITUDINAL
ACTUAL	23370 LBF
REQUIRED	21039 LBF

ALTERNATE IF TRUSSES ARE PERPENDICULAR TO SHEARWALL

NOTE: IF THE ABOVE DETAIL IS USED ON THE REAR WALL SHEARWALL, THE PORCH CEILING DOES NOT NEED TO BE SHEATHED



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

Amira Custom Homes

Hicks Res.

PROJECT ADDRESS:
SW Marynuk Drive Parcel #16-7S-17-10006-201
High Springs, FL 32643

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



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JOB NUMBER:
211086

S-3
OF 3 SHEETS

CONNECTIONS WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING BY THE TRUSS BUILDER. BUILDERS FIRST SOURCE JOB #2889120