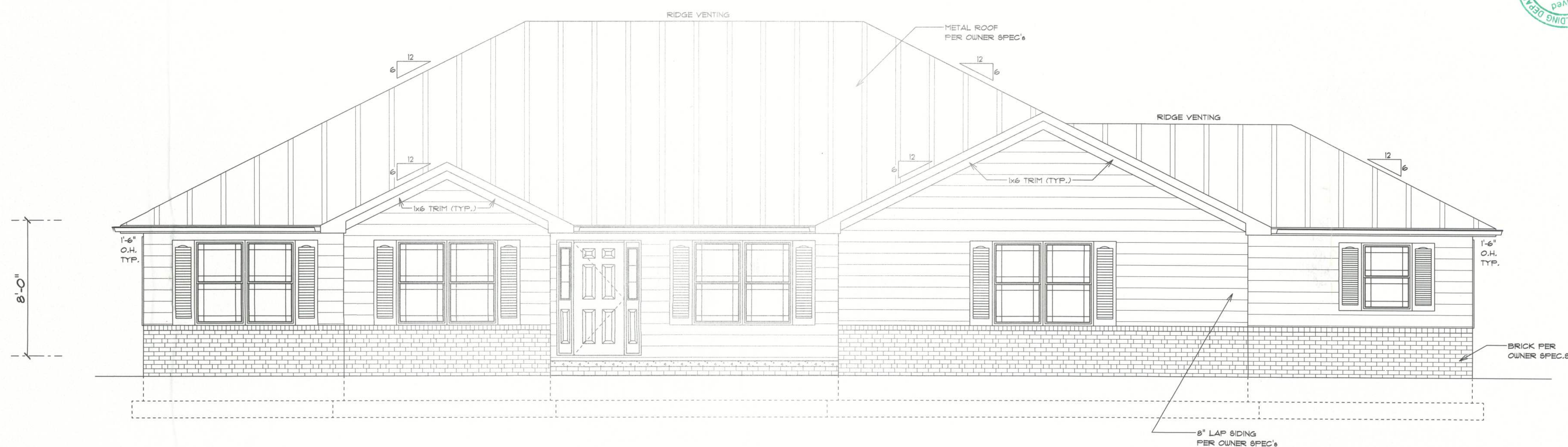
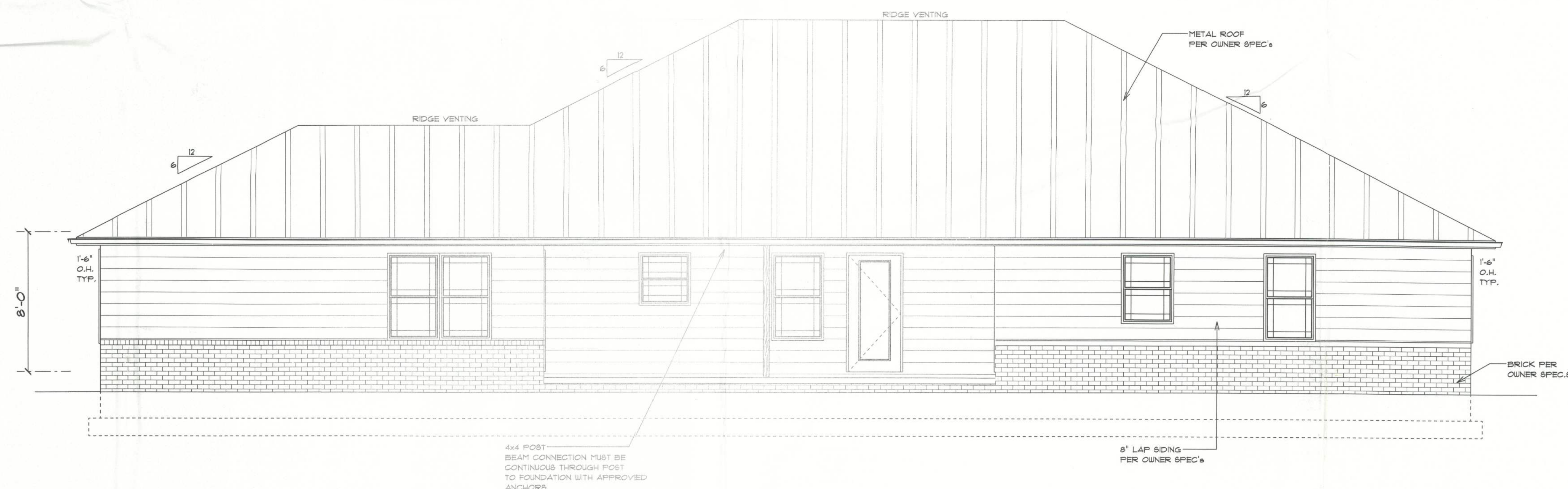


SITE PLAN
LOT II
WESTERWOODS S/D





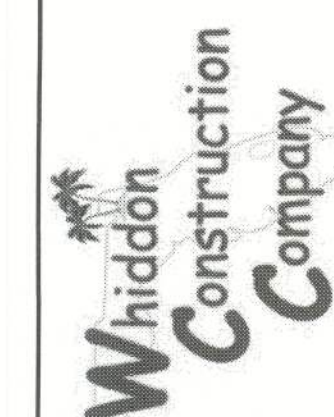
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

NO	DATE	REVISION
1	10/22/13	INITIAL RELEASE

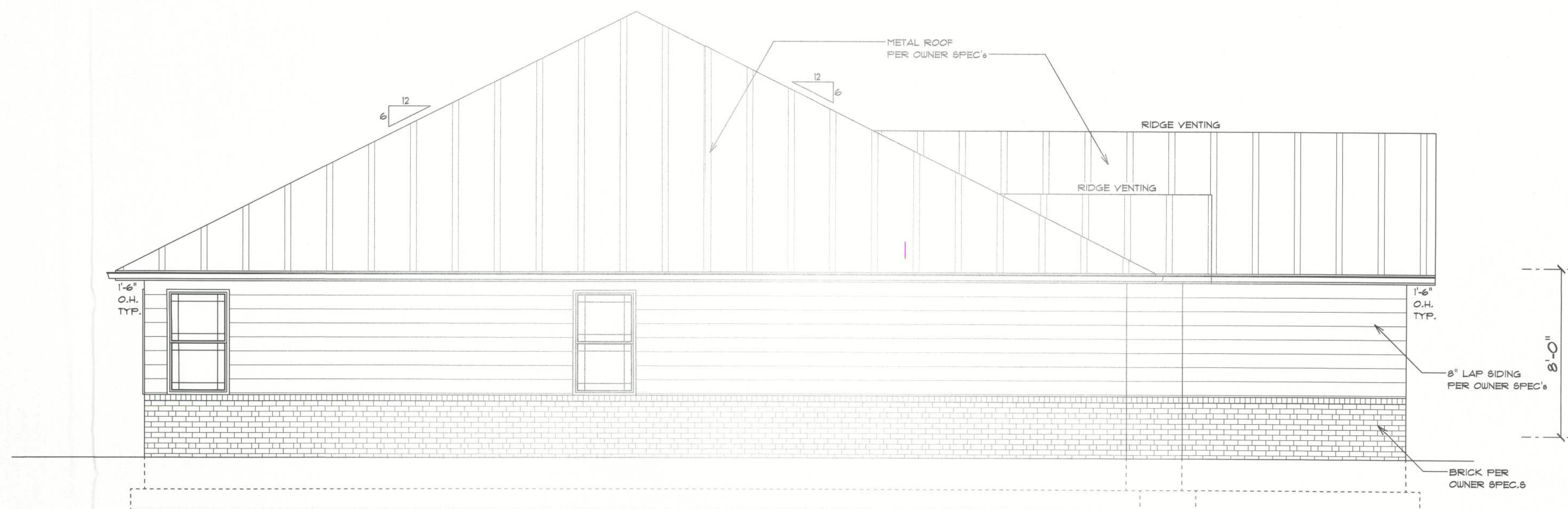
STATE CERTIFIED CONTRACTOR
LICENSE #CRC1328025
295 NW COMMONS LOOP
SUITE 115-262
LAKE CITY, FL 32055
OFFICE/FAX (386) 754-1361
WWW.WHIDDONCONSTRUCTION.COM



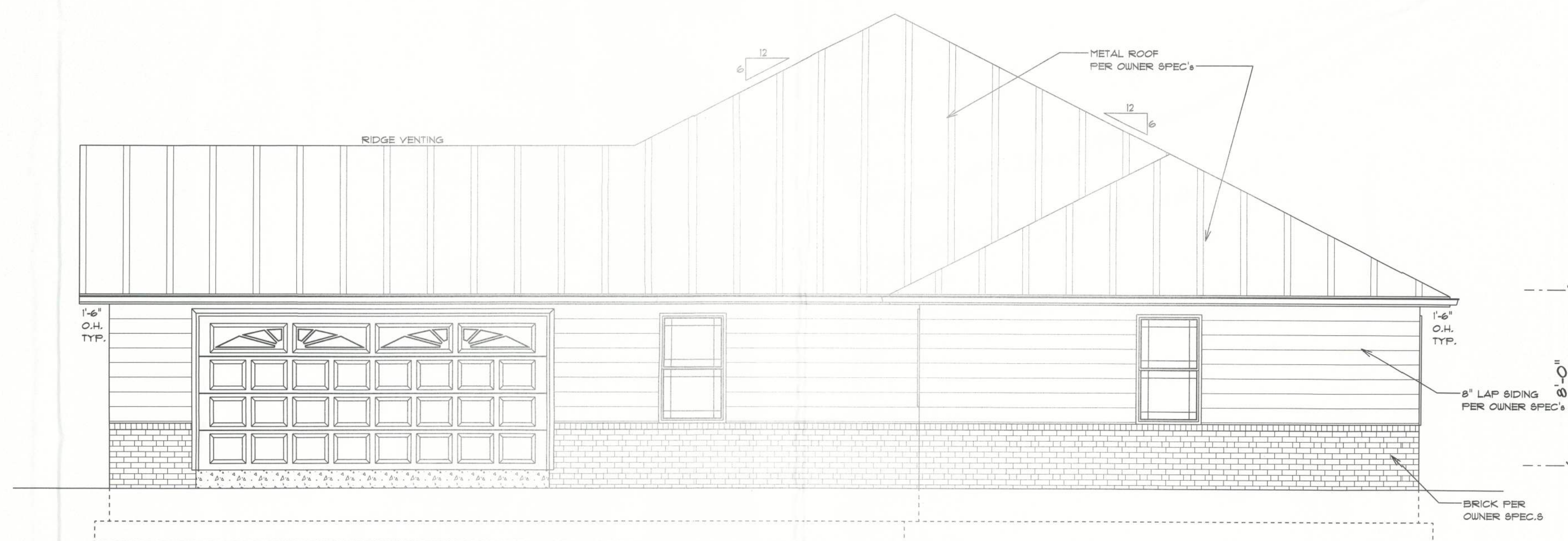
WHIDDON FAMILY RESIDENCE
LAKE CITY, FLORIDA

DATE
1/21/2013
PROJECT NO.
WHIDDON
DESIGNED BY
RDW
DRAWN BY
RDW
CHECKED BY
RDW
DWG. NO.

A-1
ELEVATIONS



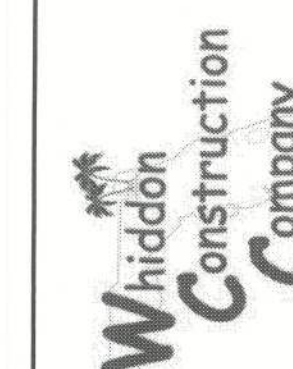
LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

NO.	DATE	REVISION
1	10/22/13	INITIAL RELEASE

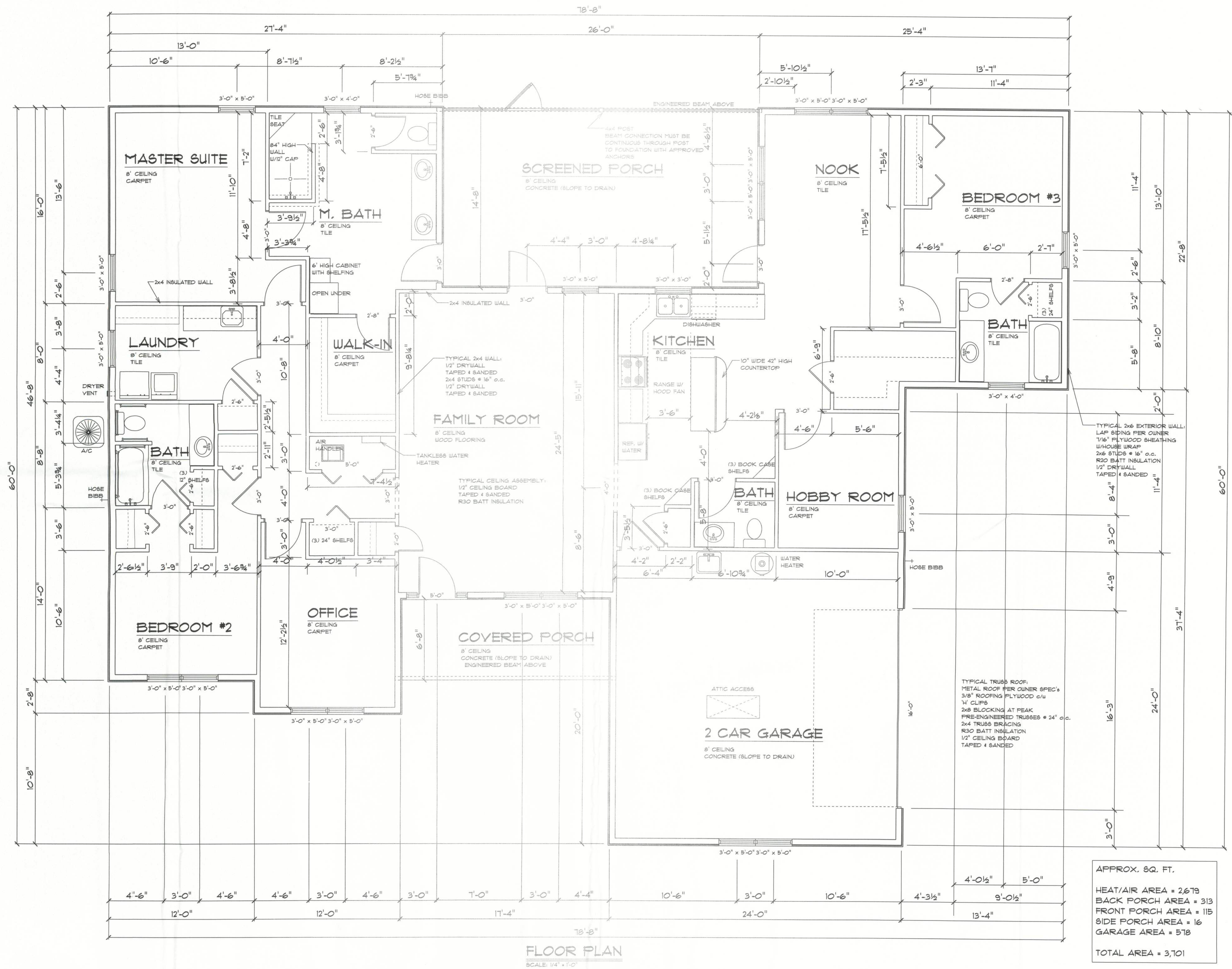
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LICENSE #CRC1328025
295 NW COMMONS LOOP
SUITE 115-262
LAKE CITY, FL 32055
OFFICE/FAX (386) 754-7361
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WHIDDON FAMILY RESIDENCE
LAKE CITY, FLORIDA

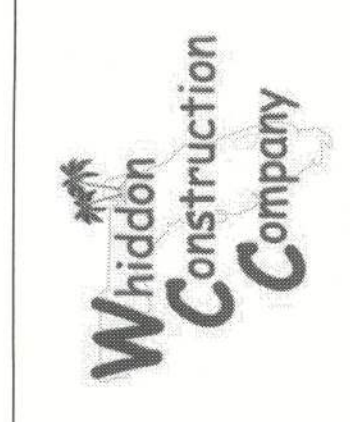
DATE
1/21/2013
PROJECT NO.
WHIDDON
DESIGNED BY
RDW
DRAWN BY
RDW
CHECKED BY
RDW
DWG. NO.

A-1a
ELEVATIONS



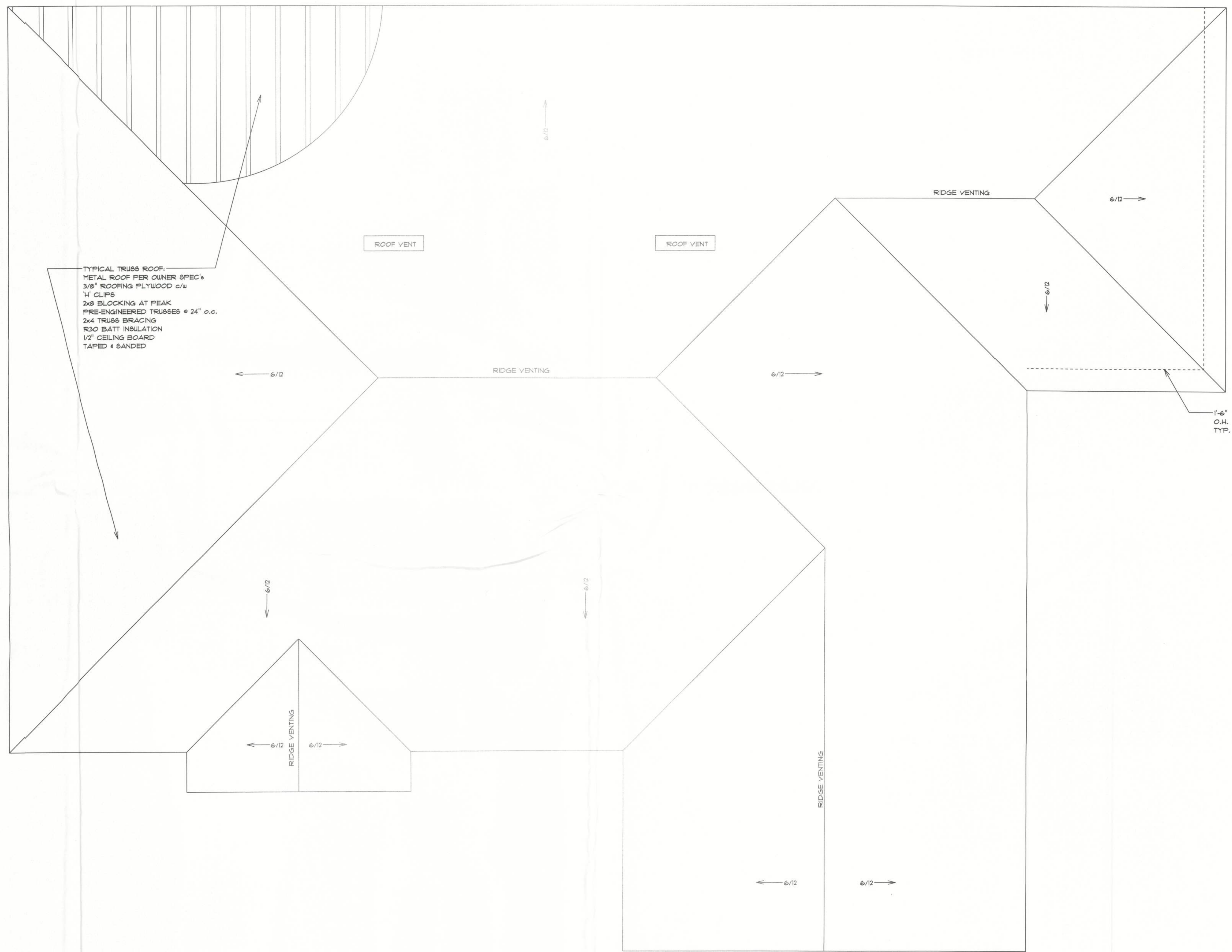
NO	DATE	REVISION
1	10/22/13	INITIAL RELEASE

STATE CERTIFIED CONTRACTOR
LICENSE #CRC1328025
295 NW COMMONS LOOP
SUITE 115-262
LAKE CITY, FL 32055
OFFICE/FAX (386) 754-1361
WWW.WHIDDONCONSTRUCTION.COM



WHIDDON FAMILY RESIDENCE LAKE CITY, FLORIDA

DATE 1/21/2013
PROJECT NO. WHIDDON
DESIGNED BY TEB
DRAWN BY TEB
CHECKED BY TEB
DWG. NO. A-3
FLOOR PLAN



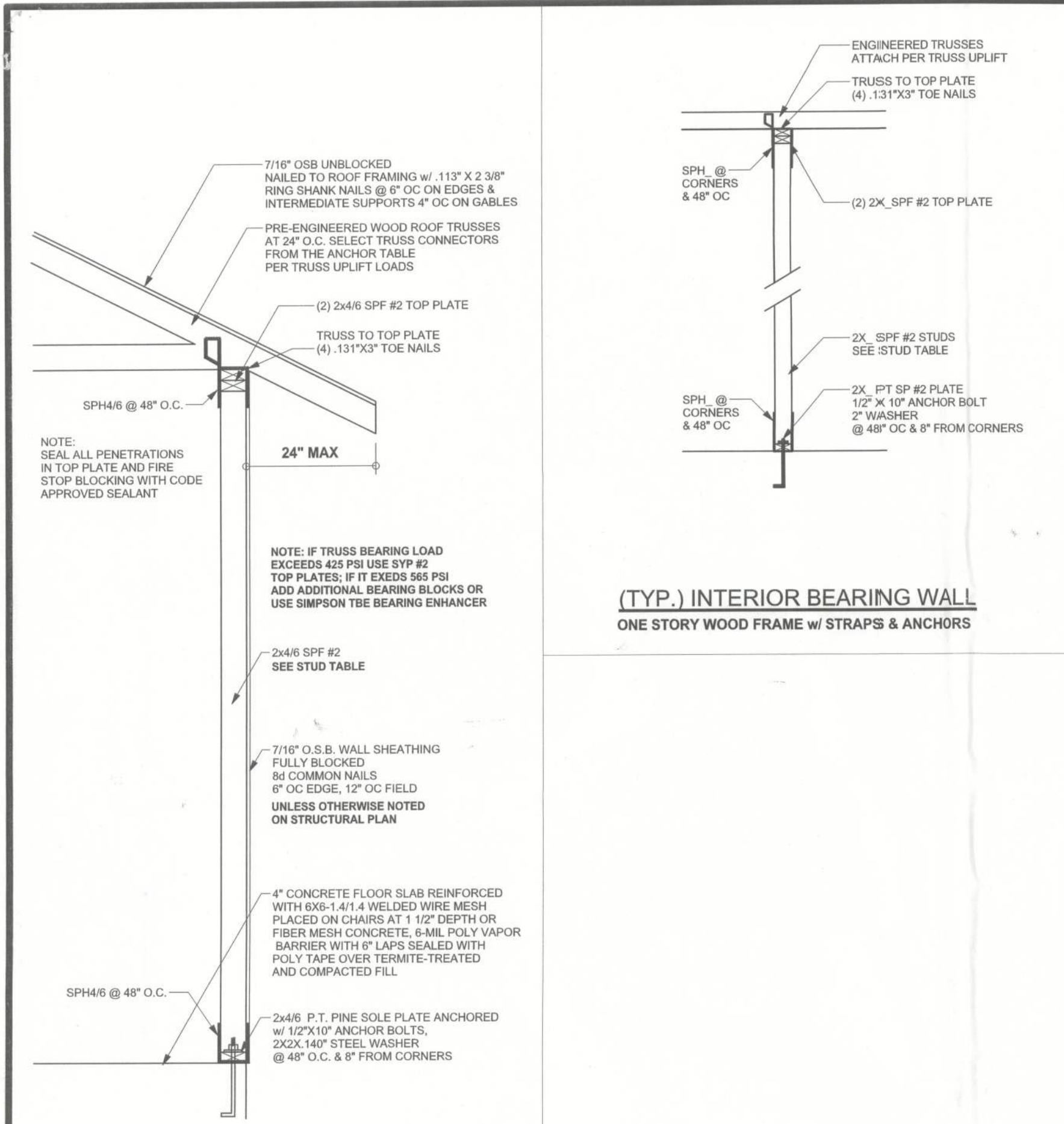
ROOF PLAN
SCALE: 1/4" = 1'-0"

DATE		1/21/2013	
PROJECT NO.		WHIDDON	
DESIGNED BY		RDW	
DRAWN BY		RDW	
CHECKED BY		RDW	
DWG. NO.		A-5	
		ROOF PLAN	

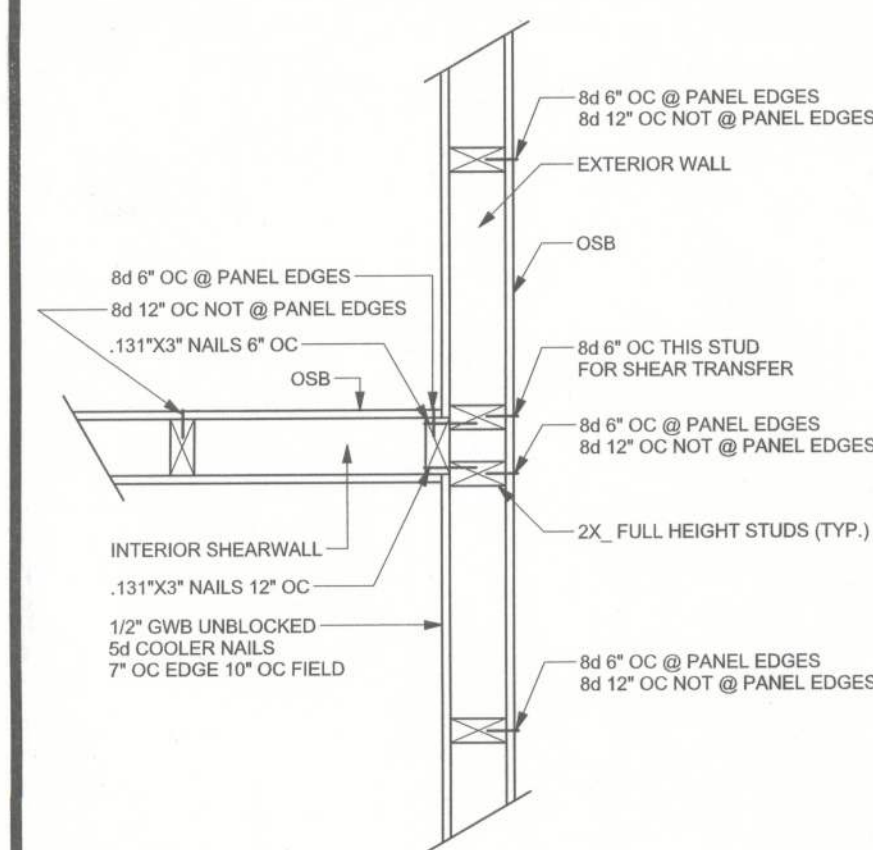
WHIDDON CONSTRUCTION COMPANY
LAKE CITY, FLORIDA

STATE CERTIFIED CONTRACTOR
LICENSE #CRC1328025
295 NW COMMONS LOOP
SUITE 115-262
LAKE CITY, FL 32055
OFFICE/FAX (386) 754-1361
WWW.WHIDDONCONSTRUCTION.COM

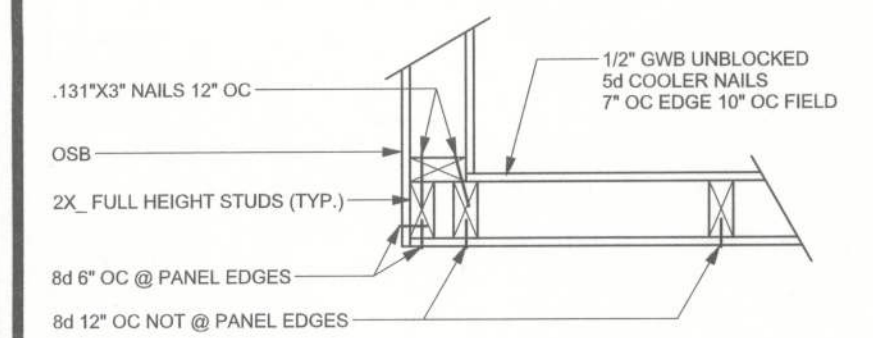
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1	10/22/13	INITIAL RELEASE



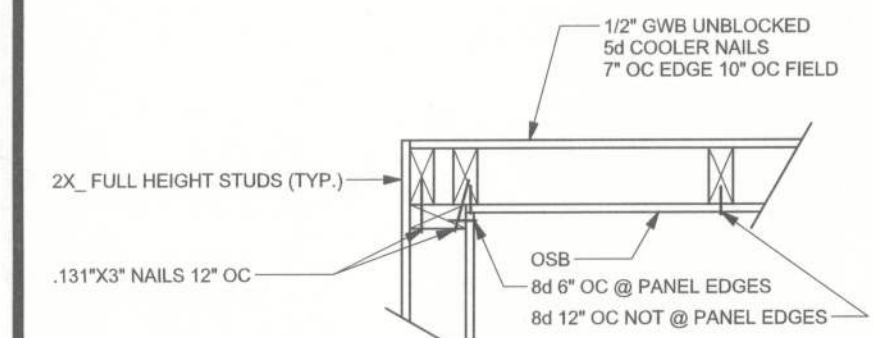
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



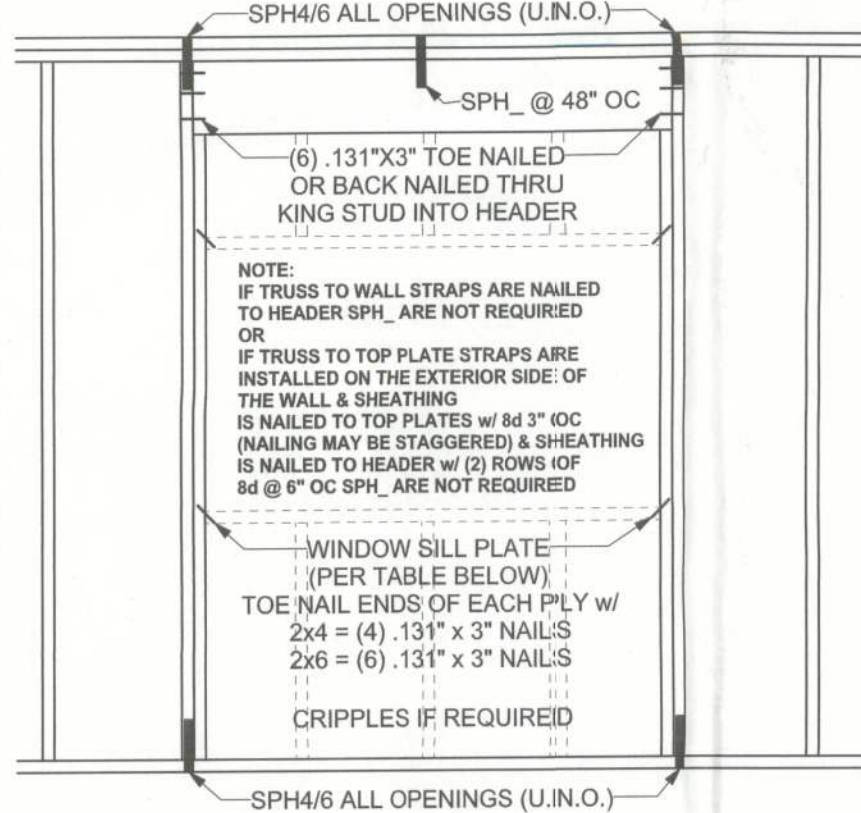
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



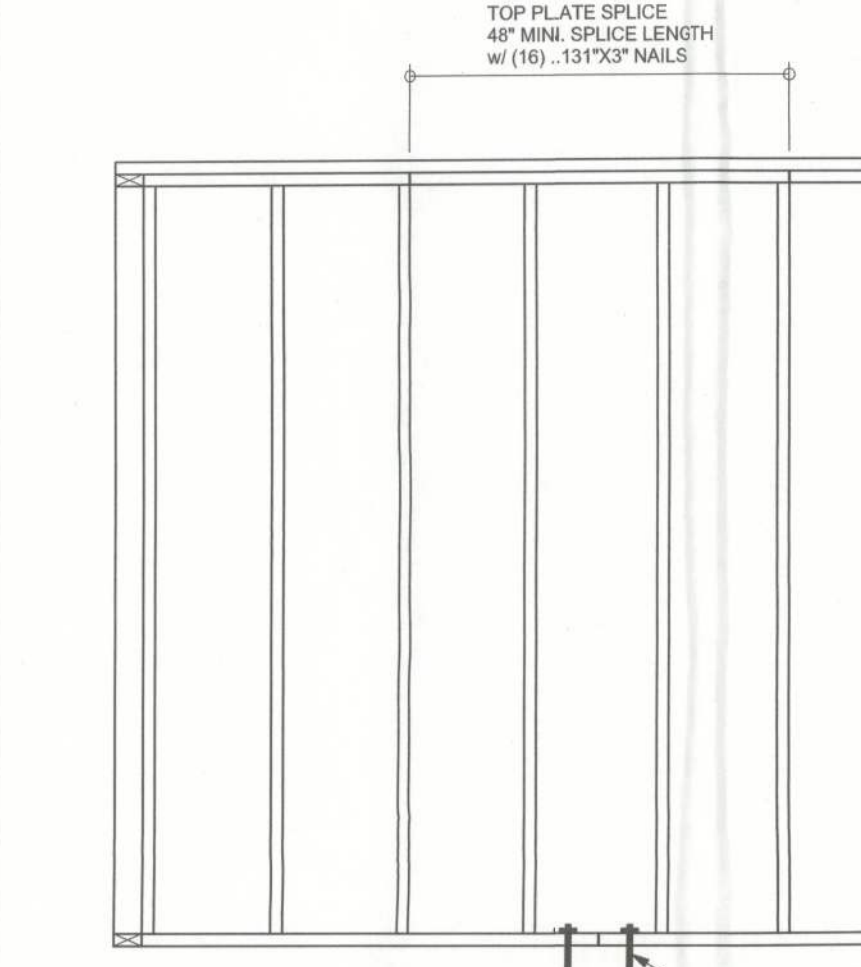
OUTSIDE CORNER



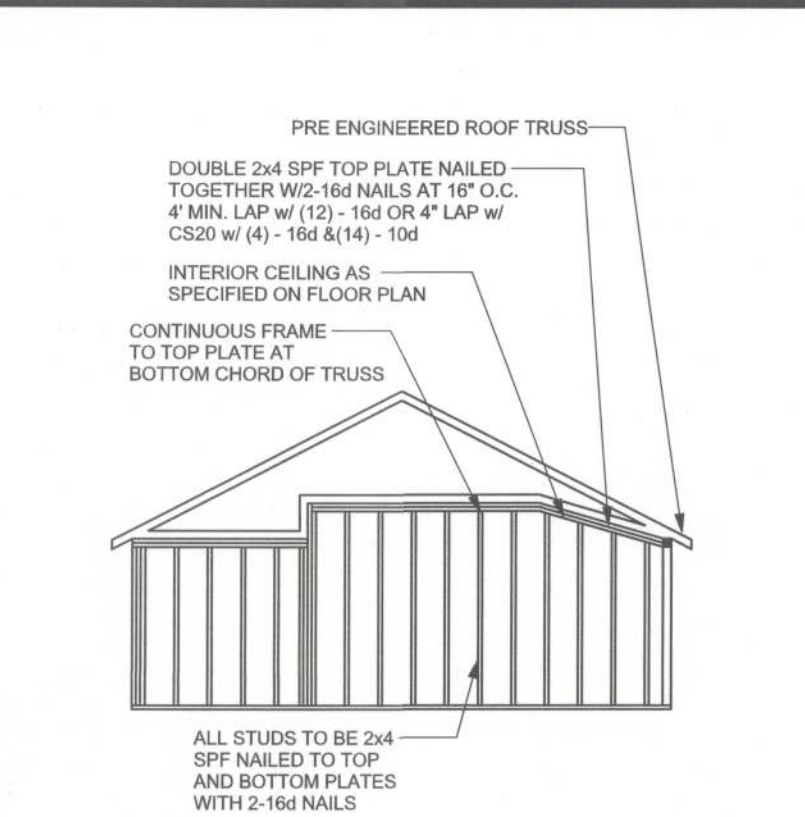
(TYP.) CORNER FRAMING
WOOD FRAME



TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

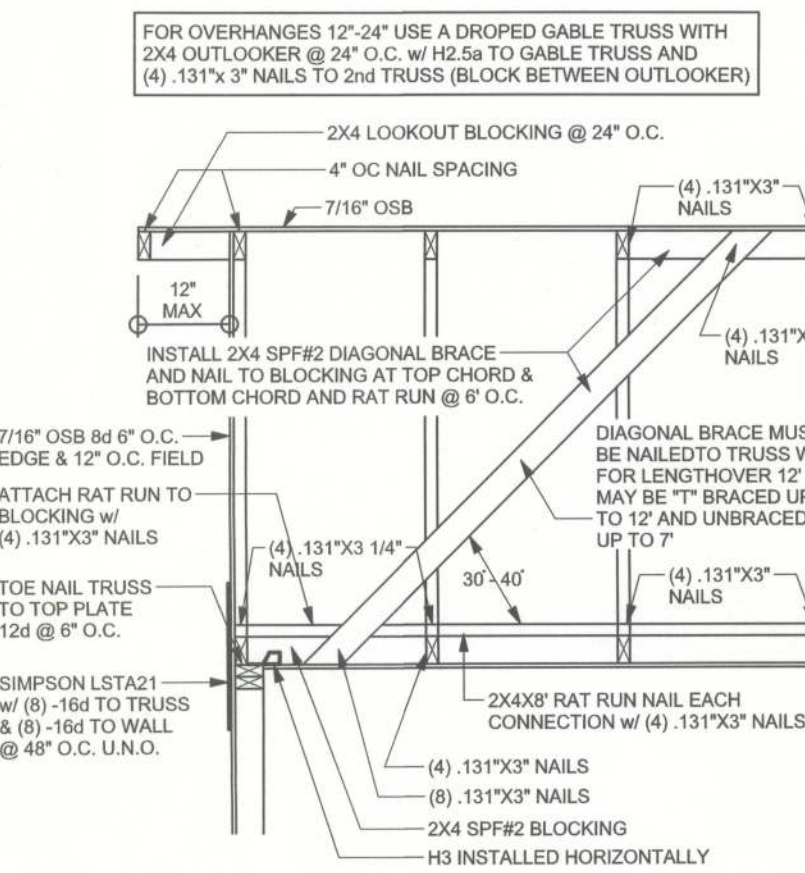


(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



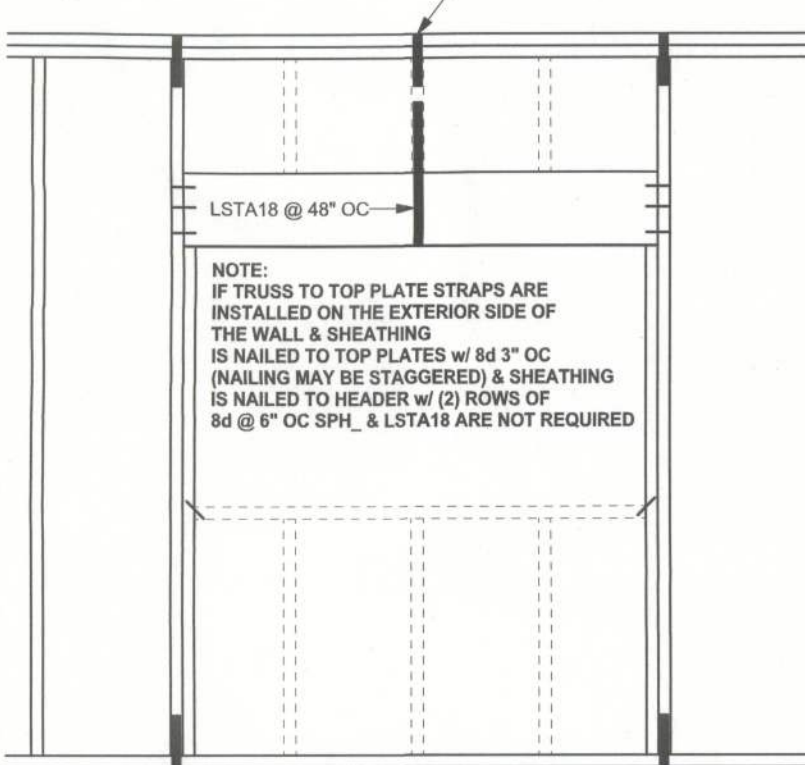
CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



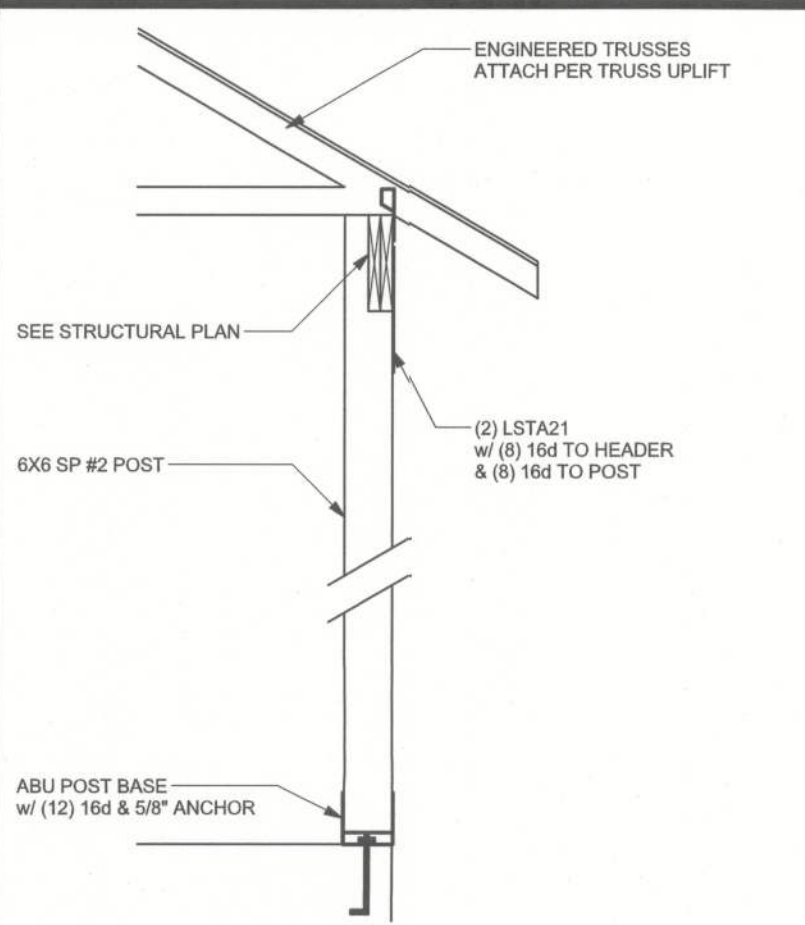
SPACE RAT RUN & DIAGONAL BRACE 6'-0\"/>

(TYP.) GABLE BRACING DETAIL
WOOD FRAME



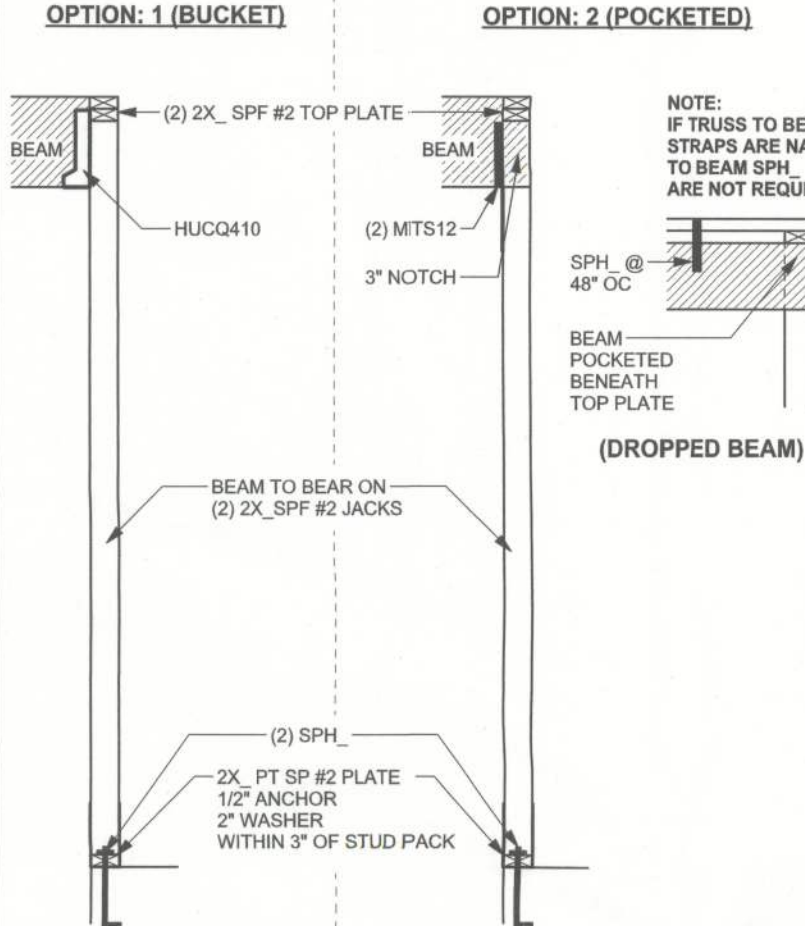
OPTION: 2 (DROPPED HEADER)
SILL PLATE SPANS FOR 10'-0\"/>

DESIGN WIND SPEED (1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6
115-130 MPH 5'-3\"/>



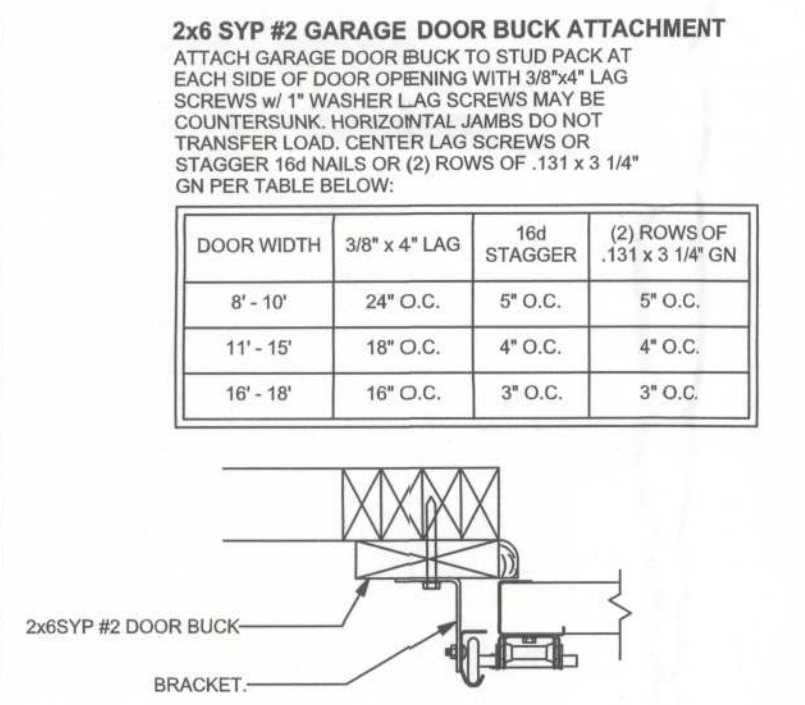
(TYP.) PORCH POST
ONE STORY WOOD

ALLOWABLE UPLIFT: 2200 LB



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

ALLOWABLE UPLIFT: 2000 LB



GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS BEARING ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12\"/>

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPLICES 40\"/>

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2.4$ ksi, $E = 1800$ ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8\"/>
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	GROUT	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8\"/>
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5\"/>
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia. (30\"/>
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	925	1.6
2x10	SYP #2	800	1.6
2x12	SYP #2	750	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16\"/>	TO 10'-1\"/>
(1) 2x4 @ 12\"/>	TO 11'-2\"/>
(1) 2x6 @ 16\"/>	TO 15'-7\"/>
(1) 2x6 @ 12\"/>	TO 17'-3\"/>

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2.0B4. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2\"/>

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H6	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG1		22-10d	1-5/8\"/>
< 10980	< 6485	HGT-2		16-10d	2-5/8\"/>
< 10530	< 9035	HGT-3		16-10d	2-5/8\"/>
< 9250	< 9250	HGT-4		16-10d	2-5/8\"/>
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8-16d		1-2\"/>
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1-2\"/>
< 2775	< 2570	HD2A	2-5/8\"/>		5/8\"/>
< 4175	< 3695	HT116	18-16d		5/8\"/>
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1-2\"/>
< 2300	< 2300	ABU66	12-16d		1-2\"/>
< 2320	< 2320	ABU88	18-16d		2-5/8\"/>

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)
2.) WIND EXPOSURE = C. BUILDER MUST FIELD VERIFY
3.) TOPOGRAPHIC FACTOR = 1.0. BUILDER MUST FIELD VERIFY
4.) RISK CATEGORY = II, (MRI = 700 YR)
5.) ROOF ANGLE = 7-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	
1	39	-43
2	39	-48
3	39	-100
4	43	-46
5	43	-57

Garage Door	2010 FBCR, Table R301.2.4)	
8x7 Garage Door	37	-42
16x7 Garage Door	36	-40

DESIGN LOADS
FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Whiddon Construction

Whiddon Residence

ADDRESS:
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
Friday, October 25, 2013

DRAWN BY: STRUCTURAL BY:

FINALS DATE: 19Oct13

JOB NUMBER:
1309026

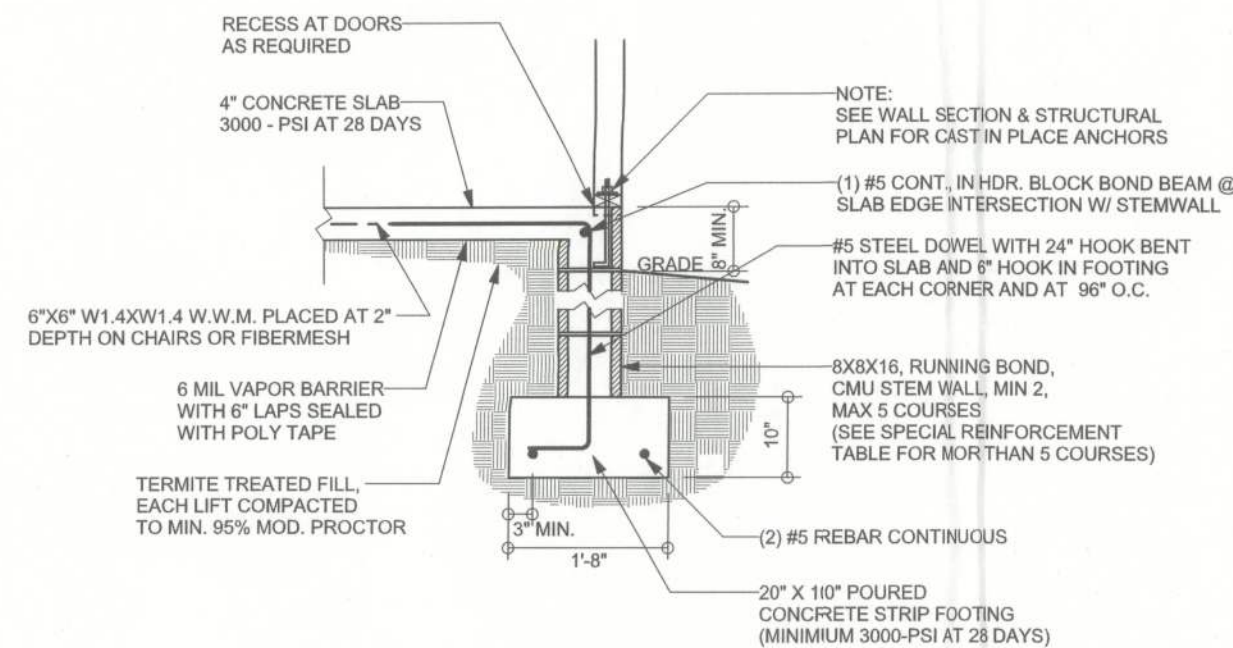
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S-1

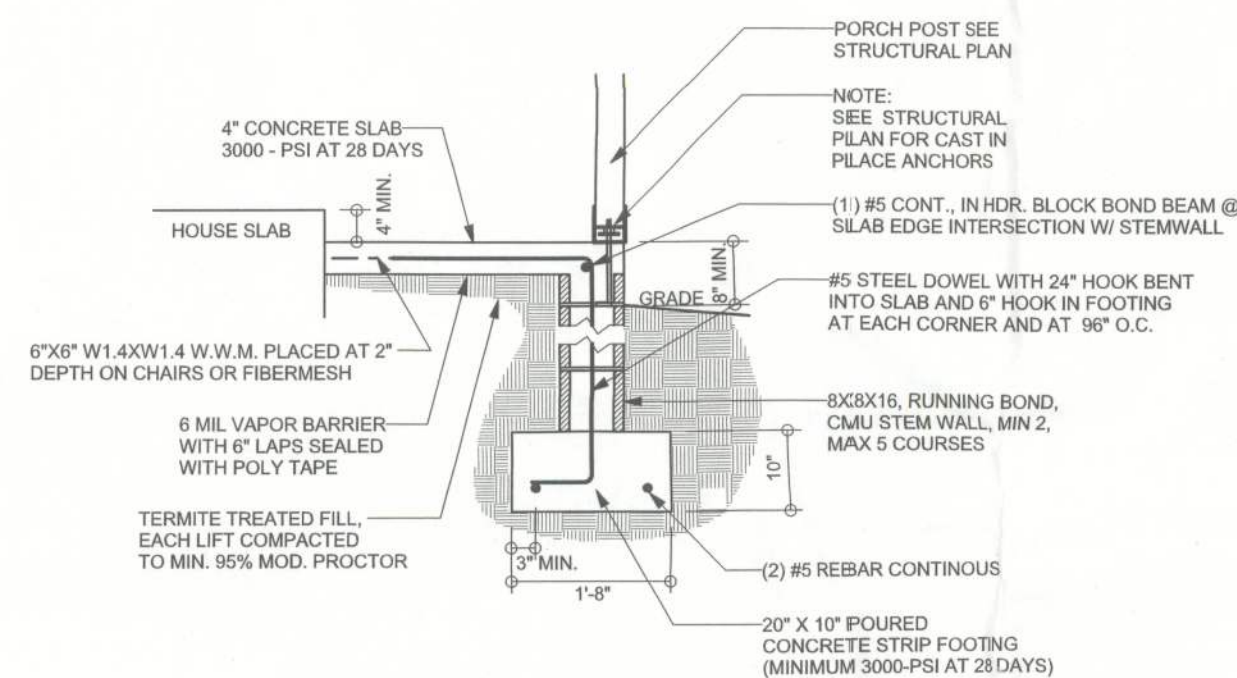
OF 3 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



F1 S-2
STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

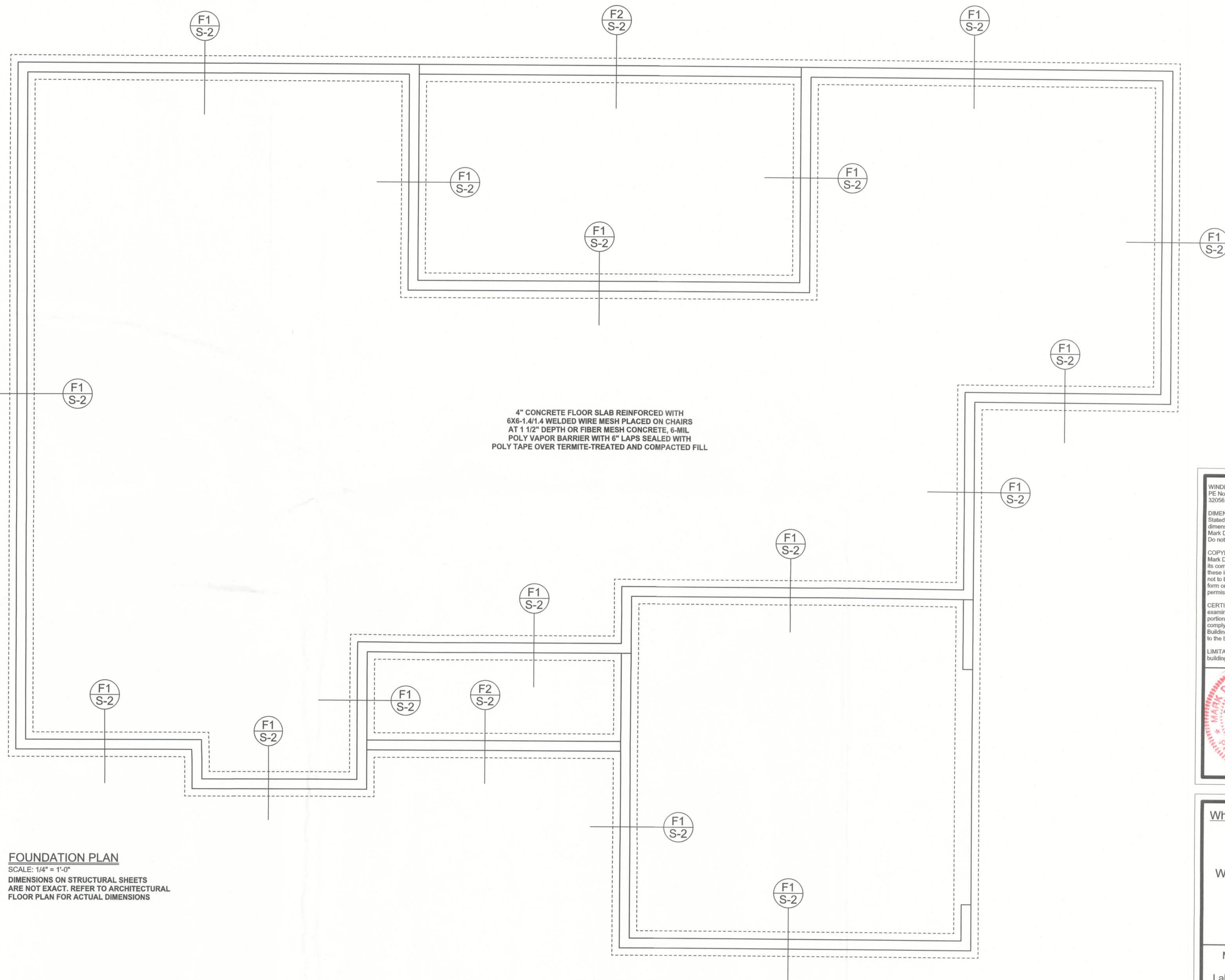


F2 S-2
STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall, 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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form or manner without first the express written
permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section R301.2.1, 2010 Florida
Building Code Residential
to the best of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.



Whiddon Construction

Whiddon Residence

ADDRESS:
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
Friday, October 25, 2013

DRAWN BY: STRUCTURAL BY:

FINALS DATE:
19Oct13

JOB NUMBER:
1309026

DRAWING NUMBER
S-2
OF 3 SHEETS