

58765

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2042308 - MIKE ROBERTS - SPEC HSE

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Mike Roberts Project Name: Spec House Model: Custom

Lot/Block: N/A Subdivision: N/A

Address: 508 SW Stewart Loop, N/A

City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:

Address:

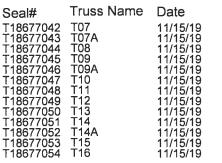
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2

Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 37.0 psf Floor Load: N/A psf

This package includes 35 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.





Mevised Set.



The truss drawing(s) referenced above have been prepared by MiTek IIS under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

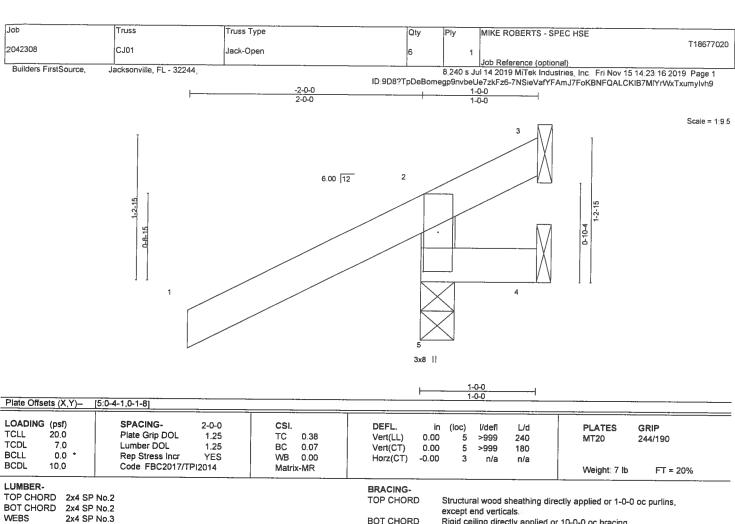
My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019



**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (lb/size) 5=295/0-3-8, 3=-84/Mechanical, 4=-32/Mechanical

Max Horz 5=65(LC 12)

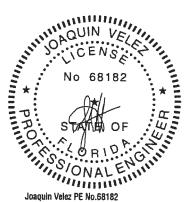
Max Uplift 5=-161(LC 12), 3=-84(LC 1), 4=-32(LC 1) Max Grav 5=295(LC 1), 3=46(LC 16), 4=18(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-5=-245/344

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 5=161.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

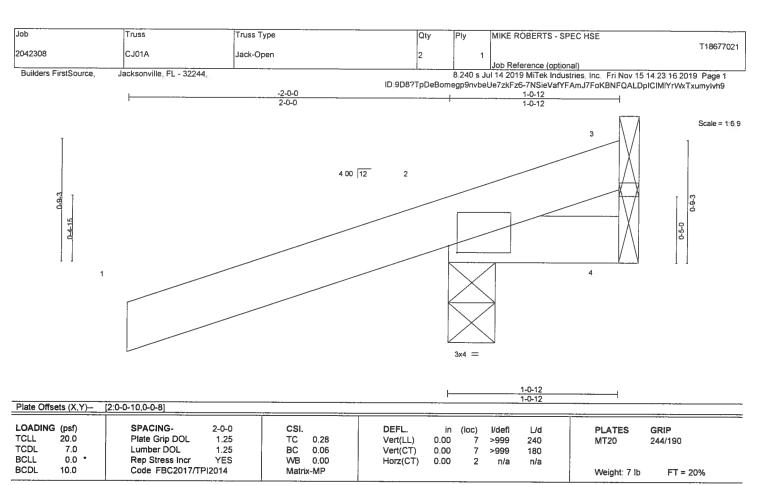


Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design fracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Austral Order of the Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314





**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

x4 SP No.2

(lb/size) 3=-25/Mechanical, 2=250/0-3-8, 4=-39/Mechanical Max Horz 2=60(LC 8)

Max Uplift 3=-25(LC 1), 2=-255(LC 8), 4=-39(LC 1) Max Grav 3=34(LC 8), 2=-250(LC 1), 4=48(LC 8)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

### NOTES- (7

- 1) Wind: ASCÉ 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=255.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 1-0-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019



Job Truss Truss Type Qty MIKE ROBERTS - SPEC HSE Ply T18677022 2042308 CJ02 Jack-Open Job Reference (optional)
8.240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14 23 17 2019 Page 1 Builders FirstSource. Jacksonville, FL - 32244 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-bZ04πaHJZldxHq\_uuuUzOttMZiXx5Cn\_laDVQDylvh8 Scale = 1:14.6 6.00 12 1-10-4 2 0-8-15 3x8 II Plate Offsets (X,Y)- [5:0-4-1,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL (loc) I/defl Ľ∕d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.41 Vert(LL) 0.01 4-5 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL вс 1 25 0.10 Vert(CT) 0.01 4-5 >999 180 **BCLL** 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 n/a n/a BCDL Code FBC2017/TPI2014 10.0 Matrix-MR Weight: 13 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, **BOT CHORD** 2x4 SP No.2 except end verticals. **WEBS** 2x4 SP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. (lb/size) 5=264/0-3-8, 3=44/Mechanical, 4=15/Mechanical

REACTIONS.

Max Horz 5=104(LC 12)

Max Uplift 5=-118(LC 12), 3=-53(LC 12), 4=-26(LC 9)

Max Grav 5=264(LC 1), 3=44(LC 1), 4=48(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-223/266

### NOTES-(7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone, end vertical left exposed, porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (|t=|b|) 5=118.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

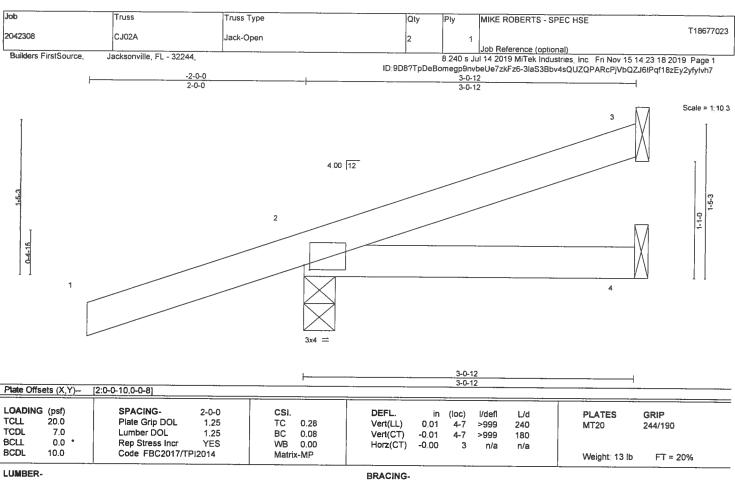


Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev., 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

(lb/size) 3=53/Mechanical, 2=255/0-3-8, 4=22/Mechanical

Max Horz 2=92(LC 8)

Max Uplift 3=-43(LC 12), 2=-230(LC 8), 4=-23(LC 9) Max Grav 3=53(LC 1), 2=255(LC 1), 4=48(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=230.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 3-0-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent uchilapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/THY Quality Criteria, DSB-89 and BCSI Building Component safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



6904 Parke East Blvd. Tampa, FL 36610

Job Truss Truss Type Qty MIKE ROBERTS - SPEC HSE T18677024 2042308 CJ03 Jack-Open Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14.23 18 2019 Page 1 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-3laS3Bbv4sQUZQPARcPjVbQXm6pFqf18zEy2yfylvh7 -2-0-0 2-0-0 Scale = 1:20.1 6.00 12 2-10-4 2 0-8-15 3x8 Plate Offsets (X,Y)-[5:0-4-1,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) I/defl 1 /d **PLATES** GRIP TCLL 20,0 Plate Grip DOL 1.25 TC 0.44 Vert(LL) 0.07 >768 4-5 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.35 0.07 >855 180 Vert(CT) 4-5 BCLL 0.0 \* Rep Stress Incr YES WB 0.00 -0.03Horz(CT) 3 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-AS Weight: 20 lb FT = 20%LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied, except end verticals. **BOT CHORD** 2x4 SP No.2 **BOT CHORD** Rigid ceiling directly applied. **WEBS** 2x4 SP No.3

REACTIONS.

(lb/size) 5=319/0-3-8, 3=106/Mechanical, 4=45/Mechanical

Max Horz 5=161(LC 12)

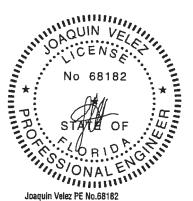
Max Uplift 5=-128(LC 12), 3=-108(LC 12), 4=-45(LC 9) Max Grav 5=319(LC 1), 3=106(LC 1), 4=86(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-5=-269/295

### NOTES.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (ft=lb) 5=128, 3=108,
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

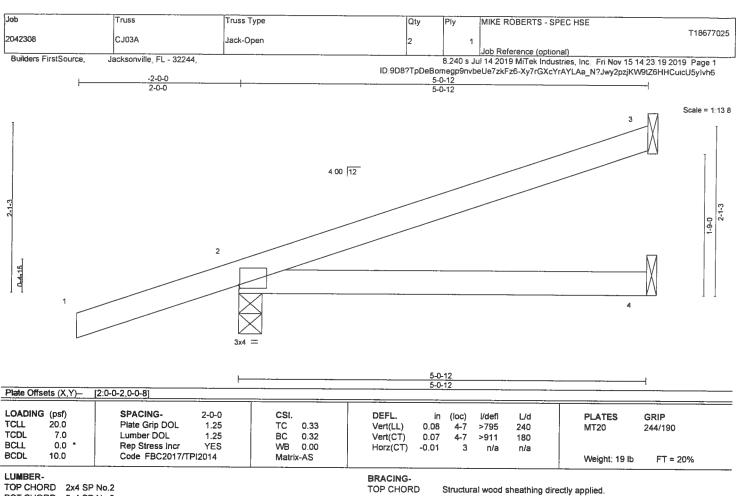


Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. 





**BOT CHORD** 

Rigid ceiling directly applied.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

(lb/size) 3=111/Mechanical, 2=315/0-3-8, 4=52/Mechanical

Max Horz 2=124(LC 8)

Max Uplift 3=-92(LC 8), 2=-266(LC 8), 4=-43(LC 9) Max Grav 3=111(LC 1), 2=315(LC 1), 4=86(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

NOTES-

REACTIONS.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=266.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

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Job Truss Truss Type Qty Ply MIKE ROBERTS - SPEC HSE T18677026 2042308 CJ04 Jack-Open 1 Job Reference (optional) Builders FirstSource. Jacksonville, FL - 32244 8,240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14:23:20 2019 Page 1 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-?8hDUtdAcUgCokZZZ1SBa0VuKwZxlZXRRYR91Xylvh5 1-3-10 Scale = 1:10.3 6.00 12 2 1-4-12 0-B-15 3x8 11 1-3-10 Plate Offsets (X,Y)— [5:0-4-1,0-1-8] LOADING (psf) SPACING-2-0-0 CSI DEFI in (loc) I/defi L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC BC 0.38 Vert(LL) 0.00 5 >999 240 MT20 244/190

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3

7.0

0.0

10.0

BRACING-

Vert(CT)

Horz(CT)

0.00

-0.00

TOP CHORD

Structural wood sheathing directly applied or 1-3-10 oc purlins,

Weight 8 lb

FT = 20%

except end verticals.

5 >999

3 n/a

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

180

n/a

REACTIONS. (lb/size) 5=267/0-3-8, 3=-45/Mechanical, 4=-22/Mechanical

Max Horz 5=72(LC 12)

Max Uplift 5=-140(LC 12), 3=-45(LC 1), 4=-22(LC 19) Max Grav 5=267(LC 1), 3=26(LC 8), 4=19(LC 16)

Lumber DOL

Rep Stress Incr

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-5=-222/301

### NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone, porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

0.08

0.00

WB

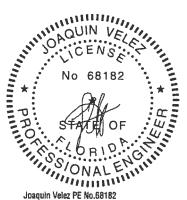
Matrix-MR

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 5=140.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

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| D:9D8?TpDeBomegp9nvbeUe7zkFz6-?8hDUtdAcUgCokZZZ -2-0-0 2-7-10 2-0-0 2-7-10

Scale = 1 13.7

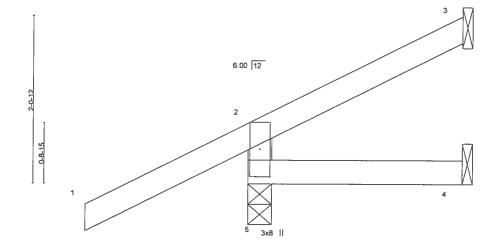


Plate Offsets (X,Y)-	[5:0-4-1,0-1-8]			7-10			<b>⊣</b> 		
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.25           Lumber DOL         1.25           Rep Stress Incr         YES           Code FBC2017/TPI2014	CSI. TC 0.40 BC 0.08 WB 0.00 Matrix-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.00 -0.00	(loc) 4-5 4-5 3	l/defi >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 12 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-7-10 oc purlins,

except end verticals.

2-7-10

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=257/0-3-8, 3=30/Mechanical, 4=8/Mechanical

Max Horz 5=104(LC 12)

Max Uplift 5=-118(LC 12), 3=-42(LC 12), 4=-21(LC 9)
Max Grav 5=257(LC 1), 3=30(LC 1), 4=41(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-5=-216/264

### NOTES- (7)

- 1) Wind: ASCÉ 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 5=118.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



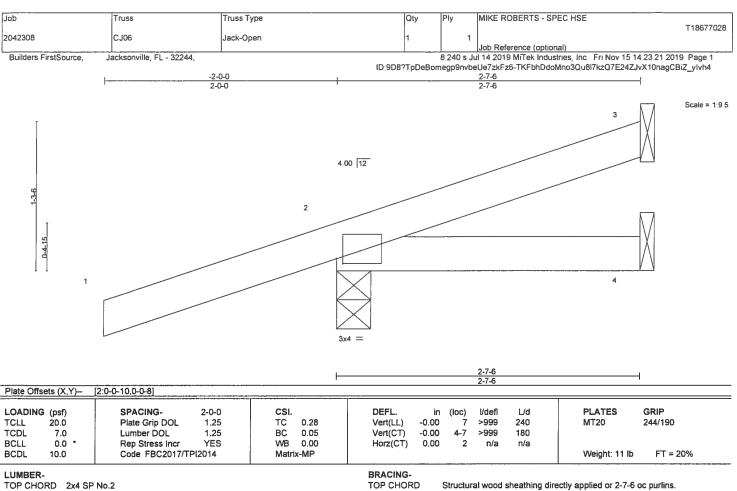
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD 2x4 SP No.2 **BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

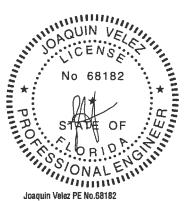
REACTIONS. (lb/size) 3=39/Mechanical, 2=245/0-3-8, 4=13/Mechanical

Max Horz 2=85(LC 8)

Max Uplift 3=-32(LC 12), 2=-225(LC 8), 4=-18(LC 9) Max Grav 3=39(LC 1); 2=245(LC 1), 4=39(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=225.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent uccliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply MIKE ROBERTS - SPEC HSE T18677029 2042308 EJ01 Jack-Partial 13 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8,240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14,23,22 2019 Page 1 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-xXpzuZeQ75ww12jygSUfgRb6Qj4AmS1juswG5Qylvh3 Scale = 1.24 5 6.00 12 3x4 / 3 0.8-15 5 3x8 II Plate Offsets (X,Y)-[2:0-2-12,0-0-6] SPACING-2-0-0 CSI. DEFL. (loc) I/defl L/d **PLATES** GRIP

LOADING (psf) TCLL 20.0 Plate Grip DOL 1.25 TC 0.85 Vert(LL) 0.32 5-8 >262 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.73 Vert(CT) 0.28 5-8 >299 180 BCLL 0.0 Rep Stress Incr YES **WB** 0.00 Horz(CT) -0.06 4 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-AS Weight: 28 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 4=165/Mechanical, 2=380/0-3-8, 5=76/Mechanical

Max Horz 2=144(LC 12)

Max Uplift 4=-100(LC 12), 2=-109(LC 9), 5=-61(LC 9) Max Grav 4=165(LC 1), 2=380(LC 1), 5=120(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-347/315

# NOTES- (8)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (jt=lb) 2=109.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



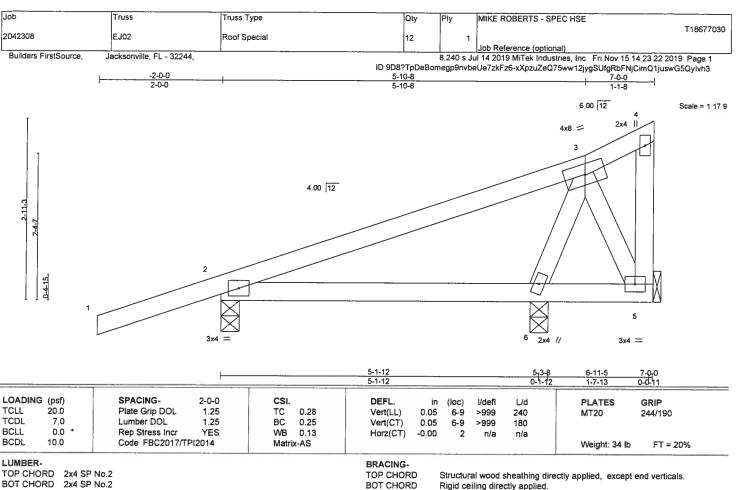
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute. 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.3

REACTIONS. (lb/size) 2=336/0-3-8, 5=118/Mechanical, 6=161/0-3-8

Max Horz 2=105(LC 8)

Max Uplift 2=-206(LC 8), 5=-70(LC 12), 6=-67(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-5=-241/433

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6 except (it=lb) 2=206.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

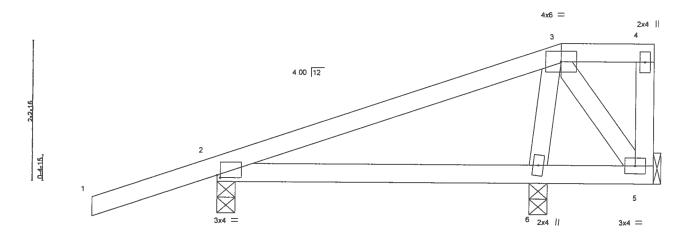
Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE, Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

Job Truss Truss Type Qty Ply MIKE ROBERTS - SPEC HSE T18677031 2042308 E.103 Half Hip Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244 8 240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14 23 23 2019 Page 1  $ID\ 9D8? TpDeBomegp9nvbeUe7zkFz6-QjNL6uf2uP2mfCH8E9?uCf7QA7YAVuit7WgpdsyIvh2$ -2-0-0 2-0-0 7-0-0

Scale = 1:17.8



						5-3-8					6-11-4 7	7-Q <sub>1</sub> 0
Plate Off	sets (X,Y) I	2:0-0-10.0-0-81				5-3-8			- 94			d-12
Tate Off	sets (A, T)=	2.0-0-10,0-0-0]		T								
LOADING FCLL FCDL BCLL	G (psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	BC 0	.27 .23 .10	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.05 0.04 -0.00	(loc) 6-9 6-9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code FBC2017/TI	PI2014	Matrix-A	s	774.2(01)	0.00	-	71704	1114	Weight: 33 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

**REACTIONS.** (lb/size) 2=329/0-3-8, 6=192/0-3-8, 5=94/Mechanical

Max Horz 2=96(LC 8)

Max Uplift 2=-204(LC 8), 6=-94(LC 8), 5=-39(LC 9) Max Grav 2=329(LC 1), 6=195(LC 23), 5=94(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEB\$ 3-5=-182/338

**NOTES-** (10)

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 5 except (jt=lb) 2=204.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss MIKE ROBERTS - SPEC HSE Truss Type Qty Ply T18677032 2042308 HJ01 Diagonal Hip Girder 3 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8 240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14 23 24 2019 Page 1 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-uvxkJEggfiAdHLsKotW7lsgSKXosElB0MAPMAJylvh1 4-6-0 4-6-0 9-10-13 Scale = 1.25.2 4.24 12 3x4 = 3x4 = 3 D-8-11 15 8 2x4 || 4x8 -11 3x4 = 4-6-0 9-10-13 Plate Offsets (X,Y)-[2:0-3-0,0-0-7] LOADING (psf) SPACING-2-0-0 CSI. DEFL. I/defl **PLATES** GRIP (loc) TCLL 20.0 Plate Grip DOL 1.25 TC 0.89 Vert(LL) 0.13 7-8 240 244/190 >942 **MT20** TCDL 7.0 Lumber DOL 1.25 вс 0.59 -0.13 >915 Vert(CT) 7-8 180 **BCLL** 0.0 Rep Stress Incr NO WB 0.28 -0.02 Horz(CT) 5 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Weight: 48 lb FT = 20% Matrix-MS LUMBER-BRACING-

TOP CHORD

**BOT CHORD** 

2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 5=154/Mechanical, 2=437/0-4-15, 6=227/Mechanical

Max Horz 2=234(LC 22)

Max Uplift 5=-152(LC 4), 2=-417(LC 4), 6=-243(LC 5) Max Grav 5=154(LC 1), 2=437(LC 1), 6=252(LC 3)

FORCES. (lb) - Max, Comp./Max, Ten, - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-483/471

**BOT CHORD** 2-8=-490/425, 7-8=-490/425

WEBS 4-7=-457/527

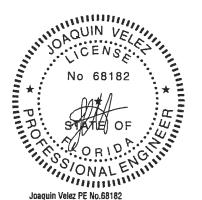
# NOTES-

- 1) Wind; ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat, II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone, porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=152, 2=417, 6=243
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 112 lb down and 160 lb up at 1-5-12, 112 lb down and 160 lb up at 1-5-12, 23 lb down and 41 lb up at 4-3-11, 23 lb down and 41 lb up at 4-3-11, and 49 lb down and 106 lb up at 7-1-10, and 49 lb down and 106 lb up at 7-1-10 on top chord, and 42 lb down and 61 lb up at 1-5-12, 42 lb down and 61 lb up at 1-5-12, 50 lb down and 32 lb up at 4-3-11, 50 lb down and 32 lb up at 4-3-11, and 38 lb down and 58 lb up at 7-1-10, and 38 lb down and 58 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-5=-54, 6-9=-20

Continued on page 2



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 7-9-15 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

November 15,2019

📤 WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing individual telep to revent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see. ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
2042308	HJ01	Diagonal Hip Girder	3	1	T18677032
Builden CirclCourse	Jack III- Et - 00044				Job Reference (optional)

Builders FirstSource,

Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Fr. Nov 15 14 23 24 2019 Page 2 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-uvxkJEggfiAdHLsKotW7lsgSKXosEIB0MAPMAJylvh1

LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 8=12(F=6, B=6) 3=82(F=41, B=41) 13=-59(F=-30, B=-30) 14=62(F=31, B=31) 15=-34(F=-17, B=-17)



Job	Truss	Truss Type		Qty	Ply	MIKE ROBERTS - SPEC HSE	
				1			T18677033
2042308	HJ02	Jack-Closed Girder		1	1		
						Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,				8 240 s Ju	ul 14 2019 MiTek Industries, Inc. Fri I	Nov 15 14 23 25 2019 Page 1
				ID:9D8?TpDeBon	egp9nvbe	Ue7zkFz6-M6V6XahlQ0lUuVRWMa1	MH4DiMxClzoSAaq9wilylvh0
	-2-9-15	I.	4-6-7			8-3-11	9-9-5
(	2-9-15	T	4-6-7	T		3-9-4	1-5-9

4.24 12 2x4 || 5 6 4x4 = 15 2.83 12 3x4 = 3 0-1-12 16 10 8 2x4 || 3x6 = 7

Dieta Office	4- (V )()	120050010	<u> </u>	4-6-7 4-6-7		7-2-9 2-8-2	9-9-5 2-6-12
Plate Offse	15 (X,Y)—	[2:0-0-5,0-0-13]					1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.55	Vert(LL) -0.05 10-13	>999 240	MT20 244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.38	Vert(CT) -0.05 10-13	>999 180	
BCLL	0.0 *	Rep Stress Incr	NO	WB 0.08	Horz(CT) 0.01 2	n/a n/a	
BCDL	10,0	Code FBC2017/TPI	2014	Matrix-MS			Weight: 49 lb FT = 20%

LUMBER-TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

3x4

except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 7-8.

REACTIONS.

**WEBS** 

(lb/size) 2=317/0-4-9, 9=586/0-4-15, 8=-38/Mechanical

Max Horz 2=163(LC 4)

Max Uplift 2=-207(LC 4), 9=-461(LC 5), 8=-76(LC 26) Max Grav 2=318(LC 35), 9=586(LC 1), 8=55(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-319/61

**WEBS** 3-9=-406/262, 4-9=-354/271

2x4 SP No.3

### NOTES-

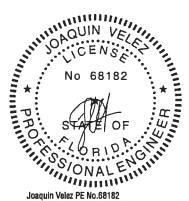
- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=207, 9=461,
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 96 lb down and 101 lb up at 1-6-1, 96 lb down and 101 lb up at 1-6-1, 26 lb down and 39 lb up at 4-4-0, 26 lb down and 39 lb up at 4-4-0, and 50 lb down and 96 lb up at 7-1-15, and 50 lb down and 96 lb up at 7-1-15 on top chord, and 76 lb down and 68 lb up at 1-6-1, 76 lb down and 68 lb up at 1-6-1, 52 lb down and 30 lb up at 4-4-0, 52 lb down and 30 lb up at 4-4-0, and 39 lb down and 61 lb up at 7-1-15, and 39 lb down and 61 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert. 1-4-54, 4-5-54, 5-6-14, 7-11-20

Concentrated Loads (lb)

Vert: 10=3(F=2, B=2) 9=48(F=24, B=24) 14=48(F=24, B=24) 15=-70(F=-35, B=-35) 16=66(F=33, B=33)



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

November 15,2019

Scale = 1:23 4

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 10/03/2015 BEFORE USE, Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see 

ANSITPH Quality Criteria, DSB-89 and BCSI Building Componently information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

Job Truss Truss Type Qty MIKE ROBERTS - SPEC HSE T18677034 2042308 HJ03 Roof Special Girder Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244 8 240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14 23 26 2019 Page 1 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-ql2UkwhxBKQLWf0jvHYbqHlvdKbliG0JpUuTEBylvh? 5-0-4 5-0-4 Scale ≈ 1.16.3 2x4 || 3 3.33 12 10 11 4x6 2x4 3x8 MT20HS II 5-0-0 4-11-7 Plate Offsets (X,Y)-[2:0-0-0,0-1-6], [2:0-2-8,0-7-14] LOADING (psf) SPACING-2-0-0 CSL DEFL. I/defl **PLATES GRIP** TCLL 20.0 Plate Grip DOL 1.25 TC 0.39 Vert(LL) -0.03 4-7 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.14 Vert(CT) -0.03 >999 180 MT20HS 187/143 BCLL 0.0 Rep Stress Incr NO WB 0.00 Horz(CT) 0.01 2 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MP Weight: 32 lb FT = 20% LUMBER-**BRACING-**TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, **BOT CHORD** 2x4 SP No.2 except end verticals. WEBS 2x4 SP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. WEDGE Left: 2x4 SP No.3 REACTIONS. (lb/size) 4=94/0-3-11, 2=430/0-5-3 Max Horz 2=131(LC 4) Max Uplift 4=-106(LC 5), 2=-390(LC 4) Max Grav 4=114(LC 35), 2=430(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=106, 2=390.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 21 lb down and 21 lb up at 1-10-10, and 86 lb down and 110 lb up at 2-4-2, and 19 lb down and 38 lb up at 4-10-8 on top chord, and 57 lb down and 23 lb up at 1-10-10, and 44 lb down and 48 lb up at 2-4-2 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-3=-54, 4-5=-20

Concentrated Loads (lb)

Vert: 3=-0(B) 9=28(B) 10=4(F)

No 68182

No 68182

No 68182

Joaquin Veiz PE No.68182

Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE	
2042308	T01	Common	9	1		T18677035
					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244		-	8 240 s Ju	Il 14 2019 MiTek Industries, Inc. Fri Nov 1	5 14 23 27 2019 Page 1
			ID:9D8?TpDeBor	negp9nvbe	eUe7zkFz6-IUcsyGiZydYC8pbvT?4gNVI2x	kmbRhHT28e1mevlvh
-2-0-0	5-1-7	10-0-0	1	10-9	20-0-0	22-0-0
2-0-0	5-1-7	4-10-9	4-1	0-9	5-1-7	2-0-0

Scale = 1:40.1

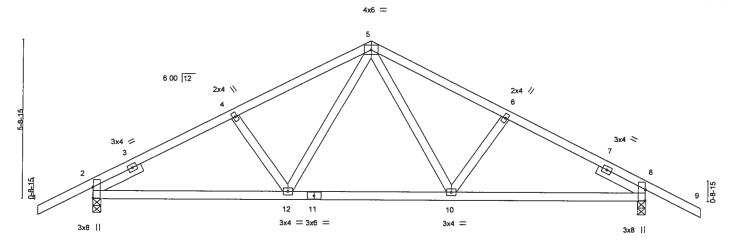


Plate Offsets (X,Y)	7-0-10 7-0-10 [2:0-4-12,0-0-2], [8:0-4-12,0-0-2]		12-11-6 5-10-13	20-0-0 7-0-10	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2017/TPI2014	CSI. TC 0.54 BC 0.78 WB 0.19 Matrix-AS	DEFL. in (loc) Vert(LL) 0.17 10-12 Vert(CT) -0.26 10-12 Horz(CT) 0.05 8	>999 240	PLATES GRIP MT20 244/190 Weight: 103 lb FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1025/0-3-8, 8=1025/0-3-8

Max Horz 2=126(LC 12)

Max Uplift 2=-425(LC 12), 8=-425(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1435/788, 4-5=-1320/779, 5-6=-1320/779, 6-8=-1435/788

BOT CHORD 2-12=-541/1223, 10-12=-321/922, 8-10=-559/1223

WEBS 5-10=-259/497, 5-12=-259/497

## NOTES- (9)

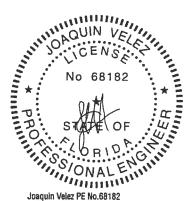
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. It; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=425, 8=425.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

# LOAD CASE(S) Standard

 Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=54, 5-9=54, 12-13=-20, 10-12=-80(F=-60), 10-17=-20



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	MIKE ROBERTS - SPEC HSE	
2042308	TOTAL						T18677036
2042308	T01G	Common Supported Gable		1	1		
L.,						Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,				8.240 s Ju	il 14 2019 MiTek Industries, Inc. Fri Nov 15 14	23.29 2019 Page 1
			ID:9D83	TpDeBor	negp9nvbe	Ue7zkFz6-EtkdMykpUFowN7llbQ6lSwNSgYe	SvdulVS77qWylvgy
-2-0-0	<del></del>	10-0-0				20-0-0	22-0-0
2-0-0	'	10-0-0	,			10-0-0	2-0-0

Scale = 1 39.7

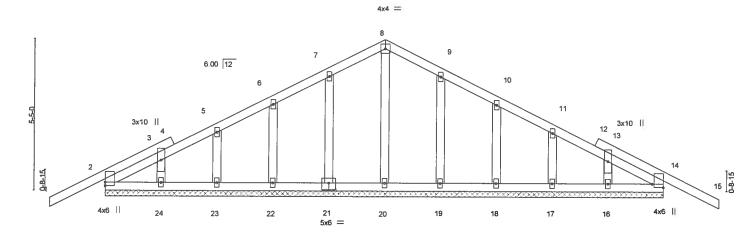


Plate Offsets (X,Y)-	14:Edge,0-4-13], [21:0-3-0,0-3-0]		20-0-0		
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.27 BC 0.05 WB 0.06 Matrix-S	DEFL. in (Vert(LL) -0.02 Vert(CT) -0.03 Horz(CT) 0.00	(loc) Vdefi L/d 15 n/r 120 15 n/r 120 14 n/a n/a	PLATES GRIP MT20 244/190 Weight: 113 lb FT = 20%

20-0-0

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 20-0-0

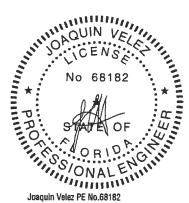
Max Horz 2=-119(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21, 22, 23, 24, 19, 18, 17, 16 Max Grav All reactions 250 lb or less at joint(s) 2, 14, 20, 21, 22, 23, 24, 19, 18, 17, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-(12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21, 22, 23, 24, 19, 18, 17, 16,
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

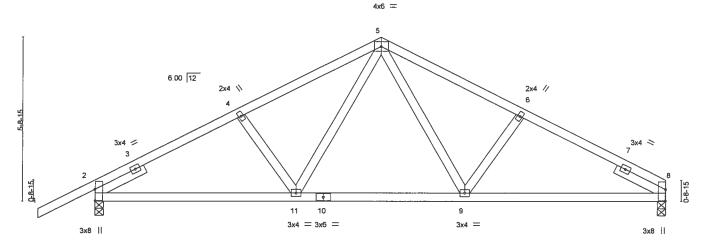
Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314



Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS	S - SPEC HSE	
1							T18677037
2042308	T02	Common	3	1			
					Job Reference (	optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s Ju	Il 14 2019 MiTek	Industries, Inc. Fri Nov 15 14 23 30 20	19 Page 1
			ID:9D8?TpD	Bomegp9nvb	eUe7zkFz6-i3l?a	lkRFYwn?GKU87dX_7wZ6yoPe1ovk6	shMyylvgx
2-0-0	, 5-1-7	, 10-0-0		14-10-	9	20-0-0	
2-0-0	5-1-7	4-10-9		4-10-9	}	5-1-7	_

Scale = 1 38 8



Distr Off	r1- (M ) M	10044000010044	7-0-10			5-10-13			1		7-0-10	1
Plate Off	fsets (X,Y)	[2:0-4-12,0-0-2], [8:0-4-12	2,0-0-2]	T		<del>-</del>					<del></del>	
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	Ļ/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.55	Vert(LL)	0.16	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.77	Vert(CT)	-0.26	9-11	>941	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.21	Horz(CT)	0.04	8	n/a	n/a		
BCDL	10.0	Code FBC2017/TI	PI2014	Matri	c-AS						Weight: 100 lb	FT = 20%

12-11-6

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 8=912/0-3-8, 2=1030/0-3-8

Max Horz 2=89(LC 16)

Max Uplift 8=200(LC 13), 2=240(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1445/802, 4-5=-1330/794, 5-6=-1346/806, 6-8=-1464/817

BOT CHORD 2-11=-618/1232, 9-11=-382/932, 8-9=-636/1256 WEBS 5-9=-278/523, 5-11=-255/496

## NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4,2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

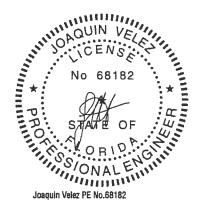
7-0-10

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=200, 2=240.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

### LOAD CASE(S) Standard

 Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-8=-54, 11-16=-20, 9-11=-80(F=-60), 9-12=-20



20-0-0

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

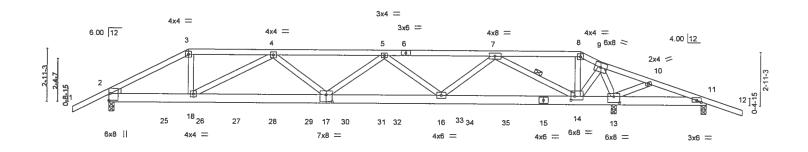
November 15,2019

WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 10/03/2015 BEFORE USE.

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	Job	Truss	Truss Type	Qty Ply	MIKE ROBERTS - SPEC HSE	
	2042308	тоз	Hip Girder	1 1		T18677038
1					Job Reference (optional)	
	Builders FirstSource, Ja	acksonville, FL - 32244		8,240 s J	Jul 14 2019 MiTek Industries, Inc. Fri Nov	15 14 23 31 2019 Page 1
		_		ID 9D8?TpDeBomegp9nvbe	eUe7zkFz6-BFsNnel30s2ecQvgir8mXLSe	3L60NKN2zmcEuPvlvaw
		1-8 9-0-14	15-2-4	21-3-10	26-0-0 27-1-8, 29-9-6	33-0-0 , 35-0-0 ,
	2-0-0 4-4	1-8 4-8-6	6-1-6	6-1-6	4-8-6 1-1-8 2-7-14	3-2-10 2-0-0



	4-4-8	12-0-4	18-	4-4	26	-0-0	27-10-4	33-0-0	9
	4-4-8	7-7-12	6-4	1-0	7-7	7-12	1-10-4		
late Offsets (X,Y)-	[2:0-0-4,0-0-8], [2:0-0-8,0-	5-10], [2:Edge,0	-0-15], [11:0-3-0,0-1-5],	[13:0-4-0,0-4-0].	[14:0-2-8,0-2-8]	. [17:0-4-0	0-4-8]		
OADING (psf) CLL 20.0 CDL 7.0 CLL 0.0 CDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TP	2-0-0 1.25 1.25 NO	CSI. TC 0.85 BC 0.94 WB 0.89 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.26 17-18 -0.41 17-18 0.07 13	l/defi >999 >817 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 188 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x6 SP No.2 WEBS

2x4 SP No.3 WEDGE

BRACING-

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 2-6-15 oc purlins.

Rigid ceiling directly applied or 4-5-4 oc bracing. 7-14

1 Row at midpt

Left: 2x4 SP No.3

REACTIONS. (lb/size) 2=1553/0-3-8, 13=2949/0-3-8, 11=-630/0-3-8

Max Horz 2=-43(LC 6)

Max Uplift 2=-617(LC 5), 13=-1459(LC 5), 11=-814(LC 19) Max Grav 2=1553(LC 19), 13=2949(LC 1), 11=341(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2471/1111, 3-4=-2199/1025, 4-5=-3857/1853, 5-7=-3038/1495, 7-8=-124/425,

8-9=-144/473, 9-10=-1308/2819, 10-11=-1213/2615

**BOT CHORD** 2-18=-894/2146, 17-18=-1526/3426, 16-17=-1667/3656, 14-16=-871/1893,

13-14=-1662/836, 11-13=-2458/1181

3-18-438/945, 4-18-1438/718, 4-17-307/590, 5-17-125/257, 5-16-795/357,

7-16=-708/1517, 7-14=-2656/1243, 8-14=-355/130, 9-14=-1138/2333, 9-13=-2894/1419,

10-13=-305/159

### NOTES-(10)

WERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=617, 13=1459, 11=814.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 74 lb down and 55 lb up at 3-0-12, 98 lb down and 90 lb up at 5-0-12, 98 lb down and 90 lb up at 7-0-12, 98 lb down and 90 lb up at 9-0-12, 98 lb down and 90 lb up at 11-0-12, 98 lb down and 90 lb up at 13-0-12, 98 lb down and 90 lb up at 15-0-12, 98 lb down and 90 lb up at 15-11-4, 98 lb down and 90 lb up at 17-11-4, 98 lb down and 90 lb up at 19-11-4, 98 lb down and 90 lb up at 21-11-4, and 98 lb down and 90 lb up at 23-11-4, and 86 lb down and 194 lb up at 25-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

6904 Parke East Blvd. Tampa FL 33610

November 15,2019

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Joaquin Velez PE No.68182

MiTek USA, Inc. FL Cert 6634

No 68182

No 68182

No 68182

Joaquin Velez PE No.68182

6904 Parke East Blvd. Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
00 40000			l.		T18677038
2042308	T03	Hip Girder	13	1	
					Job Reference (optional)

Builders FirstSource, Ja

Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14 23 31 2019 Page 2 ID:9D8?TpDeBomegp9nvbeUe7zkFz6-BFsNnei30s2ecQvgir8mXLSe3L60NKN2zmcEuPylvgw

LOAD CASE(S) Standard

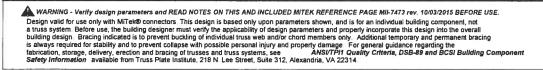
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=54, 3-8=54, 8-9=54, 9-12=54, 19-22=20

Concentrated Loads (lb)

Vert: 14=-60(F) 15=-98(F) 25=-74(F) 26=-98(F) 27=-98(F) 28=-98(F) 29=-98(F) 30=-98(F) 31=-98(F) 32=-98(F) 33=-98(F) 34=-98(F) 35=-98(F)





-	Job	Truss	Truss Type		Qty	Ply	MIKE ROBERTS - SPEC	HSE	
	2042308	T04	Hip		1	1			T18677039
Į	<u> </u>	1					Job Reference (optional)		
	Builders FirstSource, J	acksonville, FL - 32244,		-		8,240 s JL	il 14 2019 MiTek Industrie	s, Inc. Fri Nov 15 14 23 33	2019 Page 1
				ID 9D8?1				lsk23qGAEcmY4G9qOrJtL0	
	-2-0-0	6-4-8	12-5-3	17-11-5		24-0-0	, 27-1-8 ,	33-0-0	35-0-0
	2-0-0	6-4-8	6-0-11	5-6-2		6-0-11	3-1-8	5-10-8	2-0-0

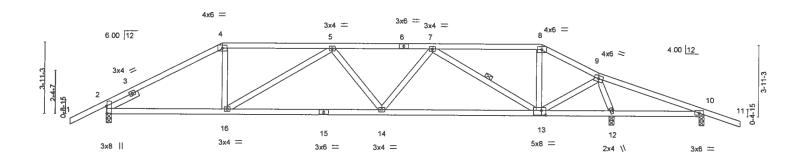


Plate Offse	ets (X.Y) I	6-4-8 6-4-8 [2:0-4-12,0-0-6], [13:0-2-	12 0-3-01	15-2-4 8-9-12	24-0-0 8-9-12	27-10-4 3-10-4	33-0-0 5-1-12	
LOADING TCLL TCDL BCLL BCDL		SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TI	2-0-0 1.25 1.25 YES	CSI. TC 0.49 BC 0.82 WB 0.57 Matrix-AS	Vert(LL) -0.14 14-16	l/defl L/d >999 240 >999 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 161 lb FT = 20%	

**BRACING-**

**WEBS** 

TOP CHORD BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

1 Row at midpt

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 WEBS

SLIDER Left 2x4 SP No.3 1-11-8

(lb/size) 2=1087/0-3-8, 12=1573/0-3-8, 10=-2/0-3-8

Max Horz 2=-57(LC 10)

Max Uplift 2=-220(LC 9), 12=-373(LC 8), 10=-199(LC 23) Max Grav 2=1087(LC 1), 12=1573(LC 1), 10=6(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1530/769, 4-5=-1320/751, 5-7=-1734/928, 7-8=-612/424, 8-9=-725/438,

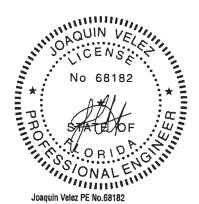
**BOT CHORD** 2-16=-504/1304, 14-16=-763/1771, 13-14=-672/1551, 12-13=-385/125, 10-12=-854/390 WEBS

4-16=-103/449, 5-16=-604/298, 7-14=-45/375, 7-13=-1118/556, 9-13=-364/1109,

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \*This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=220, 12=373, 10=199.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



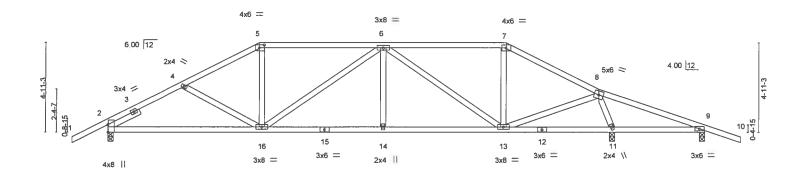
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

November 15,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design in the overall building design enrust verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC	HSE	
							T18677040
2042308	T05	Hip	1	1			
					Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL - 32244,			8 240 s Ju	il 14 2019 MiTek Industries	Inc. Fri Nov 15 14 23 3	4 2019 Page 1
			ID 9D8?TpDeBor	negp9nvb	eUe7zkFz6-bqXWQfnyJnR	DTudFNziT9z4GAZBHa	gfUfkquVkylvgt
-2-0-0 , 4	2-7 , 8-4-8	15-2-4	22-0-0		27-1-8	33-0-0	35-0-0
2-0-0 4	-2-7 4-2-1	6-9-12	6-9-12		5-1-8	5-10-8	2-0-0



		8-4-8		15-2-4		22-0	<b>-</b> 0	1	27-10-4	, 33-0-0	
	1	8-4-8	'	6-9-12		6-9-	12	1	5-10-4	, 5-1-12	1
Plate Offse	ets (X,Y)-	[2:0-5-0,Edge], [5:0-3-4,0	)-2-0]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.42	Vert(LL)	-0.09 14-16	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.65	Vert(CT)	-0.18 16-19	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.05 11	n/a	n/a	1	
BCDL	10.0	Code FBC2017/T	PI2014	Matrix	-AS					Weight: 171 lb	FT = 20%

**BRACING-**

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2

BOT CHORD 2x4 SP No.3 WEBS

Left 2x4 SP No.3 1-11-8 SLIDER

REACTIONS. (lb/size) 2=1109/0-3-8, 11=1430/0-3-8, 9=119/0-3-8

Max Horz 2=-70(LC 10)

8-9=-186/576

Max Uplift 2=-216(LC 12), 11=-296(LC 8), 9=-193(LC 9) Max Grav 2=1109(LC 1), 11=1430(LC 1), 9=143(LC 24)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

2-4=-1549/838, 4-5=-1436/768, 5-6=-1270/750, 6-7=-911/586, 7-8=-1077/589, TOP CHORD

2-16=-579/1324, 14-16=-584/1479, 13-14=-584/1479, 9-11=-499/279 **BOT CHORD** 

5-16=-96/386, 6-16=-348/147, 6-14=0/261, 6-13=-739/339, 7-13=-23/259, WEBS

8-13=-325/987, 8-11=-1367/737

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 11=296, 9=193.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Practing individual et by revern buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent uchilapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		101	75.	T		
1000	riuss	Truss Type		Qty	Ply	MIKE ROBERTS - SPEC	CHSE	
2042308	T06	Hip		1	1			T18677041
						Job Reference (optional)	)	
Builders FirstSou	urce, Jacksonville, FL - 32	244,		-	8.240 s Ju	ul 14 2019 MiTek Industrie	s Inc. Fri Nov 15 14 23	35 2019 Page 1
200	5.0.45			ID 9D8?TpDeBor	negp9nvbe	Ue7zkFz6-315ud?oa35Z	452CRxgDihBdQlzW0JF	3euOaS1Aylvgs
-2-0-0	5-0-15	10-4-8	15-2-4	20-0-0	ı	27-1-8	33-0-0	35-0-0
2-0-0	5-0-15	5-3-9	4-9-12	4-9-12	-	7-1-8	5-10-8	2-0-0

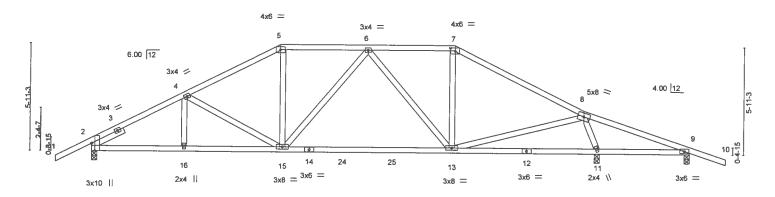


Plate Offsets (X,Y)	5-0-15 10-4-8 5-0-15 5-3-9 [2:0-5-0,Edge], [7:0-3-4,0-2-0]	20-0-0 9-7-8	27-10-4 7-10-4	33-0-0 5-1-12
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. DEFL. TC 0.49 Vert(LL) BC 0.74 Vert(CT) WB 0.40 Horz(CT) Matrix-AS	in (loc) I/defl L/d -0.20 13-15 >999 240 -0.38 13-15 >882 180 0.04 11 n/a n/a	PLATES GRIP MT20 244/190 Weight: 175 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1116/0-3-8, 11=1380/0-3-8, 9=161/0-3-8

Max Horz 2=-84(LC 10)

Max Uplift 2=-231(LC 12), 11=-249(LC 8), 9=-212(LC 9) Max Grav 2=1116(LC 1), 11=1380(LC 1), 9=185(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1581/837, 4-5=-1349/751, 5-6=-1173/737, 6-7=-987/649, 7-8=-1188/634,

8-9=-126/421

**BOT CHORD** 2-16=-581/1358, 15-16=-581/1358, 13-15=-429/1182, 9-11=-350/220 **WEBS** 

5-15=-104/350, 6-13=-384/180, 7-13=-31/283, 8-13=-257/903, 8-11=-1305/752

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=231, 11=249, 9=212.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

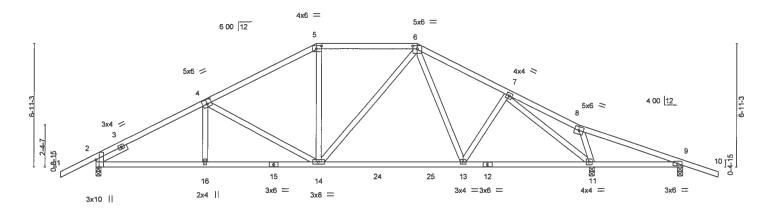
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters and properly incorporate this design into the overall atruss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property amanage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314



Job		Truss	Truss Type		Qty	Ply	MIKE ROBERTS - SPEC HS	SE	
									T18677042
2042308		T07	Hip		1	1			
L			<u> </u>				Job Reference (optional)		
Builders	FirstSource, Ja	acksonville, FL - 32244,				8.240 s Ju	I 14 2019 MiTek Industries, I	nc. Fri Nov 15 14 23 3	36 2019 Page 1
				ID 9D8?	TpDeBome	egp9nvbel	Je7zkFz6-XDfGqLpCqOhwjB	neVOkxEOAbZMu_2a	idn62J?acylvgr
	-2-0-0	6-1-9	12-4-8	18-0-0	2	3-0-13	, 27-1-8	33-0-0	35-0-0
	2-0-0	6-1-9	6-2-15	5-7-8		-0-14	4-0-11	5-10-8	2-0-0

Scale = 1:62:3



	1_	6-1-9	12-4	-8	20-7-5		2	7-10-4	33-0-0	
		6-1-9	6-2-	15	8-2-13		7	7-2-15	5-1-12	'
Plate Offse	ets (X,Y)-	[2:0-5-0,Edge], [4:0-3-0,0	<b>-3-0], [5:0-3-8,</b> 0	-2-0], [6:0-3-0,0-2-0]						
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.25	TC 0.49	Vert(LL)	-0.15 13-14	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC 0.63	Vert(CT)	-0.27 13-14	>999	180		
3CLL	0.0 *	Rep Stress Incr	YES	WB 0.95	Horz(CT)	0.05 11	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix-AS					Weight: 178 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No 2

BOT CHORD 2x4 SP No.3 WEBS

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1129/0-3-8, 11=1295/0-3-8, 9=234/0-3-8

Max Horz 2=-97(LC 10)

Max Uplift 2=-244(LC 12), 11=-245(LC 13), 9=-209(LC 9) Max Grav 2=1129(LC 1), 11=1295(LC 1), 9=254(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-1611/858, 4-5=-1263/728, 5-6=-1068/723, 6-7=-1157/708 TOP CHORD 2-16=-590/1381, 14-16=-591/1380, 13-14=-293/954, 11-13=-369/924 **BOT CHORD** 

WEBS 4-14=-374/315, 5-14=-65/300, 6-14=-68/277, 7-11=-1306/539, 8-11=-319/354

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=244, 11=245, 9=209.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

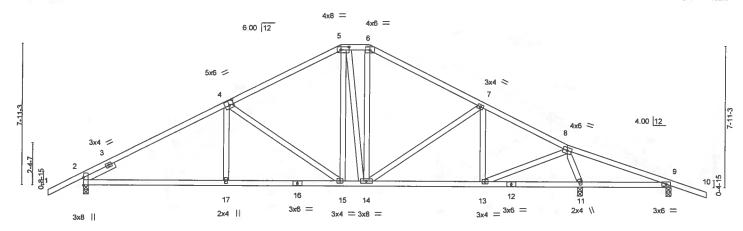
🗥 WARNING - Vertfy design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Pracing individual designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Pracing individual temporary and permanent bracing is always required for stability and to prevent ucklings of midrivalual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucklings with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314



6904 Parke East Blvd Tampa, FL 36610

Job Truss Truss Type Ply Qtv MIKE ROBERTS - SPEC HSE T18677043 2042308 TO7A Hip 1 Job Reference (optional) Jacksonville, FL - 32244, Builders FirstSource, 8 240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14 23 37 2019 Page 1 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-?PDe2hqqbipnKLMq35FAncin9mEZn62xLi3Y62ylvgq 2-0-0 8-0-0 14-4-8 27-1-8 33-0-0 35-0-0 8-0-0 6-4-8 4-9-0

Scale = 1:62.3



	E	8-0-0		14-4-8	16-0-0	22-4-8		10	27-10-4	33-0-0	
		8-0-0		6-4-8	1-7-8	6-4-8		0.0	5-5-12	5-1-12	
Plate Offse	ts (X,Y)-	[2:0-5-0,Edge], [4:0-3-0,0	-3-0], [5:0-5-8,	0-2-0]							
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	n (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.	43		7 15-17	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.	61	Vert(CT) -0.1	5 15-17	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB 0.	62	Horz(CT) 0.0	4 11	n/a	n/a		
BCDL	10.0	Code FBC2017/TF	PI2014	Matrix-AS	S	, ,				Weight: 191 lb	FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1116/0-3-8, 11=1384/0-3-8, 9=158/0-3-8

Max Horz 2=-111(LC 10)

Max Uplift 2=-254(LC 12), 11=-257(LC 13), 9=-230(LC 9) Max Grav 2=1116(LC 1), 11=1384(LC 1), 9=195(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1547/843, 4-5=-1082/695, 5-6=-882/683, 6-7=-1063/682, 7-8=-1089/654,

8-9=-99/470

BOT CHORD 2-17=-557/1313, 15-17=-558/1310, 14-15=-247/898, 13-14=-377/929, 9-11=-400/198 WEBS 4-17=0/286, 4-15=-526/400, 5-15=-194/353, 6-14=-111/285, 7-13=-285/193

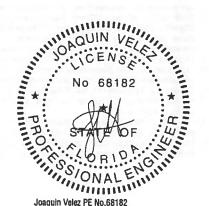
4-17=0/286, 4-15=-526/400, 5-15=-194/353, 6-14=-111/285, 7-13=-265/193, 8-13=-329/936, 8-11=-1323/721

0-13-329/930, 0-11-1323/721

## NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=254, 11=257, 9=230.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

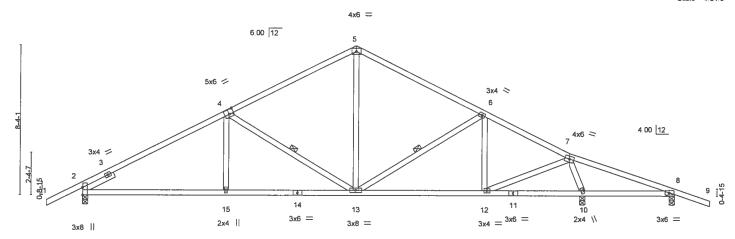


Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019



Job	Truss	Truss Type			Qty	Ply	MIKE ROBERTS - SPEC	HSE	
									T18677044
2042308	T08	Roof Special			2	1			
							Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL - 32244,					8.240 s Ju	Il 14 2019 MiTek Industries	, Inc. Fri Nov 15 14 2	3 38 2019 Page 1
				ID 9D8?	pDeBome	egp9nvbel	Je7zkFz6-Tcn0F1rSM0xey	Vx0cpmPJpFyjAaoWd	dH4aMo6eVvlvgp
2-0-0	8-0-0	1	15-2-4	1	22-4-8		, 27-1-8	33-0-0	, 35-0-0
2-0-0	8-0-0		7-2-4	1	7-2-4		4-9-0	5-10-8	2-0-0



	l l	8-0-0	1	15-2-4	22-4-8	, 27-10-4	, 33-0-0
		8-0-0	,	7-2-4	7-2-4	5-5-12	5-1-12
Plate Offsets	(X,Y)-	[2:0-5-0,Edge], [4:0-3-0,0	-3-4]				<del></del>
LOADING (	psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL 2	0.0	Plate Grip DOL	1.25	TC 0.44	Vert(LL) -0.07 13-15	>999 240	MT20 244/190
TÇDL .	7.0	Lumber DOL	1.25	BC 0.61	Vert(CT) -0.15 13-15	>999 180	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.37	Horz(CT) 0.04 10	n/a n/a	
BCDL 1	0.0	Code FBC2017/TI	PI2014	Matrix-AS			Weight: 172 lb FT = 20%

**BRACING-**

WEBS

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied.

4-13, 6-13

Rigid ceiling directly applied,

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 BOT CHORD WEBS

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1114/0-3-8, 10=1396/0-3-8, 8=148/0-3-8

Max Horz 2=-116(LC 10)
Max Uplift 2=-256(LC 12), 10=-264(LC 13), 8=-232(LC 9)
Max Grav 2=1114(LC 1), 10=1396(LC 1), 8=194(LC 24)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-4=-1544/848, 4-5=-1050/680, 5-6=-1049/674, 6-7=-1083/656, 7-8=-120/501

BOT CHORD 2-15=-562/1312, 13-15=-563/1309, 12-13=-382/925, 8-10=-430/218

4-15=0/286, 4-13=-564/415, 5-13=-281/526, 6-12=-269/201, 7-12=-351/958, WEBS

7-10=-1338/732

### NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
  6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=256, 10=264, 8=232.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

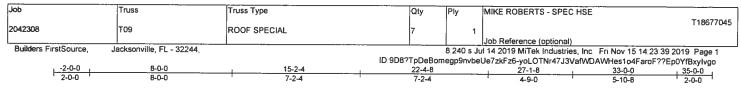


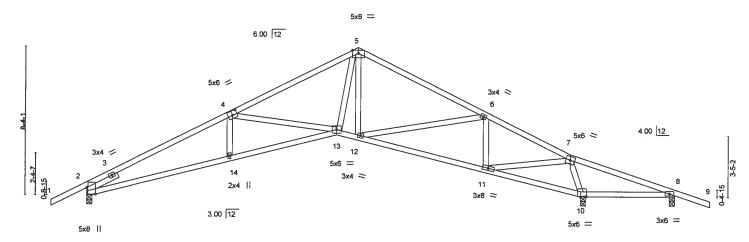
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

November 15,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent uccliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see. ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







	-	8-0-0		14-0-0	15-2-4		2-4-8		27-8-8	27-10-4	33-0-0	
Plate Off	sets (X,Y)- [	8-0-0 2:0-4-7,Edge], [4:0-3-0,0	3-01 [10:0-4-0	6-0-0	1-2-4	7	-2-4		5-4-0	0-1-12	5-1-12	
	Solo (XIII)	2.0 + 1,2agej, 14.0-0-0,0	70 0], [10.0 4 0	1,0-0-0]	T							
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defi	L/d	PLA	TES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.59	Vert(LL)	-0.20 13-14	>999	240	MT2	0	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.95	Vert(CT)	-0.38 13-14	>872	180			
3CLL	0.0 *	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.20 10	n/a	n/a			
BCDL	10.0	Code FBC2017/TI	PI2014	Matrix	c-AS					Wei	ht, 165 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\* 1-4: 2x4 SP M 31

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1049/0-3-8, 8=-138/0-3-8, 10=1747/0-3-8

Max Horz 2=-116(LC 10)

Max Uplift 2=-245(LC 12), 8=-265(LC 11), 10=-291(LC 13) Max Grav 2=1049(LC 1), 8=37(LC 12), 10=1747(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2386/1205, 4-5=-1709/846, 5-6=-1438/735, 6-7=-1006/612, 7-8=-480/1357

BOT CHORD 2-14=-904/2112, 13-14=-912/2121, 12-13=-297/1239, 11-12=-358/898, 10-11=-889/350,

8-10=-1238/540

4-13=-659/527, 5-13=-389/989, 6-12=-41/446, 6-11=-575/331, 7-11=-667/1722,

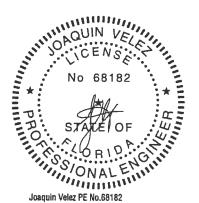
7-10=-1437/783

# NOTES- (9

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \*This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=245, 8=265, 10=291.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

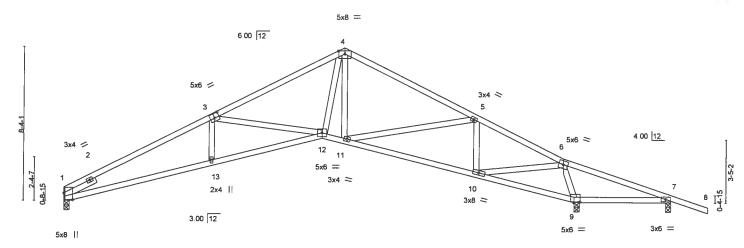
ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply MIKE ROBERTS - SPEC HSE T18677046 2042308 T09A ROOF SPECIAL Job Reference (optional) Jacksonville, FL - 32244 Builders FirstSource. 8 240 s Jul 14 2019 MiTek Industries, Inc Fri Nov 15 14 23 40 2019 Page 1 ID:9D8?TpDeBomegp9nvbeUe7zkFz6-Q\_vngisjudBMCp5PkEpt0EK9Y\_AQ\_R6N1fHDjNylvgn 22-4-8 7-2-4 35-0-0 27-1-8 33-0-0 4-9-0

Scale = 1:60.2



		8-0-0		14-0-0	15-2-4	22-4-8			27-8-8	27-10-4	33-0-0	1
	'	8-0-0	'	6-0-0	1-2-4	7-2-4			5-4-0	0-1-12	5-1-12	
Plate Offse	ets (X,Y)	[1:0-4-7,Edge], [3:0-3-0,0	-3-0], [9:0-4-0,0	0-0-8]								
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC BC WB Matri	1.00 0.92 0.66 x-AS	Vert(LL) -0.20	n (loc) 0 12-13 9 12-13 1 9	l/defl >999 >848 n/a	L/d 240 180 n/a	MT	ATES 20 ight: 161 lb	GRIP 244/190 FT = 20%

**BRACING-**

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 1=935/0-3-8, 7=-151/0-3-8, 9=1766/0-3-8

Max Horz 1=-124(LC 8)

Max Uplift 1=-206(LC 12), 7=-274(LC 11), 9=-292(LC 13) Max Grav 1=935(LC 1), 7=37(LC 12), 9=1766(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

TOP CHORD 1-3=-2414/1252, 3-4=-1715/870, 4-5=-1441/753, 5-6=-994/621, 6-7=-470/1395

BOT CHORD 1-13=-967/2143, 12-13=-973/2151, 11-12=-312/1241, 10-11=-373/887, 9-10=-923/355,

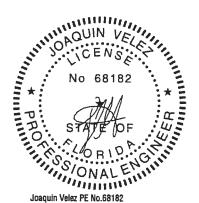
7-9=-1274/545

WEBS 3-12=-684/546, 4-12=-408/999, 5-11=-41/456, 5-10=-584/333, 6-10=-671/1744, 6-9=-1448/788

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=206, 7=274, 9=292.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



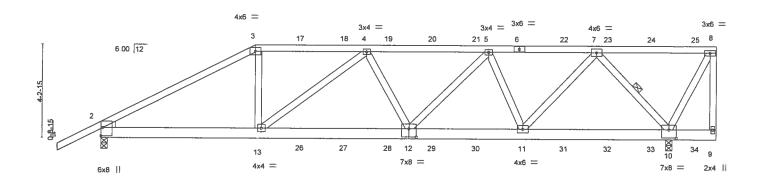
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019



Job		Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC	HSE	
204	2308	T10	Half Hip Girder	1	1			T18677047
						Job Reference (optional	)	
Βι	ilders FirstSource, Ja	acksonville, FL - 32244			8 240 s Ju	l 14 2019 MiTek Industri	es, Inc. Fri Nov 15 14 23 42 2	019 Page 1
				ID 9D8?TpDe	Bomegp9nvbe	Ue7zkFz6-MN0X5OuzQ	ER4R6EorerLTfQYVnrjSLzgV	zmJnGylvgi
	-2-0-0	7-0-0	12-0-13	17-7-4		22-6-4	, 28-0-0	
	2-0-0	7-0-0	5-0-13	5-6-7	1	4-11-0	5-5-12	

Scale = 1:50.5



	<u> </u>	7-0-0 7-0-0		7-	-0-3 0-3		19-2-2 5-2-0			26-0-0 6-9-14	28-0-0
Plate Offsets	s (X,Y)-	[2:0-0-4,0-0-8], [2:0-0-8,0	-5-10], [2:Edge	.0-0-15], [10	0-4-0,0-4-8	[ [12:0-4-0,0-4-8]					
TCDL BCLL	psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TF	2-0-0 1.25 1.25 NO Pl2014	CSI. TC BC WB Matrix	0.78 1.00 0.64 (-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (I 0.25 12 -0.29 12 0.07	 l/defi >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 171 II	GRIP 244/190 b FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\* 1-3: 2x4 SP M 31

1-3: 2x4 SP M ; BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3

WEDGE Left: 2x4 SP No.3

Lett: 2x4 SP No.3

REACTIONS. (lb/size) 2=1889/0-3-8, 10=2403/0-3-8

Max Horz 2=146(LC 23)

Max Uplift 2=-1043(LC 8), 10=-1422(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-3148/1947, 3-4=-2759/1778, 4-5=-3379/2076, 5-7=-2567/1544

BOT CHORD 2-13=-1757/2726, 12-13=-2123/3431, 11-12=-1794/2969, 10-11=-898/1504

WEBS 3-13=-546/900, 4-13=-854/478, 5-12=-414/601, 5-11=-1066/664, 7-11=-988/1627, 7-10=-2435/1451

### NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1043, 10=1422.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 189 lb down and 267 lb up at 7-0-0, 111 lb down and 106 lb up at 9-0-12, 111 lb down and 106 lb up at 11-0-12, 111 lb down and 106 lb up at 13-0-12, 111 lb down and 106 lb up at 15-0-12, 111 lb down and 106 lb up at 15-0-12, 111 lb down and 106 lb up at 19-0-11, 111 lb down and 106 lb up at 21-0-11, 111 lb down and 106 lb up at 21-0-11, 111 lb down and 106 lb up at 22-0-11, and 111 lb down and 106 lb up at 25-0-11, and 116 lb down and 361 lb up at 27-0-18 lb down and 81 lb up at 11-0-12, 80 lb down and 81 lb up at 11-0-12, 80 lb down and 81 lb up at 11-0-12, 80 lb down and 81 lb up at 11-0-11, 80 lb down and 81 lb up at 13-0-12, 80 lb down and 81 lb up at 23-0-11, and 80 lb down and 81 lb up at 25-0-11, and 84 lb down and 79 lb up at 27-0-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

# NO 68182 NO 68182 NO 68182 A COLINIA COLINI

Structural wood sheathing directly applied or 2-6-3 oc purlins,

7-10

Rigid ceiling directly applied or 4-10-7 oc bracing.

except end verticals.

1 Row at midpt

Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

# COARGA SE(SheStandard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
2042308	T10	Half Hip Girder	1		T18677047
2042308	110	Hall htp Gilder	'	ļ '	Job Reference (optional)

Builders FirstSource,

Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 15 14 23 42 2019 Page 2 ID 9D8?TpDeBomegp9nvbeUe7zkFz6-MN0X5OuzQER4R6EorerLTfQYVnrjSLzgVzmJnGylvgl

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 3-8=-54, 9-14=-20

Concentrated Loads (lb)

Vert: 3=-189(B) 6=-111(B) 13=-255(B) 11=-56(B) 17=-111(B) 18=-111(B) 19=-111(B) 20=-111(B) 21=-111(B) 22=-111(B) 23=-111(B) 24=-111(B) 25=-116(B) 26=-56(B) 27=-56(B) 28=-56(B) 29=-56(B) 30=-56(B) 31=-56(B) 32=-56(B) 33=-56(B) 34=-58(B)



Job	Truss	Truss Type		Qty	Ply	MIKE ROBERTS - SPEC	CHSE	-
2042308	Т11	Hip		1	1			T18677048
L						Job Reference (optional)	)	
Builders FirstSource,	Jacksonville, FL - 32	244,			8 240 s Ju	il 14 2019 MiTek Industrie	es, Inc. Fri Nov 15 14 23 43	2019 Page 1
			ID	9D8?TpDeBo	megp9nvbe	Ue7zkFz6-qZavlkubBYZ	x3Gp_PMMa0tyoCBFJBmD	pidWtKivlvak
-2-0-0	4-11-2	9-0-0	15-2-4	1	21-4-	-8 ,	28-0-0	
2-0-0	4-11-2	4-0-14	6-2-4		6-2-	4	6-7-8	<del></del>

Scale = 1:50.5

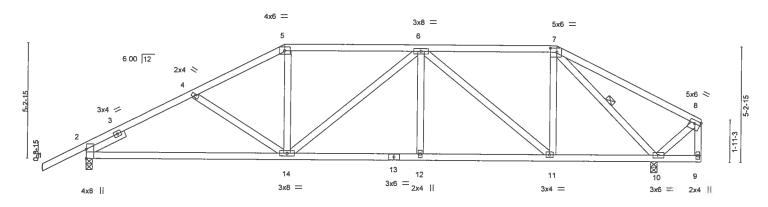


Plate Offsets (X,Y)-	9-0-0 9-0-0 [2:0-5-0,Edge], [7:0-3-8,0-2-4], [8:Edge,0	15-2-4 6-2-4 0-1-12]		21-4-8 6-2-4			Q-028-0-0 1-12 2-0-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.46 BC 0.72 WB 0.77 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl -0.12 14-17 >999 -0.23 14-17 >999 0.05 10 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 158 lb	GRIP 244/190 D FT = 20%

**BRACING-**

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

SLIDER Left 2x4 SP No.3 1-11-8

(lb/size) 2=1069/0-3-8, 10=1100/0-3-8 Max Horz 2=110(LC 12) REACTIONS.

Max Uplift 2=-215(LC 12), 10=-190(LC 8)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown. TOP CHORD 2-4=-1473/789, 4-5=-1318/709, 5-6=-1160/692, 6-7=-765/501 **BOT CHORD** 2-14=-674/1262, 12-14=-564/1250, 11-12=-564/1250, 10-11=-321/750

5-14=-86/358, 6-11=-670/308, 7-11=-156/533, 7-10=-1200/583 WEBS

NOTES-(9)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=215, 10=190.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

1 Row at midpt

Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer, Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and personnent bracking is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracking of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Sefety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Jop	Truss	Truss Type	•	Qty	Ply	MIKE ROBERTS - SPEC HSE	
		399					T18677049
2042308	T12	Hip		1	1	0.75	
						Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244				8.240 s Ju	1 14 2019 MiTek Industries, Inc. Fri	Nov 15 14 23 44 2019 Page 1
			ID 9D8	?TpDeBon	negp9nvbe	eUe7zkFz6-II8HW4vDyrhogQOAz3t	pZ4V_dbepwKVzyHFQs8ylvgi
-2-0-0	5-11-9	11-0-0	15-2-4	i i	19-4-8	24-3-3	28-0-0
2-0-0	5-11-9	5-0-7	4-2-4	1	4-2-4	4-10-11	3-8-13

Scale = 1:50.1

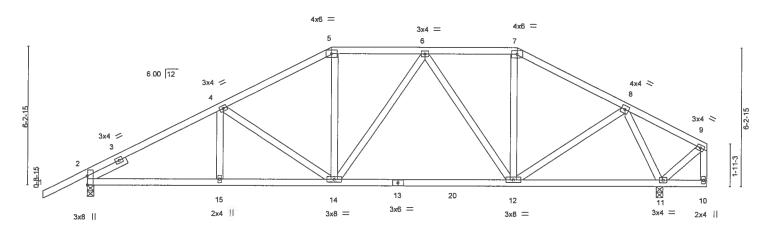


Plate Offsets (X,Y)	5-11-9 5-11-9 [2:0-4-12,0-0-2]	11-0-0	+	19-4- 8-4-8	-			25-10-4 6-5-12	26-0-028-0-0 0-1-12 2-0-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T/I	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.42 BC 0.57 WB 0.38 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.12 12-14 -0.23 12-14 0.04 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 164	GRIP 244/190 4 lb FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1066/0-3-8, 11=1103/0-3-8

Max Horz 2=123(LC 12)

Max Uplift 2=-227(LC 12), 11=-194(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1481/775, 4-5=-1206/680, 5-6=-1031/666, 6-7=-804/545, 7-8=-958/548

BOT CHORD 2-15=-657/1266, 14-15=-657/1266, 12-14=-432/994, 11-12=-208/413

WEBS 4-14=-295/265, 5-14=-101/307, 6-12=-385/206, 8-12=-130/471, 8-11=-1050/600

### NOTES- (9)

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat, II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=227, 11=194.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

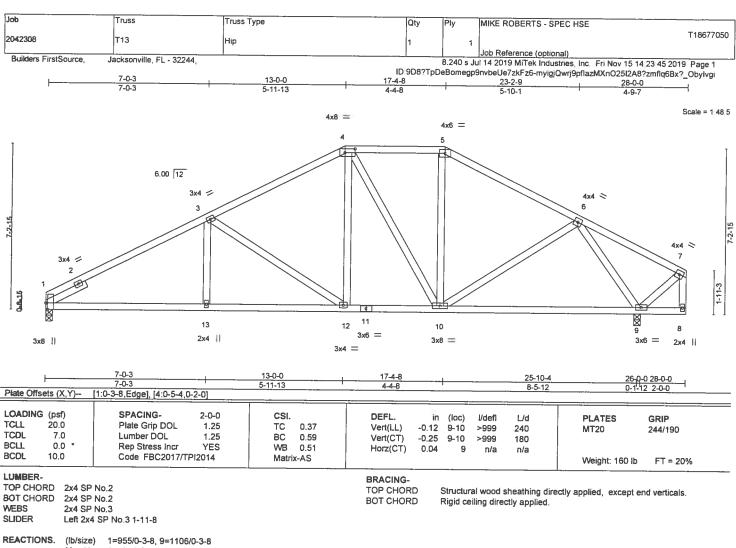
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and lis for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see 
ANSUPTH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.





SUIDER

Max Horz 1=108(LC 12)

Max Uplift 1=-199(LC 12), 9=-209(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

1-3=-1503/799, 3-4=-1083/653, 4-5=-811/592, 5-6=-980/590 **BOT CHORD** 

1-13-671/1286, 12-13-671/1286, 10-12-367/906, 9-10-302/583

3-13=0/258, 3-12=-468/365, 4-12=-174/346, 4-10=-260/112, 6-10=-38/310, WEBS

6-9=-1075/659

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb)
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



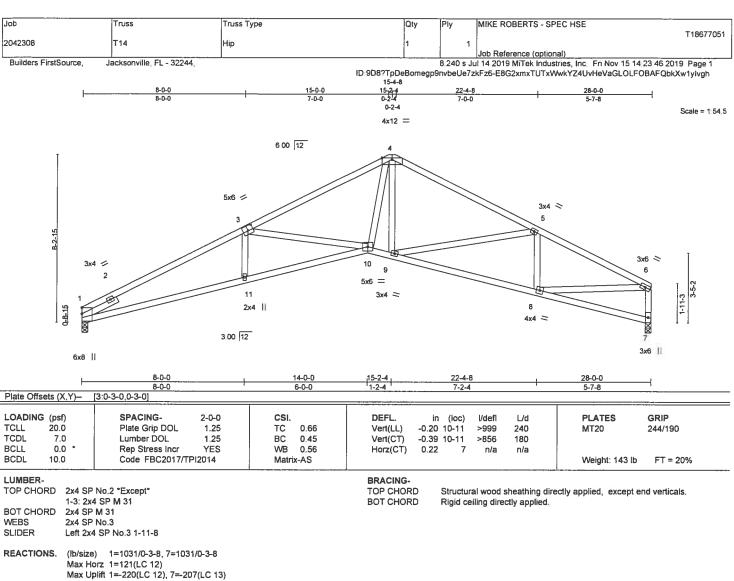
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 15.2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTeMe connectors. This design is based only upon parameters and properly discussed and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see 
ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

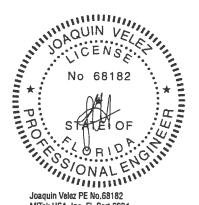
TOP CHORD 1-3=-2743/1464, 3-4=-2103/1120, 4-5=-1786/961, 5-6=-1782/949, 6-7=-993/568 **BOT CHORD** 1-11=-1277/2439, 10-11=-1286/2453, 9-10=-626/1563, 8-9=-807/1610

WEBS 3-10=-637/501, 4-10=-563/1150, 5-8=-344/283, 6-8=-733/1480, 4-9=-48/253

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord five load nonconcurrent with any other five loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=220, 7=207.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



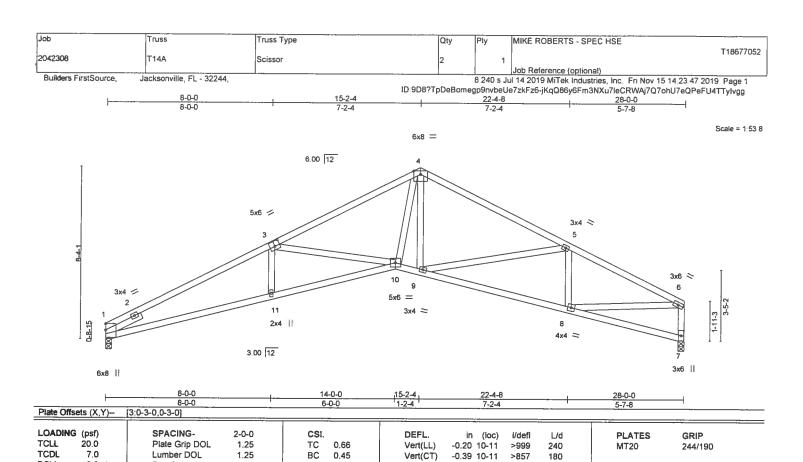
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

November 15,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTTek® connectors. This design is based only upon parameters and non-middle building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see 

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Sefety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312. Alexandra, VA 22314





Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.22

n/a

Rigid ceiling directly applied.

n/a

Structural wood sheathing directly applied, except end verticals.

LUMBER-

BCLL

BCOL

TOP CHORD 2x4 SP No.2 \*Except\* 1-3; 2x4 SP M 31

BOT CHORD 2x4 SP M 31 WEBS 2x4 SP No.3

0.0

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 1=1031/0-3-8, 7=1031/0-3-8

Max Horz 1=122(LC 12)

Max Uplift 1=-220(LC 12), 7=-207(LC 13)

Rep Stress Incr

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-2743/1464, 3-4=-2089/1113, 4-5=-1775/955, 5-6=-1782/949, 6-7=-993/568
BOT CHORD 1-11=-1277/2439, 10-11=-1287/2453, 9-10=-617/1548, 8-9=-808/1610
WEBS 3-10=-650/512, 4-10=-562/1149, 4-9=-52/258, 5-8=-344/283, 6-8=-733/1480

### NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

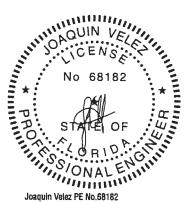
Matrix-AS

0.56

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=220, 7=207.
   8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum
- sheetrock be applied directly to the bottom chord.
  9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Weight: 143 lb

FT = 20%

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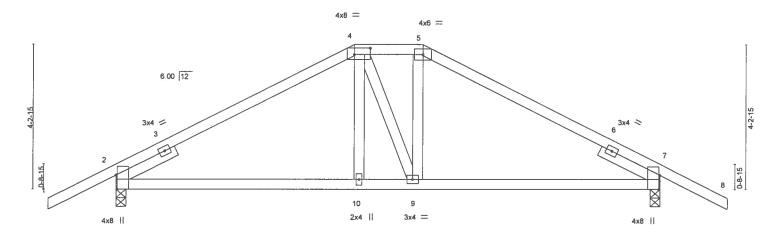
November 15,2019



6904 Parke East Blvd. Tampa, FL 36610

Job		Truss	Truss Type		Qty	Ply	MIKE ROBERTS - SPEC HSE	
			V3 V4					T18677053
204	2308	T15	Hip Girder		1	1	FE.	
L							Job Reference (optional)	
Bu	uilders FirstSource, .	lacksonville, FL - 32244;				8 240 s Ju	Il 14 2019 MiTek Industries, Inc., Fri Nov	15 14 23 48 2019 Page 1
				ID 9D	3?TpDeBo	megp9nvb	eUe7zkFz6-BXOoMSyk?4CD91ixCvyljwg	gZ1CxMsBzYtvDe?wylvgf
		7-0-0	1	, 9-0-0		-	16-0-0	18-0-0
	2-0-0	7-0-0	-	2-0-0			7-0-0	2-0-0

Scale = 1:32.6



			7-0-0 7-0-0			9-0-0				16-0-0 7-0-0		1
Plate Offse	ets (X,Y)-	[2:0-5-0,Edge], [4:0-5-8,0		Edge]		2-0-0				7-0-0		
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	тс	0.77	Vert(LL)	0.12	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.79	Vert(CT)	0.12	9-10	>999	180		
BCLL	0.0 *	Rep Stress incr	NO	WB	0.16	Horz(CT)	-0.06	7	n/a	n/a		
BCDL	10.0	Code FBC2017/TI	Pl2014	Matrix	-MS	, ,					Weight: 80 lb	FT = 20%

**BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

SLIDER Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=1144/0-3-8, 7=1144/0-3-8

Max Horz 2=-61(LC 6)

Max Uplift 2=-743(LC 5), 7=-743(LC 4)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-4-1598/1234, 4-5-1370/1131, 5-7-1599/1235 2-10=-1054/1357, 9-10=-1067/1369, 7-9=-1026/1359 BOT CHORD

4-10=-339/418, 5-9=-347/421

### NOTES-(10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (t=lb) 2=743, 7=743.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 236 lb down and 267 lb up at 7-0-0, and 236 lb down and 267 lb up at 9-0-0 on top chord, and 276 lb down and 361 lb up at 7-0-0, and 276 lb down and 361 lb up at 8-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-54, 5-8=-54, 11-15=-20

Concentrated Loads (lb)

Vert: 4-189(B) 5-189(B) 10-255(B) 9-255(B)



Structural wood sheathing directly applied or 3-4-0 oc purlins,

Rigid ceiling directly applied or 5-2-15 oc bracing.

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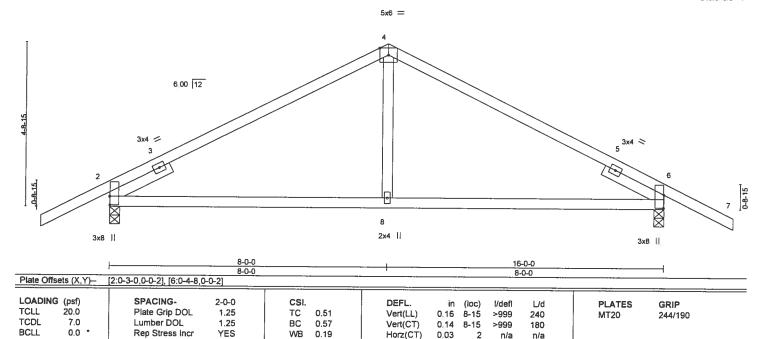
November 15,2019

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE	
2042308	T16	Common	2	1		T18677054
					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s Ju	Il 14 2019 MiTek Industries, Inc. Fri Nov 1	5 14 23 49 2019 Page 1
			ID:9D8?TpDeBom	egp9nvbel	Ue7zkFz6-fjxAZnzMmOK4nBH8mdT_G8Cj	pwcL0beji6ZzBXMylvge
-2-0-0		8-0-0	1		16-0-0	18-0-0
2-0-0		8-0-0	1		8-0-0	2-0-0

Scale 3/8"=1"



**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

10.0

SLIDER Left 2x4 SP No.3 1-11-8, Right 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=700/0-3-8, 6=700/0-3-8

Max Horz 2=68(LC 11) Max Uplift 2=-247(LC 9), 6=-247(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code FBC2017/TPI2014

TOP CHORD 2-4=-654/988, 4-6=-654/989 BOT CHORD 2-8=-711/585, 6-8=-711/585

WEBS 4-8=-499/330

### NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-AS

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=247, 6=247.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Weight: 69 lb

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

FT = 20%

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November 15,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



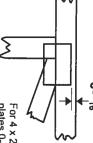
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## **Symbols**

# PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y and fully embed teeth. offsets are indicated Apply plates to both sides of truss Dimensions are in ft-in-sixteenths



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

required direction of stots in connector plates. This symbol indicates the

\*Plate location details available in MiTek 20/20 software or upon request.

## PLATE SIZE

4 × 4

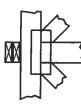
to slots. Second dimension is width measured perpendicular The first dimension is the plate the length parallel to slots.

# LATERAL BRACING LOCATION



output. Use T or I bracing by text in the bracing section of the if indicated Indicated by symbol shown and/or

### BEARING



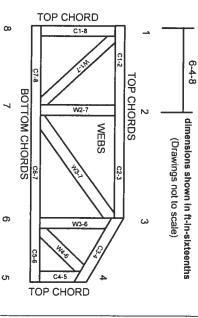
number where bearings occur.

Min size shown is for crushing only (supports) occur. Icons vary but reaction section indicates joint Indicates location where bearings

### ANSI/TPI1: Industry Standards:

National Design Specification for Metal Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling, **Building Component Safety Information,** Plate Connected Wood Truss Construction Design Standard for Bracing

# Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown, Trusses are designed for wind loads in the plane of the

established by others. section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For bracing should be considered may require bracing, or alternative Tor I wide truss spacing, individual lateral braces themselves
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

ω

- Provide copies of this truss design to the building all other interested parties. designer, erection supervisor, property owner and
- Cut members to bear tightly against each other.

G

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

### T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

1	Nailing Pattern	
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

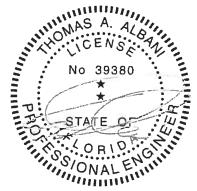
	Nails	
WEB	SPACING	
	T-BRACE	M
N. 7		
Nails	Section Detail	
	T-Brace	
	Veb	

Nails	
Web	I-Brace
Nails	

		Brace Size for One-Ply Truss				
	Specified Rows of La	Continuous ateral Bracing				
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

		ce Size -Ply Truss		
	Specified Continuous Rows of Lateral Bracing			
Web Size	1	2		
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace		
2x6	2x6 T-Brace	2x6 I-Brace		
2x8	2x8 T-Brace	2x8 I-Brace		

T-Brace / I-Brace must be same species and grade (or better) as web member.



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### SCAB-BRACE DETAIL

### MII-SCAB-BRACE

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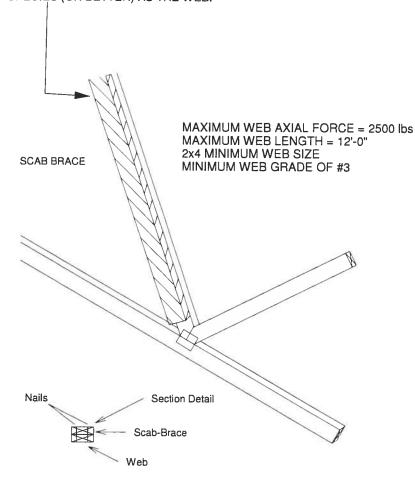
Page 1 of 1



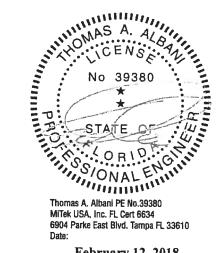
Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

\*\*\* THIS DETAIL IS NOT APLICABLE WHEN BRACING IS \*\*\* REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x\_ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.

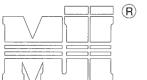


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### STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

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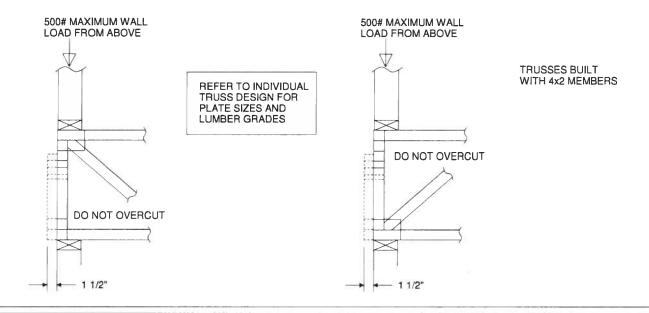
- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

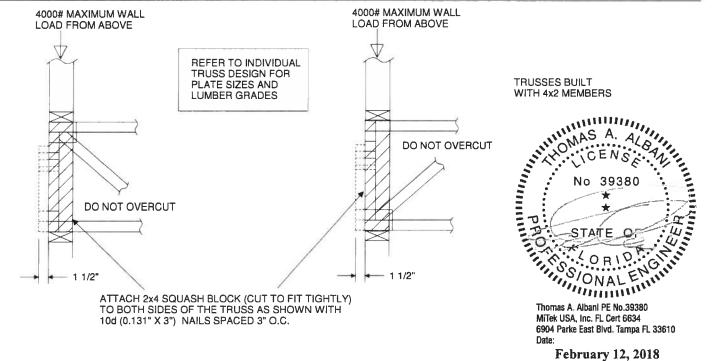
  3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE
- SUCH AS TO AVOID SPLITTING OF THE WOOD.

  4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

  5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X\_ORIENTATION ONLY.

  6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.



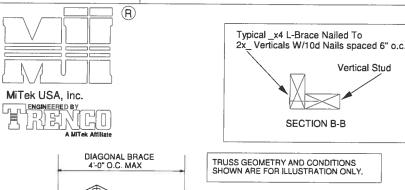


### Standard Gable End Detail

### MII-GE130-D-SP

Page 1 of 2

MiTek USA, Inc.



Vertical Stud DIAGONAL (4) - 16d Nails BRACE 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA \*\* 3x4 =В \* - Diagonal Bracing - L-Bracing Refer

TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS. (4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

Refer to Section A-A

to Section B-B

### NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT
BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB
OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES. 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

24" Max	M MILVIZION 77
1'-	3" (2) - 10d NAILS (2) - 10d NAILS
	Trusses @ 24" o.c
Diag. Brace	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d
at 1/3 points if needed	NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)
GONAL	1 1

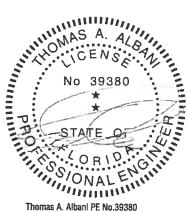
Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximu	n Stud Lei	ngth	
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH

**DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



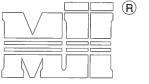
MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

### Standard Gable End Detail

### MII-GE130-SP

Page 1 of 2

MiTek USA, Inc.



MiTek USA, Inc. ENGINEERED BY 13/5

Typical \_x4 L-Brace Nailed To 2x\_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS 4'-0" O.C. MAX SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA \*\* 3x4 =

- L-Bracing Refer

to Section B-B

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d

Roof Sheathing

1'-3"

24" Max

NOTE

\* - Diagonal Bracing

Refer to Section A-A

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB

OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.

(REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIGNATED 16d ARE (0.131" X 3.5")

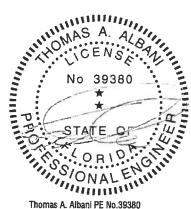
NAILS (2) - 10d NAILS
7russes @ 24" o.c.
2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	2 DIAGONAL BRACES AT 1/3 POINTS					
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6		
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1		
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



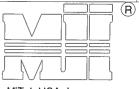
### Standard Gable End Detail

### MII-GE140-001

MiTek USA, Inc.

Page 1 of 2

O.C.



MiTek USA, Inc. ENGINEERED BY 深點

Typical x4 L-Brace Nailed To 2x\_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. 4'-0" O.C. MAX 12 Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA \*\* 3x4 =- Diagonal Bracing L-Bracing Refer to Section B-B

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK

Roof Sheathing-

NOTE:

Refer to Section A-A

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, GRADES: 1x4 SRB
OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
DIAPHRAM AT 4"-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

24" Max	
1'-3 _ Ma	1 (2) - 100° PI
	***
,	7russes @ 24" o.c
Diag. Brace	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d
at 1/3 points if needed	NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Size Spacing		1x4 2x4 L-Brace L-Brace		DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length							
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4			
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11			
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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### Standard Gable End Detail

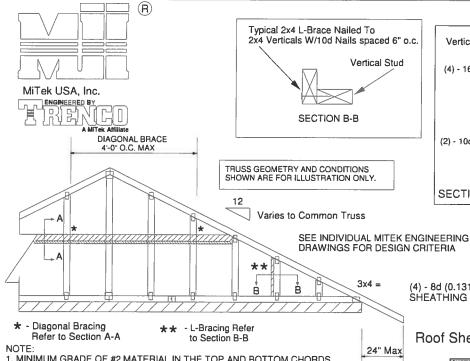
### MII-GE170-D-SP



Page 1 of 2

(2) - 10d NAILS

Trusses @ 24" o.c.



Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A 2X4 SP OR SPF No. 2

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD, SHEATHING TO 2x4 STD SPF BLOCK

- 10d

NAILS

Roof Sheathing

1'-0"

Max.

Diag. Brace

at 1/3 points

End Wall

if needed

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3

OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

06-01-13 BY SPIB/ALSC.

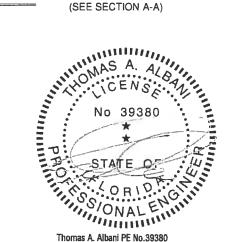
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH **DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

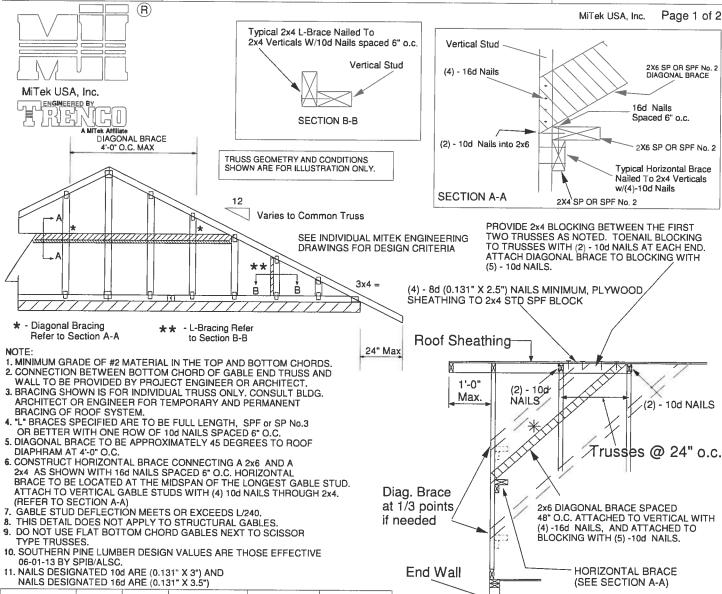


2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH

(4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

HORIZONTAL BRACE

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

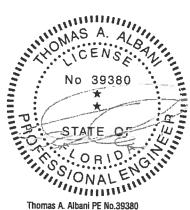


Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7			
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

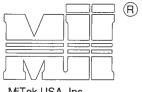
MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc. ENGINEERED BY 1:33

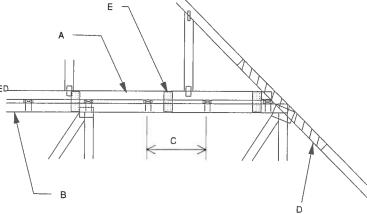
MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C

**ASCE 7-10** 

**DURATION OF LOAD INCREASE: 1.60** 

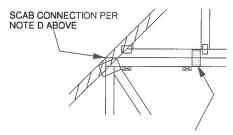
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
  SHALL BE CONNECTED TO EACH PURLIN
  WITH (2) (0.131" X 3.5") TOE-NAILED.
  B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
  C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
  UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
  CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
  D 2 X \_ X 4-0" SCAB, SIZE TO MATCH TOP CHORD OF
  PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
  ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
  SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
  IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
  DIRECTIONS AND:
  1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
- I. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft. E FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72° O.C. W/ (4) (0.131° X 1.5°) NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5° EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

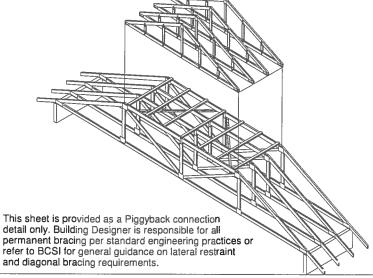


### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

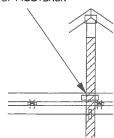
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48° O.C. W/(4) (0.131° X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

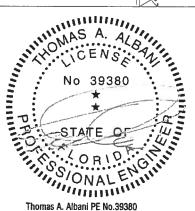


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- AS SHOWN IN DETAIL.

  ATTACH 2 x x 4"-0" SCAB TO EACH FACE OF
  TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS

  SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
  VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
- (MINIMUM 2X4)
  THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
  CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
  4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS.
- NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
  CONCENTRATED LOAD MUST BE APPLIED TO BOTH
  THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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### STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-10



MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING

EXPOSURE B or C ASCE 7-10

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.



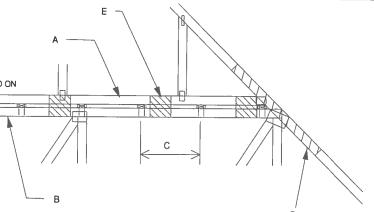
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

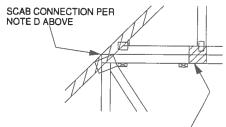
E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9' x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH

3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

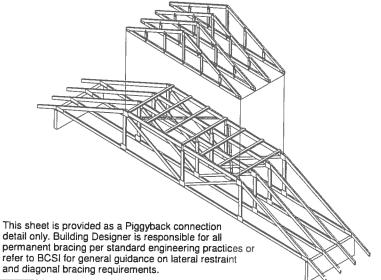


### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

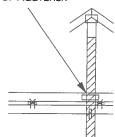
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

ATTACH 2 x \_\_\_ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

VEHICLE WEBS OF PIGGEBACK AND BASE THUSS.)
(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,

NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.



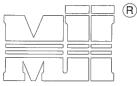
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

### STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

### MII-REP01A1

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Page 1 of 1



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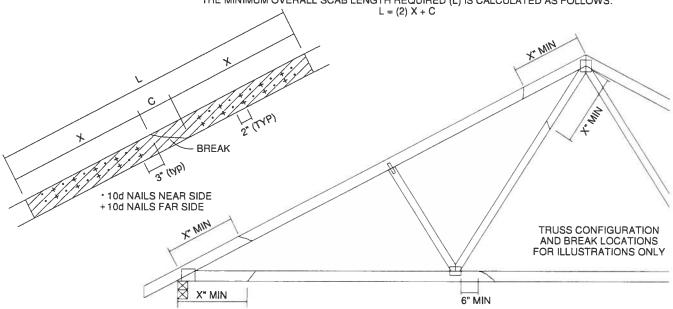
ENGINEERED BY 

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *				MAXIMUM FORCE (lbs) 15% LOAD DURATION								
		X INCHES	SP		DF		SPF		HF			
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6		
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028		
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608		
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187		
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767		
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347		

\* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x\_SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

### DO NOT USE REPAIR FOR JOINT SPLICES

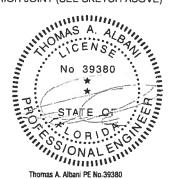
### NOTES:

- NOTES:

  1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

  2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
- AND HELD IN PLACE DURING APPLICATION OF REPAIR.

  THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
   THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x\_ ORIENTATION ONLY.
   THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



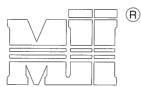
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### LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

NOTES:

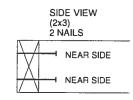
- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

  2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

  3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

### THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY



-	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)									
	DIAM. SP DF HF SPF SPF-S									
<u>o</u>	.131	88.0	80.6	69.9	68.4	59.7				
LONG	.135	93.5	85.6	74.2	72.6	63.4				
.5.	.162	108.8	99.6	86.4	84.5	73.8				
<u>က</u>										
LONG.	.128	74.2	67.9	58.9	57.6	50.3				
	.131	75.9	69.5	60.3	59.0	51.1				
3.25	.148	81.4	74.5	64.6	63.2	52.5				
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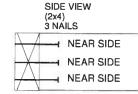
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

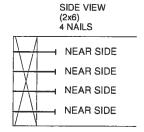
### **EXAMPLE:**

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

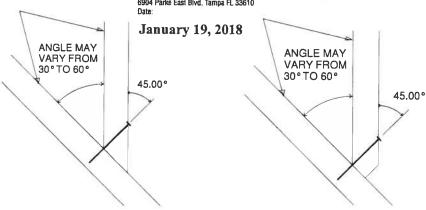
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

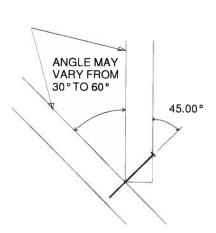






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### TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

MiTek USA, Inc.

Page 1 of 1



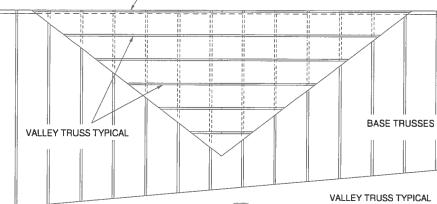
MiTek USA, Inc.

ENGINEERED BY HI

GABLE END, COMMON TRUSS OR GIRDER TRUSS

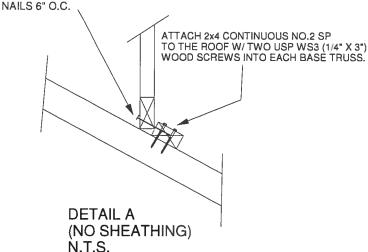
### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
- SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS 12 Р SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05  $\,$  146 MPH WIND DESIGN PER ASCE 7-10  $\,$  160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



### TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2

MiTek USA, Inc.

Page 1 of 1



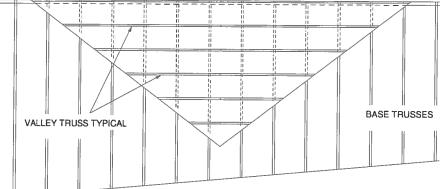
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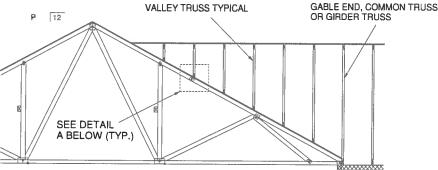
ENGINEERED 

GABLE END, COMMON TRUSS OR GIRDER TRUSS

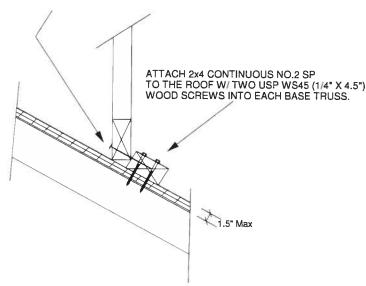
### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
- 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
- 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
- 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.





SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



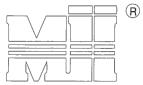
WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



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Page 1 of 1



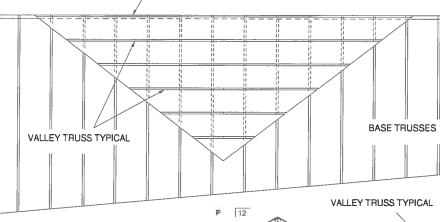
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ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

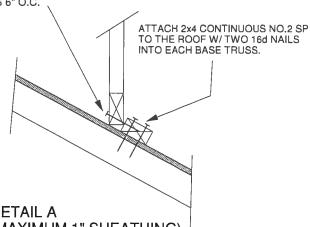
### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 16d (0.131" X 3.5") 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

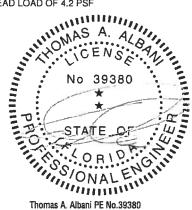
SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C.



DETAIL A (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE : 1.60

MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

AUGUST 1, 2016

TRUSSED VALLEY SET DETAIL
(HIGH WIND VELOCITY)

MITek USA, Inc.
Page 1 of 1

TO EXCEED 48" O.C. SPACING

FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE

CLIP MAY BE APPLIED TO THIS FACE UP TO THIS FACE UP TO THIS FACE UP TO THIS FACE UP TO TO THIS FACE UP TO TO THIS FACE UP TO THIS FACE UP TO THE PAGE UP TO THE

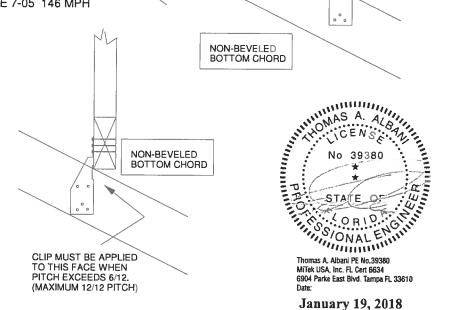
WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY)

ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT

SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF 2'-10" ON AFFECTED TOP CHORDS.

### NOTES:

- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.

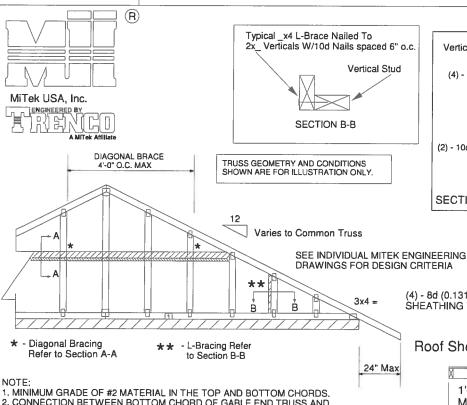


### Standard Gable End Detail

### MII-GE146-001

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Page 1 of 2



Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" n.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 Nn 2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A

2x4 Stud

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

(2) - 10pt

Roof Sheathing

1'-3"

CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
 BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:

2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4"-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

- TYPE TRUSSES.

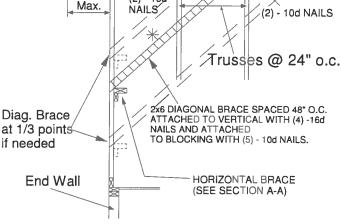
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade		Maxin	num Stud Length			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10	
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11	
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





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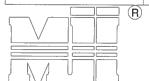
### **OCTOBER 5, 2016**

### REPLACE BROKEN OVERHANG

MII-REP13B

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Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

A MITOR ARTILIATE

TRUSS CRITERIA:

LOADING: 40-10-0-10 DURATION FACTOR: 1.15 SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

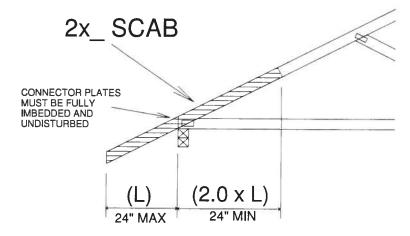
**END BEARING CONDITION** 

### NOTES:

1. ATTACH 2x\_SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



### **IMPORTANT**

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

### LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

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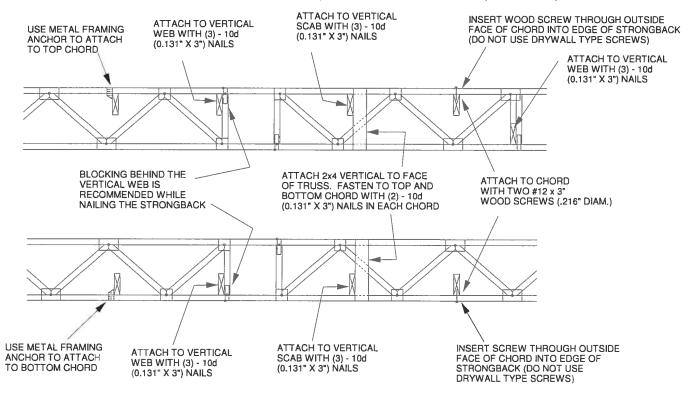
Page 1 of 1

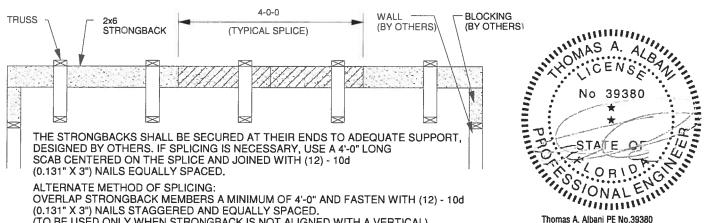


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Thomas A. Albani PE No.39380