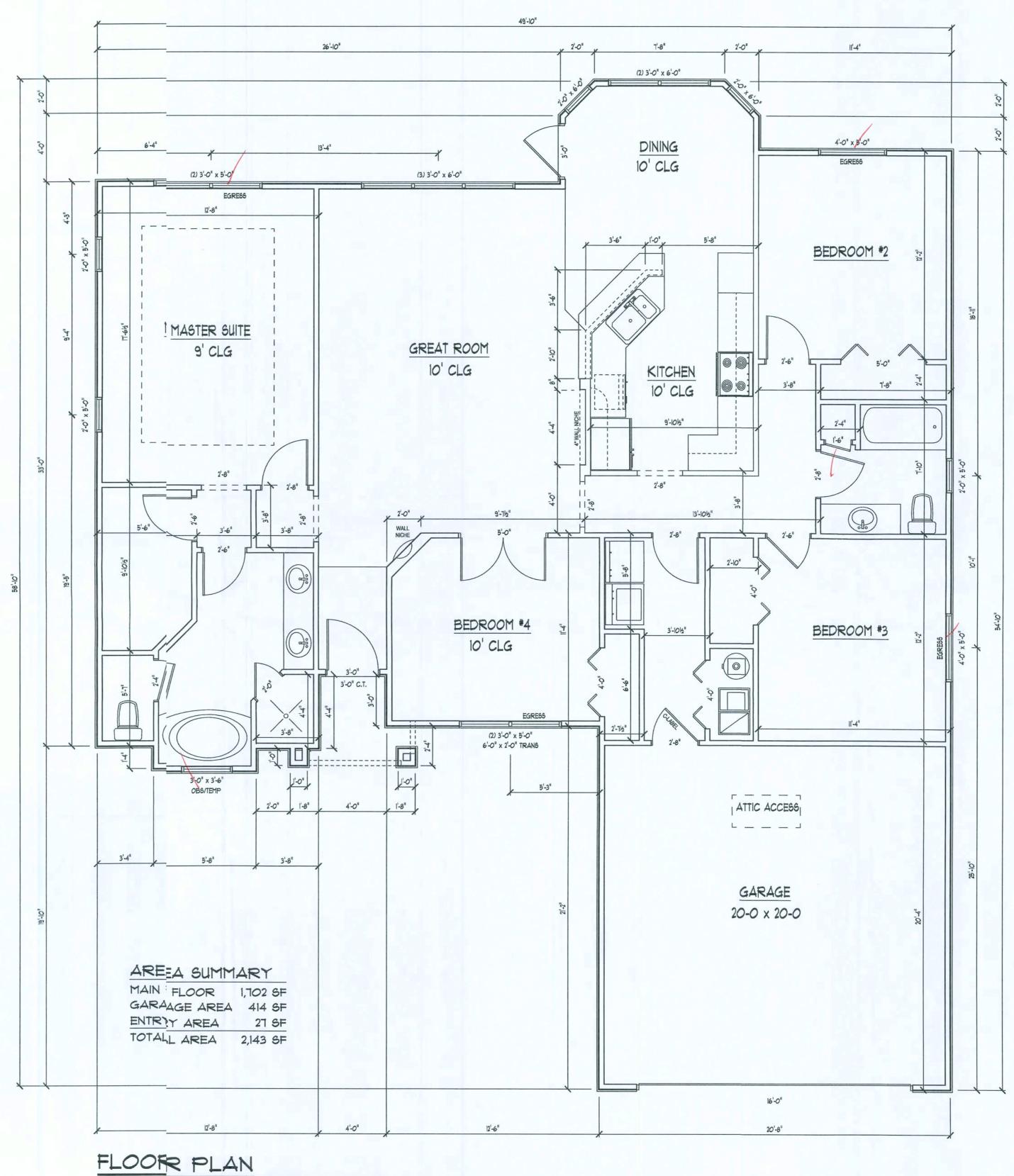




SCALE: |" = 1'0"



SCALE: 11/4" = 1'

Daniel Shaheen

April 06, 200

D.D.S. Studios

D.D.S. STUDIOS

P.O. Box 273 Lake City FL. 32054 (904) 754-0181

RRE

S

SHEET NUMBER 2 of 3

All work shall comply with the standard building cole, and all applicable local codes and ordinances. Contractor shall verify al

dimensions prior to commencing construction

PROJECT NUMBER 801010

D.D.S. STUDIOS

P.O. Box 273 Lake City FL. 320% (904) 754-0181

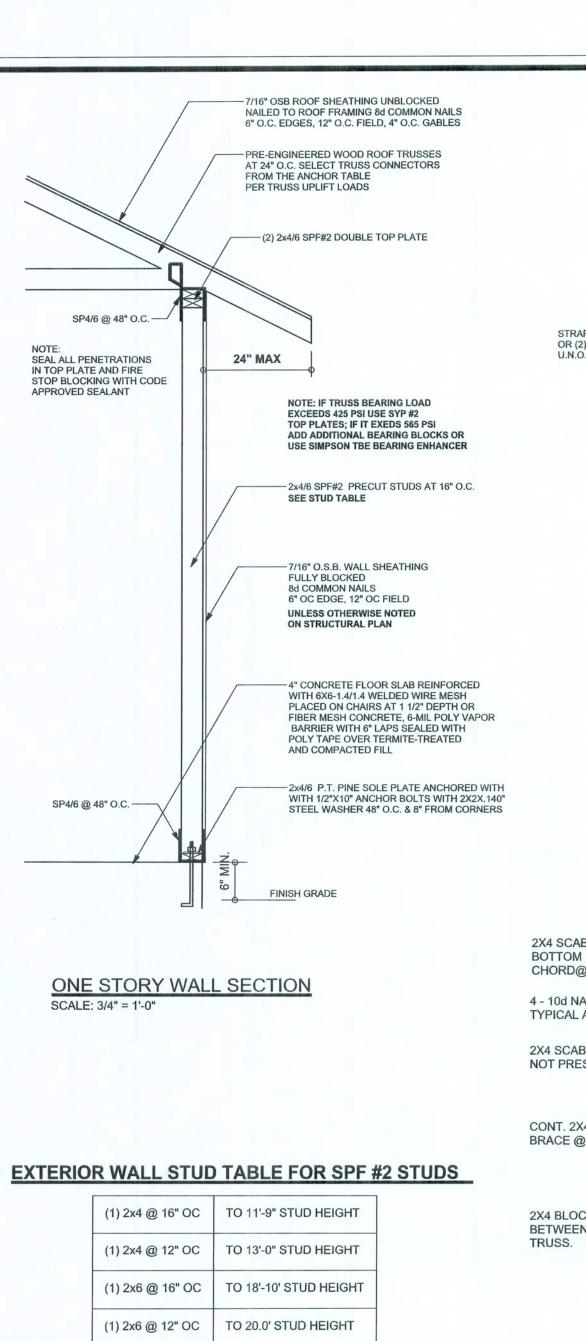
S

SHEET NUMBER 3 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances Contractor shall verifyall dimensions prior to commencing construction.

PROJECT NUMBER 801010

Daniel Shaheen



THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS ESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

-(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

—SEE FOOTING DETAILS

ANCHOR BOLT

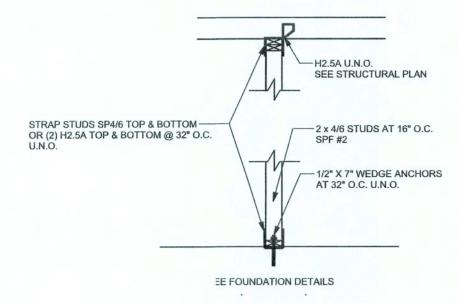
SIMPSON H2.5A U.N.O. -

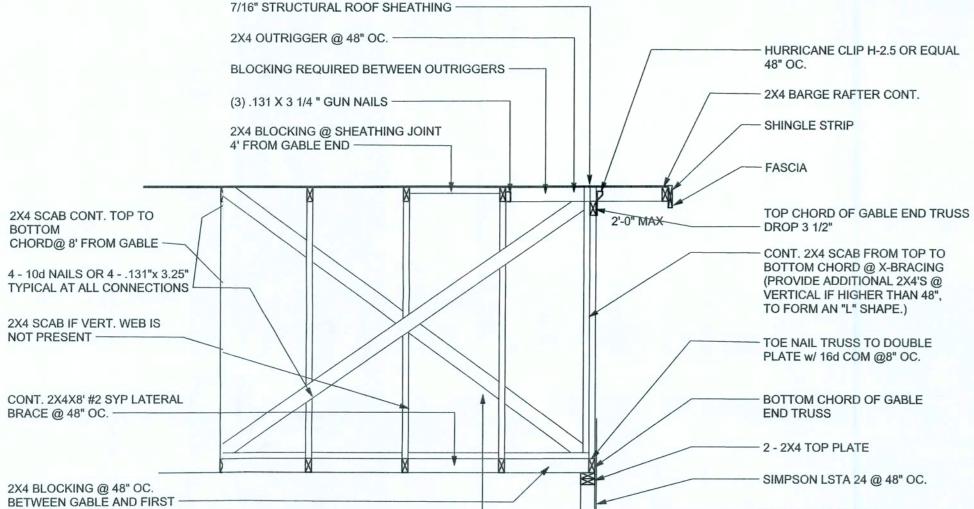
(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

SEE STRUCTURAL PLAN





T'PICAL GABLE END (X-BRACING)

2X4 X-BRACE @ 6'-0" OC.

ALL MEMBERS SHALL BE SYP

2X4 LADDER BEAM SUPPORTIVE 3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON -OPPOSITE SIDE) EA. NAILED WITH 14-10d SUPPORTIVE POST TO BEAM

DETAIL FOR SINGLE BEAM

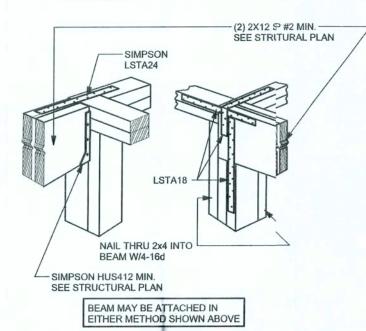
BEAM MID-WALL CONNETION DETAIL SCALE: N.T.S.

NAILS AT 16" C.

(4)-2x4 SPF #2 IILED

MIN. (SEE STRITURAL PLAN)

TOGETHER W/16d



BEAM CORNER CONNECTIONDETAIL SCALE: N.T.S.

2) 2X12 SYP #2 MIN. -

SEE STRUCTURAL PLAN

SEE STRUCTURAL PLAN

SUPPORTIVE BEAM -IF BEAM JOINT IS AT-POST CONNECTION. INSTALL ONE SIMPSON LSTA18 ON ONE SIDE

(2-ONE SIDE, 2-ON

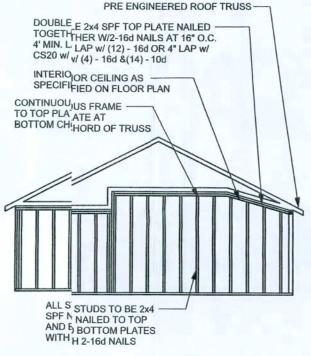
OTHER SIDE)

SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
² 2x10	SYP #2	1050	1.6
² 2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
FPSL	PARALAM	2900	2.0



2X4 STUDS @16" OC.

(6) .131 x 3 1/4" GUN NAILS-

INTO KING STUD

TOE NAILED THRU HEADER

CONTINUOUS FRAME TO **CEILLING DIAPHRAGM DETAIL**

-LSTA18 (U.N.O. -

CRIRIPPLES IF REQUIRED

(4) .13 31 x 3 1/4" GUN NAILS

TOEE NAILED THRU SILL -

INTOO JACK STUD U.N.O.

TYPIC CAL STRAPPING (U.N.O.)

-SP4 COR (2) H2.5A OR (2) SSP-

(SEE S STRUCTURAL PLAN)

ALL OOPENINGS (U.N.O.)

(1) 2X6 SPF F #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 110 MPIPH, 10'-0" WALL HIGHT U.N.O.)

TYPICAL HEAADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

SCALE: N.T.S.

-NAIL SHEATHING TO HEADER AND TOP

(6) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU HEADER

INTO KING STUD

PLATE WITH 8d AT 4" O.C. FOR UPLIFT

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIF	ILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH A CALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE REQUIRE	MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 MENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE	A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL LOAD ENGINEER IMMEDIATELY.
DESIGN, TRUSS-T	HE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, D-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTES: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

DESIGN DATA

PLIFT LBS. SYP			TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		oro rab
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
	< 2300	ABU66	12-16d 12-16d		1/2" AB
< 2300			1 LOU		1/2 AD

REVISONS

SOFTPIXIN

INDLOAD ENGIIEER: Mark Disosway, PE No.53915, POE868, Lake City, FL

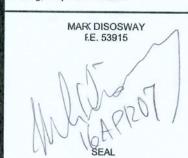
dimensions. Refer III questions to Mark Disosway, P.i. for resolution. Do not proceed witout clarification.

32056, 386-754-549

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CERTIFICATION: hereby certify that I have amined this plan and that the applicable tions of the plan relating to wind engineer omply with section R301.2.1, florida building de residential 2014, to the best of my

LIMITATION: This esign is valid for one building, at specifiel location.



Compass Builders

Spec House _ot 7

ADDRESS: Lot 7 Rollng Meadows S/D

Columbia County, Florida

Rolling Meadows S/D

Mark Dsosway P.E. P.O Box 868 Lake City Florida 32056 Phone: (336) 754 - 5419 Fax: (383) 269 - 4871

PRINTED DATE: April 13, 2007 DRAWN BY: CHECKED BY:

FINALS DATE: 16 / Apr / 07

> JOB NUMBER: 704131 DRAWING NUMBER

> > OF (SHEETS

Zone Effective Wind Area (ft2) 19.9 -21.8 | 18.1 | -18.1 2 | 19.9 | -25.5 | 18.1 | -21.8 -40.6 2 O'hg 3 | 19.9 | -25.5 | 18.1 | -21.8 3 O'hg -68.3 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows | 21.8 | -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9 16x7 Garage Door 18.5 -21.0 DESIGN LOADS FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER)

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT</p>

WIND IMPORTANCE FACTOR = 1.0

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

2.) WIND EXPOSURE = B

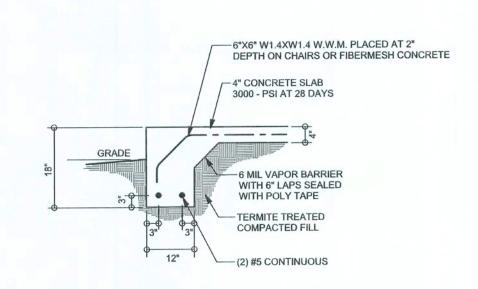
4.) BUILDING CATEGORY = II

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;

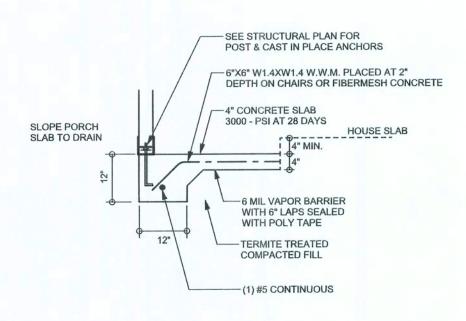
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%

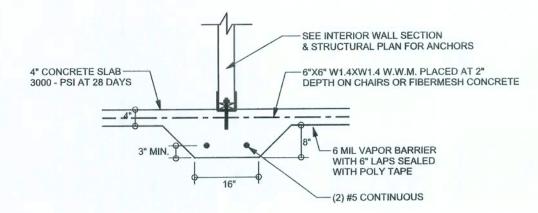
SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.



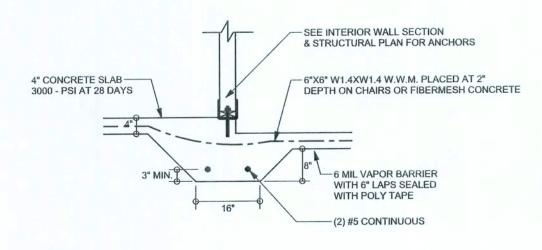
F1 MONOLITHIC FOOTING S-2 SCALE: 1/2" = 1'-0"



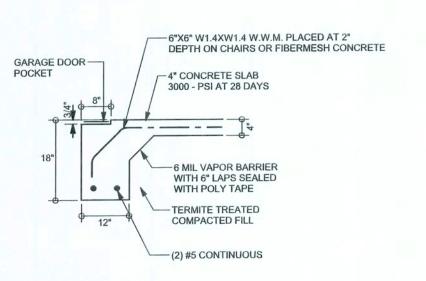
F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"



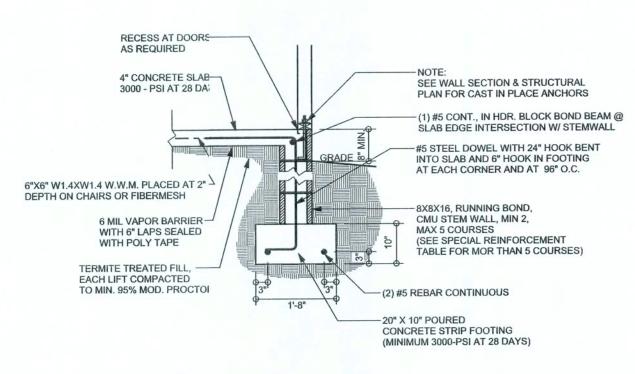
F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



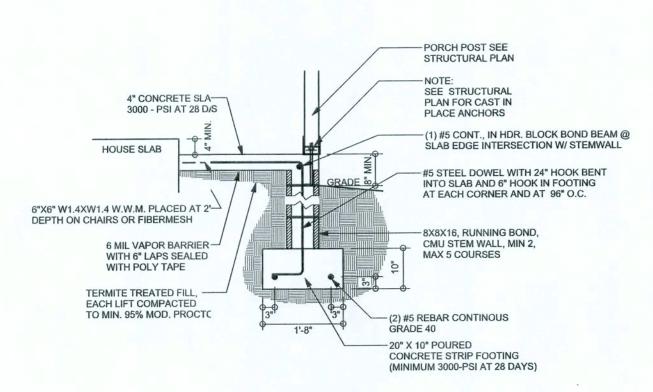
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"



F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"

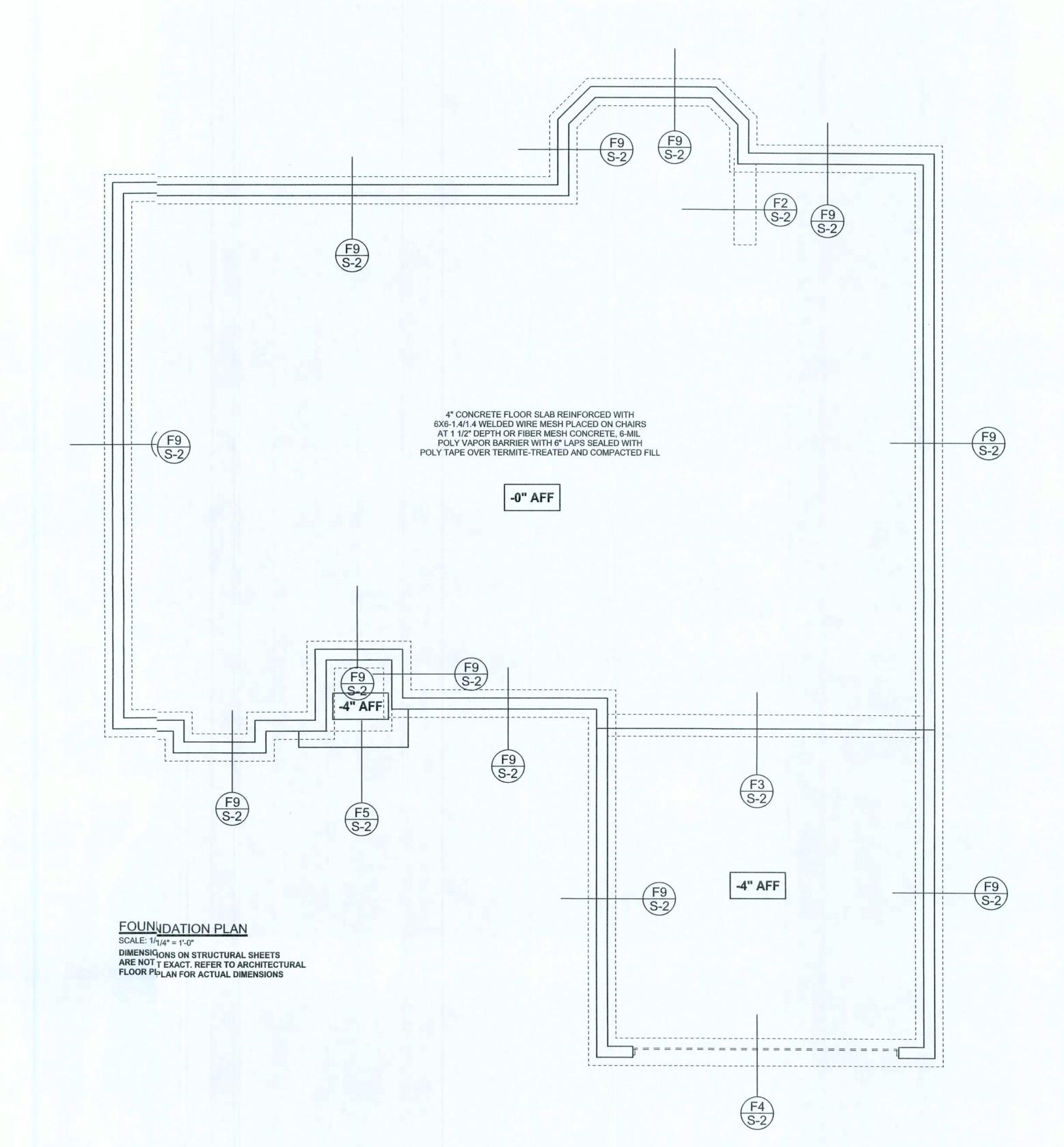


S-2 SCALE: 1/2" = 1'-0"

TALL TEM WALL TABLE

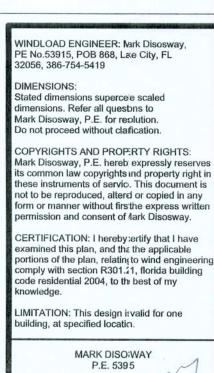
The table assnes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slaat the top. The vertical steel is to be placed toward the tension side of the CMU wall (aw from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' highed Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcent as shown in the table below.

STEMWALI HEIGHT (FEET)	JNBALANCED BACKFILL HEIGHT	FOR 8	AL REINFOR " CMU STEN INCHES O.C	WALL	FOR 1	AL REINFOR 2" CMU STEI INCHES O.C	MWALL
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



REVISIONS

SOFTPLAN ARCHITECTURAL DESIG SOFTMARE



Compass Fuilders

Spec House

Lot 7

Rolling Mealows S/D

ADDRES: Lot 7 Rolling M:adows S/D Columbia Courty, Florida

Mark Disosvay P.E. P.O. Box 868 Lake City, Floida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

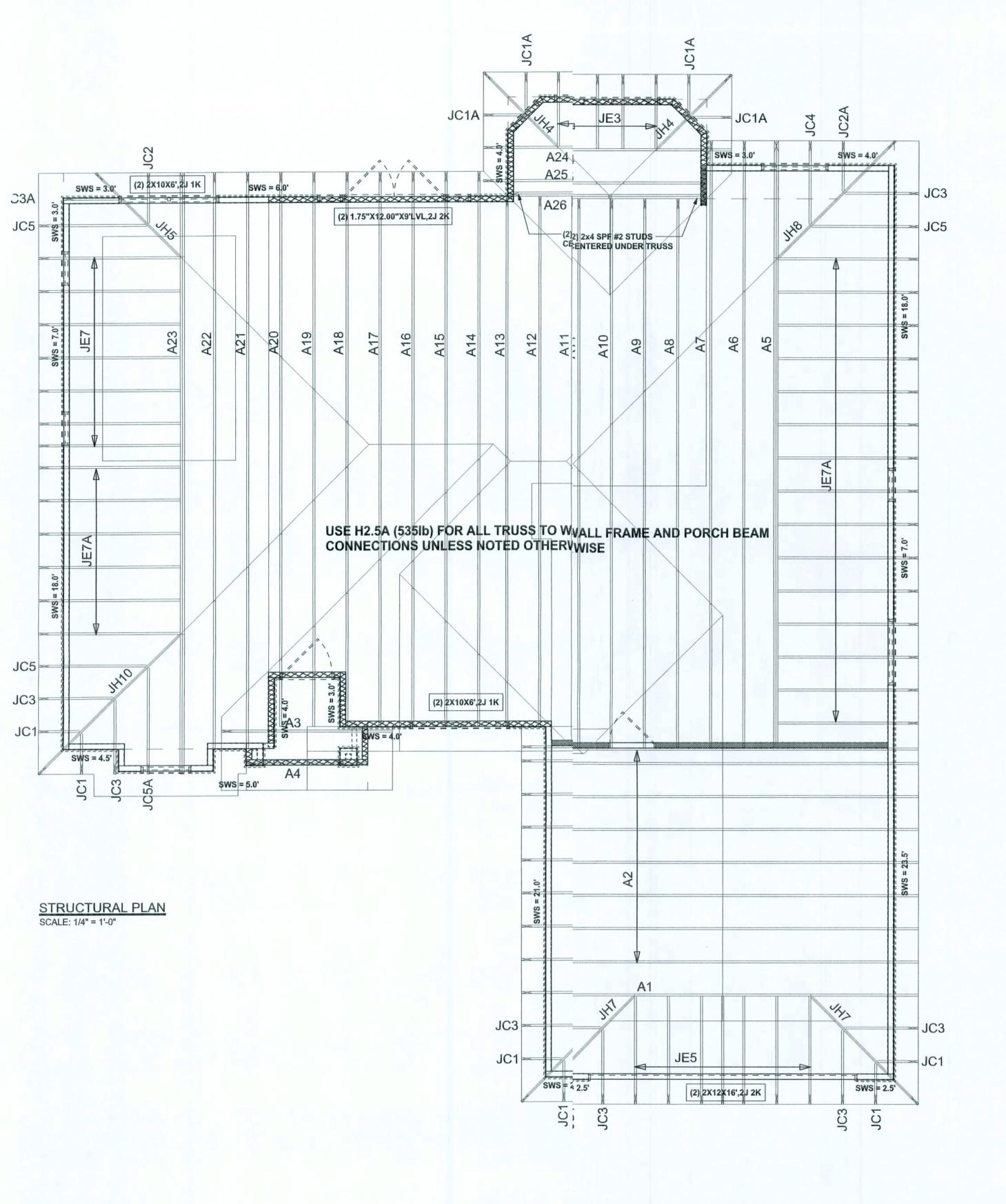
PRINTED FATE:
April 16, 20(7

DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE:

JOB NUNBER: 704131 DRAWING NUMBER

> S-2 OF 3 SHETS



STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3

ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW = = = = 100000000	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND

(2) 2X1	10X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.)
	NUMBER OF KING STUDS (FULL LENGTH)
	NUMBER OF JACK STUDS (UNDER HEADER
	SPAN OF HEADER
	SIZE OF HEADER MATERIAL
	NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	35.4'	87.5'
LONGITUDINAL	29.6'	34.5

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W. B. HOWLAND TRUSS JOB #4494 REVISIONS

SOFTPIAN

WINDLOAD ENGINEER: Mrk Disosway, PE No.53915, POB 868, Lae City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercee scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.
Do not proceed without clarication.

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CERTIFICATION: I hereby ertify that I have examined this plan, and that he applicable portions of the plan, relating to wind engineering comply with section R301.21, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design isvalid for one building, at specified locatio.

MARK DISO:WAY
P.E. 5395

SEAL

Compass Fuilders

Spec House

Rolling Mealows S/D

ADDRESS: Lot 7 Rolling Madows S/D Columbia Couny, Florida

Mark Disosvay P.E. P.O. Box868 Lake City, Florda 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
April 16, 2007

DRAWN BY: CHECKED BY:

FINALS DATE: 16 / Apr / 07

David Disosway

JOB NUNBER: 704131 DRAWING NUMBER

S-3OF 3 SHEETS