4.000	Columbia Coun			PERMIT
ADDLICANT	This Permit Expires One	e Year From the Date of PHONE	352 258-8603	000025765
APPLICANT ADDRESS	JERRY RYE 3817 NW 28TH TERR	GAINESVILLE		FL 32605
OWNER	WALT & KRISSY SMITH	PHONE	755-4235	
ADDRESS	1214 SW WALTER AVE	LAKE CITY		FL 32024
CONTRACTO		PHONE	352 258-8603	
LOCATION O	F PROPERTY 47S, TL ON WALTER RD,1	MILE ON RIGHT, RCCI SIG	N BY GATE	
TYPE DEVEL	OPMENT SFD,UTILITY	ESTIMATED COST OF CO	NSTRUCTION	153100.00
HEATED FLO	OR AREA 3062.00 TOTAL	AREA 4702.00	HEIGHT	STORIES 1
FOUNDATION	N CONC WALLS FRAMED	ROOF PITCH 7/12	FLO	OR SLAB
LAND USE &	ZONING A-3	MAX	HEIGHT 19	
Minimum Set I	Back Requirments: STREET-FRONT 3	0.00 REAR	25.00	SIDE 25.00
NO. EX.D.U.	0 FLOOD ZONE X	DEVELOPMENT PERM	MIT NO.	
PARCEL ID	02-5S-16-03437-003 SUBDIV	ISION		
LOT	BLOCK PHASE UNIT	Control of the Contro	L ACRES 10.0	0
	BEOCK THASE UNIT		LACKES 10.0	
000001067	CGC1511121	J.M	m Rose	•
Culvert Permit 1	No. Culvert Waiver Contractor's License	Number A	pplicant/Owner/C	ontractor
WAIVER	07-271-N BK	<u>J</u>		Y
Driveway Conn	· · ·	Zoning checked by Appr	oved for Issuance	New Resident
COMMENTS:	ONE FOOT ABOVE THE ROAD,			
			Check # or Cas	h 1068
			Check # or Cas	h 1068
		NING DEPARTMENT	ONLY	h 1068 (footer/Slab)
Temporary Pow	ver Foundation _	NING DEPARTMENT		(footer/Slab)
	date/app. by	NING DEPARTMENT	ONLY Monolithic	(footer/Slab) date/app. by
	date/app. by	NING DEPARTMENT	ONLY Monolithic	(footer/Slab) date/app. by
	date/app. by gh-in plumbing date/app. by Rough-in plumbi	date/app. by	ONLY Monolithic Sheathing/Na	(footer/Slab) date/app. by ailing date/app. by
Under slab roug	date/app. by gh-in plumbing date/app. by date/app. by Rough-in plumbing date/app. by	date/app. by date/app. by date/app. by ng above slab and below wood	ONLY Monolithic Sheathing/Na	(footer/Slab) date/app. by ailing
Under slab roug	date/app. by gh-in plumbing date/app. by date/app. by Rough-in plumbing date/app. by	date/app. by ab date/app. by ab date/app. by ng above slab and below wood	ONLY Monolithic Sheathing/Na	(footer/Slab) date/app. by ailing date/app. by date/app. by
Under slab roug	date/app. by ch-in plumbing date/app. by date/app. by Rough-in plumbing date/app. by h-in date/app. by C.O. Final	date/app. by date/app. by date/app. by gabove slab and below wood	ONLY Monolithic Sheathing/Na	(footer/Slab) date/app. by ailing date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent power	date/app. by gh-in plumbing date/app. by Rough-in plumbing date/app. by Heat & Air Duct date/app. by C.O. Final	date/app. by ab date/app. by ab date/app. by ng above slab and below wood	ONLY Monolithic Sheathing/Na floor Peri. beam (Lintel)	(footer/Slab) date/app. by ailing date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent power	date/app. by gh-in plumbing date/app. by Rough-in plumbi date/app. by Heat & Air Duct date/app. by C.O. Final date/app. by blocking, electricity and plumbing	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by	ONLY Monolithic Sheathing/Na floor Peri. beam (Lintel)	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent power	date/app. by gh-in plumbing SI date/app. by Rough-in plumbin date/app. by h-in Heat & Air Duct date/app. by cr C.O. Final date/app. by blocking, electricity and plumbing Pump pole	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by date/app. by Utility Pol	ONLY Monolithic Sheathing/Na floor Peri. beam (Lintel) Culvert Pool	(footer/Slab) date/app. by ailing date/app. by date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent power M/H tie downs, Reconnection	date/app. by gh-in plumbing gh-in plumbing date/app. by Rough-in plumbing date/app. by h-in date/app. by C.O. Final date/app. by blocking, electricity and plumbing date/app. by Pump pole date/app. by	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by date/app. by	ONLY Monolithic Sheathing/Na floor Peri. beam (Lintel) Culvert Pool date/app. by	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent powe M/H tie downs, Reconnection M/H Pole	date/app. by gh-in plumbing SI date/app. by Rough-in plumbin date/app. by h-in Heat & Air Duct date/app. by cr C.O. Final date/app. by blocking, electricity and plumbing Pump pole	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by date/app. by Utility Pol	ONLY Monolithic Sheathing/Na floor Peri. beam (Lintel) Culvert Pool date/app. by Re-roof	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent powe M/H tie downs, Reconnection M/H Pole dat	date/app. by gh-in plumbing gh-in plumbing Adate/app. by Rough-in plumbing date/app. by h-in date/app. by C.O. Final date/app. by blocking, electricity and plumbing date/app. by Travel Trailer	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by Utility Pol- date/app. by date/app. by	ONLY Monolithic Sheathing/Na floor Peri. beam (Lintel) Culvert Pool date/app. by Re-roof	(footer/Slab) date/app. by ailing date/app. by date/app. by date/app. by date/app. by date/app. by
Under slab roug Framing Electrical roug Permanent powe M/H tie downs, Reconnection M/H Pole dat BUILDING PEI	date/app. by gh-in plumbing gh-in plumbing Adate/app. by Rough-in plumbing date/app. by h-in date/app. by C.O. Final date/app. by blocking, electricity and plumbing date/app. by Travel Trailer re/app. by RMIT FEE \$ 770.00 CERTIFICATION	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by date/app. by Utility Pole date/app. by A FEE \$ 23.51	ONLY Monolithic Sheathing/Na floor Peri. beam (Lintel) Culvert Pool date/app. by Re-roof SURCHARGE F	(footer/Slab) date/app. by ailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by
Under slab roug Framing Electrical rough Permanent power M/H tie downs, Reconnection M/H Pole dat BUILDING PEL MISC. FEES \$	date/app. by gh-in plumbing SI date/app. by Rough-in plumbi date/app. by h-in Heat & Air Duct date/app. by cr C.O. Final date/app. by blocking, electricity and plumbing date/app. by Travel Trailer se/app. by RMIT FEE \$ 770.00 CERTIFICATION 0.00 ZONING CERT. FEE \$ 5	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by Utility Pol- date/app. by date/app. by 23.51	Monolithic Sheathing/Na floor Peri. beam (Lintel) Culvert Pool date/app. by Re-roof SURCHARGE F WASTE	date/app. by EE\$ 23.51 FEE\$
Under slab roug Framing Electrical rough Permanent power M/H tie downs, Reconnection M/H Pole dat BUILDING PEL MISC. FEES \$	date/app. by gh-in plumbing SI date/app. by Rough-in plumbi date/app. by h-in Heat & Air Duct date/app. by C.O. Final date/app. by blocking, electricity and plumbing date/app. by Travel Trailer re/app. by RMIT FEE \$ 770.00 CERTIFICATION 0.00 ZONING CERT. FEE \$ 5	date/app. by ab date/app. by ng above slab and below wood date/app. by date/app. by date/app. by Utility Pole date/app. by A FEE \$ 23.51	Monolithic Sheathing/Na floor Peri. beam (Lintel) Culvert Pool date/app. by Re-roof SURCHARGE F WASTE	(footer/Slab) date/app. by ailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

mo

NOTICE OF COMMENCEMENT

To Whom It May Concern:

The undersigned hereby informs you that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is stated in this NOTICE OF COMMENCEMENT.

Description of Property: ATTACHED SCHEDULE A.

General Description of Improvements: CONSTRUCTION

Owner and Address: WALTER G SMITH

KRISTINE A SMITH

Property Address: 289 SW KAMAN DRIVE

LAKE CITY, FL 32024

STATE OF FLORIDA, COUNTY OF COLUMBIA I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.

P. DeWITT CASON, CLERK OF COURTS

Deputy Clerk Deputy Clerk

COUNTY

Owner's Interest in Site of the Improvement: Fee Simple

Contractor and Address:

JERRY C RYE CONSTURCTION

3817 NW 28th Terrace Gainesville, FL 32605

Surety (if any):	NA	<u> </u>	
Address:	***************************************	Amount of Bond \$	

Name and address of person within the State of Florida designated by owner upon whom notices or other documents may be served:

In addition to himself, owner designated the following person to receive a copy of Lienor's Notice as provided in Section 713.06 (2) (b) Florida Statutes:

Name and Address:

Ameris Bank

P O Box 899

Newberry, FL 32669

WALTER G SMITH

KRISTINE A SMITH

State of Florida County of Alachua

I hereby certify that on this day, before me, an officer duly authorized to administer oaths and take acknowledgements, personally appeared Walter G Smith and Kristine A Smith who is known to me to be the person(s) described in and who executed the foregoing instrument, who acknowledged before me that they executed the same.

Witness my hand and official seal in the County and State aforesaid this 23rd day of FEBRUARY, 2007..



Michelle M. Brady Notary Public

SCHEDULE A

BEGIN at the Southeast corner of the Southwest 1/4 of the Southeast 1/4 of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run S.88°26'27"W. along the South line of said Section 2 a distance of 666.82 feet; thence N.00°17'12"W. parallel to the East line of said Southwest 1/4 of the Southeast 1/4 a distance of 653.25 feet; thence N.88°26'27"E. parallel to the South line of said Section 2 a distance of 666.82 feet to a point on the East line of said Southwest 1/4 of the Southeast 1/4; thence S.00°17'12"E. along said East line 653.25 feet to the POINT OF BEGINNING. Containing 10.00 acres, more or less.

TOGETHER WITH: An Easement 30.00 feet in width, for ingress and egress lying 30.00 feet left (North) of and adjacent to the following described line: COMMENCE at the Southwest corner of the Southeast 1/4 of the Southeast 1/4 (being also the Southeast corner of the Southwest 1/4 of the Southeast 1/4) of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run N.00°17'12"W. along the West line of said Southeast 1/4 of the Southeast 1/4 a distance of 424.16 feet to the Northwest corner of the South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 and the POINT OF BEGINNING; thence N.88°26'55"E. along the North line of said South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 a distance of 1249.33 feet to a point on the Westerly maintained Right-of-Way line of SW Walter Avenue and the TERMINAL POINT of herein described line and easement.

AGS KS ropy.

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328 Inst:2007003854 Date:02/15/2007 Time:14:48

Doc Stamp-Deed: 0.70

______DC,P.DeWitt Cason,Columbia County B:1110 P:2714

File No. 07-59

Property Appraiser's Parcel Identification No. 02-55-16-03437-000 Chesyl

WARRANTY DEED

THIS INDENTURE, made this day of February, 2007, BETWEEN GLADYS L. SMITH, a single woman, whose post office address is 1018 SW Walter Avenue, Lake City, Florida 32024, of the County of Columbia, State of Florida, grantor*, and WALTER G. SMITH, whose post office address is 289 SW Kaman Drive, Lake City, Florida 32024, of the County of Columbia, State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

TOWNSHIP 5 SOUTH - RANGE 16 EAST

SECTION 2: BEGIN at the Southeast corner of the Southwest 1/4 of the Southeast 1/4 of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run S 88°26'27"W along the South line of said Section 2 a distance of 666.82 feet; thence N 00°17'12" W parallel to the East line of said Southwest 1/4 of the Southeast 1/4 a distance of 653.25 feet; thence N 88°26'27" E parallel to the South line of said Section 2, a distance of 666.82 feet to a point on the East line of said Southwest 1/4 of the Southeast 1/4; thence S 00°17'12" E along said East line 653.25 feet to the POINT OF BEGINNING.

TOGETHER WITH: An Easement 30.00 feet in width, for ingress, egress and utilities, lying 30.00 feet left (North) of and adjacent to the following described line: COMMENCE at the Southwest corner of the Southeast 1/4 of the Southeast 1/4 (being also the Southeast corner of the Southwest 1/4 of the Southeast 1/4) of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run N 00°17'12" W along the West line of said Southeast 1/4 of the Southeast 1/4 a distance of 424.16 feet to the Northwest corner of the South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 and the POINT OF BEGINNING; thence N 88°26'55" E along the North line of said South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 a distance of 1249.33 feet to a point on the Westerly maintained Right-of-Way line of SW Walter Avenue and the TERMINAL POINT of herein described line and easement.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(First Witness)

DeEtte F. Brown Printed Name

(Second Witness) Crystal L. Brunner

Printed Name

Sladys Z. Smith (SEAL)

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this day of February, 2007, by GLADYS L. SMITH, who is personally known to me and who did not take an oath.

My Commission Expires:

Notary Public

Printed, typed, or stamped name:

DE ETTE F. BROWN
MY COMMISSION # DD 578151
EXPIRES: October 22, 2010
Bonded Thru Notary Public Underwriters

Inst:2007003854 Date:02/15/2007 Time:14:48
Doc Stamp-Deed: 0.70
DC.P. Dewitt Correct

_DC,P.DeWitt Cason,Columbia County B:1110 P:2715

Columbia County Building Permit Application

22 /// 02 /// 1377
For Office Use Only Application # 070 4-11 Date Received 10 By W Permit # 2570
Application Approved by - Zoning Official Date Date Date Plans Examiner of TH Date 7-25
Flood Zone Development Permit Zoning Land Use Plan Map Category A
Comments SITE PLAN ON PLANS
NOC #EH Deed or PA Site Plan (ell 352) LOC- Fox 352-378-9003
Name Authorized Person Signing Permit JERRY RYE 258 8603 Phone 352-378-3006
Address 3817 N. W. 28 TERRACE GAINESVILLE, FL. 32605
Owners Name WALT AND KRISSY Smith Phone 386-755-4235
911 Address 1214 S.W. WALTER AVE (C 4) 30009
Contractors Name RYE CONSTRUCTION CO. INC., Phone OFF- 352-378-300
Address 3817 N.W. 28 TERRALE
Fee Simple Owner Name & Address WALTERG, SMITH AND HRISTINE A. SMITH
Bonding Co. Name & Address N/A
Architect/Engineer Name & Address BRAD MUNN P.D. BOX 773063 OCALAFL, 34477
Mortgage Lenders Name & Address AMERIS BANK 25365 West Newberry Rd Newberry
Circle the correct power company - FL Power & Light - Clay Elec Suwannee Valley Elec Progressive E
Property ID Number 02-55-16-03437-003 Estimated Cost of Construction 195,000.00
Subdivision Name N/A PRIVATE PROPERTY Lot Block Unit Phase
Driving Directions Hwy 47 South Approx 1.3 miles South of I-75
TURN LEFT ON WALTER Rd-WALTER RD SOUTH APPROX IMI
TO PROPERTY ON RIGHT-RCCI SION BY GATE
Type of Construction Frame OF CONC SLAB Number of Existing Dwellings on Property Nowe
Total Acreage 10 Lot Size 1,2 AL, Do you need a - Culvert Permit or Culvert Walver or Have an Existing
Actual Distance of Structure from Property Lines - Front 87 / Side 85 Rear 94's
Total Building Height 19'51/2" Number of Stories 1 Heated Floor Area 2057 Roof Pitch 7/12
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standar all laws regulating construction in this jurisdiction.
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAY! TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH Y LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.
JERRY L. RYE
Owner Builder or Authorized Person by Notarized Letter Contractor Signature
Contractors License Number CGC 151112
STATE OF FLORIDA Competency Card Number DEBORAH A. DEAN NOTARY STAMP/SEAL
Sworn to (or affirmed) and subscribed before Expires 4/18/2010
this 10th day of April Bonded thru Clebaral Allegen
Personally known or Produced Identification Notary Signature
William Carlo

DESCRIPTION - 10 ACRES

Wednesday, January 31, 2007

FOR: Walter Smith

BEGIN at the Southeast corner of the Southwest 1/4 of the Southeast 1/4 of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run S.88°26'27"W. along the South line of said Section 2 a distance of 666.82 feet; thence N.00°17'12"W. parallel to the East line of said Southwest 1/4 of the Southeast 1/4 a distance of 653.25 feet; thence N.88°26'27"E. parallel to the South line of said Section 2 a distance of 666.82 feet to a point on the East line of said Southwest 1/4 of the Southeast 1/4; thence S.00°17'12"E. along said East line 653.25 feet to the POINT OF BEGINNING. Containing 10.00 acres, more or less.

TOGETHER WITH: An Easement 30.00 feet in width, for ingress and egress lying 30.00 feet left (North) of and adjacent to the following described line: COMMENCE at the Southwest corner of the Southeast 1/4 of the Southeast 1/4 (being also the Southeast corner of the Southwest 1/4 of the Southeast 1/4) of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run N.00°17'12"W. along the West line of said Southeast 1/4 of the Southeast 1/4 a distance of 424.16 feet to the Northwest corner of the South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 and the POINT OF BEGINNING; thence N.88°26'55"E. along the North line of said South 424.16 feet of the Southeast 1/4 a distance of 1249.33 feet to a point on the Westerly maintained Right-of-Way line of SW Walter Avenue and the TERMINAL POINT of herein described line and easement.

Year T Property	CamaUSA Appraisal Legal Description 0,3,4,3,7,-0,0,3	Maintenance Sel	72000	lumbia Land AG Bldg Xfea	County 001 000 000
SMITH WALT	ΓER G		72000	TOTAL	В*
3 666.82 FT, 8 5 829-1383, 82 7 WD 2220-2714 9 11 13 15 17 19 21 23 25	S 653.25 FT TO POB.	W 666.82 FT, N 653.25 661-675, 676, 829-138 DC REMA SMITH 1000-25 Mnt 2/23/2 To PgUp/PgDn F24=More	0. 28,,	4 6 8 10 12 14 16 18 20 22 24 26 28	

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787 PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED:

2/5/2007

DATE ISSUED:

3/5/2007

ENHANCED 9-1-1 ADDRESS:

1214

SW WALTER

AVE

LAKE CITY

FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

02-5S-16-03437-003

Remarks:

Address Issued By:

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

649

Approved Address

MAR 0 5 2007

911Addressing/GIS Dept



STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-2711

PART II - SITE PLAN
Scale: Each block represents 5 feet and 1 inch = 50 feet. 2341 ±
Shope 25
1 30 19 35 24 35 COME PARKING
FRONT HE THE THE PRONT HE THE TOTAL THE
Propose o DRIVEWAY
FUTURE (SLOPE)
Notes: House PAD Approx 1500' OFF WALTER AVE-PRIVATE BOAD BACK TO
PAD. 1.2 ACRES - PART OF 10 ACRE TRACT
Site Plan submitted by: Plan Approved PRESIDENT Title Not Approved UICO 7 Date 3-26-07
By County Health Departm
ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

1	AC	ORD CERTIFIC	ATE OF LIABIL	LITY INS	URANCE	Ξ	10/11/2006				
300	UCER	(352)377-2002 F	AX (352)376-8393	THIS CERT	TIFICATE IS ISSU	JED AS A MATTER OF					
		rough Company Insurance	e, Inc.	HOLDER.	THIS CERTIFICA	RIGHTS UPON THE CEI TE DOES NOT AMEND	EXTEND OR				
		W 41st Street		ALTER TH	E COVERAGE A	FFORDED BY THE POL	ICIES BELOW.				
		Box 147050 sville, FL 32614-7050		INSURERS A	AFFORDING COV	/ERAGE	NAIC#				
		Rye Construction Company	y, Inc.	INSURER A: AU	INSURER A: Auto Owners Insurance Co.						
		3817 NW 28th Terrace		INSURER B:							
		Gainesville, FL 32605		INSURER C:							
				INSURER D:		,					
				INSURER E:							
:01	/ER/	AGES									
AN	IE PO	DUCIES OF INSURANCE LISTED BEL- QUIREMENT, TERM OR CONDITION ERTAIN, THE INSURANCE AFFORDER ES. AGGREGATE LIMITS SHOWN MA	OF ANY CONTRACT OR OTHER D IN BY THE POLICIES DESCRIBED H	EREIN IS SUBJECT	ZESPECT TO WHICE	HIMSCHRIEGAIRMAY	BE ISSUED OR				
			POLICY NUMBER	POLICY EFFECTIVE	POLICY EXPIRATION	LIMIT	s				
IR.	ADD'L NSRD	TYPE OF INSURANCE GENERAL LIABILITY	B06101102258	10/11/2006	10/11/2007	EACH OCCURRENCE	\$ 500,000				
		X COMMERCIAL GENERAL LIABILITY	2002022	,,		DAMAGE TO RENTED PREMISES (Ea occurence)	\$ 100,000				
		CLAIMS MADE X OCCUR			}	MED EXP (Any one person)	\$ 10,000				
4			_			PERSONAL & ADV INJURY	\$ 500,000				
						GENERAL AGGREGATE	\$ 1,000,000				
		GEN'L AGGREGATE LIMIT APPLIES PER: POLICY PRO- JECT LOC	- Sun - 1 TE		20.00	PRODUCTS - COMP/OP AGG	\$ 1,000,000				
_		AUTOMOBILE LIABILITY ANY AUTO				COMBINED SINGLE LIMIT (Ea accident)	\$				
		ALL OWNED AUTOS SCHEDULED AUTOS				BODILY INJURY (Per person)	s				
		HIRED AUTOS NON-OWNED AUTOS				BODILY INJURY (Per accident)	s				
		NON-OWNED ACTOS				PROPERTY DAMAGE (Per accident)	\$				
-	-	GARAGE LIABILITY				AUTO ONLY - EA ACCIDENT	\$				
		ANY AUTO		at		OTHER THAN EA ACC	\$				
						AUTO ONLY: AGG	\$				
		EXCESS/UMBRELLA LIABILITY				EACH OCCURRENCE	\$				
		OCCUR CLAIMS MADE				AGGREGATE	\$				
							\$				
		DEDUCTIBLE		-			\$				
		RETENTION \$			 	WC STATU- OTH-	\$				
		RKERS COMPENSATION AND		,		E.L. EACH ACCIDENT	s				
	ANY	PROPRIETOR/PARTNER/EXECUTIVE ICER/MEMBER EXCLUDED?				E.L. DISEASE - EA EMPLOYEE					
	If yes	s, describe under				E.L. DISEASE - POLICY LIMIT	s				
-	OTH	CIAL PROVISIONS below ER									
			y 8			ana .					
)Fe	CBIPT	ION OF OPERATIONS / LOCATIONS / VEHICL	LES / EXCLUSIONS ADDED BY ENDORSE	MENT / SPECIAL PROV	VISIONS	Annager					
en	era	1 Contractor Licence# C	GC1511121; FEIN# 74-31	64785							
CE	DTIE	ICATE HOLDER		CANCELLA	TION						
SE	RIII	ICATE HOLDER	•	And the second lives and the second		CRIBED POLICIES BE CANCELL	ED BEFORE THE				
				The state of the s	/ \ /	ISSUING INSURER WILL ENDE					
						THE CERTIFICATE HOLDER					
		City of Lake City, Bldg	Department			ICE SHALL IMPOSENO OBLIGA					
		150 N Alachua Street		OF ANY/KIN	DUPON THE INSURER	, ITS AGENTS OR REPRESENTA	ATIVES.				
		Lake City, FL 32055			Haythorne	1 Att					
				Caro Ann	y Haychorne	MANA STATE	20220247014000				
AC	ORD	25 (2001/08)		l	1	/// GACORD	CORPORATION 1988				
				0.000							

THIS INSTRUMENT WAS PREPARED BY:

Recording Fee \$ Documentary Stamp \$

TERRY MCDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328 Inst:2007003854 Date:02/15/2007 Time:14:48

0.70

Doc Stamp-Deed : DC,P.DeWitt Cason,Columbia County B:1110 P:2714

File No. 07-59

Property Appraiser's Parcel Identification No. 02-55-16-03:137-000

WARRANTY DEED

THIS INDENTURE, made this 15th day of February, 2007, BETWEEN GLADYS L. SMITH, a single woman, whose post office address is 1018 SW Walter Avenue, Lake City, Florida 32024, of the County of Columbia, State of Florida, grantor*, and WALTER G. SMITH, whose post office address is 289 SW Kaman Drive, Lake City, Florida 32024, of the County of Columbia, State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

TOWNSHIP 5 SOUTH - RANGE 16 EAST

SECTION 2: BEGIN at the Southeast corner of the Southwest 1/4 of the Southeast 1/4 of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run S 88°26'27"W along the South line of said Section 2 a distance of 666.82 feet; thence N 00°17'12" W parallel to the East line of said Southwest 1/4 of the Southeast 1/4 a distance of 653.25 feet; thence N 88°26'27" E parallel to the South line of said Section 2, a distance of 666.82 feet to a point on the East line of said Southwest 1/4 of the Southeast 1/4; thence S 00°17'12" E along said East line 653.25 feet to the POINT OF BEGINNING.

TOGETHER WITH: An Easement 30.00 feet in width, for ingress, egress and utilities, lying 30.00 feet left (North) of and adjacent to the following described line: COMMENCE at the Southwest corner of the Southeast 1/4 of the Southeast 1/4 (being also the Southeast corner of the Southwest 1/4 of the Southeast 1/4) of Section 2, Township 5 South, Range 16 East, Columbia County, Florida and run N 00°17'12" W along the West line of said Southeast 1/4 of the Southeast 1/4 a distance of 424.16 feet to the Northwest corner of the South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 and the POINT OF BEGINNING; thence N 88°26'55" E along the North line of said South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 a distance of 1249.33 feet to a point on the Westerly maintained Right-of-Way line of SW Walter Avenue and the TERMINAL POINT of herein described line and easement.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(First Withess)

DeEtte F. Brown Printed Name

(Second Witness) Crystal L. Brunner

Printed Name

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this day of February, 2007, by GLADYS L. SMITH, who is personally known to me and who did not take an oath.

My Commission Expires:

Notary Public Printed, typed, or stamped name:

S V. SMITH (SEAL)

DE ETTE F. BROWN
MY COMMISSION & DD 578151
EXPIRES: October 22, 2010
Borised Thru Notery Public Uniderwittens

Inst:2007003854 Date:02/15/2007 Time:14:48
Doc Stamp-Deed: 0.70

_DC,P.Dewitt Cason,Columbia County B:1110 P:2715



Columbia County, Florida Planning & Zoning Department

Review of Building Permit for compliance with County's Comprehensive Plan and Land Development Regulations

To: Jerry Rye

Fax: 352.378.9003

From: Brian L. Kepner, County Planner

Fax: 386.758.2160

Number of pages: 1

Date: 27 April 2007

RE: Building Permit Application 0704-22, Walter Smith

Dear Jerry:

This 10 acre parcel has a fairly good amount of the property in a flood zone. The site plan submitted only shows the distances from a 1.2 acre portion of the 10 acres. Please submit a site plan that shows the distances from the proposed house to the property lines of the entire 10 acres in order to make a better determination if the proposed home site is located within the flood zone. Thank you.

If you have any questions concerning this matter, please do not hesitate to contact me at 386.758.1007.

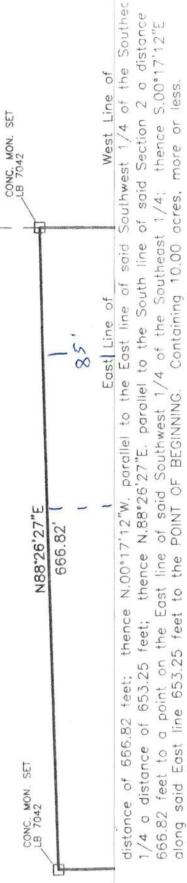
Sincerely,

Brian L. Kepner Land Development Regulation Administrator, County Planner

Confidentiality Notice: This facsimile transmission is confidential and is intended only for the review of the party to whom it is addressed. It may contain proprietary and/or privileged information protected by law. If you are not the intended recipient, you may not use, copy or distribute this facsimile message or its attachments. If you have received this transmission in error, please immediately telephone the sender above to arrange for its return.

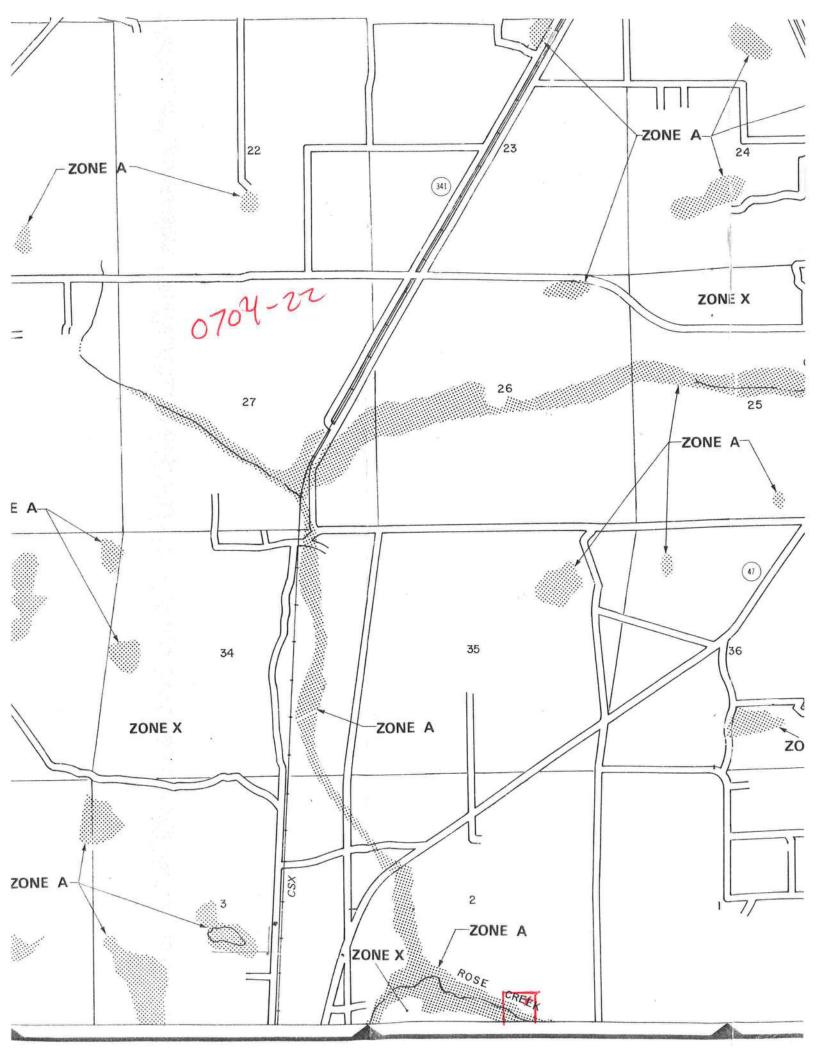
3000

IN THE S.E. 1/4 OF SECTION 2 TOWNSHIP 5 SOUTH, RANGE 16 EAST COLUMBIA COUNTY, FLORIDA



TOGETHER WITH: An Easement 30.00 feet in width, for ingress and egress lying 30.00 feet left (North) of and Township 5 South, Range 16 East, Columbia County, Florida and run N.00°17'12"W. along the West line of said Southeast 1/4 of the Southeast 1/4 a distance of 424.16 feet to the Northwest corner of the South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 and the POINT OF BEGINNING; thence N.88°26'55"E, along the North line of said South 424.16 feet of the Southeast 1/4 of the Southeast 1/4 a distance of 1249.33 feet a point on the Westerly maintained Right-of-Way line of SW Walter Avenue and the TERMINAL POINT of herein Southeast 1/4 (being also the Southeast corner of the Southwest 1/4 of the Southeast 1/4) of Section 2, adjacent to the following described line: COMMENCE at the Southwest corner of the Southeast 1/4 of the described line and easement

0704-22



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name:
Address:

RYE - SMITH RESIDENCE

Builder: Permitting Office: Permit Number:

RYE CONSTRUCTION **COLUMBIA COUNTY**

Cap: 42.0 kBtu/hr

SEER: 13.00

SEER: 13.00

Cap: 24.0 kBtu/hr

Cap: 42.0 kBtu/hr

Cap: 24.0 kBtu/hr

Cap: 50.0 gallons

EF: 0.93

MZ-C, MZ-H __

HSPF: 8.50

HSPF: 8.30

City, State: Owner:

WALT & KRISSY SMITH

25765 Jurisdiction Number: 22/000

Climate Zone:

North

New Single family

4. Number of Bedrooms 5. Is this a worst case?

3062 ft²

Conditioned floor area (ft2) Glass type 1 and area: (Label reqd. by 13-104.4.5 if not default)

a. U-factor: Description Area (or Single or Double DEFAULT) 7a. (Dble, U=0.9) 90.0 ft2

b. SHGC:

(or Clear or Tint DEFAULT)

New construction or existing

3. Number of units, if multi-family

2. Single family or multi-family

7b. (Clear) 339.0 ft²

8. Floor types

a. Slab-On-Grade Edge Insulation b. Raised Wood, Adjacent

R=0.0, 200.5(p) ft R=8.0, 1006.0ft²

c. N/A

Wall types

a. Frame, Wood, Exterior b. Frame, Wood, Adjacent R=13.0, 2908.0 ft² R=13.0, 816.0 ft²

c. N/A

d. N/A e. N/A

10. Ceiling types

a. Under Attic

R=30.0, 3062.0 ft²

b. N/A c. N/A

11. Ducts(Leak Free) a. Sup: Unc. Ret: Unc. AH: Attic

b. Sup: Unc. Ret: Unc. AH: Attic

Sup. R=6.0, 260.0 ft Sup. R=6.0, 120.0 ft 12. Cooling systems

a. Central Unit

b. Central Unit

c. N/A

13. Heating systems

a. Electric Heat Pump

b. Electric Heat Pump

c. N/A

14. Hot water systems

a. Electric Resistance

b. N/A

c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)

15. HVAC credits

(CF-Ceiling fan, CV-Cross ventilation,

HF-Whole house fan,

PT-Programmable Thermostat, MZ-C-Multizone cooling,

MZ-H-Multizone heating)

Glass/Floor Area: 0.11

Total as-built points: 36038 Total base points: 39680

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY Farry Resmondo

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____ DATE:

Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

specifications covered by this

with the Florida Energy Code.

calculation indicates compliance

Review of the plans and

BUILDING OFFICIAL:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: ,,,

PERMIT #:

	BASE			4		AS-	BUI	LT				
GLASS TYPES												
.18 X Condition	ed X BSF	PM = P	oints	- 100		erhang	Hat	Area X	SDM	×	SOF	= Points
Floor Area	a			Type/SC	Ornt	Len	ngt	Alea A	OF IV	/	001	The state of the state of
.18 3062.0	18	8.59	10246.0	1.Double,U=0.87,Clear	W	10.0	7.0	36.0	38.52		0.46	633.0
A A CONTRACTOR				2.Double,U=1.45,Clear	W	10.0	8.0	24.0	37.46		0.48	429.0 150.0
			1	3.Double,U=0.69,Clear	W	10.0	8.0	8.0	39.28		0.48	651.0
				4.Double,U=0.87,Clear	W	1.5	7.0	18.0 25.0	38.52		0.94	450.0
				5.Double,U=0.87,Clear	N	1.5 10.0	6.0 7.0	90.0	42.0		0.44	1671.
				6.Double,U=0.87,Clear	E	7.5	7.0	24.0	42.0		0.50	504.
				7.Double,U=0.87,Clear	NE	8.0	7.0	18.0	29.5		0.56	295.
				8.Double,U=0.87,Clear 9.Double,U=0.87,Clear	NE	8.0	7.0	18.0	29.5		0.56	295.0
				10.Double,U=0.87,Clear	W	1.5	5.5	78.0	38.5	2	0.90	2695.
				10.D0uble,0=0.07,010a1		30.00	70 W.3050					
				As-Built Total:				339.0				7773.
WALL TYPES	Area X	BSPM	= Points	Туре		R	-Value	e Area	Х	SPN	/I =	Points
	816.0	0.70	571.2	1. Frame, Wood, Exterior			13.0	2908.0		1.50		4362.
Adjacent	2908.0	1.70	4943.6	2. Frame, Wood, Adjacent			13.0	816.0		0.60		489.
Exterior	2900.0	1.70	4040.0									
Base Total:	3724.0		5514.8	As-Built Total:				3724.0				4851.
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	a X	_		Points
Adiacont	0.0	0.00	0.0	1.Exterior Wood				84.0		6.10		512
Adjacent Exterior	84.0	6.10	512.4	THE PROPERTY OF THE PROPERTY O								
LXterior	3.715			and the second of the				24.0				512
Base Total:	84.0		512.4	As-Built Total:	-		-	84.0				200 000000
CEILING TYPES	Area X	BSPM	= Points	Туре		R-Va	lue	Area X	SPM	XS	CM =	Points
		1.73	5297.3	1. Under Attic			30.0	3062.0	1.73)	(1.00		5297
Under Attic	3062.0	1.73	3237.0	, one or the								
Base Total:	3062.0		5297.3	As-Built Total:				3062.0				5297
		DCDM	- Doints	Туре		F	R-Valu	e Area	а Х	SP	M =	Points
FLOOR TYPES	Area X	BSPIN	= Points			territor e de la	11000			41.20	1	-8260
Slab	200.5(p)	-37.0	-7418.5	1. Slab-On-Grade Edge Ins			0.0	200.5(p 1006.0		0.77		779
Raised	1006.0	-3.99	-4013.9	2. Raised Wood, Adjacent			8.0	1000.0		0.77		
			44420 4	As-Built Total:				1206.5				-7481
Base Total:			-11432.4	AS-Duit Total.					.,	00	8.4	Doint
INFILTRATION	Area X	BSPM	= Points					Are	a X	SP	M =	Point
								3062	2.0	10.2	21	31263.
	3062.0	10.21	31263.0			- 70		0002	in the same of	THE REAL PROPERTY.	10°	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

	BASE		AS-BUILT						
Summer Bas	se Points: 4	1401.0	Summer As-Built Points:	42216.3					
	X System = Multiplier		Total X Cap X Duct X System X Cred Component Ratio Multiplier Multiplier Multip (System - Points) (DM x DSM x AHU)						
			(sys 1: Central Unit 42000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Att(AH),R(42216 0.64 (1.09 x 1.000 x 1.11) 0.260 0.95 (sys 2: Central Unit 24000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Att(AH),R(4,12) (sys 2: Central Unit 24000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Att(AH),Unc(R),Att(AH),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),Unc(R),U	50 8028.5					
41401.0	0.3250	13455.3	42216 0.36 (1.09 x 1.000 x 1.11) 0.260 0.95 42216.3 1.00 1.210 0.260 0.95	50 4587.7					

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

PERMIT #: ADDRESS: ,,,

BASE						AS-	BUI	LT				
GLASS TYPES		Callate Co.										
.18 X Condition Floor Are		/PM = 1	Points	Type/SC		erhang Len	Hgt	Area X	WP	мх	WOF	= Point
.18 3062.	0 2	20.17	11117.0	1.Double,U=0.87,Clear	W	10.0	7.0	36.0	20.7		1.20	895.0
.10			VI	2.Double,U=1.45,Clear	W	10.0	8.0	24.0	32.5		1.19	930.0
				3.Double,U=0.69,Clear	W	10.0	8.0	8.0	15.8		1.19	151.0 379.0
				4.Double,U=0.87,Clear	W	1.5	7.0	18.0	20.7		1.02	615.0
				5.Double,U=0.87,Clear	N	1.5	6.0	25.0 90.0	18.7		1.38	2330.
				6.Double,U=0.87,Clear	E	10.0	7.0	24.0	18.7		1.31	588.
				7.Double,U=0.87,Clear	E	7.5 8.0	7.0 7.0	18.0	23.5		1.05	444.
				8.Double,U=0.87,Clear	NE NE	8.0	7.0	18.0	23.		1.05	444.0
				9.Double,U=0.87,Clear	W	1.5	5.5	78.0	20.		1.03	1662.
				10.Double,U=0.87,Clear	VV	1.5	5.5	70.0	20.			
				As-Built Total:			_	339.0	-			8438.
WALL TYPES	Area X	BWPM	= Points	Туре		R	-Value	Are	аХ	WPN	1 =	Points
Adissent	816.0	3.60	2937.6	1. Frame, Wood, Exterior			13.0	2908.0		3.40		9887.
Adjacent Exterior	2908.0	3.70	10759.6	2. Frame, Wood, Adjacent			13.0	816.0		3.30		2692.
Exterior	2500.0	0.70										12580.
Base Total:	3724.0		13697.2	As-Built Total:				3724.0		-		
DOOR TYPES	Area X	BWPM	= Points	Туре				Area	a X	WPN	1 =	Points
Adiocent	0.0	0.00	0.0	1.Exterior Wood				84.0		12.30		1033
Adjacent Exterior	84.0	12.30	1033.2									
Exterior	04.0							121012				1033
Base Total:	84.0		1033.2	As-Built Total:				84.0			ocones	1033
CEILING TYPE	S Area X	BWPM	= Points	Туре		R-Valu	ie A	rea X	WPM	X W	CM =	Points
		2.05	6277.1	1. Under Attic			30.0	3062.0	2.05	X 1.00		6277
Under Attic	3062.0	2.05	0211.1	1. Olidor Antis								5000
Base Total:	3062.0		6277.1	As-Built Total:				3062.0		A		6277
		DIA/DIA	- Doints	Туре		F	R-Valu	e Are	ea X	WPI	M =	Points
FLOOR TYPES	Area X	RWHW	= Points			- Carrier		- Control		18.80	-	3769
Slab	200.5(p)	8.9	1784.4	1. Slab-On-Grade Edge Ins			0.0	200.5(p 1006.0		4.20		4225
Raised	1006.0	0.96	965.8	2. Raised Wood, Adjacent			8.0	1000.0		1.20		1907
STREET, AD			2750.2	As-Built Total:				1206.5				7994
Base Total:				As-Duilt Total.				Are	a X	WP	M =	Point
INFILTRATION	Area X	BWPN	l = Points						- North			-
	3062.0	-0.59	-1806.6					306	52.0	-0.5	9	-1806.

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

PERMIT #: ADDRESS: ,,,

BASE		AS-BUILT						
Winter Base Points:	33068.1	Winter As-Built Points: 34516.3						
Total Winter X System Points Multipli	= Heating er Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)						
33068.1 0.55	10 18319.7	(sys 1: Electric Heat Pump 42000 btuh ,EFF(8.5) Ducts:Unc(S),Unc(R),Att(AH),R6.0 34516.3 0.636 (1.069 x 1.000 x 1.10) 0.401 0.950 9843.7 (sys 2: Electric Heat Pump 24000 btuh ,EFF(8.3) Ducts:Unc(S),Unc(R),Att(AH),R6.0 34516.3 0.364 (1.069 x 1.000 x 1.10) 0.411 0.950 5760.5 34516.3 1.00 1.176 0.405 0.950 15602.2						

EnergyGauge® 4.5

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

PERMIT #: ADDRESS: ,,,

BASE				AS-BUILT								
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier	X Credit = Multiplier	Total
3		2635.00		7905.0	50.0	0.93	3		1.00	2606.67	1.00	7820.0
					As-Built To	otal:						7820.0

				CODE	C	OMPLI	ANCE	ST	TATUS	3			
BASE				AS-BUILT									
Cooling Points	+	Heating	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
13455		18320		7905		39680	12616		15602		7820		36038

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS	PERMIT #:
ADDRESS: ,,,	

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	-
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

	OFOTION	REQUIREMENTS	CHECK
COMPONENTS	SECTION	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir	1 .
Water Heaters	612.1	breaker (electric) or cutoff (gas) must be provided. External or built-in neat trap required.	-
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
LIVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
HVAC Controls	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.	
Insulation	004.1, 002.1	Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.2

The higher the score, the more efficient the home.

WALT & KRISSY SMITH, , , ,

	VV/ (E) G		500 MINISTER 1970 1970 1970 1970 1970 1970 1970 1970		
 New construction or existing Single family or multi-family Number of units, if multi-family Number of Bedrooms Is this a worst case? Conditioned floor area (ft²) Glass type¹ and area: (Label reqd. tag. U-factor: (or Single or Double DEFAULT) SHGC:	Description Area	a b c 13 b c 14 a.	Cooling systems Central Unit Central Unit N/A Heating systems Electric Heat Pump Electric Heat Pump N/A Hot water systems Electric Resistance	Cap: 42.0 kBtu/hr SEER: 13.00 Cap: 24.0 kBtu/hr SEER: 13.00 Cap: 42.0 kBtu/hr HSPF: 8.50 Cap: 24.0 kBtu/hr HSPF: 8.30 Cap: 50.0 gallons EF: 0.93	
c. N/A d. N/A e. N/A 10. Ceiling types a. Under Attic b. N/A c. N/A 11. Ducts(Leak Free) a. Sup: Unc. Ret: Unc. AH: Attic b. Sup: Unc. Ret: Unc. AH: Attic	R=30.0, 3062.0 ft ² Sup. R=6.0, 260.0 ft Sup. R=6.0, 120.0 ft		Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, MZ-H	
I certify that this home has comple Construction through the above e in this home before final inspection based on installed Code complian Builder Signature: Address of New Home:	nergy saving features which on. Otherwise, a new EPL in features.	Display Ca Date: City/FL	Zip:	COD WE THE STATE OF THE STATE O	OF FLORIDA
*NOTE: The home's estimated en	iergy perjormance score is		86 for a US FPA/DOE EnergySta	ır' ^m designation),	

*NOTE: The home's estimated energy performance score is only available inrough the FLA/RES compater programs. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.5)



RE: J0700223 -

Site Information:

Project Customer: RYE CONSTRUCTION Project Name: WALT SMITH RES

Lot/Block:

Subdivision:

Address: WALTER RD

City: LAKE CITY

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.5

Truss Name

Date

3/16/07

3/16/07

Wind Code: ASCE 7-02 Wind Speed: 120 mph

Floor Load: 55.0 psf

PB01

Roof Load: 47.0 psf

This package includes 19 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31,003, section 5 of the Florida Board of Professional Engineers Rules.

T2501091 PB02

Seal# T2501090

No.	Seal#	Truss Name	Date	No
1	T2501073	1A	3/16/07	18
2	T2501074	1AA	3/16/07	19
3	T2501075	1B	3/16/07	
4	T2501076	1BA	3/16/07	
5	T2501077	2A	3/16/07	
6	T2501078	2B	3/16/07	
7	T2501079	3A	3/16/07	
8	T2501080	3B	3/16/07	
9	T2501081	4A	3/16/07	
10	T2501082	4B	3/16/07	
11	T2501083	6A	3/16/07	
12	T2501084	6B	3/16/07	
13	T2501085	A01	3/16/07	
14	T2501086	A01A	3/16/07	
15	T2501087	A04	3/16/07	
16	T2501088	A06	3/16/07	
17	T2501089	FL	3/16/07	

The truss drawing(s) referenced above have been prepared by Robbins Engineering, Inc. under my direct supervision based on the parameters provided by HD Supply-Ocala, FL.

Truss Design Engineer's Name: ORegan, Philip

My license renewal date for the state of Florida is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

B

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert #5555

March 16,2007

6904 Parke East Boulevard Tampa, FL 33610-4115

ORegan, Philip

Phone: 813-972-1135 Fax: 813-971-6117 www.robbinseng.com

DALLAS • TAMPA • FT. WORTH

Truss Type Qty Ply Job Truss T2501073 J0700223 1A **ROOF TRUSS** 20 Job Reference (optional) HD SUPPLY LBM, OCALA, FL. 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Mar 16 15:12:19 2007 Page 1 15-0-11 | 19-9-6 | 24-6-0 | 29-10-0 | 34-6-10 | 39-3-5 | 43-11-15 | 48-7-8 | 54-4-0 55-6-8 -1-2₁8 5-8-8 10-4-1 4-8-10 1-2-8 5-8-8 4-8-10 2x4 RL20 || Scale = 1:143.3 3x8 RL 20 = 7x8 RL20 = 7x8 RL20 = 6x6 RL20 = 6x6 RL 20 = 7.00 12 8 10 11 3x6 RL20 II 3x6 RL20 | 12 4x8 RL20 / 4x8 RL20 > 3.00 12 14 6x6 RL20 = 6x6 RL20 = 15 3x4 RL20 = 3x4 RL20 = 16 2-10-14 2-10-14 17 18 4 1 2 5x6 RL20 = 5x6 RL20 = 26 25 24 23 5x5 RL20 = 5x5 RL20 = 22 21 20 19 3x6 RL20 || 6x8 RL20 = 5x5 RL20 = 8x8 Rt 20 = 5x8 RL20 = 5x8 RL20 = 5x10 RL20 = 5x5 RL20 = 6x8 RL20 = 3x6 RL20 || 5x5 RL20 = 5x5 RL20 = 10-4-1 48-7-8 8x8 RL20 = 5x10 RL20 = 53-11-12 10-2-0 17-5-0 24-6-0 29-10-0 36-11-0 43-11-15 44-2-0 0-4-4 5-8-8 5-4-0 5-4-4 0-4-4 7-1-0 7-1-0 7-1-0 7-1-0 0-2-1 0-4-4 5-4-4 4-5-8 0-2-1 Plate Offsets (X,Y): [2:0-8-3,0-0-8], [7:0-5-14,0-0-3], [8:0-3-0,0-1-12], [11:0-3-0,0-1-12], [12:0-5-14,0-0-3], [17:0-8-3,0-0-8], [21:0-1-8,0-4-0], [24:0-1-8,0-4-0] LOADING (psf) SPACING CSI DEFL L/d **PLATES** GRIP 2-0-0 in (loc) I/def 253/171 TC Vert(LL) -0.76 21-24 >533 360 RL20 TCLL Plates Increase 1.25 0.61 20.0 -1.08 21-24 0.59 >378 180 1.25 BC TCDL 70 Lumber Increase Vert(TL) BCLL 10 0 Rep Stress Incr YES WB 0.96 Horz(TL) 0.10 20 n/a n/a Weight: 470 lb BCDL 10.0 Code FBC2004/TPI2002 (Matrix) LUMBER BRACING TOP CHORD 2 X 4 SYP No.2D *Except* TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins. Except: 4-8 2 X 8 SYP SS, 11-15 2 X 8 SYP SS 4-3-0 oc bracing: 7-9 BOT CHORD 2 X 8 SYP SS * 4-7-0 oc bracing: 9-12 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 4-8-10 oc bracing. WEBS WEBS 2 Rows at 1/3 pts 5-25, 14-20 JOINTS 1 Brace at Jt(s): 9 REACTIONS (lb/size) 25=3143/0-4-0, 20=3246/0-4-0, 2=150/0-4-0, 17=107/0-4-0 Max Horz 2=221(LC 4) Max Uplift25=-690(LC 5), 20=-683(LC 6), 2=-461(LC 3), 17=-452(LC 4) Max Grav 25=3143(LC 1), 20=3246(LC 1), 2=172(LC 2), 17=172(LC 2) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/22, 2-3=-126/1076, 3-4=-210/1430, 4-5=-135/1593, 5-6=-2646/555, 6-7=-2247/634, 7-8=-267/678, 8-10=-486/795, TOP CHORD 10-11=-487/795, 11-12=-405/671, 12-13=-2283/629, 13-14=-2614/544, 14-15=-92/1593, 15-16=-180/1430, 16-17=-80/1030, 17-18=0/22, 7-27=-2031/426, 9-27=-2037/425, 9-28=-1872/412, 12-28=-1864/414 **BOT CHORD** 2-26=-950/41, 25-26=-950/41, 24-25=-277/1807, 23-24=-351/1964, 22-23=-339/2047, 21-22=-343/1965, 20-21=-324/1767, 19-20=-950/108, 17-19=-950/108 3-26=0/180, 3-25=-745/342, 4-25=0/369, 5-25=-4287/573, 5-24=-172/657, 14-21=-152/766, 14-20=-4460/547, 15-20=0/358, WEBS 16-20=-734/335, 16-19=0/180, 6-24=0/858, 13-21=0/858, 9-10=-48/28, 8-9=-365/531, 9-11=-363/343 **NOTES** 1) This truss has been checked for uniform roof live load only, except as noted. 2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00. Provide adequate drainage to prevent water ponding. *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection. 6) Ceiling dead load (5.0 psf) on member(s). 6-7, 12-13, 7-9, 9-12 Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 21-24 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 690 lb uplift at joint 25, 683 lb uplift at joint 20, 461 lb uplift at joint 2 and 452 lb uplift at joint 17.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.





Job Truss Type Qty Ply Truss T2501074 6 ROOF TRUSS J0700223 1AA Job Reference (optional) HD SUPPLY LBM, OCALA, FL. 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Mar 16 15:12:21 2007 Page 1 24-6-0 | 29-10-0 | 34-6-10 | 39-3-5 | 43-11-15 | 48-7-8 54-4-0 55-6-8 -1-278 5-8-8 10-4-1 15-0-11 19-9-6 4-8-10 4-8-10 4-8-10 4-8-10 5-8-8 1-2-8 5-8-8 4-8-10 4-8-10 5-4-0 Scale = 1:143.3 2x4 RL20 11 3x8 RL20= 7x8 RL20 = 6x6 RL20 = 7x8 RL20 = 6x6 RL20 = 7.00 12 10 11 3x8 RL20 || 3x8 RL20 || 12 4x12 RL20 / 4x12 RL20 > 3.00 12 14 6x6 RL20 = 6x6 RL20 = 15 3x4 RL20 = 3x4 RL20 = 2-10-14 0-3-14 5x6 RL20 = 5x6 RL20 = 23 5x5 RL20 = 5x5 RL20 = 22 20 19 3x6 RL20 11 6x8 RL20 = 5x5 RL20 = 8x8 RL20 = 8x8 RL20 = 8x8 RL20 = 5x10 RL20 = 5x5 RL20 = 6x8 RL20 = 3x6 RL20 || 5x5 RL20 = 5x5 RL20 = 10-4-1 48-7-8 54-4-0 8x8 RL20 = 5x10 RL20 = 53-11-12 H 10-2-0 16-10-8 17-5-0 24-6-0 29-10-0 36-11-0 43-11-15 44-2-0 4-5-8 0-4-4 5-4-4 6-6-7 0-6-8 7-1-0 5-4-0 7-1-0 7-1-0 0-2-1 5-4-4 0-4-4 0-2-1 Plate Offsets (X,Y): [2:0-8-3,0-0-8], [7:0-5-14,0-0-3], [8:0-3-0,0-1-12], [11:0-3-0,0-1-12], [12:0-5-14,0-0-3], [17:0-8-3,0-0-8], [21:0-1-8,0-3-12], [24:0-1-8,0-3-12] GRIP LOADING (psf) CSI DEFL in (loc) PLATES SPACING 2-0-0 I/defl 1 /d TCLL 20.0 Plates Increase 1.25 TC 0.78 Vert(LL) -0.77 21-24 >529 360 RI 20 253/171 TCDL 7.0 1.25 BC 0.69 Vert(TL) -1.11 21-24 >369 180 Lumber Increase NO WB 0.97 0.10 BCLL 10.0 Rep Stress Incr Horz(TL) 20 n/a n/a BCDL Code FBC2004/TPI2002 (Matrix) Weight: 474 lb 10.0 LUMBER BRACING TOP CHORD 2 X 4 SYP No.2D *Except* TOP CHORD Structural wood sheathing directly applied or 4-8-1 oc purlins. Except: 4-2-0 oc bracing: 7-9 4-8 2 X 8 SYP SS, 11-15 2 X 8 SYP SS 4-4-0 oc bracing: 9-12 BOT CHORD 2 X 8 SYP SS ** **BOT CHORD** Rigid ceiling directly applied or 4-8-6 oc bracing. WEBS 2 X 4 SYP No.3 *Except* 5-26 2 X 4 SYP No.2D, 14-20 2 X 4 SYP No.2D WERS 1 Row at midpt 5-26, 14-20 JOINTS 1 Brace at Jt(s): 9 REACTIONS (lb/size) 26=3693/0-4-6 (0-4-0 + bearing block), 20=3268/0-4-0, 2=-17/0-4-0, 17=116/0-4-0 Max Horz 2=221(LC 4) Max Uplift26=-890(LC 5), 20=-684(LC 6), 2=-396(LC 3), 17=-473(LC 4) Max Grav 26=3693(LC 1), 20=3268(LC 1), 2=99(LC 2), 17=181(LC 2) FORCES (lb) - Maximum Compression/Maximum Tension $1-2=0/22,\ 2-3=-407/1319,\ 3-4=-462/1932,\ 4-5=-445/2150,\ 5-6=-2803/624,\ 6-7=-2309/665,\ 7-8=-218/642,\ 8-10=-423/752,\ 1-2=0/22,\ 2-3=-407/1319,\ 3-4=-462/1932,\ 4-5=-445/2150,\ 5-6=-2803/624,\ 6-7=-2309/665,\ 7-8=-218/642,\ 8-10=-423/752,\ 1-2=0/2$ TOP CHORD 10-11=-423/751, 11-12=-334/624, 12-13=-2355/663, 13-14=-2725/596, 14-15=-54/1659, 15-16=-146/1482, 16-17=-93/1112, 17-18=0/22, 7-28=-2160/492, 9-28=-2166/490, 9-29=-2040/492, 12-29=-2032/494 2-27=-1276/206, 26-27=-1276/206, 25-26=-305/1823, 24-25=-305/1823, 23-24=-403/2043, 22-23=-392/2124, **BOT CHORD** 21-22=-394/2040, 20-21=-386/1848, 19-20=-1008/81, 17-19=-1008/81 3-27=0/190, 3-26=-771/322, 4-26=0/447, 5-26=-5292/913, 5-24=-238/825, 14-21=-131/756, 14-20=-4602/560, 15-20=0/387, WEBS 16-20=-803/335, 16-19=0/200, 6-24=-14/938, 13-21=0/887, 9-10=-53/32, 8-9=-359/517, 9-11=-380/375 NOTES 1) 2 X 8 SYP SS bearing block 12" long at jt. 26 attached to front face with 4 rows of 10d (0.148"x3") nails spaced 3" o.c. 16 Total fasteners. Bearing is assumed to be SYP. This truss has been checked for uniform roof live load only, except as noted. 3) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00. Provide adequate drainage to prevent water ponding. *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection. 7) Ceiling dead load (5.0 psf) on member(s). 6-7, 12-13, 7-9, 9-12 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 21-24 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 890 lb uplift at joint 26, 684 lb uplift at joint 20, 396 lb uplift at joint 2 and 473 lb uplift at joint 17. Philip J. O'Regan, FL Lic. #58126 10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails. Robbins Engineering 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 415 lb down and 201 lb up at 6904 Parke East Blvd 16-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. Tampa, FL, 33610 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). FL Cert.#5555 March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

LOAD CASE(S) Standard Continued on page 2

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



6904 Parke East Blvd. Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply		5-00-00
J0700223	1AA	ROOF TRUSS	6	1		T2501074
HD SUPPLY LBM, OCALA	, FL.			6.500 s Mar 8 2	ence (optional) 2007 MiTek Industries, Inc. Fri	Mar 16 15:12:21 2007 Page 2
Uniform Loads (olf)	ase=1.25, Plate Increase=1. 4-6=-54, 6-7=-64, 7-8=-54, 8	25 3-11=-54, 11-12=-94, 12-13=-64, 1	13-15=-54, 15-18=-54, :	2-24=-20, 23-24=-1	00, 22-23=-100, 21-22=-100,	17-21=-20, 7-29=-64,
						-
						-
						-
						4



Qty Ply Job Truss Truss Type T2501075 1B GABLE J0700223 Job Reference (optional) HD SUPPLY LBM, OCALA, FL. 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Mar 16 15:12:24 2007 Page 1 -1-2-8 15-0-11 19-9-6 24-6-0 29-10-0 34-6-10 39-3-5 43-11-15 48-7-8 54-4-0 55-6-8 5-8-8 10-4-1 5-8-8 4-8-10 4-7-8 1-2-8 Scale = 1:105.5 5x5 RL20 = 5x5 RL20 = 8 9 7.00 12 3x4 RL20 / 3x4 RL20 > 12 3x4 RL20 / 3x4 RL20 > 3x4 RL20 || 3.00 12 5x6 RL20 = 3x4 RI 20 = 3x4 RL20 = 3x4 RL20 = 3x4 RL20 = 15 16 5x8 RL20 II 5x8 RL20 || 32 28 26 25 24 23 21 20 34 33 3x4 RL20 = 3x6 RL20 = 3x4 RL20 = 5x5 RL20 = 3x4 RL20 = 3x4 RL20 = 3x8 RL20 = 3x8 RL20= 3x6 RL20 = 3x10 RL20 = 5-8-8 10-2-0 10-4-1 17-5-0 24-6-0 29-10-0 36-11-0 43-11-15 48-7-8 54-4-0 4-7-8 5-8-8 5-8-8 4-5-8 0-2-1 7-1-0 7-1-0 5-4-0 7-1-0 7-1-0 Plate Offsets (X,Y): [2:0-3-8,Edge], [7:0-1-12,0-0-4], [8:0-2-8,0-2-1], [9:0-2-8,0-2-1], [14:0-3-15,0-2-7], [17:0-6-12,Edge], [17:0-3-8,Edge], [32:0-2-8,0-0-4], [34:0-2-0,0-0-5], [52:0-2-0,0-5], [52:0-2-0,0-5], [52:0-2-0,0-5], [52:0-2-0,0-5], [52 0-2-0] LOADING (psf) CSI DEFL PLATES GRIP SPACING 2-0-0 in (loc) I/defl L/d 0.12 31-33 253/171 TCLL 20.0 Plates Increase 1.25 TC 0.39 Vert(LL) >999 360 RL₂₀ BC 0.38 -0.27 31-33 TCDL 7.0 Lumber Increase 1 25 Vert(TL) >999 180 WB BCLL 10.0 Rep Stress Incr NO 0.53 Horz(TL) 0.05 20 n/a n/a BCDL Code FBC2004/TPI2002 (Matrix) Weight: 471 lb 10.0 LUMBER BRACING TOP CHORD 2 X 4 SYP No.2D * TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Except: BOT CHORD 2 X 4 SYP No.2D 9-10-0 oc bracing: 10-11 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing, Except: WEBS 6-0-0 oc bracing: 19-20,17-19. 2 X 4 SYP No.3 **OTHERS** WEBS 6-34, 7-33, 7-31, 12-24 1 Row at midpt JOINTS. 1 Brace at Jt(s): 30, 29 REACTIONS (lb/size) 2=338/0-4-0, 17=301/21-8-0, 34=1770/0-4-0, 24=407/21-8-0, 20=776/21-8-0, 19=552/21-8-0, 27=3/21-8-0, 25=70/21-8-0, 23=38/21-8-0, 22=42/21-8-0, 21=37/21-8-0 Max Horz 2=219(LC 4) Max Uplift2=-359(LC 3), 17=-251(LC 4), 34=-587(LC 5), 24=-579(LC 3), 20=-292(LC 5), 19=-241(LC 4), 27=-21(LC 3), 25=-24(LC 4) Max Grav 2=338(LC 1), 17=301(LC 1), 34=1770(LC 1), 24=407(LC 1), 20=776(LC 1), 19=552(LC 1), 27=97(LC 2), 25=70(LC 1), 23=66(LC 2), 22=86(LC 2), 21=70(LC 2) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/25, 2-3=-182/468, 3-4=-176/479, 4-5=-311/677, 5-6=-270/776, 6-7=-1055/386, 7-8=-1025/423, 8-9=-836/419, TOP CHORD 9-12=-1005/434, 12-13=-858/484, 13-14=-387/226, 14-15=-298/161, 15-16=-97/61, 16-17=-102/22, 17-18=0/25, 10-11=-5/16 **BOT CHORD** 2-35=-179/88, 34-35=-179/88, 33-34=-187/721, 32-33=-194/900, 31-32=-194/900, 28-31=-147/821, 27-28=-192/790, 26-27=-192/790, 25-26=-192/790, 24-25=-192/790, 23-24=-206/708, 22-23=-206/708, 21-22=-206/708, 20-21=-206/708, 19-20=-22/126, 17-19=-22/126, 29-30=-1/2 4-35=0/88, 4-34=-722/448, 5-34=-69/148, 6-34=-1933/600, 6-33=0/327, 7-33=-141/69, 7-31=-148/214, 30-31=-100/205, WEBS 10-30=-75/270, 8-10=-75/270, 28-29=-108/201, 11-29=-83/226, 9-11=-83/226, 12-28=-126/143, 12-24=-374/384, 13-24=-134/218, 13-20=-617/260, 14-20=-341/215, 15-20=-65/290, 15-19=-434/293 NOTES 1) This truss has been checked for uniform roof live load only, except as noted. 2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00. 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002. 4) Provide adequate drainage to prevent water ponding. 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. Philip J. O'Regan, FL Lic. #58126 6) All plates are 2x4 RL20 unless otherwise indicated. Robbins Engineering 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection. 6904 Parke East Blvd Gable studs spaced at 2-0-0 oc. Tampa, FL, 33610 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 359 lb uplift at joint 2, 251 lb uplift at joint 17,

WARNING - Verify design parameters and READ NOTES ON THIS

25 Continued on page 2

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

587 Ib uplift at joint 34, 579 Ib uplift at joint 24, 292 Ib uplift at joint 20, 241 Ib uplift at joint 19, 21 Ib uplift at joint 27 and 24 Ib uplift at joint



March 16,2007

6904 Parke East Blvd Tampa, FL 33610

FL Cert.#5555

Job	Truss	Truss Type	Qty	Ply		parameter and the second
J0700223	18	GABLE	1	1		T2501075
HD SUPPLY LBM, OCALA,				6,500	Job Reference (optional) s Mar 8 2007 MiTek Industries, Inc.	Fri Mar 16 15:12:24 2007 Page
NOTES	*					onisana makananan 19.1
10) Design assumes 4x2	(flat orientation) purlins at oc	spacing indicated, fastened to truss TC w/ 2-	10d nails	•		
LOAD CASE(S) Standard	d					
						-)
					ű.	

Truss Type Qty Ply Job Truss T2501076 GABLE J0700223 1BA Job Reference (optional) 6.500 s Mar. 8.2007 MiTek Industries, Inc., Fri Mar. 16.15:12:27.2007, Page 1 HD SUPPLY LBM, OCALA, FL. -1-2-8 43-11-15 48-7-8 54-4-0 55-6-8 15-0-11 19-9-6 24-6-0 29-10-0 34-6-10 39-3-5 4-8-10 4-8-10 4-8-10 4-7-8 5-8-8 1-2-8 4-7-8 4-8-10 4-8-10 4-8-10 5-4-0 Scale = 1:105.5 5v5 RI 20 = 5x5 RL20 = 8 9 7.00 12 3x4 RL20 / 3x4 RL20 > 3x4 RL20 / 3x4 RL20 > 3x4 RL20 || 3.00 12 5x5 RL 20 = 5x5 RL20 = 8-10-6 d 3x4 RL20 = 3x4 RL20 = 3x4 RL20 = 3x4 RL20 = 15 16 5x8 RL20 || 5x8 RL20 || 26 25 24 23 37 35 29 28 21 20 3x6 RL20 = 3x4 RL20 = 3x8 RL20 = 3x8 RL20 = 5x5 RL20 = 3x4 RL20 = 3x4 RL20 = 3x4 RL20 = 3x10 RL20 = 3x6 RL20 = 36-11-0 43-11-15 48-7-8 54-4-0 24-6-0 29-10-0 5-8-8 10-2-0 10-4-1 17-5-0 7-1-0 7-1-0 4-5-8 0-2-1 7-1-0 7-1-0 5-4-0 5-8-8 Plate Offsets (X,Y): [2:0-3-8,Edge], [7:0-1-12,0-0-4], [8:0-2-8,0-2-1], [9:0-2-8,0-2-1], [14:0-3-15,0-2-7], [17:0-6-12,Edge], [17:0-3-8,Edge], [35:0-2-8,0-0-4], [37:0-2-0,0-0-5], [53:0-2-0,0-5], [53:0-2-0,0-5], [53:0-2-0,0-5], [53:0-2-0,0-5], [53 GRIP LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d PLATES 0.08 34-36 253/171 1.25 TC 0.39 Vert(LL) >999 360 RL20 TCLL 20.0 Plates Increase BC -0.22 34-36 180 7.0 Lumber Increase 1.25 0.69 Vert(TL) >837 TCDL Rep Stress Incr WB 0.36 0.03 n/a NO BCLL 10.0 Horz(TL) n/a Weight: 471 lb Code FBC2004/TPI2002 BCDL 100 (Matrix) BRACING LUMBER TOP CHORD 2 X 4 SYP No.2D * TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Except: **BOT CHORD** 2 X 4 SYP No.2D 6-0-0 oc bracing: 10-11 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing, Except: **WEBS OTHERS** 2 X 4 SYP No.3 6-0-0 oc bracing: 19-20,17-19. WEBS 1 Row at midpt 6-37, 7-34, 8-32, 12-24 JOINTS 1 Brace at Jt(s): 32 REACTIONS (lb/size) 2=375/0-4-0, 17=291/29-0-0, 37=1387/0-4-0, 29=47/29-0-0, 24=551/29-0-0, 20=506/29-0-0, 19=479/29-0-0, 30=-237/29-0-0, 33=598/29-0-0, 28=24/29-0-0, 27=46/29-0-0, 25=33/29-0-0, 23=33/29-0-0, 22=42/29-0-0, 21=37/29-0-0, 11=71/29-0-0, 31=50/29-0-0 Max Horz 2=219(LC 4) Max Uplift2=-330(LC 3), 17=-245(LC 4), 37=-462(LC 5), 29=-125(LC 6), 24=-268(LC 6), 20=-230(LC 6), 19=-199(LC 4), 30=-237(LC 1), 33=-205(LC 5), 11=-203(LC 3) Max Grav 2=375(LC 1), 17=291(LC 1), 37=1387(LC 1), 29=83(LC 4), 24=551(LC 1), 20=506(LC 1), 19=479(LC 1), 30=107(LC 5), 33=598(LC 1), 28=74(LC 2), 27=84(LC 2), 25=69(LC 2), 23=66(LC 2), 22=86(LC 2), 21=70(LC 2), 11=71(LC 1), 31=101(LC 2) FORCES (lb) - Maximum Compression/Maximum Tension $1-2=0/25,\ 2-3=-282/329,\ 3-4=-258/340,\ 4-5=-237/520,\ 5-6=-168/600,\ 6-7=-676/279,\ 7-8=-512/270,\ 8-9=-381/284.$ TOP CHORD $9-12=-504/288,\ 12-13=-321/178,\ 13-14=-163/96,\ 14-15=-103/48,\ 15-16=-56/114,\ 16-17=-61/75,\ 17-18=0/25,\ 10-11=-5/10$ 2-38=-113/252, 37-38=-113/252, 36-37=-139/464, 35-36=-119/520, 34-35=-119/520, 33-34=-57/384, 30-33=-57/384, 30-35=-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-57/384, 30-**BOT CHORD** 29-30=-57/384, 28-29=-4/342, 27-28=-4/342, 26-27=-4/342, 25-26=-4/342, 24-25=-4/342, 23-24=0/283, 22-23=0/283, 21-22=0/283, 20-21=0/283, 19-20=-74/97, 17-19=-74/97, 31-32=-0/1 4-38=0/90, 4-37=-750/434, 5-37=-114/169, 6-37=-1341/396, 6-36=-30/158, 7-36=-29/120, 7-34=-255/248, 32-34=-81/81, WEBS 10-32=-31/107, 8-10=-31/107, 29-31=0/0, 11-31=0/0, 9-11=-71/203, 12-29=-99/164, 12-24=-422/125, 13-24=-150/210, 13-20=-283/72, 14-20=-276/220, 15-20=0/148, 15-19=-362/252 NOTES 1) This truss has been checked for uniform roof live load only, except as noted. 2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00. 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002. Philip J. O'Regan, FL Lic. #58126 4) Provide adequate drainage to prevent water ponding. Robbins Engineering 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6904 Parke East Blvd 6) All plates are 2x4 RL20 unless otherwise indicated. Tampa, FL, 33610 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection. FL Cert.#5555 Gable studs spaced at 2-0-0 oc. March 16,2007 Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

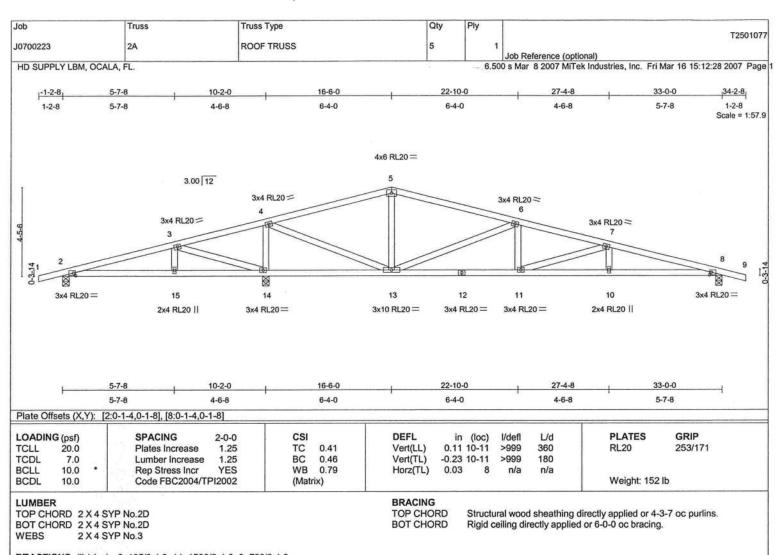
Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Design Valid for use only with whitek or koolins controlled is a design to great a design of the controlled in the contr



6904 Parke East Blvd. Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply		
	L	ALLO SATINGS OF SATINGS	1	1		T2501076
J0700223	1BA	GABLE	1		Job Reference (optional) 0 s Mar 8 2007 MiTek Industries, Inc. Fri Mar 16 15:12:27 2	2 700
 Provide mechanical of ioint 29, 268 lb uplift a 	31 considers parallel to grai connection (by others) of tru at joint 24, 230 lb uplift at jo	ss to bearing plate capable of withstar	nding 330 lb uplift plift at joint 30, 20	ng designer at joint 2, 2 5 lb uplift a	r should verify capacity of bearing surface. 245 lb uplift at joint 17, 462 lb uplift at joint 37, 125 lb uplift at joint 33 and 203 lb uplift at joint 11.	
LOAD CASE(S) Standar	d					

4 Robbins



REACTIONS (lb/size) 2=185/0-4-0, 14=1582/0-4-0, 8=799/0-4-0

Max Horz 2=-79(LC 4)

Max Uplift2=-152(LC 3), 14=-463(LC 5), 8=-347(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/15, 2-3=-119/309, 3-4=-224/1000, 4-5=-579/261, 5-6=-578/252, 6-7=-1499/575, 7-8=-2117/755, 8-9=0/15

BOT CHORD 2-15=-261/217, 14-15=-261/217, 13-14=-939/353, 12-13=-446/1428, 11-12=-446/1428, 10-11=-672/2012, 8-10=-672/2012

3-15=0/203, 3-14=-711/269, 4-14=-1253/433, 4-13=-452/1578, 5-13=-97/122, 6-13=-976/412, 6-11=-20/356, 7-11=-612/238, WEBS

7-10=0/194

NOTES

This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 2, 463 lb uplift at joint 14 and 347 lb uplift at joint 8.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



6904 Parke East Blvd. Tampa, FL 33610

Qty Truss Type Ply Job Truss T2501078 J0700223 2B **ROOF TRUSS** Job Reference (optional) 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Mar 16 15:12:30 2007 Page 1 HD SUPPLY LBM, OCALA, FL. 16-6-0 34-2-8 -1-2-8 1-2-8 16-6-0 16-6-0 1-2-8 Scale = 1:60.0 4x4 RL20= 3.00 12 10 11 9 12 14 15 3x4 RL20= 3x4 RL20 = 35 ₁₉ [29 28 27 26 2524 23 22 21 20 4x8 RL20 || 4x8 RL20 || 32 31 30 33 3x4 RL20 = 3x4 RL20 = 3x4 RL20 = 33-0-0 33-0-0 Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-6-12,Edge], [18:0-3-8,Edge], [18:0-6-12,Edge] GRIP SPACING CSI DEFL L/d **PLATES** LOADING (psf) 2-0-0 in (loc) I/defl TC 0.28 Vert(LL) 0.00 180 RL20 253/171 1.25 19 TCLL 20.0 Plates Increase n/r BC 0.00 120 TCDL 7.0 Lumber Increase 1.25 0.10 Vert(TL) 19 n/r WB 0.05 0.00 BCLL 10.0 Rep Stress Incr NO Horz(TL) 18 n/a n/a Code FBC2004/TPI2002 Weight: 149 lb BCDL 10.0 (Matrix)

LUMBER

TOP CHORD 2 X 4 SYP No.2D **
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

Nigit ceiling directly applied of 0-0-0 oc bracing

REACTIONS (lb/size) 2=277/33-0-0, 18=277/33-0-0, 27=154/33-0-0, 28=148/33-0-0, 29=149/33-0-0, 30=145/33-0-0, 31=160/33-0-0, 32=101/33-0-0, 33=272/33-0-0, 26=148/33-0-0, 24=149/33-0-0, 23=145/33-0-0, 21=101/33-0-0, 20=272/33-0-0

Max Horz 2=-83(LC 4)

Max Uplift2=-188(LC 5), 18=-194(LC 6), 28=-61(LC 3), 29=-62(LC 5), 30=-61(LC 3), 31=-61(LC 5), 32=-62(LC 3), 33=-70(LC 5), 26=-61(LC 4), 24=-62(LC 6), 23=-61(LC 4), 22=-61(LC 6), 21=-60(LC 4), 20=-73(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-34=0/10, 2-34=0/25, 2-3=-68/29, 3-4=-65/42, 4-5=-35/57, 5-6=-13/70, 6-7=0/86, 7-8=0/101, 8-9=0/117, 9-10=0/132,

10-11=0/129, 11-12=0/105, 12-13=0/81, 13-14=0/58, 14-15=0/33, 15-16=-4/22, 16-17=-24/42, 17-18=-28/14, 18-35=0/25,

19-35=0/10

2-33=-14/75, 32-33=-14/75, 31-32=-14/75, 30-31=-14/75, 29-30=-14/75, 28-29=-14/75, 27-28=-14/75, 26-27=-14/75,

25-26=-14/75, 24-25=-14/75, 23-24=-14/75, 22-23=-14/75, 21-22=-14/75, 20-21=-14/75, 18-20=-14/75

WEBS 10-27=-114/20, 9-28=-108/81, 8-29=-108/82, 7-30=-106/81, 6-31=-115/84, 5-32=-79/72, 4-33=-192/111, 11-26=-108/81,

12-24=-108/82, 13-23=-106/80, 14-22=-115/84, 15-21=-79/71, 16-20=-192/115

NOTES

BOT CHORD

1) This truss has been checked for uniform roof live load only, except as noted.

 Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 RL20 unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Gable requires continuous bottom chord bearing.

8) Gable studs spaced at 2-0-0 oc.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 2, 194 lb uplift at joint 18, 61 lb uplift at joint 28, 62 lb uplift at joint 29, 61 lb uplift at joint 30, 61 lb uplift at joint 31, 62 lb uplift at joint 32, 70 lb uplift at joint 33, 61 lb uplift at joint 26, 62 lb uplift at joint 24, 61 lb uplift at joint 23, 61 lb uplift at joint 26, 62 lb uplift at joint 27, 61 lb uplift at joint 28, 61 lb uplift at joint 29.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cett.#5555

March 16,2007

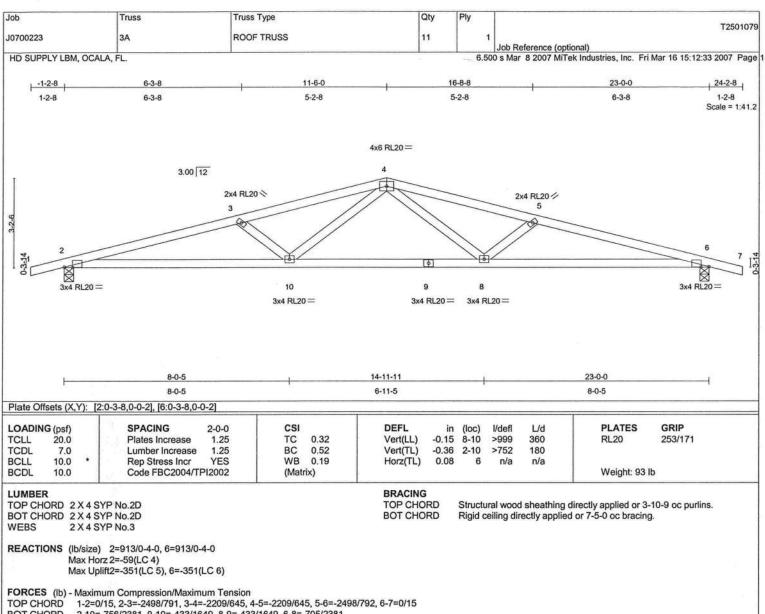
WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing insures stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult — ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Madison, WI 53719.



6904 Parke East Blvd. Tampa, FL 33610



BOT CHORD 2-10=-756/2381, 9-10=-433/1649, 8-9=-433/1649, 6-8=-705/2381

WEBS 3-10=-353/269, 4-10=-154/598, 4-8=-154/598, 5-8=-353/269

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 2 and 351 lb uplift at joint

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type			Qty	Ply				T2501080
J0700223	3B	ROOF TRUSS			1	1	lab Deference (aution	=0		
HD SUPPLY LBM, OC	CALA, FL.					6.50	Job Reference (option 0 s Mar 8 2007 MiTek		Mar 16 15:12:34	2007 Page
1-1-2-8		11-6-0					23-0-0		24-2-8	3
1-2-8		11-6-0		,			11-6-0		1-2-8 Sc	cale = 1:43.8
				4x4 RL20=						
		3.00 12	2x4 RL20	7	2x4 RL20 8		n no II			- 1
1		2x4 RL20 2x4 RL20 5	6	4			9 2x4 RL20			- 1
12	3x4 RL20=	4 A	0				10	3x4 RL20 =		1
2-10-12	3	9			- 11			11		
\$ 22 2 \$ 20 2	0 10						(a)	- B-	12	23 13 7
X			***************************************	********	******	*****			XXXXXXXX	
*	4x8 RL20	21 20 2x4 RL20 2x4 RL20	19 2x4 RL20	18 2x4 RL20	17 2x4 RL20		6 15 14 3x4 RL20=	4x8 R	3x4 RL20=	*
3x4	RL20=	2X4 RL20 2X4 RL20	2X4 KL20	2X4 RL20 []	2X4 NL20		RL20 2x4 RL20		3X4 NL20 —	
										- 1
										1
_				23-0-0						
i.				23-0-0						
Plate Offsets (X,Y):	[2:0-3-8,Edge], [2:0-6-12	2,Edge], [12:0-3-8,Edge], [12	:0-6-12,Edge]					-1163-1-1-1		
LOADING (psf)	SPACING	2-0-0 CSI		DEFL	in	(loc)	I/defl L/d	PLATES	GRIP	

0.01

0.01

0.00

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

BOT CHORD

13

13

12

n/r

n/r

n/a

180

120

n/a

Rigid ceiling directly applied or 6-0-0 oc bracing.

LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2D *

BOT CHORD 2 X 4 SYP No.2D

20.0

7.0

10.0

2 X 4 SYP No.3 **OTHERS**

REACTIONS (lb/size) 2=287/23-0-0, 12=287/23-0-0, 18=175/23-0-0, 19=176/23-0-0, 20=47/23-0-0, 21=362/23-0-0, 17=176/23-0-0, 16=47/23-0-0, 14=362/23-0-0

Max Horz 2=-62(LC 4)

Max Uplift2=-193(LC 5), 12=-198(LC 4), 18=-25(LC 5), 19=-68(LC 3), 20=-41(LC 3), 21=-112(LC 5), 17=-67(LC 4), 16=-40(LC 6), 14=-114(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

1-22=0/10, 2-22=0/25, 2-3=-65/84, 3-4=-59/121, 4-5=-17/86, 5-6=-1/102, 6-7=0/98, 7-8=0/98, 8-9=0/102, 9-10=0/86, TOP CHORD

TC

BC

WB

(Matrix)

0.28

0.16

0.06

10-11=-30/121, 11-12=-36/84, 12-23=0/25, 13-23=0/10

1.25

1.25

NO

2-21=-83/85, 20-21=-83/85, 19-20=-83/85, 18-19=-83/85, 17-18=-83/85, 16-17=-83/85, 15-16=-83/85, 14-15=-83/85, **BOT CHORD**

12-14=-83/85

7-18=-140/44, 6-19=-126/91, 5-20=-43/49, 4-21=-252/156, 8-17=-126/91, 9-16=-43/48, 10-14=-252/158 WEBS

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

Gable requires continuous bottom chord bearing.

- Gable studs spaced at 2-0-0 oc.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2, 198 lb uplift at joint 12, 25 lb uplift at joint 18, 68 lb uplift at joint 19, 41 lb uplift at joint 20, 112 lb uplift at joint 21, 67 lb uplift at joint 17, 40 lb uplift at joint 16 and 114 lb uplift at joint 14.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

253/171

RL20

Structural wood sheathing directly applied or 10-0-0 oc purlins.

Weight: 96 lb

March 16,2007

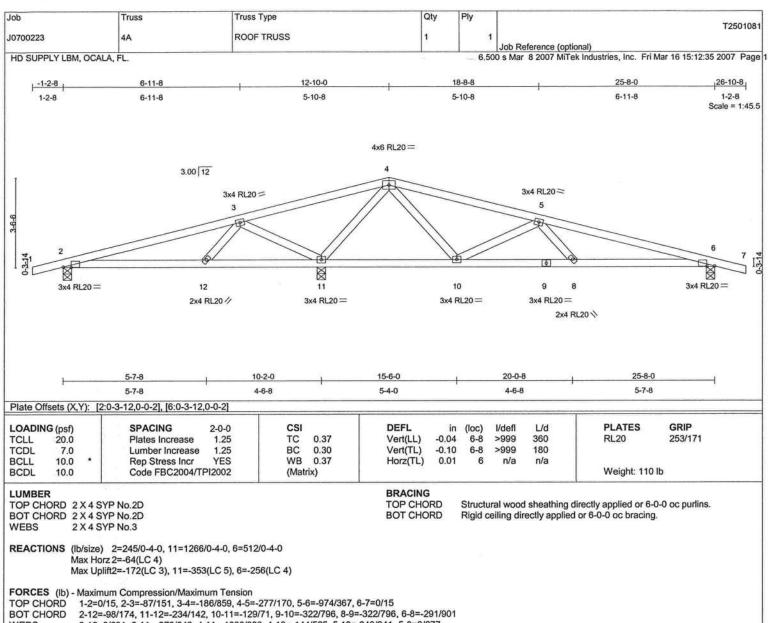
WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design volid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer. Practice is the responsibility of the insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



6904 Parke East Blvd. Tampa, FL 33610



WEBS 3-12=0/281, 3-11=-676/349, 4-11=-1090/363, 4-10=-144/525, 5-10=-648/341, 5-8=0/277

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 172 lb uplift at joint 2, 353 lb uplift at joint 11 and 256 lb uplift at joint 6.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Madison, WI 53719.



	Truss	Truss Type		Qty	Ply				T25010
0700223	4B	ROOF TRUSS		1		1			125010
	1003						eference (o)		
ID SUPPLY LBM, O	CALA, FL.				6.	500 s Mar	8 2007 Mi	Tek Industries, Inc. Fri	Mar 16 15:12:36 2007 Pag
-1-2-8		12-10-0					25-8-0		26-10-8
1-2-8		12-10-0					12-10-0		1-2-8 Scale: 1/4"=
			4x4	RL20=					
		3.00 12	2x4 RL20 8	2x4 RL2 9		RL20 II			
Ī		2x4 RL20 2x4 RL20 6	11 7	· ·			4 RL20		
	2x4 RL20	ENTITLE OIL	8	8		_	11 ;	2x4 RL20	
	3x4 RL20 = 2x4 RL20	- B		1 11			- P	12 3x4 RL20 - 13	
	3							1	
4 2	0						a b	0	14 15
\$ X			***************************************	***************************************	*******	·	×	·	
*									- · · · · · · · · · · · · · · · · · · ·
*	4x8 RL20 25	24 23	22 2	1 20	1	9	18 17	' 16 4xi	8 RL20 *
3x4 R		24 23 0 2x4 RL20 2x4 RL20					18 17 3x4 R		3x8 RL20 =
1.00						RL20		L20 =	
1.9%						RL20	3x4 R	L20 =	
1.00						RL20	3x4 R	L20 =	
1.9%						RL20	3x4 R	L20 =	
19/						RL20	3x4 R	L20 =	
19/			2x4 RL20 2x4			RL20	3x4 R	L20 =	
1.97			2x4 RL20 2x4	RL20 2x4 RL2		RL20	3x4 R	L20 =	
3x4 R) 2x4 RL20 2x4 RL20	2x4 RL20 2x4 	RL20 2x4 RL2		RL20	3x4 R	L20 =	
3x4 R	[2:0-3-8,Edge], [2:0-6-12,Ed) 2x4 RL20 2x4 RL20	25- 25- 25- 25- 25- 25-	8-0 8-0	0 2x4 F	RL20	3x4 R	L20 =	
3x4 R Plate Offsets (X,Y): OADING (psf) CLL 20.0	[2:0-3-8,Edge], [2:0-6-12,Ed SPACING Plates Increase	ge], [14:0-3-8,Edge], [14 2-0-0 CSI 1.25 TC	25- 25- 25- 25- 25- 25- 25- 25- 25- 25-	8-0 8-0 DEFL Vert(LL) 0.0	in (loc)	Vdefl	3x4 RI 4 RL20 II :	L20 = 2x4 RL20	3x8 RL20 =
Plate Offsets (X,Y): OADING (psf) CLL 20.0 CDL 7.0	[2:0-3-8,Edge], [2:0-6-12,Ed SPACING Plates Increase Lumber Increase	ge], [14:0-3-8,Edge], [14 2-0-0 CSI 1.25 BC	25- 25- 25- 20-5-0,Edge 0.28 0.12	8-0 8-0 DEFL Vert(LL) 0.0 Vert(TL) 0.0	in (loc)	l/defl	3x4 RI 4 RL20 II : L/d 180 120	L20 = 2x4 RL20 PLATES	3x8 RL20 =
OADING (psf) CCLL 20.0 CCDL 7.0 CCLL 10.0	[2:0-3-8,Edge], [2:0-6-12,Ed SPACING Plates Increase Lumber Increase	ge], [14:0-3-8,Edge], [14 2-0-0 CSI 1.25 TC 1.25 BC NO WB	25- 25- 25- 25- 25- 20-5-0,Edge] 0.28 0.12 0.05	8-0 8-0 DEFL Vert(LL) 0.0	in (loc)	Vdefl	3x4 RI 4 RL20 II :	L20 = 2x4 RL20 PLATES	3x8 RL20 =
JAMBER	[2:0-3-8,Edge], [2:0-6-12,Ed SPACING 2 Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TPI	ge], [14:0-3-8,Edge], [14 2-0-0 CSI 1.25 TC 1.25 BC NO WB	25- 25- 25- 25- 25- 25- 25- 25- 25- 25-	8-0 8-0 8-0 Vert(LL) 0.0 Vert(TL) 0.0 Horz(TL) 0.0	in (loc) 0 15 11 15 0 14	I/defl n/r n/a	3x4 RI 4 RL20 II : L/d 180 120 n/a	PLATES RL20 Weight: 110	3x8 RL20 =
Plate Offsets (X,Y): LOADING (psf) FCLL 20.0 FCDL 7.0 BCLL 10.0 BCLL 10.0 LUMBER FOP CHORD 2 X 4	[2:0-3-8,Edge], [2:0-6-12,Ed] SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TPI:	ge], [14:0-3-8,Edge], [14 2-0-0 CSI 1.25 TC 1.25 BC NO WB	25- 25- 25- 25- 25- 25- 25- 25- 25- 25-	8-0 8-0 PEFL Vert(LL) 0.0 Vert(TL) 0.0 Horz(TL) 0.0 BRACING	in (loc) 0 2x4 F	I/defl n/r n/r n/a	3x4 Rl. 20 : : :	PLATES RL20 Weight: 110	3x8 RL20 = GRIP 253/171 Olb 0-0-0 oc purlins.
Plate Offsets (X,Y): OADING (psf) CLL 20.0 CDL 7.0 CDL 10.0 CDL 10.0 UMBER OP CHORD 2 X 4 OT CHORD 2 X 4	[2:0-3-8,Edge], [2:0-6-12,Ed] SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TPI:	ge], [14:0-3-8,Edge], [14 2-0-0 CSI 1.25 TC 1.25 BC NO WB	25- 25- 25- 25- 25- 25- 25- 25- 25- 25-	8-0 8-0 8-0 Vert(LL) 0.0 Vert(TL) 0.0 Horz(TL) 0.0	in (loc) 0 2x4 F	I/defl n/r n/r n/a	3x4 Rl. 20 : : :	PLATES RL20 Weight: 110	3x8 RL20 = GRIP 253/171 Olb 0-0-0 oc purlins.

16=301/25-8-0

Max Horz 2=-68(LC 4)

Max Uplift2=-192(LC 5), 14=-197(LC 6), 21=-13(LC 5), 22=-62(LC 3), 23=-63(LC 5), 24=-55(LC 3), 25=-85(LC 5), 20=-61(LC 4), 19=-63(LC 6), 18=-54(LC 4), 16=-87(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/25, 2-3=-54/34, 3-4=-50/65, 4-5=-16/52, 5-6=0/63, 6-7=0/80, 7-8=0/94, 8-9=0/91, 9-10=0/67, 10-11=0/49, 11-12=0/40,

12-13=-17/65, 13-14=-21/34, 14-15=0/25

BOT CHORD 2-25=-33/68, 24-25=-33/68, 23-24=-33/68, 22-23=-33/68, 21-22=-33/68, 20-21=-33/68, 19-20=-33/68, 18-19=-33/68,

17-18=-33/68, 16-17=-33/68, 14-16=-33/68

8-21=-124/34, 7-22=-106/81, 6-23=-118/85, 5-24=-68/65, 4-25=-211/127, 9-20=-106/81, 10-19=-118/86, 11-18=-68/64,

12-16=-211/129

NOTES

WEBS

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 2, 197 lb uplift at joint 14, 13 lb uplift at joint 21, 62 lb uplift at joint 22, 63 lb uplift at joint 23, 55 lb uplift at joint 24, 85 lb uplift at joint 25, 61 lb uplift at joint 20, 63 lb uplift at joint 19, 54 lb uplift at joint 18 and 87 lb uplift at joint 16.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

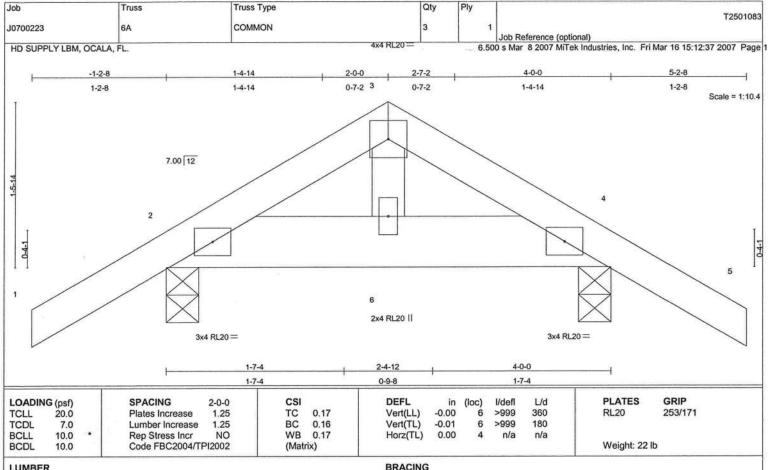
March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.3 WEBS

REACTIONS (lb/size) 2=512/0-3-8, 4=512/0-3-8

Max Horz 2=-46(LC 6)

Max Uplift2=-246(LC 5), 4=-246(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-612/190, 3-4=-612/192, 4-5=0/37

BOT CHORD 2-6=-113/491, 4-6=-113/491

WEBS

3-6=-174/544

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 2 and 246 lb uplift at joint
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 604 lb down and 226 lb up at 2-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 2-4=-20, 1-3=-54, 3-5=-54

Concentrated Loads (lb)

Vert: 6=-604(F)

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

Structural wood sheathing directly applied or 4-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

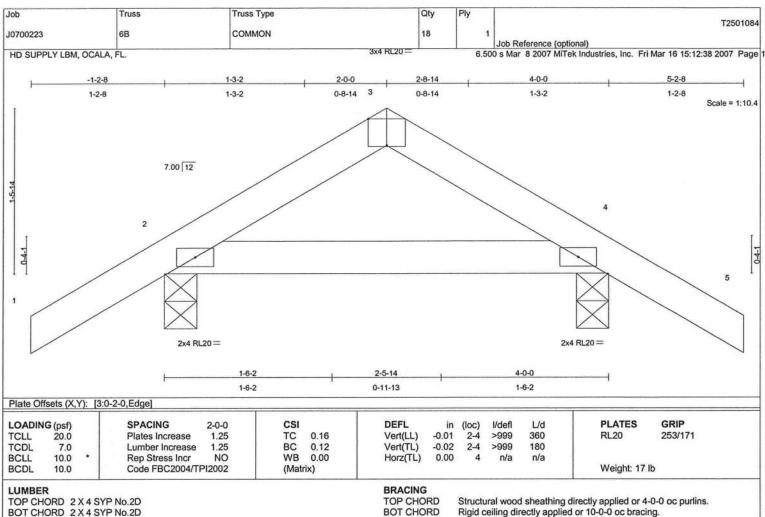
March 16,2007

MARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

REACTIONS (lb/size) 2=210/0-3-8, 4=210/0-3-8

Max Horz 2=-44(LC 3) Max Uplift2=-132(LC 5), 4=-132(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/34, 2-3=-81/25, 3-4=-81/25, 4-5=0/34 TOP CHORD

BOT CHORD 2-4=0/71

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust), h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 2 and 132 lb uplift at joint

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

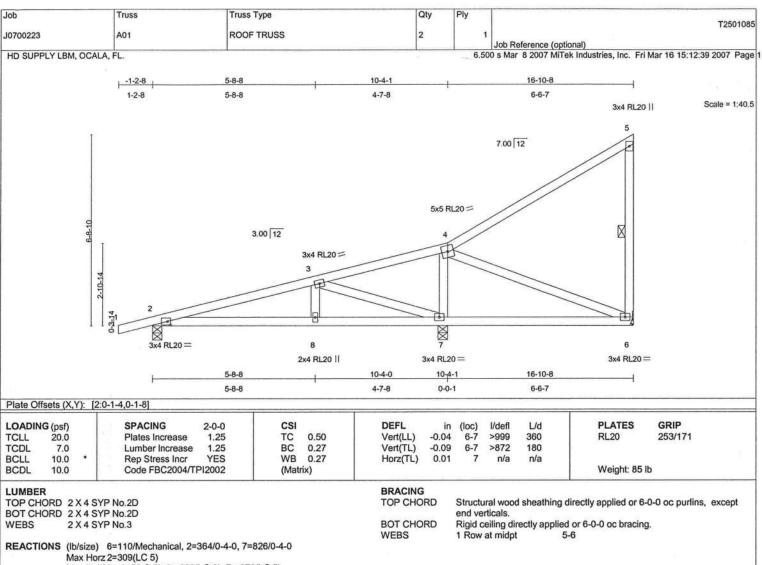
March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer. Proceedings shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





Max Uplift6=-115(LC 5), 2=-228(LC 3), 7=-276(LC 5) Max Grav6=133(LC 3), 2=364(LC 1), 7=826(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/15, 2-3=-440/172, 3-4=-235/291, 4-5=-123/50, 5-6=-146/149

BOT CHORD 2-8=-249/390, 7-8=-249/390, 6-7=-222/147

WEBS 3-8=0/193, 3-7=-673/425, 4-7=-504/266, 4-6=-147/268

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

 Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 6, 228 lb uplift at joint 2 and 276 lb uplift at joint 7.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

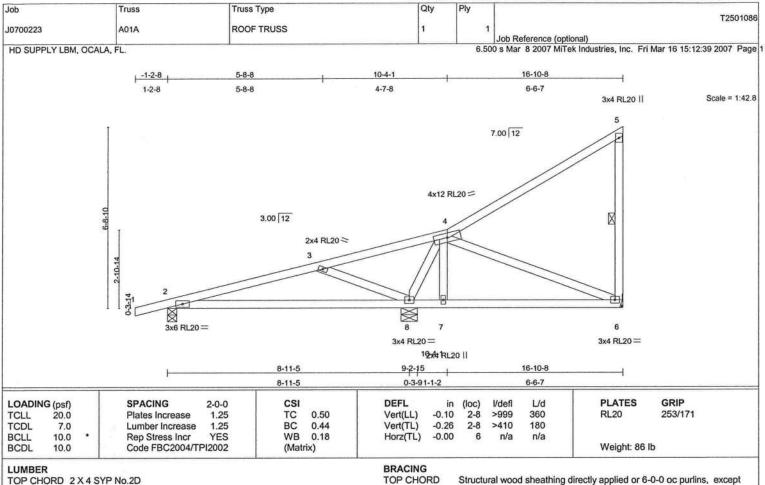
March 16,2007

▲ WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design volid for use only with Mifek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





end verticals.

1 Row at midpt

Rigid ceiling directly applied or 6-0-0 oc bracing.

5-6

BOT CHORD

WEBS

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 6=195/Mechanical, 2=323/0-4-0, 8=782/0-7-3

Max Horz 2=309(LC 5)

Max Uplift6=-140(LC 5), 2=-209(LC 3), 8=-267(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/15, 2-3=-228/152, 3-4=-280/282, 4-5=-124/49, 5-6=-146/148

BOT CHORD 2-8=-229/198, 7-8=-51/115, 6-7=-54/114

WEBS 3-8=-472/447, 4-8=-465/239, 4-7=-9/49, 4-6=-110/52

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

 Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 6, 209 lb uplift at joint 2 and 267 lb uplift at joint 8.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

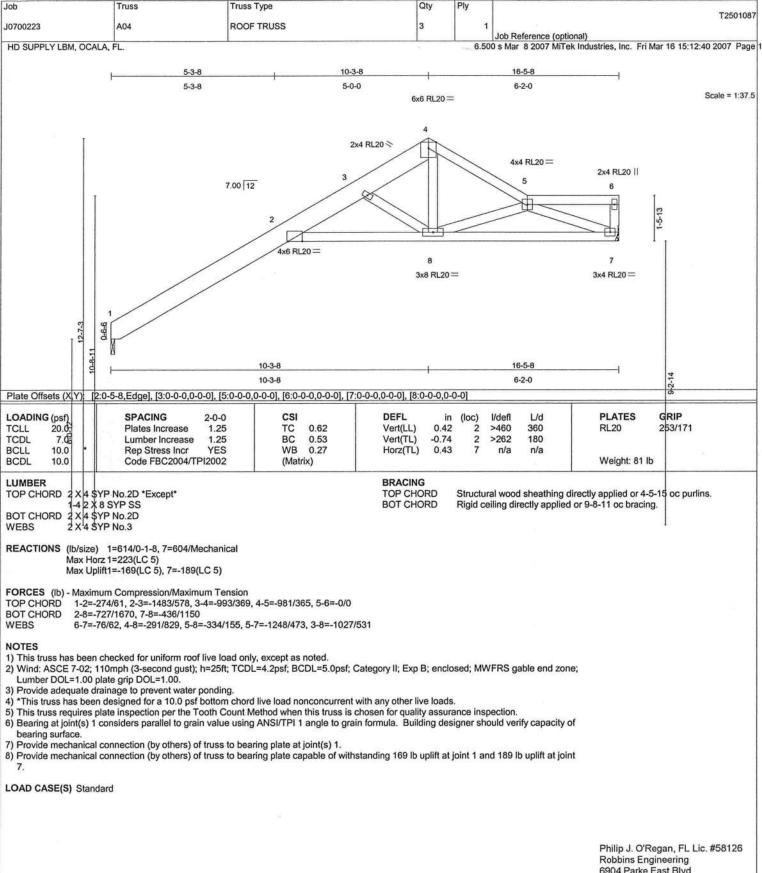
March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Madison, WI 53719.



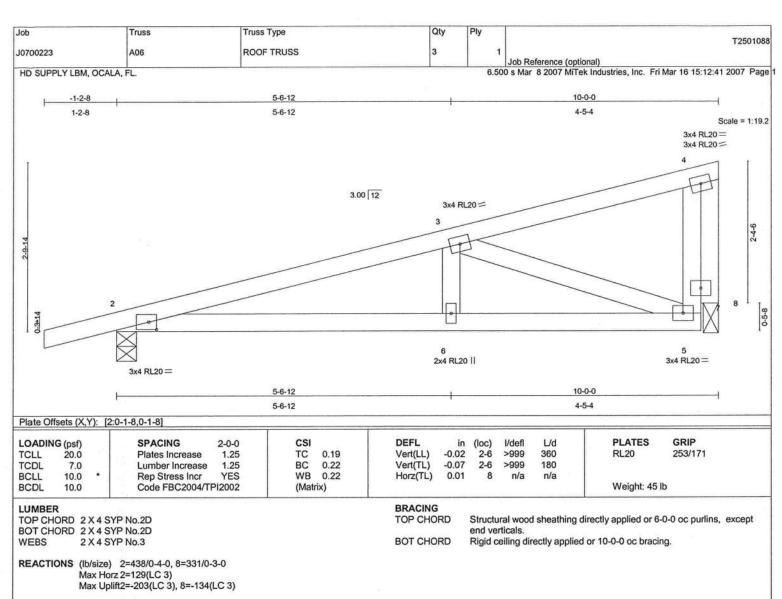


6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/15, 2-3=-749/206, 3-4=-106/17, 5-7=-72/257, 4-7=-72/257

BOT CHORD 2-6=-271/690, 5-6=-271/690

WEBS 3-6=0/208, 3-5=-653/253, 4-8=-336/137, 7-8=-28/60

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf, Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

- 5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 203 lb uplift at joint 2 and 134 lb uplift at joint 8.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



	Truss	Truss Type		Qty	Ply		12		T25010
700223	FL	ROOF TRUSS		3		1			125010
D SUPPLY LBM, C	CALA EL				- 6		eference (optio		Mar 16 15:12:42 2007 Pa
D SUPPLY LBIM, C	JCALA, FL.				- 0.	JUU S IVIAI	0 2007 WILLER	industries, inc. 111	Wai 10 10.12.42 2007 F 8
1	5-8-8	10-4-1				27-5-8			
	5-8-8	4-7-8	1.7			17-1-7			Scale = 1:4
									Scale - 1.4
1		2 3	4						5
0	0 0			0 0	- O		0 0	- Fo	
		The -	M	О О				, ,	
₩ 4x6 RL20	4x6 RL20	8x8 RL20 = 4x6 R	L20 ⊠ 4x6 RL20	4x6 RL20 4x6 RL2	0 4x6 RI	_20 4x6	RL20 4x6 R	L20 4x6 RL20	
	3 RL20 4x6 RL20	4x6 RL20	4X6 KL20 II						8x12 RL20
420	3 NL20	8x10 RL20							
		4 0 01 00 11							
		4x6 RL20							
		4x6 RL20 II							
		4x6 RL20 II							
		4x6 RL20 II							
TD1100 N	OT OVARIETDIS	4x6 RL20 II							
	OT SYMMETRIC ERECT THE TRUSS BACKV								
	ERECT THE TRUSS BACKV			19-6-5			20-11-1	27-5-8	
	ERECT THE TRUSS BACKV			19-6-5 8-3-9			20-11-1	27-5-8 6-6-7	
DO NOT E	ERECT THE TRUSS BACKV	VARDS], [3:0-2-10,0-3-6	8-3-9					
DO NOT E	11-2-12 11-2-12 : [2:0-3-10,0-2-1], [2:0-3-0,0-	VARDS 2-0], [3:0-3-13,0-3-10		8-3-9], [5:0-5-10,0-0-15]	in Haal		1-4-12	6-6-7	CDIR
DO NOT B ate Offsets (X,Y): DADING (psf)	ERECT THE TRUSS BACKV 11-2-12 11-2-12	VARDS 2-0], [3:0-3-13,0-3-10 2-0-0 C	SI	8-3-9], [5:0-5-10,0-0-15] DEFL	in (loc)				GRIP 253/171
ate Offsets (X,Y): DADING (psf) CLL 20.0 CDL 7.0	11-2-12 11-2-12 11-2-13 11-2-13 11-2-14 11-2-15 11-2-16 11-2-16 11-2-17 11-2-18	VARDS 2-0], [3:0-3-13,0-3-10 2-0-0 1.25 1.25 B	SI C 0.17 C 0.00	8-3-9], [5:0-5-10,0-0-15] DEFL Vert(LL) 0 Vert(TL) -0.	09 4-5 12 4-5	l/defl >999 >999	L/d 360 180	6-6-7	1 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
DO NOT B ate Offsets (X,Y): DADING (psf) CLL 20.0 CDL 7.0 CLL 0.0	11-2-12 11-2-12 11-2-12 11-2-12 11-2-13 11-2-12 11-2-1	VARDS 2-0], [3:0-3-13,0-3-10 2-0-0 C 1.25 Tu 1.25 B YES W	SI C 0.17 C 0.00 /B 0.00	8-3-9], [5:0-5-10,0-0-15] DEFL Vert(LL) 0 Vert(TL) -0.	09 4-5	l/defl >999	L/d 360	PLATES RL20	253/171
ate Offsets (X,Y): DADING (psf) CLL 20.0 CDL 7.0	11-2-12 11-2-12 11-2-13 11-2-13 11-2-14 11-2-15 11-2-16 11-2-16 11-2-17 11-2-18	VARDS 2-0], [3:0-3-13,0-3-10 2-0-0 C 1.25 Tu 1.25 B YES W	SI C 0.17 C 0.00	8-3-9], [5:0-5-10,0-0-15] DEFL Vert(LL) 0 Vert(TL) -0.	09 4-5 12 4-5	l/defl >999 >999	L/d 360 180	6-6-7	253/171
DO NOT E ate Offsets (X,Y): DADING (psf) CLL 20.0 CDL 7.0 CLL 0.0 CDL 10.0 JMBER	11-2-12 11-2-12 11-2-12 11-2-12 11-2-13 11-2-12 11-2-1	VARDS 2-0], [3:0-3-13,0-3-10 2-0-0 C 1.25 Tu 1.25 B YES W	SI C 0.17 C 0.00 /B 0.00	8-3-9], [5:0-5-10,0-0-15] DEFL Vert(LL) 0 Vert(TL) -0. Horz(TL) 0	09 4-5 12 4-5 00	l/defl >999 >999 n/a	L/d 360 180 n/a	PLATES RL20 Weight: 169	253/171
DO NOT B ate Offsets (X,Y): DADING (psf) CLL 20.0 CDL 7.0 CLL 0.0 CDL 10.0	11-2-12 11-2-12 11-2-12 11-2-13 11-2-13 11-2-13 11-2-14 11-2-15 11-2-15 11-2-16 11-2-17 11-2-1	VARDS 2-0], [3:0-3-13,0-3-10 2-0-0 C 1.25 Tu 1.25 B YES W	SI C 0.17 C 0.00 /B 0.00	8-3-9], [5:0-5-10,0-0-15] DEFL Vert(LL) 0 Vert(TL) -0 Horz(TL) 0	09 4-5 12 4-5 00 Struct	l/defl >999 >999 n/a	L/d 360 180 n/a	PLATES RL20	253/171 1 lb -6-6 oc purlins.

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/0, 2-3=0/0, 3-4=0/0, 4-5=0/0

1) This truss has been checked for uniform roof live load only, except as noted.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

Provide adequate drainage to prevent water ponding.
 This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 1, 243 lb uplift at joint 5 and 663 lb uplift at joint 4.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

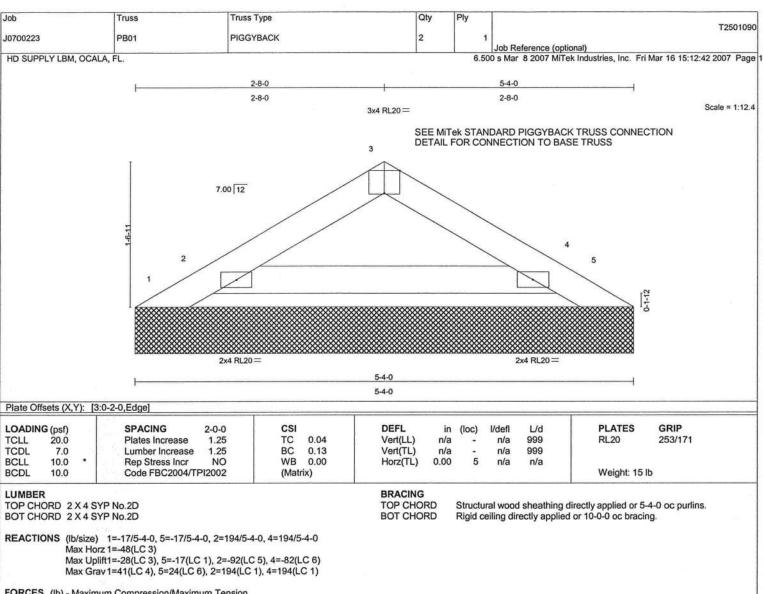
WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

AND INCLUDED ROBBINS REFERENCE PAGE REID-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-62/61, 2-3=-107/50, 3-4=-107/50, 4-5=-11/28

BOT CHORD 2-4=-17/65

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 17 lb uplift at joint 5, 92 lb uplift at joint 2 and 82 lb uplift at joint 4.

7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4.

8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Design Valid to two drily with inflex or incolors connectors. Inis design is based only upon parameters shown, and is for an individual building comp. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Raccing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult MSI/TPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Truss Type Qty Ply Job Truss T2501091 PB02 PIGGYBACK 24 J0700223 Job Reference (optional) 6.500 s Mar 8 2007 MiTek Industries, Inc. Fri Mar 16 15:12:43 2007 Page 1 HD SUPPLY LBM, OCALA, FL. 2-8-0 2-8-0 2-8-0 Scale = 1:12.4 3x4 RL20 = SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION 3 DETAIL FOR CONNECTION TO BASE TRUSS 7.00 12 2 2x4 RL20 2x4 RL20 = 5-4-0 5-4-0 Plate Offsets (X,Y): [3:0-2-0,Edge] SPACING DEFL **PLATES** GRIP LOADING (psf) 2-0-0 I/def L/d TC 0.04 999 RL20 253/171 TCLL 20.0 Plates Increase 1.25 Vert(LL) n/a n/a 1.25 BC 0.13 Vert(TL) n/a n/a 999 TCDL 7.0 Lumber Increase WB 0.00 0.00 BCLL 10.0 Rep Stress Incr NO Horz(TL) n/a n/a Code FBC2004/TPI2002 (Matrix) Weight: 15 lb BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 5-4-0 oc purlins. TOP CHORD TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. REACTIONS (lb/size) 1=-17/5-4-0, 5=-17/5-4-0, 2=194/5-4-0, 4=194/5-4-0 Max Horz 1=-48(LC 3) Max Uplift1=-28(LC 3), 5=-17(LC 1), 2=-92(LC 5), 4=-82(LC 6) Max Grav 1=41(LC 4), 5=24(LC 6), 2=194(LC 1), 4=194(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-62/61, 2-3=-107/50, 3-4=-107/50, 4-5=-11/28

BOT CHORD 2-4=-17/65

NOTES

1) This truss has been checked for uniform roof live load only, except as noted.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.00 plate grip DOL=1.00.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 17 lb uplift at joint 5, 92 lb uplift at joint 2 and 82 lb uplift at joint 4.

7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4.

8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

Philip J. O'Regan, FL Lic. #58126 Robbins Engineering 6904 Parke East Blvd Tampa, FL, 33610 FL Cert.#5555

March 16,2007

WARNING - Verify design parameters and READ NOTES ON THIS

AND INCLUDED ROBBINS REFERENCE PAGE RE10-10-06 BEFORE USE.

Design valid for use only with MiTek or Robbins connectors. This design is based only upon parameters shown, and is for an individual building component. Design Valid to be only with wheek or koolins controlled in a design is absent of the proposal parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and 8CSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



J.L. Rye

Rye Construction

Re: Smith Residence

Mr. Rye,

Regarding the Smith residence, for the garage door header for the main house, it shall be a double 2x12 SYP #2 and strapped at each end for a min of 450 lbs uplift resistance (Simpson LSTA9 top, H3 bottom). Also, all other headers and beams are to be double 2x12 SYP #2. For the separate garage, the 18' garage door header is to be a 2 ply 16" LVL and strapped per the wind load engineering.

If you have any questions regarding this matter please feel free to contact me.

Sincerely

Frank J. Sapienza Jr.

License Professional Engineer Florida License Number 48566

Wind Load Analysis and Certification

Smith Residence by Rye Construction

2004 Florida Building Code (Residential) section 1609 according to ASCE 7-02

Basic Wind Speed = 110 MPH

Importance Factor = 1.0

Exposure Category = B

Applicable Internal Pressure Coefficient = .18

Design Wind Pressure for use of External Components = 31.1 psf

Mean Roof Height = 16.5'

Roof Decking

7/16" OSB, 5/8" CDX or 3/4" CDX Decking; 48"x96" Sheets, Perpendicular to Roof Framing Members 8d common (.131" dia) nails at 4" O.C. on Ends, 8" O.C. in Interior or 8d (.113") ring shank nail @ 4" O.C. ends, 6" O.C. interior.

Trusses or Rafters at 2' O.C. (horizontal distance), No Intermediate Blocking Required Rafters: 2x6 SYP #2 up to 10' horizontal span, 2x8 SYP #2 up to 14' horizontal span

Shear Wall Segments

7/16" OSB, 48" Wide Sheets Placed Vertical - Sheathing Continuous from Top Plate down to Pressure Treated Sole Plate Bearing on Foundation.

8d common (.131" dia) nails at 3" O.C. on Edges and Ends, 8" O.C. in Interior

Transverse Shearwall = 45', Longitudinal Shearwall = 52'

2x4 SPF (No. 1&2) Studs at 16" O.C., up to 12' wall height

or: 2x6 SPF (No. 1&2) Studs at 16" O.C., up to 18' wall height

Nail Together Double Top Plate 6" O.C. w/12-d Common Nails (SYP top plates) New Header 51 Ze. Other Wall Segments - Same as Shear Walls

Gabled End Wall Framing

Balloon Frame (see detail) or see attached alternate details.

T-Block (with 2x4's) bottom chord of porch gable trusses at 4' O.C to 6' from wall

Special Notes: Other than double sheathed sections as shown on plans, no special corner framing required. Sheath interior shearwalls to roof sheathing or floor sheathing or see attached alternate Interior Shearwall Detail (Interior footer required under all interior shearwalls)

Footings and Foundations (Based on Truss Engineering)

20" deep x 14" wide monolithic with 2-#5's, Continuous

or: 20" Wide x 10" Deep 2500 psi Concrete Strip Footing with 2-#5's, Continuous

8"x8"x16" Concrete Masonry Stemwall, Minimum 2 Courses, Maximum 5 Courses, Fully Grouted, except sections over 3 courses need only cells with rebar to be grouted. 1-#5 Vertical Dowel at Corners and 8'-0" O.C. (10" hook top and bottom) (min 25" lap all #5 rebar) (1) #5 continuous top course. All 4" slabs requires 6x6 WWM

Interior footers: 16" wide by 10" deep (including 4" slab) with 2-#5's, Continuous,

Porch Footers: see above or: 8" wide by 8" deep bell footing with 1-#5, Continuous with minimum of 24"x24" x 12" pad under each post (w/ 2- #5 each way) or 16" deep x 12" wide monolithic with 2-#5, Cont. with no pads.

Note: footer design based on continuous bearing. Continuous footers (grade beams) for pier foundation systems must be designed by pier foundation subcontractor.

Hurricane-Resistance Hardware (Based on Truss Engineering)

Truss Clips/Headers/Girders/Posts/Beams /Top and Bottom of Wall Unit - See Table Anchor Bolts- A-307 (1/2"Dia. x 8" with min 6" embedment) at 48"O.C. (First bolt at 9" from Corner, then 48" O.C.) and at each end of Each Shearwall Segment (2" round or square washers).

I hereby certify that the accompanying Wind Load Analysis for the Smith Residence, demonstrates compliance with the 2004 FBC section 1609 according to ASCE 7-02, to the best of my knowledge.

> License Professional Engineer Florida License Number 48566

Wood Sections	Jplift				
				Bottom Connector	Rating
	Lbs	Simpson **	Lbs	Simpson **	Lbs
HEADERS	455 lb -	LOTAG	775	H3	455
	455 lbs		775 970	2-H3	910
	910 lbs	The state of the s	1235	LTT19	1350
	1235 lbs		1940	LTT20	1750
	1750 lbs 2470 lbs		2470	HD2A-2.5	2565
	2775 lbs		3705	HD2A-3.5	2775
	3705 lbs		3705	HD5A-3	3705
To determine uplift force on header at e					
(assumes uniform load) Note	acii eiiu,	ise proper holt at	nchors s	ufficient to support requ	ired load
(assumes uniform load)	s. must u	ise proper bolt a	1011013 3	amoioni to support roqu	an ou roug
Trusses/Girders -					
up to 600 lbs - use H2.5A t	op. no s	pecial device rec	uired at	bottom	
over 600 lbs but under 990) lbs use	H10 top, no spe	cial devi	ce required at bottom	
up to 1215 lbs use TS22 o	r equival	ent at top and LT	TT19 at b	oottom	
up to 1750 lbs use 2-TS22	or equiv	alent at top and	LTT20 a	t bottom	
up to 2430 lbs use 2-TS22	or equiv	alent at top and	HD2A bo	ottom	
up to 3645 lbs use 3-TS22					
up to 3040 lbs use 0-1022	or equiv	Must Use prope			
Note: it is the contractors respons	sibility t				
truss/rafter/ridge beam to foundar	tion	o provide a con	illiaoao	loud patil iloiii	
Strap rafters to truss or at each end	with mir	n unlift resistance	e of 450	lbs each end	
Strap ridge beam at each end with	min unlif	t resistance of 18	300 lbs		
Strap ridge bearn at each cha with	min apin	t robiotarios er re	300 100	(2)	
Note: Four (4) 12d comm toenails (2	on each	side) required pe	r truss/ra	fter per bearing point in	nto plate
to resist both lateral loads (wall to	truss) a	nd transverse loa	ds (max	plate height =12', not in	ncluding gable)
Horizontal Resistance (from truss	loads)	- Note: these de	evices a	re in addition to requi	red toe-nails
up to 110 lbs - use H2.5A				dware to be used must s	
				I horizontal resistance, co	
up to 525 lbs use H10	10 102			s is acceptable	momadon
up to 1090 lbs use H10 plu	15 A23		or device	s is acceptable	
Note: for combination of loads (u	nlift and	l horizontal/late	ral) on a	single device the ra	tio of
actual uplift/allowable uplift + act	ual bari	zontal load/allo	wahle h	orizontal cannot exce	ed 1
actual upilitrallowable upilit + act	uai iioii	top	Wabie iii	bottom	ou i
BEAM SEATS		LSTA18*	1235	LTT19*	1350
		2-LSTA18	2470	ABU44	2200
POSTS (max post spacing = 14')	nor true		2410	Must Use proper bolt	
	per tru	ss engineering		wust ose proper boit	anchors
STUDS	-1 1A/-II-	hattana / Od naila	-t 2"O C	\	
Wall Sheathing Nailing Adequate Exte	rior walls	To a (Od poils at 2	at 3 O.C.	long on shoothing cover	e ton
Wall Sheathing Nailing Adequate Exte	rior walls	dition to shoothing	o.c.), as	s long as sheathing cover	s top
plate, otherwise use SP2 @32" O	.c. in ad	union to sneathir	for all !-	torior load bearing wells	hat
Use SP2 top and SP1 bottom each stu	u an anco	9" A 2 O 7 or 1 / 2"	. IOI all In	de anchor or equiv	ııal
have uplift. Interior anchor bolts to b	DE 1/2 X	0 A307 01 1/2	x o wed	ge allollol of equiv.	

Please Note: All Beams must be sheathed or strapped to Double Top Plate (if applicable)

**an equivalent device of same or other manufactures can be substituted for any of the devices specified on this
page as long as it meets the required load capacities

Note: For nailing into SPF members, multiply table values by .86

12/ 3/9/01

Calculation of Lateral (wall to truss) and Transverse (truss to wall) Loads Smith Residence

Lateral Loads (loads perpendicular to wall)

Max Max (lbs/ft)
Wall Height (ft) Wall Pressure* Mult factor Total load on wall
12 17.7 1.3 276.12

wall height x wall pressure x factor

Trusses @2' O.C.

Lateral Load at top of wall = 138.06 (1/2 total load)

Lateral Load at truss = 276.12 (load at top of wall x 2)

Lateral Resistance of 12d comm toe-nail into SYP is 88 lbs Resistance of four (4) 12d comm toe-nails in SYP 4*88= 352 lbs

352 > 276.12 thus acceptable

*per ASCE7-02 (in lbs/sq ft)

Transverse Loads (loads parallel to wall, wall loads only)

worst case condiditon 17.7 x 10 x 89 = 15753 lbs total load 15753/2 = 7877 load at top 7877/2 = 3938 load per side 3938/54 = 73 lbs/ft

 $73 \times 2 = 146 \text{ lbs/truss}$ transverse resistance of (4) 12d comm toe-nail in SYP = 4 x 44 = 176 lbs 176 > 146 therefore acceptable

Acceptable Framing Method for Balloon Framed Gable End-Wall with trusses

Balloon Frame with 2x4 SPF No.1&2 @ 16" O.C. with the Following Conditions:

Up to 12' — Block at 8'

Over 12' but Under 14' — 2x4 SYP #2 at 16" O.C. and Block at 4',8'&12'

Over 14' but Under 17' — Double 2x4 SYP #2 at 16" O.C. and block at 4',8',12'&16'

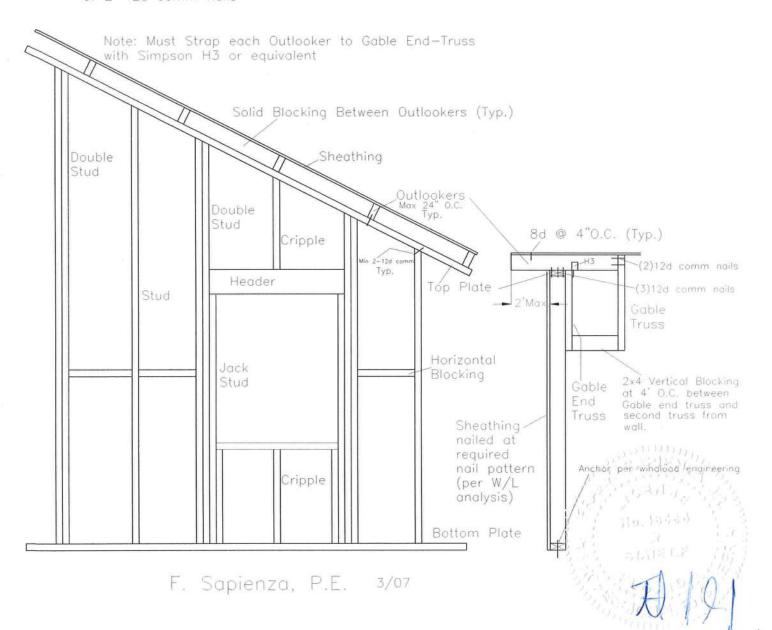
Over 17' but Under 20' — Triple 2x4 SYP #2 at 16" O.C. and block at 4',8',12'&16'

Over 20' but Under 23' — Quadruple 2x4 SYP #2 at 16" O.C.and block at 4',8',12',16'&20'

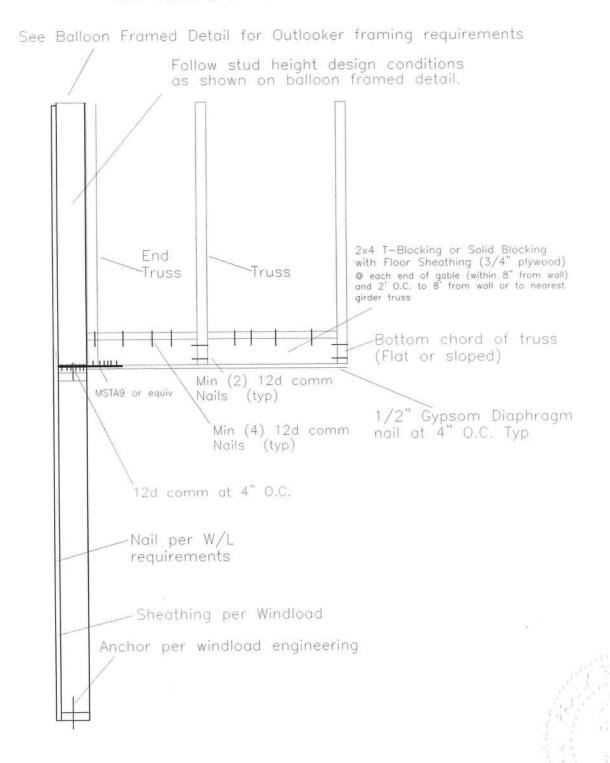
Over 23' — Must be Engineered

In all cases a minimum of a double full length stud is required at each side of openings such as doors and windows

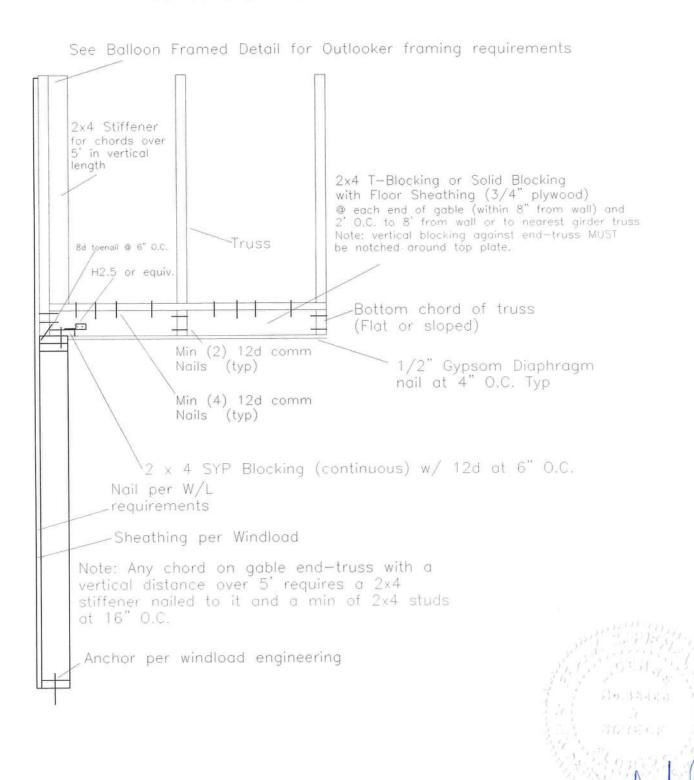
Blocking must be parallel to top and bottom plates with a minimum of 2-12d comm nails



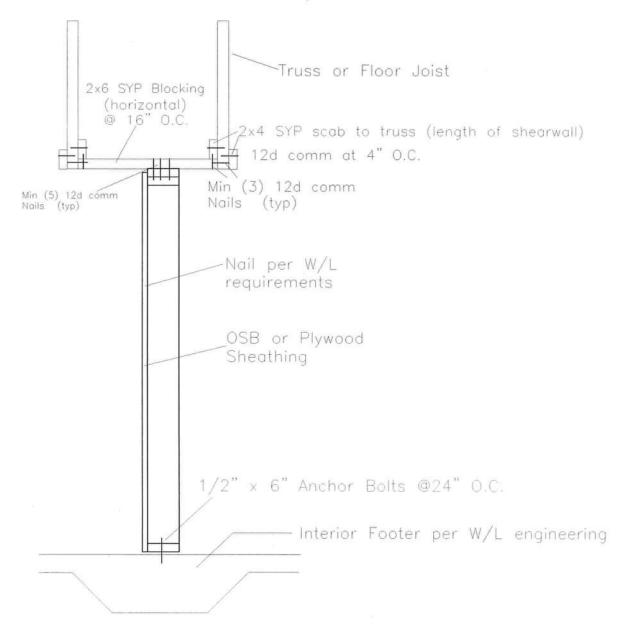
Gable Endwall Framing with Gable End-Truss



3/19/01



Interior Shearwall Sheathed to Bottom Truss Chord or Floor System



Note: If truss or floor system crosses perpindicular to shearwall, attach to shearwall using H2.5 or equiv. where each member crosses shearwall Jan Maria

X / X / 3 / 19/01

Number of Jack and Stud Requirements per Opening Width 2x4 or 2x6 SPF #1&2 Construction — max Wall Height=12' (based on 16" O.C. Stud Spacing)

acks			ПШ
Opening Width up to 4' up to 6' up to 9' up to 12' up to 14' up to 18' over 18' must	1 1 2 2 3 3	1 2 2 3 3 4	
Open	ning Width —		

Note — Based on uniform loads. Heavy concentrated loads require engineering review



Project Name: Smith Residence



Location:

By: F Sapienza

Start Date: 3/19/2007

Comments:

Local Information

Wind Dir.	Exposure
1	В
2	В
3	В
4	В

Basic Wind Speed: 110 mph

Topography: None

Optional Factors

This project uses load combinations from ASCE 7.

Section - Main Section

Enclosure Classification:

Enclosed

Building Category: II

Wall	Length(ft)	Overhang(ft)
1	89.0	2.0
2	54.0	2.0
3	89.0	2.0
4	54.0	2.0

Eave Height:

10 ft

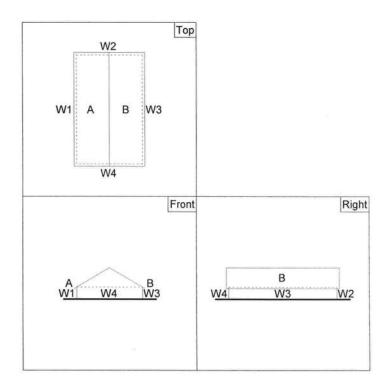
Parapet Height:

0 ft

Parapet Enclosure: Solid

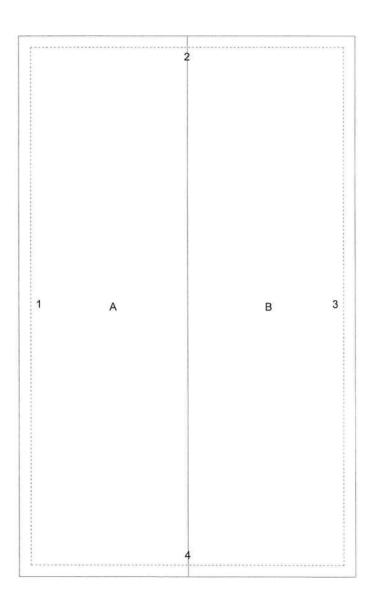
Roof Shape: Gabled

Roof	Slope(:12)
A&B	7.0



Composite Drawing









MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 1

#	Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi	(psf)
1	Windward Wall	10.0	15.1	0.85	0.80	0.18	10.3	7.4	13.1	
	Overhang Top	17.9	15.9		0.27	0	3.6			
		17.9	15.9		-0.19		-2.6			
	Overhang Bot	10.0	15.1		0.80		10.3			
2	Side Wall	17.9	15.9	0.85	-0.70	0.18	-9.5	-12.3	-6.6	9
3	Leeward Wall	17.9	15.9	0.85	-0.50	0.18	-6.8	-9.6	-3.9	
4	Side Wall	17.9	15.9	0.85	-0.70	0.18	-9.5	-12.3	-6.6	8
А	Windward Roof	17.9	15.9	0.85	0.27	0.18	3.6	0.8	6.5	
		17.9	15.9		-0.19		-2.6	-5.4	0.3	
В	Leeward Roof	17.9	15.9	0.85	-0.60	0.18	-8.1	-11.0	-5.2	
This	is load case 1 in	ASCE 7-02 Figur	e 6-9. S	ee Fig	ure 6-9	for ot	her cases.			

MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 2

#	Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi (psf)
1	Side Wall	17.9	15.9	0.86	-0.70	0.18	-9.6	-12.4	-6.7
2	Windward Wall	15.0	15.1		0.80		10.4	7.5	13.3
		17.9	15.9				10.9	8.1	13.8
		20.0	16.4				11.3	8.4	14.1
		25.8	17.7				12.2	9.3	15.0
	Overhang Top	17.9	15.9		-0.90	0	-12.3		
	Overhang Bot	17.9	15.9		0.80		10.9		
3	Side Wall	17.9	15.9	0.86	-0.70	0.18	-9.6	-12.4	-6.7
4	Leeward Wall	17.9	15.9	0.86	-0.37	0.18	-5.1	-7.9	-2.2
A&B	Roof	0 to 8.9 *	15.9	0.86	-0.90	0.18	-12.3	-15.2	-9.4
		8.9 to 17.9 *	15.9				-12.3	-15.2	-9.4
		17.9 to 35.8 *	15.9		-0.50		-6.8	-9.7	-4.0
		35.8 to 89.0 *	15.9		-0.30		-4.1	-7.0	-1.2
		0 to 89.0 *	15.9		-0.18		-2.5	-5.3	0.4

This is load case 1 in ASCE 7-02 Figure 6-9. See Figure 6-9 for other cases.

^{*} Distance from windward edge.

MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 3

#	Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi (psf)
1	Leeward Wall	17.9	15.9	0.85	-0.50	0.18	-6.8	-9.6	-3.9
2	Side Wall	17.9	15.9		-0.70		-9.5	-12.3	-6.6
3	Windward Wall	10.0	15.1	0.85	0.80	0.18	10.3	7.4	13.1
	Overhang Top	17.9	15.9		0.27	0	3.6	8	
		17.9	15.9		-0.19		-2.6		
	Overhang Bot	10.0	15.1		0.80		10.3		
4	Side Wall	17.9	15.9	0.85	-0.70	0.18	-9.5	-12.3	-6.6
В	Windward Roof	17.9	15.9	0.85	0.27	0.18	3.6	0.8	6.5
		17.9	15.9		-0.19		-2.6	-5.4	0.3
А	Leeward Roof	17.9	15.9	0.85	-0.60	0.18	-8.1	-11.0	-5.2
This	is load case 1 in	ASCE 7-02 Figur	e 6-9. S	ee Fig	ure 6-	9 for ot	her cases.		*)

MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 4

Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi (psf
Side Wall	17.9	15.9	0.86	-0.70	0.18	-9.6	-12.4	-6.7
Leeward Wall	17.9	15.9		-0.37		-5.1	-7.9	-2.2
Side Wall	17.9	15.9	0.86	-0.70	0.18	-9.6	-12.4	-6.7
Windward Wall	15.0	15.1	0.86	0.80	0.18	10.4	7.5	13.3
	17.9	15.9				10.9	8.1	13.8
	20.0	16.4				11.3	8.4	14.1
	25.8	17.7				12.2	9.3	15.0
Overhang Top	17.9	15.9		-0.90	0	-12.3		
Overhang Bot	17.9	15.9		0.80		10.9		
Roof	0 to 8.9 *	15.9	0.86	-0.90	0.18	-12.3	-15.2	-9.4
	8.9 to 17.9 *	15.9				-12.3	-15.2	-9.4
	17.9 to 35.8 *	15.9		-0.50		-6.8	-9.7	-4.0
	35.8 to 89.0 *	15.9		-0.30		-4.1	-7.0	-1.2
	0 to 89.0 *	15.9		-0.18		-2.5	-5.3	0.4
	Side Wall Leeward Wall Side Wall Windward Wall Overhang Top Overhang Bot	Side Wall 17.9 Leeward Wall 17.9 Side Wall 17.9 Windward Wall 15.0 17.9 20.0 25.8 Overhang Top 17.9 Overhang Bot 17.9 Roof 0 to 8.9 * 8.9 to 17.9 * 17.9 to 35.8 * 35.8 to 89.0 *	Side Wall 17.9 15.9 Leeward Wall 17.9 15.9 Side Wall 17.9 15.9 Windward Wall 15.0 15.1 17.9 15.9 20.0 16.4 25.8 17.7 Overhang Top 17.9 15.9 Overhang Bot 17.9 15.9 Roof 0 to 8.9 * 15.9 8.9 to 17.9 * 15.9 17.9 to 35.8 * 15.9 35.8 to 89.0 * 15.9	Side Wall 17.9 15.9 0.86 Leeward Wall 17.9 15.9 0.86 Windward Wall 15.0 15.1 0.86 Windward Wall 15.0 15.1 0.86 17.9 15.9 20.0 16.4 25.8 17.7 Overhang Top 17.9 15.9 Overhang Bot 17.9 15.9 Roof 0 to 8.9 * 15.9 17.9 to 35.8 * 15.9 35.8 to 89.0 * 15.9	Side Wall 17.9 15.9 0.86 -0.70 Leeward Wall 17.9 15.9 -0.37 Side Wall 17.9 15.9 0.86 -0.70 Windward Wall 15.0 15.1 0.86 0.80 17.9 15.9 0.86 0.80 17.9 15.9 -0.90 Overhang Top 17.9 15.9 -0.90 Overhang Bot 17.9 15.9 0.86 -0.90 Roof 0 to 8.9 * 15.9 0.86 -0.90 8.9 to 17.9 * 15.9 -0.50 35.8 to 89.0 * 15.9 -0.30	Side Wall 17.9 15.9 0.86 -0.70 0.18 Leeward Wall 17.9 15.9 -0.37 Side Wall 17.9 15.9 0.86 -0.70 0.18 Windward Wall 15.0 15.1 0.86 0.80 0.18 17.9 15.9 0.86 0.80 0.18 25.8 17.7 0.86 0.80 0.80 Overhang Top 17.9 15.9 -0.90 0 Overhang Bot 17.9 15.9 0.86 -0.90 0.18 Roof 0 to 8.9 * 15.9 0.86 -0.90 0.18 8.9 to 17.9 * 15.9 -0.50 35.8 to 89.0 * 15.9 -0.30	Side Wall 17.9 15.9 0.86 -0.70 0.18 -9.6 Leeward Wall 17.9 15.9 -0.37 -5.1 Side Wall 17.9 15.9 0.86 -0.70 0.18 -9.6 Windward Wall 15.0 15.1 0.86 0.80 0.18 10.4 17.9 15.9 10.9 10.9 20.0 16.4 11.3 25.8 17.7 12.2 Overhang Top 17.9 15.9 -0.90 0 -12.3 Overhang Bot 17.9 15.9 0.80 10.9 Roof 0 to 8.9 * 15.9 0.86 -0.90 0.18 -12.3 8.9 to 17.9 * 15.9 -0.50 -6.8 35.8 to 89.0 * 15.9 -0.50 -6.8 35.8 to 89.0 * 15.9 -0.30 -4.1	Side Wall 17.9 15.9 0.86 -0.70 0.18 -9.6 -12.4 Leeward Wall 17.9 15.9 -0.37 -5.1 -7.9 Side Wall 17.9 15.9 0.86 -0.70 0.18 -9.6 -12.4 Windward Wall 15.0 15.1 0.86 0.80 0.18 10.4 7.5 17.9 15.9 16.4 11.3 8.4 20.0 16.4 11.3 8.4 25.8 17.7 12.2 9.3 Overhang Top 17.9 15.9 -0.90 0 -12.3 Overhang Bot 17.9 15.9 0.80 10.9 Roof 0 to 8.9 * 15.9 0.86 -0.90 0.18 -12.3 -15.2 8.9 to 17.9 * 15.9 -0.50 -6.8 -9.7 35.8 to 89.0 * 15.9 -0.50 -6.8 -9.7 35.8 to 89.0 * 15.9 -0.30 -4.1 -7.0

This is load case 1 in ASCE 7-02 Figure 6-9. See Figure 6-9 for other cases.

^{*} Distance from windward edge.

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ------110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Applicant	Plans Exa	EMENTS: Two (2) complete sets of plans containing the following:
er ·	0	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be at the state of the state
Ø	0	The state of the s
_	-	Designers name and signature on document (FBC 106.1). If licensed
62		architect or engineer, official seal shall be affixed. Site Plan including:
		a) Dimensions of lot
		b) Dimensions of building set backs
		c) Location of all other buildings on lot, well and septic tank if
		"PPANULY, all all milly excements
5 /		d) Provide a full legal description of property.
021	0	Wind-load Engineering Summers, colored
		Plans or specifications must state compliance with FBC Section 1609.
		TOTAL THE HILL HIGHER HILLS TO SHOW OF THE STATE OF THE S
		William Shrail 1 - Section oneth males 1
		. White importance factor by and building slavete
		1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.
		c. Wind exposure, if more than one wind exposure is utilized, the
		applicable circlosure classifications and if dani-
		, michai nicking conficient
		c. Components and Cladding The design wind
		F (0 M / LO UC USCI IOI IIIP (APCION AT ANTANIA
		cladding materials not specifally designed by the registered design
		Elevations including:
3 /	D	a) All sides
Y _	0	b) Roof pitch
r		c) Overhang dimensions and detail with attic ventilation
		and uctain with affic ventilation

o NA	0	NIA	d) Location size and haight above and a t
0 NIA	0	NIA	d) Location, size and height above roof of chimneys. e) Location and size of skylights
	0	,,,	f) Building height
	0		e) Number of stories
			Floor Plan including:
0			a) Rooms labeled and dimensioned.
02			b) Shear walls identified.
Q/	0		c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
		NA	d) Show safety glazing of glass, where required by code
			e) Identify egress windows in bedrooms, and size
U		MIA	f) Fireplace (gas vented), (gas non-vented) or wood burning with
₽/	0		nearm, (Please circle applicable type).
	u		g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
0			2000 Sept. (Control of the Control o
2			h) Must show and identify accessibility requirements (accessible bathroom) Foundation Plan including:
G .			a) Location of all load-bearing wall with required footings indicated as standard
	943		or monorate and universions and remiorcing
			b) All posts and/or column footing including size and reinforcing
	0		c) Any special support required by soil analysis such as piling
W			d) Location of any vertical steel.
₽.			Roof System: a) Truss package including:
-			1. Truss layout and truss details signed and and and and
			 Truss layout and truss details signed and sealed by Fl. Pro. Eng. Roof assembly (FBC 106.1.1.2)Roofing system, materials,
			manuacturer, lastening requirements and product evaluation with
	_		wind resistance rating)
0			b) Conventional Framing Layout including:
*			Rafter size, species and spacing
			2. Attachment to wall and uplift 3. Ridge beam sized and weller forming
			 Ridge beam sized and valley framing and support details Roof assembly (FBC 106.1.1.2)Roofing systems, materials,
			manuacturer, tastening requirements and product evaluation with
			while resistance rating)
m			Wall Sections including:
	Ц		a) Masonry wall
			All materials making up wall Block size and mortar type with size and specing of minformation.
			 Block size and mortar type with size and spacing of reinforcement Lintel, tie-beam sizes and reinforcement
			4. Gable ends with rake beams showing reinforcement or gable trues
			and wan dracing details
			5. All required connectors with uplift rating and required number and
			Size of lasteners for continuous tie from roof to foundation shall be
			designed by a Windload engineer using the engineered roof truss plans.
			6. Roof assembly shown here or on roof system detail (FBC
			100.1.1.2) Rooning system, materials, manufacturer fastening
			requirements and product evaluation with resistance rating)
			/. Fire resistant construction (if required)
			8. Fireproofing requirements 9. Shoe type of termite treatment formiticide by the state of the s
	¥0		 Shoe type of termite treatment (termiticide or alternative method) Slab on grade
12			a. Vapor retarder 6mil. Polyethylene with joints lapped 6
			inches and sealed)
			b. Must show control joints synthetic fiber reinforcement or
			weiged life labric reinforcement and supports
			11. Indicate where pressure treated wood will be placed
			12. Provide insulation R value for the following:

				 a. Attic space b. Exterior wall cavity c. Crawl space (if applicable)
			ı	 b) Wood frame wall All materials making up wall Size and species of studs Sheathing size, type and nailing schedule Headers sized Gable end showing balloon framing detail or gable truss and wall hinge bracing detail All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
				 Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating) Fire resistant construction (if applicable) Fireproofing requirements Show type of termite treatment (termiticide or alternative method) Slab on grade
				a. Vapor retarder 6Mil. Polyethylene with joints lapped 6 inches and sealed b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports 12. Indicate where pressure treated wood will be placed 13. Provide insulation R value for the following:
	ŝ			b. Exterior wall cavity
				c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)
				Floor Framing System: a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer N/A
				b) Floor joist size and spacing
		0		c) Girder size and spacing
				d) Attachment of joist to girder
				e) Wind load requirements where applicable where applicable Plumbing Fixture layout Electrical layout including:
Ø				a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
⊡ ∠				o) coming rais
				c) Smoke detectors
D.				d) Service panel and sub-panel size and location(s)
	*			e) Meter location with type of service entrance (overhead or underground) f) Appliances and HVAC equipment
□				g) Arc Fault Circuits (AFCI) in bedrooms
				h) Exhaust fans in bathroom
	50			HVAC information
				a) Energy Calculations (dimensions shall match plans)
0			NIO	b) Manual J sizing equipment or equivalent computation
			10/14	c) Gas System Type (LP or Natural) Location and BTU demand of equipment Disclosure Statement for Owner Builders
				Disclosure Statement for Owner Builders
V				*** Notice Of Commencement Required Before Any Inspections Will Be Done Private Potable Water

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- 1. <u>Building Permit Application:</u> A current Building Permit Application form is to be completed and submitted for all residential projects.
- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
 (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- 4. <u>City Approval:</u> If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- 5. Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.
 - A development permit will also be required. Development permit cost is \$50.00
- 6. <u>Driveway Connection:</u> If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. <u>If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.</u>
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL, NOT ALLOW THIS –PLEASE DO NOT ASK

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS	THERMA- TALL	EXT FIDERGLASS 13/4	FL-5262
A. SWNGING	11 11	EXT STEEL 13/4	F6-3462
B. SLIDING			
C. SECTIONAL/ROLL UP	RAYNOR		FL 3610
D. OTHER			FL 3610
2. WINDOWS	BETTER-BILT	SERIES 740 FRAME FIN	F4 5438,23
A. SINGLE/DOUBLE HUNG	NIA	The transfer the	F4 5438,23
B. HORIZONTAL SLIDER	NIA		
C. CASEMENT			-
D. FIXED	NIA		
E. MULLION	BETTER - BILT	SERIES 740 FRAME FIR	EL 21124 ==
F. SKYLIGHTS	NIA	TRAILE PIN	FL 5438,23
G. OTHER	NA		
3. PANEL WALL			
A. SIDING	JAMES HARDI	7/4 XIZ PLANK	61
B. SOFFITS	BEYNOLDS	VENTED ALLM	FL 889, 122
C. STOREFRONTS		72071	
D. GLASS BLOCK	NIA		
E. OTHER	N) A		
4. ROOFING PRODUCTS	<u> </u>		
A. ASPHALT SHINGLES	CERTAIN-TEED	ARCHITECTURAL BOYR	
B. NON-STRUCT METAL	1	ARCHITECTURAL BOYR	FL250-R-1
C. ROOFING TILES	NA		
D. SINGLE PLY ROOF	NIA		
E. OTHER	FELT TAMKO	30 LB. ASPNALT	FL1814,3
5. STRUCT COMPONENTS		,	
A. WOOD CONNECTORS	SIMPSON/HUGHES	AS DER STRUCTURAL ENG	
B. WOOD ANCHORS	The state of the s	A A COPPA	
C. TRUSS PLATES		11 0	
D. INSULATION FORMS	NA	AS PER TRUSS ENG	
E. LINTELS	N/A N/A		
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS	G.P.	7/16 4x10 WINDSTROM DS8	
A.		7/16 4x10 WINDSTORM 058	

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE

3-26-07

System Summary:

Product Line: Smooth-Star Embossed Panel

Glass Style:
Crystalline, Brushed Nickel Caming
Door Style:
S900-1C
Sidelite Style:
S916SL-1C

Transom Style:

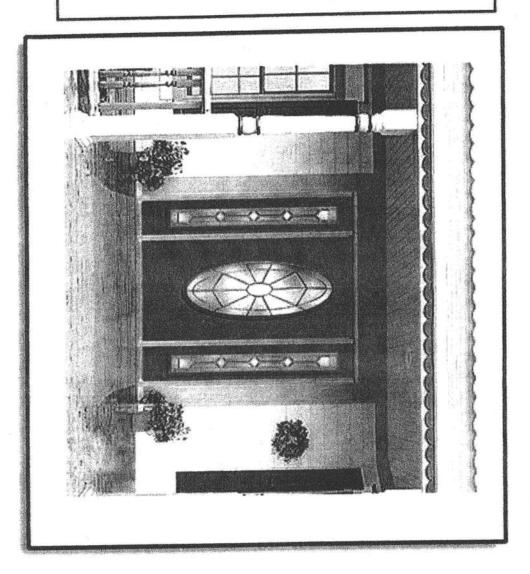
None

Door Height/Width:
3'0" x 6'8"
Sidelite Height/Width:
12" x 6'8"

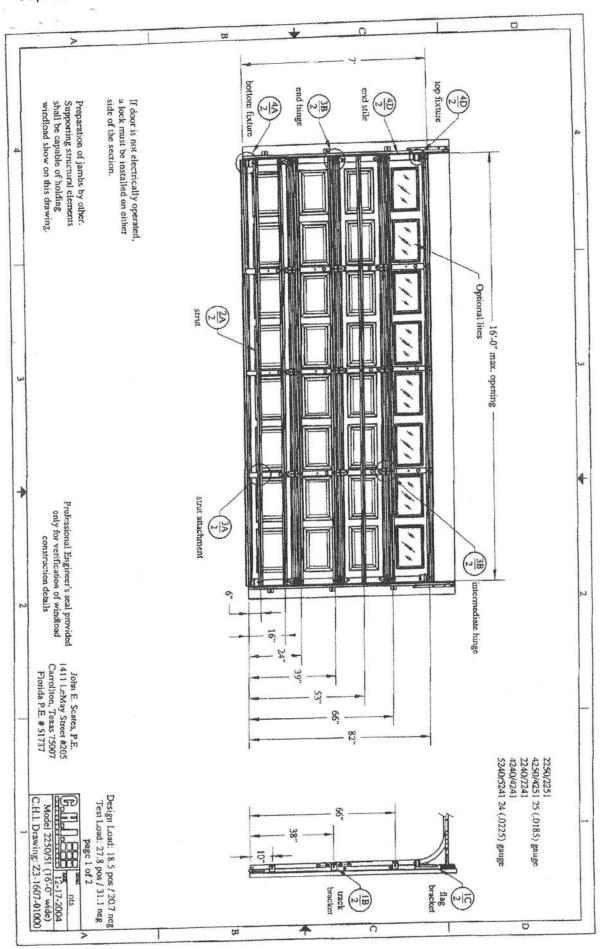
Number of Sidelites :

Stain :

Paint : Burgundy









Color Selections*



White



Almond



Sandstone



Brown

PANEL CONSTRUCTION

resistant feature, the tongue and groove design was engineered to ensure a secure fit while strengthening the structural integrity of the door.

2250/4250 Uninsulated

2251/4251

1-3/8" Polystyrene insulation with a vinyl back.

2255/4255

9/16" Polystyrene insulation with a viny back.

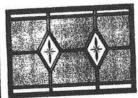
window glass options

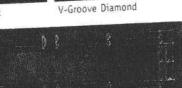
Add a touch of elegance to the outside of your home with the addition of decorative window & glass options from C.H.I. Our glass options are designed to enhance the natural beauty of your exterior while adding to the value of your home.

Designer Lites**

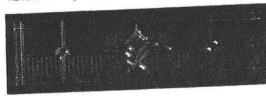


V-Groove Sunburst





Hawthorne Long



Somerset Long



V-Groove Cathedral



Hawthorne Short



Somerset Short

Note: Hawthorne and Somerset glazing options are transparent.

MODEL 2250 4250 2251 2	4251 3 2255 3 4255 WIDT	Hx HEIGHT
IOTES		
		7

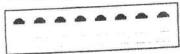
Standard Window Design Hims

SHORT PANEL

8 Piece Sunburst



2-4 Piece Sunburst



Cathedral

-27	1/2	's a'	74	-27	150	24	12

Sherwood

22		22	22	20 MH
				i

Stockton

183	222	123	500	220	10h	220	#HH

Cascade

LONG PANEL



4 Piece Sunburst



2-2 Piece Sunburst



Sherwood

2222	記書概念		なる。
自然を		AND THE SEA SEA	MID 400 AUT W

Stockton

1555	Seman.	公司的 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公司 公 公 公 公 公 公 公 公

Cascade

C.H.I. Doors Distributed by:

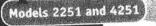
See your distributor for information about C.H.I.'s Limited Lifetime Warranty.

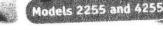
C.H.J. doors are manufactured in Arthur, Illinois, USA.

- Refer to samples at your local C.H.I. Distributor for exact color match.
- For all available window options, please see our Glass Options Brochure, or visit your C.H.I. Distributor.

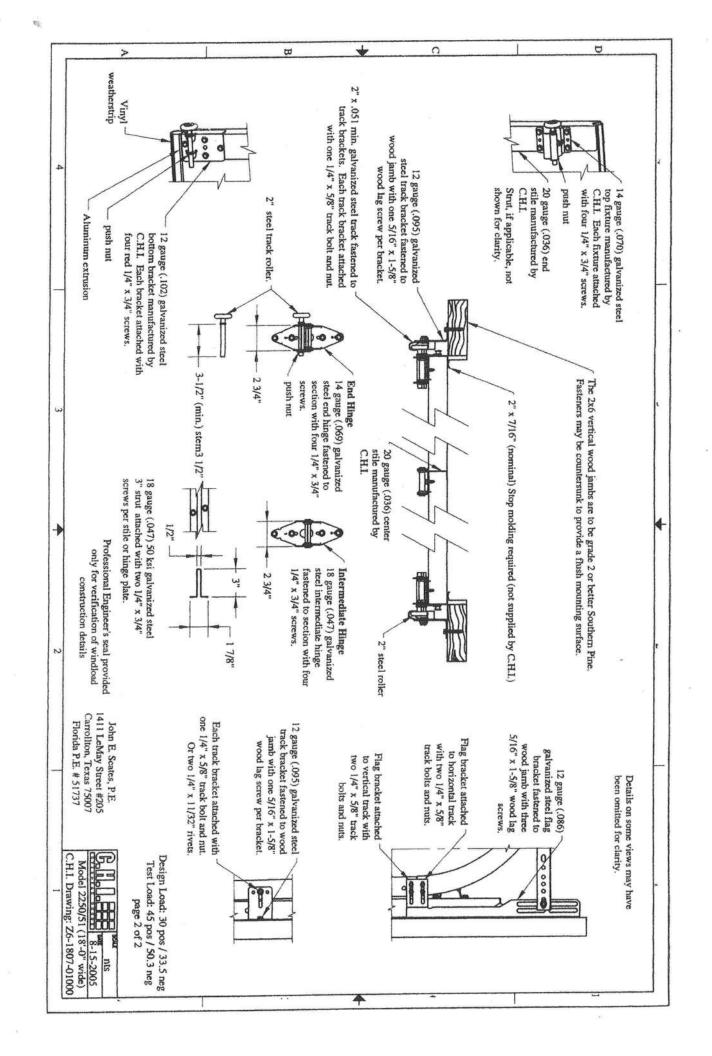
C.H.I. OVERHEAD DOORS 1485 SUNRISE ORIVE, ARTHUR, IL 6191 www.chiohd.com

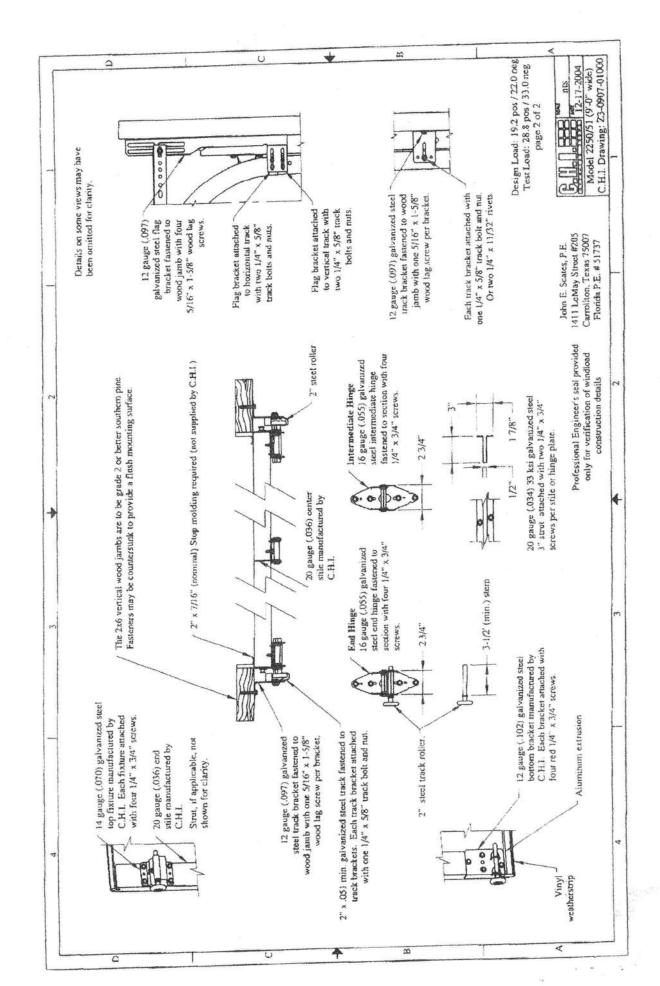






....

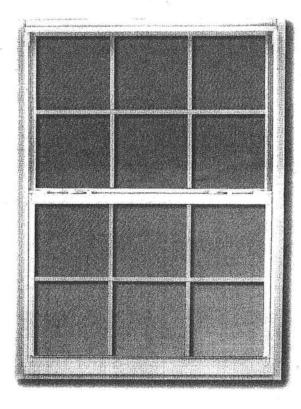




SERIES

7/4.0

Insulated Single Hung Aluminum Windows



- M Aluminum Single Hung Window
- 1/2" Insulated Glass
- Swiggle® Seal Glass Spacer System
- Bottom Sash Tilts, Top Sash Removable for Drywall Pass-Through
- **■** Spiral Balances
- Sweep Lock System at Meeting Rail
- Optional Decorative Grids Between the Glass (Insulated Glass Units)
- Single Glazed Available with Snap-In Grid System
- Twin and Triple Units in One Continuous Header and Sill Frame
- BetterBilt 10 Year Limited Warranty
- AAMA Labeled and NFRC Certified



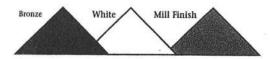


SINGLE HUNG WINDOW SIZES
CODE — 1-6 CTUAL SIZE — 17 1/4 ROUGH — 17 3/4 OPENING — 17 3/4 OPENING — 17 3/4
27.14
- 43 1/4 - 43 1/2 - 4
47.74
- 51 14 - 51 12 - 51 1
59.74 - 59.74 - 59.74
— 6-0 ORIEL — 71 1/4 — 71 1/2 — 11/2
7-0 ORIEL—83 1/4 83 1/2
95 1/4 95 1/4

	PIC	TU	RE W	INDO)W S	IZES		
CC ACTUAL S ROU OPENI	DE -1-0 IZE -111 GH -113 NG -113	4	1-6 - 2-0 17 44 - 23 1/4 1734 - 23 3/4	2-6 291/4 293/4	3-0 - 35 14- 35 34-	- 4-0 - - 47 1/4 - - 47 3/4 -	5-0 — 591/4 — 593/4 —	6-0 — — 71 1/4 — — 71 3/4 —
	江			125	國際		EZODER	
	174	0			++-		EHEE	
	231/2						144	
	- 35 74 35 72							
	- 3-8 - - 4314 - - 4312 -							
	- 40 - - 4744 - - 4712 -							
	51 1/4							
	50 T 59 T							
	02							

Also available: Series 744 High Performance Window with DH-R50 Rating

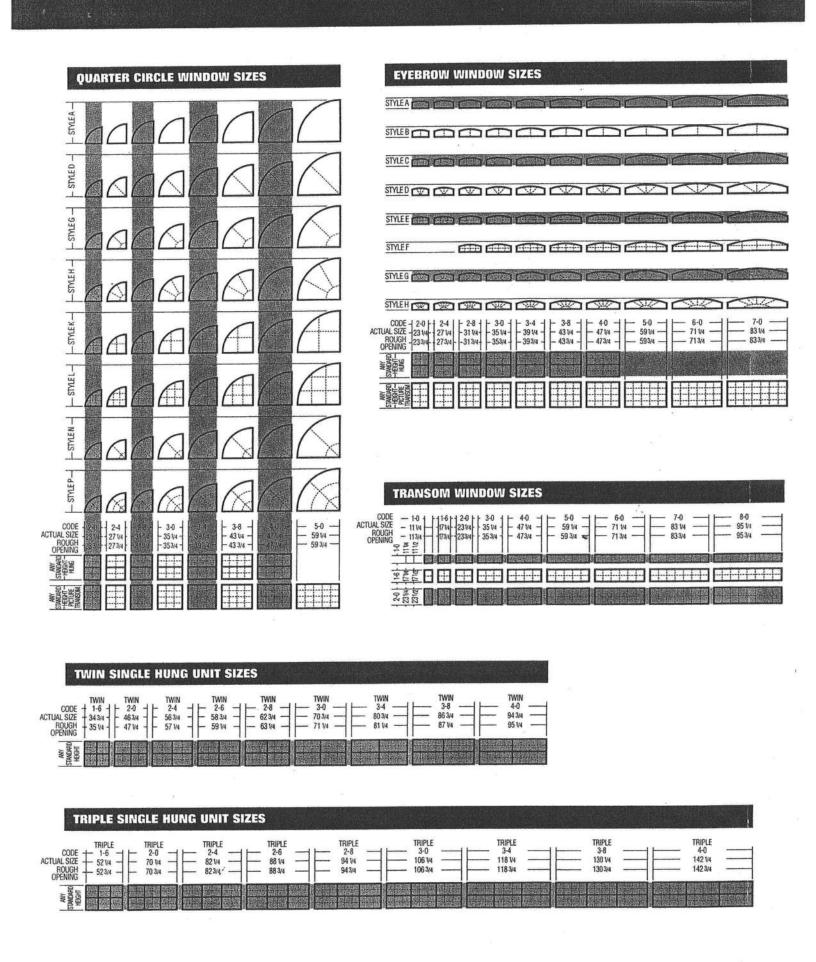
Colors











HALF EYE	EBROW WINDOW	I SIZES		
STYLEA				
STYLE B				
STYLEC				
STYLED				
STYLEE				
STYLEF				
STYLEH CODE 2-0 2-4 ACTUAL SIZE 23 Val 27 Va				
ACTUAL SIZE -23 1/4 - 27 1/4 ROUGH -233/4 - 273/4	1 - 31 14 35 14 39 14	- 3-8 - 4-0 - - 43 ¼ - 47 ¼ - - 43 ¾ - 47 ¾ -	5-0 — 6-0 — 59 ¼ — 71 ¼ — 59 ¾ — 71 ¾	83 1/4
STANDAS HEISHT- HUNG				
ANY STAUDARD HEIGHT PHEIGHT PHEIGHT TRANSON				

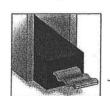
CIRCLE TOP WINDOW SIZES					
STYLEA	200				
STYLEB					
\$25\$255 E					
製鋼 .					
- 開始 補					
ACTUAL SIZE - 271/4	- 3-0 - - 35 ¼4 - - 35¾4 - - 35¾4 -	38 - 13 - 4314 - 17 - 4334 - 47	5-0 6-0	7-0 8314 8334 8334	

Swiggle Seal Glass Spacer System

Swiggle Seal is a revolutionary seal system which works two ways to prevent condensation and the transference of heat and cold between panes of glass:

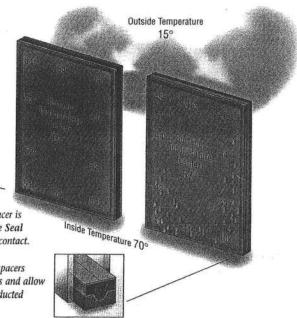
- **1** The advanced seal uses a specially formulated desiccant to actually absorb stray moisture and prevent damaging condensation.
- **2** A corrugated aluminum spacer is completely surrounded by the seal, eliminating the conductive metal to glass contact that causes traditional windows to lose much of their insulating properties.

You'll enjoy a clearer view and less energy loss with BetterBilt windows using Swiggle Seal.



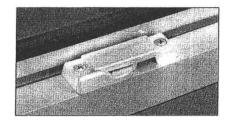
A corrugated aluminum spacer is encapsulated in the Swiggle Seal to eliminate glass to metal contact.

> Traditional aluminum spacers directly contact the glass and allow heat and cold to be conducted through the window.





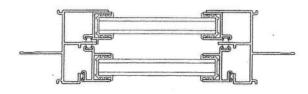
Bottom sash tilts for easy cleaning.



Two sweep locks at the meeting rail provide extra security.

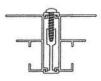
Vertical Detail

Horizontal Detail



Mullion Available

TWO PIECE MULL SYSTEM
INTERIOR-558, EXTERIOR-557, #10 x 1" SCREW
1/4" ADD ON





SINGLE HUNG OPENING SPECIFICATIONS

		SHOLE	NOMO OPENIN	u specificali	UNS	
CODE SIZE	ACTUAL WINDOW STZE	SASH RAISED Clear Opening Sq. Ft		SCREEN SIZE Width & Height	TOP GLASS SIZE Width x Height	RTM GLASS SIZE Width x Height
1624 1630 1638 1640	17 1/4 x 27 1/4 17 1/4 x 35 1/4 17 1/4 x 43 1/4 17 1/4 x 47 1/4	1.01 1.42 1.82 2.02	14 1/2 x 10 1/16 14 1/2 x 14 1/16 14 1/2 x 18 1/16 14 1/2 x 20 1/16	15 1/8 x 13 7/16 15 1/8 x 17 7/16 15 1/8 x 21 7/16	15 x 11 3/4 15 x 15 3/4 15 x 19 3/4	15 x 11 3/4 15 x 15 3/4 15 x 19 3/4
1644 1650 1660 Oriel	17 1/4 x 51 1/4 17 1/4 x 59 1/4 17 1/4 x 71 1/4	2.22 2.62 2.54	14 1/2 x 22 1/16 14 1/2 x 22 1/16 14 1/2 x 26 1/16 14 1/2 x 25 3/16	15 1/8 x 23 7/16 15 1/8 x 25 7/16 15 1/8 x 29 7/16 15 1/8 x 29 7/16	15 x 21 3/4 15 x 23 3/4 15 x 27 3/4 15 x 39 3/4	15 x 21 3/4 15 x 23 3/4 15 x 27 3/4 15 x 27 3/4
2024 2030 2038 2040	23 1/4 x 27 1/4 23 1/4 x 35 1/4 23 1/4 x 43 1/4 23 1/4 x 47 1/4	1.43 2.00 2.57 2.86	20 1/2 x 10 1/16 20 1/2 x 14 1/16 20 1/2 x 18 1/16 20 1/2 x 20 1/16	21 1/8 x 13 7/16 21 1/8 x 17 7/16 21 1/8 x 21 7/16 21 1/8 x 23 7/16	21 x 11 3/4 21 x 15 3/4 21 x 19 3/4 21 x 21 3/4	21 x 11 3/4 21 x 15 3/4 21 x 19 3/4
2044 2050 2060 Oriel	23 1/4 x 51 1/4 23 1/4 x 59 1/4 23 1/4 x 71 1/4	3.14 3.71 3.59	20 1/2 x 22 1/16 20 1/2 x 26 1/16 20 1/2 x 25 3/16	21 1/8 x 25 7/16 21 1/8 x 29 7/16 21 1/8 x 29 7/16	21 x 23 3/4 21 x 23 3/4 21 x 27 3/4 21 x 39 3/4	21 x 21 3/4 21 x 23 3/4 21 x 27 3/4 21 x 27 3/4
2424 2430 2438 2440	27 1/4 x 27 1/4 27 1/4 x 35 1/4 27 1/4 x 43 1/4 27 1/4 x 47 1/4	1.71 2.39 3.07 3.41	24 1/2 x 10 1/16 24 1/2 x 14 1/16 24 1/2 x 18 1/16 24 1/2 x 20 1/16	25 1/8 x 13 7/16 25 1/8 x 17 7/16 25 1/8 x 21 7/16 25 1/8 x 23 7/16	25 x 11 3/4 25 x 15 3/4 25 x 19 3/4 25 x 21 3/4	25 x 11 3/4 25 x 15 3/4 25 x 19 3/4 25 x 21 3/4
2444 2450 2460 Oriel 2624	27 1/4 x 51 1/4 27 1/4 x 59 1/4 27 1/4 x 71 1/4 29 1/4 x 27 1/4	3.75 4.43 4.29 1.85	24 1/2 x 22 1/16 24 1/2 x 26 1/16 24 1/2 x 25 3/16 29 1/2 x 10 1/16	25 1/8 x 25 7/16 25 1/8 x 29 7/16 25 1/8 x 29 7/16 27 1/8 x 13 7/16	25 x 23 3/4 25 x 27 3/4 25 x 39 3/4	25 x 23 3/4 25 x 27 3/4 25 x 27 3/4
2630 - 2638 2640 2644	29 1/4 x 35 1/4 29 1/4 x 43 1/4 29 1/4 x 47 1/4 29 1/4 x 51 1/4	2.59 3.32 3.69 4.06	29 1/2 x 14 1/16 29 1/2 x 14 1/16 29 1/2 x 18 1/16 29 1/2 x 20 1/16 29 1/2 x 22 1/16	27 1/8 x 17 7/16 27 1/8 x 17 7/16 27 1/8 x 21 7/16 27 1/8 x 23 7/16 27 1/8 x 25 7/16	27 x 11 3/4 27 x 15 3/4 27 x 19 3/4 27 x 21 3/4 27 x 23 3/4	27 x 11 3/4 27 x 15 3/4 27 x 19 3/4 27 x 21 3/4
2650 2660 Oriel 2830 2838	29 1/4 x 59 1/4 29 1/4 x 71 1/4 31 1/4 x 35 1/4 31 1/4 x 43 1/4	4.80 5.16 2.78 3.57	29 1/2 x 26 1/16 29 1/2 x 25 3/16 28 1/2 x 14 1/16 28 1/2 x 18 1/16	27 1/8 x 29 7/16 27 1/8 x 29 7/16 29 1/8 x 17 7/16	27 x 27 3/4 27 x 39 3/4 29 x 15 3/4	27 x 23 3/4 27 x 27 3/4 27 x 27 3/4 29 x 15 3/4
2840 2844 ~ 2850 2860 Oriel	31 1/4 x 47 1/4 31 1/4 x 51 1/4 31 1/4 x 59 1/4 31 1/4 x 71 1/4	3.97 4.37 5.16 4.99	28 1/2 x 20 1/16 28 1/2 x 20 1/16 28 1/2 x 22 1/16 28 1/2 x 26 1/16 28 1/2 x 25 3/16	29 1/8 x 21 7/16 29 1/8 x 23 7/16 29 1/8 x 25 7/16 29 1/8 x 29 7/16 29 1/8 x 29 7/16	29 x 19 3/4 29 x 21 3/4 29 x 23 3/4 29 x 27 3/4 29 x 39 3/4	29 x 19 3/4 29 x 21 3/4 29 x 23 3/4 29 x 27 3/4 29 x 27 3/4
3030 3038 3040 3044	35 1/4 x 35 1/4 35 1/4 x 43 1/4 35 1/4 x 47 1/4 35 1/4 x 51 1/4	3.17 4.08 4.53 4.98	32 1/2 x 14 1/16 32 1/2 x 18 1/16 32 1/2 x 20 1/16 32 1/2 x 22 1/16	33 1/8 x 17 7/16 33 1/8 x 21 7/16 33 1/8 x 21 7/16 33 1/8 x 23 7/16 33 1/8 x 25 7/16	33 x 15 3/4 33 x 19 3/4 33 x 21 3/4	33 x 15 3/4 33 x 19 3/4 33 x 21 3/4
3050 3060 Oriel 3070 Oriel 3080 Oriel	35 1/4 x 59 1/4 35 1/4 x 71 1/4 35 1/4 x 83 1/4 35 1/4 x 95 1/4	5.88 5.68 5.68 5.68	32 1/2 x 26 1/16 32 1/2 x 25 3/16 32 1/2 x 25 3/16 32 1/2 x 25 3/16 32 1/2 x 25 3/16	33 1/8 x 29 7/16 33 1/8 x 29 7/16 33 1/8 x 29 7/16 33 1/8 x 29 7/16 33 1/8 x 29 7/16	33 x 23 3/4 33 x 27 3/4 33 x 39 3/4 33 x 51 3/4	33 x 23 3/4 33 x 27 3/4 33 x 27 3/4 33 x 27 3/4
3430 3438 3440 3444	39 1/4 x 35 1/4 39 1/4 x 43 1/4 39 1/4 x 47 1/4	3.56 4.58 5.09	36 1/2 x 14 1/16 36 1/2 x 18 1/16 36 1/2 x 20 1/16	37 1/8 x 17 7/16 37 1/8 x 21 7/16 37 1/8 x 23 7/16	33 x 63 3/4 37 x 15 3/4 37 x 19 3/4 37 x 21 3/4	33 x 27 3/4 37 x 15 3/4 37 x 19 3/4 37 x 21 3/4
3450 3460 Oriel 3470 Oriel 3480 Oriel	39 1/4 x 51 1/4 39 1/4 x 59 1/4 39 1/4 x 71 1/4 39 1/4 x 83 1/4 39 1/4 x 95 1/4	5.59 6.61 6.38 6.38 6.38	36 1/2 x 22 1/16 36 1/2 x 26 1/16 36 1/2 x 25 3/16 36 1/2 x 25 3/16 36 1/2 x 25 3/16	37 1/8 x 25 7/16 37 1/8 x 29 7/16 37 1/8 x 29 7/16 37 1/8 x 29 7/16	37 x 23 3/4 37 x 27 3/4 37 x 39 3/4 37 x 51 3/4	37 x 23 3/4 37 x 27 3/4 37 x 27 3/4 37 x 27 3/4
3830 3838 3840 3844	43 1/4 x 35 1/4 43 1/4 x 43 1/4 43 1/4 x 47 1/4	3.96 5.08 5.64	40 1/2 x 14 1/16 40 1/2 x 18 1/16 40 1/2 x 20 1/16	37 1/8 x 29 7/16 41 1/8 x 17 7/16 41 1/8 x 21 7/16 41 1/8 x 23 7/16	37 x 63 3/4 41 x 15 3/4 41 x 19 3/4 41 x 21 3/4	37 x 27 3/4 41 x 15 3/4 41 x 19 3/4 41 x 21 3/4
3850 3860 Oriel 3870 Oriel 3880 Oriel	43 1/4 x 51 1/4 43 1/4 x 59 1/4 43 1/4 x 71 1/4 43 1/4 x 83 1/4 43 1/4 x 95 1/4	6.21 7.33 7.08 7.08 7.08	40 1/2 x 22 1/16 40 1/2 x 26 1/16 40 1/2 x 25 3/16 40 1/2 x 25 3/16 40 1/2 x 25 3/16	41 1/8 x 25 7/16 41 1/8 x 29 7/16 41 1/8 x 29 7/16 41 1/8 x 29 7/16 41 1/8 x 29 7/16	41 x 23 3/4 41 x 27 3/4 41 x 39 3/4 41 x 51 3/4 41 x 63 3/4	41 x 23 3/4 41 x 27 3/4 41 x 27 3/4 41 x 27 3/4
4030 4038 4040 4044	47 1/4 x 35 1/4 47 1/4 x 43 1/4 47 1/4 x 47 1/4 47 1/4 x 51 1/4	4.35 5.58 6.20 6.82	44 1/2 x 14 1/16 44 1/2 x 18 1/16 44 1/2 x 20 1/16	45 1/8 x 17 7/16 45 1/8 x 21 7/16 45 1/8 x 23 7/16	45 x 15 3/4 45 x 19 3/4 45 x 21 3/4	41 x 27 3/4 45 x 15 3/4 45 x 19 3/4 45 x 21 3/4
4050 4060 Oriel 4070 Oriel 4080 Oriel	47 1/4 x 59 1/4 47 1/4 x 59 1/4 47 1/4 x 71 1/4 47 1/4 x 83 1/4 47 1/4 x 95 1/4	8.05 7.78 7.78 7.78	44 1/2 x 22 1/16 44 1/2 x 26 1/16 44 1/2 x 25 3/16 44 1/2 x 25 3/16 44 1/2 x 25 3/16	45 1/8 x 25 7/16 45 1/8 x 29 7/16 45 1/8 x 29 7/16 45 1/8 x 29 7/16 45 1/8 x 29 7/16	45 x 23 3/4 45 x 27 3/4 45 x 39 3/4 45 x 51 3/4	45 x 23 3/4 45 x 27 3/4 45 x 27 3/4 45 x 27 3/4
	,	7.70	11 1/2 x 25 5/10	45 1/6 X 29 //10	45 x 63 3/4	45 x 27 3/4



BetterBilt Doors & Windows

East Region: 704 12th Avenue • Smyrna, TN 37167 • 1-800-545-5413 • Fax: 1-800-255-8106 West Region: 7555 East Highway 69 • Prescott Valley, AZ 86314 • 1-800-468-0304 • Fax: 1-800-635-2718

www.mihomeproducts.com

Hung Windows.pdf
PTID 5438 I Installation instructions BetterBilt Nail Fin Alum Windows.pdf
PTID 5438 I Installation instructions BetterBilt Nail Fin Vinyl Windows.pdf
PTID 5438 I Installation instructions Capitol Nail Fin Alum Windows.pdf
PTID 5438 I Installation instructions Capitol Nail Fin Alum Windows.pdf
PTID 5438 I Installation instructions Capitol Nail Fin Vinyl Windows.pdf

Product Approval Method:

Method 1 Option A

Application Status:

Approved

Date Validated:

10/14/2005

Date Approved:

10/17/2005

Date Certified to the 2004 Code:

Go.

|< | < | p

Page 2 / 2

App/Seq	Product Model # or Name	Model Description	Limits of Use
	740/3740 Fin Frame	52x71 Insulated DSB Annealed	R-45 DP-47.2 Per manufacturers installation instructions.
5438.22	740/3740 Fin Frame	52x71 Single Glazed 3/16" Annealed	R-35 DP-47.2 Per manufacturers installation instructions.
5438.23	740/3740 Fin Frame Oriel	52x71 Single Glazed 3/16" Annealed	R-35 DP-47.2 Per manufacturers installation instructions.
5438.24	740/3740 Fin Frame Oriel	47x89 Single Glazed 3/16" Annealed	R-35 DP-47.2 Per manufacturers installation instructions.
5438.25	740/3740 Fin Frame Oriel	39x90 Single Glazed 3/16" Annealed Sash / DSB Tempered Fixed	R-35* DP-47.2 Per manufacturers installation instructions.
5438.26	740/3740 Flange Frame	52x71 Single Glazed DSB Tempered	R-45 DP-47.2 Per manufacturers installation instructions.
5438.27	740/3740 Flange Frame	52x71 Insulated DSB Annealed	R-45 DP-47.2 Per manufacturers installation instructions.
5438.28	740/3740 Flange Frame	53x72 Single Glazed 3/16" Annealed	R-25 DP-34.7 Per manufacturers installation instructions.
5438.29	740/3740 Flange Frame Oriel	47x89 Single Glazed DSB Tempered	R-35 DP-42.9 Per manufacturers installation instructions.
5438.30	740/3740 Flange Frame Oriel	47x89 Insulated 3/16" Annealed	R-35 DP-42.7 Per manufacturers installation instructions.
	740/3740 Flange Frame	36x88 Insulated 3/16" Annealed	R-35* DP-47.2 Per

		*	
5418.13	740 Flange Frame	71x71 Insulated DSB Tempered	R-35 DP -45.3 Per manufacturers installation instructions.
5418.14	740 Flange Frame	59x72 Insulated 3/16" Annealed	R-45 DP -47.2 Per manufacturers installation instructions.
5418.15	740/3740 Flange Frame	109x53 Single Glazed DSB Tempered	R-40 DP -40 Per manufacturers installation instructions.
5418.16	8500 Fin Frame	65x84 Insulated DSB Annealed	R-30 DP -40 Per manufacturers installation instructions.
5418.17	8500/1250 Finless Frame	72x96 Insulated DSB Tempered	R-30 DP -35 Per manufacturers installation instructions.
5418.18	8500/1250 Finless Frame	62x80 Insulated DSB Annealed	R-45 DP -45 Per manufacturers installation instructions.
5418.19	8500/1250 Finless Frame	48x48 Insulated DSB Annealed	R-65 DP -70 Per manufacturers installation instructions.
5418.20	Insight Series	62x63 Insulated DSB Annealed	R-25 DP -34.7 Per manufacturers installation instructions.

Next



Copyright and Disclaimer; ©2000 The State of Florida. All rights reserved.



10. 2000 1. 2

BetterBilt Nail Fin Alum Windows.pdf PTID 5418 I Installation instructions -BetterBilt Nail Fin Vinyl Windows.pdf PTID 5418 I Installation instructions -Capitol Nail Fin Alum Windows.pdf PTID 5418 I Installation instructions -Capitol Nail Fin Vinyl Windows.pdf

Product Approval Method:

Method 1 Option A

Application Status:

Approved

Date Validated:

10/14/2005

Date Approved:

10/17/2005

Date Certified to the 2004 Code:

Page:

Page 1/2 |>| >|

100 2001

App/Seq #	Product Model # or Name	Model Description	Limits of Use
	165 Fin Frame		C-35 DP -47.2 Per manufacturers installation instructions.
5418.2	165 Fin Frame	71x71 Single Glazed DSB Tempered	R-45 DP -47.2 Per manufacturers installation instructions.
5418.3	165 Fin Frame	59x72 Insulated 3/16" Annealed	R-45 DP -47.2 Per manufacturers installation instructions.
5418.4	165 Flange Frame	59x72 insulated 3/16" Annealed	R-45 DP -47.2 Per manufacturers installation instructions.
5418.5	165/3000 Flange Beveled Frame	72x72 Single Glazed 3/16" Tempered	C-45 DP -47.2 Per manufacturers installation instructions.
5418.6	4300/4340 Fin Frame	60x72 DSB Annealed	LC-45 DP -50 Per manufacturers installation instructions.
5418.7	4300/4340 Fin Frame	48x48 SSB Annealed	LC-60 DP -60 Per manufacturers installation instructions.
5418.8	4300/4340 Fin Frame	65x84 DSB Annealed	LC-30 DP -35 Per manufacturers installation instructions.
5418.9	650 Flange Frame	60x80 Insulated 3/16* Annealed	R-45 DP -47.2 Per manufacturers installation instructions.
5418.10	740 Fin Frame	59x72 insulated 3/16" Annealed	R-45 DP -47.2 Per manufacturers installation instructions.
5418.11	740 Fin Frame	71x71 Single Glazed DSB Tempered	R-45 DP -47.2 Per manufacturers installation instructions.
5418.12	740 Flange Frame	59x72 Insulated 3/16" Annealed	R-45 DP -47.2 Per manufacturers installatio instructions.

MI HOME PRODUCTS - PRIME ALUMINUM WINDOWS INSTALLATION INSTRUCTIONS FOR "NAIL FIN" PRODUCTS

MI Home Products appreciates your recent purchase of a maintenance free prime window, which will not rust, rot, mildew, or warp. This is a quality product that left our factory in good condition – proper handling and installation are just as important as good design and workmanship. Please follow these recommendations to allow this product to complete its function.

- Handle units one at a time in the closed and locked position and take care not to scratch frame or glass
 or to bend the nailing fin.
- 2. Set unit plumb and square into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Fasten unit into opening in the closed and locked position, making sure that fasteners are screwed in straight in order to avoid twisting or bowing of the frame. Make sure that sill is straight and level. Check operation of unit before any and all fasteners are set.
- 3. Use # 8 sheet metal or wood screws with a minimum of 1" penetration into the framing (stud). Place first screws (two at each comer) 3" from end of fin. For positive and negative DPs (design pressures) up to 35, do not exceed 24" spacing of additional screws. For DPs from 35.1 to 50, do not exceed 18". Install load bearing shim adjacent to each anchor. Use shim where space exceeds 1/16".
- Flash over head and caulk outside perimeter in accordance with code requirements and good installation practices.
- Fill voids between frame and construction with loose batten type insulation or <u>non-expanding</u> aerosol foam specifically formulated for windows and doors to eliminate drafts. The use of <u>expanding</u> aerosol type insulating foam, which can bow the frame, waives all stated warranties.
- 6. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks and interlocks are also clear. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent as you would you automobile.

- CAUTION -

MI Home Products or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. MI Home Products window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing near doors, bathtubs, and shower enclosures. Also be aware of emergency egress code requirements.

Corporate Headquarters: 350 West Market St. Gratz, PA 17030-0370 (717) 365-3300







BCIS Home Log In Hot Topics Submit Surcharge

Stats & Facts

Publications

FBC Staff



Product Approval Menu > Product or Application Search > Application List > Application Detail

DMMUNITY PLANNING

FICE OF THE

FL #

Application Type

Code Version

Application Status

Comments

Archived

FL250-R1

Revision

2004

Approved

Product Manufacturer

Address/Phone/Email

CertainTeed Corporation-Roofing

PO Box 1100

1400 Union Meeting Rd Blue Bell, PA 19422 (610) 341-6678

allan.r.snyder@saint-gobain.com

Authorized Signature

Richard Snyder

allan.r.snyder@saint-gobain.com

Technical Representative Address/Phone/Email

Quality Assurance Representative Address/Phone/Email

Category

Subcategory

Roofing

Asphalt Shingles

Compliance Method

Certification Mark or Listing

Certification Agency

Miami-Dade BCCO - CER

Referenced Standard and Year (of	Standard
Standard)	ASTM D3462
	ASTM E108
	TAS 100
	TAS 107
	TAS 110

UL 790

Equivalence of Product Standards Certified By

Product Approval Method	Method 1 Option A
Date Submitted	09/08/2005
Date Validated	09/08/2005
Date Pending FBC Approval	09/27/2005
Date Approved	10/11/2005

Summary of P	roducts	
FL#	Model, Number or Name	Description
250.1	Carriage House Shangle	Fiberglass laminated shin
Approved for Impact Resis Design Press	(See Other) use in HVHZ: use outside HVHZ: stant:	Certification Agency Certification Instruction PTID 250 R1 I 010503 Series.pdf PTID 250 R1 I 010612 PTID 250 R1 I 010612 PTID 250 R1 I 020110 Series.pdf PTID 250 R1 I 021216 CT20 XT25 XT30 Patriot PTID 250 R1 I 021219 Series.pdf PTID 250 R1 I 040616 Grand Manor.pdf Verified By:
250.2	Classic Horizon Shangle	Fiberglass 3 tab overlay s
Impact Resist Design Pressu	use in HVHZ: use outside HVHZ: ant:	Certification Agency Ce Installation Instruction Verified By:

250.3	CT 20 (and AR)	Fiberglass 3 tab shingle
Impact Resista Design Pressur	ee Other) se in HVHZ: se outside HVHZ: nt:	Certification Agency Ce Installation Instruction Verified By:
250.4	Grand Manor Shangle	Fiberglass laminated shin
Impact Resista Design Pressure	se in HVHZ: se outside HVHZ: nt:	Certification Agency Ce Installation Instruction Verified By:
250.5	Hatteras	Fiberglass 4 tab shingle
Impact Resistar Design Pressure	se in HVHZ: se outside HVHZ: nt:	Certification Agency Ce Installation Instruction Verified By:
250.6	Landmark 30 (and AR)	Fiberglass laminated shing
Impact Resistar Design Pressure	e in HVHZ: e outside HVHZ: at:	Certification Agency Ce Installation Instruction Verified By:
250.7	Landmark 40 (and AR)	Fiberglass laminate shingl
Limits of Use (Se Approved for us Approved for us Impact Resistan Design Pressure Other: Refer to cu	e Other) e in HVHZ: e outside HVHZ: t:	Certification Agency Ce Installation Instruction Verified By:
250.8	Landmark 50 (and AR)	Fiberglass laminated shing
Limits of Use (See Approved for use Approved for use Impact Resistan Design Pressure Other: Refer to cu	e Other) e in HVHZ: e outside HVHZ: t:	Certification Agency Ce Installation Instruction Verified By:
250.9	Landmark TL	Fiberglass laminated shine
Limits of Use (See Approved for use Approved for use	e Other)	Certification Agency Ce Installation Instruction Verified By:

Fiberglass 3 tab shingle Certification Agency Ce Installation Instruction Verified By:
Certification Agency Co Installation Instruction Verified By:
Fiberglass architectural sh
Certification Agency Ce Installation Instruction Verified By:
Fiberglass architectural sh
Certification Agency Ce Installation Instruction Verified By:
Fiberglass architectural sh
Certification Agency Ce Installation Instruction Verified By:
Fiberglass 3 tab shingle
Certification Agency Ce Installation Instruction Verified By:
Fiberglass 3 tab shingle
Certification Agency Ce Installation Instruction Verified By:

Back

Next

DCA Administration

Department of Community Affairs Florida Building Code Online Codes and Standards

2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
© 2000-2005 The State of Florida. All rights reserved. Copyright and Discl
Product Approval Accepts:



















BCIS Home | Log In | Hot Topics | Submit Surcharge | Stats & Facts | Publications

FBC Staff B



Product Approval Menu > Product or Application Search > Application List > Application Detail

FL #

Application Type

Code Version

Application Status

Comments

Archived

FL250-R1

Revision

2004

Approved

Product Manufacturer

Address/Phone/Email

CertainTeed Corporation-Roofing

PO Box 1100

1400 Union Meeting Rd Blue Bell, PA 19422 (610) 341-6678

allan.r.snyder@saint-gobain.com

Authorized Signature

Richard Snyder

allan.r.snyder@saint-gobain.com

Technical Representative Address/Phone/Email

Quality Assurance Representative Address/Phone/Email

Category

Subcategory

Roofing

Asphalt Shingles

Compliance Method

Certification Mark or Listing

Certification Agency

Miami-Dade BCCO - CER

Referenced Standard and Year (of	Standard		
Standard)	ASTM D3462		
	ASTM E108		
	TAS 100		
	TAS 107		
	TAS 110		
	UL 790		

Equivalence of Product Standards Certified By

Product Approval Method	Method 1 Option A
Date Submitted	09/08/2005
Date Validated	09/08/2005
Date Pending FBC Approval	09/27/2005
Date Approved	10/11/2005

Summary of Products							
FL#	Model, Number or Name	Description					
250.1	Carriage House Shangle	Fiberglass laminated shine					
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: Refer to current Miami-Dade NOA.		Certification Agency Certification Instruction PTID 250 R1 I 0105036 Series.pdf PTID 250 R1 I 0105166 PTID 250 R1 I 0106126 PTID 250 R1 I 0201106 Series.pdf PTID 250 R1 I 0212166 CT20 XT25 XT30 Patriot PTID 250 R1 I 0212196 Series.pdf PTID 250 R1 I 0212196 Series.pdf PTID 250 R1 I 0212196 Grand Manor.pdf Verified By:					
250.2	Classic Horizon Shangle	Fiberglass 3 tab overlay s					
Impact Resistar Design Pressure	e in HVHZ: e outside HVHZ: at:	Certification Agency Ce Installation Instruction Verified By:					
E-	11	1					

250.3	CT 20 (and AR)	Eiborolpon 2 to be altitud
Limits of Use Approved for Approved for Impact Resis Design Press	(See Other) use in HVHZ: use outside HVHZ: tant:	Fiberglass 3 tab shingle Certification Agency Ce Installation Instruction Verified By:
250.4	Grand Manor Shangle	Fiberglass laminated shine
Approved for Impact Resis Design Press	(See Other) use in HVHZ: use outside HVHZ: tant:	Certification Agency Ce Installation Instruction Verified By:
250.5	Hatteras	Fiberglass 4 tab shingle
Impact Resist Design Pressu	use in HVHZ: use outside HVHZ: ant:	Certification Agency Ce Installation Instruction Verified By:
250.6	Landmark 30 (and AR)	Fiberglass laminated shine
Impact Resist Design Pressu	use in HVHZ: use outside HVHZ: ant:	Certification Agency Ce Installation Instruction Verified By:
250.7	Landmark 40 (and AR)	Fiberglass laminate shingl
Impact Resist Design Pressu	See Other) use in HVHZ: use outside HVHZ: ant:	Certification Agency Ce Installation Instruction Verified By:
250.8	Landmark 50 (and AR)	Fiberglass laminated shing
Impact Resista Design Pressu	See Other) use in HVHZ: use outside HVHZ: ant:	Certification Agency Ce Installation Instruction Verified By:
250.9	Landmark TL	Fiberglass laminated shine
Limits of Use (S Approved for u Approved for u		Certification Agency Ce Installation Instruction Verified By:

Impact Resistant: Design Pressure: +/- Other: Refer to current Miami-Dade NOA.	
250.10 Patriot AR	Fiberglass 3 tab shingle
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: Refer to current Miami-Dade NOA.	Certification Agency Ce Installation Instruction Verified By:
250.11 Presidential Shake (and AR)	Fiberglass architectural sh
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: Refer to current Miami-Dade NOA.	Certification Agency Ce Installation Instruction Verified By:
250.12 Presidential Shake TL	Fiberglass architectural sh
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: Refer to current Miami-Dade NOA.	Certification Agency Ce Installation Instruction Verified By:
250.13 Presidential Shake TL AR	Fiberglass architectural sh
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: Refer to current Miami-Dade NOA.	Certification Agency Ce Installation Instruction Verified By:
250.14 XT 25 (and AR)	Fiberglass 3 tab shingle
Limits of Use (See Other) Approved for use in HVHZ: Approved for use outside HVHZ: Impact Resistant: Design Pressure: +/- Other: Refer to current Miami-Dade NOA.	Certification Agency Ce Installation Instruction Verified By:
250.15 XT 30 (and AR)	Fiberglass 3 tab shingle
Limits of Use (See Other)	Certification Agency Ce Installation Instruction Verified By:

Back

Next

DCA Administration

Department of Community Affairs Florida Building Code Online Codes and Standards

2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
© 2000-2005 The State of Florida. All rights reserved. Copyright and Discleration Product Approval Accepts:













Load Short Form ZONE 1 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

By:

Project Information

For: JERRY RYE, RYE CONSTRUCTION

(18) (19) (19) (19) (19) (19)	ESPECTATION OF	Design	Information	
	Htg	Clg		Infiltration
Outside db (°F)	17	91	Method	Simplified
Inside db (°F)	70	75	Construction quality	Average
Design TD (°F)	53	16	Fireplaces	7.10.090
Daily range	-	M		· ·
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	35		

HEATING EQUIPMENT

COOLING EQUIPMENT

Make Trade Model	Ruud Ruud UPNE Series UPNE-042J*Z			Make Trade Cond Coil	Ruud Ruud UPNE Serie UPNE-042J*Z 21AHSA42HM+R		1A*
Efficiend Heating		8.5 HSPF		Efficiency Sensible co		13 SEER 28350	Btuh
Heating	output		Btuh @ 47°F	Latent cooli	ng	12150	Btuh
Temper Actual a	ature rise	28 1350	°F cfm	Total cooling Actual air flo		40500 1350	Btuh cfm
Air flow	factor		cfm/Btuh	Air flow fact	or	0.045	(CC) 17 C (C)
Static pr Space th	ressure hermostat	0.01	in H2O	Static press Load sensib	ure le heat ratio	0.01 0.86	in H2O

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
LAUNDRY	109	3870	4213	99	191
BEDROOM 3	163	5739	1916	147	87
BEDROOM 2	181	5604	2240	143	101
BATH	63	128	167	3	8
COMPUTER AREA	92	2272	2258	58	102
HALL	44	89	116	2	5
DINING	174	3504	1505	90	68
KITCHEN	195	396	3729	10	169
NOOK	90	5325	2744	136	124
1/2 BATH / HALL	54	110	143	3	6
ENTRY/STAIRS	109	4597	1582	118	72
LIVING	323	6196	3530	159	160
M/BEDROOM	218	7958	3209	204	145
M/BATH/CLOSETS	243	6945	2500	178	113

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

ZONE 1 Other equip loads Equip. @ 0.96 RSM Latent cooling	d	2057	52733 4316	29852 1303 29909 4923	1350	1350
TOTALS	L	2057	57049	34832	1350	1350

Load Short Form ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

By:

Project Information

For: JERRY RYE, RYE CONSTRUCTION

Design Information						
	Htg	Clg		Infiltration		
Outside db (°F)	17	91	Method	Simplified		
Inside db (°F)	70	75	Construction quality	Average		
Design TD (°F)	53	16	Fireplaces	0		
Daily range	-	M	50 50 1 C C C C C C C C C C C C C C C C C C			
Inside humidity (%)	-	50				
Moisture difference (gr/lb)	-	35				

HEATING EQUIPMENT

COOLING EQUIPMENT

Make Trade Model	Ruud Ruud UPNE Series UPNE-024J*Z			Make Trade Cond	Ruud Ruud UPNE Serie UPNE-024J*Z		
Efficience	cv	8.3 HSPF		Coil Efficiency	RCFA-H*2417A*-	13 SEER	
Heating				Sensible c	ooling	16380	Btuh
Heating		22600	Btuh @ 47°F	Latent coo	ling	7020	Btuh
Temper	ature rise	26	°F	Total cooli	ng	23400	Btuh
Actual a	ir flow	780	cfm	Actual air	flow	780	cfm
Air flow	factor	0.043	cfm/Btuh	Air flow fac	ctor	0.071	cfm/Btuh
Static pr		0.10	in H2O	Static pres			in H2O
Space th	hermostat			Load sens	ible heat ratio	0.84	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
BONUS	1006	18287	11029	780	780
ZONE 2 d Other equip loads Equip. @ 0.96 RSM Latent cooling	1006	18287 4540	11029 432 11003 2187	780	780
TOTALS	1006	22827	13189	780	780

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Building Analysis Entire House LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

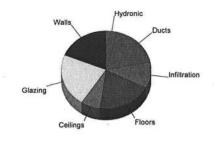
Project Information

JERRY RYE, RYE CONSTRUCTION For:

Design Conditions					
Location: Washington National AF Elevation: 0 ft Latitude: 38 °N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	P, DC, US Heating 17 - 15.0	Cooling 91 18 (M) 74 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 53 30 22.3 Simplified Average 0	Cooling 75 16 50 34.7

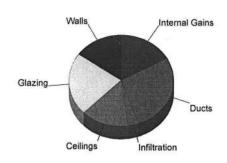
Heating

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments	3.8 48.1 0.0 1.7 4.7 2.5	14025 16293 0 5194 14431 8044 13034 3110 0	18.9 22.0 0.0 7.0 19.5 10.9 17.6 4.2 0.0
Total		74130	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments Total	1.7 26.7 0.0 2.0 0.0 0.4	6219 9063 0 6076 0 1301 11303 0 6920 0 40882	15.2 22.2 0.0 14.9 0.0 3.2 27.6 0.0 16.9 0.0



Overall U-value = 0.112 Btuh/ft2-°F

Data entries checked.

Building Analysis ZONE 1 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

By:

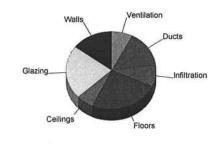
Project Information

For: JERRY RYE, RYE CONSTRUCTION

Design Conditions					
Location: Washington National Al Elevation: 0 ft Latitude: 38°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	P, DC, US Heating 17 - 15.0	Cooling 91 18 (M) 74 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 53 30 22.3 Simplified Average	Cooling 75 16 50 34.7

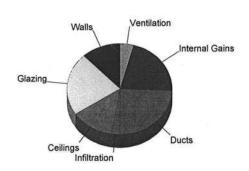
Heating

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments Total	3.3 48.6 0.0 1.7 7.0 2.5	8411 12696 0 3488 14431 4967 8740 0 4316 0 57049	14.7 22.3 0.0 6.1 25.3 8.7 15.3 0.0 7.6



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments	1.5 26.6 0.0 2.0 0.0 0.4	3730 6949 0 4081 0 803 7600 1303 6690	12.0 22.3 0.0 13.1 0.0 2.6 24.4 4.2 21.5
Total		31155	100.0



Overall U-value = 0.134 Btuh/ft2-°F

Data entries checked.

Building Analysis ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

By:

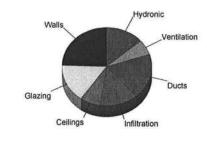
Project Information

JERRY RYE, RYE CONSTRUCTION For:

Design Conditions					
Location: Washington National AF Elevation: 0 ft Latitude: 38°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	P, DC, US Heating 17 - 15.0	Cooling 91 18 (M) 74 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 53 30 22.3 Simplified Average 0	Cooling 75 16 50 34.7

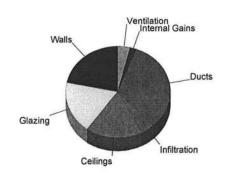
Heating

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments Total	4.8 46.1 0.0 1.7 0.0 2.5	5614 3597 0 1705 0 3077 4294 3110 0 1430 0 22827	24.6 15.8 0.0 7.5 0.0 13.5 18.8 13.6 0.0 6.3



Cooling

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments Total	2.1 27.1 0.0 2.0 0.0 0.4	2489 2114 0 1995 0 498 3703 432 230 0 0	21.7 18.4 0.0 17.4 0.0 4.3 32.3 3.8 2.0 0.0



Overall U-value = 0.063 Btuh/ft2-°F

Data entries checked.

Project Summary Entire House LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

By:

Project Information

For:

JERRY RYE, RYE CONSTRUCTION

Weather:

Notes:

Design Information

Washington National AP, DC, US

Winter	Design Conditions	S	Summer Desig	n Conditions
Outside db Inside db Design TD	17 70 53	°F °F °F	Outside db Inside db Design TD Daily range Relative humidity Moisture difference	91 °F 75 °F 16 °F M 50 % 35 gr/lb
Hea	ating Summary		Sensible Cooling Equ	ipment Load Sizing
Structure	57986	Btuh	Structure	29579 Btuh

Structure	57986	Btuh	Structure	29579	Btuh
Ducts	13034	Btuh	Ducts	11303	Btuh
Central vent (99 cfm)	0	Btuh	Central vent (99 cfm)	0	Btuh
Humidification	0	Btuh	Blower	0	Btuh
Piping	3110	Btuh			
Equipment load	74130	Btuh	Use manufacturer's data	n	
			Rate/swing multiplier	0.96	
Infiltratio			Fauinment sensible load	30247	Rtuh

intiltration		Equipment sensible load	39247 Btun		
Method Construction quality		Simplified Average	Latent Cooling Equipment Load Sizing		
Fireplaces		0	Structure Ducts	2542 Btuh 2245 Btuh	
	Heating	Cooling	Central vent (99 cfm)	0 Btuh	
Area (ft²) Volume (ft³)	3062 29565	3062 29565	Equipment latent load	4787 Btuh	
Air ahangaa/haur	0.00	0.45	Carriage and total land	44004 Dtl-	

	Heating	Cooling	Central vent (99 cfm)	0	Btuh
Area (ft²) Volume (ft³)	3062 29565	3062 29565	Equipment latent load	4787	Btuh
Air changes/hour	0.28	0.15	Equipment total load	44034	Btuh
Equiv. AVF (cfm)	138	74	Req. total capacity at 0.70 SHR	4.7	ton

Equiv. AVF (cfm)	138	74	Req. total capacity at 0.70 SHR	4.7 ton
Heating Equip	ment Summa	ary	Cooling Equipment	Summary
Make n/a Trade n/a Model n/a Efficiency Heating input Heating output Temperature rise Actual air flow Air flow factor Static pressure Space thermostat	0 0 0 0.000 0.00 n/a	n/a Btuh °F cfm cfm/Btuh in H2O	Make n/a Trade n/a Cond n/a Coil n/a Efficiency Sensible cooling Latent cooling Total cooling Actual air flow Air flow factor Static pressure Load sensible heat ratio	n/a 0 Btuh 0 Btuh 0 Btuh 0 cfm 0.000 cfm/Btuh 0.00 in H2O 0.00

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Project Summary ZONE 1 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

By:

Project Information

For:

JERRY RYE, RYE CONSTRUCTION

Notes:

Design Information

Weather: Washington National AP, DC, US

Winter Design C	onditions	Summer Design	Conditions	
Outside db Inside db Design TD	17 °F 70 °F 53 °F	Outside db Inside db Design TD Daily range Relative humidity Moisture difference	91 °F 75 °F 16 °F M 50 % 35 gr/lb	
Heating Summary		Sensible Cooling Equipment Load Sizing		
Structure Ducts Central vent (74 cfm) Humidification Piping	43994 Btuh 8740 Btuh 4316 Btuh 0 Btuh 0 Btuh	Structure Ducts Central vent (74 cfm) Blower	22253 Btuh 7600 Btuh 1303 Btuh 0 Btuh	
Equipment load Infiltration	57049 Btuh	Use manufacturer's data Rate/swing multiplier Equipment sensible load	n 0.96 29909 Btuh	
Method Construction quality	Simplified	Latent Cooling Equipm	nent Load Sizing	
Fireplaces	Average 0	Structure Ducts	1675 Btuh	

Heating Equipment Summary

Heating

2057 20515

0.25

Cooling

2057 20515

0.13 46

Make Trade Model	Ruud Ruud UPNE Series UPNE-042J*Z		
Air flow	input output ature rise air flow	41000 28 1350	HSPF Btuh @ 47°F °F cfm cfm/Btuh in H2O

Cooling Equipment Summary

Central vent (74 cfm)

Equipment total load

Req. total capacity at 0.70 SHR

Equipment latent load

Make	Ruud		
Trade	Ruud UPNE Series		
Cond	UPNE-042J*Z		
Coil	21AHSA42HM+RCS	A-H*4821A*	
Efficien	CV		SEER
Sensibl	e cooling	28350	Btuh
Latent cooling		12150	Btuh
Total cooling		40500	Btuh
Actual a	air flow	1350	cfm
Air flow factor		0.045	cfm/Btuh
Static p	ressure	0.01	in H2O
Load se	ensible heat ratio	0.86	

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Area (ft²) Volume (ft³)

Air changes/hour Equiv. AVF (cfm)

1744 Btuh

34832 Btuh

3.6 ton

Btuh

4923

Project Summary ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

3703 Btuh

0.96

11003 Btuh

432 Btuh 0 Btuh

By:

Project Information

For:

JERRY RYE, RYE CONSTRUCTION

Notes:

Design Information

Washington National AP, DC, US

Winter Design Conditions **Summer Design Conditions** 17 °F 70 °F Outside db 91 75 16 °F °F Outside db Inside db Inside db Design TD 53 °F Design TD Daily range Relative humidity M 50 35 % Moisture difference gr/lb **Heating Summary** Sensible Cooling Equipment Load Sizing 7326 Btuh

Structure	13993	Btuh	Structure
Ducts	4294	Btuh	Ducts
Central vent (25 cfm)	1430	Btuh	Central vent (25 cfm)
Humidification	0	Btuh	Blower
Piping	3110	Btuh	
Equipment load	22827	Btuh	Use manufacturer's data
			Rate/swing multiplier

Simplified

Weather:

Infiltration	
--------------	--

Method

Fireplaces		Average 0
Area (ft²) Volume (ft³) Air changes/hour Equiv. AVF (cfm)	Heating 1006 9050 0.35 53	Cooling 1006 9050 0.19 28

Heating Equipment Summary

Make Trade Model	Ruud Ruud UPNE Series UPNE-024J*Z		
Efficiend Heating	cy input	8.3 I	HSPF
Heating	output	22600	Btuh @ 47°F
Actual a		26 780	°F cfm
Air flow			cfm/Btuh
Static property	ressure hermostat	0.10	in H2O

Latent Cooling Equipment Load Sizing

Structure Ducts Central vent (25 cfm) Equipment latent load	866 743 578 2187	Btuh Btuh Btuh Btuh
Equipment total load Req. total capacity at 0.70 SHR	13189 1.3	Btuh ton

Equipment sensible load

Cooling Equipment Summary

Make Trade Cond Coil	Ruud Ruud UPNE Series UPNE-024J*Z RCFA-H*2417A*+RX	(MD-C04	
Efficient Sensible Latent of Total co	cy e cooling cooling		SEER Btuh Btuh Btuh
Actual a Air flow	air flow	780 0.071 0.10 0.84	cfm cfm/Btuh in H2O

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Duct System Summary ZONE 1 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

By:

Project Information

For:

JERRY RYE, RYE CONSTRUCTION

	He		Cooling		
External static pressure	0.01	in H2O		0.01	in H2O
Pressure losses	0.25	in H2O		0.25	in H2O
Available static pressure	-0.2	in H2O		-0.2	in H2O
Supply / return available pressure	-0.14 / -0.10	in H2O		-0.14 / -0.10	in H2O
Lowest friction rate		in/100ft			in/100ft
Actual air flow	1350	cfm		1350	cfm
Total effective length (TEL)			250 f	t	

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
LAUNDRY-A	С	2107	50	95	0.100	7	8x5	VIFx	150.0	0.0	ST1
LAUNDRY	C	2107	50	95	0.100	7	8x5	VIFx	150.0	0.0	ST1
BEDROOM 3	h	5739	147	87	0.100	8	8x6	VIFx	150.0	0.0	ST1
BEDROOM 2	h	5604	143	101	0.100	8	8x6	VIFx	150.0	0.0	ST1A
BATH	С	167	3	8	0.100	4	8x1	VIFx	150.0	0.0	ST1
COMPUTER AREA	C	2258	58	102	0.100	7	8x5	VIFx	150.0	0.0	ST1
HALL	С	116	2	5	0.100	4	8x1	VIFx	150.0	0.0	ST1
DINING	h	3504	90	68	0.100	6	8x4	VIFx	150.0	0.0	ST1
KITCHEN	С	3729	10	169	0.100	8	8x6	VIFx	150.0	0.0	ST1
NOOK	h	5325	136	124	0.100	8	8x6	VIFx	150.0	0.0	ST1
1/2 BATH / HALL	С	143	3	6	0.100	4	8x1	VIFx	150.0	0.0	ST1
ENTRY/STAIRS	h	4597	118	72	0.100	7	8x5	VIFx	150.0	0.0	ST1
LIVING	С	3530	159	160	0.100	8	8x6	VIFx	150.0	0.0	ST1
M/BEDROOM	h	7958	204	145	0.100	9	8x8	VIFx	150.0	0.0	ST1
M/BATH/CLOSETS	h	6945	178	113	0.100	8	8x6	VIFx	150.0	0.0	ST1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
ST1	Peak AVF	1350	1350	0.100	506	21	16 x 24	RectFbg	ST1
ST1A	Peak AVF	143	101	0.100	323	10	16 x 4	RectFbg	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)		Stud/Joist Opening (in)	Duct Matl	Trunk
rb3	0×0	147	87	100.0	0.100	441	8	8x	6		VIFx	
rb2	0×0	143	101	100.0	0.100		8	8x	6		VIFx	
rb4	0x0	159	160	100.0	0.100		8	8x	6		VIFx	
rb5	0×0	204	145	100.0	0.100	458	9	8x	8		VIFx	

Duct System Summary ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: WALT AND KRISSY SMI...

Date: Feb 28, 2007

Project Information

For:

JERRY RYE, RYE CONSTRUCTION

	He	Cooling			
External static pressure	0.10	in H2O		0.10	in H2O
Pressure losses	0.25	in H2O		0.25	in H2O
Available static pressure	-0.2	in H2O		-0.2	in H2O
Supply / return available pressure	-0.08 / -0.07	in H2O		-0.08 / -0.07	in H2O
Lowest friction rate	0.010	in/100ft		0.010	in/100ft
Actual air flow	780	cfm		780	cfm
Total effective length (TEL)			130	ft	

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
BONUS-A	С	6095	260	260	0.010	12	8x15	VIFx	70.0	0.0	st1
BONUS-B	C	6095	260	260	0.010	12	8x15	VIFx	70.0	0.0	st1
BONUS	С	3677	260	260	0.010	12	8x 15	VIFx	70.0	0.0	st1A

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	780	780	0.010	305	21	16 x 23	RectFbg	st1
st1A	Peak AVF	0	0	0.010	0	0	16 x 0	RectFbg	

Bold/italic values have been manually overridden

FNGINFERING CONSULTANTS IN GEOTECHNICAL • ENVIRONMENTAL • CONSTRUCTION MATERIALS TESTING

February 27, 2007 Project No. 073206.01G

Rye Construction Company, Inc. 3817 N.W. 28th Terrace Gainesville, Florida 32605

Attention:

Jerry Rye

Reference: Proposed Smith Residence

Walter Little Road

Columbia County, Florida

Dear Mr. Rye,

As requested, Geo-Tech, Inc. has performed a preliminary geotechnical engineering investigation and evaluation of the site for the proposed Smith residence to be constructed near Walter Little Road in Columbia County, Florida. The purposes of our investigation were to identify shallow subsurface soils within the proposed building limits, to comment on the suitability of these soils for conventional foundation systems, and to provide preliminary recommendations for site preparation and other geotechnical concerns as required.

We understand the residence will be single-story and have a plan area of about 2,500 square feet. Support for the residence is to be provided by conventional, shallow spread footings and stem walls. Foundation loads were not provided; however, we believe column and wall loads will not exceed 25 kips and 1.5 kips per foot, respectively.

Site Investigation

Subsurface conditions at the site were investigated by performing four (4) auger borings advanced to depths of 10 feet. Borings were performed at the relative locations indicated on the attached drawing. These locations were selected by Geo-Tech, Inc., and the building limits were staked on site. Representative samples of the site soils were collected and returned to our laboratory for visual examination and classification by a geotechnical engineer.

Auger borings are performed by mechanically advancing a slender, solid-stem, flight auger into the soil to the desired depth, by retracting the auger, and by examining cuttings recover on the auger flights. Samples are examined for soil type and color.

Findings

Feb 27 07 03:19p

The soil borings generally encountered three soil strata. The first layer consists of 1 to 6 feet of grayish brown, tannish gray or grayish tan sand (SP) and/or sand with silt (SP/SM).

The second layer consists of 1 to 4 or more feet of generally grayish orange or gray and orange, clayey sand (SC). The third layer consists of 5 to 8 or more feet of generally gray, orange and red, sandy clay (CL) or clay with sand (CH).

Ground water was not encountered at any boring location at the time of our investigation, and we believe the wet season water table will occur at a depth of more than 4 feet below the existing surface grade. Note however that storm water will perch on the clayey soils encountered near the ground surface that may make site preparation, especially compaction, difficult.

For a more detailed description of the subsurface conditions encountered, please refer to the attached borings logs. Note specifically the transition between soil layers is typically gradual and not abrupt as indicated by the logs; therefore, the thickness of soil layers should be considered approximate.

Discussion and Recommendations

Based upon our findings, we believe the building site should be moved in an easterly direction to possibly avoid the clay soils encountered at boring locations A-1 and A-2. We believe the clay soils encountered at these two boring locations are moderately active implying a moderate risk of shrink/swell behavior that could cause movement of the foundations or floor slabs. Alternatively, the building site can be filled to provide separation between the foundations and the clay soils, or some combination of fill placement with limited excavation and replacement of clay soils can be performed. Should the building site be moved, additional borings should be performed as required to investigate the new building area.

The local standard-of-care for placing conventional, shallow footings over active clay soils is to provide separation between the active soils and the bottoms of the foundations. For slightly active soils 3 feet of separation is typically suggested. For moderately to highly active soils 4 to 5 or more feet is suggested. Note however that providing separation between foundations and active soils does not provide a guarantee that some foundation or slab movement will not occur. It does however reduce the likelihood of significant movement.

Should you choose to excavate and replace the clay soils, we suggest the clay soils be excavated and replaced to a minimum depth of 4 feet below the bottoms of the foundations and 3 feet below floor slabs. The lateral limits of excavation should extend a minimum of 2.5 feet beyond the edges of the foundations. We recommend a sufficient portion of the excavated clay soil be retained on site and used as backfill at the outer

edges of the stem walls. These clay replacement soils will help reduce the transfer of moisture between the ground surface and the clay soils that remain below the foundations. These clay soils should be proof-compacted for full depth to a minimum of 95% of the Standard Proctor maximum dry density or to 90% of the Modified Proctor maximum dry density, as you prefer. Additionally, the upper surface of the clay replacement soils should be sloped away from the structure in order to direct storm water away from the foundations. Grading of the site and/or elevations should be selected such that a minimum of 6 inches of sandy surface soil is present above the clay replacement soils. The residence should also be equipped with storm gutters and down spouts that will transfer storm water away from foundation areas. Swales should also be provided as required to direct surface flow away from the residence.

Fill or replacement soil generally should consist of clean, fine sand containing less than 10% passing the No. 200 sieve. Replacement soil may however consist of clayey sand containing less than about 25% passing the No. 200 sieve. For either soil the maximum lift thickness should not exceed 12 inches, and each lift should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density.

With exception of the clay soils encountered, we believe the site soils are suitable to provide support for the residence using conventional, shallow spread footings and recommend footings be sized to exert a maximum soil bearing pressure of 2,500 pounds per square foot. Additionally, we recommend footings have minimum widths of 18 and 24 inches for strip and isolated footings, respectively. The bottoms of foundations should be embedded a minimum of 18 inches below the finished surface grade.

The building site should be stripped of grass, roots, topsoil and other deleterious materials that may be present. Excavation should then be performed as required to establish the appropriate site grading and to remove clay soils. Suitable replacement soils should then be placed as required and compacted.

The site should then be thoroughly proof-rolled using heavy, rubber-tired equipment (a large, loaded front-end loader, for example). Proof-rolling helps to compact the bearing soils and to locate zones of especially loose soil that may be present, such as former stump areas or areas previously excavated and filled. Such zones of loose soil should be excavated and replaced or otherwise treated as directed by the geotechnical engineer.

The site should then be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density to a depth of 2 feet in foundation areas and 1 foot in floor areas. We recommend using a vibratory-drum roller having a minimum static weight of 5 tons to perform this compaction.

Additional replacement or fill soils may be placed as required. These soils generally should consist of clean, fine sand containing less than 10% passing the No. 200 sieve. However, clayey sands containing less than about 25% passing the No. 200

sieve may also be used if desired. Maximum lift thickness should not exceed 12 inches, and each lift should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density.

Field density testing should be performed in the compacted subgrade, in each lift of fill, and in foundation excavations to verify the recommended compaction has been achieved.

Note again that excavation and replacement of active soils to the suggested depth of 4 feet below the bottoms of foundations provides no guarantee that some foundation or slab movement will not occur. Also note that Standard Penetration Test borings to determine soil densities were not performed. Therefore, the site preparation recommendations provided within this report are preliminary and are based upon the assumption the sandy surface soils present at this site are not in a very loose condition. For all residential construction of significance and commercial construction, we recommend site investigation specifically include Standard Penetration Test borings to evaluate densities of the existing site soils in order to prepare specific site preparation recommendations.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us if you have questions concerning this report or if we may be of further assistance.

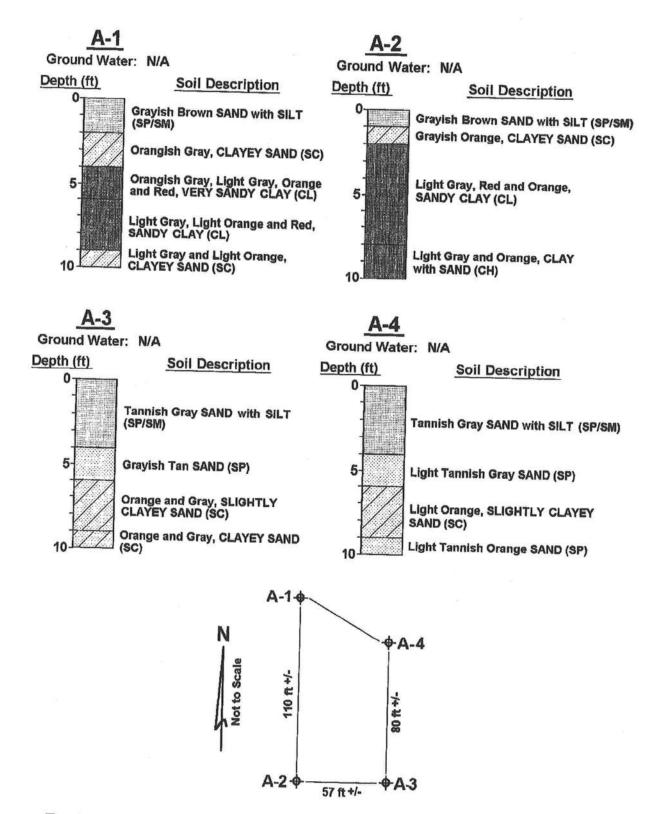
Respectfully submitted, Geo-Technologies, Inc.

John C. Dorman, Jr., Ph.D., P.E.

Geotechnical Engineer

2/27/07

GEN-TECH



Boring Logs and Location Plan: Walter Smith Residence Columbia County, Florida

GEO-TECH, INC.

25765 (SPD) 25766

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

FIELD DENSITY WORKSHEET

PROJECT NAME SMITH DES. EARTH CONTRACTOR 1214 SWI V COMPACTION REQUIREMENT (%) 95. TOTAL ON-SITE TIME Limerock Subgrade Pipe Backfill	Jalter 90	AVC □ Sta ■ Mo	andard Fodified P	Proctor	PROJE PERMI TESTE FIELD MILES	CT NO T NO D BY CONTACT FROM OF	50002 5146 5007	5465
	LAB PR	OCTOR	TEST	PROBE	%	WET	DRY DENSITY	%
TEST LOCATION	DENS.	ОМС	DEPTH	DEPTH	MOIST.	(PCF)	(PCF)	COMP.
House FtGs.								
CtR. of W. Ftt.	110,4	10.1	Fb	12"	4.6		107.6	97.5
CTR. of C. FtG				-	4.0	112.5	108.2	98.0
CtR. of S. Ftb					4.2	112.2	107.7	97.6
CAR. OF IHT. Ftb					3.8	112.1	108.0	97.8
SHOP 1=t65.								
CAR of N. FHG					3,6	112,3	108.4	98.2
CIR of S. FtG.	f	1	8	1	4.4	113.5	108.7	98.5
REMARKS						minir requi ** Rete dens obtai () Clien	ity requirer ned. It is aware	s minimum nent was

GEO-TECH, INC.

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

FIELD DENSITY WORKSHEET

CLIENT RYE COHST.							JUH 07	
2011		1			PROJE	CT NO.		
PROJECT NAME SMITH RES.								165
EARTH CONTRACTOR 1214 541					TESTE	D BY	JUL	
COMPACTION REQUIREMENT (%)						CONTACT		
TOTAL ON-SITE TIME							FICE	
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☐ E	Building	Pad 🗅	Building	Footing	Oth	ner		
	LAB PR	OCTOR				WET	DRY	
TEST LOCATION	DENS.		TEST DEPTH	PROBE DEPTH	% MOIST.	DENSITY (PCF)	DENSITY (PCF)	% COMP.
CtR. of PAP	110.4	10.1	F6	1211	4.6	111.9	109.0	96.9
CIR of S. EHD)))	5.2	114.0	108.4	98.2
CHR OF GARAGE ACEA	2	2	2	1	6.0	116.8	110.2	99.8
							- 37 =	
								1/2
						AT THE RESERVE	-	
REMARKS						minir requi ** Rete dens obtai () Clien	sity failed to mum project frement st indicates ity requirent ned. It is aware of tisfactory to	t minimum nent was

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infectation is enecified by the builder architect or required by the lander architect EUA

terrine intestation is specified by the builder, architect, or required by the leftder, architect, FTIA, or VA.
All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. 25765
Section 1: General Information (Treating Company Information)
Company Name:
Section 2: Builder Information
Company Name: Rye Construction Company Phone No. 352-258-8603
Section 3: Property Information
Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) Location Location
Section 4: Treatment Information
Date(s) of Treatment(s)
Attachments (List)
Comments
Name of Applicator(s) 5. Gregory Certification No. (if required by State law) JF/04376
The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations. Authorized Signature Date

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802) Form NPCA-99-B may still be used



COLUMBIA COUNTY, FLORIDA

epartment of Building and Zoning Inspection

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code. This Certificate of Occupancy is issued to the below named permit holder for the building

Parcel Number 02-5S-16-03437-003

Building permit No. 000025765

Fire:

122.20

Permit Holder RYE CONSTRUCTION

Waste: 167.50

Owner of Building WALT & KRISSY SMITH

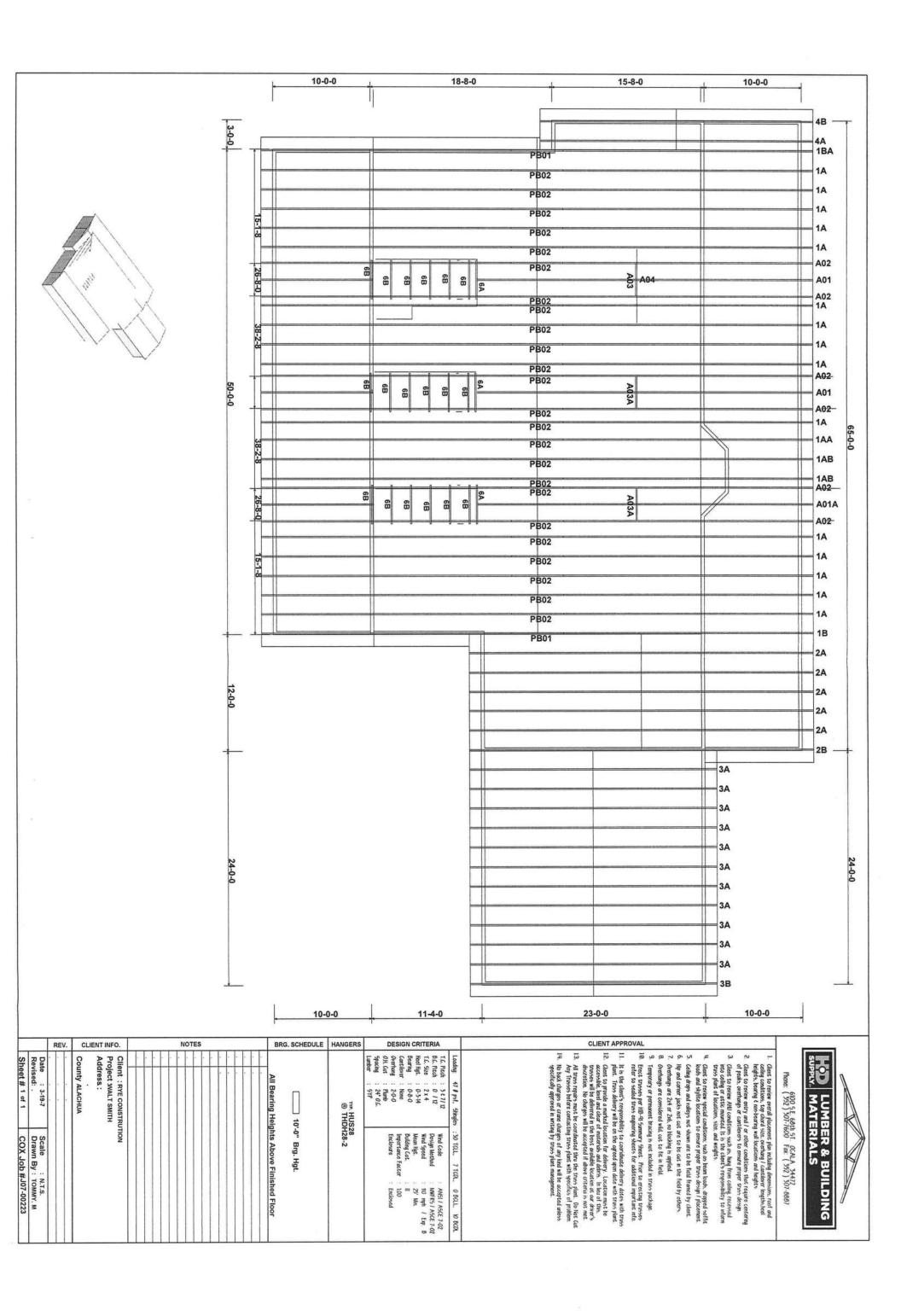
Total: 289.70

Location: 1214 SW WALTER AVENUE

Date: 12/21/2007

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



TOM SMITH Truss Contany 18-8-0 10-0-0 15-8-0 10-0-0 4B 1BA 1A PB02 1A PB02 1A PB02 1A PB02 1A PB02 A02 PB02 6B 6B 68 A01 A02 1A 1A PB02 1A PB02 1A PB02 A02 6A PB02 68 6B A01 A02 PB02 1A PB02 1AA PB02 1AB PB02 1AB A02 6A 6B 68 A01A A02 PB02 1A PB02 1A PB02 1A PB02 1A 1A PB02 1B PB01 2.4 2A 2A 2A 2A 2B -3A 3A 23-0-0 10-0-0 11-4-0 10-0-0 CLIENT INFO. NOTES CLIENT APPROVAL 5. Gelling drops and valleys not shown are to be field framed by client.
6. Hip and corner jocks not cut are to be cut in the field by others.
7. Overhangs are 2.44 or 2.66, no blocking is applied.
8. Overhangs are considered wild, cut to fit in field.
9. Temporary or permanent browing is not included in trues package.
9. Temporary or permanent browing is not excluded in trues package.
18. Erect trueses per HID-91 Summary Sheet. Prior to erecting trueses per HID-91 Summary Sheets for additional important into positions. REV. BRG. SCHEDULE HANGERS DESIGN CRITERIA A 11. It is the client's responsibility to coordinate delivery dates with truss plant. Truss delivery will be on the agreed upon date with truss plant.

12. Client to provide a marked bocation for delivery. Location must be accessible, level and clear of materiels and delhis, in level of this, trusses will be delivered in the test available bocation at our driver's descretion, the clear yes will be accepted if above criteria is not max.

13. All truss regains must be coordinated thru the truss plant. Do Not Cut Any Trusses before contacting truss plant with specifics of problem.

14. No back charges or crare charges of any kind will be accepted whesh specifically approved in writing by truss plant management. 16. Pitch : 3 t 71/2
B6. Pitch : 0 1/2
16. Life : 2 x 4
Red Hgt. : 0-3-M
Bearing : 0-8-0
Cantilener : None
Overhang : 2-0-0
OH Cut : Plumb
Spacing : 24*06.
Lumber : 5YP Client to review entry and I or other conditions that require centering
of peaks, arethrapy or cantiberers to ensure proper truss design.
 Client to review ANU conditions such as, hang from colling recessed
into ceiling or actic mounced. If is the client's responsibility to inform
truss plant of locations, suc, and weights. Sheet # 1 of 1 Client: RYE CONSTRUTION Project: WALT SMITH Address: Loading: 47 # psf. Stingles County ALACHUA Client to review special conditions; such as beam loads, dropped soffit loads and skylite locations to ensure proper truss design I placement. Client to review overall placement plan including dimensions, roof and coiling conditions, top chard size, overlong I contilever lengths, beet heights, bearing 4 non-bearing wall locations and heights. Phone: (352) 307-7600 Fax: (352) 307-8887 All Bearing Heights Above Finished Floor : 3-19-7 LUMBER & BUILDING MATERIALS ® THDH28-2 ☐ 10'-0" Brg. Hgt. Wind Code
Design Method
Wind Speed
Mean Hgt.
Building Cat.
Importance Factor
Enclosure COX Job #J07-00223 :30 TALL, 7 TADL, O BALL, 10 BADL Scale : N.T.S.
Drawn By : TOMMY. M ANSI / ASCE 7-02 MNFR5 / ASCE 7-02 110 mph / Exp. B 25' Min. Enclosed