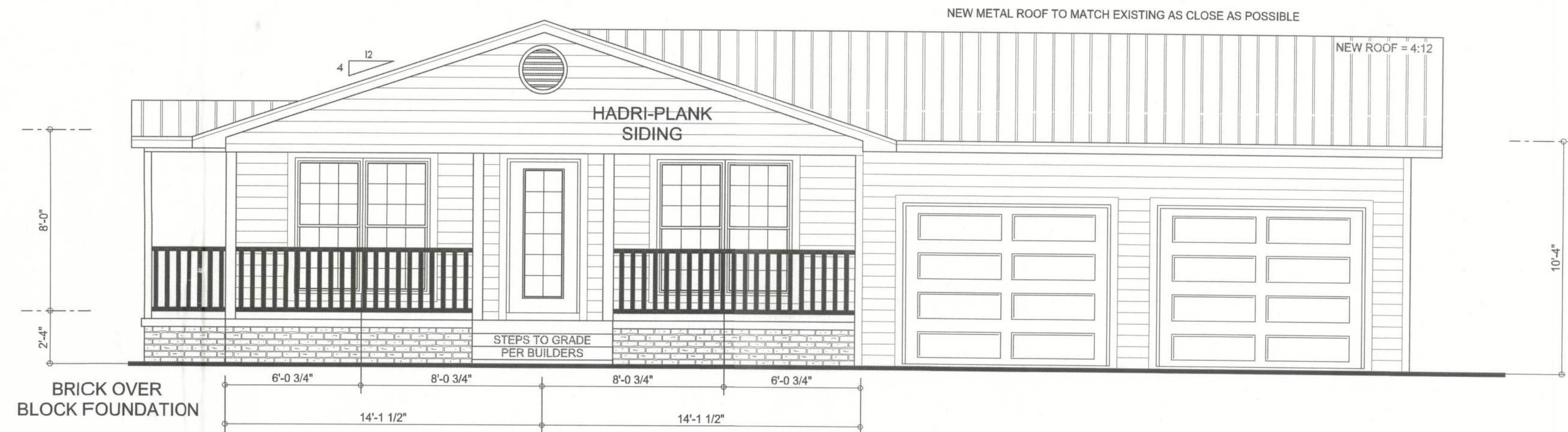


REVISIONS	

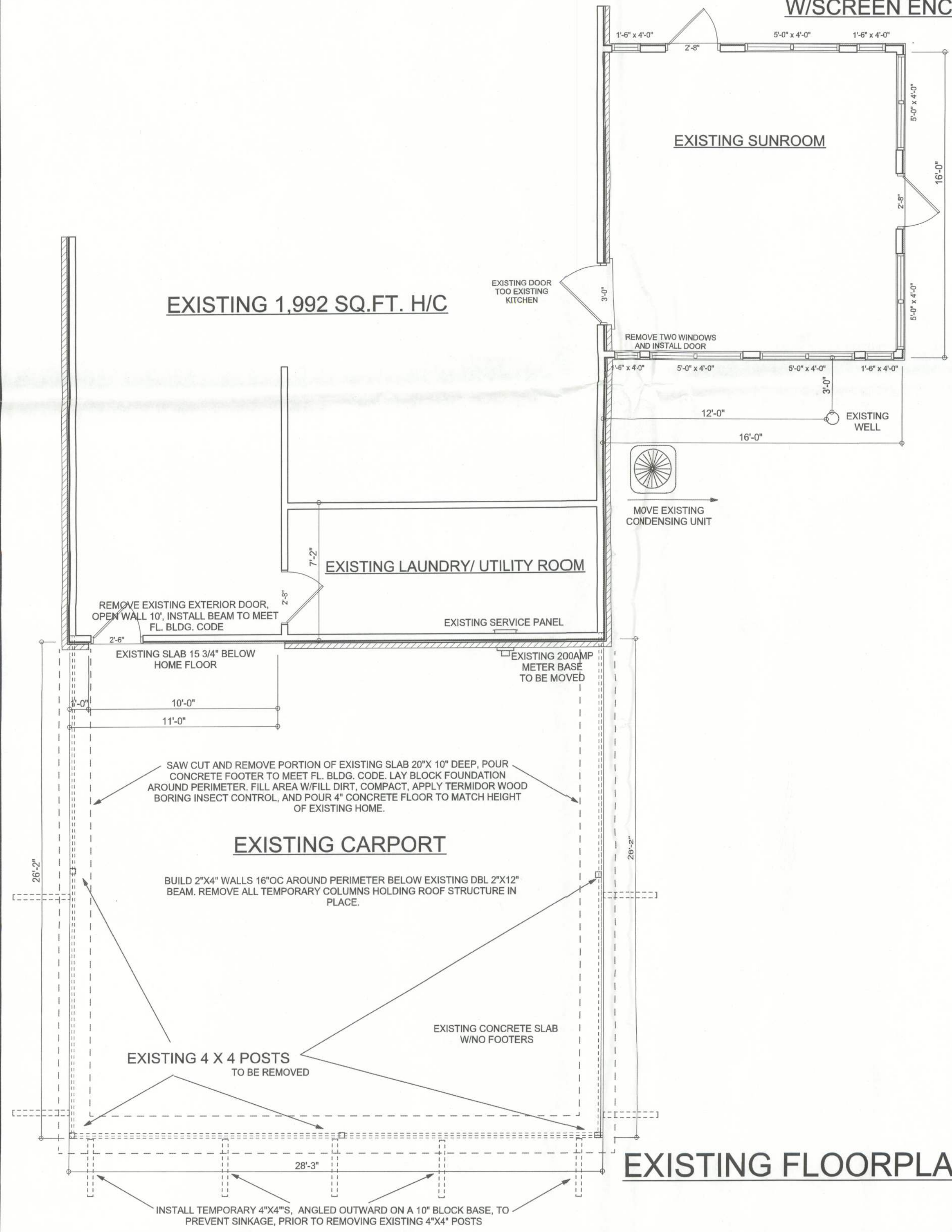
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



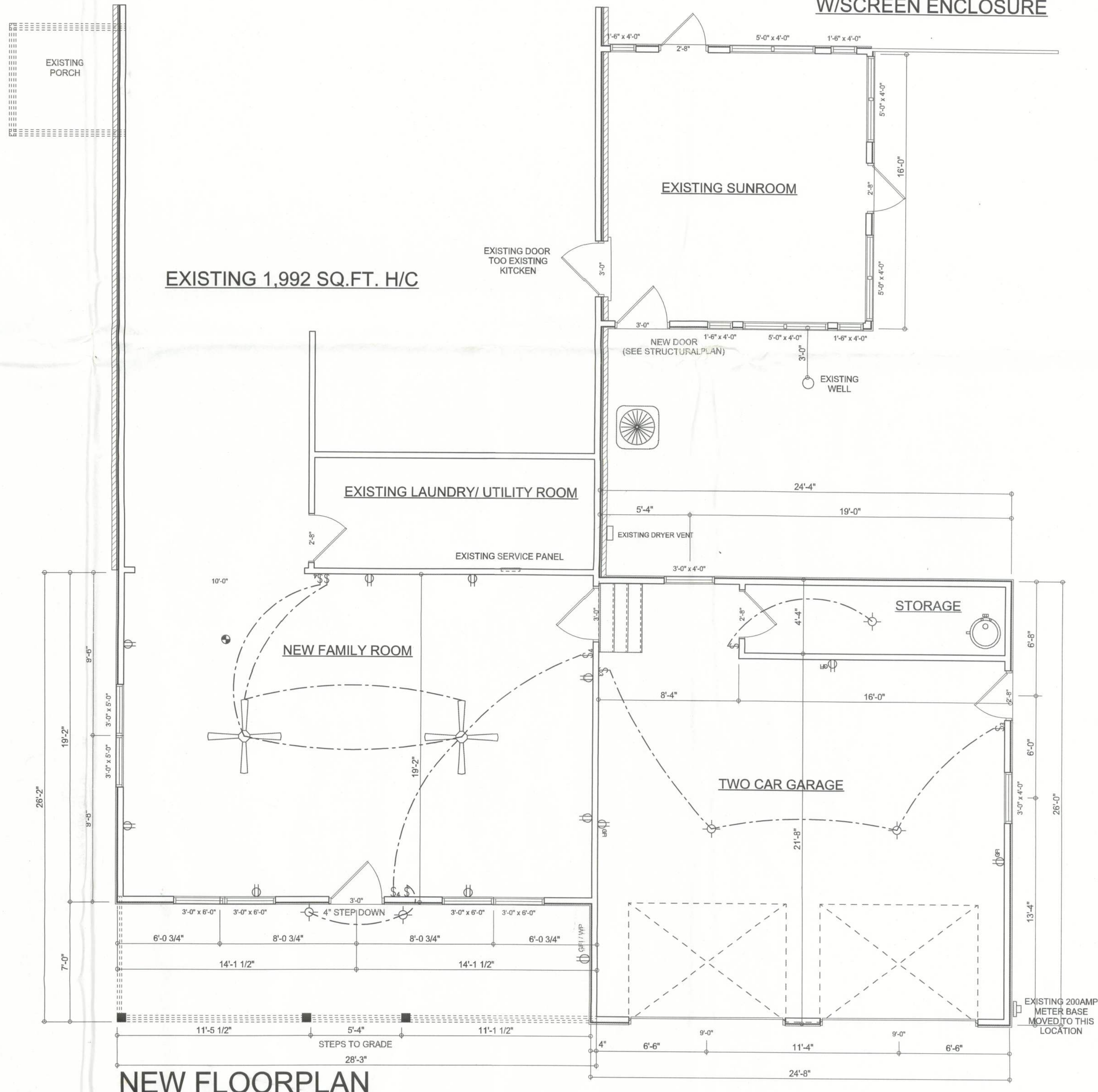
FRONT VIEW

EXISTING SWIMMING POOL
W/SCREEN ENCLOSURE

EXISTING SWIMMING POOL
W/SCREEN ENCLOSURE



EXISTING FLOORPLAN



NEW FLOORPLAN



ERKINGER CONSTRUCTION GROUP

JEANETTE STEEDLEY

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LAKE CITY FL 32024

Mark Disosway P.E.
P.O. Box 868
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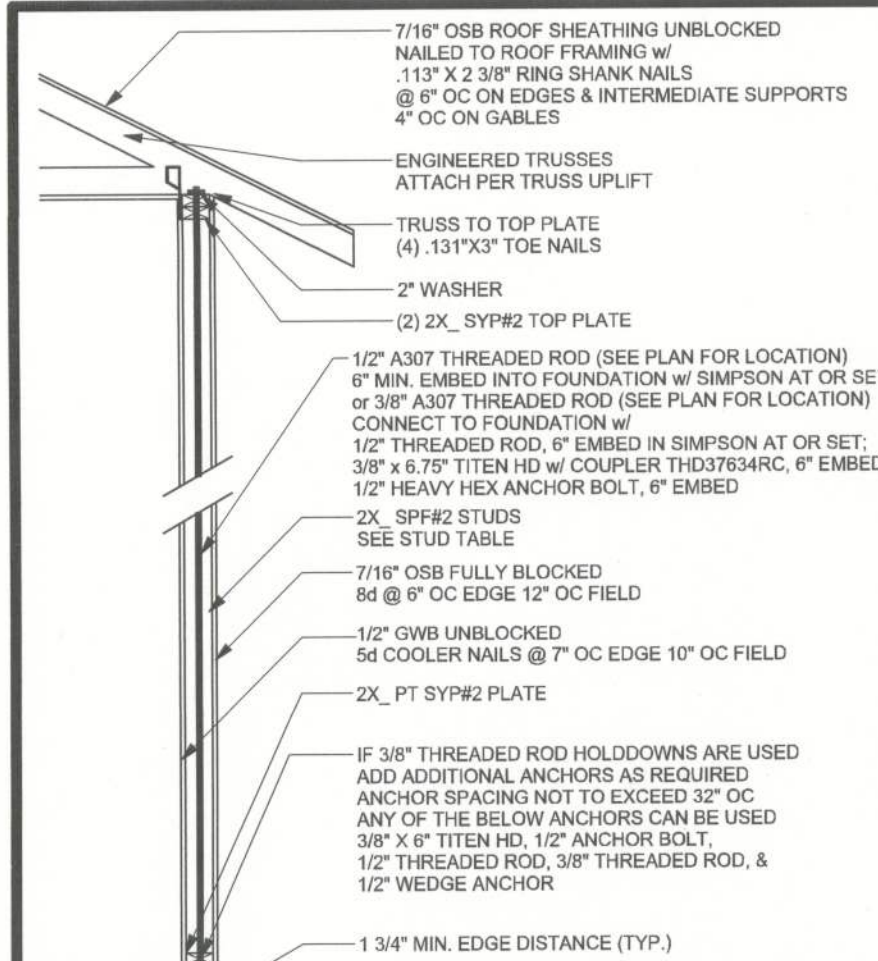
PRINTED DATE:
May 13, 2013

DRAWN BY: Matthew A. Erkinger STRUCTURAL BY: Evan Beamsley

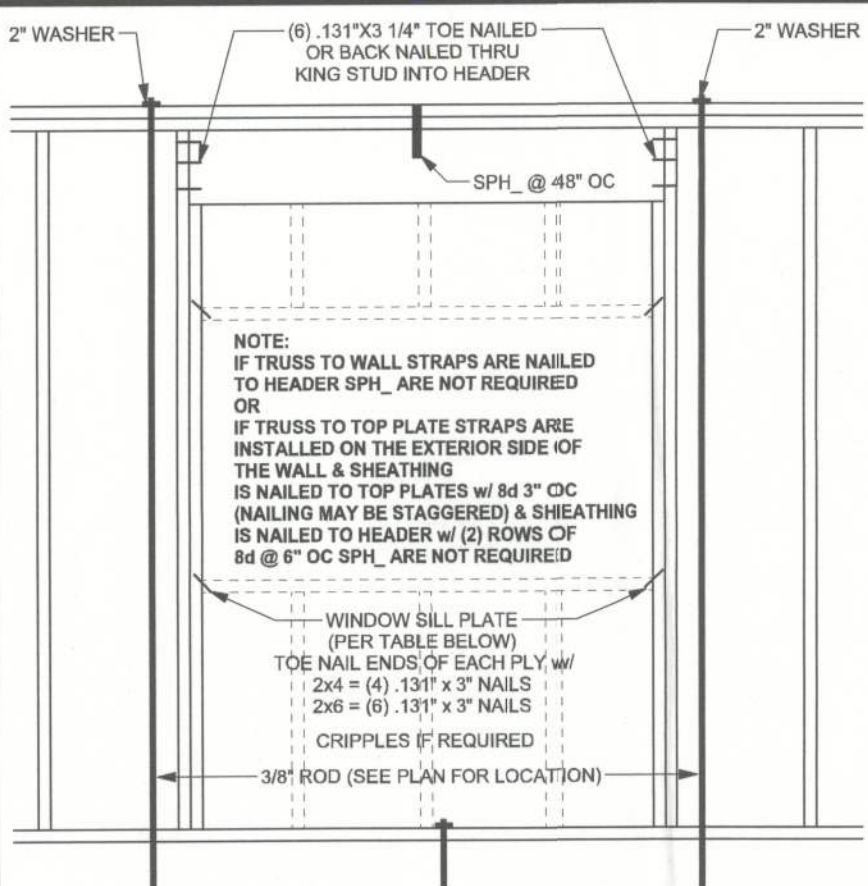
FINALS DATE:
2013-05-13

JOB NUMBER:
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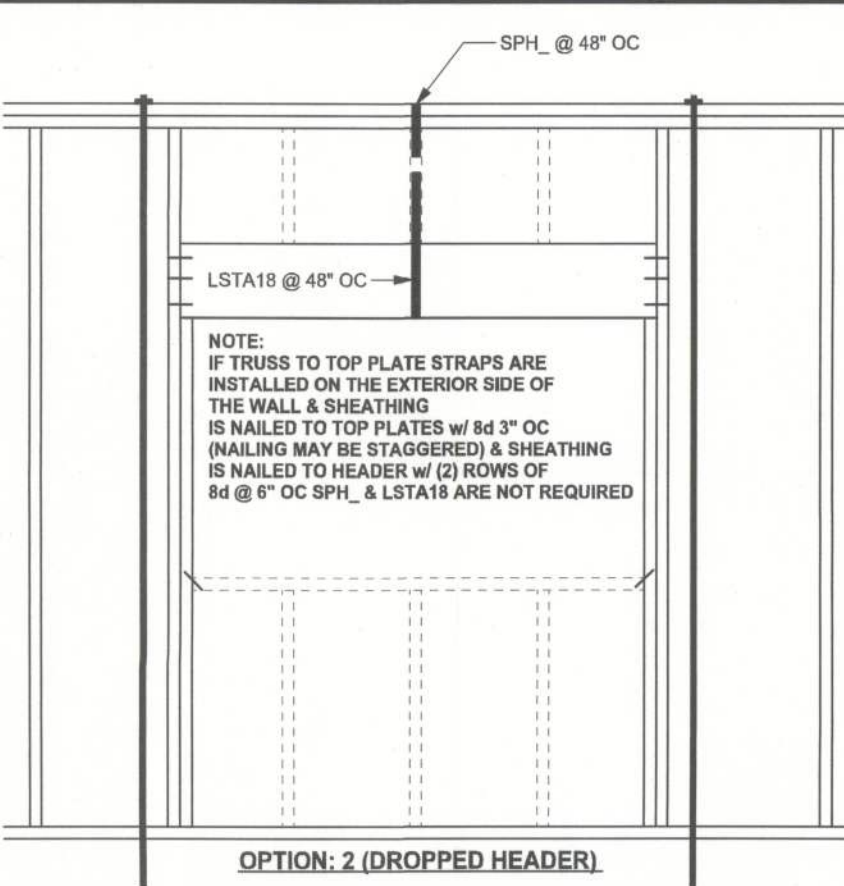
DRAWING NUMBER
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OF 1 SHEET



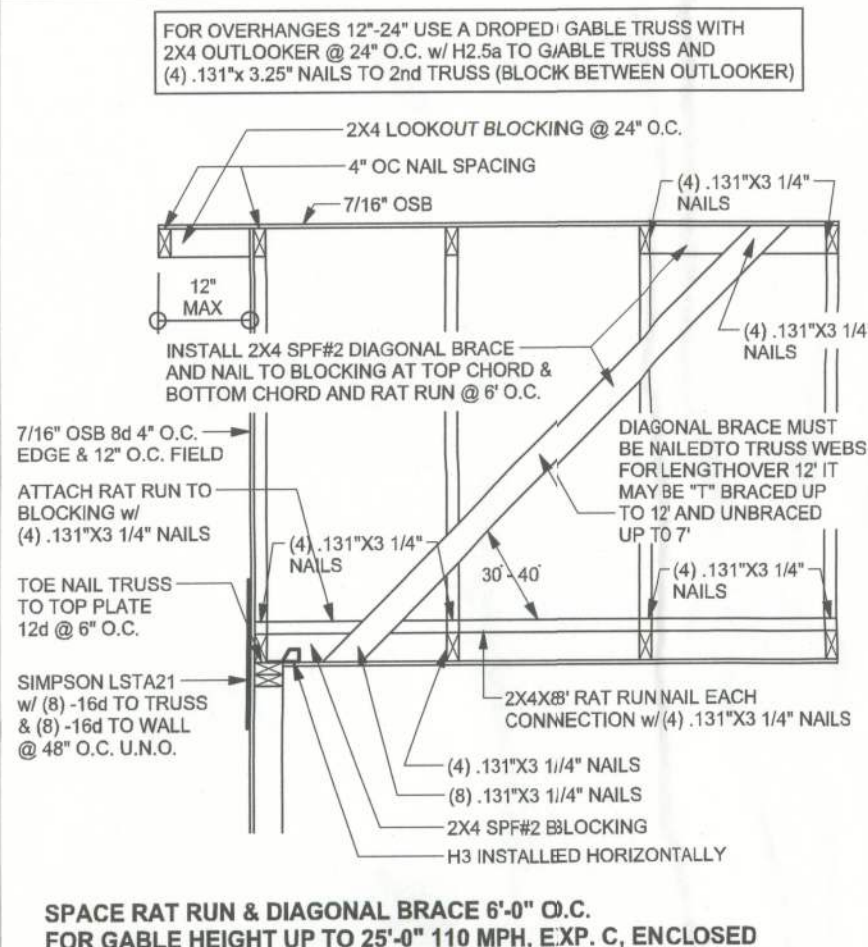
(TYP.) EXTERIOR WALL
ONE STORY WOOD FRAME w/ RODS



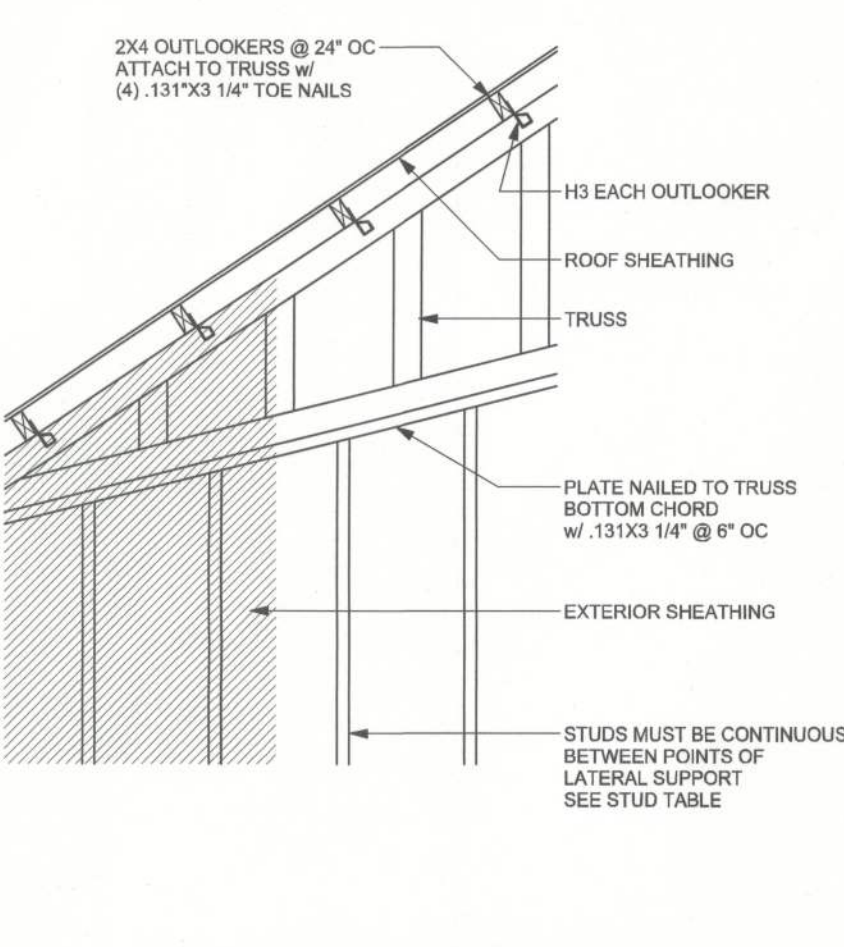
(TYP.) HEADER
ONE STORY WOOD FRAME w/ RODS



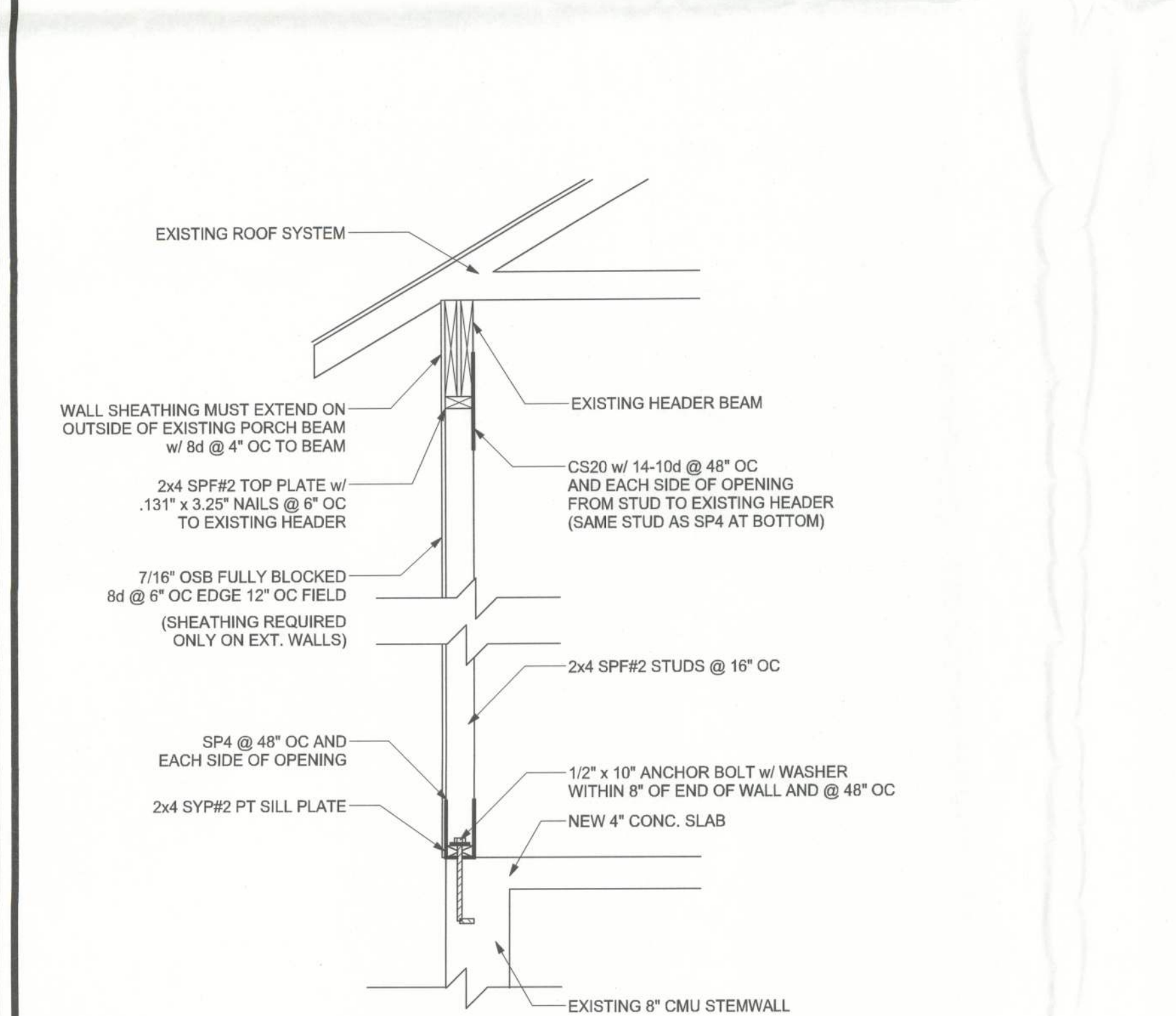
OPTION: 1 (FLUSH HEADER)
SILL PLATE SPANS FOR 10'-0\"/>



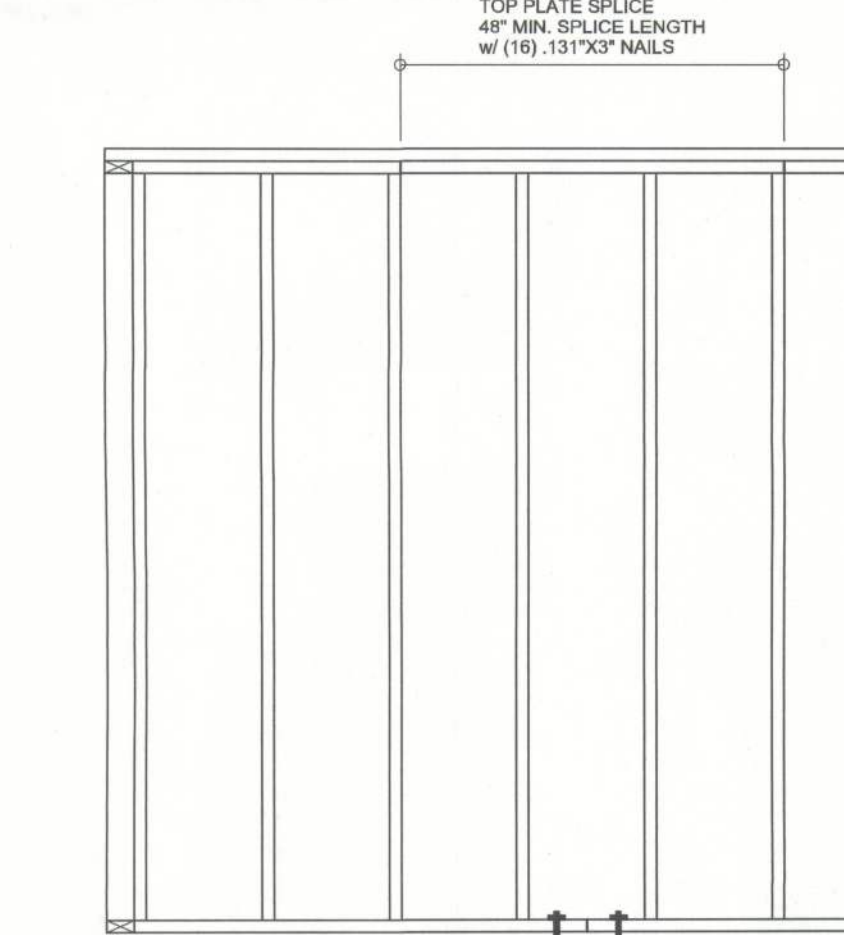
(TYP.) GABLE BRACING DETAIL
WOOD FRAME



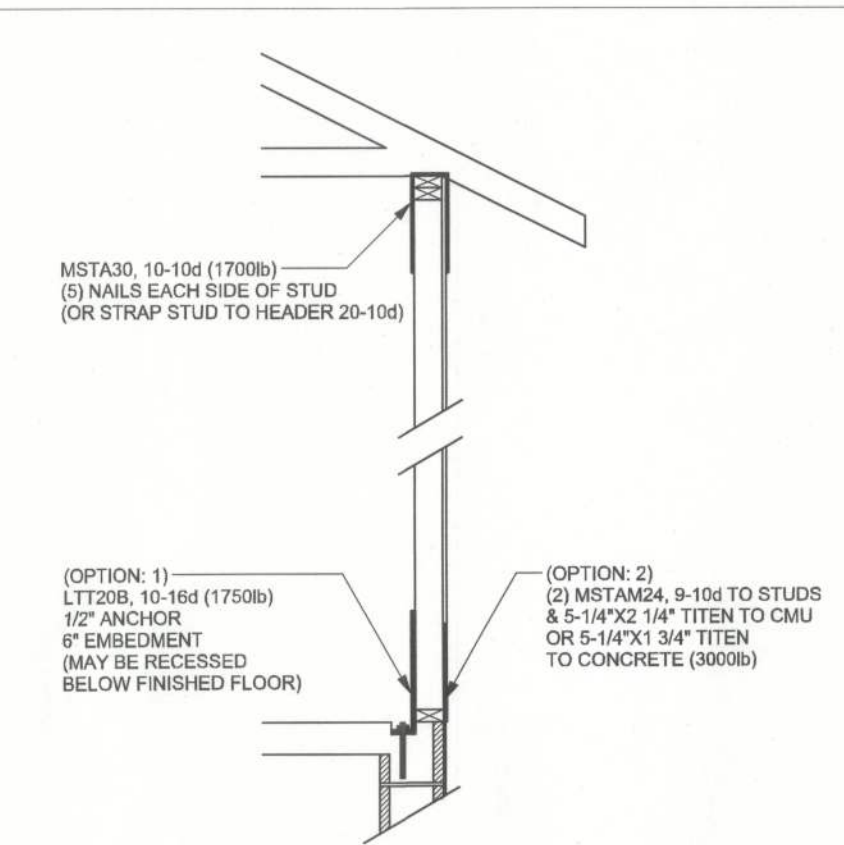
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



STRUCTURAL DETAIL FOR INCLOSING CARPORT
SCALE: 3/4\"/>



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



ALTERNATE CONNECTION WHERE
ROD CANNOT BE PLACED IN WALL
ONE STORY WOOD FRAME w/ RODS

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H8	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12-LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12-MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16-HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEDOWNS								TO FOUNDATION
LG12	2050	1785	700	170	700	170	14-16d	14-16d
LG13-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16d5
LG14-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16d5
MG1	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								TO STUDS
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	800					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	
SPH4	1240	1065					10-10d x 1 1/2"	
SP6	885	760					6-10d x 1 1/2"	
SPH6	1240	1065					10-10d x 1 1/2"	
LST1A16	1235	1110					14-10d	
LST1A21	1235	1235					16-10d	
CS20	1030	1030					14-10d	
CS16	1705	1705					22-10d	
STUD ANCHORS								TO STUDS
LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT131	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3685					18-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU4A	2200	2200					12-16d	5/8" ANCHOR
ABU6B	2300	2300					12-16d	5/8" ANCHOR
ABU8B	2320	2320					18-16d	2-5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16d5 OPTIONAL NAIL HOLES
(2) FOR SPY GIRDER & SPF STUDS

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH EXPOSURE C. STUD DEFLECTION LIMIT H240 (NOT OK FOR SOME BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE: 16" O.C. x 0.8 = 12.8" O.C.)

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE TRUSS DESIGNER. TRUSS ENGINEERING IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 4 x W1 4, F_y = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (WWR) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12'-0". DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPICES 40" DB (25' FOR 8 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, UNO.

GLULAM BEAMS: GLB, 24F-V3SP, F_b = 2.4ksi, E = 1890ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 9/64", UNO.

NAILS: ALL NAILS ARE COMMON NAIL UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

REVISIONS

NO.	DESCRIPTION

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

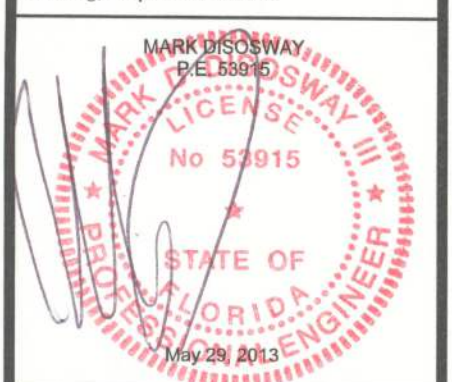
WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Erkinger
Construction Group

Jeanette Steedley
Addition

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Phone: (386) 754 - 5419
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PRINTED DATE:
May 29, 2013

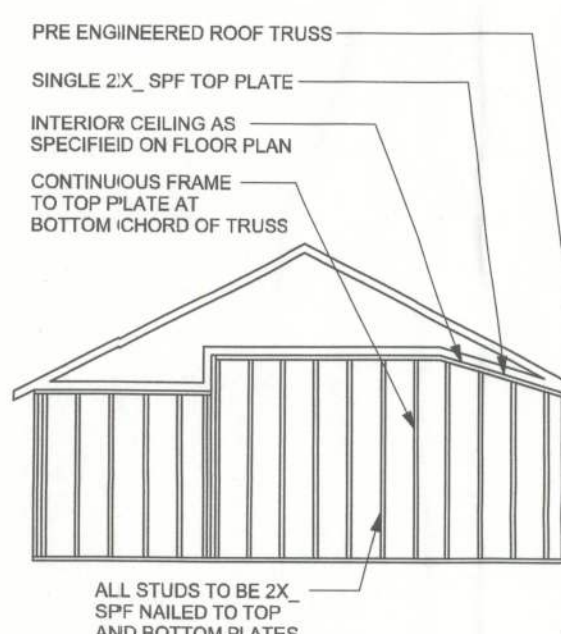
DRAWN BY: STRUCTURAL BY:
Evan Beamley

FINALS DATE:
2013-05-29

JOB NUMBER:
1304098

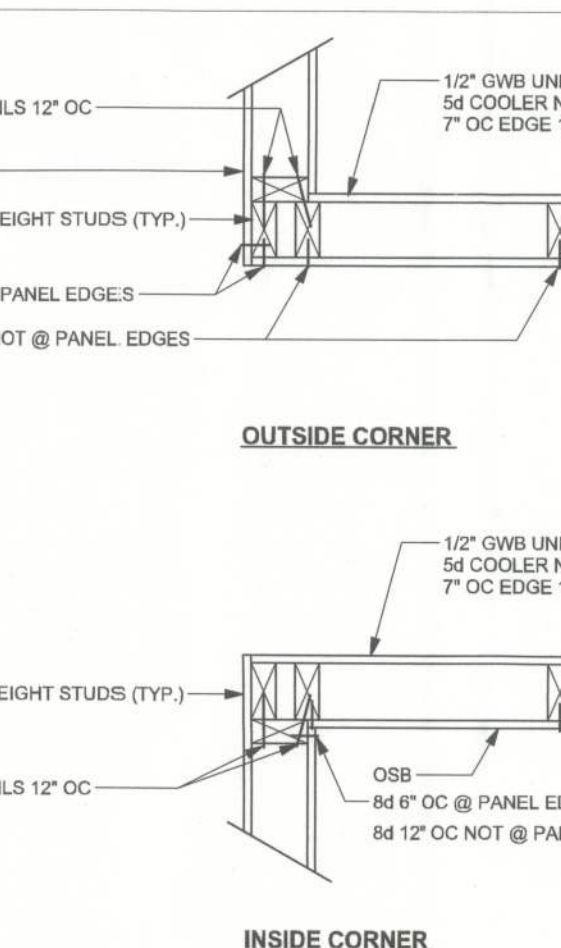
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S-1
OF 2 SHEETS

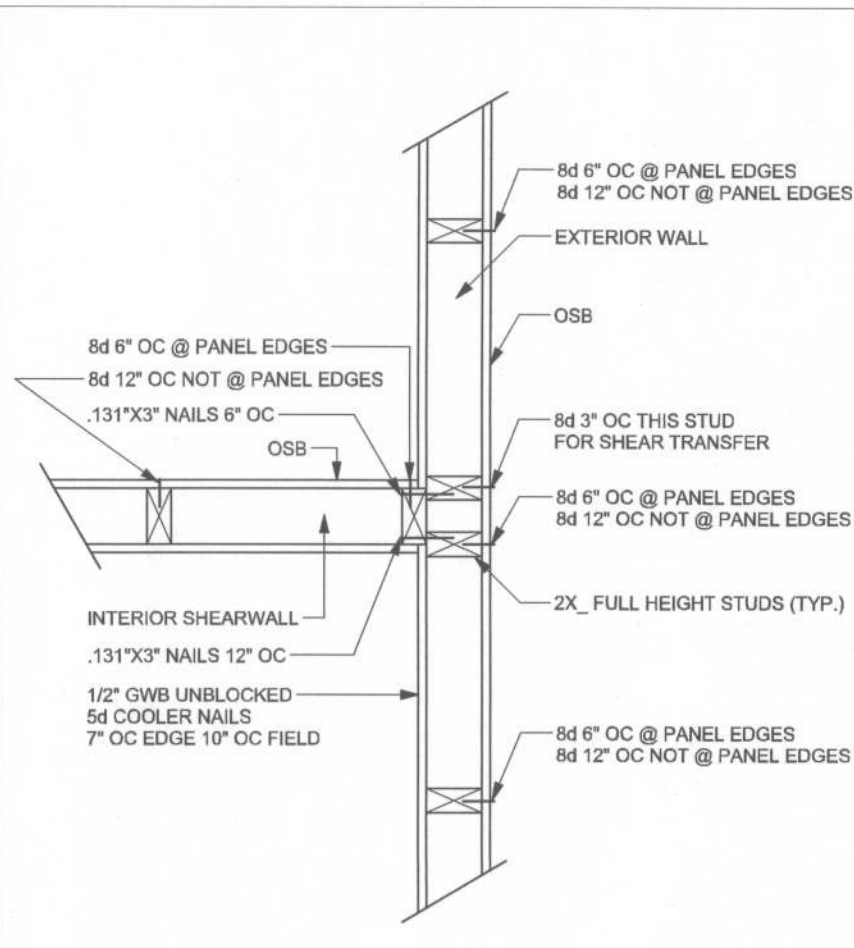


CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

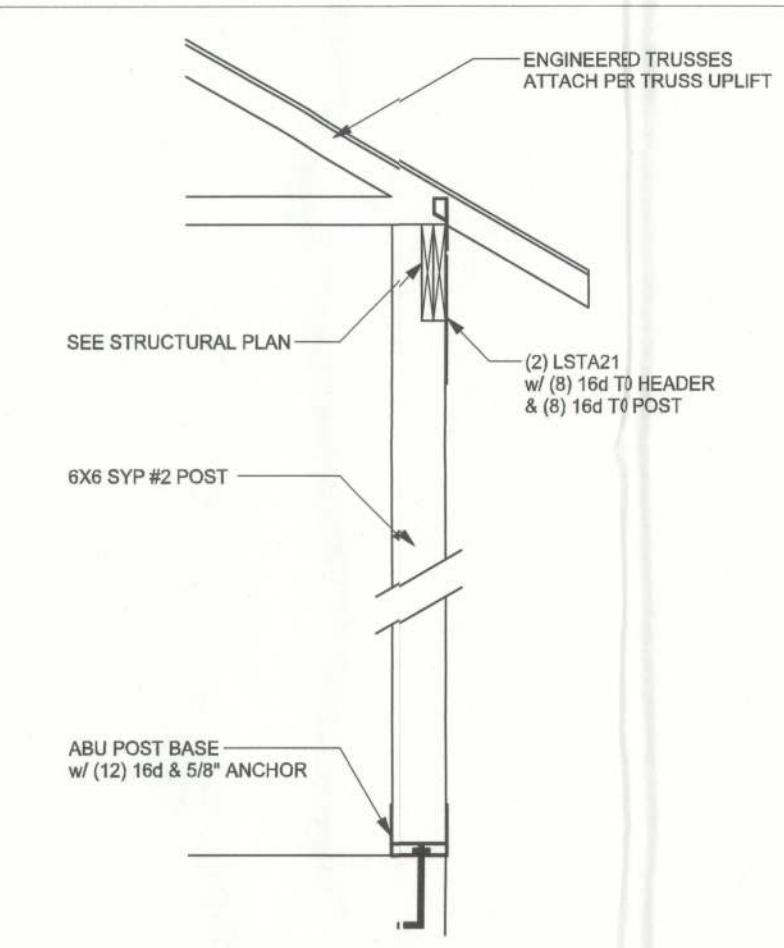
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME



(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

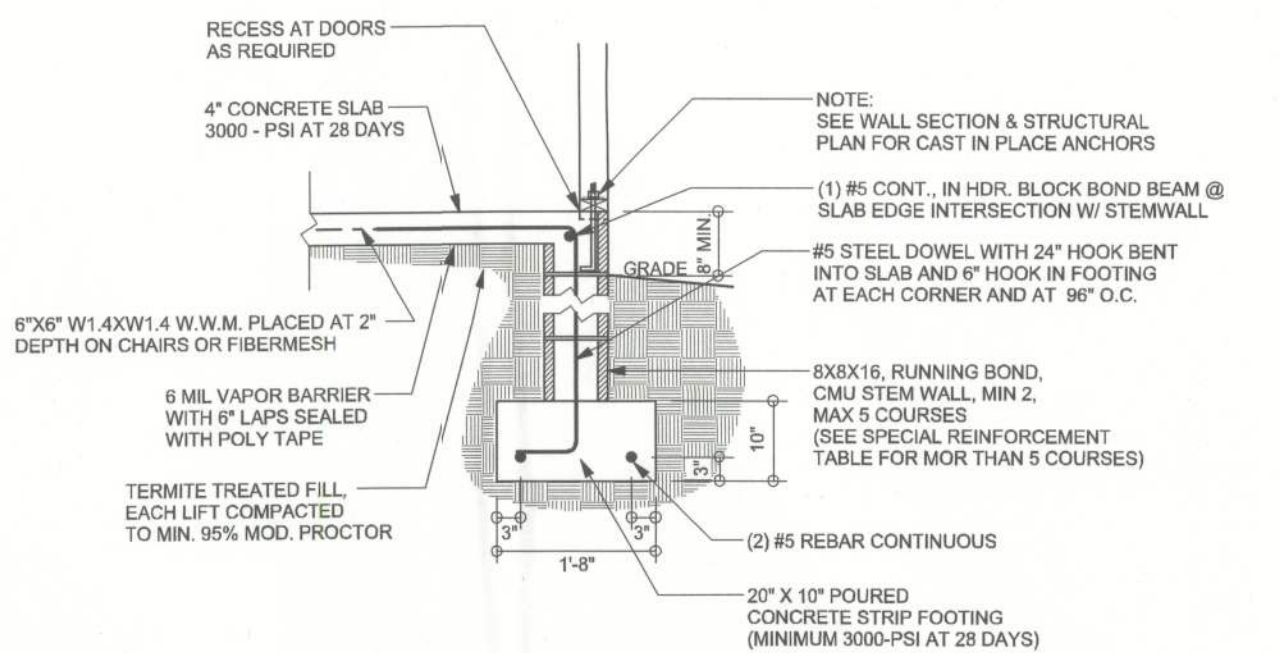


(TYP.) PORCH POST
ONE STORY WOOD

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
#5	#7	#8	#5	#7	#8		
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS		
	REQUIRED	ACTUAL
TRANSVERSE	15.2'	19.5'
LONGITUDINAL	19.1'	21.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #13-168A

HEADER LEGEND

- (2) 2X10X0' 1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
 - NUMBER OF JACK STUDS (UNDER HEADER)
 - SPAN OF HEADER
 - SIZE OF HEADER MATERIAL
 - NUMBER OF PLIES IN HEADER

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 3/8" A307 ALL THREADED ROD
- INDICATES LOCATION OF: 2ND FLOOR 3/8" A307 ALL THREADED ROD

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED (PER BCSI-1-03, BCSI-81, BCSI-82, & BCSI-83. BCSI-81, BCSI-82, & BCSI-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X8 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

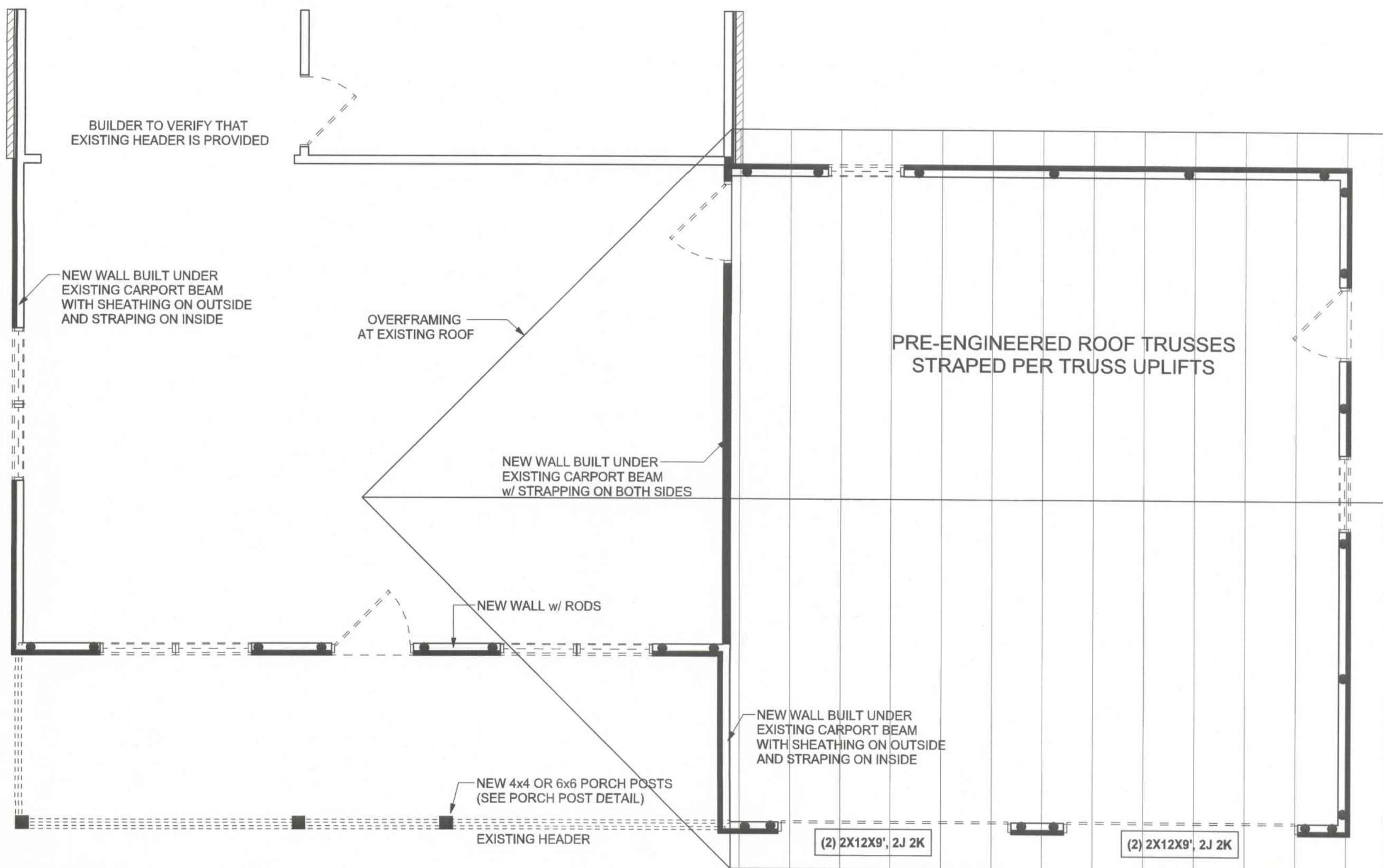
1 2X4 RAFTERS TO RIDGE	3-16d OR 6 - .131 x 3" TOE NAILS
2 CRIPPLE TO RIDGE	3-16d OR 6 - .131 x 3" FACE NAILS
3 CRIPPLE TO RAFTERS	3-16d OR 6 - .131 x 3" FACE NAILS
4 RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12 - .131 x 3" TOE NAILS
5 SLEEPER TO TRUSS	4-16d OR 8 - .131 x 3" FACE NAILS EACH TRUSS
6 RIDGE BOARD TO ROOF BLOCK	3-16d OR 6 - .131 x 3" TOE NAILS
7 RIDGE BOARD TO TRUSS	3-16d OR 6 - .131 x 3" TOE NAILS
8 PURLIN TO TRUSS (TYP.)	3-16d OR 6 - .131 x 3" NAILS
9 PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 8 - .131 x 3" NAILS
10 TRUSS TO BLOCKING	3-16d OR 6 - .131 x 3" END NAILS
11 CRIPPLE TO TRUSS	3-16d OR 6 - .131 x 3" FACE NAILS
12 CRIPPLE TO PURLIN	3-16d OR 6 - .131 x 3" FACE NAILS

GENERAL NOTES

- MAXIMUM RAFTER SPANS: 6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2
- MAXIMUM ROOF AREA PER SUPPORT: 1982 IN ZONES 2 & 3, 2412 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1982 OR 2'-0" X 8'-0" SPAN = 1682)
- PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
- PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
- IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8-6d COMMON WIRE NAILS.
- THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
 - SPANS (DISTANCES BETWEEN HEELS) 4'-0" OR LESS
 - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
 - MAXIMUM WIND SPEED: 120 MPH
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MAXIMUM TOTAL LOADING: 40 psf
 - MEETS FBC 2001/ASCE 7-08 WIND REQUIREMENTS
 - EXPOSURE CATEGORY "B", I = 1.0, Kd = 1.0
 - ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILLED w/ 2 - 16d NAILS OR 2X4 1" OR SCAB BRACE NAIL TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. 1" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBs OR BOTH FACES w/ 1" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

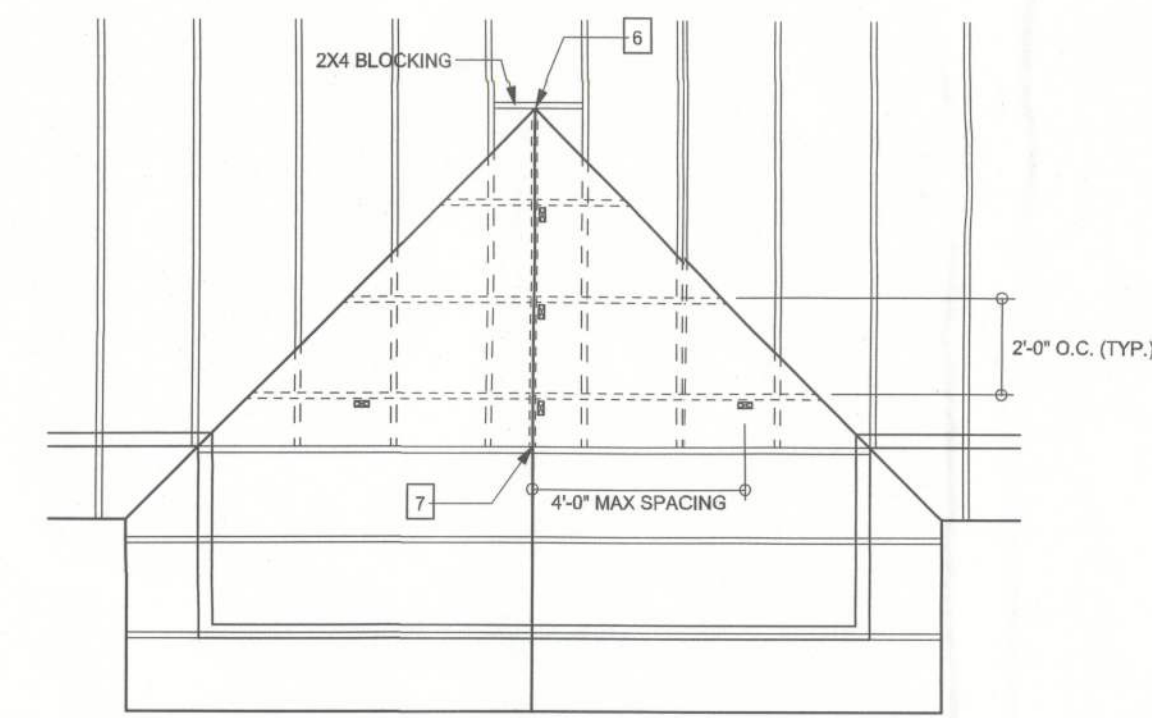


STRUCTURAL PLAN

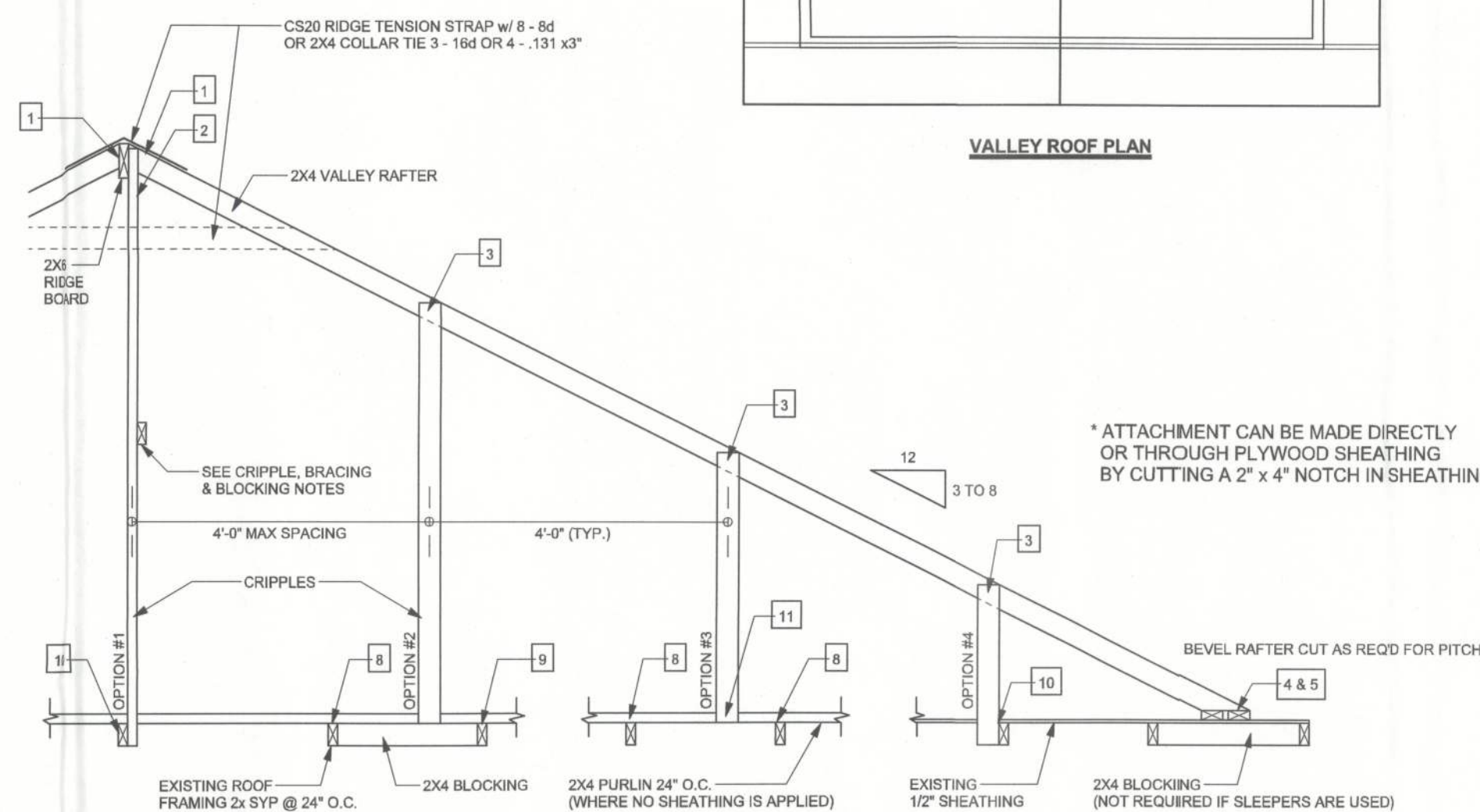
SCALE: 1/4" = 1'-0"

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT



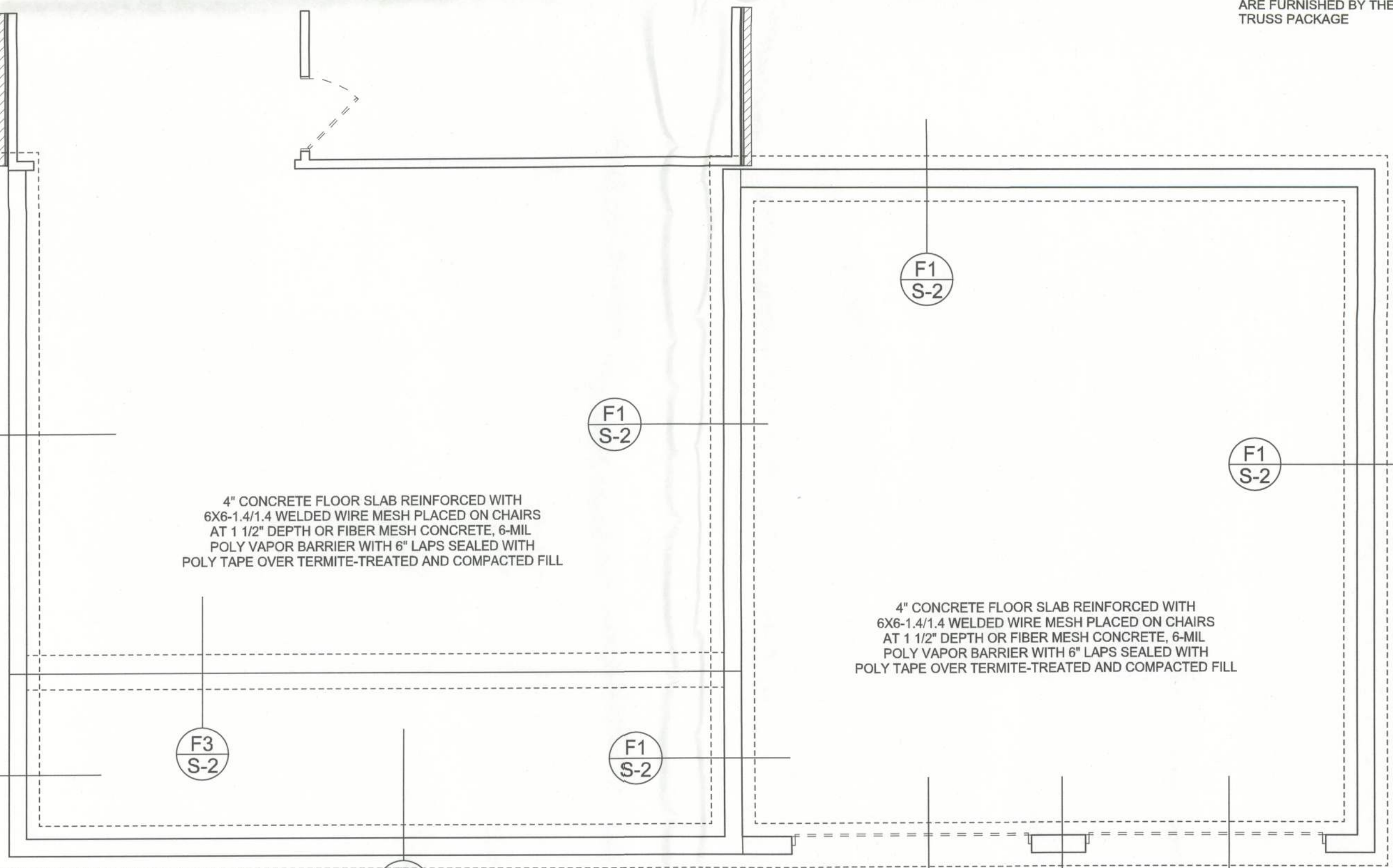
VALLEY ROOF PLAN



SECTION CUT PARALLEL TO VALLEY RAFTER

RETROFIT ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

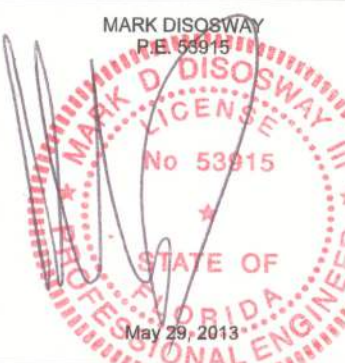
WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915, P.O. Box 868, Lake City, FL
32055, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Erkinger
Construction Group

Jeannette Steedley
Addition

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PRINTED DATE:
May 29, 2013
DRAWN BY: STRUCTURAL BY:
Evan Beamley

FINALS DATE:
2013-05-29

JOB NUMBER:
1304098

DRAWING NUMBER

S-2
OF 2 SHEETS