RESIDENTIAL ENERGY CONSERVATION CODE DOCUMENTATION CHECKLIST

Florida Department of Business and Professional Regulation Simulated Performance Alternative (Performance) Method

Applications for compliance with the 2017 Florida Building Code, Energy Conservation via the

residential Simulated Performance Method shall include:

	This checklist
	This checklist
	A Form R405 report that documents that the Proposed Design complies with Section R405.3 of the Florida Energy Code. This form shall Include a summary page indicating home address, e-ratio and the pass or fail status along with summary areas and types of components, whether the home was simulated as a worst-case orientation, name and version of the compliance software tool, name of individual completing the compliance report (one page) and an input summary checklist that can be used for field verification (usually four pages/may be greater).
	Energy Performance Level (EPL) Display Card (one page)
	HVAC system sizing and selection based on ACCA Manual S or per exceptions provided in Section R403.7
	Mandatory Requirements (five pages)
Rec	guired prior to CO for the Performance Method:
	Air Barrier and Insulation Inspection Component Criteria checklist (Table R402.4.1.1 - one page)
	A completed Envelope Leakage Test Report (usually one page)
	If Form R405 duct leakage type Indicates anything other than "default leakage", then a completed Form R405 Duct Leakage Test Report (usually one page)

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name: Chrismill Homes (Richter Job) Street: City, State, Zip: , FL , Owner: Paul & Pam Richter Design Location: FL, Gainesville	Bullder Name: Permit Office: Permit Number: Jurisdiction: County:: . Columbia (Florida Climate Zone 2)
1. New construction or existing 2. Single family or multiple family 3. Number of units, if multiple family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area above grade (ft²) 7. Windows(146.7 sqft.) Description a. U-Factor: Dbl, U=0.35 SHGC: SHGC=0.29 b. U-Factor: N/A SHGC: c. U-Factor: N/A SHGC: d. U-Factor: N/A SHGC: d. U-Factor: N/A SHGC: d. U-Factor: N/A SHGC: d. U-Factor: N/A SHGC: Area Weighted Average Overhang Depth: 0.000 ft. Area Weighted Average SHGC: 0.290 8. Floor Types (1920.0 sqft.) Insulation Area a. Slab-On-Grade Edge Insulation R=0.0 1920.00 ft² R= ft² Total Proposed Modifile	9. Wall Types (1656.0 sqft.) a. Frame - Wood, Exterior b. N/A c. N/A d. N/A R= ft² c. N/A d. N/A R= ft² d. N/A 10. Ceiling Types (1920.0 sqft.) b. N/A c. N/A c. N/A R= ft² R=30.0 1920.00 ft² b. N/A R= ft² R= ft² R= R= ft² R= ft² R=
Glass/Floor Area: 0.076 Total Proposed Modifie Total Baseline	
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: DATE: I hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT: DATE:	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553,908 Florida Statutes. BUILDING OFFICIAL: DATE:

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.
- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 7.00 ACH60 (R402.4.1.2).
- Compliance with a proposed duct leakage Qn requires a Duct Leakage Test Report confirming duct leakage to outdoors, tested in accordance with ANSI/RESNET/ICC 380, is not greater than 0.000 Qn for whole house.

				PROJ	ECT	_					
Title: Building Ty Owner Nan # of Units: Builder Nar Permit Offic Jurisdiction Family Typ New/Existir Comment:	pe: User ne: Paul & Pam R 1 ne: ce: : Single-family		Bedrooms Conditions Total Stor Worst Cas Rotate An Cross Ver Whole Ho	ed Area: les: se: gle: atilation:	3 1920 1 No 0		Address Lot # Block/Su PlatBook Street: County: City, State	ibdivision:	Street A		
		*	*	CLIMA	TE						
/	Design Location	TMY Site			esign Temp .5 % 2.5 %		gn Temp Summer	Heating Degree Da		elgn D sture	aily Temp Range
	FL, Gainesville	FL_GAINESVILLE	_REGI		32 92	70	75	1305.5	ŧ	51	Medium
				BLQC	KS						
Number	Name	Area	Volume								11-11-11
1	Block1	1920	17280								
	v			SPAC	ES	*					155
Number	Name	Area	Volume	Kitchen	Occupants	Bedrooms	infil II	D Finish	ad (Cooled	Heat
1	Main	1920	17280	Yes	5	3	1	Yes		Yes	Yes
		*		FLOO	RS			E. William			
V #	Floor Type	Space	Peri	meter	R-Value	Area		9	Tile	Mood	Carpet
1	Slab-On-Grade Edge		ain 184		11 10100	1920 ft²			0	0	1
				ROO	F					-76=	
√ #	Туре	Materials	Roof Area	Gable Area	20 M TO THE	Solar Absor.	SA Tested	Emitt	Emitt Tested	Deck	
1	Hip	Composition shing	les 2080 ft²	O ft²	Medium	0.96	No	0.9	No	0	22,6
		•		ATTI	C			***************************************			
√ #	Турв	Ventile	ation	Vent Ratio	o (1 in)	Area	RBS	IRCC			
1	Full attic	Vent	lad	300		1920 ft²	Υ	N			
-		V-V		CEILIN	IG						
V #	Ceiting Type		Space	R-Value	Ins T	ype Are	98	Framing Fr	ac Tr	іва Тур	0
	Under Attic (Ve		Main	30	Blow		O ft ²	0.11	11(-oo ryp	

	9	1					W	ALLS							
V	# O	nt	Adjace	nt Wall	СТуре	Spac	Cavity	Wid	ith In	Height Et In	Area	Sheathin	g Framing Fraction	Solar	Belov Grade
_		N	Exterior		me - Wood	Meir		32		9	288.0 ft²		0.23	0.75	(
_	2	E	Exterior	Fra	me - Wood	Mair	13	60		9	540.0 ft²		0.23	0.75	(
	3	S	Exterior	Fra	me - Wood	Main	13	32		9	288.0 ft³		0.23	0.75	(
_	4	E	Exterior	Fra	me - Wood	Mair	13	60	7	9	540.0 ft²		0.23	0.75	(
							DO	ORS							
V	#	!	Ornt		Door Type	Space		, , , , , , , , , , , , , , , , , , ,	Storms	U-Val	ue F	Width t In	Heigh Ft	t In	Area
	_ 1		E		Insulated	Main			None	.46		3	6	8	40 ft²
_	_ 2	!	E		Insulated	Main			None	.46	6	3	6	8	40 ft²
			440			Orientation sh		DOWS		i orientation	.				
. /			Wall				•				AND DESCRIPTION OF THE PERSON	rhang			
V	#	Or	nt ID	Frame	Panes	NFRC	U-Factor	SHGC	fmp	Area		Separation	Int She	ide .	Screeni
_	_ 1	E		Vinyl	Double (Tinte		0.35	0.29	N	36.0 ft²	0 ft 0 In	0 ft 0 in	Drapes/b	linds .	None
	_ 2	E	4	Vinyl	Double (Tinte	d) Yes	0.35	0.29	N	110.7 ft²	0 ft 0 in	0 ft 0 in	Drapes/b	linds	None
							INFILT	RATIO	N		•				
ŧ	Scope	•	M	lethod		SLA	CFM 50	ELA	E	EqLA	ACH	AC	H 50		
٧	Vholeho	use	Propo	sed AC	CH(50)	.0004	2016	110.68	20	08.14	.3082		7		
					,		HEATING	SYS	rem						
V	#	:	System T	урв		Subtype .			Efficience	cy (Capacity		E	Block	Ducts
	_ 1	J	Electric H	eat Pur	mp/	None	12	6	HSPF:8.	.2 36	3 kBtu/hr			1	sys#1
310							COOLING	3 SYS	TEM						
V	#		System T	уре		Subtype		E	fficiency	/ Capac	ity A	ir Flow	SHR E	Block	Ducts
_	_ 1	(Central U	nlt/		None		s	EER: 14	36 kBtu	/hr 10	80 cfm (0.76	1	sys#1
						ŀ	OT WATE	ER SY	STEM	1,-13					
V	#		System		SubType	Location	EF	Car		Use	SetPn	ıt	Conse	vation	
	_ 1		Electric		None	Attic	0.98	40 g	al .	60 gal	120 de	9	No	ne	
7						SOLA	R HOT W	ATER	SYST	EM					
V		SEC ert #	Comp	any Na	me		System Mod	el #	С	allector Mo		Callector Area	Storage Volume	FI	EF
		one	None												

								DUCTS								
√	# [upply R-Value	Area	Local	Return	 Area	Leaka	ge Type	Air Handler	CFM 25 TOT	CFM25 OUT	QN	RLF	HV.	AC#
	1	Attic	8	384 ft²	Atti	ic (96 ft²	Propo	sed Qn	Main	cſm	0.0 cfm	0.00	0.50	1	1
							TEM	PERATU	RE\$		*					
Programa	ble Therm	ostat: Y				Cellin	g Fan	6:	·	V6						
Cooling Heating Venting	X Jan Jan Jan	X Fe		Mar Mar Mar	Apr Apr X Apr		lay lay lay	X Jun Jun Jun	(X) Jul	X Aug Aug Aug	X Sep	X	Oct Oct Oct	X Nov X Nov X Nov	×	Dec Dec Dec
Thermostat		HERS 2	2006 Refe	20 (0.7)					Ho	กเล						
Schedule T	ype		1		2	3	4	5	6	7	8	8	10	11	1	12
Cooling (Wi	D)	AM PM	78 80	7	8	78 78	78 78	78 78	78· 78	78 78	78 78	80 78	80 78	80 78		30 78
Cooling (Wi	EH)	AM PM	78 78	7	8 8	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	7	8
leating (WI	D)	AM PM	66 68	6	8 6	86 88	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	6	88
leating (Wi	EH)	AM PM	66 68	6	6 6	36 38	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	9	86

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 100

The lower the EnergyPerformance Index, the more efficient the home.

, , FL,

2. 3. 4. 5.	Area Weighted Average 8. Floor Types	family ple family		From Plar o-family	ns)	9. Wall Types a. Frame - Wood, Exterior b. N/A c. N/A d. N/A 10. Ceiling Types a. Under Attic (Vented) b. N/A	Insulation R=13.0 R= R= R= Insulation R=30.0 R=	1656.00 ft² ft² ft² ft²
	Windows** a. U-Factor: SHGC: b. U-Factor: SHGC: c. U-Factor: SHGC: d. U-Factor:	Description Dbl, U=0.35 SHGC=0.29 N/A N/A N/A Overhang Depth		ļ	ft² ft²	c. N/A 11. Ducts a. Sup: Attle, Ret: Attle, AH: Main 12. Cooling systems a. Central Unit 13. Heating systems a. Electric Heat Pump	R■ kBtu/hr	R ft² 8 384 Efficiency SEER:14.00
8.	a. Slab-On-Grade Edge b. N/A	Insulation	Insulation R=0.0 R= R=		ft² ft²	Hot water systems a. Electric b. Conservation features None 15. Credits	Ca	p: 40 gallons EF: 0.96 Pstat

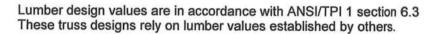
I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature:	Date:
Address of New Home:	City/FL Zip:



*Note: This is not a Building Energy Rating. If your Index is below 70, your home may qualify for energy efficient mortgage (EEM) Incentives if you obtain a Florida EnergyGauge Rating. Email EnergyGauge tech support at techsupport@energygauge.com or see the EnergyGauge web site at energygauge.com for information and a list of certified Raters. For information about the Florida Building Code, Energy Conservation, contact the Florida Building Commission's support staff.

**Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, If not DEFAULT.





2258671 - CHRISMILL HOMES - RICHTER RES.

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Chrismill Homes Project Name: Richter Res. Model: Custom

Subdivision: N/A

Lot/Block: N/A Address: 249 SW Grassland Way, N/A City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Wind Code: ASCE 7-10 Roof Load: 37.0 psf

Design Program: MiTek 20/20 8.2

Wind Speed: 130 mph Floor Load: N/A psf

This package includes 4 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T19483895	T01_	2/21/20
2	T19483896 T19483897	T01G T02	2/21/20
4	T19483898	T03	2/21/20 2/21/20



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

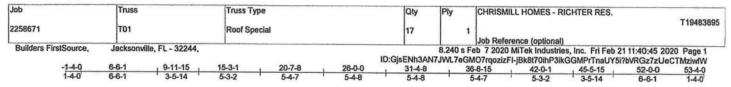
Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2021.

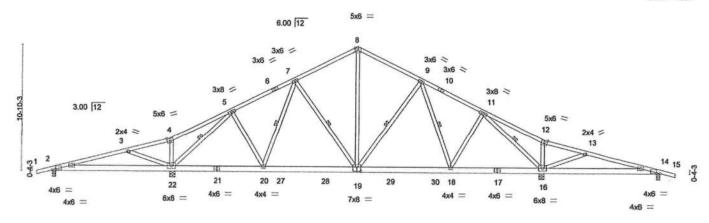
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



February 21,2020



Scale = 1:93.1



	1-	9-11-15	10-2-12	17-11-15		26-0-0		34-0-1	1	41-9-4	42-0-1	52-0-0	
Plate Offse	ets (X,Y)-	9-11-15 [2:0-3-6,0-0-1], [14:0-3	0-2-13 3-6,0-0-1],	7-9-3 [19:0-4-0,0-4-	31	8-0-1		8-0-1	-	7-9-3	0-2-13	9-11-15	'
LOADING TCLL TCDL BCLL	20.0 7.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	1.2 YE	5 5 S	CSI. TC 0.3 BC 0.4 WB 0.7	1	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.15 16-26 -0.14 22-24 0.04 14	I/defl >806 >893 n/a	L/d 240 180 n/a	PLATE MT20		GRIP 244/190
BCDL	10.0	Code FBC2017	7/TPI2014		Matrix-MS	3	77. 10				Weight	: 330 lb	FT = 20%

LUMBER-

 $e^{\frac{1}{2}-y}$

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WERS 2x4 SP No.3

BRACING.

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-5-11 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midot 9-19, 9-18, 11-16, 7-19, 7-20, 5-22

REACTIONS. All bearings 0-3-8 except (jt=length) 16=0-5-8, 22=0-5-8.

(lb) -Max Horz 2=227(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-315(LC 8), 16=-663(LC 13), 22=-682(LC 12),

14=-309(LC 9)

Max Grav All reactions 250 lb or less at joint(s) except 2=360(LC 23), 16=1644(LC 1), 22=1644(LC 1), 14=360(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-259/298, 4-5=-204/380, 5-7=-1097/621, 7-8=-990/673, 8-9=-990/673,

9-11=-1097/623, 11-12=-209/380, 12-13=-263/298

BOT CHORD 20-22=-324/937, 19-20=-294/1031, 18-19=-210/982, 16-18=-198/891

WEBS 8-19=-370/616, 9-19=-276/279, 11-18=-9/280, 11-16=-1539/831, 13-16=-477/489,

7-19=-276/268, 5-20=-4/276, 5-22=-1539/828, 3-22=-477/489

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 2, 663 lb uplift at joint 16, 682 lb uplift at joint 22 and 309 lb uplift at joint 14.



February 21,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

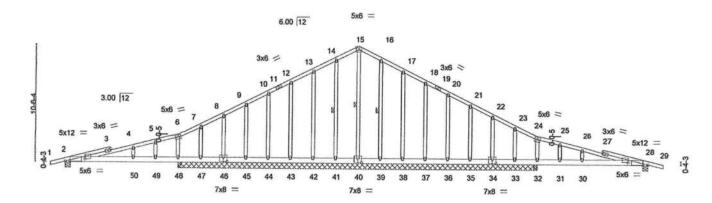
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Quality Criterio, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

Job Truss Truss Type Qty CHRISMILL HOMES - RICHTER RES. T19483896 2258671 T01G Roof Special Supported Gable Job Reference (optional) 8.240 s Feb 7 2020 MITek Industries, Inc. Fri Feb 21 11:40:48 2020 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID:GjsENh3AN7JWL7eGMO7rqozizFI-8mQGW93b_KRHbj?x5_1UC6AZCCbeengZfSss4gziwfT 52-0-0

Scale: 1/8"=1"



	10-0-0		-0-0 -0-0		52-0-0						
Plate Offsets (X,Y)-	[2:0-5-4,0-0-3], [2:1-9-0,0-2-7],	[28:0-5-4,0-0-3], [28:1-9-0,0-2-7], [34:0-4-0,0	-5-4,0-0-3], [28:1-9-0,0-2-7], [34:0-4-0,0-4-8], [40:0-4-0,0-4-8], [46:0-4-0,0-4-8]								
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0 Plate Grip DOL 1.3 Lumber DOL 1.3 Rep Stress Incr YE Code FBC2017/TPI2014	25 TC 0.51 Vei 25 BC 0.51 Vei 25 WB 0.13 Ho	L. in (loc) I/defl (LL) 0.18 2-50 >664 (CT) -0.18 2-50 >659 (CT) 0.01 28 n/a	L/d 240 180 n/a	PLATES GRIP MT20 244/190 Weight: 364 lb FT = 20%						

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No 2 **OTHERS** 2x4 SP No.3

* * *

BRACING-

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 8-10-10 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midot 15-40, 14-41, 16-39

REACTIONS. All bearings 32-0-0 except (jt=length) 2=0-5-8, 28=0-5-8.

Max Horz 48=221(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 41, 42, 43, 44, 45, 39, 38, 37, 36, 35 except 2=-302(LC 8), 28=-299(LC 9), 46=-154(LC 12), 47=-365(LC 23), 48=-664(LC 8), 34=-155(LC 13), 33=-365(LC 24), 32=-659(LC

All reactions 250 lb or less at joint(s) 41, 42, 43, 44, 45, 39, 38, 37, 36, 35 except 2=363(LC 23), 28=363(LC 24), 40=281(LC 22), 46=283(LC 23), 47=413(LC 8), 48=875(LC 23), 48=863(LC 1), 34=283(LC 24),

33=413(LC 9), 32=875(LC 24), 32=863(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-4=-320/162, 4-5=-285/156, 5-6=-256/157, 24-25=-257/157, 25-26=-285/156,

26-28=-321/162 **BOT CHORD**

2-50=-159/372, 49-50=-159/372, 48-49=-159/372, 47-48=-157/370, 46-47=-157/370,

45-48=-157/370, 44-45=-157/370, 43-44=-157/370, 42-43=-157/370, 41-42=-157/370, 40-41=-157/370, 39-40=-157/370, 38-39=-157/370, 37-38=-157/370, 36-37=-157/370,

35-36=-157/370, 34-35=-157/370, 33-34=-157/370, 32-33=-157/370, 31-32=-157/370,

30-31=-157/370, 28-30=-157/370

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 41, 42, 43, 44, 45, 39, 38, 37, 36, 35 except (jt=lb) 2=302, 28=299, 46=154, 47=365, 48=664, 34=155, 33=365, 32=659.



February 21,2020

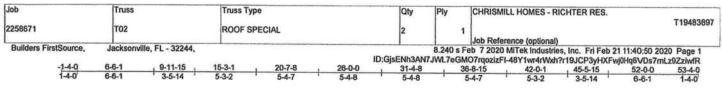
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MN-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters shown and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Eracing individual building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Eracing individual to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the abrication, storage, delivery, erection and bracing of trusses and truss systems, see

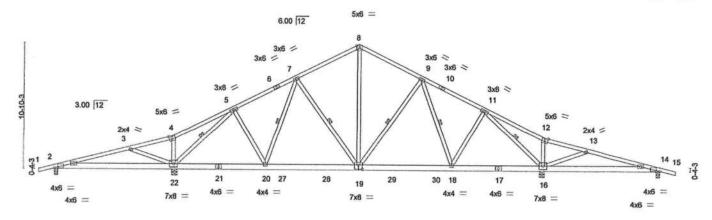
**ANSUTY Quality Criterie, DSB-89 and BCSI Building Composition of the property of the property incorporate the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the solution of the property damage. For general guidance regarding the property damage and property damage. For general guidance regarding the property damage and property damage. For general guidance regarding the property damage and property damage. For general guidance regarding



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Scale = 1:93.1



	1		10-2-12	17-11-15	-	26-0-0		34-0-1	-1	41-9-4	42-Q-1	52-0-0)
Plate Offse	ets (X Y)-	9-11-15 [2:0-3-6,0-0-1], [14:0-3	0-2-13 3-6 0-0-11	7-9-3	,	8-0-1		8-0-1	-	7-9-3	0-2-13	9-11-1	5
idio Onoc	715 (71,1)	[2.000,000],[14.00	-0,0-0-1],	[13.0-4-0,0-4-0	_								
LOADING	(psf)	SPACING-	2-4-	0	CSI.		DEFL.	in (loc)	I/defl	L/d	PLAT	ES	GRIP
TCLL	20.0	Plate Grip DOL	1.2	5	TC	0.45	Vert(LL)	0.18 16-26	>691	240	MT20		244/190
CDL	7.0	Lumber DOL	1.2	5	BC	0.53	Vert(CT)	-0.16 22-24	>766	180			
BCLL	0.0 *	Rep Stress Incr	. N	0 1	WB	0.90	Horz(CT)	0.04 14	n/a	n/a			
BCDL	10.0	Code FBC2017	7/TPI2014		Matrix		(01)	-1			Weigh	nt: 330 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3

* * *

BRACING-

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 5-0-5 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 9-19, 9-18, 11-16, 7-19, 7-20, 5-22

REACTIONS. All bearings 0-3-8 except (jt=length) 16=0-5-8, 22=0-5-8.

Max Horz 2=265(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) except 2=-367(LC 8), 16=-773(LC 13), 22=-795(LC 12),

Max Grav All reactions 250 lb or less at joint(s) except 2=420(LC 23), 16=1918(LC 1), 22=1918(LC 1),

14=420(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-259/237, 3-4=-302/347, 4-5=-238/443, 5-7=-1280/724, 7-8=-1155/785,

8-9=-1155/785, 9-11=-1280/726, 11-12=-243/443, 12-13=-306/347, 13-14=-259/212

BOT CHORD 2-22=-229/283, 20-22=-378/1094, 19-20=-343/1203, 18-19=-245/1145, 16-18=-230/1040 WEBS

8-19=-432/719, 9-19=-321/326, 11-18=-11/326, 11-16=-1795/970, 13-16=-556/571,

7-19=-321/313, 5-20=-4/322, 5-22=-1795/966, 3-22=-556/571

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 367 lb uplift at joint 2, 773 lb uplift at joint 16, 795 lb uplift at joint 22 and 361 lb uplift at joint 14.



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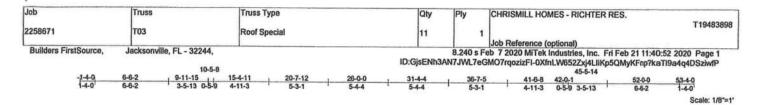
🔈 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIJ-7473 rev. 10/03/2015 BEFORE USE.

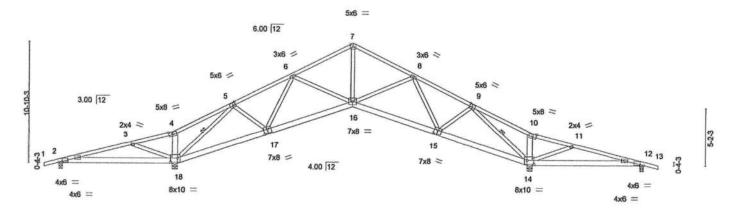
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ANSITPH Quality Criteria, DSB-69 and BCSI Building Compositely Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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	-		0-2-12	18-4-0 7-10-8		26-0-0		-8-0	-	41-6		41-9-4	52-0-0	
Plate Offse	ets (X,Y)-	[2:0-3-6,0-0-1], [5:0-2-8]. [12:0-3	7-8-0 -6,0-0-1], [14:	0-7-4,0-4-4], [15	8-0 5:0-4-0,0	4-8], [7-10 17:0-4-0,0)-8)-4-8],	0-2-12 [18:0-7-4,0-	10-2-12 -4-4]	
LOADING TCLL TCDL BCLL	20.0 7.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	1.	1-0 25 25 25	BC	0.47 0.38 0.67	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.20 1 0.16 1 0.16	-	l/defl >642 >783 n/a	L/d 240 180 n/a		PLATES MT20	GRIP 244/190
BCDL	10.0	Code FBC2017	/TPI201	4	Matrix-	MS						1	Weight: 308 lb	FT = 20%

LUMBER-

WERS

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TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x6 SP No.2

WEBS 2x4 SP No.3 *Except* 4-18,10-14: 2x6 SP No.2 BRACING-

WEBS

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 4-6-5 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 9-14, 5-18

REACTIONS. All bearings 0-3-8 except (jt=length) 18=0-5-8, 14=0-5-8.

(lb) - Max Horz 2=227(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-333(LC 8), 18=-719(LC 12), 14=-679(LC 13),

12=-321(LC 9)

Max Grav All reactions 250 lb or less at joint(s) except 2=275(LC 23), 18=1760(LC 1), 14=1760(LC 1),

12=275(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-120/501, 3-4=-458/862, 4-5=-428/985, 5-6=-1310/599, 6-7=-1486/623.

7-8=-1486/623, 8-9=-1310/596, 9-10=-434/985, 10-11=-463/862, 11-12=-117/461 **BOT CHORD** 2-18=-368/214, 17-18=-336/844, 16-17=-425/1365, 15-16=-255/1365, 14-15=-116/789,

12-14=-385/143

7-16=-327/1001, 8-16=-130/306, 8-15=-406/184, 9-15=-71/533, 9-14=-2194/1026, 11-14=-520/537, 6-17=-406/182, 5-17=-58/533, 5-18=-2194/1023, 3-18=-520/537

NOTES-1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 333 lb uplift at joint 2, 719 lb uplift at joint 18, 679 lb uplift at joint 14 and 321 lb uplift at joint 12.



February 21,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



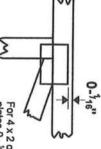
6904 Parke East Blvd. Tampa, FL 36610

Symbols

PLATE LOCATION AND ORIENTATION



Apply plates to both sides of truss and fully embed teeth. offsets are indicated Center plate on joint unless x, y Dimensions are in ft-in-sixteenths.



edge of truss. plates 0- 1/18" from outside For 4 x 2 orientation, locate

connector plates. This symbol indicates the required direction of slots in

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

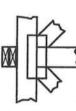
the length parallel to slots. width measured perpendicular to slots. Second dimension is The first dimension is the plate

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Min size shown is for crushing only number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

industry Standards:

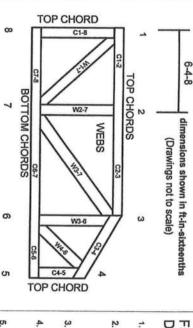
ANSI/TPI1: National Design Specification for Metal

BCSI: DSB-89

1 1 1 x x

Guide to Good Practice for Handling, Building Component Safety Information, Installing & Bracing of Metal Plate Connected Wood Trusses. Plate Connected Wood Truss Construction. Design Standard for Bracing.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other

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- Place plates on each face of truss at each locations are regulated by ANSI/TPI 1. joint and embed fully. Knots and wane at joint
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions idicated are minimum plating requirements
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- Install and load vertically unless indicated otherwise
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.