DATE 01/22/2008	DATE 01/22/2008 Columbia County Building Permit							
-	This Permit Must Be Prominently Posted	on Premises During Con	struction	000026646				
APPLICANT LINDA	RODER	PHONE	752-2281					
ADDRESS 387	SW KEMP COURT	LAKE CITY		FL 32024				
OWNER MART	TN HOME BUILDERS	PHONE	397-4534					
ADDRESS 148	SW GERALD CONNER DR	LAKE CITY		FL 32024				
CONTRACTOR	BEN MARTIN	PHONE	397-4534					
LOCATION OF PROF	ERTY 47S, TR ON 242, TR ON ARROV	VHEAD, TL ON GERAL	D CONNER DRIV	/Е,				
	2ND LOT ON RIGHT							
TYPE DEVELOPMEN	SFD,UTILITY ES	TIMATED COST OF CO	NSTRUCTION	95000.00				
HEATED FLOOR AR	EA 1400.00 TOTAL ARE	EA1900.00	HEIGHT	STORIES 1				
FOUNDATION CO	ONC WALLS FRAMED F	ROOF PITCH 6/12	FLC	OOR SLAB				
LAND USE & ZONIN	G RSF-2	MAX	. HEIGHT 16	5				
Minimum Set Back Re	quirments: STREET-FRONT 25.00	REAR	15.00	SIDE 10.00				
NO. EX.D.U. 0	FLOOD ZONE X PP	DEVELOPMENT PERI	MIT NO.					
	-16-03114-148 SUBDIVISIO							
LOT 48 BLOC	K PHASE UNIT _		AL ACRES0.0	5				
000001530	CBC059077	T.	On Al	01				
Culvert Permit No.	Culvert Waiver Contractor's License Nur		Applicant/Owner/C	Contractor				
CULVERT	08-0045 BK		JH	N				
Driveway Connection			proved for Issuance					
COMMENTS: FLOC	OR ONE FOOT ABOVE THE ROAD MFE		tion Conti	maxon /ENEV				
	758	ded & S/AB	Cl	-1. 2683				
	NSE.	ded & S/AB	Check # or Ca	ash 2683				
	FOR BUILDING & ZONII			ash 2683 (footer/Slab)				
Temporary Power	FOR BUILDING & ZONII	NG DEPARTMENT	ONLY	(footer/Slab)				
_	FOR BUILDING & ZONII Foundation date/app. by	NG DEPARTMENT	ONLY Monolithic	(footer/Slab) date/app. by				
_	FOR BUILDING & ZONII Foundation date/app. by umbing Slab	NG DEPARTMENT date/app. by	ONLY Monolithic	(footer/Slab) date/app. by Nailing				
Under slab rough-in p	FOR BUILDING & ZONII Foundation date/app. by lumbing Slab date/app. by	date/app. by	Monolithic Sheathing/N	(footer/Slab) date/app. by				
Under slab rough-in p	FOR BUILDING & ZONII Foundation date/app. by lumbing Slab date/app. by	NG DEPARTMENT date/app. by	Monolithic Sheathing/N	(footer/Slab) date/app. by Nailing				
Under slab rough-in p Framingda	FOR BUILDING & ZONII Foundation date/app. by umbing Slab date/app. by Rough-in plumbing a te/app. by Heat & Air Duct	date/app. by date/app. by date/app. by	Monolithic Sheathing/N	(footer/Slab) date/app. by Nailing date/app. by date/app. by				
Under slab rough-in p Framingda	FOR BUILDING & ZONII Foundation date/app. by lumbing Slab date/app. by Rough-in plumbing a	date/app. by	Monolithic Sheathing/Nod floor Peri. beam (Linter	(footer/Slab) date/app. by Nailing				
Under slab rough-in p Framing da Electrical rough-in	FOR BUILDING & ZONII Foundation date/app. by umbing date/app. by Rough-in plumbing a te/app. by Heat & Air Duct date/app. by C.O. Final	date/app. by date/app. by date/app. by above slab and below woo date/app. by	Monolithic Sheathing/N	(footer/Slab) date/app. by Nailing				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Jumbing date/app. by Rough-in plumbing a te/app. by Heat & Air Duct date/app. by C.O. Final	date/app. by date/app. by date/app. by	MonolithicSheathing/Nod floorPeri. beam (Lintel Culvert	(footer/Slab) date/app. by Nailing				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by umbing date/app. by Rough-in plumbing a te/app. by Heat & Air Duct date/app. by C.O. Final	date/app. by date/app. by date/app. by above slab and below woo date/app. by date/app. by	Monolithic Sheathing/Nd floor Peri. beam (Lintel Culvert Pool	(footer/Slab) date/app. by Nailing				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Mate/app. by Rough-in plumbing a te/app. by Heat & Air Duct date/app. by C.O. Final date/app. by ng, electricity and plumbing Pump pole	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Po	Monolithic Sheathing/Nd floor Peri. beam (Lintel Culvert Pool ble	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Mate/app. by Rough-in plumbing a date/app. by Heat & Air Duct date/app. by C.O. Final date/app. by ng, electricity and plumbing date/app. by Travel Trailer	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Poe/app. by	Monolithic Sheathing/Nd floor Peri. beam (Lintel Culvert Pool ble date/app. by	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Mate/app. by Rough-in plumbing a date/app. by Heat & Air Duct date/app. by C.O. Final date/app. by ng, electricity and plumbing date/app. by Travel Trailer	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Po	Monolithic Sheathing/Nd floor Peri. beam (Lintel Culvert Pool ble date/app. by	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Slab date/app. by Rough-in plumbing a te/app. by C.O. Final date/app. by ng, electricity and plumbing date/app Pump pole date/app. by Travel Trailer	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Pole/app. by date/app. by	Monolithic Sheathing/None	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Jumbing Rough-in plumbing a te/app. by Heat & Air Duct date/app. by C.O. Final date/app. by ng, electricity and plumbing date/app. by Travel Trailer by FEE \$ 475.00 CERTIFICATION FI	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Pole/app. by date/app. by EE \$ 9.50	Monolithic Sheathing/None floor Sheathing/None floor Peri. beam (Lintel Culvert Pool Surcharge Su	(footer/Slab) date/app. by Nailing				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Iumbing Rough-in plumbing a te/app. by Heat & Air Duct date/app. by C.O. Final date/app. by ng, electricity and plumbing date/app Pump pole date/app. by Travel Trailer by FEE \$ 475.00 CERTIFICATION FI 0.00 ZONING CERT. FEE \$ 50.0	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Pole/app. by date/app. by EE\$ 9.50	Monolithic Sheathing/Nod floor Sheathing/Nod floor Peri. beam (Lintel Culvert Pool Bate/app. by Re-roof SURCHARGE WAST_	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by EFEE \$ 9.50 EFEE \$ 9.50				
Under slab rough-in p Framing	FOR BUILDING & ZONII Foundation date/app. by Jumbing Rough-in plumbing a te/app. by Heat & Air Duct date/app. by C.O. Final date/app. by ng, electricity and plumbing date/app. by Travel Trailer by FEE \$ 475.00 CERTIFICATION FI	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by Utility Perecapp. by date/app. by Grapp. by date/app. by Column 1	Monolithic Sheathing/Node floor Peri. beam (Linter Pool by Re-roof SURCHARGE WAST	(footer/Slab) date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by All FEE \$ 9.50 E FEE \$				

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

UC# 2683

Columbia County Building Permit Application

For Office Use Only Application # 0801-31 Date Received 1/9/08 By 9 Permit # 1530/ 2664
Date 6.0/.08 Flood Zone FEMA Man # 41/4 Zoning O(F-2
Land Use NEX Low Va Elevation N/A MFE profet River N/A Plans Examiner OK JTM Date 1-17-0
Comments 195 and NOC MISSING (Well letter) Elevation Continued a Letter Oc
Deed or PA Site Plan State Road Info Parent Parcel #
Dev Permit # In Floodway Detter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Fax_752-2287
Name Authorized Person Signing Permit Linda Or Melanic Roder Phone 752-228/
Address 387 SW Kemp Ct Lake City FL 32024
Owners Name Martin Home Builders" 397-4534
911 Address 148 SW Gerald Conner Dr. Lake City FL 37024
Contractors Name Ben Martin Phone 397-4534
Address P. D. Box 1831 Lake GtyFL 32056
Fee Simple Owner Name & Address
Bonding Co. Name & AddressNA
Architect/Engineer Name & Address Mark DISOSWAY Mortgage Lenders Name & Address NA
Circle the correct power company - FL Power & Light - Clay Elec Suwannee Valley Elec Progress Energy
Property ID Number 24-45-16-53 114-148 Estimated Cost of Construction
SUDDIVISION NAMA (CIAIA EL) (SE C) () ()
Driving Directions 475, Rob 242, Rob 242, Rob 242, Driving Directions 475, Rob 242,
Driving Directions 475, Ron 242, Ron Arrow lead, Lon Sw Gerald Conner Drive, Lot is 2nd down on R
Drive, Loi) is 2nd down on R
Number of Existing Dwellings on Property
Do you need a Culvert Permit or Cut Live Total Acreage 510 Lot Size 519
CONTRACT TENTION OF CUIVER WRIVER OF HOVE ON Evidence D.
Actual Distance of Structure from Property Lines - Front Side 47
Heated Floor Area 1900 Total Heated Floor Area 1900 Post Black
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or of all laws regulating construction in the issuance of a permit and that all work be performed to meet the standards

Application #	L	

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY, A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have oald your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit. Owners Signature Affirmed under penalty of perjury to by the Owner and subscribed before me this _____ day of _ Personally known ____ or Produced Identification Linda R. Roder Commission #DD303275 State of Florida Notary Signature (For the Owner) Expires: Mar 24, 2008 Bonded Thru Atlantic Bonding Co., Inc. CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining ontractor's Signature (Permitee) Contractor's License Number CBC 059077 **Columbia County** Competency Card Number Affirmed under penalty of perjury to by the Contractor and subscribed before me this 2 day of 300 or Produced Identification_ State of Florida Notary Signature (For the Contractor) Linda R. Roder

07/05/5008 10:10 386752288

Revised 11-13-07

Commission #DD303275 Expires: Mar 24, 2008

Bonded Thru Atlantic Bonding Co., Inc.

Page 2 of 2 (Both Pages must be submitted together.)

Notice of Authorization

Martin Hone Builders 1 Ben Martin do hereby authorize Linda Roder or Melanie Roder	
The Martin do hereby suthorize Linda Roder or Melanie Roder, to be my representative and act on my behaf in all aspects of applying for any building permit to be located in Columbia county. Any homeowner and legal description 24.45-16-03114-148 Cannon Circle Place Contractor's signature Linda R. Roder Commission #D030375 Expires: Mar 24, 2008 Bonded Thru Atlantic Bonding Co., Inc. Sworn and subscribed before me this 2 day of 2008 Notary Public My commission expires:	
building permit to be located in Columbia county.	
Any homeowner and legal description 24-45-16-03114-148 Lot 48 Cannon Creek F	Place
XBan Mark	
Commission #DD303275 Expires: Mar 2008 Ronded Thu	
	4
Sworn and subscribed before me this day of	>
Luda Pilili	
Notary Public	
My commission expires:	
Personally known Produced ID (Type):	
N N	

Prepared by: Michael H. Harrell Abstract & Title Services, Inc. 111 East Howard Street Live Oak, Florida 32064

Warranty Deed

Individual to Individual

THIS WARRANTY DEED made the 2nd day of June, 2006 by

Peter W. Giebeig, A Single Person

hereinafter called the grantor, to

Martin Home Builders, Inc.

whose post office address is: PO Box 1831, Lake City, FL 32056 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the grantee, all that certain land situate in COLUMBIA County, FLORIDA, viz: Parcel ID# PART OF: R03114-001

Lot 48, of Cannon Creek Place, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 31-34, of the Public Records of Columbia County, Florida.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2005.

IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Chu Bet

Elline K. Folar

Printed Name

inst:2006013521 Date:06/06/2006 Time:11:56

W. Giebeig

loc Stamp-Deed: 279.30

2. 7. DC,P. DeWitt Cason,Columbia County B: 1085 P:2215

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 2nd day of June, 2006 by Peter W. Giebeig, A Single Person personally known to me or, if not personally known to me, who produced for identification and who did not take an oath.

(SEAL)

DORIS M DRAKE
MY COMMISSION # DD537517
EXPIRES: Apr. 5, 2010
EXPIRES: Apr. 5, 2010

Wotary Public

My Commission Expires:

Lot 48 Cannon Creek Place Martin Home Builders 24-45-16-03/14-148 510 acres 150' -85 ,051 dribe SW Gerald Conner Dr 180 150



Lake City (386) 755-3611 Gainesville (352) 494-5751 Fax (386) 755-3885 Toll Free 1-800-616-4707

Notice of Intent for Preventative Treatment for Termites (as required by Florida Building Code (FBC) 104.2.6)

Aspen Pest Control, Inc. (386) 755-3611 State License # - JB109476 State Certification # - JF104376

Lot 48 Cannon Creek 24-45-16-03114-148 (Martin Home Builders)

Address of Treatment or Lot/Block of Treatment

Bora-Care Wood Treatment - 23% Disodium Octaborate Tetrahydrate

(Method of Termite Prevention Treatment - Soil Barrier, Wood Treatment, Bait System, Other

Application onto Structural Wood

Description of Treatment

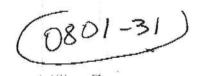
The above named structure will receive a complete treatment for the prevention of subterranean termites at the dried-in stage of construction. Treatment is done in accordance with the rules and laws established by the Florida Department of Agriculture and Consumer Services and according to EPA registered label directions as stated in Florida Building Code Section 1861.1.8

Authorized Signature

Date







Jan 18 08 09:37a

Linda Newcomb

386-752-1477

p. 1

Water Wells Pumps & Service Phone: (386) 752-6677 Fax: (386) 752-1477

Lynch Well Drilling, Inc.

173 SW Young Place Lake City, FL 32025 www.lynchwelldrilling.com

January 17, 2008

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the Martin Home Builders well Cannon Creek Pl Lot 48, Parcel # 24-4S-16-03114-148.

Size of Pump Motor:

1 Horse Power 20 gallon GPM

Size of Pressure Tank:

81 -Gallon Bladder Tank - 25.1 Draw down

Cycle Stop Valve Used: Constant Pressure System:

inda Newcombe

No No

Should you require any additional information, please contact us.

Sincerely,

Linda Newcomb

Lynch Well Drilling, Inc.

(0801-31)

1-18-08; 10:40AM; ENVIRONMENTAL

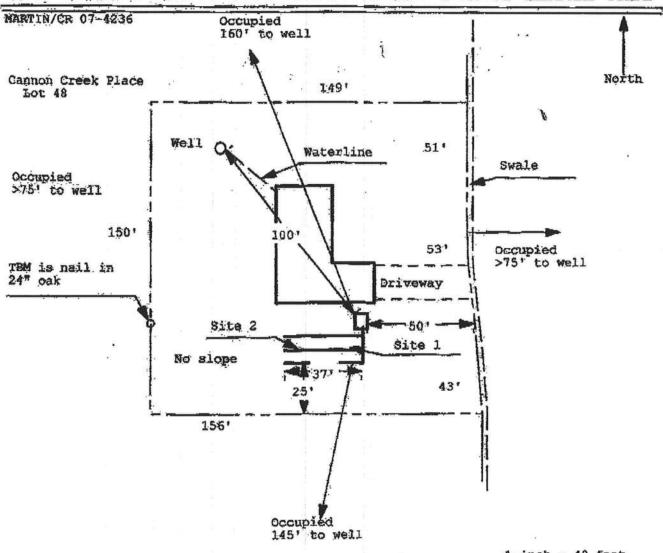
RODER

:3867582187

1/

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number:

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



			145' to Well			*	
				00	1 11	nch = 40 feet	
Site Plan	Plan Submitted	By Not Appro	Tool L	Salar	14-08	80/08	<u>-</u> ,
By	Mar s	2and		Colu	~k;s	CREU	
Note	51						

Project Name:

Address:

801011MartinBen

Lot: 48. Sub: Cannon Creek Plat-

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Builder: Martin

Permitting Office: Calum 6.1

City, State:	Lake City, FL	mon orden, rian			6646	
Owner: Climate Zone:	Spec House North			Jurisdiction Number:	221000	
a. U-factor:	ulti-family f multi-family ms ? area (ft²) ea: (Label reqd. by 13-104. Des ble DEFAULT) 7a. (Dble DEFAULT) 7b. ge Insulation	cription Area	b. N/A c. N/A 14. Hot wa a. Electric b. N/A c. Conser (HR-H DHP-I 15. HVAC (CF-Ce HF-W PT-Pro MZ-C-	g systems c Heat Pump ater systems c Resistance vation credits eat recovery, Solar Dedicated heat pump)	Cap: 30.0 kBtu/h SEER: 13.00 Cap: 30.0 kBtu/h HSPF: 7.90 Cap: 40.0 gallon: EF: 0.93	or
Glass	s/Floor Area: 0.12	Total as-built p Total base p		$D \cap C$	3	
esa an esas.						

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY:

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT:

DATE:

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code.
Before construction is completed this building will be inspected for compliance with Section 553,908 Florida Statutes.

COD WE THUS

BUILDING OFFICIAL:

DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 48, Sub: Cannon Creek, Plat: , Lake City, FL,

PERMIT #:

	BASE					AS-	BU	LT				
GLASS TYPES .18 X Condition	oned X BS	SPM =	Points	Type/SC	Ove Ornt	rhang		Area X	SPI	M X	SOF	= Points
.18 1400	0.0	20.04	5050.1	Double, Clear	E	1.5	5.5	30.0	42.0		0.90	1131.0
				Double, Clear Double, Clear	E	1.5 1.5	3.5 7.3	9.0 40.0	42.0		0.78	293.6 1590.5
				Double, Clear	E	1.5	5.5	15.0	42.0		0.95	565.5
				Double, Clear	w	1.5	5.5	40.0	38.5		0.90	1382.1
				Double, Clear	W	5.0	7.3	13.3	38.5		0.63	323.4
				Double, Clear	N	1.5	5.5	20.0	19.2		0.03	356.4
.'				As-Built Total:				167.3				5642.5
WALL TYPES	Area X	BSPM	= Points	Туре		R-\	/alue	Area	Х	SPM	=	Points
Adjacent	140.0	0.70	98.0	Frame, Wood, Exterior			13.0	948.7		1.50		1423.1
Exterior	948.7	1.70	1612.8	Frame, Wood, Adjacent			13.0	140.0		0.60		84.0
Base Total:	1088.7		1710.8	As-Built Total:				1088.7		5 700-5 5		1507.1
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	Х	SPM	=	Points
Adjacent	20.0	1.60	32.0	Exterior Insulated				20.0		4.10		82.0
Exterior	20.0	4.10	82.0	Adjacent Insulated				20.0		1.60		32.0
Base Total:	40.0		114.0	As-Built Total:				40.0				114.0
CEILING TYPE	S Area X	BSPM	= Points	Туре	F	R-Value	e A	rea X S	PM	x sc	M =	Points
Under Attic	1400.0	1.73	2422.0	Under Attic		,	30.0	1400.0	1.73 >	(1.00		2422.0
Base Total:	1400.0		2422.0	As-Built Total:				1400.0				2422.0
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-V	/alue	Area	X	SPM	=	Points
Slab	162.0(p)	-37.0	-5994.0	Slab-On-Grade Edge Insulatio	n		0.0	162.0(p	٠.	41.20		-6674.4
Raised	0.0	0.00	0.0	Section and the Section of the Secti								A 1 1 124-44 (\$1 1 0 0 0 1 1)
Base Total:			-5994.0	As-Built Total:				162.0				-6674.4
INFILTRATION	Area X	BSPM	= Points					Area	X	SPM	=	Points
	1400.0	10.21	14294.0					1400.0)	10.21		14294.0

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 48, Sub: Cannon Creek, Plat: , Lake City, FL, PERMIT #:

	BASE		AS-BUILT									
Summer Ba	se Points:	17596.9	Summer As-Built Points:	17305.1								
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)									
17596.9	0.4266	7506.8	(sys 1: Central Unit 30000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6 17305 1.00 (1.09 x 1.147 x 0.91) 0.263 1.000 17305.1 1.00 1.138 0.263 1.000	5168.9								

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 48, Sub: Cannon Creek, Plat: , Lake City, FL, PERMIT #:

E	BASE			AS-BUILT									
GLASS TYPES													
.18 X Conditions Floor Area		SWPM =	Points	Type/SC	Ornt	erhang Len	Hgt	Area X	W	PM	Х	WOF	= Point
.18 1400.0		12.74	3210.5	Double, Clear	Е	1.5	5.5	30.0	18	.79		1.04	587.1
				Double, Clear	Ε	1.5	3.5	9.0	18	.79		1.09	185.0
				Double, Clear	E	1.5	7.3	40.0		.79		1.02	769.8
				Double, Clear	E	1.5	5.5	15.0		.79		1.04	293.5
				Double, Clear	W	1.5	5.5	40.0		.73		1.03	852.4
11				Double, Clear	W	5.0	7.3	13.3		.73		1.12	309.5
): 				Double, Clear	N	1.5	5.5	20.0	24	.58		1.00	493.0
				As-Built Total:				167.3					3490.4
WALL TYPES	Area X	BWPM	= Points	Туре		R-\	/alue	Area	Х	WF	PM	=	Points
Adjacent	140.0	3.60	504.0	Frame, Wood, Exterior			13.0	948.7		3.4	40		3225.6
Exterior	948.7	3.70	3510.2	Frame, Wood, Adjacent			13.0	140.0		3.3	30		462.0
Base Total:	1088.7		4014.2	As-Built Total:				1088.7					3687.6
DOOR TYPES	Area X	BWPM	= Points	Туре				Area	Х	WF	М	=	Points
Adjacent	20.0	8.00	160.0	Exterior Insulated				20.0		8.4	40		168.0
Exterior	20.0	8.40	168.0	Adjacent Insulated				20.0		8.0	00		160.0
Base Total:	40.0		328.0	As-Built Total:				40.0					328.0
CEILING TYPES	Area X	BWPM	= Points	Туре	R-	Value	Ar	ea X WI	PM	×ν	VCN	M =	Points
Under Attic 1	400.0	2.05	2870.0	Under Attic			30.0	1400.0	2.05	X 1.0	00		2870.0
Base Total:	1400.0		2870.0	As-Built Total:				1400.0					2870.0
FLOOR TYPES A	Area X	BWPM	= Points	Туре		R-V	/alue	Area	х	WP	M	=	Points
Slab 162	2.0(p)	8.9	1441.8	Slab-On-Grade Edge Insulation	n		0.0	162.0(p		18.8	30		3045.6
Raised	0.0	0.00	0.0		100		202	W.			150).		
Base Total:			1441.8	As-Built Total:				162.0					3045.6
INFILTRATION A	Area X	BWPM	= Points					Area 2	X	WP	M	=	Points
	1400.0	-0.59	-826.0					1400.0)	-0.	59		-826.0

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 48, Sub: Cannon Creek, Plat: , Lake City, FL, PERMIT #:

	BASE		AS-BUILT								
Winter Base	Points:	11038.5	Winter As-Built Points:	2595.6							
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	, reating							
11038.5	0.6274	6925.5	(sys 1: Electric Heat Pump 30000 btuh ,EFF(7.9) Ducts:Unc(S),Unc(R),Int(AH) 12595.6 1.000 (1.069 x 1.169 x 0.93) 0.432 1.000 12595.6 1.00 1.162 0.432 1.000	,R6.0 6318.6 6318.6							

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 48, Sub: Cannon Creek, Plat: , Lake City, FL, PERMIT #:

	Е	BASE			AS-BUILT								
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X	Credit Multipli	= Total er	
3		2635.00		7905.0	40.0	0.93	3		1.00	2606.67	1.00	7820.0	
					As-Built To	tal:						7820.0	

	CODE COMPLIANCE STATUS												
BASE							AS-BUILT						
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
7507		6926		7905		22337	5169		6319		7820		19308

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 48, Sub: Cannon Creek, Plat: , Lake City, FL, PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: 3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.7

The higher the score, the more efficient the home.

Spec House, Lot: 48, Sub: Cannon Creek, Plat: , Lake City, FL,

1.	New construction or existing	New		12.	Cooling systems		
2.	Single family or multi-family	Single family		a.	Central Unit	Cap: 30.0 kBtu/hr	
3.	Number of units, if multi-family	1				SEER: 13.00	-
4.	Number of Bedrooms	3		b.	N/A		
5.	Is this a worst case?	Yes					
6.	Conditioned floor area (ft²)	1400 ft ²		C.	N/A		_
7.	Glass type 1 and area: (Label reqd.)	by 13-104.4.5 if not default)	-				
a.	U-factor:	Description Area		13.	Heating systems		
	(or Single or Double DEFAULT)	7a. (Dble Default) 167.3 ft ²			Electric Heat Pump	Cap: 30.0 kBtu/hr	
b.	SHGC:	,	Street's			HSPF: 7.90	
	(or Clear or Tint DEFAULT)	7b. (Clear) 167.3 ft ²		b.	N/A		
8.	Floor types	(0.000) 107.0 11	3 1111				7.7
	Slab-On-Grade Edge Insulation	R=0.0, 162.0(p) ft		c.	N/A		_
	N/A				5100		-
C.	N/A		_	14.	Hot water systems		_
9.	Wall types		-		Electric Resistance	Cap: 40.0 gallons	
a.	Frame, Wood, Exterior	R=13.0, 948.7 ft ²				EF: 0.93	
	Frame, Wood, Adjacent	R=13.0, 140.0 ft ²		b.	N/A		-
c.	N/A	•					_
d.	N/A			c.	Conservation credits		_
e.	N/A		_		(HR-Heat recovery, Solar		_
	Ceiling types		_		DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 1400.0 ft ²		15	HVAC credits		
	N/A	100.0, 1100.0 1	_		(CF-Ceiling fan, CV-Cross ventilation,		
c.	N/A		_		HF-Whole house fan,		
11.	Ducts		_		PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 156.0 ft			MZ-C-Multizone cooling,		
	N/A		-		MZ-H-Multizone heating)		
					, and the same state of the sa		
Lce	rtify that this home has compli-	ed with the Florida France	n Effic	ienc	y Code For Building		
Con	struction through the above en	erov caving factures which	h will	he i	estalled (or exceeded)	OF THE STATE	
in th	nis home before final inspection	n Otherwise a new FDI	Dienla	v Ca	rd will be completed	3	A
hase	ed on installed Code compliant	features	Dispia	y Ca	nd will be completed	2 2	8
	100000		ъ.				
וווום	der Signature:		Date:			13	Z
						1.	
Add	ress of New Home:		City/F	L Zi	p:	GOD WE TRUST	9
			-			WE I TOO	

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStd^M designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.



OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection This Certificate of Occupancy is issued to the below named permit holder for the building

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03114-148

Building permit No. 000026646

Use Classification SFD,UTILITY

Permit Holder BEN MARTIN

Fire: 19.26

Owner of Building MARTIN HOME BUILDERS

Date: 07/22/2008

Location:

148 SW GERALD CONNER DR., LAKE CITY, FL

Waste: 50.25

Total: 69.51

Wayne M. Rue

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



BRITT SURVEYING

830 West Duval Street • Lake City, FL 32055 Phone (386) 752-7163 • Fax (386) 752-5573

03/14/08

L-19170

To Whom It May Concern:

C/o: Ben Martin

Re: Lot 48 Cannon Creek Place

The elevation of the proposed foundation is found to be 104.00 feet. The recommended finished floor elevation is 101.00 feet as per the plat of record. The highest adjacent grade is 102.5 feet and the lowest adjacent grade is 102.5 feet. The elevations shown hereon are based on NGVD 29 Datum.

L. Scott Britt PLS #5757

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001530

DATE 01/22	2/2008 PARCEL I	D# 24-4S-16-03114-148	
APPLICANT	LINDA RODER	PHONE 1	752-2281
ADDRESS 3	87 SW KEMP COURT	LAKE CITY	FL 32024
OWNER MA	RTIN HOME BUILDERS	PHONE 3	97-4534
ADDRESS 148	8 SW GERALD CONNER DR	LAKE CITY	FL 32024
CONTRACTOR	BEN MARTIN	PHONE 3	397-4534
LOCATION OF	PROPERTY 47S, TR ON 242, TR	ON ARROWHEAD, TL ON GERAL	D CONNER DRIVE,
2ND LOT ON RIGH	-TT		7
SUBDIVISION/	LOT/BLOCK/PHASE/UNIT CA	NNON CREEK PLACE	48
SIGNATURE	Lithe Jolly		
	INSTALLATION REQUIREM	<u>IENTS</u>	
X	Culvert size will be 18 inches in did driving surface. Both ends will be thick reinforced concrete slab.	ameter with a total lenght of 32 mitered 4 foot with a 4 : 1 slope	feet, leaving 24 feet of and poured with a 4 inch
		existing driveway turnouts are p be paved or formed with concr paved a minimum of 12 feet wi hichever is greater. The width s	ete. de or the width of the
	Culvert installation shall conform	to the approved site plan stand	dards.
	Department of Transportation Per	mit installation approved stand	lards.
	Other		
	FETY REQUIREMENTS SHOULD BE	FOLLOWED	AND DOOR

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



NOTICE OF COMMENCEMENT	Inst:2008120050:22 Date:3/12/2008 Time:4:07 PMDC,P.DeWitt Cason,Columbia County Page 1 of 1
Tax Parcel Identification Number 24-45-16-6311	4-148 County Clerk's Office Stamp or Seal
THE UNDERSIGNED hereby gives notice that improvements will be made Florida Statutes. the following information is provided in this NOTICE OF	COMMENCEMENT
1. Description of property (legal description): 148 5w Ge a) Street (job) Address:	dueiling
2. General description of improvements: Single Family	duelling
3 Owner Information	
a) Name and address: II artin to me. B	uilders POB1831 Calle CtyFL32056
c) Interest in property Speculation house	
4. Contractor Information	
a) Name and address: Ben Martin of Ma b) Telephone No: 397-4534	Min Home Builder
a) Name and address: // A	
b) Amount of Bond:	
o) receptione rio	Fax No. (Opt.)
6 Lender a) Name and address: A	
b) Phone No.	
7. Identity of person within the State of Florida designated by owner upon wh	om notices or other documents may be served:
a) Name and address:	
b) Telephone No.:	Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a c	ony of the Lienard Nation of the Lienard Nat
Florida Statutes:	opy of the Elenor's Notice as provided in Section 713.13(1)(b).
a) Name and address:	
a) Name and address:	Fax No. (Opt.)
 Expiration date of Notice of Commencement (the expiration date is one your is specified): 	ear from the date of recording unless a different date
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER	AFTER THE EXPIRATION OF THE NOTICE OF
COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS IN	DER CHAPTED 713 PART I SECTION 712 12 FLORIDA
STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMP COMMENCEMENT MUST BE RECORDED AND POSTED ON THE J	OR SITE REPORT THE TRACE OF
TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTOR	RNEY REFORE COMMENCING WORK OF RECORDING
YOUR NOTICE OF COMMENCEMENT.	THE BEFORE COMMENCING WORK OR RECORDING
STATE OF FLORIDA	~ 1
COUNTY OF COLUMBIA LINGA R. ROGET 10 X	in () last
Commission #DD303275 Expires: Mar 24, 2008	e of Owner or Owner's Authorized Office/Director/Partner/Manager
Softons Bonded Thru	en Martin
Atlantic Bonding Co., Inc. Print Nar	me
he foregoing instrument was acknowledged before me, a Florida Notary, this	11 day of March 20 08 by
Linda Roda as Notary	
	(type of authority, e.g. officer, trustee, attorney
ect) for	(name of party on behalf of whom instrument was executed).
ersonally Known OP Produced Identification	

Notary Stamp or Seal-



Project Information for:

L264912

Builder:

Martin Home Builders, Inc.

Lot:

48

Subdivision:

Cannon Creek

County:

Columbia

Truss Count:

Design Program: MiTek 20/20 6.3 Building Code: FBC2004/TPI2002

Truss Design Load Information: Gravity:

Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

Bennett G. Martin Florida License No. CBC059077

Address: P.O. Box 1831 Lake City, Florida 32056

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

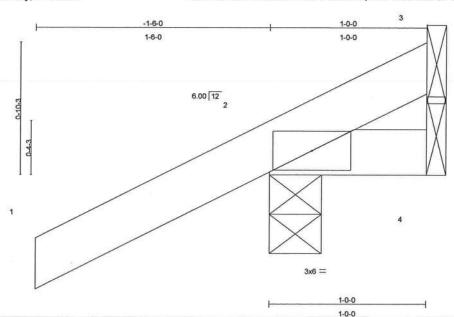
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1923388	CJ1	1/9/08
2	J1923389	CJ3	1/9/08
3	J1923390	CJ5	1/9/08
4	J1923391	EJ7	1/9/08
5	J1923392	HJ6	1/9/08
6	J1923393	HJ9	1/9/08
7	J1923394	T01	1/9/08
8	J1923395	T02	1/9/08
9	J1923396	T03	1/9/08
10	J1923397	T04	1/9/08
11	J1923398	T05	1/9/08
12	J1923399	T06	1/9/08
13	J1923400	T07	1/9/08
14	J1923401	T08	1/9/08
15	J1923402	T09	1/9/08
16	J1923403	T10	1/9/08
17	J1923404	T11	1/9/08
18	J1923405	T12	1/9/08
19	J1923406	T13	1/9/08
20	J1923407	T14	1/9/08
21	J1923408	T15	1/9/08
22	J1923409	T16	1/9/08
23	J1923410	T17	1/9/08
24	J1923411	T18	1/9/08



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	Experience of the control of the con
L264912	CJ1	JACK	12	1		J1923388
LEGIGIE	1001	UNOIX	12		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:43 2008 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240	10000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 6 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=180/0-4-0, 4=5/Mechanical, 3=-41/Mechanical

Max Horz 2=70(load case 6)

Max Uplift 2=-193(load case 6), 4=-9(load case 4), 3=-41(load case 1)

Max Grav 2=180(load case 1), 4=14(load case 2), 3=62(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/35, 2-3=-45/35

BOT CHORD

2-4=0/0

JOINT STRESS INDEX

2 = 0.10

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 Ib uplift at joint 2, 9 lb uplift at joint 4 and 41 lb uplift at joint 3. Continued on page 2

January 9,2008

Scale = 1:6.9

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



1	Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	Very and a second
	L264912	CJ1	JACK	12	1		J1923388
1	220 10 12		or tert	1.2		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:44 2008 Page 2

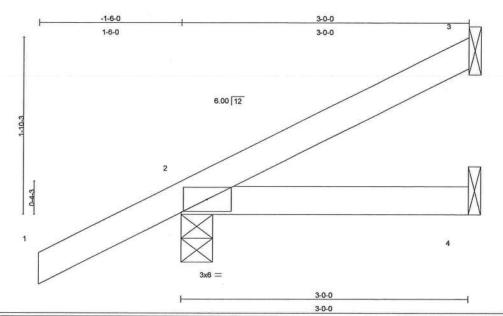
LOAD CASE(S) Standard

January 9,2008



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	CJ3	JACK	12	1		J1923389
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:44 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240	03490.0099-000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=206/0-4-0, 4=14/Mechanical

Max Horz 2=115(load case 6)

Max Uplift 3=-37(load case 6), 2=-187(load case 6), 4=-26(load case 4) Max Grav 3=48(load case 1), 2=206(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-49/16

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.10

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3, 187 lb uplift at joint 2 and 26 lb uplift at joint 4. Continued on page 2

January 9,2008

Scale = 1:11.4

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	ex verse series so i
L264912	CJ3	JACK	12	1		J1923389
1) LEO 10 12	000	JAN ST	12		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:44 2008 Page 2

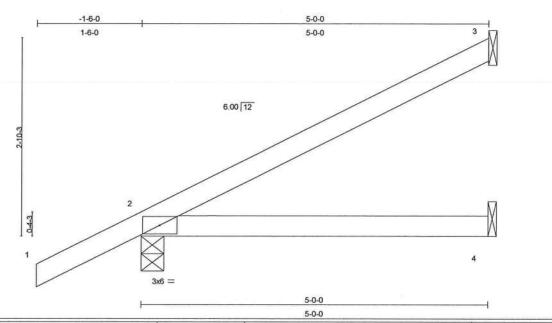
LOAD CASE(S) Standard

January 9,2008



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912 ·	CJ5	JACK	10	1		J1923390
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 16:38:14 2008 Page 1



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Matr	rix)						Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=113/Mechanical, 2=258/0-4-0, 4=24/Mechanical

Max Horz 2=162(load case 6)

Max Uplift 3=-101(load case 6), 2=-159(load case 6)

Max Grav 3=113(load case 1), 2=258(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-96/41

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 159 lb uplift at joint 2.

LOAD CASE(S) Standard

January 9,2008

Scale = 1:15.6

MiTek connectors.
bracing, is the
delivery, erection
CA Center,

Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	EJ7	MONO TRUSS	21	1		J1923391
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 16:38:05 2008 Page 1

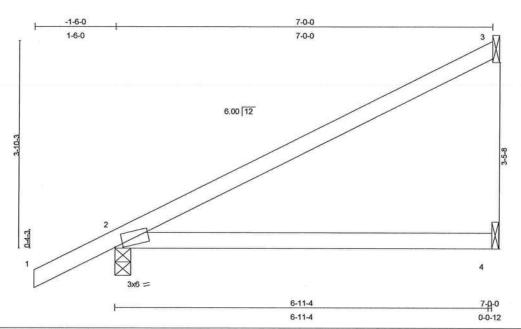


Plate Of	fsets (X,Y):	[2:0-1-13,0-0-7]										
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.43	Vert(LL)	0.11	2-4	>743	360	MT20	244/190
TCDL	7.0	1	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.17	2-4	>483	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mati	rix)						Weight: 25 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=158/Mechanical, 2=317/0-3-8, 4=49/Mechanical

Max Horz 2=149(load case 6)

Max Uplift 3=-88(load case 6), 2=-111(load case 6)

Max Grav 3=158(load case 1), 2=317(load case 1), 4=95(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-122/56

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.87

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

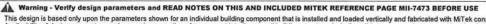
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

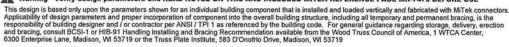
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift & joint 3 and 111 lb uplift at joint 2.

LOAD CASE(S) Standard

January 9,2008

Scale = 1:20.2







Job	Truss	Truss T	уре	Qty	Ply	MA	RTIN - CA	ANNON CF	REEK LOT 48	14000000
L264912	HJ6	JACK		1	1	777 679	Reference	e (optional		J1923392
Builders FirstSc	ource, Lake City, FI 3	32055	6.30	00 s Feb 15 200	6 MiTek I					Page 1
1	-2-1-7	-1			6-10-12					
I	2-1-7				6-10-12				_	Scale 15
				5					//	
			4.2	4 12				/		
2-9-2										
2					/					
		2								
0-3-14										-
14	///					_				
1										4
		3x6 =								
		Ĭ.			6-10-12					——
					6-10-12					
OADING (psf		2-0-0	CSI	DEFL		(loc)	I/defl	L/d	PLATES	GRIP
CLL 20.0	Lumber Incre	ase 1.25	TC 0.43 BC 0.32	Vert(LL) Vert(TL)	0.12 -0.16	2-4 2-4	>644 >491	360 240	MT20	244/1
3CLL 10.0	* Rep Stress Ir	nor NO	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC20	004/TPI2002	(Matrix)						Weight: 25 II	0
				DDAGING						
UMBER	2 X 4 SYP No.2			BRACING TOP CHO					directly applie	

BOT CHORD

REACTIONS (lb/size) 3=184/Mechanical, 2=281/0-6-7, 4=38/Mechanical

Max Horz 2=146(load case 3)

Max Uplift 3=-146(load case 3), 2=-277(load case 3), 4=-60(load case 3)

Max Grav 3=184(load case 1), 2=281(load case 1), 4=103(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-68/44

BOT CHORD 2 X 4 SYP No.2

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.11

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 3, 277 lb uplift at joint 2 and 60 lb uplift at joint 4.

Truss Design Engineer Florida FE No. 34869 1100 Caastal Bay Blyd Boynton Besch, FL 2242

6-10-12 oc purlins.

bracing.

Rigid ceiling directly applied or 10-0-0 oc

Continued on page 2

January 9,2008





Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912 ¹	HJ6	JACK	1	1		J1923392
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:45 2008 Page 2

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

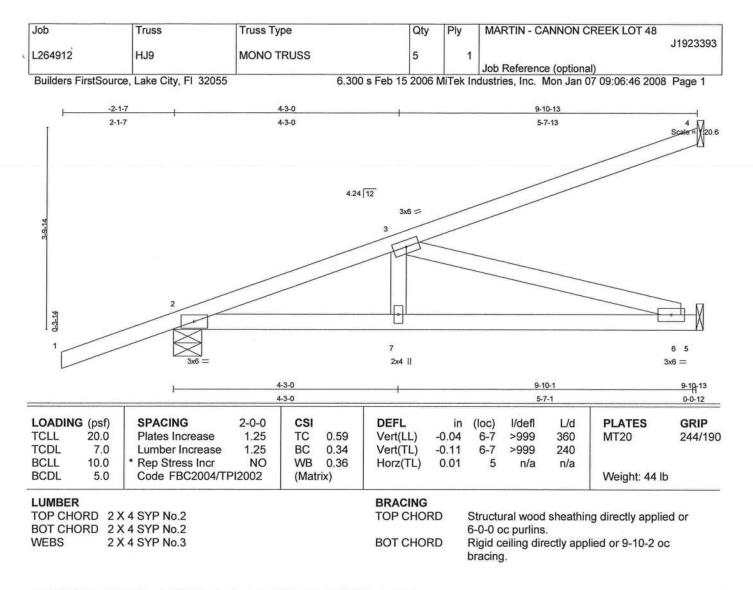
Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-3=-93(F=-20, B=-20), 2=0(F=5, B=5)-to-4=-17(F=-4, B=-4)

January 9,2008





REACTIONS (lb/size) 4=265/Mechanical, 2=409/0-6-7, 5=230/Mechanical

Max Horz 2=253(load case 3)

Max Uplift 4=-229(load case 3), 2=-231(load case 3), 5=-76(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/37, 2-3=-690/174, 3-4=-104/64

BOT CHORD

2-7=-365/643, 6-7=-365/643, 5-6=0/0

WEBS

3-7=0/189, 3-6=-669/379

JOINT STRESS INDEX

2 = 0.67, 3 = 0.17, 6 = 0.20 and 7 = 0.13

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 4, 231 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back

Continued on page 2

Truss Clesian Engineer Florida PE No. 34899 1100 Cessial Bay Blvd Boynton Beach, FL 33436

January 9,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	НЈ9	MONO TRUSS	5	1		J1923393
LEGIGIE	1.00	more moss	0		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:46 2008 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

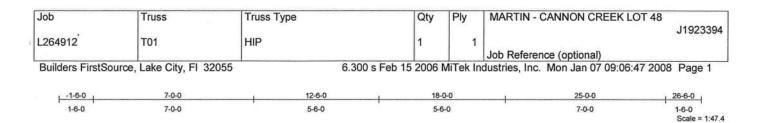
Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee Truse Design Engineer Florida FE No. 24869 1109 Coestel Bay Blvd Boydon Besch, Ft. 22426

January 9,2008





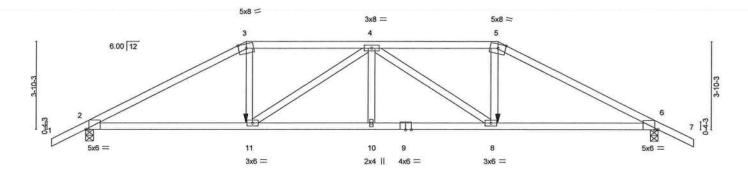


Plate Of	ffsets (X,Y): [2:0-1-11,Edge], [6	6:0-1-11,E	dge]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.17	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.33	10-11	>889	240	10000000000	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.13	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 115 lb	

5-6-0

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		3-2-11 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-1-11 oc bracing.

REACTIONS (lb/size) 2=1703/0-4-0, 6=1703/0-4-0

7-0-0

Max Horz 2=-68(load case 6)

Max Uplift 2=-544(load case 5), 6=-544(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-3171/993, 3-4=-2785/924

1-2=0/35, 2-3=-3171/993, 3-4=-2785/924, 4-5=-2785/924, 5-6=-3171/993, 6-7=0/35

5-6-0

BOT CHORD 2-11=-857/2747, 10-11=-1058/3357, 9-10=-1058/3357, 8-9=-1058/3357,

WEBS 3-11=-270/947, 4-11=-802/308, 4-10=0/204, 4-8=-802/308, 5-8=-269/947

JOINT STRESS INDEX

2 = 0.77, 3 = 0.66, 4 = 0.56, 5 = 0.66, 6 = 0.77, 8 = 0.60, 9 = 0.96, 10 = 0.33 and 11 = 0.60

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

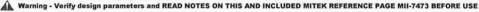
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

EbAtthrearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee Truse Design Engineer Florida PE No. 34899 1 100 Geastel Bay Blyd Boynton Beach, FL 93439

7-0-0

January 9,2008



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Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T01	HIP	1	1	J19233	394
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:47 2008 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 544 lb uplift at joint 2 and 544 lb uplift at joint 6.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-11=-10, 8-11=-22(F=-12), 6-8=-10

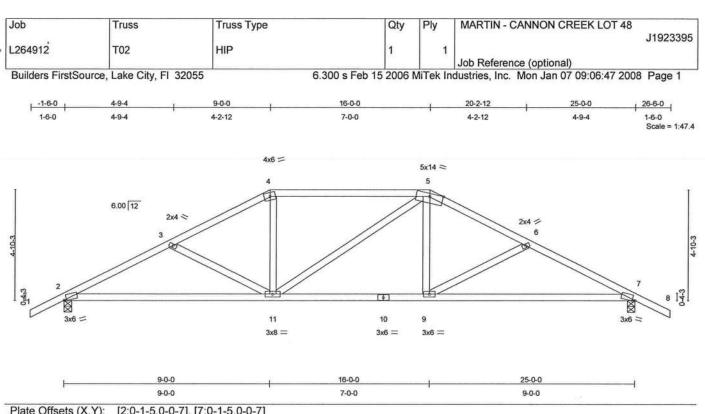
Concentrated Loads (lb)

Vert: 11=-411(F) 8=-411(F)

Julius Lee Truss Cesign Engineer Florida PE No. 34869 1109 Coastel Bay Blyri Bayeron Basel Bay Blyri

January 9,2008





LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.15	7-9	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.28	7-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 121 lb	

2 X 4 SYP No.2
2 X 4 SYP No.2
2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

5-2-5 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-6-13 oc

bracing.

REACTIONS (lb/size) 2=879/0-4-0, 7=879/0-4-0

Max Horz 2=80(load case 6)

Max Uplift 2=-226(load case 6), 7=-226(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1405/764, 3-4=-1176/659, 4-5=-1020/647, 5-6=-1176/659,

6-7=-1405/764, 7-8=0/35

BOT CHORD 2-11=-542/1195, 10-11=-358/1020, 9-10=-358/1020, 7-9=-542/1195

WEBS 3-11=-205/209, 4-11=-48/278, 5-11=-119/119, 5-9=-47/278, 6-9=-205/208

JOINT STRESS INDEX

2 = 0.81, 3 = 0.33, 4 = 0.72, 5 = 0.90, 6 = 0.33, 7 = 0.82, 9 = 0.34, 10 = 0.34 and 11 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Les Truss Design Engineer Plorida PE No. 34866 1100 Coastal Bay Blvd Boynton Besch, FL 3343

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive leads a page 2

January 9,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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	Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
	L264912	T02	HIP	1	1		J1923395
•		1.02				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:47 2008 Page 2

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

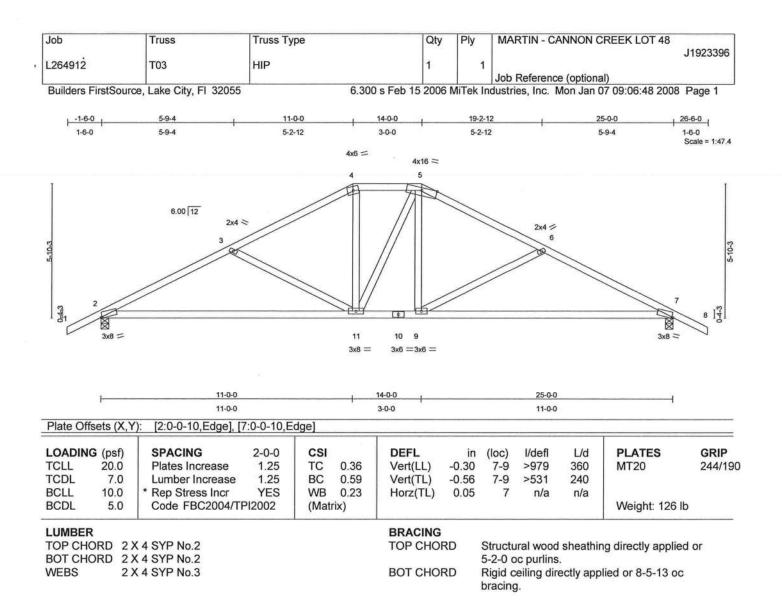
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 2 and 226 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lew Trues Cesign Engineer Florida PE No. 24865 1109 Cesstel Bay Blyd Bounda Base F. 1944

January 9,2008





REACTIONS (lb/size) 2=879/0-4-0, 7=879/0-4-0

Max Horz 2=92(load case 6)

Max Uplift 2=-238(load case 6), 7=-238(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1371/779, 3-4=-1052/614, 4-5=-883/608, 5-6=-1051/614,

6-7=-1371/779, 7-8=0/35

BOT CHORD 2-11=-548/1176, 10-11=-268/882, 9-10=-268/882, 7-9=-548/1177

WEBS 3-11=-339/317, 4-11=-104/272, 5-11=-131/135, 5-9=-104/272, 6-9=-340/318

JOINT STRESS INDEX

2 = 0.86, 3 = 0.33, 4 = 0.47, 5 = 0.58, 6 = 0.33, 7 = 0.87, 9 = 0.34, 10 = 0.82 and 11 = 0.61

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other CollyRuleadon page 2

Julius Lee Truss Design Engineer Florida PE No. 34865 1108 Ceastal Bay Blvd Boynton Beach, FL 33435

January 9,2008

🛦 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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	Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
ě.	L264912	T03	HIP	1	1		J1923396
		10.55-5	1.10	1000		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:48 2008 Page 2

NOTES

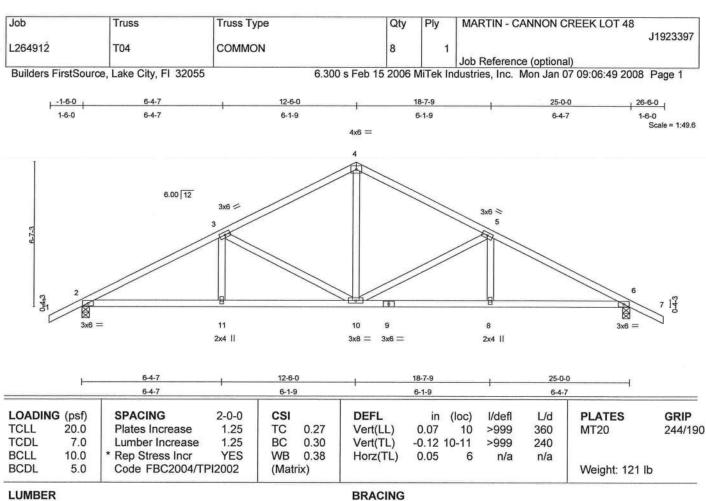
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 2 and 238 lb uplift at joint 7.

LOAD CASE(S) Standard

and Design Engineer PE No. 34868 Peach Edy Elvid In Beach, FL 20406





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or

5-1-1 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-8-11 oc

bracing.

REACTIONS (lb/size) 2=879/0-4-0, 6=879/0-4-0

Max Horz 2=101(load case 6)

Max Uplift 2=-245(load case 6), 6=-245(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1400/753, 3-4=-961/603, 4-5=-961/603, 5-6=-1400/753, 6-7=0/35

BOT CHORD

2-11=-518/1178, 10-11=-518/1178, 9-10=-518/1178, 8-9=-518/1178, 6-8=-518/1178

WEBS 3-11=0/198, 3-10=-458/327, 4-10=-268/466, 5-10=-458/327, 5-8=0/198

JOINT STRESS INDEX

2 = 0.61, 3 = 0.39, 4 = 0.68, 5 = 0.39, 6 = 0.61, 8 = 0.33, 9 = 0.39, 10 = 0.56 and 11 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 Ib uplift at joint 2 and 245 lb uplift at joint 6. Continued on page 2

January 9,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation availed from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



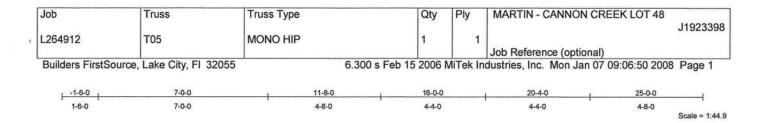
Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T04	COMMON	8	1		J1923397
					Job Reference (optional)	

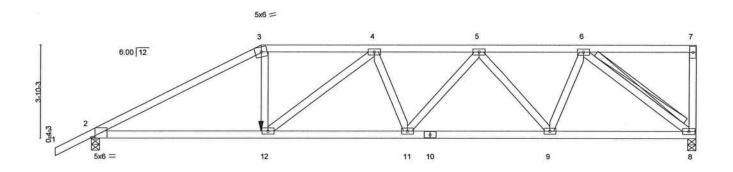
6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:49 2008 Page 2

LOAD CASE(S) Standard

us Las Se Cesign Engineer Se Cesign Engineer 20 Castal Bay Blvd Onton Beach, Ft. 99435







		7-0-0			6-0-9			5-11-0			6-0-7	
Plate Of	ffsets (X,Y): [2:0-1-11,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.15	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.30	11-12	>980	240		

13-0-9

LUMBER	2					BRACING						
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 128 lb	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.55	Horz(TL)	0.11	8	n/a	n/a		
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.30	11-12	>980	240		
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.15	11	>999	360	MT20	244/19
	- (PO.)	OI MONTO	_ 0 0				***	(100)	"acii		I LAILO	Oiti

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or 3-3-1 oc purlins, except end verticals.

25-0-0

BOT CHORD

Rigid ceiling directly applied or 6-0-7 oc

bracing.

18-11-9

WEBS

T-Brace: 2 X 4 SYP No.3 - 6-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=1761/0-4-0, 2=1663/0-4-0

7-0-0

Max Horz 2=150(load case 5)

Max Uplift 8=-607(load case 4), 2=-522(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-3079/992, 3-4=-2701/923, 4-5=-3112/1056, 5-6=-2236/751,

6-7=-62/15, 7-8=-253/127

BOT CHORD 2-12=-913/2663, 11-12=-1108/3169, 10-11=-1010/2882, 9-10=-1010/2882,

8-9=-665/1890

3-12=-275/894, 4-12=-597/289, 4-11=-159/143, 5-11=-73/361, 5-9=-1009/404,

6-9=-240/961, 6-8=-2326/827

Julius Læe Truss Design Engineer Flonde PE No. 24869 1109 Ceastal Ray Blvd Boynton Besch, FL 33436

JOINT STRESS INDEX

WEBS

2 = 0.75, 3 = 0.77, 4 = 0.47, 5 = 0.37, 6 = 0.83, 7 = 0.39, 8 = 0.71, 9 = 0.83, 10 = 0.92, 11 = 0.47 and 12 = 0.57

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T05	MONO HIP	1	1		J1923398
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:50 2008 Page 2

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 607 lb uplift at joint 8 and 522 lb uplift at joint 2.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

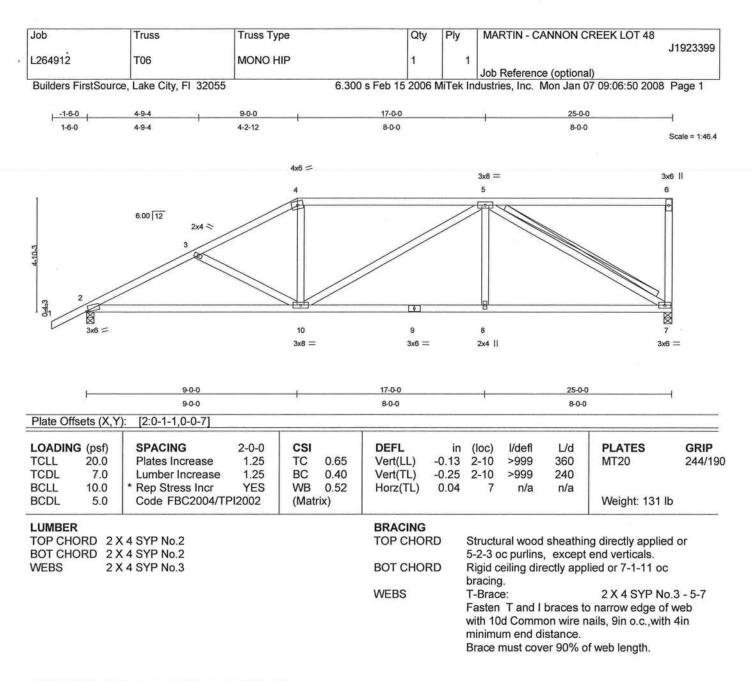
Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)

Concentrated Loads (lb)

Vert: 12=-411(F)

Julius Lee Truss Design Engineer Florida PE No. 34869 1 100 Coastal Bay Blvd.





REACTIONS (lb/size) 7=787/0-4-0, 2=883/0-4-0

Max Horz 2=182(load case 6)

Max Uplift 7=-213(load case 5), 2=-218(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1413/713, 3-4=-1190/613, 4-5=-1035/607, 5-6=-55/27, 6-7=-193/138

BOT CHORD 2-10=-786/1201, 9-10=-556/1001, 8-9=-556/1001, 7-8=-556/1001

WEBS 3-10=-192/202, 4-10=-11/276, 5-10=-59/63, 5-8=0/235, 5-7=-1095/611

JOINT STRESS INDEX

2 = 0.82, 3 = 0.33, 4 = 0.79, 5 = 0.56, 6 = 0.49, 7 = 0.52, 8 = 0.33, 9 = 0.38 and 10 = 0.56

Julius Lee Truse Design Engineer Florida PE No. 34868 1100 Coestal Bay Blvd Boyston Basch E. 19446

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T06	MONO HIP	1	1		J1923399
L204912	100	INOINO THE			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:50 2008 Page 2

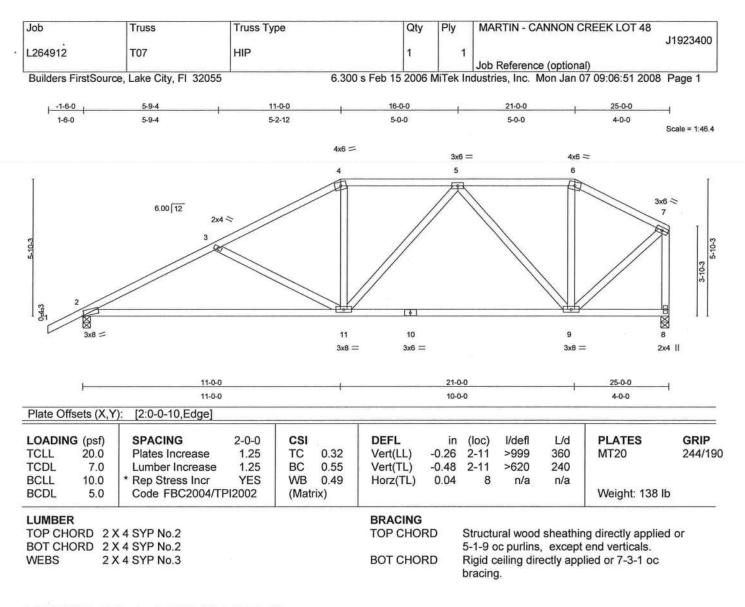
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 7 and 218 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Cesign Engineer Florida PE No. 248es 1189 Coastel Bay Blvd.





REACTIONS (lb/size) 2=883/0-4-0, 8=787/0-4-0

Max Horz 2=175(load case 6)

Max Uplift 2=-233(load case 6), 8=-132(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1392/741, 3-4=-1067/579, 4-5=-898/578, 5-6=-498/349,

6-7=-596/338, 7-8=-775/435

BOT CHORD 2-11=-755/1182, 10-11=-444/815, 9-10=-444/815, 8-9=-14/13

WEBS 3-11=-326/311, 4-11=-35/265, 5-11=-54/177, 5-9=-507/293, 6-9=-33/113,

7-9=-320/654

JOINT STRESS INDEX

2 = 0.79, 3 = 0.33, 4 = 0.58, 5 = 0.37, 6 = 0.43, 7 = 0.49, 8 = 0.33, 9 = 0.62, 10 = 0.28 and 11 = 0.56

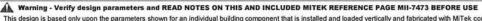
NOTES

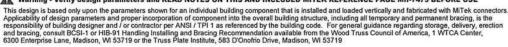
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

Continued on page 2







	Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
۰	L264912	Т07	HIP	1	1	-	J1923400
						Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:51 2008 Page 2

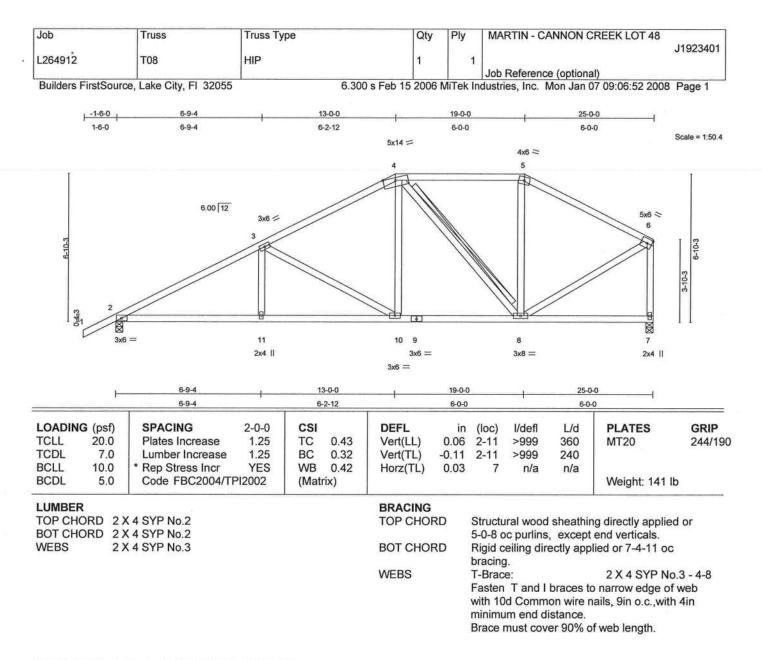
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 132 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Truse Cesign Engineer Flonda PE No. 24889 1199 Coastel Bay Blyd





REACTIONS (lb/size) 2=883/0-4-0, 7=787/0-4-0

Max Horz 2=187(load case 6)

Max Uplift 2=-242(load case 6), 7=-131(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1395/711, 3-4=-929/548, 4-5=-571/439, 5-6=-706/417, 6-7=-753/456

BOT CHORD 2-11=-719/1170, 10-11=-719/1170, 9-10=-408/767, 8-9=-408/767, 7-8=-33/40

WEBS 3-11=0/213, 3-10=-465/355, 4-10=-134/331, 4-8=-348/178, 5-8=-123/113,

6-8=-301/621

JOINT STRESS INDEX

2 = 0.64, 3 = 0.39, 4 = 0.85, 5 = 0.71, 6 = 0.74, 7 = 0.65, 8 = 0.57, 9 = 0.25, 10 = 0.34 and 11 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Truse Design Engineer Florida PE No. 34888 1100 Coastal Bay Blvd Boynton Beach, FL 93435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T08	HIP	1	1	J19.	23401
2201012			(85)		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:52 2008 Page 2

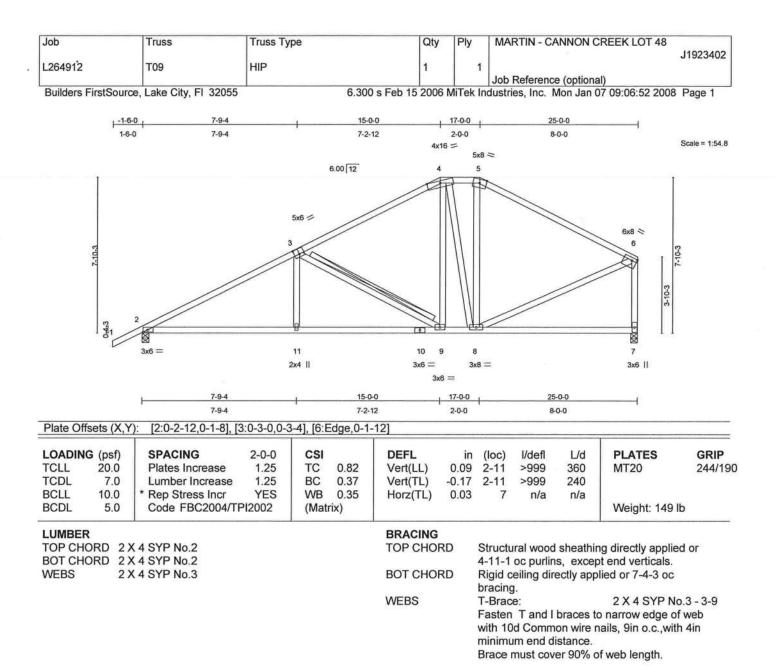
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 242 lb uplift at joint 2 and 131 lb uplift at joint 7.

LOAD CASE(S) Standard

ee Jesian Endineer Ple No. 23ses Pestel Bay Blvd. n Besich, FL 22425





REACTIONS (lb/size) 2=883/0-4-0, 7=787/0-4-0

Max Horz 2=198(load case 6)

Max Uplift 2=-249(load case 6), 7=-160(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1363/704, 3-4=-801/502, 4-5=-597/501, 5-6=-762/462, 6-7=-741/470

BOT CHORD 2-11=-699/1134, 10-11=-700/1132, 9-10=-700/1132, 8-9=-323/636, 7-8=-64/83

WEBS 3-11=0/254, 3-9=-572/433, 4-9=-191/302, 4-8=-298/106, 5-8=-180/188,

6-8=-261/565

JOINT STRESS INDEX

2 = 0.74, 3 = 0.74, 4 = 0.96, 5 = 0.66, 6 = 0.74, 7 = 0.37, 8 = 0.58, 9 = 0.34, 10 = 0.38 and 11 = 0.33 line Legel Control of the Parish English Control of the Parish Control of

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	Т09	HIP	1	1		J1923402
	1		ļė.		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:53 2008 Page 2

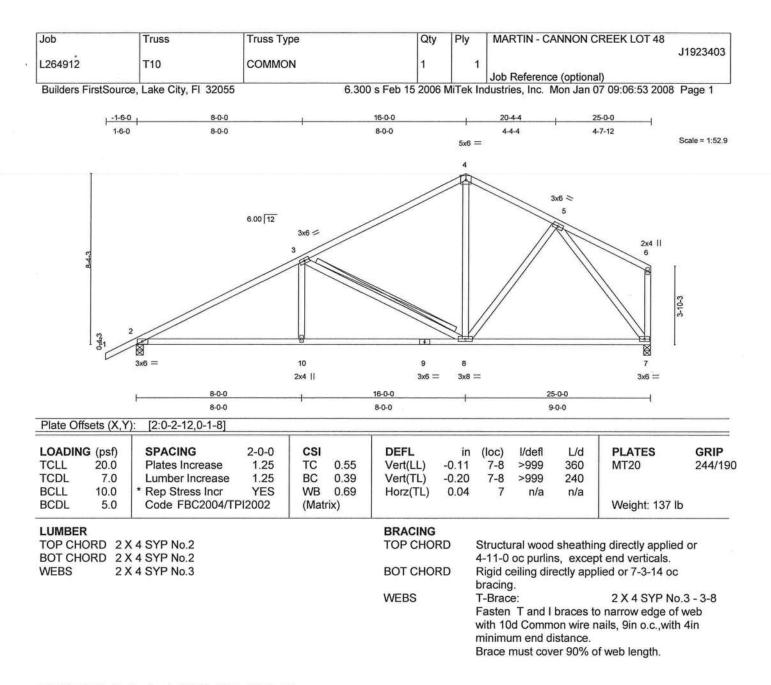
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 249 lb uplift at joint 2 and 160 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flenda PE No. 34869 I 109 Ceastal Bay Blvd.





REACTIONS (lb/size) 2=883/0-4-0, 7=787/0-4-0

Max Horz 2=204(load case 6)

Max Uplift 2=-252(load case 6), 7=-151(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1359/712, 3-4=-760/480, 4-5=-695/502, 5-6=-99/84, 6-7=-144/126

BOT CHORD 2-10=-706/1131, 9-10=-706/1131, 8-9=-706/1131, 7-8=-281/492

WEBS 3-10=0/250, 3-8=-615/469, 4-8=-146/320, 5-8=-33/224, 5-7=-750/446

JOINT STRESS INDEX

2 = 0.72, 3 = 0.39, 4 = 0.60, 5 = 0.36, 6 = 0.75, 7 = 0.68, 8 = 0.56, 9 = 0.36 and 10 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

Trues Design Engineer Florida FE No. 24868 1 100 Coastal Bay Blvd Boynton Beach, FL 32425

> Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T10	COMMON	1	1		J1923403
	1	- Common			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:53 2008 Page 2

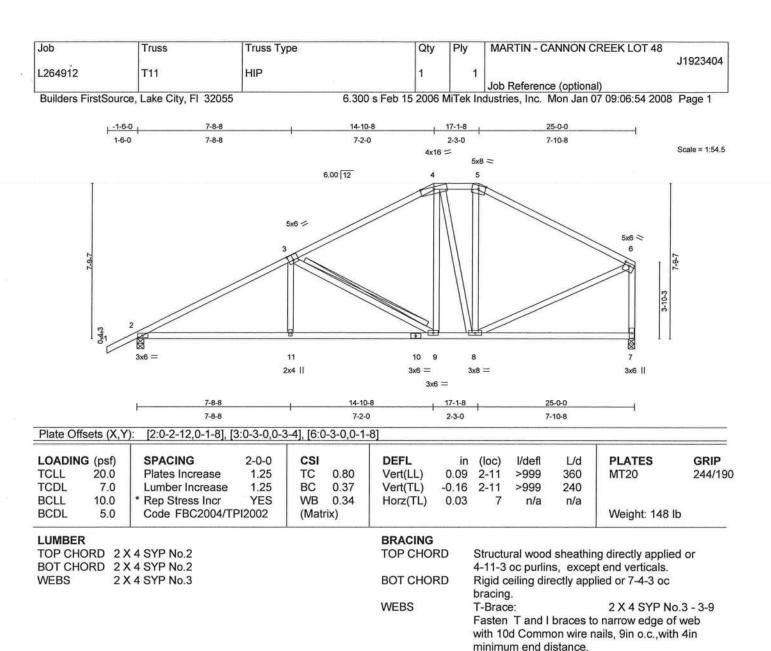
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 252 lb uplift at joint 2 and 151 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Law Truse Design Engineer Florida PE No. 34846 1109 Coestal Bay Blvd Boynton Beach, Ft. 33435





REACTIONS (lb/size) 2=883/0-4-0, 7=787/0-4-0

Max Horz 2=198(load case 6)

Max Uplift 2=-249(load case 6), 7=-161(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/35, 2-3=-1365/705, 3-4=-809/505, 4-5=-596/497, 5-6=-759/459, 6-7=-741/469

BOT CHORD 2-11=-701/1137, 10-11=-702/1135, 9-10=-702/1135, 8-9=-328/643, 7-8=-62/81

WEBS 3-11=0/253, 3-9=-567/430, 4-9=-187/303, 4-8=-296/105, 5-8=-164/179,

6-8=-262/566

JOINT STRESS INDEX

2 = 0.73, 3 = 0.73, 4 = 0.93, 5 = 0.62, 6 = 0.81, 7 = 0.36, 8 = 0.59, 9 = 0.34, 10 = 0.38 and 11 = 0.33

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 9,2008

Brace must cover 90% of web length.

Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	No. of Co. of Co
L264912	T11	HIP	1	1		J1923404
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:54 2008 Page 2

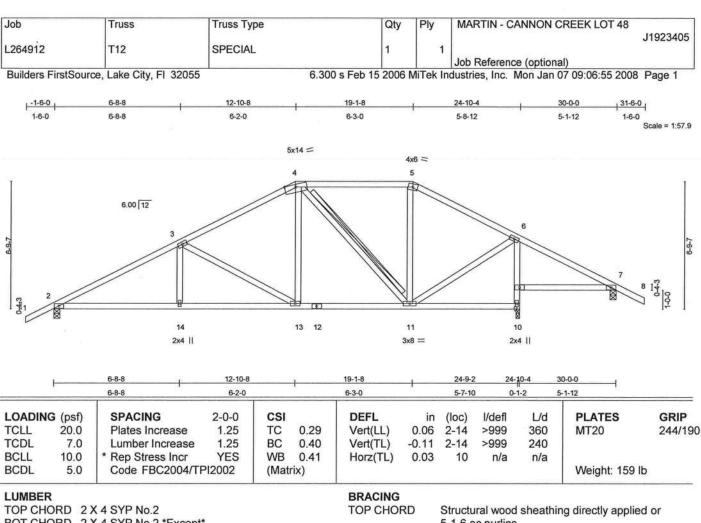
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 249 lb uplift at joint 2 and 161 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonds PE No. 24869 1100 Ceastal Bay Blvd Boynton Besch, FL 33431





BOT CHORD 2 X 4 SYP No.2 *Except*

6-10 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

5-1-6 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-7-12 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -4-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=865/0-4-0, 10=1008/0-2-4, 7=206/0-4-0

Max Horz 2=123(load case 6)

Max Uplift 2=-240(load case 6), 10=-232(load case 6), 7=-181(load case 7) Max Grav 2=865(load case 1), 10=1008(load case 1), 7=227(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1358/719, 3-4=-899/559, 4-5=-516/444, 5-6=-645/430, 6-7=-44/127,

7-8=0/35

BOT CHORD 2-14=-535/1137, 13-14=-535/1137, 12-13=-231/742, 11-12=-231/742,

10-11=-46/115, 9-10=-982/685, 6-9=-943/569, 7-9=-52/111

WEBS 3-14=0/209, 3-13=-456/349, 4-13=-128/333, 4-11=-370/181, 5-11=-128/100,

6-11=-265/669

JOINT STRESS INDEX

2 = 0.63, 3 = 0.39, 4 = 0.89, 5 = 0.62, 6 = 0.43, 7 = 0.28, 9 = 0.29, 10 = 0.58, 11 = 0.62, 12 = 0.24, 13 = 0.34 and 14 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T12	SPECIAL	-	1		J1923405
L204312	112	OI LOIAL	12.		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:55 2008 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 10.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2, 232 lb uplift at joint 10 and 181 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Law Truse Design Engineer Flonda PE No. 34868 1 109 Caastal Ray Blyd Boynton Beson, FL 99431



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48
		Constitution to Revision			J1923406
L264912	T13	SPECIAL	1	1	
					Job Reference (optional)
Builders FirstS	ource, Lake City, FI	32055 6.3	300 s Feb 15 2006	MiTek In	dustries, Inc. Mon Jan 07 09:06:56 2008 Page 1



5x6 =

4x12 = 4 4x6 = 6 6.00 12 2x4 \\ 15 14 13 12 11 3x8 = 2x4 ||

	H	7-4-0		14-1-8		21-1-8				11	30-0-0	
		7-4-0		6-9-8 T		7-0-0			3-7-10 0	-1-2	5-1-12	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.07	2-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.14	2-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 165 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except* 7-11 2 X 4 SYP No.3 **WEBS**

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-1-14 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-5-12 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -5-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=863/0-4-0, 11=1021/0-2-4, 8=195/0-4-0

Max Horz 2=121(load case 6)

Max Uplift 2=-240(load case 6), 11=-269(load case 7), 8=-191(load case 7) Max Grav 2=863(load case 1), 11=1021(load case 1), 8=216(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1353/733, 3-4=-1262/823, 4-5=-985/700, 5-6=-425/380,

6-7=-507/375, 7-8=-18/152, 8-9=0/35

BOT CHORD 2-15=-551/1136, 14-15=-256/747, 13-14=-320/853, 12-13=-320/853, 11-12=-96/123

, 10-11=-1016/662, 7-10=-972/541, 8-10=-86/95

WEBS 3-15=-311/313, 4-15=-341/496, 4-14=-258/505, 5-14=-421/292, 5-12=-548/289,

6-12=-138/64, 7-12=-308/733

JOINT STRESS INDEX

2 = 0.59, 3 = 0.33, 4 = 0.37, 5 = 0.96, 6 = 0.65, 7 = 0.72, 8 = 0.25, 10 = 0.34, 11 = 0.44, 12 = 0.70, 13 = 0.34, 14 = 0.36 and 15 = 0.50January 9,2008 Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T13	SPECIAL	1	1		J1923406
	2.52	31. = 1.1. =			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:56 2008 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2, 269 lb uplift at joint 11 and 191 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Honda PE No. 24865 1109 Coastal Bay Blvd



Job L264912	Truss T14	Truss Type SPECIAL		Qty 1	Ply 1	MARTIN - CANNO		OT 48 J1923407
Builders FirstSou	rce, Lake City, FI 3	2055	6.300 s Fel	b 15 2006	MiTek In	dustries, Inc. Mon.	Jan 07 09:06	:57 2008 Page 1
-1-6-0	6-6-4	12-6-0	16-1-8		23-1-8	24-10-4	30-0-0	31-6-0
1-6-0	6-6-4	5-11-12	3-7-8		7-0-0	1-8-12	5-1-12	1-6-0 Scale = 1:58.9
I			4					
044-3 044-3 044-3	6.00 12	2x4 \\		14 13 bx8 =		5x14 = 6 7 7 12 11 2x4 II 24-10-4		8 9 14 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 0.36 244/190 Plates Increase 1.25 TC Vert(LL) -0.10 2-15 >999 360 MT20 TCDL 7.0 Lumber Increase 1.25 BC 0.43 Vert(TL) -0.192-15 >999 240 BCLL 10.0 0.51 Rep Stress Incr YES WB Horz(TL) 0.03 11 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 160 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

7-11 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-2-3 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-3-9 oc

bracing.

REACTIONS (lb/size) 2=860/0-4-0, 11=1040/0-2-4, 8=179/0-4-0

Max Horz 2=121(load case 6)

Max Uplift 2=-240(load case 6), 11=-270(load case 7), 8=-190(load case 7) Max Grav 2=860(load case 1), 11=1040(load case 1), 8=205(load case 11)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1342/741, 3-4=-1169/747, 4-5=-1115/776, 5-6=-930/627,

6-7=-244/249, 7-8=-15/184, 8-9=0/35

BOT CHORD 2-15=-558/1128, 14-15=-258/736, 13-14=-22/215, 12-13=-22/215, 11-12=-124/135,

10-11=-1066/672, 7-10=-1021/550, 8-10=-121/108

WEBS 3-15=-312/307, 4-15=-264/438, 4-14=-294/485, 5-14=-764/547, 6-14=-452/855,

6-12=-670/351, 7-12=-375/824

JOINT STRESS INDEX

2 = 0.61, 3 = 0.33, 4 = 0.49, 5 = 0.79, 6 = 0.96, 7 = 0.72, 8 = 0.24, 10 = 0.34, 11 = 0.51, 12 = 0.71, 13 = 0.15, 14 = 0.80 and 15 = 0.43

NOTES

1) Unbalanced roof live loads have been considered for this design.

llus Las 198 Design Endineer 2nda PE No. 24868 86 Casstal Bay Blvd 2ynton Besch, FL 26436

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T14	SPECIAL	1	1		J1923407
	1.670(530)		123//		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:57 2008 Page 2

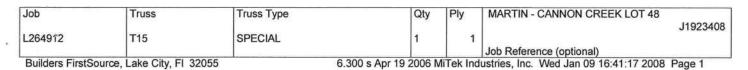
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2, 270 lb uplift at joint 11 and 190 lb uplift at joint 8.

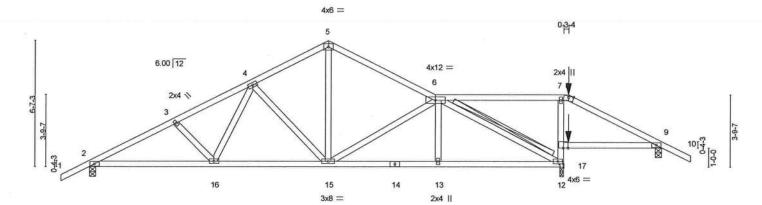
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24868 1 100 Coasial Bay Slvd









	-	6-6-4	12-6-0		+	18-1-8	1	24-9-	2	24-10-4	30-0-0	
		6-6-4	5-11-1	2		5-7-8		6-7-1	0	0-1-2	5-1-12	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.05	15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.11	15-16	>999	240	TANGS STORES	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	0.0000000000000000000000000000000000000					Weight: 156 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

7-12 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 5-3-13 oc purlins.

Rigid ceiling directly applied or 5-9-13 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 6-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=851/0-4-0, 9=160/0-4-0, 12=1379/0-2-4

Max Horz 2=121(load case 5)

Max Uplift 2=-237(load case 5), 9=-180(load case 6), 12=-488(load case 6) Max Grav 2=851(load case 1), 9=189(load case 10), 12=1379(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1374/274, 3-4=-1217/260, 4-5=-856/224, 5-6=-885/209, 6-7=-26/123,

7-8=-27/139, 8-9=-55/214, 9-10=0/35

BOT CHORD 2-16=-280/1169, 15-16=-189/951, 14-15=-144/985, 13-14=-144/985, 12-13=-142/988,

11-12=-760/391, 7-11=-607/265, 11-17=-139/74, 9-17=-139/74

WEBS 3-16=-186/109, 4-16=-40/256, 4-15=-350/146, 5-15=-93/459, 6-15=-341/154, 6-13=0/189/11111 Constitution of the constraint of the const

6-12=-1257/249

JOINT STRESS INDEX

2 = 0.61, 3 = 0.34, 4 = 0.41, 5 = 0.32, 6 = 0.96, 7 = 0.41, 8 = 0.32, 9 = 0.23, 11 = 0.31, 12 = 0.50, 13 = 0.34, 14 = 0.35, 15 = 0.57 and 16 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

January 9,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T15	SPECIAL	1	1		J1923408
	147.7458			8	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 16:41:17 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 12.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 2, 180 lb uplift at joint 9 and 488 lb uplift at joint 12.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

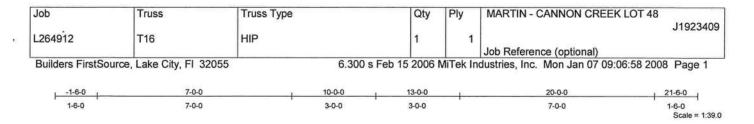
Vert: 1-5=-54, 5-6=-54, 6-7=-54, 7-8=-89(F=-35), 8-10=-54, 2-12=-10, 11-17=-16(F=-6), 9-17=-10

Concentrated Loads (lb)

Vert: 8=-184(F) 17=-109(F)

Julius Lee Truss Design Engineer Plonda PE No. 34868 1 109 Coastal Bay Blvd





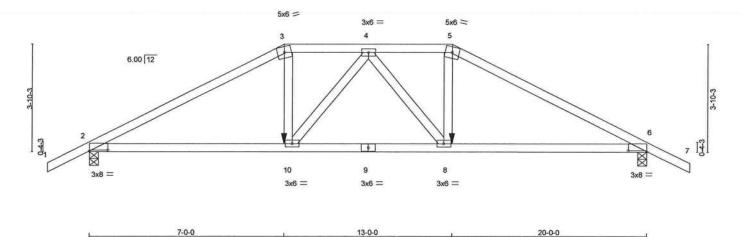


Plate Of	fsets (X,Y	<u>(): [2:0-8-0,0-0-6], [6:</u>	0-8-0,0-0-	6]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.08	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.18	8-10	>999	240	SWEJANE:	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 90 lb	

6-0-0

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WERS	2 X 4 SYP No 3

BRACING TOP CHORD

Structural wood sheathing directly applied or

3-8-13 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 7-9-8 oc

7-0-0

bracing.

REACTIONS (lb/size) 2=1354/0-4-0, 6=1354/0-4-0

Max Horz 2=68(load case 5)

7-0-0

Max Uplift 2=-451(load case 5), 6=-451(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-2404/717, 3-4=-2089/681, 4-5=-2089/681, 5-6=-2404/717, 6-7=0/35

BOT CHORD 2-10=-609/2062, 9-10=-659/2176, 8-9=-659/2176, 6-8=-572/2062

WEBS 3-10=-217/720, 4-10=-271/154, 4-8=-271/154, 5-8=-217/720

JOINT STRESS INDEX

2 = 0.73, 3 = 0.65, 4 = 0.38, 5 = 0.65, 6 = 0.73, 8 = 0.46, 9 = 0.76 and 10 = 0.46

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 9,2008



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Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T16	HIP	1	1		J1923409
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:58 2008 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 451 lb uplift at joint 2 and 451 lb uplift at joint 6.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

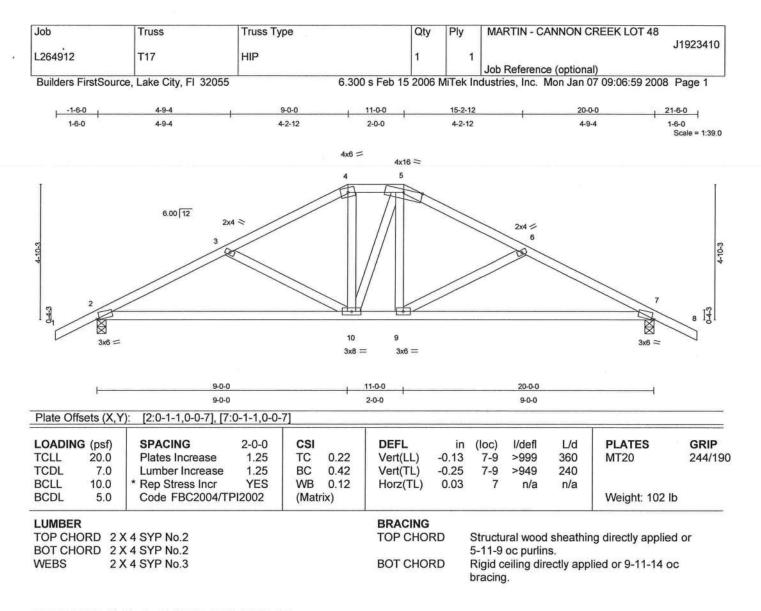
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-10=-10, 8-10=-22(F=-12), 6-8=-10

Concentrated Loads (lb)

Vert: 10=-411(F) 8=-411(F)

Julius Lee Truse Design Engineer Plonda PE No. 24869 1 100 Coastal Bay Blvd





REACTIONS (lb/size) 2=719/0-4-0, 7=719/0-4-0

Max Horz 2=80(load case 6)

Max Uplift 2=-206(load case 6), 7=-206(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1078/596, 3-4=-815/470, 4-5=-683/470, 5-6=-814/470,

6-7=-1078/596, 7-8=0/35

BOT CHORD 2-10=-396/909, 9-10=-181/682, 7-9=-396/909

WEBS 3-10=-264/245, 4-10=-77/211, 5-10=-106/111, 5-9=-77/212, 6-9=-265/245

JOINT STRESS INDEX

2 = 0.88, 3 = 0.33, 4 = 0.34, 5 = 0.45, 6 = 0.33, 7 = 0.89, 9 = 0.34 and 10 = 0.64

NOTES

1) Unbalanced roof live loads have been considered for this design.

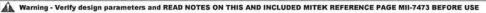
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Collygue of page 2

Julius Les Truse Cesign Engineer Flonda PE No. 34868 1 199 Ceastal Bay Blvd Boynton Beach, Ft. 33435

January 9,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	gapt become control of
L264912	T17	HIP	1	1		J1923410
2201012				**	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:06:59 2008 Page 2

NOTES

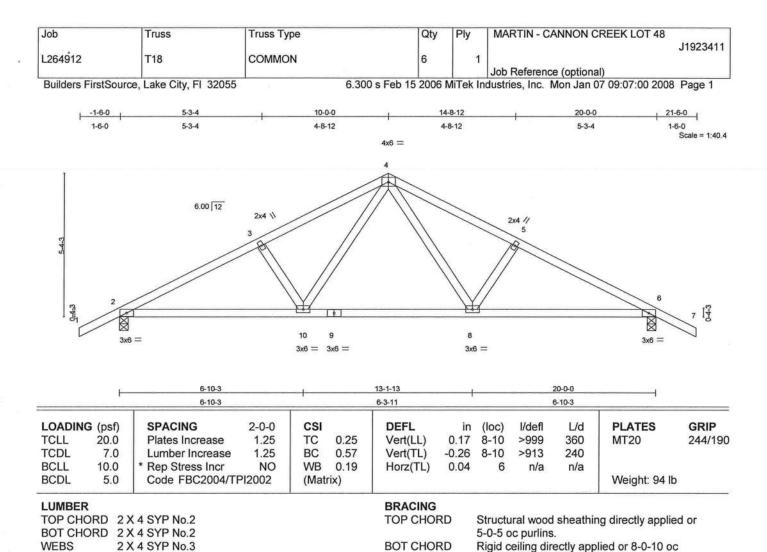
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 2 and 206 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Les Trues Design Engineer Florida PE No. 24899 1169 Ceastal Bay Blvd Boynton Beach, Ft. 23425





bracing.

REACTIONS (lb/size) 2=908/0-4-0, 6=908/0-4-0

Max Horz 2=-86(load case 7)

Max Uplift 2=-263(load case 6), 6=-263(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-1516/831, 3-4=-1372/831, 4-5=-1372/831, 5-6=-1516/831, 6-7=0/35

BOT CHORD 2-10=-597/1285, 9-10=-321/893, 8-9=-321/893, 6-8=-597/1285

3-10=-218/213, 4-10=-308/551, 4-8=-308/551, 5-8=-218/213 **WEBS**

JOINT STRESS INDEX

2 = 0.65, 3 = 0.33, 4 = 0.48, 5 = 0.33, 6 = 0.65, 8 = 0.41, 9 = 0.40 and 10 = 0.41

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp. B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 263 lb uplift at joint 2 and 263 lb uplift at joint 6. Continued on page 2

January 9,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MARTIN - CANNON CREEK LOT 48	
L264912	T18	COMMON	6			J1923411
1204912	110	COMMON	ľ	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jan 07 09:07:00 2008 Page 2

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

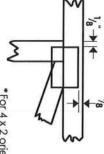


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise securely seat. plates to both sides of truss and Dimensions are in inches. Apply



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates

PLATE SIZE

4 × 4

dimension is the length paralle to slots. perpendicular to slots. Second The first dimension is the width

LATERAL BRACING



continuous lateral bracing. Indicates location of required

NER

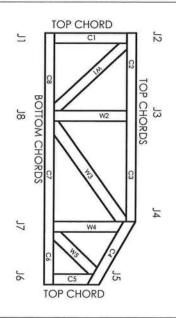
561

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO 3907, 4922

BOCA

SBCCI

9667, 9432A

WISC/DILHR 960022-W, 970036-N



MiTek Engineering Reference Sheet: MII-7473

Damage or Personal Injury Failure to Follow Could Cause Property General Safety Notes

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 12 Cut members to bear tightly against each other
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- 4 at 1/4 panel length (± 6" from adjacent joint.) Unless otherwise noted, locate chord splices
- S Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 7. 6 Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber. is the responsibility of truss fabricator. General Camber is a non-structural consideration and
- œ Plate type, size and location dimensions shown indicate minimum plating requirements

practice is to camber for dead load deflection

- in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Bottom chords require lateral bracing at 10 unless otherwise noted ft. spacing, or less, if no ceiling is installed,
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- 13. Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling erection and installation of trusses.

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DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WIEN DIAGONAL BRACE IS USED. CONNECT DIACONAL BRACE FOR 8404 AT EACH YED. MAY WEB TOTAL LENGTH IS 14*. GABLE VERTICAL LENGTH MAX SPACING SPECIES 16" 24" .C. O.C. O.C. 0 GABLE VERTICAL SPF SPF SPF DFL DFL DFI SP HH ASCE STANDARD GRADE STANDARD STANDARD STANDARD STANDARD STANDARD STUD \$3 STUD BRACE 7-02: GABLE TRUSS BRACES 130 2X4 SP #2N, DT-L #2, SPF #/#2, OR SETTER DIAGONAL BRACE: SINGLE OR DOUBLE CUT (AS SHOWN) AT UPPER END. GROUP A Ξ MPH IX4 "L" BRACE . GROUP B 7 5 6 6 7 7 5 6 6 6 4 5 5 6 6 6 WIND (1) 2X4 "L" BRACE . GROUP A SPEED GROUP B 15 THE PRINCES 18 (2) 2X4 "L" BRACE ** GROUP A 0 0 0 0 0 0 0 6 MEAN EX4 FEN OR BETTER GROUP B HEIGHT, (1) 2X6 "L" BRACE * GROUP A GROUP B 13 ENCLOSED, 12' 4" 10' 7" 13' 5" 13' 5" 12' 6" 12' 6" 13 13 14 3 8 8 0 0 12 (2) GROUP A 12 2 2 2 2 BXS "L" BRACE GROUP II 14, 0° 14.0 14' 0" 14' 0" 14. 0" 2 2 14. 0 1.00, W ATTACH EACH 'L' BRACE WITH 104 NAILS. # FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C. # FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. ## FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. CARLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLAT CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. 'L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. DOUGLAS FIR-LARCH #3 STUD STANDARD SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD PLYWOOD OVERHANG. BRACING EXPOSURE VERTICAL LENGTH LESS THAN 4: 0" BUT LESS THAN 1: 0" BUT LESS THAN 1: 0" GREATER THAN 1: 0" CABLE TRUSS SOUTHERN PINE GABLE VERTICAL PLATE SIZES GROUP SPECIES GROUP B: HEM-PIR GROUP DETAIL NOTES: C DOUGLAS FIR-LARCH SOUTHERN PINE #3 STUD A: AZ STANDARD NO SPLICE AND GRADES:

VERTICAL LENGTH IN TABLE ABOVE.

NAOHS

CONNECT DIAGONAL AT

BEW

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

ULIUS LEE'S cons. Engineers P.A.

DELRAY BEACH, FL. 33444-2161

DRWG DATE REF

NITEX SID GABLE 15 E HT

11/26/03

ASCE7-02-GAB13015

No: 34869 STATE OF FLORIDA

MAX.

SPACING

24.0

MAX.

TOT.

F

60

PSF

CONTINUOUS BEARING

0

0

4

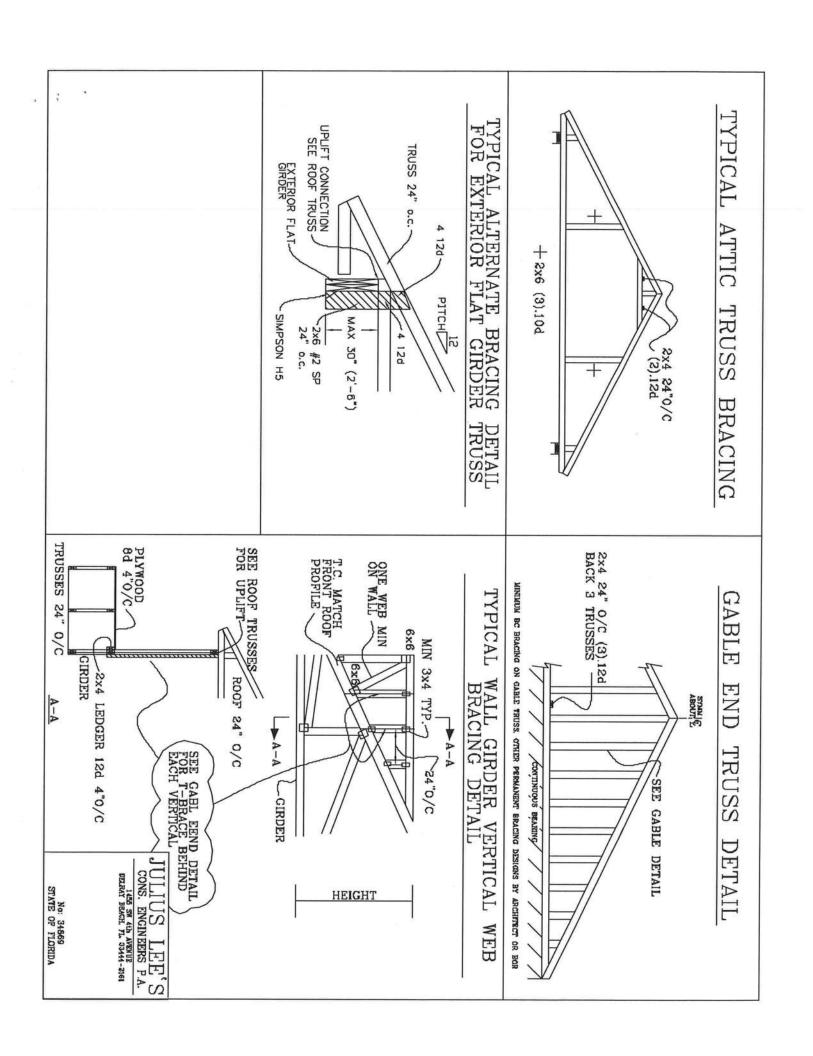
REFER TO COMMON TRUBS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

2.5X4

22

MARCHOS TRUSSES REQUIRE EXTRENE CARE IN FABRICATINE, MANCLING, SHIPPING, INSTALLING AND BRACING. REFER IO 3153 1-03 (BUILING COPPIDENT SAFETY INFORMATION, PUBLISED BY TIPI (TRUSS PLATE INSTITUTE, 383 INFORMETIO BR, SUITE 200, MONISON, VI. 323759 AND VITA (VODO TRUSS CILICATE) OF MERICA, 6300 ENTERPRISE UN, MADISIN, VI 32719) FOR SAFETY PRACTICES PRIZE TO PERFORMING THESE TAKTINISK, UNICES OFFERVISE (BUILLING) CORPORATE ALVI PROPERA, Y ATTACHED STRUCTURAL PARELS AND IDITION CHORD SMALL HAVE A PROPERA, Y ATTACHED RIGID CELLING.

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 9803 AT EACH EVID. MAX WEB **GABLE** VERTICAL LENGTH TOTAL LENGTH IS 14". MAX VERTICAL LENGTH SHOWN IN TABLE ABOVE. SPACING SPECIES 16" 24" O.C. O.C. O.C. CONNECT DIAGONAL AT GABLE VERTICAL SPF DFI SPF DFL SPF DFL H ASCE STANDARD STANDARD STANDARD STANDARD STANDARD GRADE STANDARD BEAM BRACE 7-02: SENDACINE. RECERT TO BEST 1-03 GUILLING COMPORET SAFETY INTOCHING, MADILING, SHIPPING, INSTALLING AND BRACING. RECERT TO BEST 1-03 GUILLING COMPORET SAFETY (RICHARIDSA), PUBLISHED BY FPI CIRRUSS PLATE INSTITUIC, 393 DENORTO EX. SUITE 200, MANISON, VI. 53799 AND VICA. KADDI TIRES CLONGLING OF AMERICA, 6300 ENTERPRISE IM, MINISON, VI. 53799 IN BACTITEES PRIET TO PERCORNING THESE TWICTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CEILING. GABLE TRUSS BRACES 130 GROUP A (1) 1X4 °L" BRACE • ZX4 SP OR DIT-L #2 OR BETTER DIAGONAL BRACE, SINGLE OR DOUBLE CUT (AS SHOWN) AT UPPER END MPH GROUP B WIND (1) 2X4 "L" BRACE . GROUP A GROUP B SPEED REFER TO 30 ABOUT E 18 (2) 2X4 "L" BRACE ** (1) 2X6 "L" BRACE * GROUP A GROUP B 9' 10" MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH CONTINUOUS BEARING EX4 #EN OR BETTER 9. 10. 9. 10. HEIGHT, **(** C CONS. GROUP A GROUP B 12 11 DELRAY BEACH, PL 33444-2161 18, 10 No: 34869 STATE OF FLORIDA IUS LEI Ð ENCLOSED, 13 13 11 12 13 12 PET C (2) 2XB GROUP A 12' 3' 10' 10" 14 0 S r. WAX. MAX. BRACE GROUP B Ш 10' 10" 14' 0" 14' D" 14 0 14' D" 4 TOT. SPACING 1.00, ED. ATTACH EACH 'L' BRACE WITH 104 NAILS. # FOR (1) 'L' BRACE: SPACE NAILS AT 8' O.C. # FOR (2) 'L' BRACES: AND 4" O.C. BETWEEN ZONES. ## FUR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CHITERIA IS L/240. 'L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. DOUGLAS FIR-LARCH #3 STUD STANDARD PLYWOOD OVERHANG. BRACING GROUP SPECIES SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD EXPOSURE CABLE TRUSS SOUTHERN PINE 60 GREATER THAN 4' D', BUT LESS THAN 11' B' GREATER THAN 11' 6" LESS THAN 4 0 24.0 PEAK, SPLICE, AND HEEL PLATES. CABLE VERTICAL PLATE SIZES PSF DATE REF DWG MITEK STD GABLE 90' E HT HEM-PIR GROUP B: GROUP DETAIL NOTES: 0 DOUGLAS FIR-LARCH 11/26/03 SOUTHERN PINE #3 STUD ASCB7-02-GAB13030 A: AZ STUD ND SPLICE AND STANDARD 2.5X4 20 2 224 GRADES:



TOP CHORD BOT CHORD WEBS 2X4 2X4 ### ### #### BETTER BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX. TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

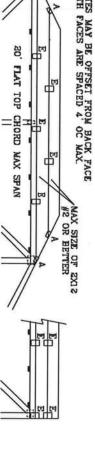
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

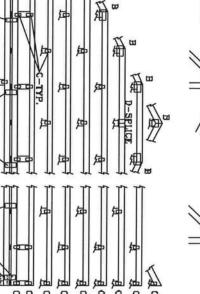
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT 1, EXP C, WIND TO DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, FBG
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND HC DL=6 PSF

FRONT FACE (5,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. MAX SIZE OF ZXIZ





TYP. B

EITHBR PLATE LOCATION IS ACCEPTABLE

NAX L

JOINT	80'	34.	34' 38'	62,
>	2X4	2.5X4	2.6X4	
В	4X8	5X6	5X6	
C	1.5X3	1.5 X 4	1.5X4	1.5X4
ם	5 X 4	6X6	5X5	
E	4X6 01	4X8 OR 3X6 TRULOX AT 4' OC,	VERTICAL	5.1

ATTACH TRUK EQUAL, PER F BE CONNECTE INFORMATION. 3°

MEMBER. AT	O' TO 7'9" NO BRACING O' TO 7'9" NO BRACE. SAME GRADE. SPI	WEB LENGTH REQUIRED BRACING	WEB BRACING CHART
DE 21 4 00.	SPECIES AS WEB	ING	

ATTACH TEETH TO THE PIGCYBACK AT THE TIME OF PABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120° X 1.375° NAILS PER FACE PER PLY. APPLY PICCYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.		100000	0
A TEETH TO THE PIGGYBACK AT THE TIME I ANTION. ATTACH TO SUPPORTING TRUSS WIT 20" X 1.375" NAILS PER FACE PER PLY. A BACK SPECIAL PLATE TO EACH TRUSS FACE 4" OC OR LESS.		٥	0
ATTACH TO THE PIGGYBACK AT THE TIME I ANTION. ATTACH TO SUPPORTING TRUSS WIT 20" X 1375" NAILS PER FACE PER PLY. A BACK SPECIAL PLATE TO EACH TRUSS FACE 4" OC OR LESS.		0	٥
H TEETH TO THE PIGGYBACK AT THE TIME I ANTION. ATTACH TO SUPPORTING TRUSS WIT 20" X 1375" NAILS PER FACE PER PLY. A BACK SPECIAL PLATE TO EACH TRUSS FACE 4' OC OR LESS.			
4 TEETH TO THE PIGCYBACK AT THE TIME. ANTION. ATTACH TO SUPPORTING TRUSS WIT 20" X 1.375" NALLS PER FACE PER PLY. A BACK SPECIAL PLATE TO EACH TRUSS FACE			
TACH TEETH TO THE PIGGYBACK AT THE TIME BRICATION. ATTACH TO SUPPORTING TRUSS WID 0.120" X 1.375" NAILS PER FACE PER PLY	PECIAL PLATE		FACE
	ATTACH TO SU	RTING PORTING	JSS WI

W

	SPACING 24.0"	No: 34868 STATE OF FLORIDA		î
	47 PSF AT 1.15 DUR. FAC.			٠
-ENG JL	50 PSF AT 1.25 DUR. FAC.	DZLRAY BEACH, FL. 33444-2161	STRECTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CEILING.	
DATE 09/12/07 DRWGMITEK STD PIGG	55 PSF AT 1.33 DUR. FAC.	CONS. ENGINEERS P.A.	ASACINA INSERT MUSICAL ELABORET ELA RECE, EMPE MARGINETA, SMULTINI, SMIPPINI, INSTALLING AND MARCINET MENTALLING AND MARCINET INSERTINITE. 283 O'DOGTROUSE, SMULTE, SMULT, SMULTE, SMULT, SMULTE, SMULTE, SMULTE, SMULTE, SMULTE, SMULTE, SMULT, SMULTE, SMULT, SMULTE, SMULTE, SMULT, SMULTE, SMULT, SMUL	
REF PIGGYBACK	MAX LOADING	S, HHI SIIIIII		
534,016 634	THIS DRAWING REPLACES DRAWINGS 634.016 834,017 & 847,045	THIS DRAW	*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALFINE PIGGYBACK SPECIAL PLATE.	

VALLEYTRUSS DETAIL

TOP CHORD BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
2X4 SP #3 OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: FHC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (CASCE 7-02 130 MPH WIND, 15' MEAN HEIGHT, ENCLI BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR OR (3) 15d ENCLOSED FOR

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9". UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

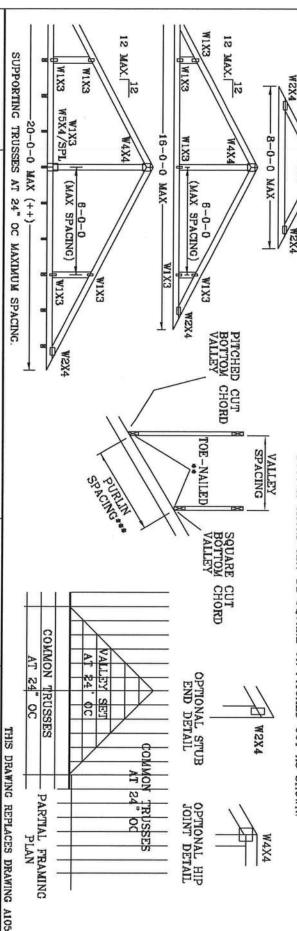
4-0-0 MAX

12 NAX.

W2X4

++ LARGER SPANS NAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



MEMORINGAM TRUSSES REQUIRE EXTREME FAME IN FABRICATING, MADILING, SKIPPING, INSTALLING AND BACING. REFER TO BEST 1-3D SOULING EDPONENT SAFETY INFORMATION, MULLICED BY TPI (TRUSS FLATE INSTITUTE, 500 DOORTOOD DE, SUITE 200, MUNISON, V. 153799 AND VICEA COOD TRUSS COUNCIL FAMERICA, GAID CATERRAISE N., MADISON, V. 153799 FOR SAFETY PARCITEES PRIDE TO PERFORMO THESE FINCTIONS UNLESS OFFICAMENTS OF SAFETY PARCITEES PRIDE TO PERFORMO THE STRUCTURAL PARCIT AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STOULD STRUCTURAL PARCIS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STOULD CELLING.

JULIUS LEE'S

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DELRAY BEACH, I'L 33444-2161

BC DL TC DL TC

U

PSF DRWG PSF DATE PSF REF

VALTRUSS1103 11/26/03 VALLEY DETAIL

BC LL

0

-ENG

TOT. LD.

32

PSF PSF

No: 34869 STATE OF FLORIDA

SPACING

24 1.25 40 0

DUR.FAC. 1.25

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

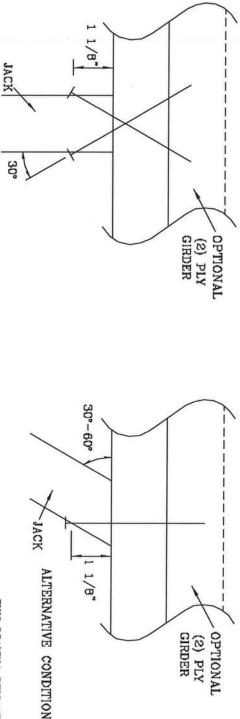
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXINUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL VALUE	5	4	အ	ผ	TOE-NAILS	NUMBER OF
IS WAY BE	493#	394#	296#	197#	1 PLY	SOUTHE
II Idullin	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE
DAY ADD	452#	361#	271#	181#	1 PLY	DOUGLAS
RODRIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
NOTEVALIA	390#	312#	234#	156#	1 PLY	
ALL VALUES MAY BE MILLTIPLIED BY APPROPRIATE DIRECTION OF LOAD EACTION	507#	406#	304#	203#	2 PLIES	HEM-FIR
ACTION D	384#	307#	230#	154#	1 PLY	SPRUCE
	496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR

イスしつじむ NI CIN MOLLIFLIED DI AFFROFRIAIE DUNAIION C. LOAD FACTOR.



SIE
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WING
REPI
LACES
O2
DRA
WING
784040
0

			STRUCTURAL PARIES AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING	, 383 YONGFRIG DR. SUITE 200, MADISON, W. 33719) AND VYCA (WOOD TRUS BRITERPRISE LM. MADISON, VI 33719) FOR SAFETY PRACTICES PRIDE TO PER BRITERPRISE LMS (WINTENTED TO COMPANY PRACTICES A 41745	HHVARMINGHH TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPINIG, INSTALLING AND BRACING. RETER TO BEST 1-03 CIVILLING COMPONENT SAFETY (BETOWNTIDE), PUBLISHED BY TPI (TRUSS	
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	S, HELL SULLUL
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF REF
			-ENG JL	PSF DRWG	DATE	REF
			JL	CNTONAIL1103	09/12/07	TOE-NAIL

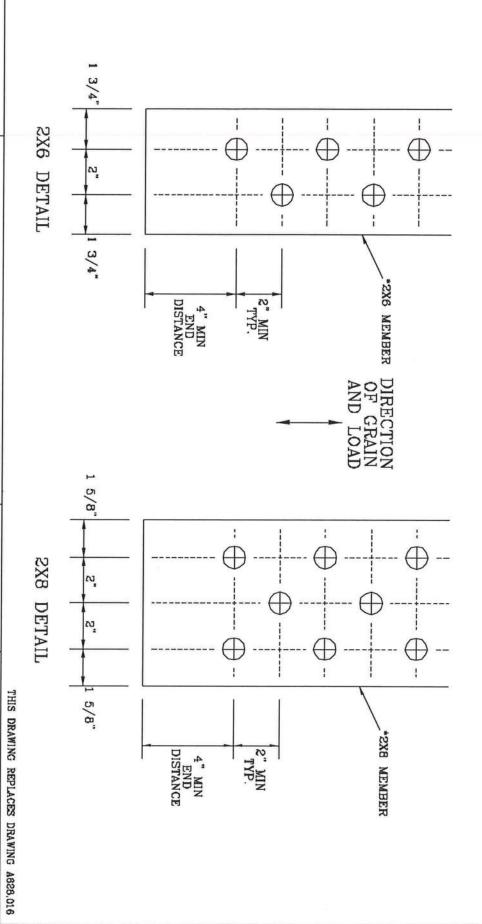
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW. APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



JULIUS LEE'S cons. ENGINEERS P.A. DELRAY BEACH, FL 33444-2161

1

BC DL TC DL TC

PSF PSF PSF

DRWG -ENG

> CNBOLTSP1103 11/26/03 BOLT SPACING

DATE REF

PSF PSF

BC LL

No: 34869 STATE OF FLORIDA

SPACING

DUR. FAC TOT. LD.

TRULOX CONNECTION DETAII

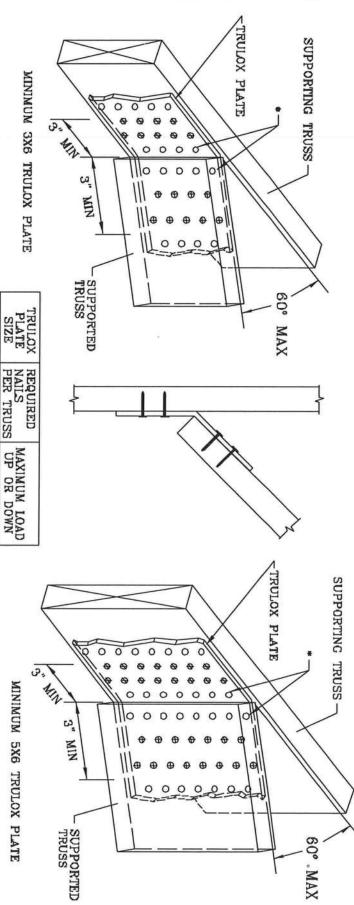
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (4).

NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

INFORMATION NOT SHOWN. THIS DETAIL FOR LUMBER, PLATES, AND OTHER REFER TO ENGINEER'S SEALED DESIGN REFERENCING



MINIMUM 3X6 TRULOX PLATE

3X6

MAXIMUM LOAD
UP OR DOWN

MINIMUM 5X6 TRULOX PLATE

15 9

#066 350#

JULIUS LEE'S DELRAY BEACH, IL 38444-2161

LEE'S

THIS DRAWING REPLACES DRAWINGS 1.158,989 1.158,989/R 1,154,944 1.152,217 1,152,017 1.159,154 & 1,151,524

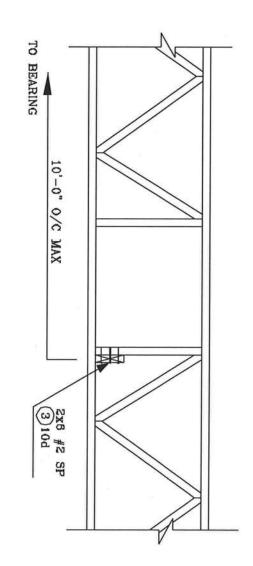
DATE REF

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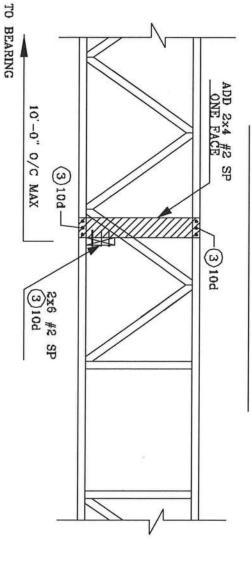
CNTRULOX1103 11/26/03 TRULOX

No: 34869 STATE OF FLORIDA

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S CONS. ENGINEERS P.A.

1455 SM 4th AMERIC
1555 SM 4th AMER

No: 34869 STATE OF FLORIDA

Residential System Sizing Calculation

Summary Project Title:

801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

Lake City, FL

Spec House

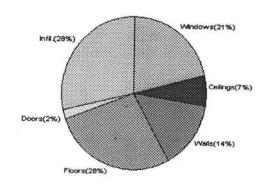
1/7/2008

				1/1/2000	
Location for weather data: Gaine	esville - De	faults: Lati	itude(29) Altitude(152 ft.) Temp Ran	ae(M)	
Humidity data: Interior RH (50%	6) Outdoo	r wet bulb ((77F) Humidity difference(54gr.)	3-()	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	100
Winter temperature difference	37	F	Summer temperature difference	17	
Total heating load calculation	25309	Btuh	Total cooling load calculation	21491	
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	
Total (Electric Heat Pump)	118.5	30000	Sensible (SHR = 0.75)		22500
Heat Pump + Auxiliary(0.0kW)	118.5	30000	Latent	165.1	7500
			Total (Electric Heat Pump)	139.6	30000

WINTER CALCULATIONS

Winter Heating Load (for 1400 soft)

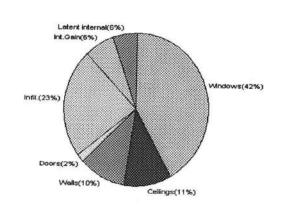
Load component			Load	
Window total	167	sqft	5385	Btuh
Wall total	1089	sqft	3575	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1400	sqft	1650	Btuh
Floor total	162	saft	7073	Btuh
Infiltration	175	cfm	7108	Btuh
Duct loss			0	Btuh
Subtotal		- 1	25309	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			25309	Btuh

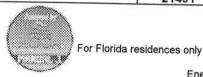


SUMMER CALCULATIONS

Summer Cooling Load (for 1400 sqft)

Load component			Load	
Window total	167	sqft	8966	Btuh
Wall total	1089	sqft	2190	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1400	sqft	2318	Btuh
Floor total			0	Btuh
Infiltration	91	cfm	1702	Btuh
Internal gain		1-27-1 MANUE	1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain		1055,614-1172-11	16948	Btuh
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			3343	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occ	Latent gain(internal/occupants/other)			Btuh
Total latent gain			1200 4543	Btuh
TOTAL HEAT GAIN			21491	Btuh





EnergyGauge® System Sizing PREPARED BY: 93

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House

Project Title: 801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/7/2008

This calculation is for Worst Case. The house has been rotated 315 degrees.

Compone	ant I aar	da far lil	bala L	
Counthous	sin Luai	12 IOI AA	HOIG L	ouse

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	30.0	32.2	Load 966 Btuh
2	2, Clear, Metal, 0.87	NW	9.0	32.2	290 Btuh
3	2, Clear, Metal, 0.87	NW	40.0	32.2	
4	2, Clear, Metal, 0.87	NW	15.0	32.2	1288 Btuh
5	2, Clear, Metal, 0.87	SE	40.0	32.2	483 Btuh 1288 Btuh
6	2, Clear, Metal, 0.87	SE	13.3	32.2	to the first of th
7	2, Clear, Metal, 0.87	sw	20.0	32.2	428 Btuh
	Window Total	OVV	20.0 167(sqft)		644 Btuh
Walls	Туре	R-Value	Area X	HTM=	5385 Btuh Load
1	Frame - Wood - Ext(0.09)	13.0	949	3.3	3116 Btuh
2	Frame - Wood - Adj(0.09)	13.0	140	3.3	
	Wall Total	10.0	1089	3.3	460 Btuh
Doors	Туре		Area X	HTM=	3575 Btuh
1	Insulated - Adjacent		20	12.9	Load 259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40	12.5	518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1400	1.2	1650 Btuh
	Ceiling Total		1400	1.2	1650 Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
· 1	Slab On Grade	0	162.0 ft(p)		7073 Btuh
	Floor Total	577.4	162	7 40.7	7073 Btuh
			102		7073 Bluit
		Z	one Envelope	Subtotal:	18201 Btuh
Infiltration	Туре	ACH X	Zone Volume	CFM=	
	Natural	0.94	11200	175.5	7108 Btuh
			11200	175.5	7 106 Blun
Ductload	, R6.0, Supply(), Return()		11	(DLM of 0.00)	0 Btuh
Zone #1		Sens	ible Zone Sub	ototal	25309 Btuh

	5 4 A A S A A A A A A A A A A A A A A A A	A 475, 4750, 4750, 475, 475	
UU 1-24-34	200 ±200 ± 7 00 1 2	ISE TO	ar. x xx « x
	ოქოდეგ გათავთ	200 1 x00000 20 00 20	Ør∴s xxx∞ xx

	Subtotal Sensible Ventilation Sensible Total Btuh Loss	25309 Btuh 0 Btuh 25309 Btuh
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Manual J Winter Calculations

Residential Load - Component Details (continued)
Project Title: Class

Spec House

Lake City, FL

801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear

(Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House

Project Title: 801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/7/2008

This calculation is for Worst Case. The house has been rotated 315 degrees.

Y**COCCOCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC	
TO A CONTROL OF THE PARTY OF TH	Loads for Zone #1: Main
	I DODE TOT ZONG WITE BESTIN
A 20 20 20 20 20 20 20 20 20 20 20 20 20	

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	30.0	32.2	966 Btuh
2	2, Clear, Metal, 0.87	NW	9.0	32.2	290 Btuh
3	2, Clear, Metal, 0.87	NW	40.0	32.2	1288 Btuh
4	2, Clear, Metal, 0.87	NW	15.0	32.2	483 Btuh
5	2, Clear, Metal, 0.87	SE	40.0	32.2	1288 Btuh
6	2, Clear, Metal, 0.87	SE	13.3	32.2	428 Btuh
7	2, Clear, Metal, 0.87	sw	20.0	32.2	644 Btuh
	Window Total	550 200100	167(sqft)	02.2	5385 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	949	3.3	3116 Btuh
2	Frame - Wood - Adj(0.09)	13.0	140	3.3	460 Btuh
	Wall Total		1089	-15	3575 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1400	1.2	1650 Btuh
	Ceiling Total		1400		1650Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	162.0 ft(p)	43.7	7073 Btuh
	Floor Total		162		7073 Btuh
		Zo	one Envelope S	ubtotal:	18201 Btuh
Infiltration	Туре		Zone Volume	CFM=	
	Natural	0.94	11200	175.5	7108 Btuh
Ductload	, R6.0, Supply(), Return()		Á	(DLM of 0.00)	0 Btuh
Zone #1		Sens	ible Zone Sub	total	25309 Btuh

	- A	9 W C
WHOLE HOUSE TO	asar≏	3893

Manual J Winter Calculations

Residential Load - Component Details (continued)
Project Title:

Spec House

Lake City, FL

801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House

Project Title: 801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

Lake City, FL

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/7/2008

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

and the	Type*		Overhang			Window Area(sqft)			НТМ		
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded	Shaded	Unshaded	*********	
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	30.0	0.0	30.0	29	60	1801	Btuh
2	2, Clear, 0.87, None, N, N	NW	1.5ft.	3.5ft.	9.0	0.0	9.0	29	60	540	Btuh
3	2, Clear, 0.87, None, N, N	NW	1.5ft.	7.33	40.0	0.0	40.0	29	60	2401	
4	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	15.0	0.0	15.0	29	60	901	
5	2, Clear, 0.87, None, N, N	SE	1.5ft.	5.5ft.	40.0	16.2	23.8	29	63	1958	
6	2, Clear, 0.87, None,N,N 2, Clear, 0.87, None,N,N	SE	5ft.	7.33	13.3	13.3	0.0	29	63	385	
	H	SW	1.5ft.	5.5ft.	20.0	8.1	11.9	29	63	979	1000000000
Walls	Window Total				167 (8966	Btuh
vvaiis	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
-1	Frame - Wood - Ext			13.0/0			8.7		2.1	1979	Btuh
2	Frame - Wood - Adj			13.0/0	0.09		0.0		1.5	211	Btuh
	Wall Total					108	9 (sqft)			2190	Btuh
Doors	Type					Area	(sqft)		HTM	Load	
1	Insulated - Adjacent					20	0.0		9.8	196	Btuh
2	Insulated - Exterior					20	0.0		9.8	196	Btuh
	Door Total		4.			4	0 (sqft)		431794561	392	Btuh
Ceilings	Type/Color/Surface	R-Value				Area(sqft) HTM		HTM	Load		
1	Vented Attic/DarkShingle			30.0		140	0.0		1.7	2318	Btuh
	Ceiling Total					1400 (sqft)			2318	Btuh	
Floors	Туре	R-Value			Size			HTM	Load		
1	Slab On Grade			0.0		16	62 (ft(p))		0.0	0	Btuh
	Floor Total					162.	0 (sqft)			0	Btuh
						Z	one Enve	elope Su	ıbtotal:	13866	Btuh
nfiltration	Туре		A	СН		Volum	e(cuft)		CFM=	Load	
	SensibleNatural			0.49		112			91.5	1702	Btuh
Internal		(Occup	ants		Btuh/oc	cupant	A	ppliance	Load	300000
gain				6	>				0	1380	Btul
Duct load	, R6.0, Supply(), Return()							DGM:	= 0.00	0.0	Btuh
							Sensibl	e Zone	Load	16948 [3tuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL

Project Title: 801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

1/7/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	16948	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	16948	Btuh
2	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	16948	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	3343	Btuh
	Latent ventilation gain	0	Btuh
te es	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	4543	Btuh
	TOTAL GAIN	21491	Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details Project Title: Class 3

Spec House

801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F This calculation is for Worst Case. The house has been rotated 315 degrees.

1/7/2008

Component Loads for Zone #1: Main

	Type*	Type*			Overhang Window Area(sqft)			H	ITM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	30.0	0.0	30.0	29	60	1801	Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	3.5ft.	9.0	0.0	9.0	29	60	540	Btuh
3	2, Clear, 0.87, None,N,N	NW	1.5ft.	7.33	40.0	0.0	40.0	29	60	2401	Btuh
4	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	15.0	0.0	15.0	29	60	901	Btuh
5	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	40.0	16.2	23.8	29	63	1958	
6 7	2, Clear, 0.87, None, N, N	SE	5ft.	7.33	13.3	13.3	0.0	29	63	385	
,	2, Clear, 0.87, None,N,N	sw	1.5ft.	5.5ft.	20.0	8.1	11.9	29	63	979	A STEEL STEEL STEEL
	Window Total				167 (sqft)				8966	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
. 1	Frame - Wood - Ext			13.0/0	0.09	948	3.7		2.1	1979	Btuh
2	Frame - Wood - Adj			13.0/0	0.09	140	0.0		1.5	211	
	Wall Total					108	9 (sqft)		2/33/56	2190	Btuh
Doors	Туре					Area	(sqft)		HTM	Load	
- 1	Insulated - Adjacent					20	.0		9.8	196	Btuh
2	Insulated - Exterior					20	.0		9.8	196	
	Door Total					4	0 (sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	lue		Area(нтм	Load	Dian
1	Vented Attic/DarkShingle			30.0		140			1.7		Btuh
	Ceiling Total						0 (sqft)		1.5.5	2318	
Floors	Туре		R-Value			Siz			НТМ	Load	Dian
1	Slab On Grade			0.0		162 (ft(p))			0.0	0	Btuh
	Floor Total						0 (sqft)		0.0	_	Btuh
						Zo	ne Enve	lope Su	ıbtotal:	13866	Btuh
nfiltration	Туре		Α	СН		Volume			CFM=	Load	
	SensibleNatural			0.49		112			91.5	1702	Btuh
Internal		(Occup	ants		Btuh/oc	cupant	Α	ppliance	Load	
gain				6	>				0	1380	Btuh
Duct load	, R6.0, Supply(), Return()							DGM =	= 0.00		Btuh
							Sensibl	e Zone	Load	16948 E	3tuh

Manual J Summer Calculations

Residential Load - Component Details (continued)
Project Title:

Spec House

Lake City, FL

801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

1/7/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	16948	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	16948	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	16948	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	3343	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	4543	Btuh
	TOTAL GAIN	21491	Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

Residential Window Diversity

MidSummer

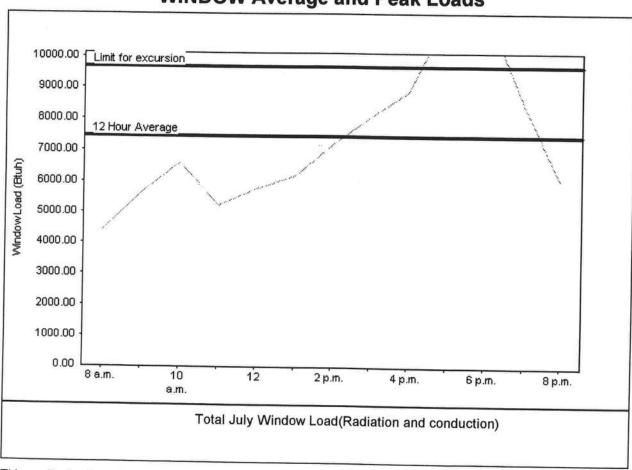
Spec House Project Title: 801011MartinBen

Class 3 Rating Registration No. 0 Climate: North

1/7/2008

Weather data for: Gainesville - Det	faults			
Summer design temperature	92 1	F	Average window load for July	7455 Btuh
Summer setpoint	75 F	F	Peak window load for July	11525 Btu
Summer temperature difference	17 F	F	Excusion limit(130% of Ave.)	9692 Btuh
Latitude	29 1	North	Window excursion (July)	1833 Btuh

WINDOW Average and Peak Loads



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY:

DATE:

EnergyGauge® FLR2PB v4.1



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Community **Affairs**

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COMMUNITY PLANNING

MANAGEMENT HOUSING & COMMUNITY DEVELOPMENT

USER: Public User Product Approval

Application Type

Code Version

Archived

OFFICE OF THE

Comments Application Status

Product Manufacturer

Address/Phone/Email

Authorized Signature

Window

Technical Representative Address/Phone/Email

Address/Phone/Email Quality Assurance Representative Product Approval Menu > Product or Application Search > Application List > Application Detail

FL5108

2004 New

Approved

650 W Market St MI Windows and Doors Gratz, PA 17030 (717) 365-3300 ext 2101

surich@miwd.com

surich@miwd.com Steven Urich FROM-AAMA



AAMA CERTIFICATION PROGRAM



AUTHORIZATION FOR PRODUCT CERTIFICATION

MI Windows & Doors, Inc. P.O. Box 370 Gratz, PA 17030-0370

Attn: Biff Emley

The product described below is hereby approved for listing in the next issue of the AAMA Certified Products Directory. The approval is based on successful completion of tests, and the reporting to the Administrator of the results of tests, accompanied by related drawings, by an AAMA Accredited Laboratory.

The listing below will be added to the next published AAMA Certified Products Directory.

SPECIFICATION		1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2				
AAMA/NWWOA 101/LS, 2-97 H-R55*-36x62	RECORD OF PRODUCT TESTED					
COMPANY AND PLANT LOCATION	CODE NO.	SERIES MODEL & PRODUCT DESCRIPTION	MUMIXAM	NO.		
MI Windows & Doors, Inc. (Oldsmar, FL) MI Windows & Doors, Inc. (Smyma, TN)	MTL-8	185/3185 SH (Fin) (AL)(O/O)(OG) (ASTM)	FRAME 3'0' x 5'2"	SASH 210° x 27°	By Request	

- This Certification will expire <u>May 14, 2008</u> and requires validation until then by continued listing in the current AAMA Certified Products Directory.
- 3. Product Tested and Reported by: Architectural Testing, Inc.

Report No.: 01-50360.02

Date of Report: June 14, 2004

NOTE: PLEASE REVIEW, AND ADVISE ALI IMMEDIATELY IF DATA, AS SHOWN, NEEDS CORRECTION.

Date:

August 1, 2005

cc: AAMA JGS/df

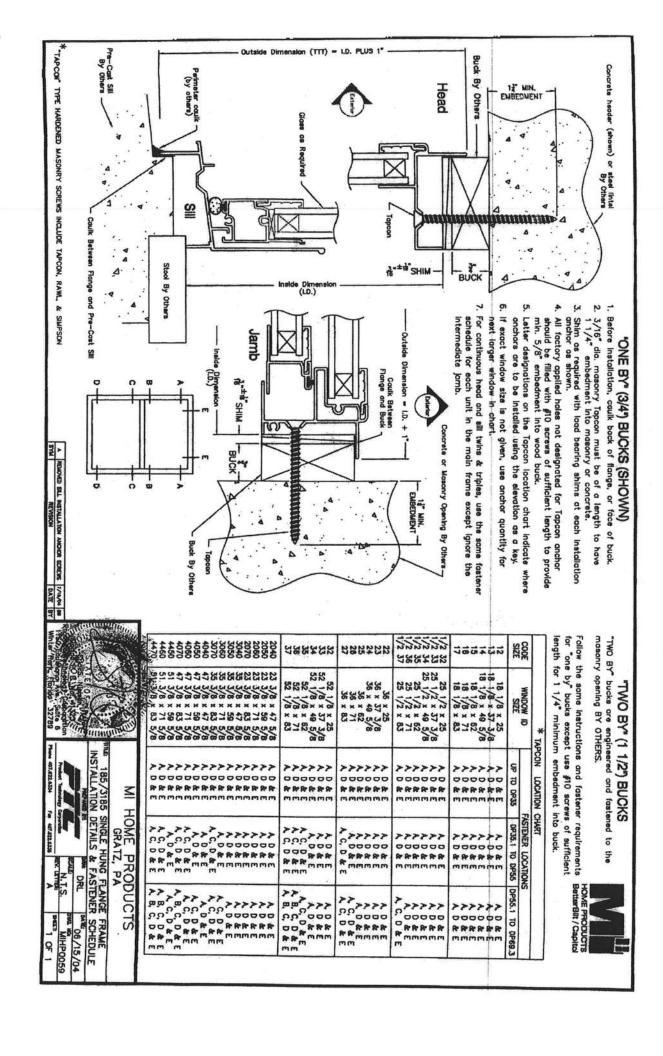
ACP-04 (Rev. 5/03)

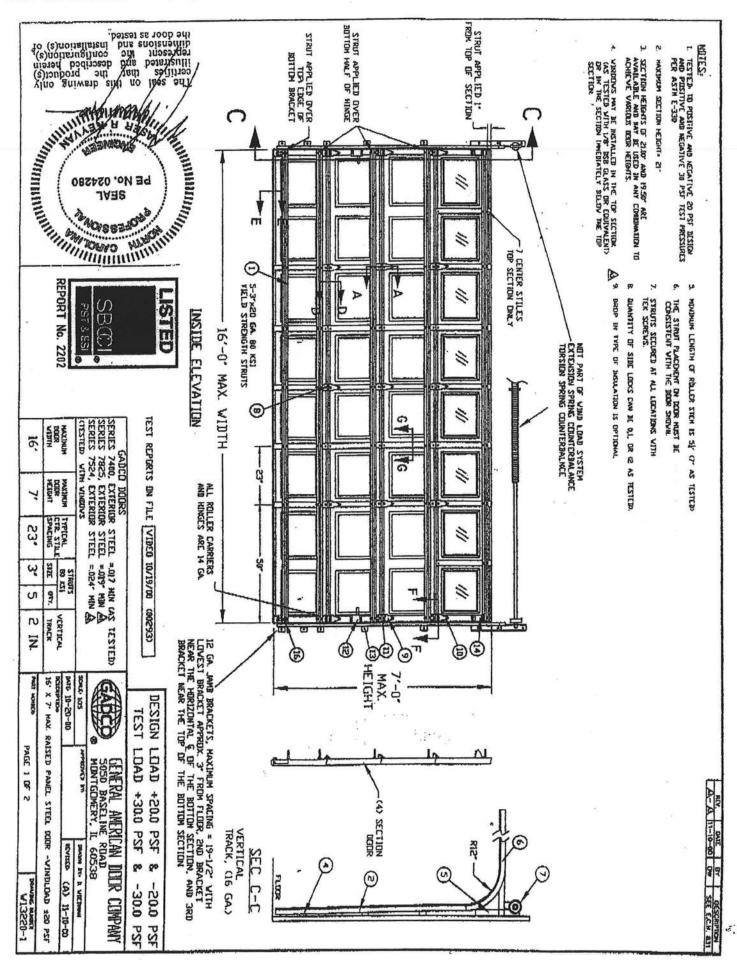
Validated for Certification:

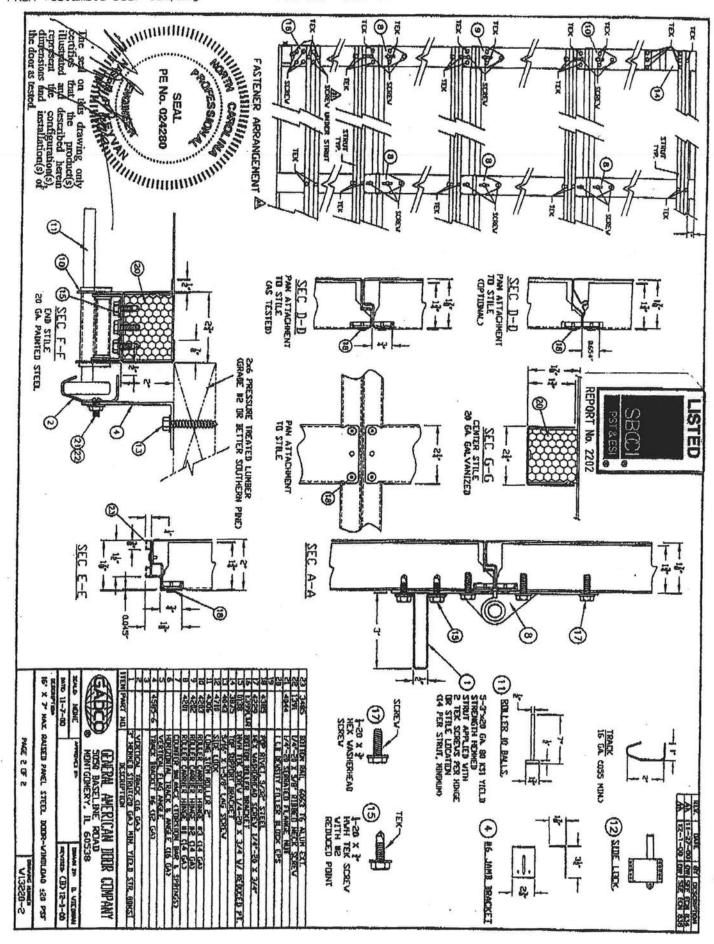
Associated Laboratories, Inc.

Authorized for Cartification:

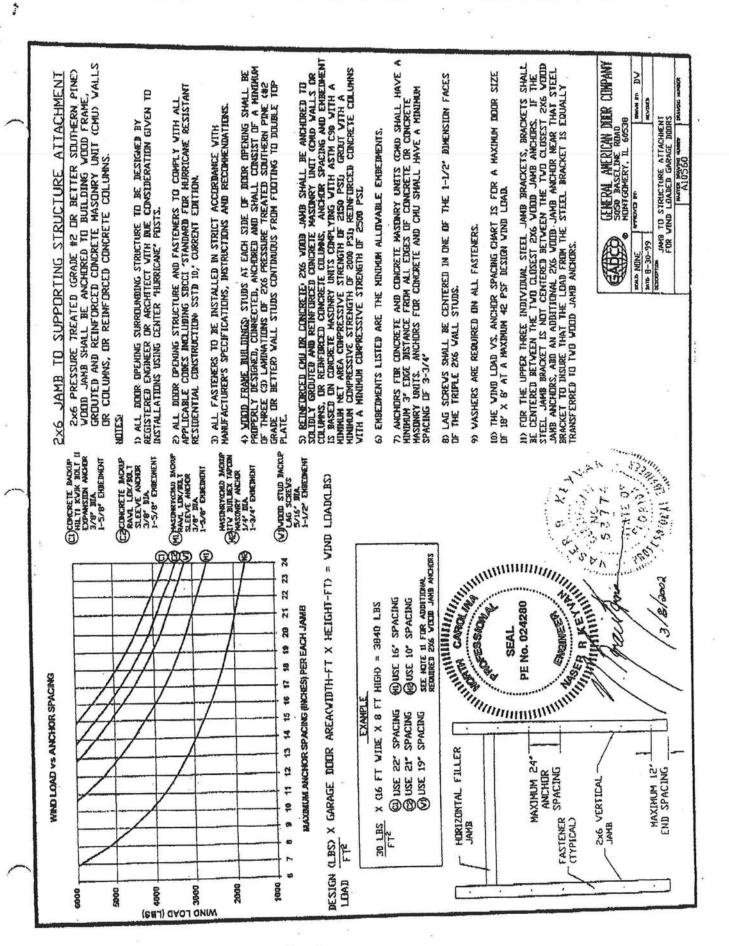
American Architectural Manufacturers Association





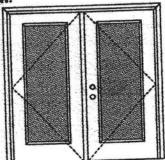


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WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door unit size = 60° x 6'8"

Design Pressure +40.5/-40.5

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES: 1/4 GLASS:











1/2 GLASS:









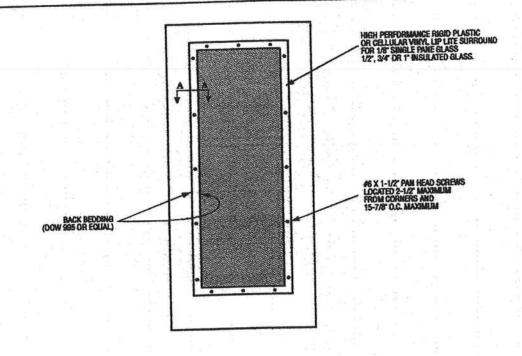




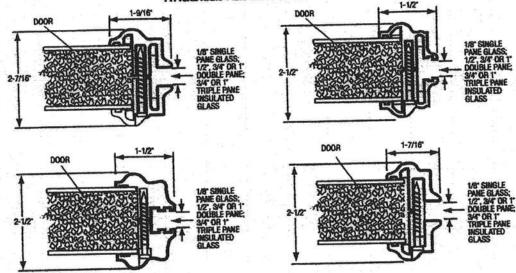
*This glass kit may also be used in the following door styles: 5-panet; 5-panet with scroll; Eyebrow 5-panet with scroll



GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A TYPICAL RISID PLASTIC LIP LITE SURROUND





WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:



















CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of stab filled with rigid polyurethane foam core. Stab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH MIAMI-DADE BCCO PA202

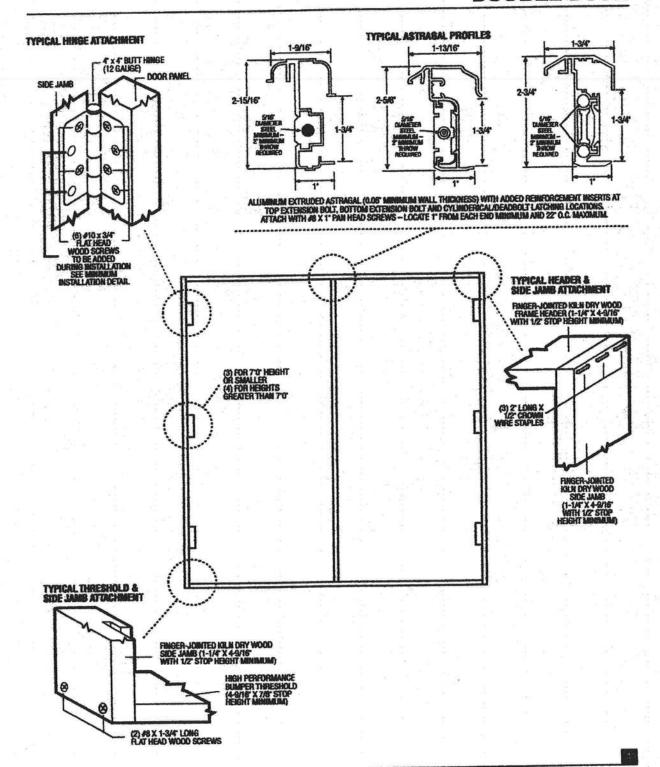
COMPANY NAME CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer Kurt Balthazor, P.E. - License Number 56533

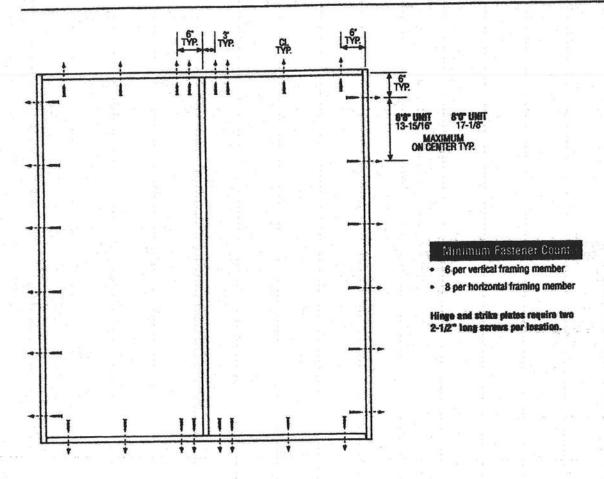


OUTSWING UNITS WITH DOUBLE DOOR





DOUBLE DOOR



Latching Hardware:

Compliance requires that GRADE 2 or better (ANSI/BHIMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

- Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners
 analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
- The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
- 3. Wood bucks by others, must be anchored properly to transfer loads to the structure.











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FL1956-R1 Approved Revision 2004 Application Status Application Type Code Version Comments Archived

Product Manufacturer Address/Phone/Email

TAMKO Building Products, Inc. fred_oconnor@tamko.com Joplin, MO 64802 (800) 641-4691 ext 2394 PO Box 1404

Authorized Signature

fred_oconnor@tamko.com

Frederick O'Connor

Technical Representative Address/Phone/Email

fred_oconnor@tamko.com Frederick J. O'Connor Joplin, MO 64802 (800) 641-4691 PO Box 1404

2/14/2007 11:22 A

Quality Assurance Representative Address/Phone/Email		
Category Subcategory	Roofing Asphalt Shingles	
Compliance Method	Certification Mark or Listing	
Certification Agency	Underwriters Laboratories Inc.	
Referenced Standard and Year (of Standard)	Standard ASTM D 3462	Year 2001
Equivalence of Product Standards Certified By		
Product Approval Method	Method 1 Option A	
Date Submitted	06/09/2005	
Date Pending FBC Approval Date Approved	06/25/2005 06/29/2005	
Summary of Products		
FL # Model, Number or Name	or Name Description	

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slopes of 2:12 or greater. Not approved for use in HVHZ.

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DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100

(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

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5 of 5





Northbreak Otvision

333 Pfingster Road Northcrook, 1, 60062-2096 USA www.i.com let 1,847,272,5900

June 17, 2005

Tamko Roofing Products Ms. Kerri Eden P.O. Box 1404 220 W. 4th Street Joplin, MO 64802-1404

Our Reference: R2919

This is to confirm that "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage 50 AR", "Glass-Seal AR" manufactured at Tuscaloosa, AL and "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage XL AR", "Heritage 50 AR" manufactured at Frederick, MD and "Heritage 30 AR", "Heritage XL AR", and "Heritage 50 AR" manufactured in Dallas, TX are UL Listed asphalt glass mat shingles and have been evaluated in accordance with ANSI/UL 790, Class A (ASTM E108), ASTM D3462, ASTM D3161 or UL 997 modified to 110 mph when secured with four nails.

Let me know if you have any further questions.

Very truly yours,

Alpesh Patel (Ext. 42522)

Engineer Project

Fire Protection Division

Reviewed by,

Randall K. Laymon (Ext. 42687)

Engineer Sr Staff

Fire Protection Division

P. K. Jayman



Application Instructions for

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS LAMINATED ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

I. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-andgroove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

TAMKO does not recommend re-roofing over existing roof.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

- 1. Vapor Condensation
- 2. Buckling of shingles due to deck movement.
- 3. Rotting of wood members.
- 4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents. FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

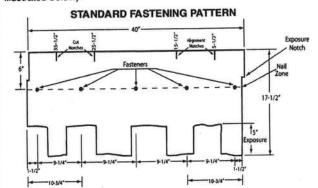
3. FASTENERS

WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, this will result in the termination of TAMKO's liabilities under the limited warranty. TAMKO will not be responsible for damage to shingles caused by winds in excess of the applicable miles per hour as stated in the limited warranty. See limited warranty for details.

FASTENING PATTERNS: Fasteners must be placed 6 in. from the top edge of the shingle located horizontally as follows:

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1-1/2 in. back from each end, one 10-3/4 in. back from each end and one 20 in. from one end of the shingle for a total of 5 fasteners. (See standard fastening pattern illustrated below).



2) Mansard or Steep Slope Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) Use standard nailing instructions with four additional nails placed 6 in. from the butt edge of the shingle making certain nails are covered by the next (successive) course of shingles.
(Continued)

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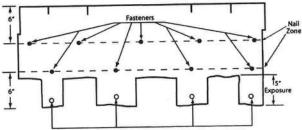


(CONTINUED from Pg. 1)

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS LAMINATED ASPHALT SHINGLES

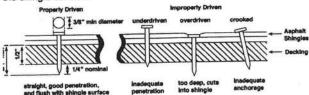
Each shingle tab must be sealed underneath with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a \$.25 piece and applied to shingles with a 5 in. exposure, use 9 fasteners per shingle.

MANSARD FASTENING PATTERN



Apply under each tab 1° diameter asphalt adhesive cement.

NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12 gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in. into the roof deck. Where the deck is less than 3/4 in. thick, the nails should be long enough to penetrate completely through plywood decking and extend at least 1/8 in. through the roof deck. Drive nail head flush with the shingle surface.



4. UNDERLAYMENT

UNDERLAYMENT: An underlayment consisting of asphalt saturated felt must be applied over the entire deck before the installation of TAMKO shingles. Failure to add underlayment can cause premature failure of the shingles and leaks which are not covered by TAMKO's limited warranty. Apply the felt when the deck is dry. On roof decks 4 in. per foot and greater apply the felt parallel to the eaves lapping each course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left exposed, the underlayment felt may be adversely affected by moisture and weathering. Laying of the underlayment and the shingle application must be done together.

Products which are acceptable for use as underlayment are:

- TAMKO No. 15 Asphalt Saturated Organic Felt
- A <u>non-perforated</u> asphalt saturated organic felt which meets ASTM: D226, Type I or ASTM D4869, Type I
- Any TAMKO <u>non-perforated</u> asphalt saturated organic felt
- TAMKO TW Metal and Tile Underlayment, TW Underlayment and Moisture Guard Plus® (additional ventilation maybe required. Contact TAMKO's technical services department for more information)

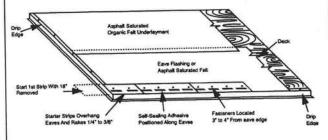
In areas where ice builds up along the eaves or a back-up of water from frozen or clogged gutters is a potential problem, TAMKO's Moisture Guard Plus® waterproofing underlayment (or any specialty eaves flashing product) may be applied to eaves, rakes, ridges, valleys, around chimneys, skylights or dormers to help prevent water damage. Contact TAMKO's Technical Services Department for more information.

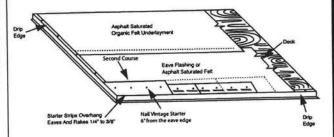
TAMKO does not recommend the use of any substitute products as shingle underlayment.

5. APPLICATION INSTRUCTIONS

STARTER COURSE: Two starter course layers must be applied prior to application of Heritage Vintage AR Shingles.

The first starter course may consist of TAMKO Shingle Starter, three tab self-sealing type shingles or a 9 inch wide strip of mineral surface roll roofing. If three tab self-sealing shingles are used, remove the exposed tab portion and install with the factory applied adhesive adjacent to the eaves. If using three tab self-sealing shingles or shingle starter, remove 18 in. from first shingle to offset the end joints of the Vintage Starter. Attach the first starter course with approved fasteners along a line parallel to and 3 in. to 4 in. above the eave edge. The starter course should overhang both the eave and rake edge 1/4 in. to 3/8 in. Over the first starter course, install Heritage Vintage Starter AR and begin at the left rake edge with a full size shingle and continue across the roof nailing the Heritage Vintage Starter AR along a line parallel to and 6 in. from the eave edge.





Note: Do not allow Vintage Starter AR joints to be visible between shingle tabs. Cutting of the starter may be required.

HERITAGE VINTAGE STARTER AR 12 1/2" x 36" 20 PIECES PER BUNDLE 60 LINEAL FT. PER BUNDLE

(Continued)

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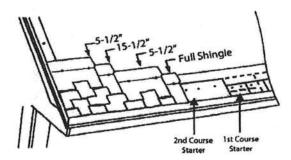
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(CONTINUED from Pg. 2)

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS LAMINATED ASPHALT SHINGLES

SHINGLE APPLICATION: Start the first course at the left rake edge with a full size shingle and overhang the rake edge 1/4 in. to 3/8 in.. To begin the second course, align the right side of the shingle with the 5-1/2 in. alignment notch on the first course shingle making sure to align the exposure notch. (See shingle illustration on next page) Cut the appropriate amount from the rake edge so the overhang is 1/4" to 3/8". For the third course, align the shingle with the 15-1/2 in. alignment notch at the top of the second course shingle, again being sure to align the exposure notch. Cut the appropriate amount from the rake edge. To begin the fourth course, align the shingle with the 5-1/2 in. alignment notch from the third course shingle while aligning the exposure notch. Cut the appropriate amount from the rake edge. Continue up the rake in as many rows as necessary using the same formula as outlined above. Cut pieces may be used to complete courses at the right side. As you work across the roof, install full size shingles taking care to align the exposure notches. Shingle joints should be no closer than 4 in.



6. LOW SLOPE APPLICATION

On pitches 2 in. per foot to 4 in. per foot cover the deck with two layers of underlayment. Begin by applying the underlayment in a 19 in. wide strip along the eaves and overhanging the drip edge by 1/4 to 3/4 in. Place a full 36 in. wide sheet over the 19 in. wide starter piece, completely overlapping it. All succeeding courses will be positioned to overlap the preceding course by 19 in. If winter temperatures average 25°F or less, thoroughly cement the laps of the entire underlayment to each other with plastic cement from eaves and rakes to a point of a least 24 in. inside the interior wall line of the building. As an alternative, TAMKO's Moisture Guard Plus self-adhering waterproofing underlayment may be used in lieu of the cemented felts.

7. VALLEY APPLICATION

TAMKO recommends an open valley construction with Heritage Vintage AR shingles.

To begin, center a sheet of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment in the valley.

After the underlayment has been secured, install the recommended corrosion resistant metal (26 gauge galvanized metal or an equivalent) in the valley. Secure the valley metal to the roof deck. Overlaps should be 12" and cemented.

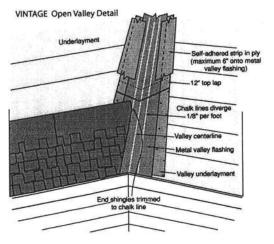
Following valley metal application; a 9" to 12" wide strip of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment should be applied along the edges of the metal valley flashing (max. 6" onto metal valley flashing) and on top of the valley underlayment. The valley will be completed with shingle application.

SHINGLE APPLICATION INSTRUCTIONS (OPEN VALLEY)

- Snap two chalk lines, one on each side of the valley centerline over the full length of the valley flashing. Locate the upper ends of the chalk lines 3" to either side of the valley centerline.
- The lower end should diverge from each other by 1/8" per foot.
 Thus, for an 8' long valley, the chalk lines should be 7" either side of the centerline at the eaves and for a 16' valley 8".

As shingles are applied toward the valley, trim the last shingle in each course to fit on the chalk line. Never use a shingle trimmed to less than 12" in length to finish a course running into a valley. If necessary, trim the adjacent shingle in the course to allow a longer portion to be used.

- Clip 1" from the upper corner of each shingle on a 45° angle to direct water into the valley and prevent it from penetrating between the courses.
- Form a tight seal by cementing the shingle to the valley lining with a 3" width of asphalt plastic cement (conforming to ASTM D 4586).



· CAUTION

Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.

(Continued)

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(CONTINUED from Pg. 3)

• HERITAGE® VINTAGE™ AR — Phillipsburg, KS LAMINATED ASPHALT SHINGLES

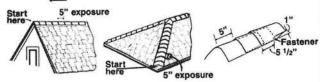
8. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener on each side, 5-1/2 in. back from the exposed end and 1 in. up from the edge. TAMKO recommends the use of TAMKO Heritage Vintage Hip & Ridge shingle products.

Fasteners should be 1/4 in. longer than the ones used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLE IN COLD WEATHER.

Direction of prevailing wind



THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

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