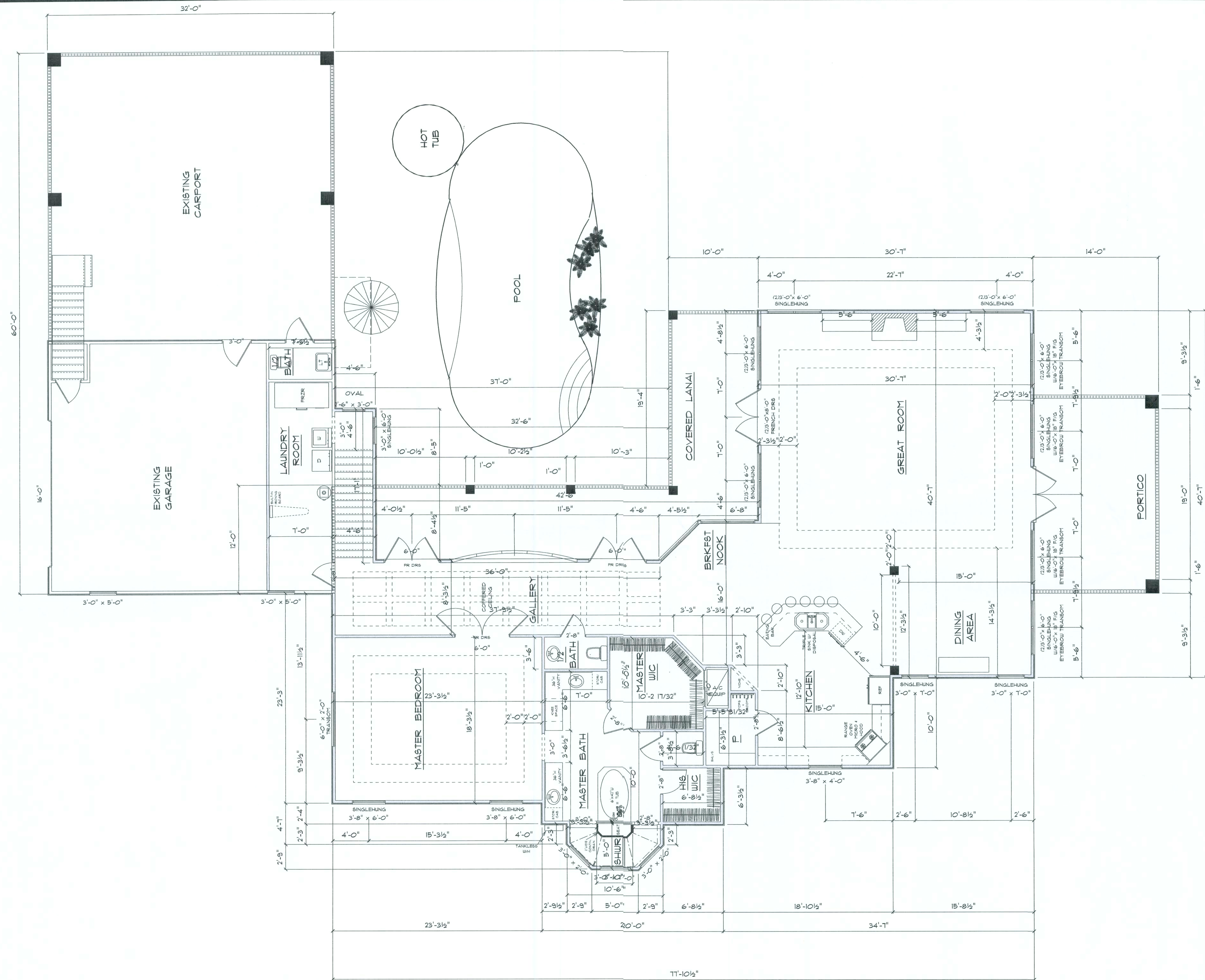




\* FLOOR PLAN \*

SCALE : 1/8" = 1'-0"

|                              |   |           |
|------------------------------|---|-----------|
| A/C LIVING AREA NEW ADDITION | = | 2827 S.F. |
| COVERED LANAI                | = | 610 S.F.  |
| PORTICO                      | = | 308 S.F.  |
| TOTAL AREA UNDER ROOF        | = | 3645 S.F. |

IMPORTANT NOTE:  
1. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY ALL GEOMETRY AND DIMENSIONS BEFORE ORDERING ANY MATERIALS OR BEGINNING CONSTRUCTION.  
2. THESE PLANS ARE FOR LAYOUT AND DESIGN PURPOSES ONLY ALL STRUCTURAL NOTES AND DETAILS TO BE PROVIDED BY AN ENGINEER.

| REVISIONS |
|-----------|
|           |
|           |
|           |

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

BRYAN & MISSY  
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PRINTED DATE:  
January 04, 2007  
DRAWN BY:  
Teena M. Ruffo  
CHECKED BY:

DESIGNED BY:  
Teena Ruffo

FINALS DATE:  
DEC 2006

JOB NUMBER:

DRAWING NUMBER

A-1  
OF 4 SHEETS

FILE COPY

| REVISIONS |  |
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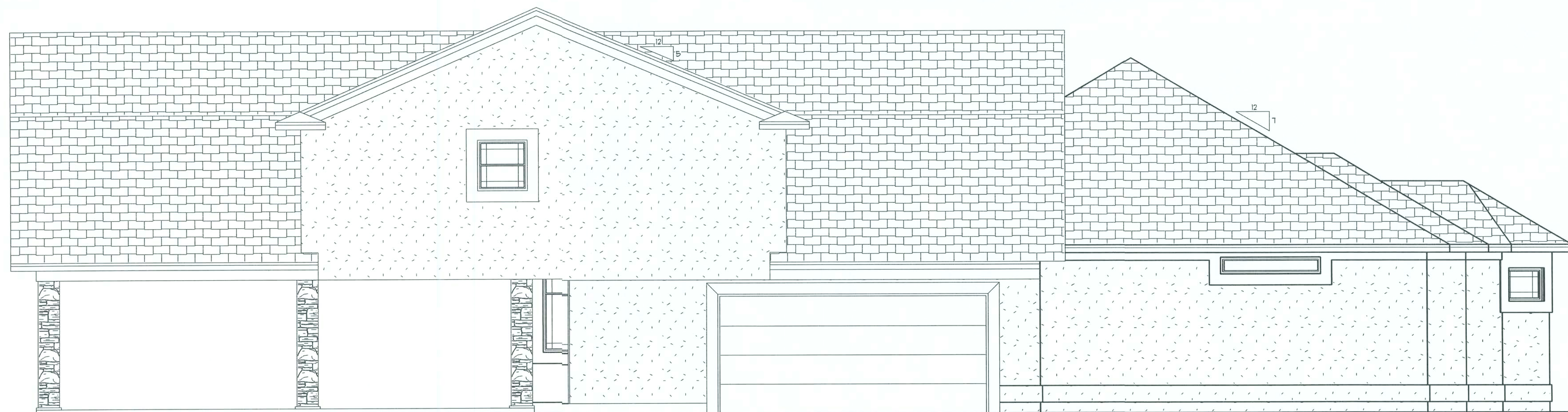
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



\* FRONT ELEVATION \*

SCALE : 1/4" = 1'-0"

18" OVERHANG ON ALL NEW ROOF EYES



\* REAR ELEVATION \*

SCALE : 1/4" = 1'-0"

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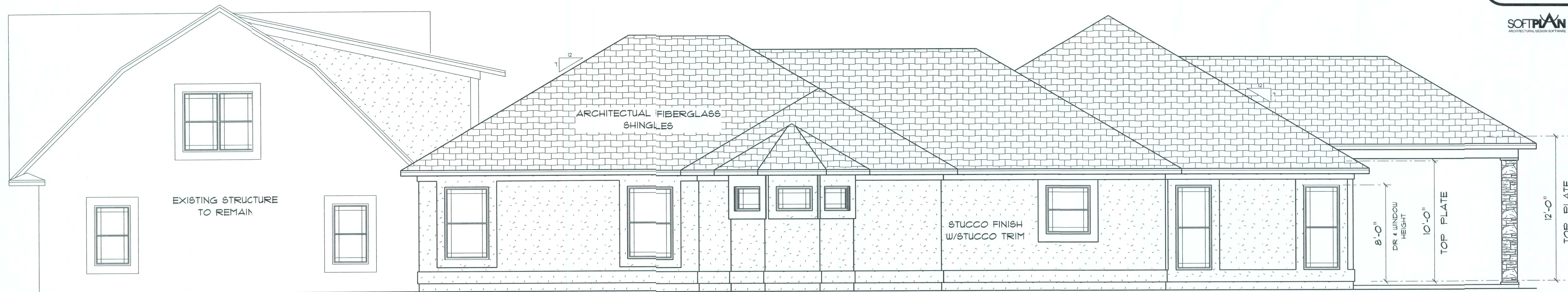
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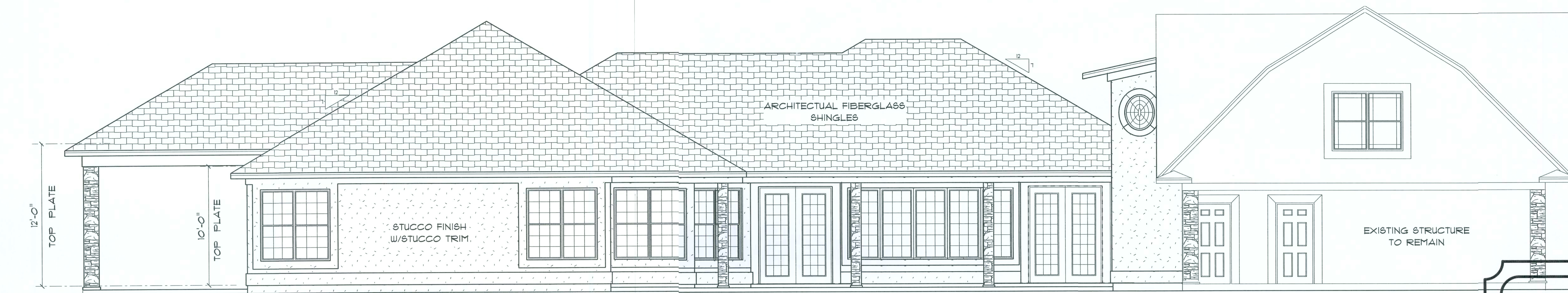
A2

OF 4 SHEETS



\* LEFT SIDE ELEVATION \*

SCALE : 1/4" = 1'-0"



\* RIGHT SIDE ELEVATION \*

SCALE : 1/4" = 1'-0"

IMPORTANT NOTE:

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DEC 2006

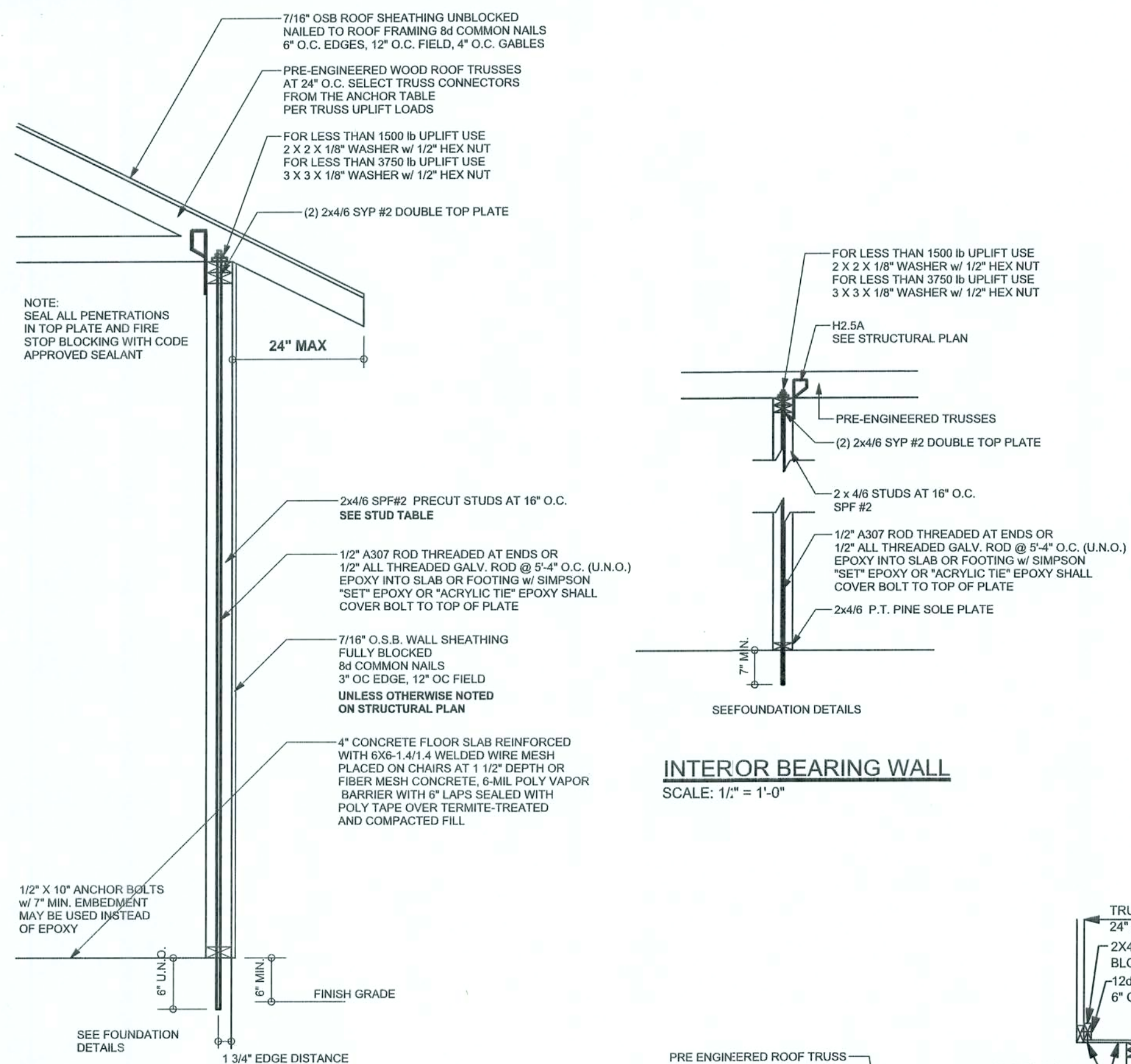
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OF 4 SHEETS



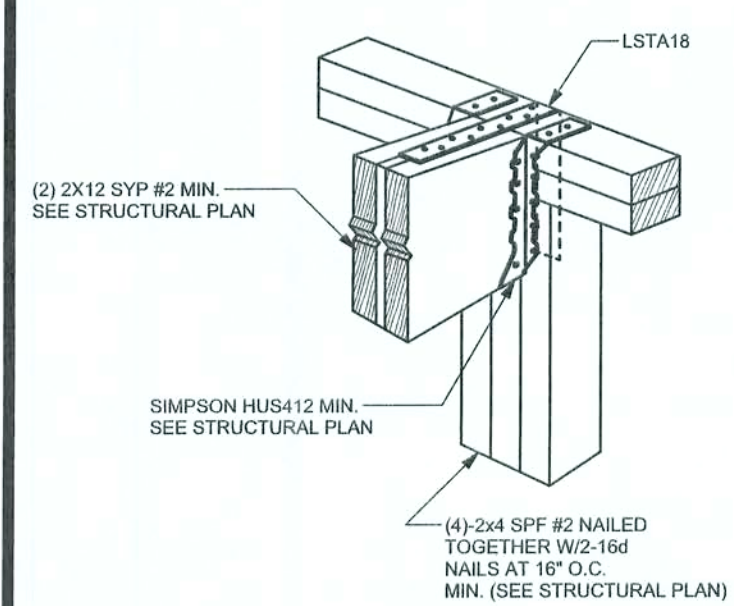


**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

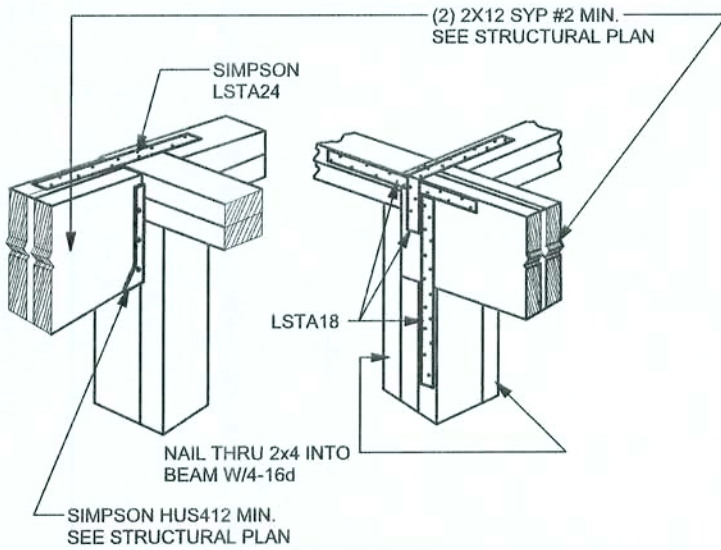
#### EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

|                  |                        |
|------------------|------------------------|
| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT  |
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT  |
| (1) 2x6 @ 16" OC | TO 18'-10" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 20'-0" STUD HEIGHT  |

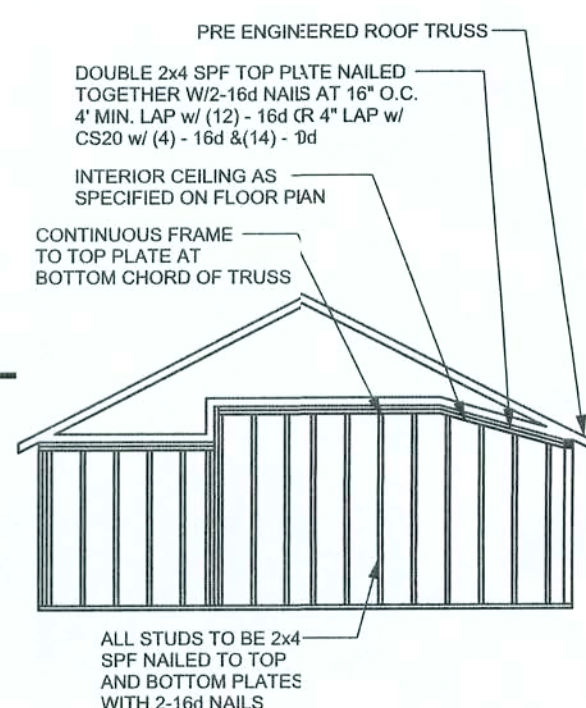
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13'-6" O.C.



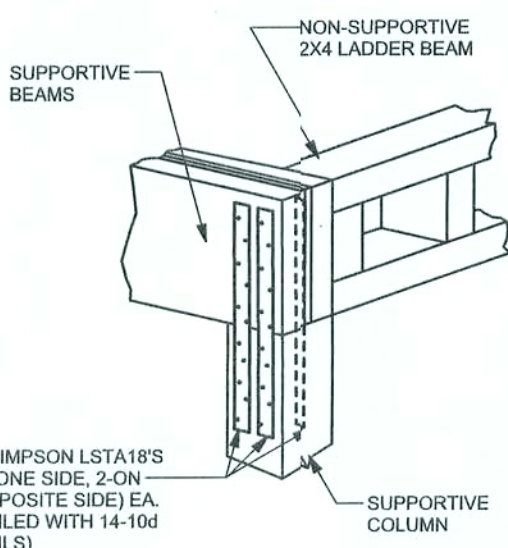
**BEAM MID-WALL CONNECTION DETAIL**  
SCALE: N.T.S.



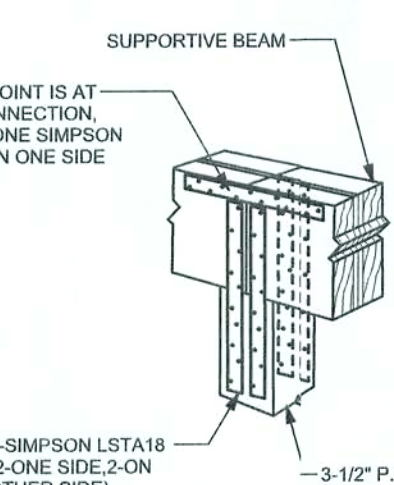
**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.



**CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL**  
SCALE: N.T.S.



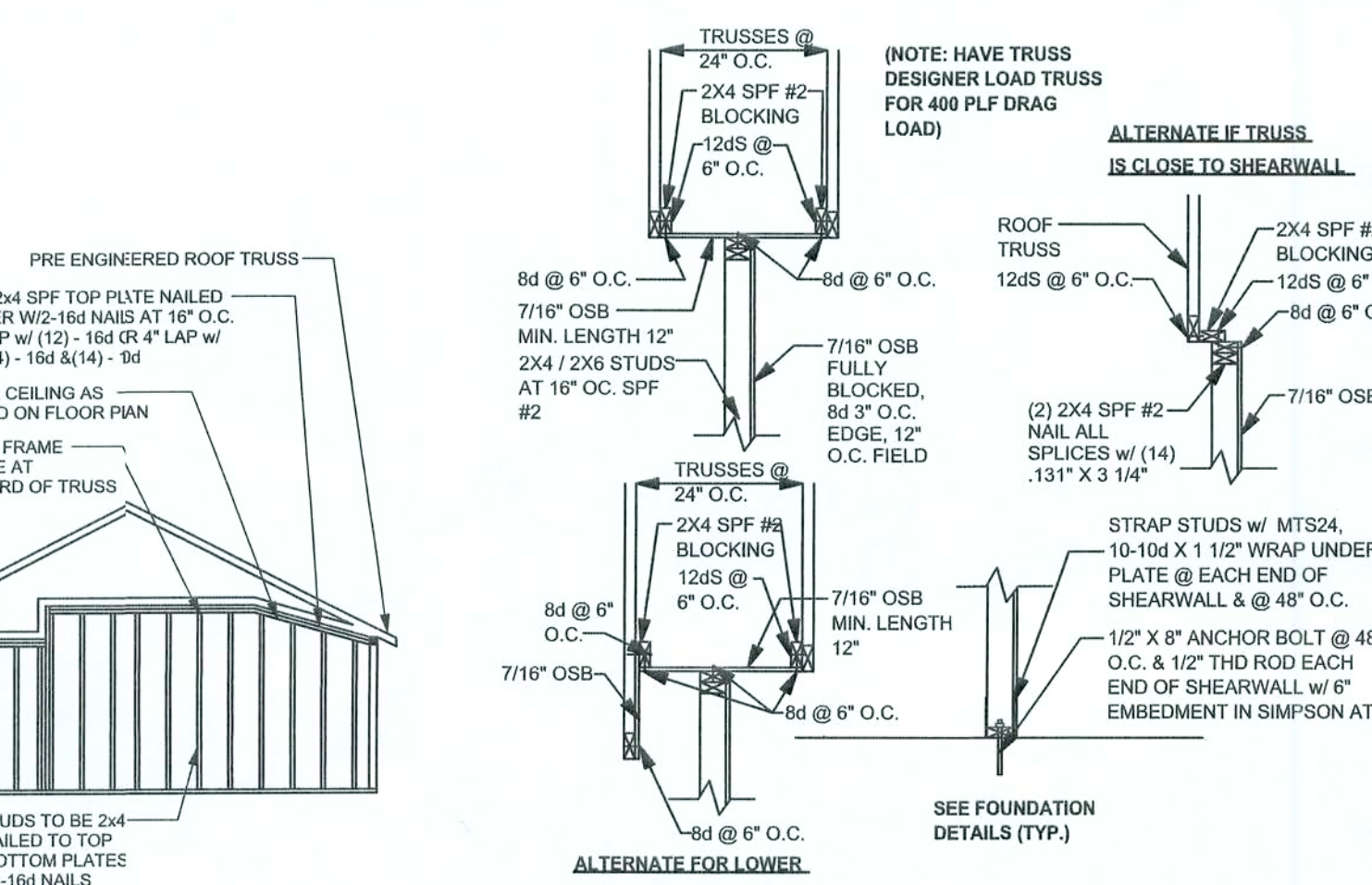
**SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.



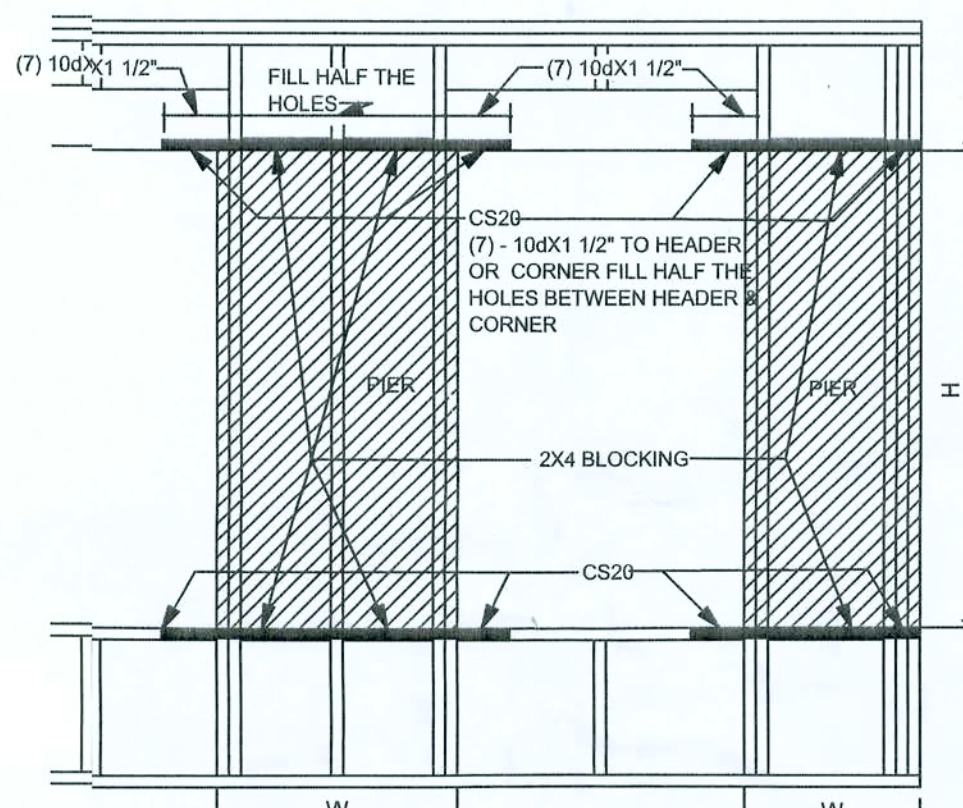
**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.

#### INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



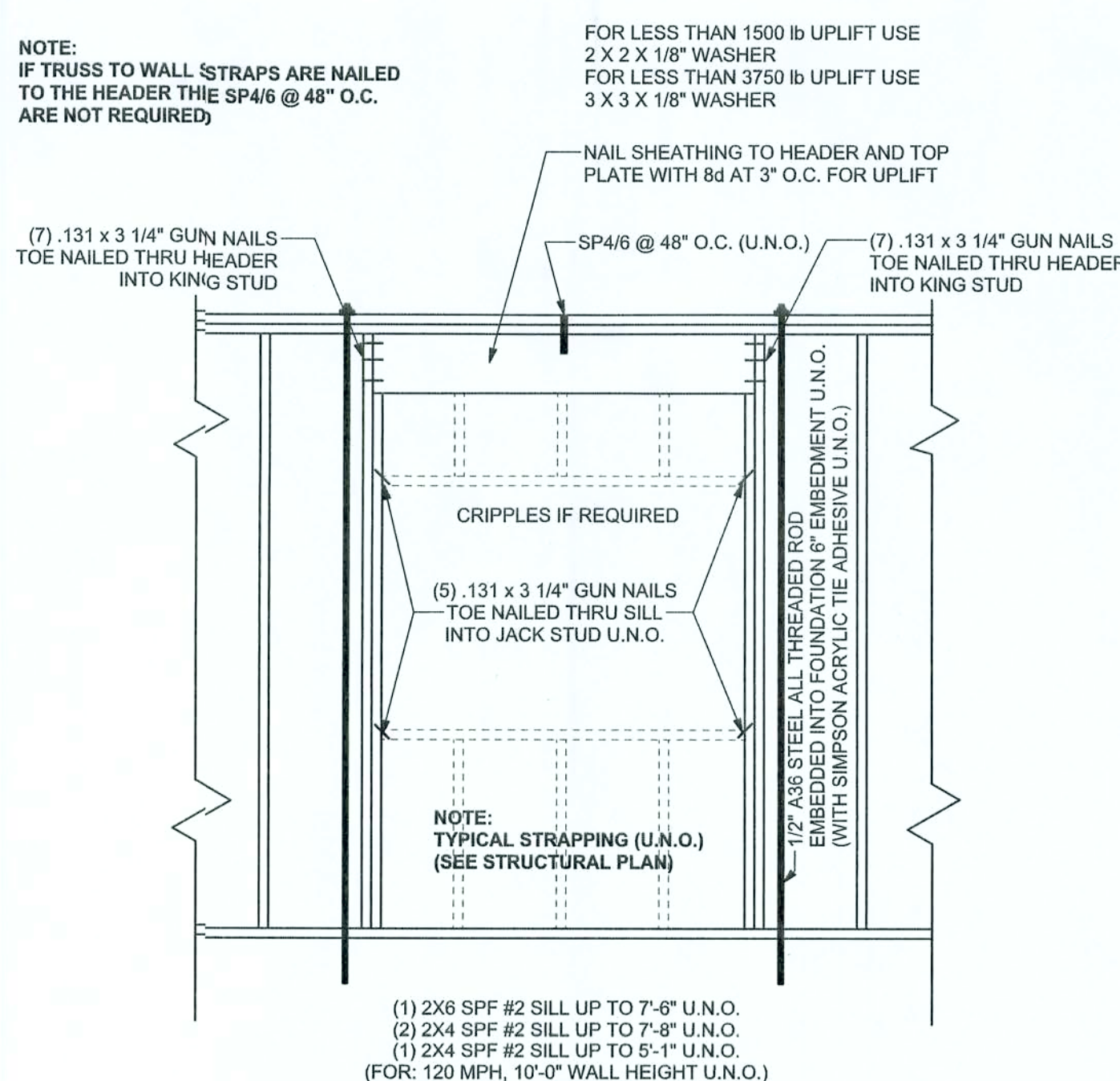
**W60 - INTERIOR SHEAR WALL DETAIL**  
SCALE: 1/2"=1'-0"



NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT  $H/W < 3.5:1$  WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HEIGHT OR 5' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

**W60 - OPENING FORCE TRANSFER DETAIL**  
SCALE: 1/2"=1'-0"

NOTE: IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THIS SP4/6 @ 48" O.C. ARE NOT REQUIRED.



**TYPICAL 1 STORY HEADER STRAPPING DETAIL**  
SCALE: 1/2" = 1'-0"

#### ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP               | UPLIFT LBS. SPF | TRUSS CONNECTOR*      | TO PLATES      | TO RAFTER/TRUSS | TO STUDS                          |
|-------------------------------|-----------------|-----------------------|----------------|-----------------|-----------------------------------|
| < 420                         | < 245           | H5A                   | 3-8d           | 3-8d            |                                   |
| < 455                         | < 265           | H5                    | 4-8d           | 4-8d            |                                   |
| < 360                         | < 235           | H4                    | 4-8d           | 4-8d            |                                   |
| < 455                         | < 320           | H3                    | 4-8d           | 4-8d            |                                   |
| < 415                         | < 365           | H2.5                  | 5-8d           | 5-8d            |                                   |
| < 600                         | < 535           | H2.5A                 | 5-8d           | 5-8d            |                                   |
| < 950                         | < 820           | H8                    | 8-8d           | 8-8d            |                                   |
| < 745                         | < 565           | H8                    | 5-10d, 1 1/2"  | 5-10d, 1 1/2"   |                                   |
| < 1465                        | < 1050          | H14-1                 | 13-8d          | 12-8d, 1 1/2"   |                                   |
| < 1465                        | < 1050          | H14-2                 | 15-8d          | 12-8d, 1 1/2"   |                                   |
| < 990                         | < 850           | H10-1                 | 8-8d, 1 1/2"   | 8-8d, 1 1/2"    |                                   |
| < 760                         | < 655           | H10-2                 | 6-10d          | 6-10d           |                                   |
| < 1470                        | < 1265          | H16-1                 | 10-10d, 1 1/2" | 2-10d, 1 1/2"   |                                   |
| < 1470                        | < 1265          | H16-2                 | 10-10d, 1 1/2" | 2-10d, 1 1/2"   |                                   |
| < 1000                        | < 860           | MTS24C                | 7-10d 1 1/2"   | 7-10d 1 1/2"    |                                   |
| < 1450                        | < 1245          | HTS24                 | 12-10d 1 1/2"  | 12-10d 1 1/2"   |                                   |
| < 2000                        | < 2490          | 2- HTS24              |                |                 |                                   |
| < 2050                        | < 1785          | LGT2                  | 14-16d         | 14-16d          |                                   |
| <b>HEAVY GIRDER TIEDOWNS*</b> |                 |                       |                |                 |                                   |
| < 3965                        | < 3330          | MG1                   |                | 22-10d          | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10680                       | < 6485          | HGT-2                 |                | 16-10d          | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530                       | < 9035          | HGT-3                 |                | 16-10d          | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250                        | < 9250          | HGT-4                 |                | 16-10d          | 2-5/8" THREADED ROD 12" EMBEDMENT |
| <b>STUD STRAP CONNECTOR*</b>  |                 |                       |                |                 |                                   |
| < 435                         | < 435           | SSP DOUBLE TOP PLATE  | 3-10d          |                 | 4-10d                             |
| < 455                         | < 420           | SSP SINGLE SILL PLATE | 1-10d          |                 | 4-10d                             |
| < 825                         | < 825           | DSP DOUBLE TOP PLATE  | 6-10d          |                 | 8-10d                             |
| < 825                         | < 600           | DSP SINGLE SILL PLATE | 2-10d          |                 | 8-10d                             |
| < 885                         | < 760           | SP4                   |                |                 | 6-10d, 1 1/2"                     |
| < 1240                        | < 1065          | SPH4                  |                |                 | 10-10d, 1 1/2"                    |
| < 885                         | < 760           | SP6                   |                |                 | 6-10d, 1 1/2"                     |
| < 1240                        | < 1065          | SPH6                  |                |                 | 10-10d, 1 1/2"                    |
| < 1235                        | < 1165          | LSTA18                | 14-10d         |                 |                                   |
| < 1235                        | < 1235          | LSTA21                | 16-10d         |                 |                                   |
| < 1030                        | < 1030          | CS20                  | 18-8d          |                 |                                   |
| < 1705                        | < 1705          | CS16                  | 28-8d          |                 |                                   |
| <b>STUD ANCHORS*</b>          |                 |                       |                |                 |                                   |
| < 1260                        | < 1305          | LTT10                 |                | 6-16d           | 1/2" AB                           |
| < 2310                        | < 2310          | LTD31                 | 18-10d, 1 1/2" |                 | 1/2" AB                           |
| < 2775                        | < 2570          | H2A                   | 2-5/8" BOLTS   |                 | 5/8" AB                           |
| < 4175                        | < 3695          | HTT16                 | 18-16d         |                 | 5/8" AB                           |
| < 1400                        | < 1400          | PHW42                 | 16-16d         |                 |                                   |
| < 3335                        | < 3335          | HPWH22                | 16-16d         |                 |                                   |
| < 2200                        | < 2200          | ABU44                 | 12-16d         |                 | 1/2" AB                           |
| < 2300                        | < 2300          | ABU66                 | 12-16d         |                 | 1/2" AB                           |
| < 2320                        | < 2320          | ABU88                 | 18-16d         |                 | 2-5/8" AB                         |

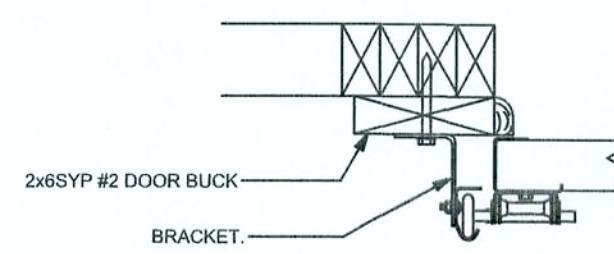
#### GRADE & SPECIES TABLE

|      |              | Fb (psi) | E (10 <sup>6</sup> psi) |
|------|--------------|----------|-------------------------|
| 2x8  | SYP #2       | 1200     | 1.6                     |
| 2x10 | SYP #2       | 1050     | 1.6                     |
| 2x12 | SYP #2       | 975      | 1.6                     |
| GLB  | 24F-V3 SP    | 2400     | 1.8                     |
| LVL  | TIMBERSTRAND | 1700     | 1.7                     |
| LVL  | MICROLAM     | 2900     | 2.0                     |
| PSL  | PARALAM      | 2900     | 2.0                     |

#### 2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER. LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 131 x 3 1/4" GN PER TABLE BELOW.

| DOOR WIDTH | 3/8" x 4" LAG | 16d STAGGER | (2) ROWS OF .131 x 3 1/4" GN |
|------------|---------------|-------------|------------------------------|
| 8'-10"     | 24" O.C.      | 5" O.C.     | 5" O.C.                      |
| 11'-15"    | 18" O.C.      | 4" O.C.     | 4" O.C.                      |
| 16'-18"    | 16" O.C.      | 3" O.C.     | 3" O.C.                      |



**GARAGE DOOR BUCK INSTALLATION DETAIL**  
SCALE: N.T.S.

#### GENERAL NOTES:

**TRUSSER:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS (T) TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING'S UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

**FOUNDATION:** CONFORM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOLS TEST PROVES OTHERWISE)

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 3000$  PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W1.4 x W1.4,  $F_y = 85$  KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 1FT, 30 NOT CUT WWW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS,  $F_y = 60$  KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-96, U.N.C.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP,  $F_b = 2.4ksi$ ,  $E = 1800ksi$ . UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, IMPLIED PERPENDICULAR TO FRAMING OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, 6" OC ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 8" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 3/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 3/64". WITH 3/4" BOLTS TO BE 3" x 3" x 3/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 3/64"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBG TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BUILDER'S RESPONSIBILITY

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL, HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

#### ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCC 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINTS AND ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

#### DESIGN DATA

##### WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING HORIZONTAL DIMENSION OR 60 FT, NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1) BASIC WIND SPEED = 110 MPH
- 2) WIND EXPOSURE = B
- 3) WIND IMPORTANCE FACTOR = 1.0
- 4) BUILDING CATEGORY = II
- 5) ROOF ANGLE = 10-45 DEGREES
- 6) MEAN ROOF HEIGHT = <30 FT
- 7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

| Zone             | Effective Wind Area (ft <sup>2</sup> ) | 10    | 100   |
|------------------|----------------------------------------|-------|-------|
| 1                | 19.9 -21.8                             | 18.1  | -18.1 |
| 2                | 19.9 -25.5                             | 18.1  | -21.8 |
| 2 On g           | -40.6                                  | -40.6 |       |
| 3                | 19.9 -25.5                             | 18.1  | -21.8 |
| 3 On g           | -42.4                                  | -42.4 |       |
| 4                | 21.8 -23.6                             | 18.5  | -20.4 |
| 5                | 21.8 -29.1                             | 18.5  | -22.6 |
| Doors & Windows  | 21.8                                   | -29.1 |       |
| 8x7 Garage Door  | 19.5                                   | -22.9 |       |
| 16x7 Garage Door | 18.5                                   | -21.0 |       |

#### DESIGN LOADS

|        |                                         |
|--------|-----------------------------------------|
| FLOOR  | 40 PSF (ALL OTHER DWELLING ROOMS)       |
|        | 30 PSF (SLEEPING ROOMS)                 |
|        | 30 PSF (ATTICS WITH STORAGE)            |
|        | 10 PSF (ATTICS WITHOUT STORAGE, <3.12)  |
| ROOF   | 20 PSF (FLAT OR <4.12)                  |
|        | 16 PSF (4.12 TO <12.12)                 |
|        | 12 PSF (12.12 AND GREATER)              |
| STAIRS | 40 PSF (ONE & TWO FAMILY DWELLINGS)     |
|        | SOIL BEARING CAPACITY 1000PSF           |
|        | (NOT IN FLOOD ZONE (BUILDER TO VERIFY)) |

#### REVISIONS

SOFTPLAN  
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WINDLOAD ENGINEER: Mark Disoway,  
P.E. No.53915, P.OB 888, Lake City, FL  
32056, 386-754-5419

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**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

15 JAN 07  
SEAL

Bryan & Missy  
Zecher Addition

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PRINTED DATE:  
January 15, 2007  
DRAWN BY: STRUCTURAL BY:  
David Disoway

FINALS DATE:  
15 / Jan / 07

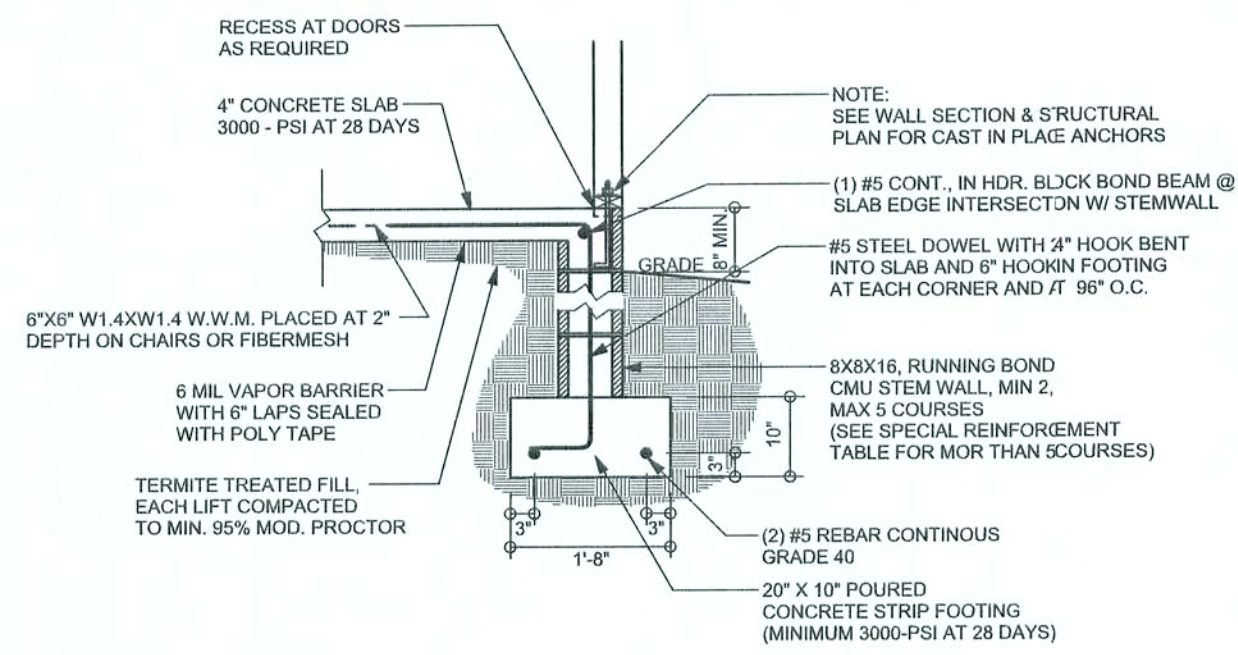
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701032  
DRAWING NUMBER

S-1

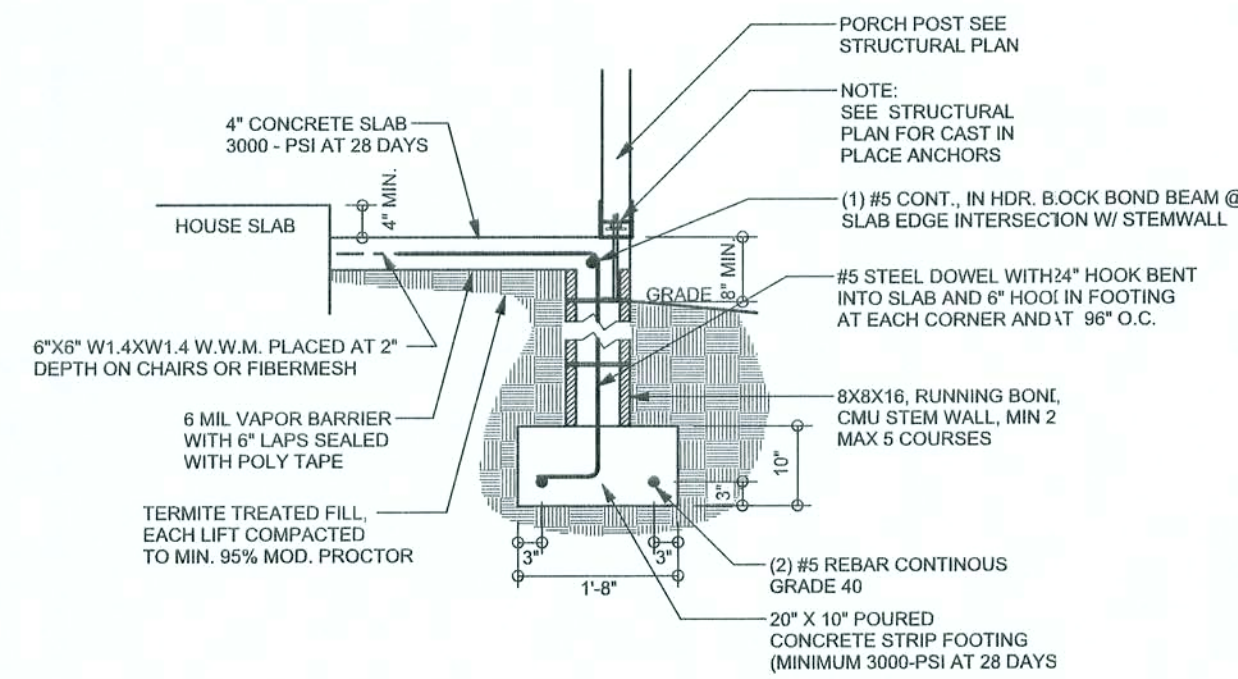
OF 3 SHEETS

| REVISIONS |  |
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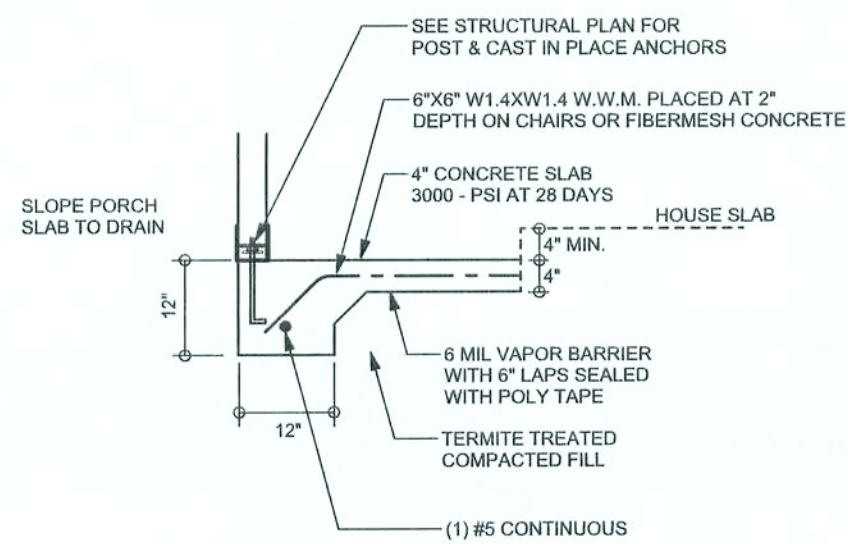
SOFTPLAN  
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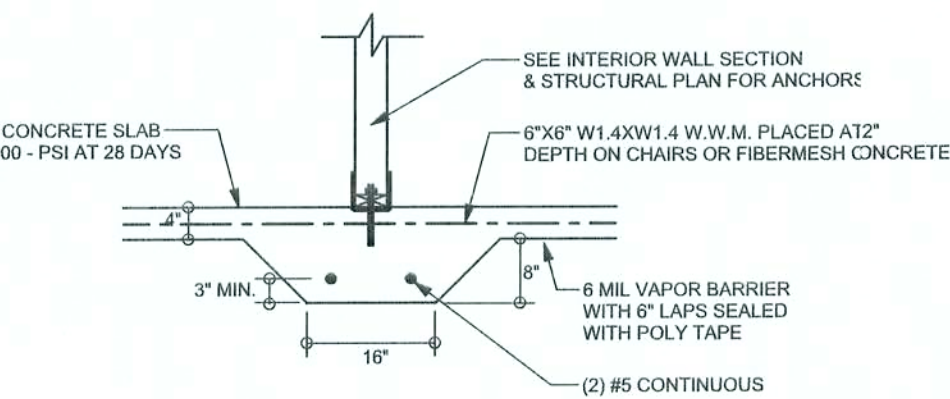
**F9 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



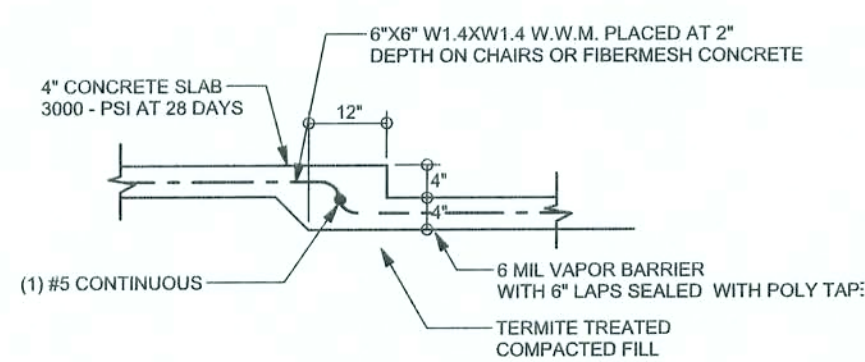
**F12 ALT. STEM WALL PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



**F5 PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



**F2 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



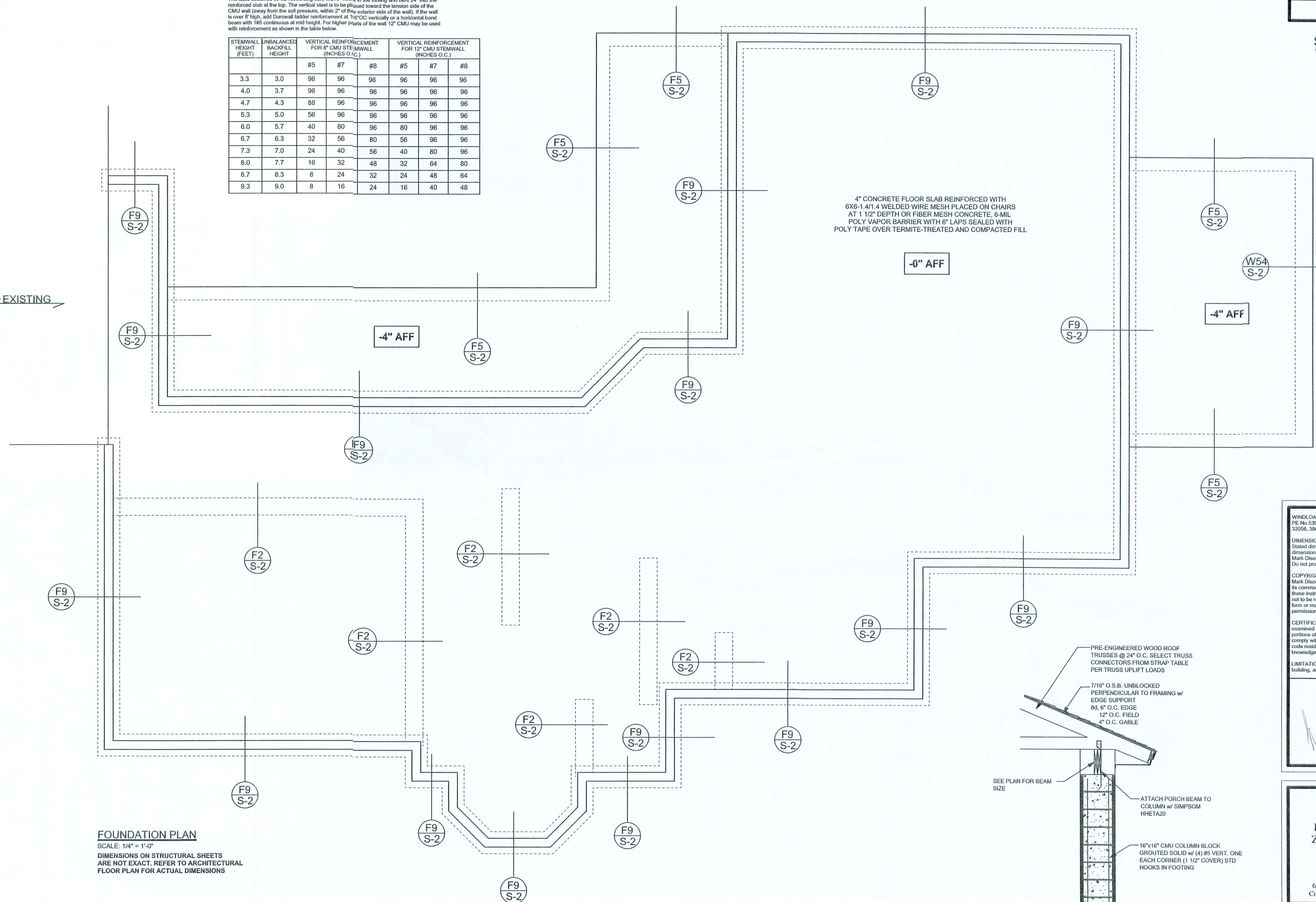
**F6 TYPICAL NON - BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"

#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 116" O.C. vertically or a horizontal bond beam with 195 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

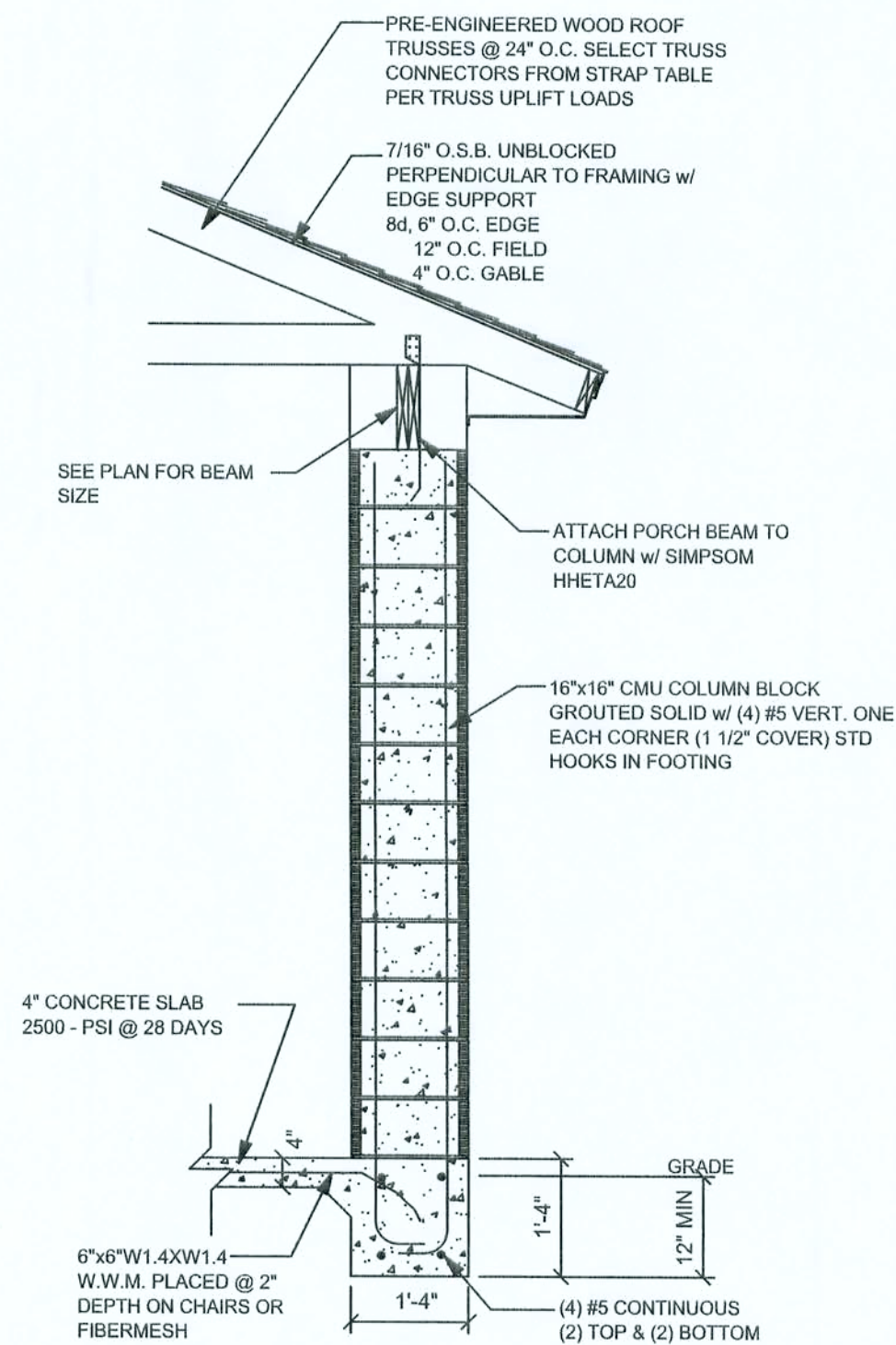
| STEM WALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.) |    | VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.) |    |
|-------------------------|----------------------------|-----------------------------------------------------------|----|------------------------------------------------------------|----|
|                         |                            | #5                                                        | #8 | #5                                                         | #8 |
| 3.3                     | 3.0                        | 96                                                        | 96 | 96                                                         | 96 |
| 4.0                     | 3.7                        | 96                                                        | 96 | 96                                                         | 96 |
| 4.7                     | 4.3                        | 88                                                        | 96 | 96                                                         | 96 |
| 5.3                     | 5.0                        | 56                                                        | 96 | 96                                                         | 96 |
| 6.0                     | 5.7                        | 40                                                        | 80 | 80                                                         | 96 |
| 6.7                     | 6.3                        | 32                                                        | 56 | 56                                                         | 96 |
| 7.3                     | 7.0                        | 24                                                        | 40 | 56                                                         | 80 |
| 8.0                     | 7.7                        | 16                                                        | 32 | 48                                                         | 64 |
| 8.7                     | 8.3                        | 8                                                         | 24 | 32                                                         | 48 |
| 9.3                     | 9.0                        | 8                                                         | 16 | 24                                                         | 40 |

EXISTING



#### FOUNDATION PLAN

SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



**W54 - 16X16 CMU PORCH COLUMN**  
SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, PCB 868, Lake City, FL 32056, 386-754-5419

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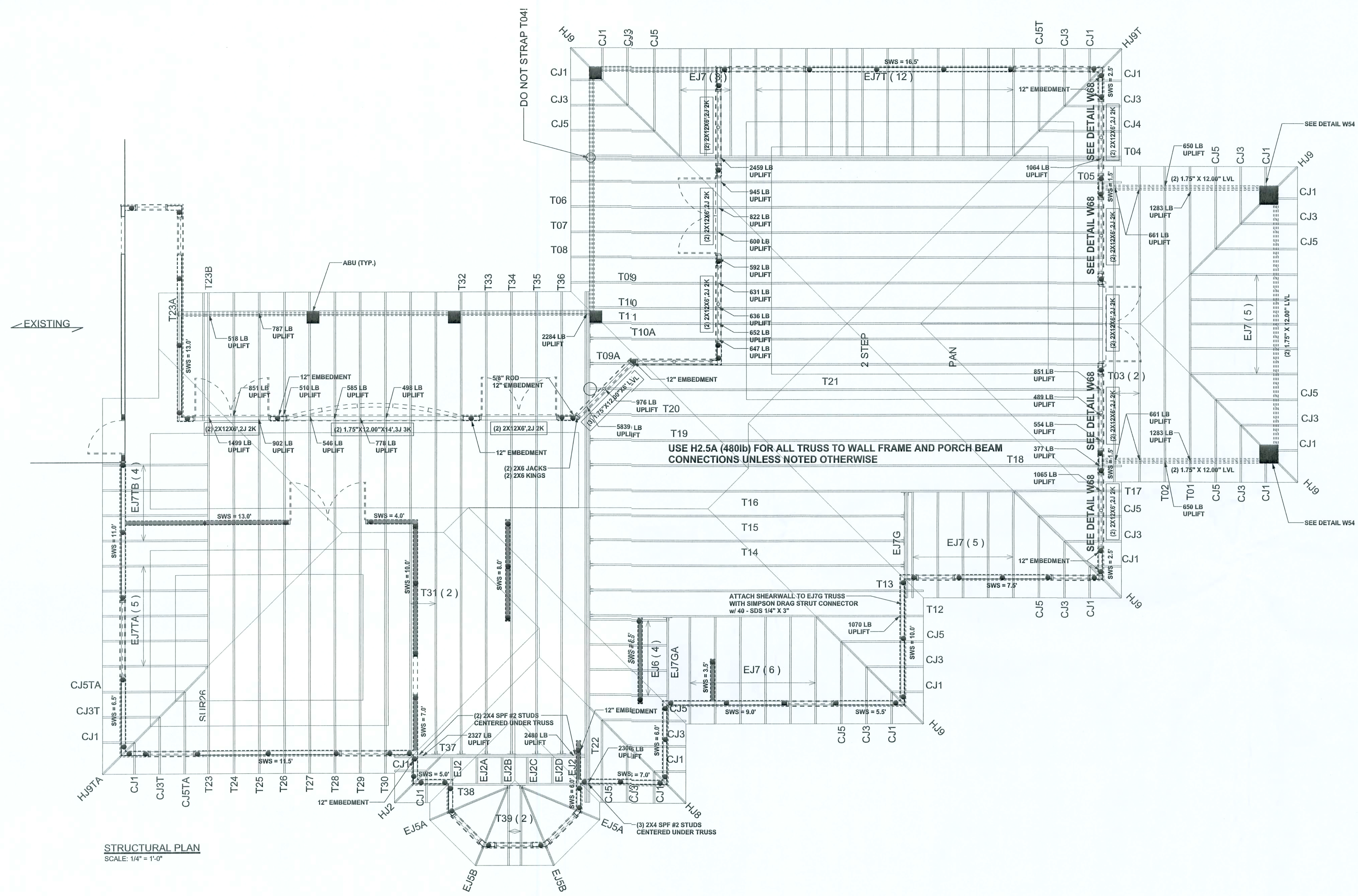
JOB NUMBER:  
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S-2

OF 3 SHEETS

| REVISIONS |  |
|-----------|--|
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ALTERNATE WALL TIE CONNECTION WHERE  
THREADED ROD CANNOT BE PLACED IN WALL.  
SCALE: 1/2" = 1'-0"

STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

### STRUCTURAL PLANNOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSSBRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

### THREADED ROD LEGEND

- INDICATES LOCATION OF:  
1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF:  
2ND FLOOR 1/2" A307 ALL THREADED ROD

### HEADER LEGEND

- (2) 2X12X0, 1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

### WALL LEGEND

|            |                                                              |
|------------|--------------------------------------------------------------|
| SWS = 0.0' | 1ST FLOOR EXTERIOR WALL                                      |
| SWS = 0.0' | 2ND FLOOR EXTERIOR                                           |
| IBW        | 1ST FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |
| IBW        | 2ND FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |

### TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

|              | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE   | 45.0'    | 95.5'  |
| LONGITUDINAL | 33.6'    | 79.0'  |

WINDLOAD ENGINEER: Mark Disoway,  
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**S-3**  
OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L221086