ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number; 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UX6487-Z0118152715

Truss Fabricator: Anderson Truss Company

Job Identification: 13-189--OWNER BUILDER Beasley Residence -- 265 SW Beasley

Truss Count: 8

Model Code: Florida Building Code 2010
Truss Criteria: FBC2010Res/TPI-2007(STD)

Engineering Software: Alpine Software, Version 12.03.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 130 MPH ASCE 7-10 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-14015EC1-GBLLETIN-

#	Ref Description	Drawing#	Date
1	61373A1 28'8" Commo	n 13169001	06/18/13
2	61374A2 28'8" Comm	n 13169002	06/18/13
3	61375B1 39'4" Commo	n 13169003	06/18/13
4	61376B2 39'4" Commo	n 13169004	06/18/13
5	61377C1 27' Common	13169005	06/18/13
6	61378DGA 28'8" Gabl	e 13169006	06/18/13
7	61379-DGB1 39'4" Gabi	e 13169007	06/18/13
8	61380DGC 27' Gable	13169008	06/18/13

William H. Krick -Truss Design Engineer-

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LIAM

1950 Marley Drive Haines City, FL 33844

13



Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

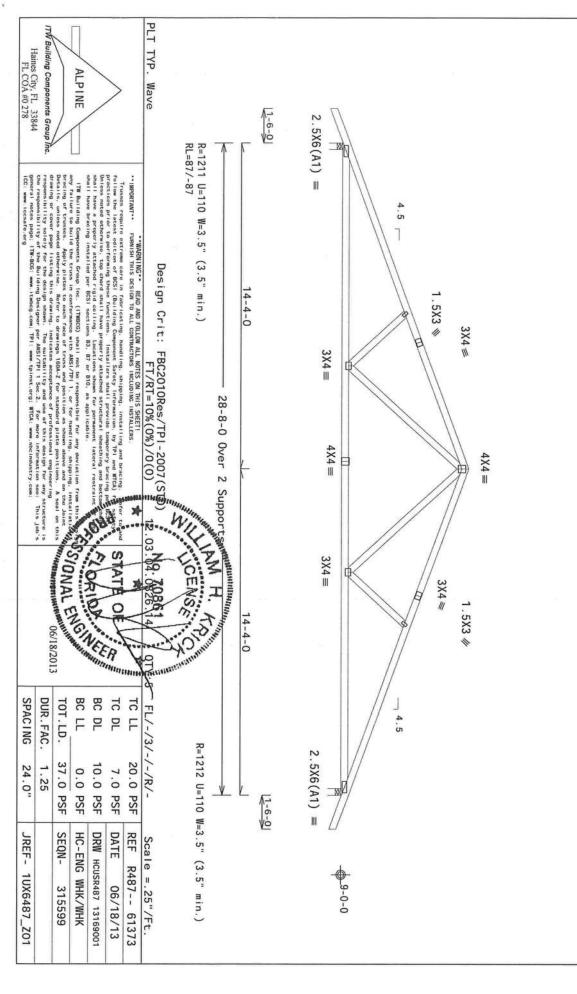
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

Truss passed check for 20 psf additional bottom chord live load in areas with 42° -high x 24° -wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



(13-189--OWNER BUILDER Beasley Residence -- 265 SW Beasley CT Columbia County, F - A2 28'8" Common)

Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having Jurisdiction, the building designer and the project owner.

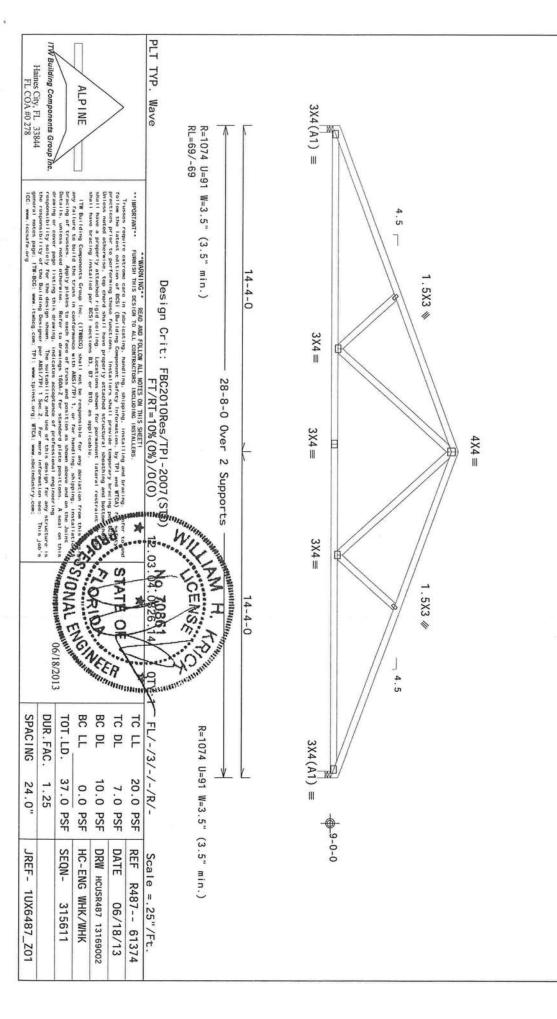
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



(13-189--OWNER BUILDER Beasley Residence -- 265 SW Beasley CT Columbia County, F - B1 39'4" Common)

Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

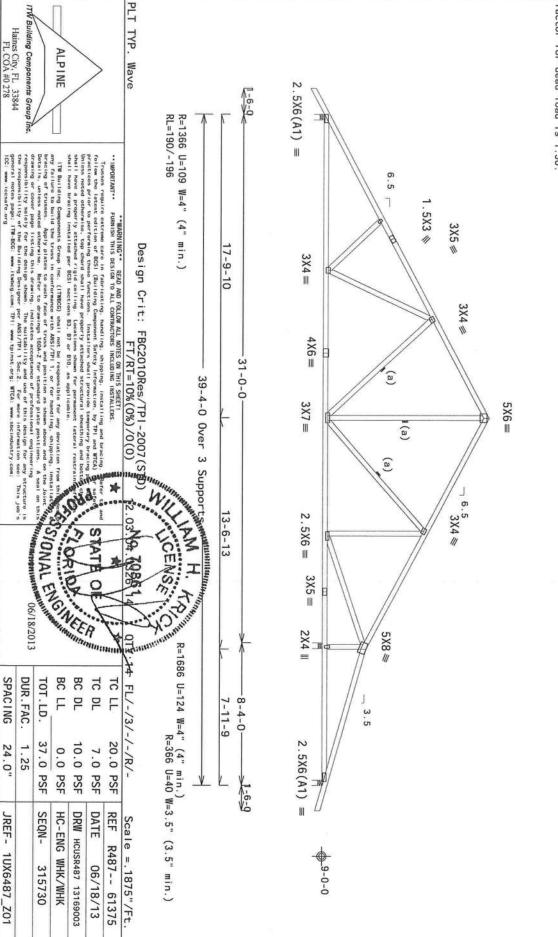
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42° -high x 24° -wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.



Top chord 2x4 SP #1_12A Bot chord 2x4 SP #1_12A Webs 2x4 SP #3_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18

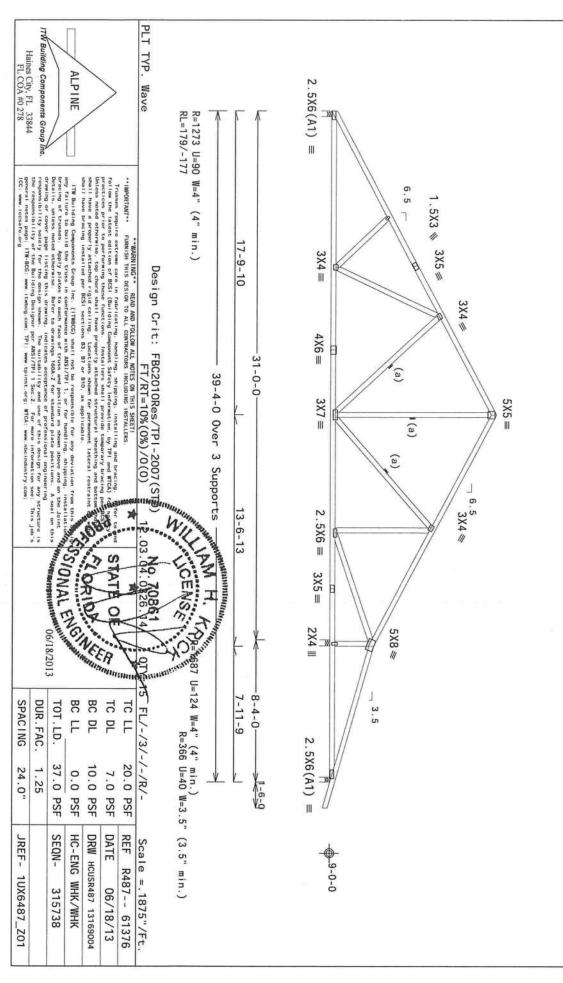
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42° -high x 24° -wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

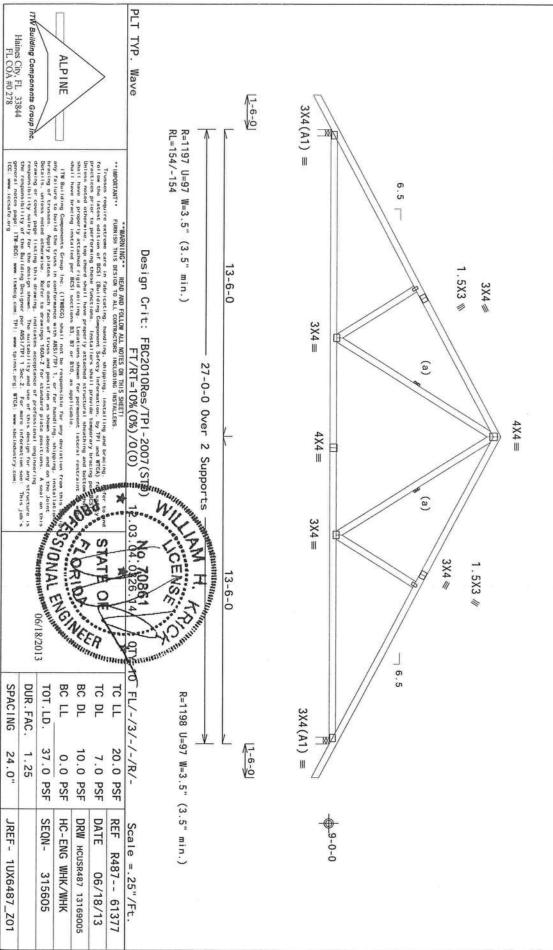
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.



Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3_12A :Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved $1/5/2012\,$ by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BL=5.0 psf. GCpi(+/-)=0.18

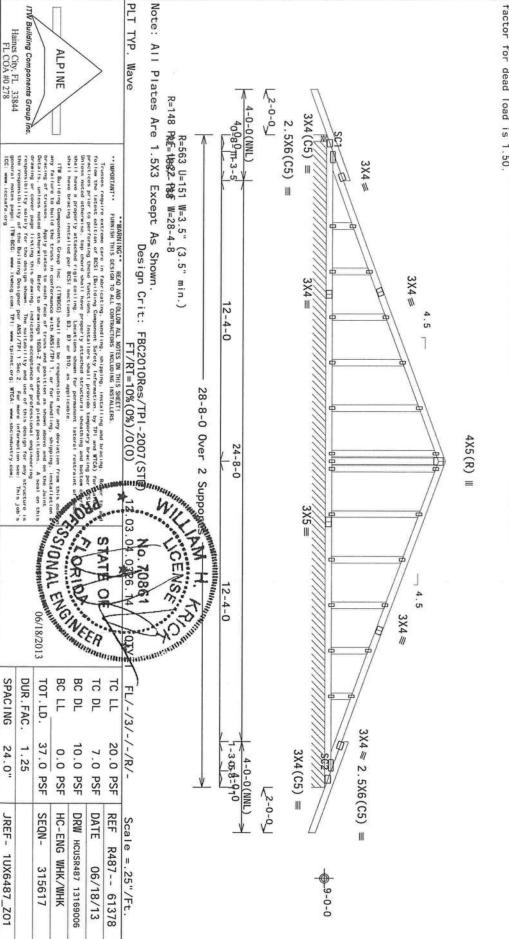
Wind loads and reactions based on MWFRS with additional C&C member

Truss spaced at 24.0" OC designed to support 2-0-0 top chord must outlookers. Cladding load shall not exceed $10.00\ PSF$. Top chord must

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements

not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area



(13-189--OWNER BUILDER Beasley Residence -- 265 SW Beasley CT Columbia County, F - DGB1 39'4" Gable) THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP_#1_12A Bot chord 2x4 SP_#1_12A Webs 2x4 SP_#3_12A :Stack Chord SC1_2x4 SP_#1_12A::Stack Chord SC2_2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project

(a) Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10.00 psf non-concurrent live load

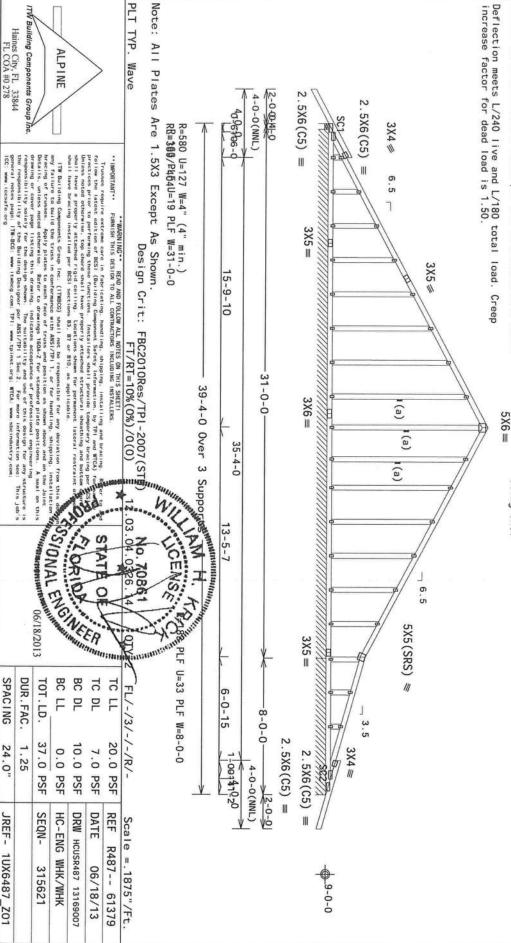
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area (SC)



Top chord 2x4 SP_#1_12A

Bot chord 2x4 SP_#1_12A

Webs 2x4 SP_#3_12A

:Stack Chord SC1_2x4 SP_#1_12A::Stack Chord SC2_2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved $1/5/2012\,$ by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

4X4 ≡

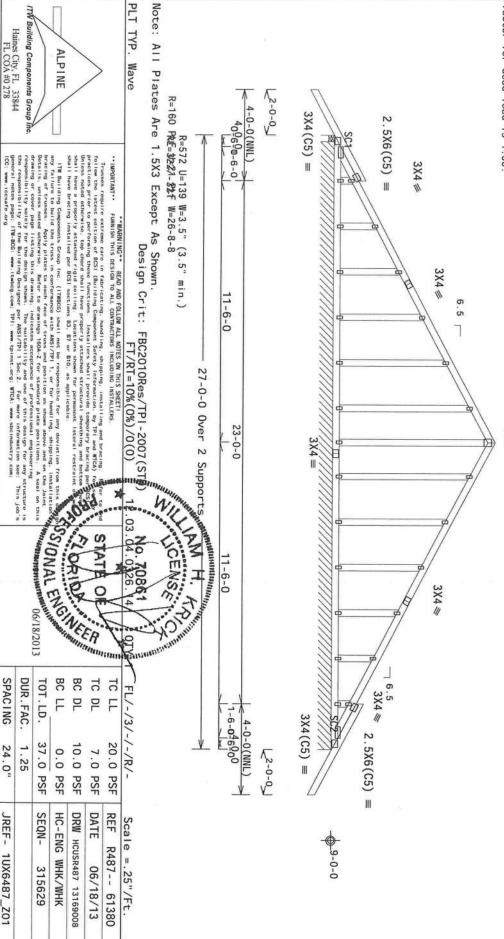
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attack stacked top chord (SC to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



₹EB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NUTES

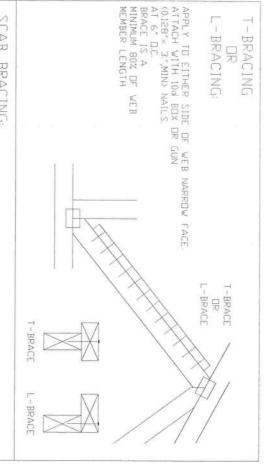
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB

FOR MINIMUM ALTERNATIVE BRACING, ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. RE-RUN DESIGN WITH APPROPRIATE

N N X X 00 00	9 X X 2	2X3 DR 2X4 2X3 DR 2X4	WEB MEMBER
D ROW	1 ROW	4 1 ROW	R SPECIFIED CLB
	2 ROWS	2 ROWS	BRACING
9X8 5X8	2X4 2X6	2×4	T DR L-BRACE
(¥,9x2−2	1-2×6	1-2×4	ATIVE BRACING
8x2−1	2-2×4(*)	2-2×4	SCAB BRACE

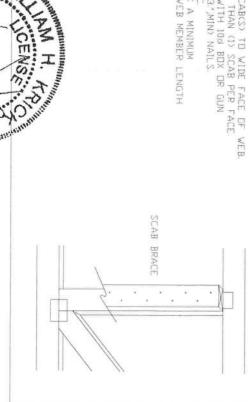
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

CENTER SCAB ON WIDE FACE OF FACE OF WEB. WEB. APPLY (1) SCAB TO EACH



SCAB BRACING:

APPLY SCABOS) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BDX OR GUN
COLEB'* 3",MIN) NAILS.
AT 6" DC. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH





***APRING** READ AND FILIDY ALL NOTES: DID THIS SHEET.

Privace or require instrume come in femonic thing, honding, ablighing, witstalling and Bracking. Bet follow IEEE (Idadeng Component: Suffery Indicates Sund). By and victor for safety proctices the performing these share thorse. Installand, while the entry shared per BEEE Makess and thereines, top chood shall have properly installand share that they are properly in the properly in the properly installand in the properly installand. The properly installand in the properly installand in the properly installand in the properly installand.

ANDERSALT FRENCHE CIDE DE HES DESIGN DE NESTALATION CONTRACTION

ITY Building Comments Group Inc. (ITYSE) and ITY INSTITUTATION FOR May deviation from the
design, only failure to build the truss of MEDIC or nest the **Planessille for any deviation, skilling instituting at breaking of trusses. ITYBEIC commences that are now of 2018/1964 AV JASAN AS
grade 37/40/60 (KYN/HAS) gain steel Apply plates to each force of truss, positioned as sense.

and on Joint [Intalia.

A seal on this drawing on cover page indicates acceptance and professional engineering responsibility and use of this component lesson shown. The suitability and use of this component for any building is the responsibility of the Building Desponsibility and use of this component for any building is the responsibility of the Building Desponsibility and use of this component for any ITV-IEC www.thecg.com/ITF was thirst.com/ITCA was elected strycom/IEC was essentially

Earth City, MO 63045

BC P. Jun 18 '13

DUR. FAC SPACING TOT. LD P PL PSF PSF PSF PSF DATE DRWG BRCLBSUB0109 60/1/1 CLB SUBST

vertical length may be doubled when diagonal brace is used Connect diagonal brace for 450% at each end Max web total length is 14: Length Vertical Max Gable Earth City, MO 63045 Vertical length shown in table above. Burnod Econnect diagonal at 16" O.C. Galole RU X In Species Grade SPF IN PH Vertical 30 ASCE 7-10: Standard Standard standard Standard 5# / [# #1 / #2 Stud Stud Stud Brace 7 #2 Trustes require extreme care in flabricating, handling, shapping, installing and draking, see folios the latest edition of BCSI (Building Component Solety Information, by IPI, and MTCA) proctices prior to performing these functions. Installers shall provide temperary bridges unless noted otherwise; top chand shall have properly attached structural shapping and chand shall have a properly attached the rigid calling. Lacetions shape for permanent latest of webs shall have properly attached structural shapping and of webs shall have macing installed per BCSI sections \$3, \$7 or \$10, as applicable. Apply each face of in trusts and positions as shapen above and on the Joint Betalls, unless nated of Refer to drawings 160A-2 for standard plate positions. ITW Building Companents Group Inc. shall not be responsible for any deviation from this drawing, any column to build the trues in conformance with ANSI/PI I, or for housing, shipping, installation is bracing of trueses. A scall on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility, callely for the design shown. The satisfity and use of this drawing for only structure is the responsibility of the Building Designer per ANSI/TPI I Sec2 for more information see this jubis general notes page and these was sites: **!WPORTANI ** FURNISH THIS DRAWING TO ALL WOTES ON THIS DRAWING THE INSTALLERS. Coble Truss Braces 4 6 No 140 better diagonal brace, single or double cut (as shown) ot aroup (1) 1x4 "L" Brace * (1) 2x4 "L" Brace * (2) 2x4 "L" Brace ** (1) 2x6 "L" Brace * (2) 2x6 "L" Brace 9999999 2x4 DF-L #2 or 300 D 555 Group B Wind 6666 440 œ œ œ œ 화 전 호 호 120 mph Wind Speed, 15' Mean Height, Partially Enclased, Exposure C, Kzt = 100 120 mph Wind Speed, 15' Mean Height, Enclased, Exposure D, Kzt = 1.00 100 mph Wind Speed, 15' Mean Height, Partially Enclased, Exposure D, Kzt = 1.00 Lakle Group A Speed 5555 ο φφφ 0 0 0 ø 00 = & & & 5 = = 8 8 5 Stud Group B 0 0 0 0 0 Ú Reinforcement Group A Group B Group A Group B Group A Mean Height, SSIONAL ENGINEER MANAGEMENT 4444 Z, j 5 Enclosed, 0 0 0 0 0 0000 0 Detail Jun 18 '13 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 Exposure 999999 Group 0 14' 0" TOT. LD. SPACING w Refer to the Building Designer for conditions not addressed by this detail. Provide uplift connections for 55 pif over continuous bearing (5 psf 76 Bead Load) "L" bracing must be a minimum of 80% of web member length. So Pine lumber design values based on the ALSC January, 2012 rulin Sable and supports load from 4' 0' outlookers with 2' 0' overhang, or 12' plywood overhang ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group values may be used with these grades. # For (1) "L" brock space nails at 2" a.c. in 18" end zones and 4" ac between zones. Attach "L" braces with 10d (0.128"x3.0" min) noils Wind Load deflection criterian is L/240 1x4 Braces shall be SRB (Stress-Rated Board) Bracing Group Species and Grades #1 / #2 Standard kFor (2) "L" braces space nails at 3" ac. in 18" and zones and 6" ac between zones Douglas fir-Larch 60 Gable Truss Detail Notes: X N t Refer to common truss design for Greather than 11' 6" Greater than 4' 0", but less than 11' 6" juglas Fir-Larch 24.0 peak, splice, and heel plates Vertical Length Standard Gable Vertical Plate Sizes PSF H DATE DRWG A14015ENC100212 #1 % Btr Group A Group 2/14/12 ASCE 7-10-GAB14015 W outhern Pinexxx outhern Pine*** No Splice 1x4 or 2x3 Standard 2.5x4 3×4 Stud Standard

