

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



FRONT ELEVATION
SCALE: 1/4" = 1'



REAR ELEVATION
SCALE: 1/4" = 1'



March 22, 2013

D.D.S. Studios
D.D.S. STUDIOS
P.O. Box 273
Lake City, FL 32056
(904) 784-0181

A CUSTOM HOME BY ADAM'S CONSTRUCTION

DUARTE RESIDENCE

COPYRIGHT: DDS STUDIOS INC. 2012

Daniel Shaheen
Daniel Shaheen

EXTERIOR ELEVATIONS

SHEET NUMBER
1 of 7

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

PROJECT NUMBER
010708

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

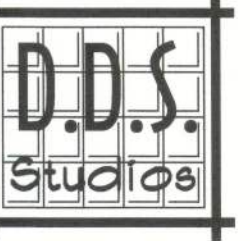


RIGHT ELEVATION
SCALE: 1/4" = 1'



LEFT ELEVATION
SCALE: 1/4" = 1'

March 22, 2013



D.D.S. STUDIOS
P.O. Box 273
Lake City FL 32056
(904) 784-0181

A CUSTOM HOME BY ADAM'S CONSTRUCTION

DUARTE RESIDENCE

COPYRIGHT: DDS STUDIOS INC. 2012

Daniel Shaheen
Daniel Shaheen

EXTERIOR ELEVATIONS

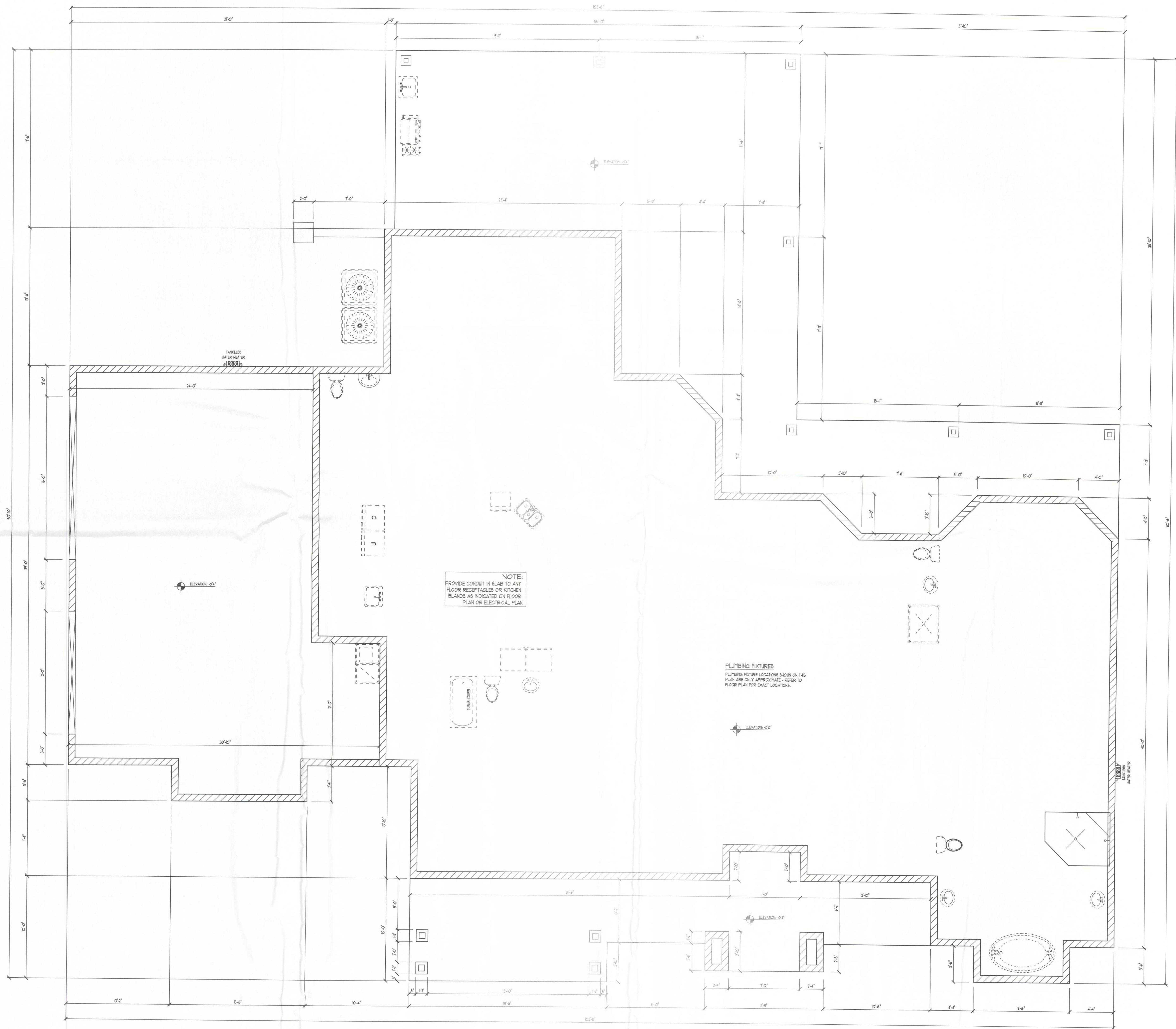
SHEET NUMBER
2 of 7

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

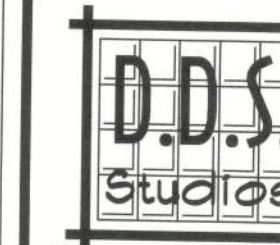
PROJECT NUMBER
010708

ALL DRAWINGS NOT TO BE SCALED. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



FOUNDATION PLAN
SCALE: 1/4" = 1'

March 22, 2013



D.D.S. STUDIOS
P.O. Box 273
Lake City, FL 32056
(904) 754-0181

A CUSTOM HOME BY ADAM'S CONSTRUCTION

DUARTE RESIDENCE

COPYRIGHT: DDS STUDIOS INC. 2012

Daniel Shaheen
Daniel Shaheen

FOUNDATION PLAN

SHEET NUMBER
3 of 7

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

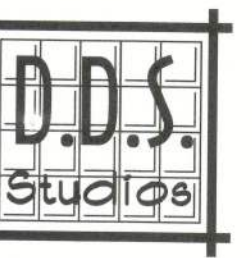
PROJECT NUMBER
010708

PROJECT NUMBER	010708
----------------	--------



ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

March 22, 2013



D.D.S. STUDIOS
P.O. Box 273
Lake City FL, 32056
(904) 754-0181

A CUSTOM HOME BY ADAM'S CONSTRUCTION:

DUARTE RESIDENCE

COPYRIGHT: DVD STUDIOS INC., 2012

Daniel Shaheen

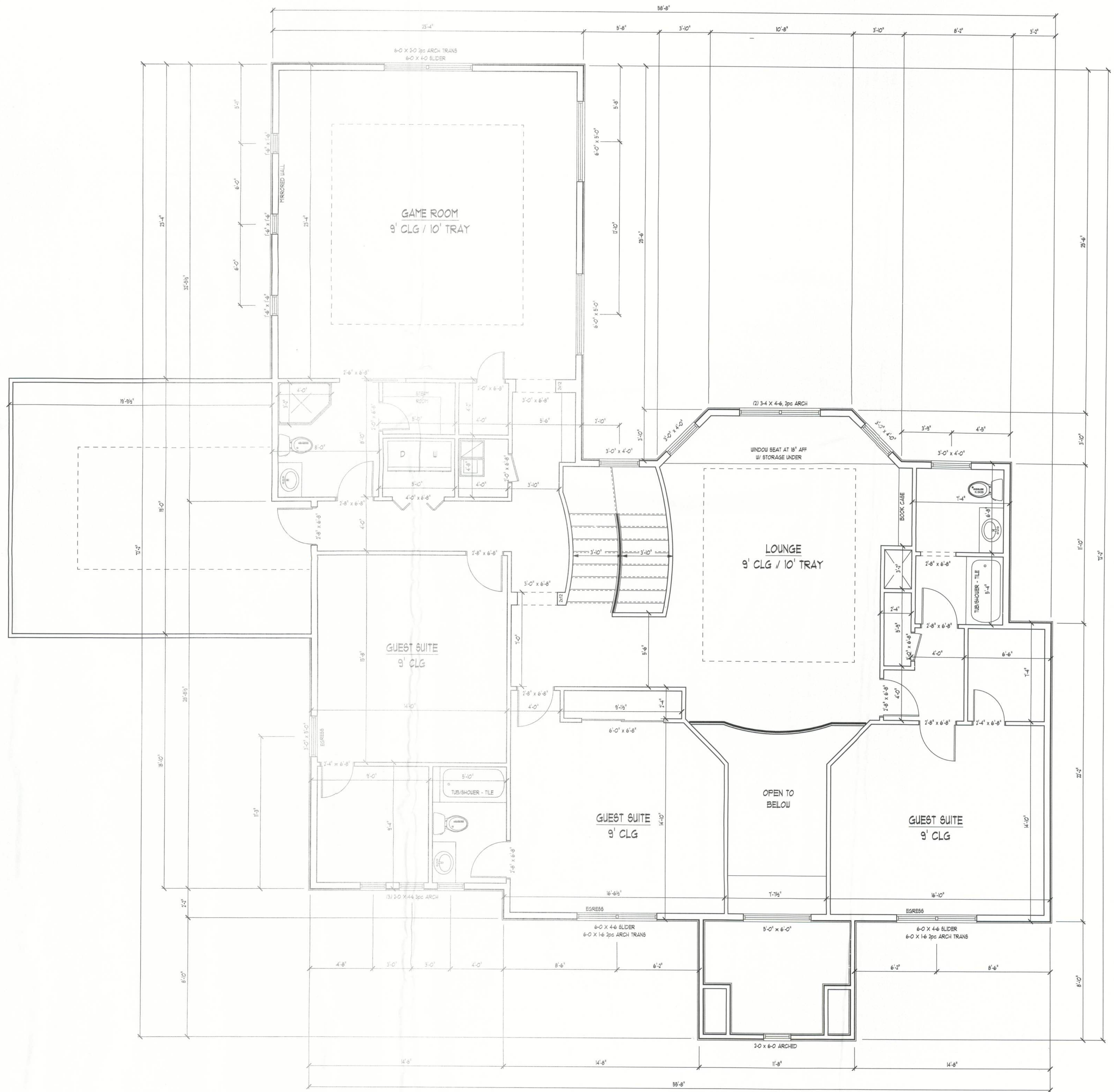
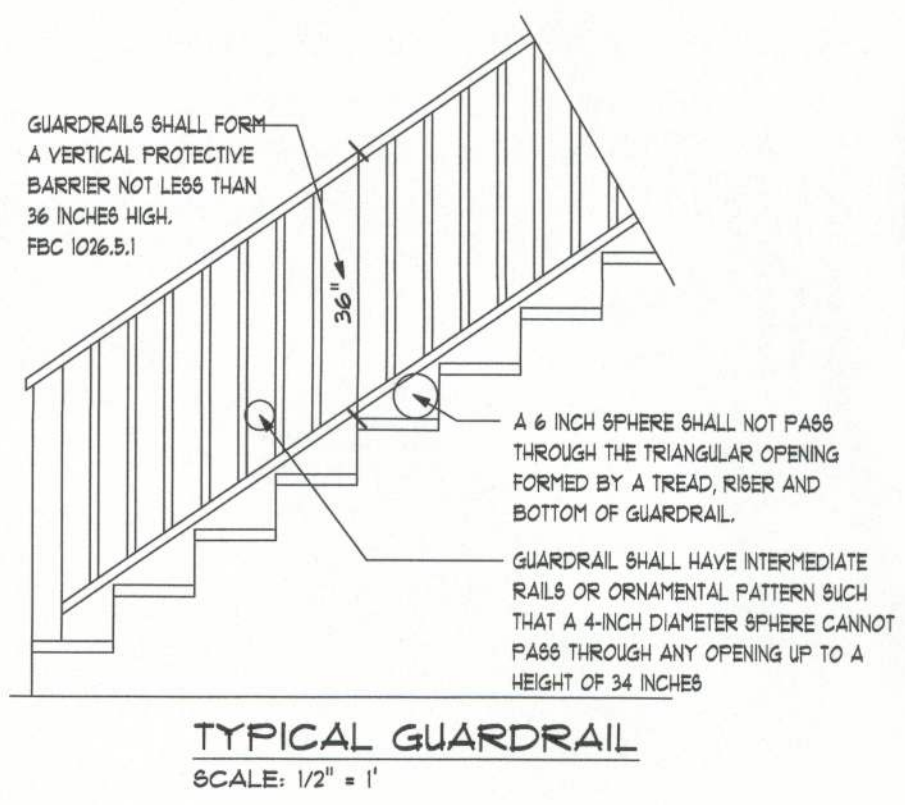
2nd FLOOR PLAN

SHEET NUMBER
5 of 7

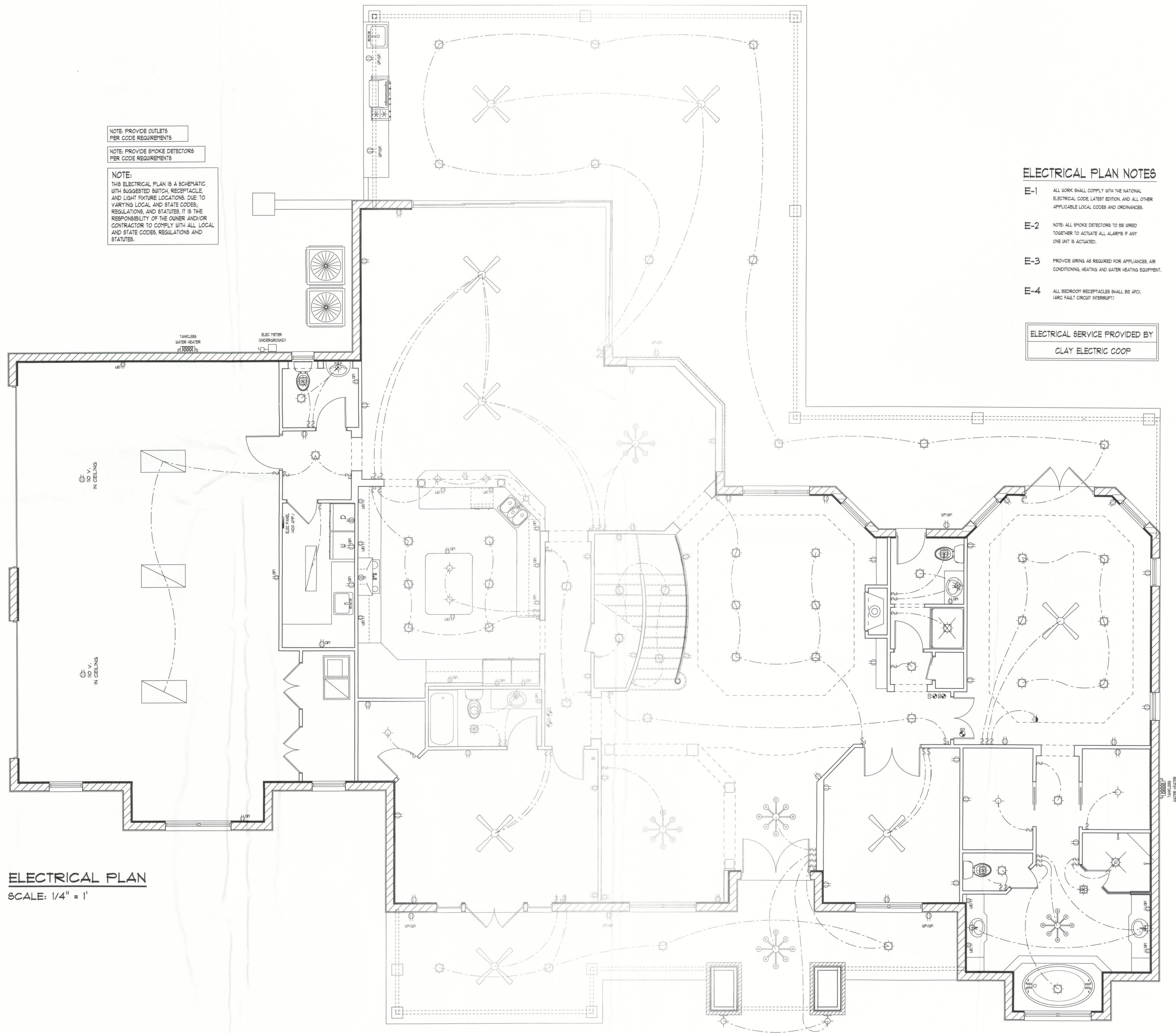
All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

PROJECT NUMBER
010708



ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



NOTE: PROVIDE OUTLETS
PER CODE REQUIREMENTS

NOTE: PROVIDE SMOKE DETECTORS
PER CODE REQUIREMENTS

NOTE:
THIS ELECTRICAL PLAN IS A SCHEMATIC
WITH SUGGESTED SWITCH, RECEPTACLE,
AND LIGHT FIXTURE LOCATIONS. DUE TO
VARYING LOCAL AND STATE CODES,
REGULATIONS, AND STATUTES, IT IS THE
RESPONSIBILITY OF THE OWNER AND/OR
CONTRACTOR TO COMPLY WITH ALL LOCAL
AND STATE CODES, REGULATIONS AND
STATUTES.

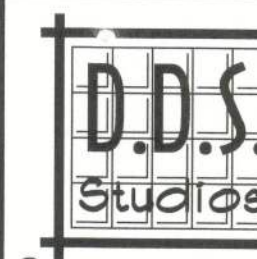
ELECTRICAL PLAN NOTES

- E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL
ELECTRICAL CODE, LATEST EDITION, AND ALL OTHER
APPLICABLE LOCAL CODES AND ORDINANCES.
- E-2 NOTE: ALL SMOKE DETECTORS TO BE WIRED
TOGETHER TO ACTUATE ALL ALARMS IF ANY
ONE UNIT IS ACTUATED.
- E-3 PROVIDE WIRING AS REQUIRED FOR APPLIANCES, AIR
CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.
- E-4 ALL BEDROOM RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT)

ELECTRICAL SERVICE PROVIDED BY
CLAY ELECTRIC COOP

ELECTRICAL PLAN
SCALE: 1/4" = 1'

March 22, 2013



D.D.S. STUDIOS
P.O. Box 273
Lake City FL 32056
(904) 784-0181

A CUSTOM HOME BY ADAM'S CONSTRUCTION

DUARTE RESIDENCE

COPYRIGHT: DDS STUDIOS INC. 2012

Daniel Shaheen
Daniel Shaheen

ELECTRICAL PLAN

SHEET NUMBER
6 of 7

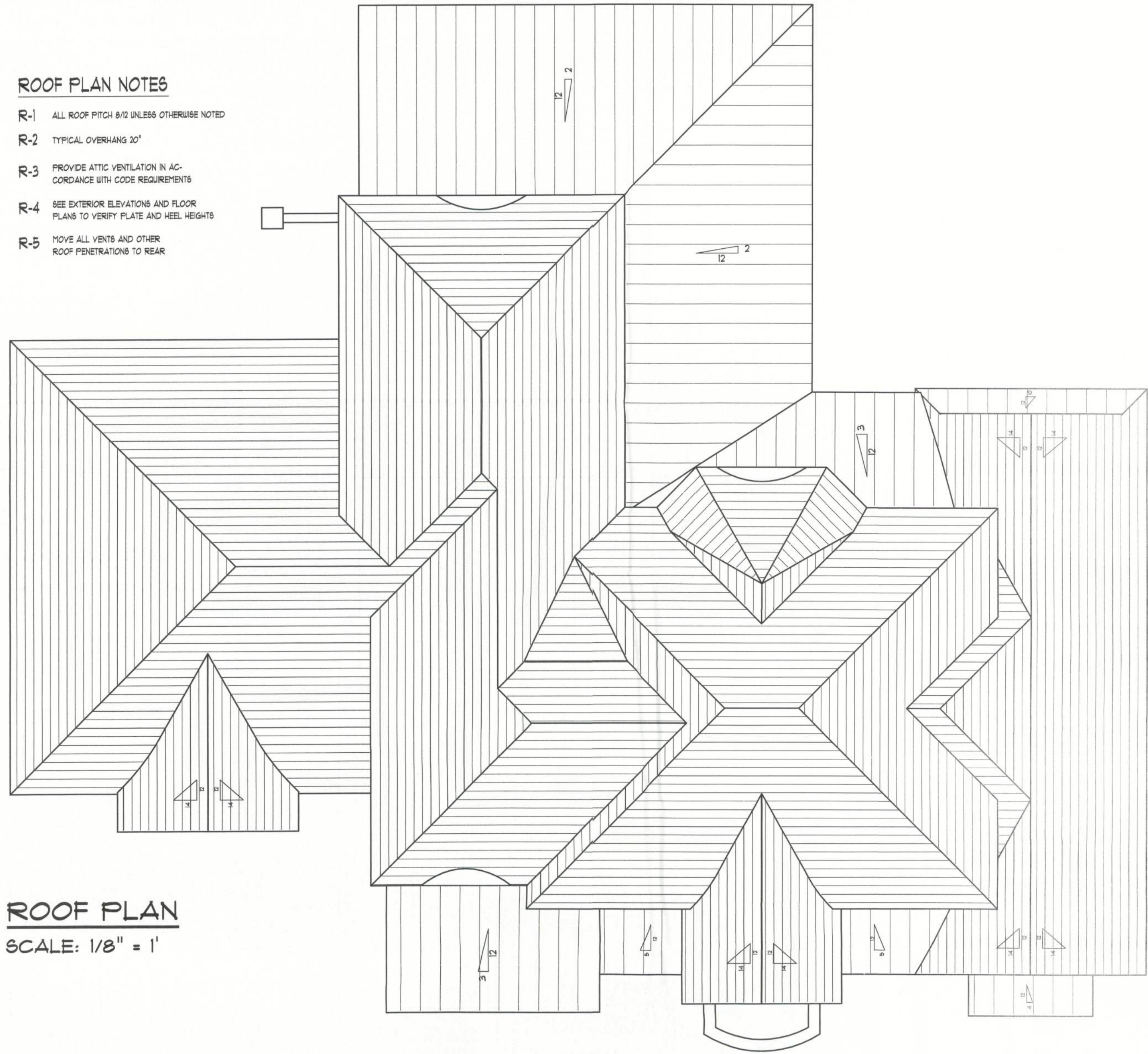
All work shall comply with
the standard building code,
and all applicable local
codes and ordinances.
Contractor shall verify all
dimensions prior to
commencing construction.

PROJECT NUMBER
010708

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

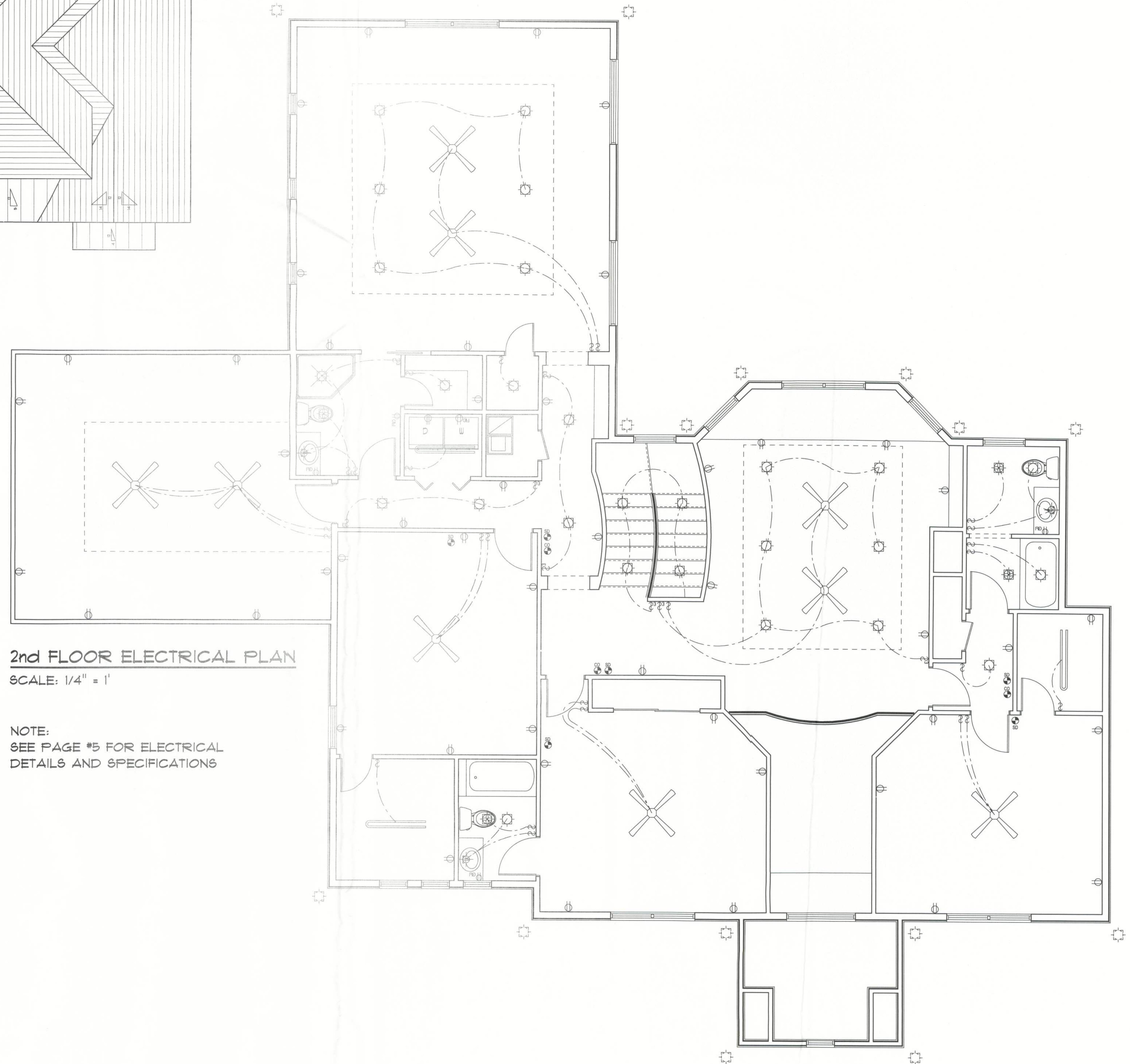
- ROOF PLAN NOTES**
- R-1 ALL ROOF PITCH 8/12 UNLESS OTHERWISE NOTED
 - R-2 TYPICAL OVERHANG 30"
 - R-3 PROVIDE ATTIC VENTILATION IN ACCORDANCE WITH CODE REQUIREMENTS
 - R-4 SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY FLATE AND RISE HEIGHTS
 - R-5 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR

ROOF PLAN
SCALE: 1/8" = 1'

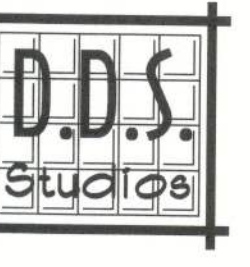


2nd FLOOR ELECTRICAL PLAN
SCALE: 1/4" = 1'

NOTE:
SEE PAGE #5 FOR ELECTRICAL
DETAILS AND SPECIFICATIONS



March 22, 2013



D.D.S. STUDIOS
P.O. Box 273
Lake City, FL 32056
(904) 784-0181

A CUSTOM HOME BY ADAM'S CONSTRUCTION

DUARTE RESIDENCE

COPYRIGHT: D.D.S. STUDIOS INC. 2012

Daniel Shaheen
Daniel Shaheen

ELECTRICAL PLAN

ROOF PLAN

SHEET NUMBER
1 of 1

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

PROJECT NUMBER
010708

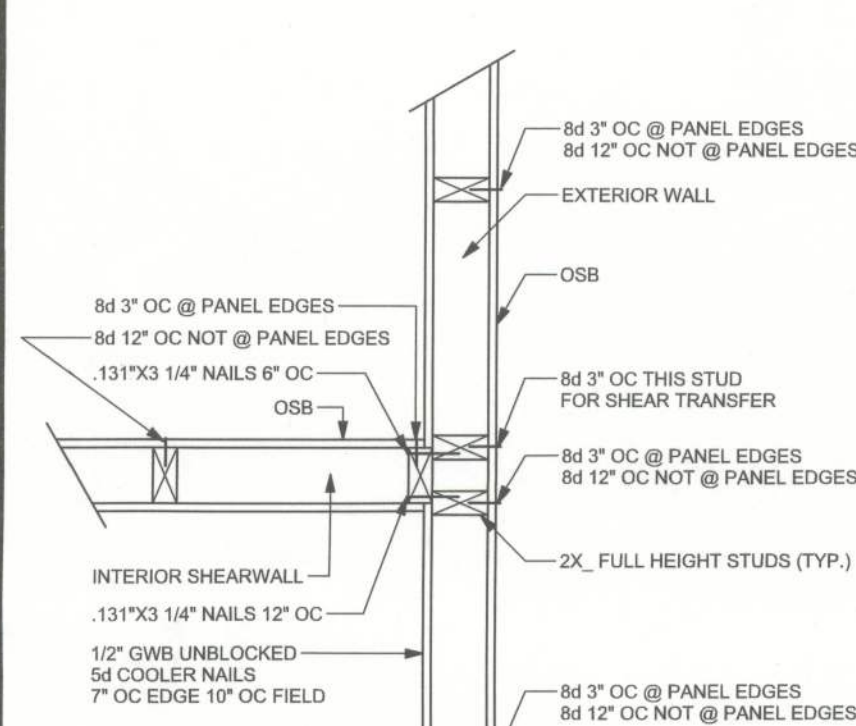
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

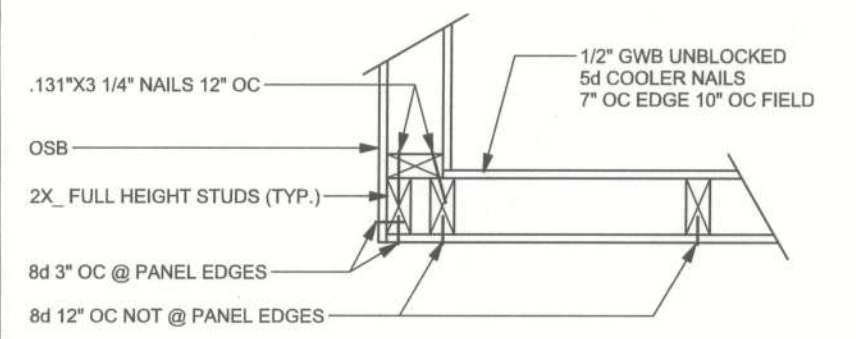
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

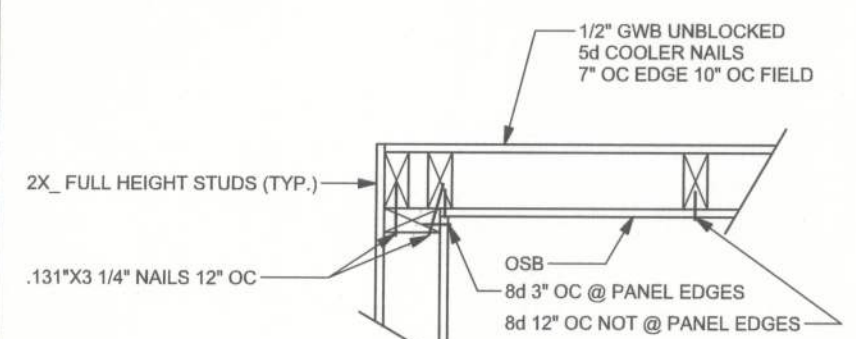
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.209A, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR SOME BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)



(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



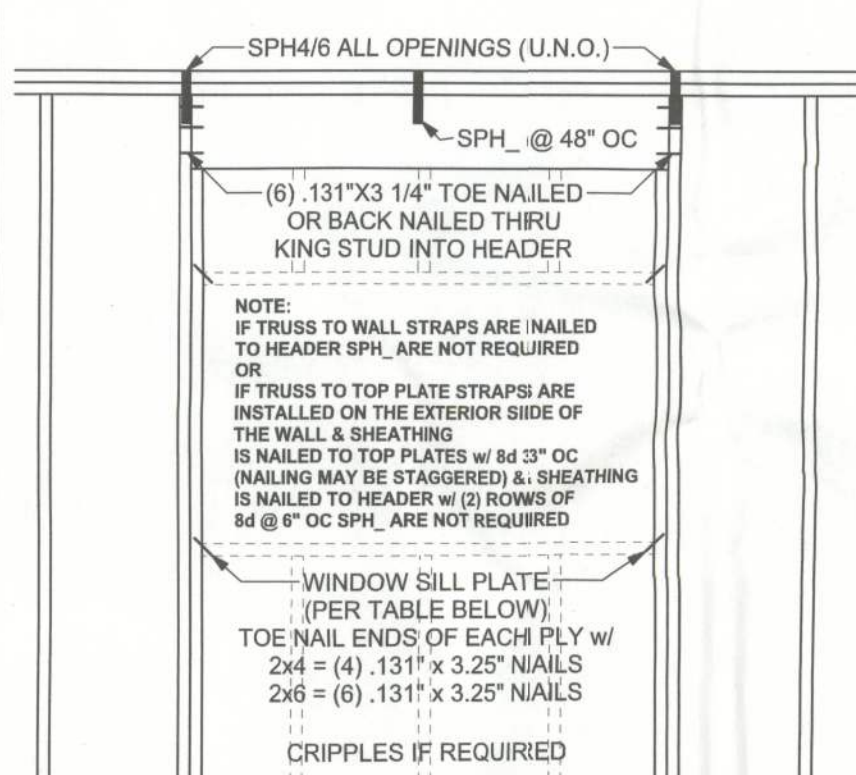
OUTSIDE CORNER



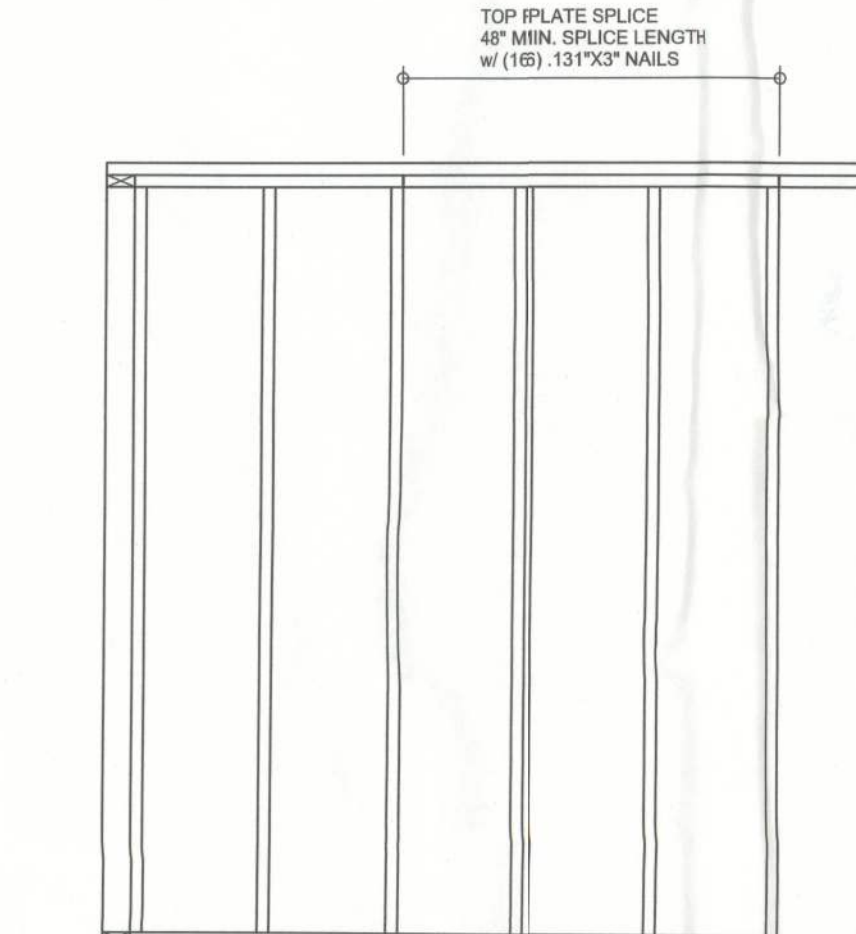
INSIDE CORNER



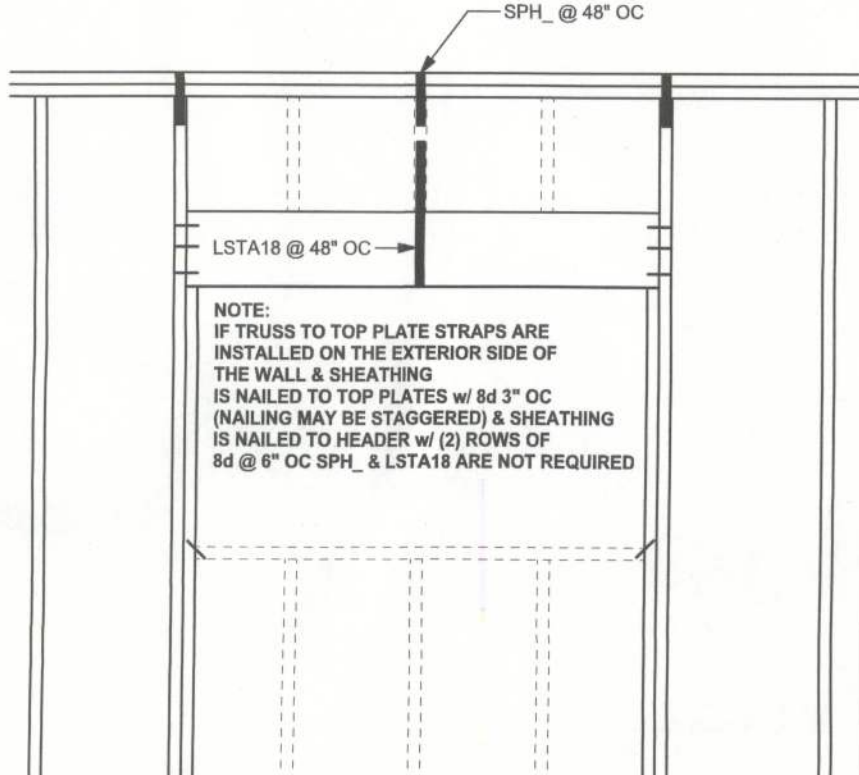
(TYP.) CORNER FRAMING WOOD FRAME



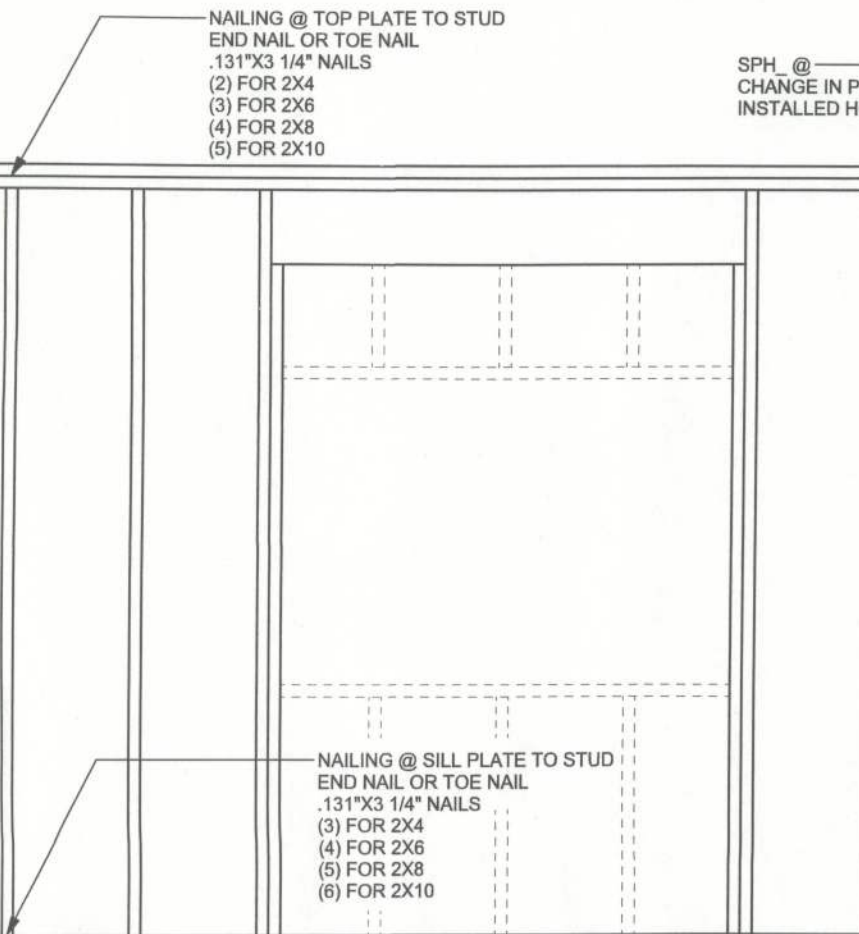
TYPICAL HEADER STRAPPING DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



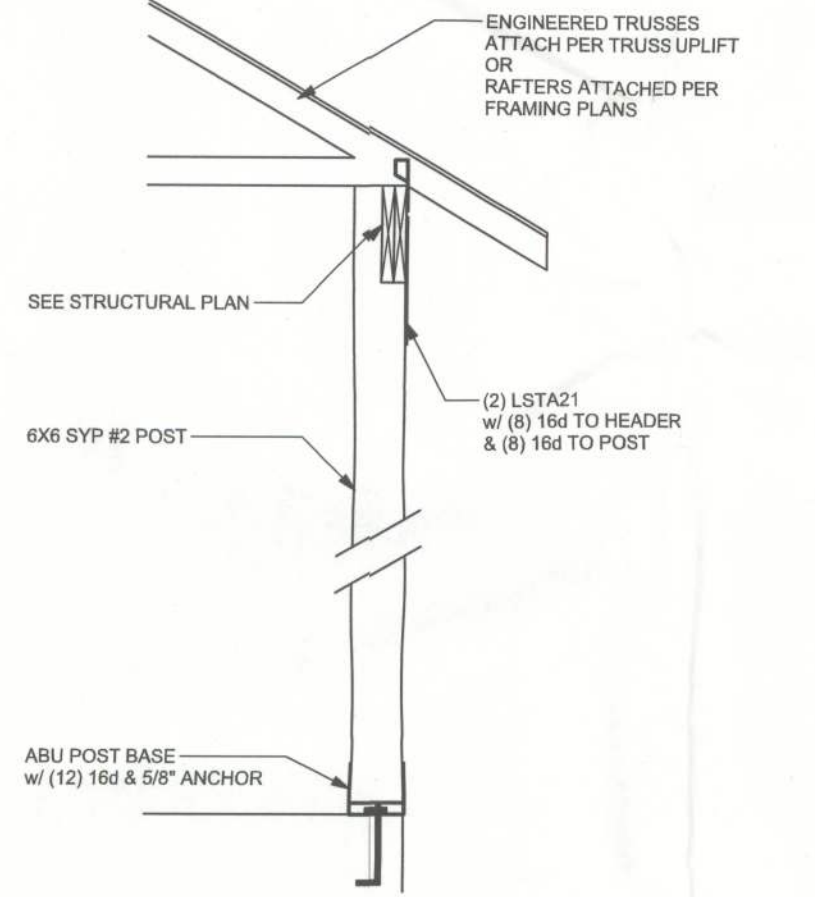
(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



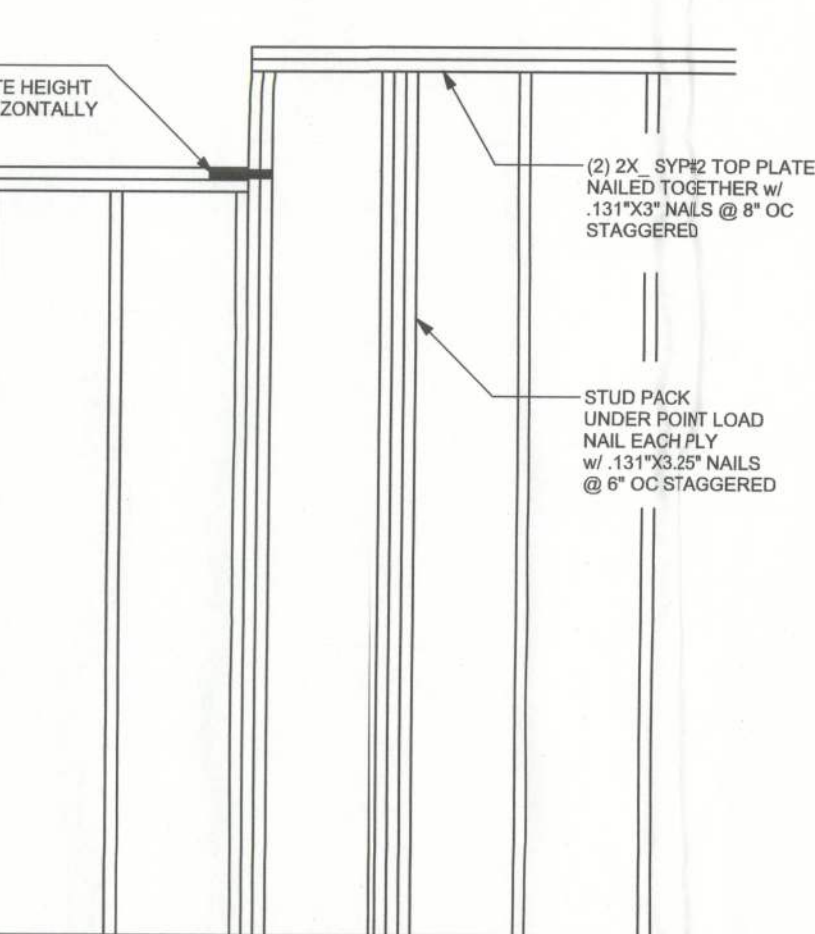
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	(1) 2x4	(2) 2x4	(1) 2x6	(1) 2x6	BASED ON WFCM TABLE A-3.23B
115-130 MPH	5'-3"	7'-9"	7'-9"	11'-4"	FOR OTHER WALL HEIGHTS (R) SILL SPAN SHALL BE DIVIDED BY (R/10)
140-150 MPH	4'-4"	6'-6"	6'-6"	9'-6"	
160 MPH	4'-0"	6'-0"	6'-11"	8'-9"	



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



(TYP.) PORCH POST ONE STORY WOOD



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



TYPICAL BOND BEAM CORNER

CMU
SCALE: 1/2" = 1'-0"

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU
2 1/2" MIN. EDGE DISTANCE
1 1/4" MIN. EMBEDMENT
3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 14" O.C.
1/4" TAPCONS @ 21" O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 9" O.C.
1/4" TAPCONS @ 14" O.C.

SLIDERS UP TO 8'X20'W

3/16" TAPCONS @ 7" O.C.
1/4" TAPCONS @ 10" O.C.

GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS @ 8" O.C.
(2) 1/4" TAPCONS @ 14" O.C.

GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS @ 4" O.C.
(2) 1/4" TAPCONS @ 7" O.C.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	380	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LT812-LT820	1000	820					6-10d x 1 1/2"	6-10d x 1 1/2"
MT812-MT830	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HT816-HT830	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEDOWNS								TO FOUNDATION
LG2	2050	1785	700	170	700	170	14-16d	14-16d
LG23-SDS2.5	3685	2655	795	410	795	410	12-8d6 1/4" x 2 1/2"	26-16d6
LG24-SDS3	4050	3680	2000	675	2000	675	12-SDS 1/4" x 3"	36-16d6
MGT	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6465					16-10d	2-5/8" ANCHOR
HGT-3	10630	6035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								TO STUDS
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
SSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
SSP SINGLE SILL PLATE	825	800					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	
SPH4	1240	1065					10-10d x 1 1/2"	
SP6	885	760					6-10d x 1 1/2"	
SPH6	1240	1065					10-10d x 1 1/2"	
LSTA18	1235	1110					14-10d	
LSTA21	1235	1235					16-10d	
CS20	1030	1030					14-10d	
CS16	1705	1705					22-10d	
STUD ANCHORS								TO STUDS
LTT19	1350	1305					8-16d	12" ANCHOR
LTT131	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					16-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR

- (1) w/ INSTALLATION OF 4-16d6 OPTIONAL NAIL HOLES
- (2) FOR SYP GIRDER & SPF STUDS

MASONRY TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	HETA20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8" AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4" AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4" AB TO WALL 15" EMBEDMENT

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft ² or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBRC. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE TRUSS DESIGNER. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 41LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 4000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/ WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, UNO.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: 3-10T ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/64", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBRC REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBRC, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBRC REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1	
ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)	
2) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY	
3) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIF	
4) RISK CATEGORY = II, (MRI = 700 YR)	
5) ROOF ANGLE = 7.45 DEGREES	
6) MEAN ROOF HEIGHT = <30 FT	
7) INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)	
8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	

Zone	Effective Wind Area (ft ²)
1	10

MATERIALS

1. Fc 8" precast lintel = 3500 psi
2. Fc prestressed lintel = 6000 psi
3. Rebar per ASTM C475 Fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

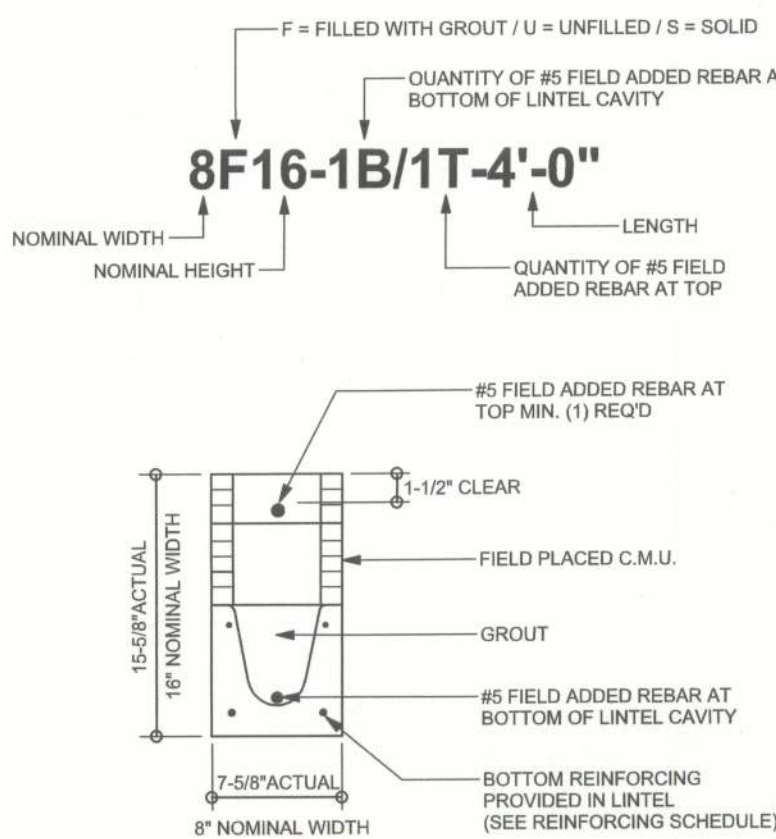
1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8' meet or exceed L/160 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-298 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

$$\frac{\text{Applied vertical load}}{\text{Safe vertical load}} + \frac{\text{Applied horizontal load}}{\text{Safe horizontal load}} \leq 1.0$$

13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

TYPE DESIGNATION



SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
- Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.

7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown
9. All safe loads in units of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar.

Example 7'-6" lintel type 8F32-1B safe gravity load = 6472(10.048)(15)(10.0781) w/ 15% reduction 6472 = (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH	TYPE	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B
2'-10" (34")	PRECAST	2231	3069	4605	6113	7547	8974	10394
3'-6" (42")	PRECAST	2231	3069	4605	6113	7547	8974	10394
4'-0" (48")	PRECAST	1969	2693	4605	6113	7547	8974	10394
4'-6" (54")	PRECAST	1595	2186	4375	6113	7547	8974	10394
5'-4" (64")	PRECAST	1217	1663	3090	5365	7547	8974	10394
5'-10" (70")	PRECAST	1062	1451	2622	4360	7168	8974	10394
6'-0" (78")	PRECAST	908	1238	2177	3480	5361	8360	10394
7'-6" (90")	PRECAST	743	1011	1729	2681	3988	5681	8467
9'-4" (112")	PRECAST	574	699	1160	1625	2564	3486	4705
10'-6" (126")	PRECAST	455	535	890	1247	2093	2777	3763
11'-4" (136")	PRECAST	362	424	706	1002	1326	1697	2127
12'-0" (144")	PRECAST	337	400	640	873	1254	1684	2193
13'-4" (160")	PRECAST	296	371	550	755	1075	1426	1838
14'-0" (168")	PRECAST	279	342	506	706	1002	1326	1697
14'-8" (176")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR
15'-4" (184")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR
17'-4" (208")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR
19'-4" (232")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR
21'-4" (256")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR
22'-0" (264")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR
24'-0" (288")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

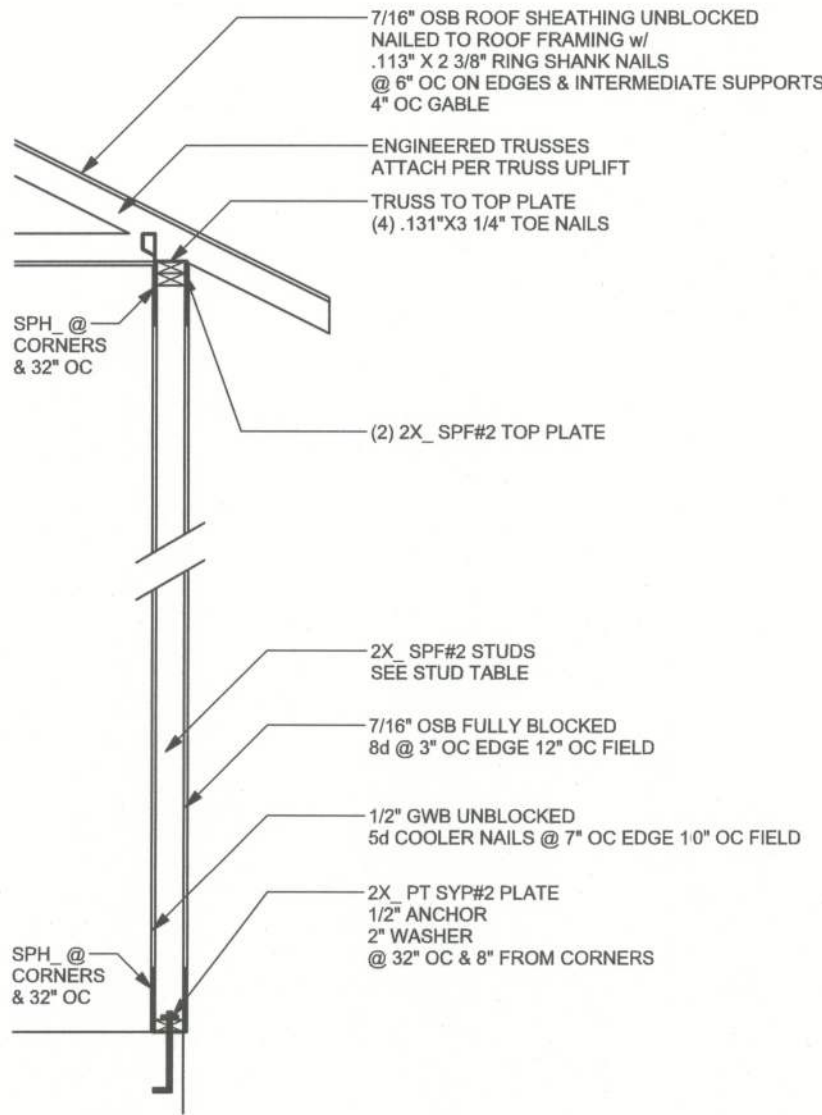
LENGTH	TYPE	8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-1T
2'-10" (34")	PRECAST	1972	3173	4460	5747	7034	8321	9608
3'-6" (42")	PRECAST	1569	2524	3547	4568	5591	6613	7636
4'-0" (48")	PRECAST	1363	2192	3079	3966	4853	5740	6627
4'-6" (54")	PRECAST	1207	1940	2724	3508	4292	5077	5861
5'-4" (64")	PRECAST	1016	1632	2290	2940	3607	4265	4924
5'-10" (70")	PRECAST	909	1492	2093	2694	3295	3897	4498
6'-0" (78")	PRECAST	835	1340	1880	2419	2959	3498	4038
7'-6" (90")	PRECAST	727	1166	1634	2102	2571	3039	3508
9'-4" (112")	PRECAST	591	880	1133	1471	1811	2150	2494
10'-6" (126")	PRECAST	530	852	1141	1459	1782	2107	2427
11'-4" (136")	PRECAST	428	686	945	1204	1464	1724	1983
12'-0" (144")	PRECAST	400	640	873	1125	1376	1627	1878
13'-4" (160")	PRECAST	362	582	795	1002	1209	1416	1623
14'-0" (168")	PRECAST	337	540	732	939	1146	1353	1560
14'-8" (176")	PRESTRESSED	246	390	555	720	885	1050	1215
15'-4" (184")	PRESTRESSED	224	362	506	671	836	1001	1166
17'-4" (208")	PRESTRESSED	187	303	424	545	666	787	908
19'-4" (232")	PRESTRESSED	166	261	362	463	564	665	766
21'-4" (256")	PRESTRESSED	142	230	321	412	503	594	685
22'-0" (264")	PRESTRESSED	137	221	304	385	466	547	628
24'-0" (288")	PRESTRESSED	124	175	247	318	389	460	531

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

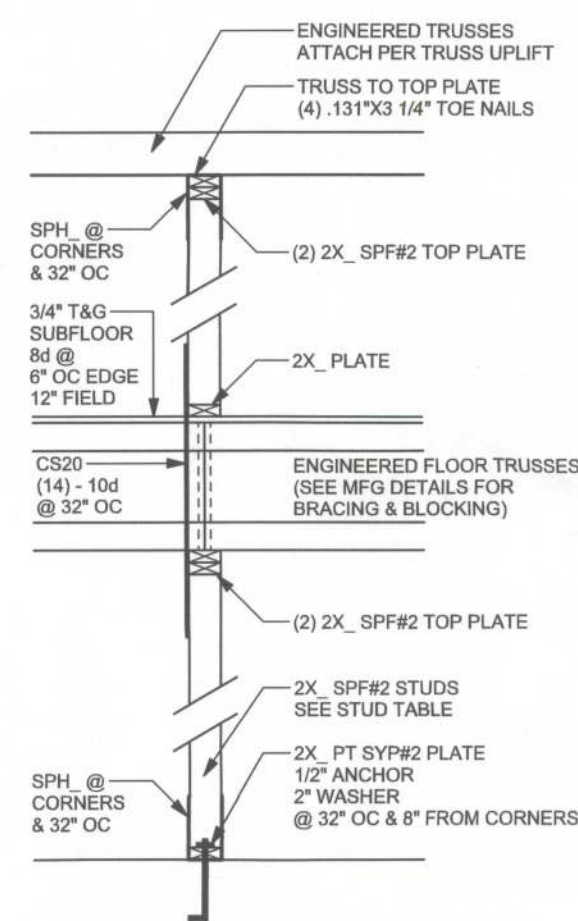
LENGTH	TYPE	8R16-0B	8R16-1B	8R16-2B	8R16-3B	8R16-4B	8R16-5B	8R16-6B
4'-4" (52")	PRECAST	1635	1749	3355	3280	4349	5421	6493
4'-6" (54")	PRECAST	1494	1881	3699	3206	4639	5600	6571
5'-8" (68")	PRECAST	866	1756	3699	3206	4639	5600	6571
5'-10" (70")	PRECAST	810	1113	2342	4242	6639	8060	9479
6'-8" (80")	PRECAST	797	901	1825	3120	5046	7747	9448
7'-6" (90")	PRECAST	669	755	1490	2459	3776	5743	7239
9'-8" (116")	PRECAST	411	466	999	1568	2253	3129	4091

SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

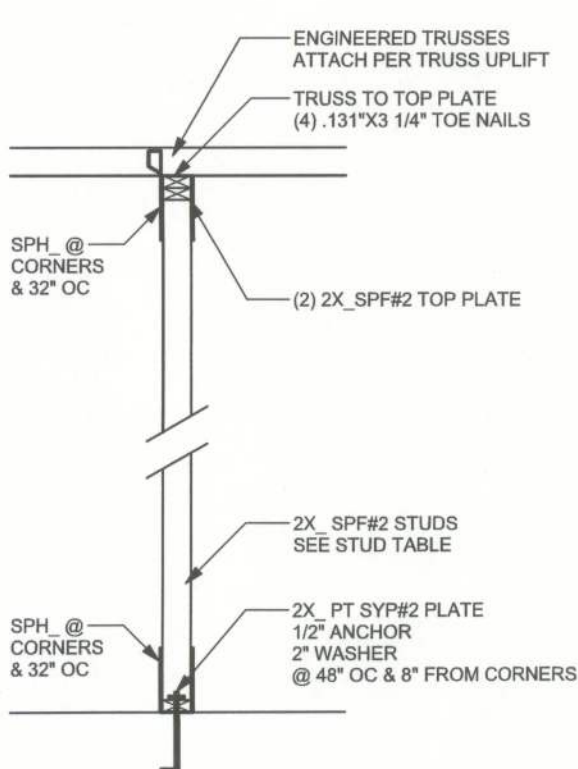
LENGTH	TYPE	8R16-1T	8R16-2T	8R16-3T	8R16-4T	8R16-5T	8R16-6T	8R16-7T
4'-4" (52")	PRECAST	905	1748	2635	3522	4409	5296	6183
4'-6" (54")	PRECAST	867	1675	2525	3374	4224	5074	5924
5'-8" (68")	PRECAST	867	1675	2525	3374	4224	5074	5924
5'-10" (70")	PRECAST	655	1262	1900	2538	3176	3815	4453
6'-8" (80")	PRECAST	570	1012	1651	2204	2758	3312	3865
7'-6" (90")	PRECAST	506	797	1462	2195	2927	3659	4391
9'-8" (116")	PRECAST	395	491	831	1301	1840	2380	2920



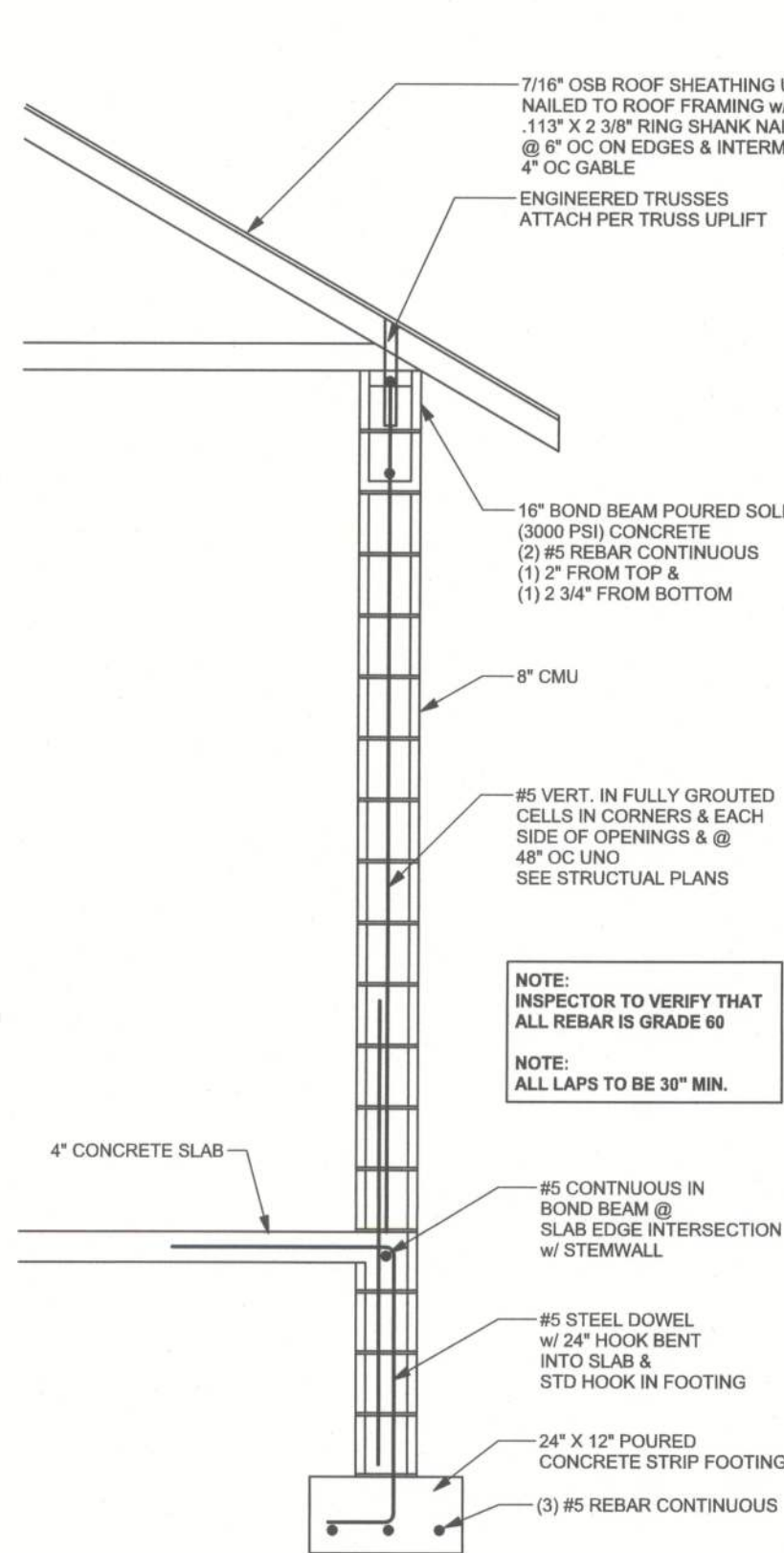
(TYP.) EXTERIOR WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



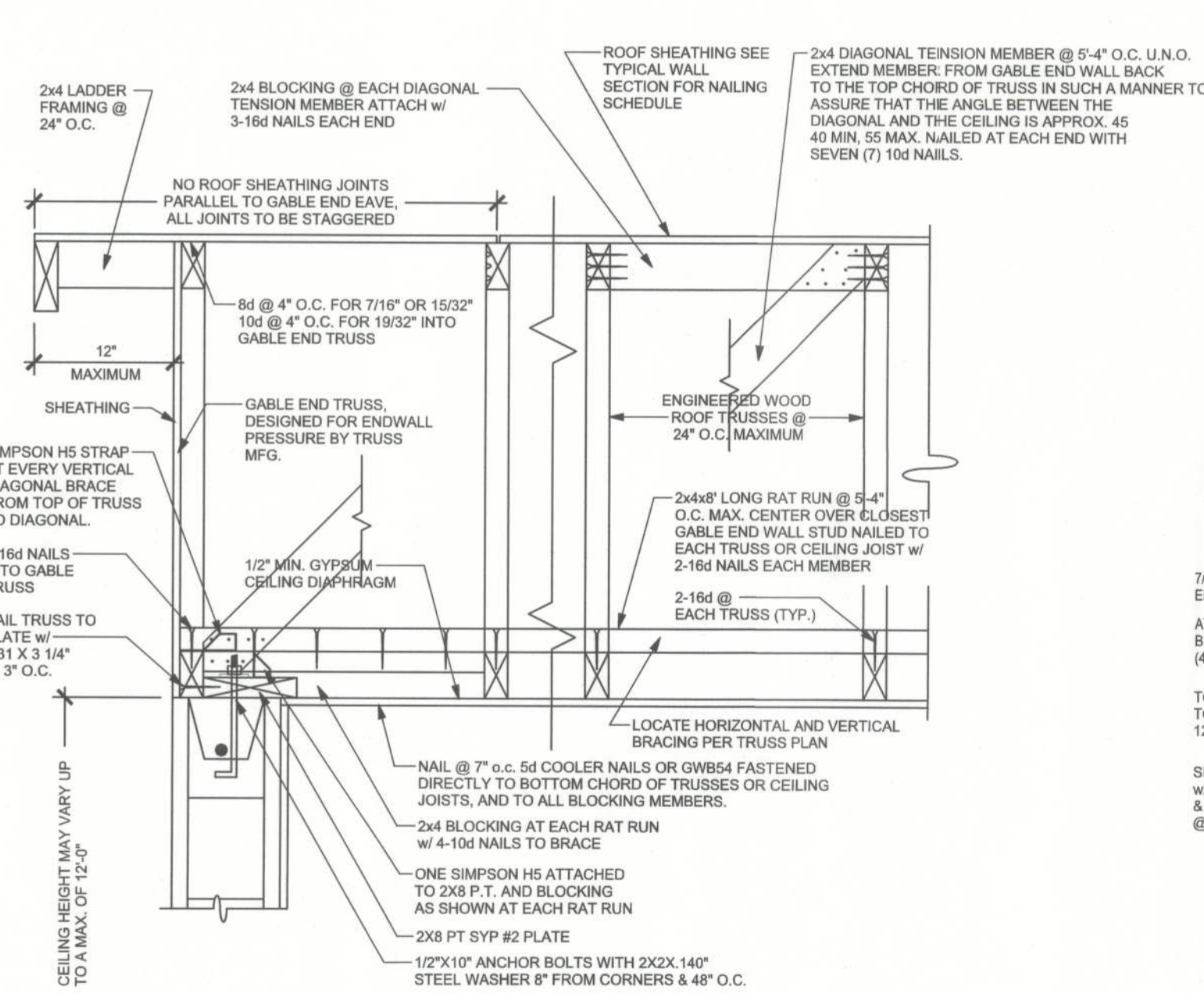
(TYP.) INTERIOR BEARING WALL
TWO STORY WOOD FRAME w/ STRAPS & ANCHORS



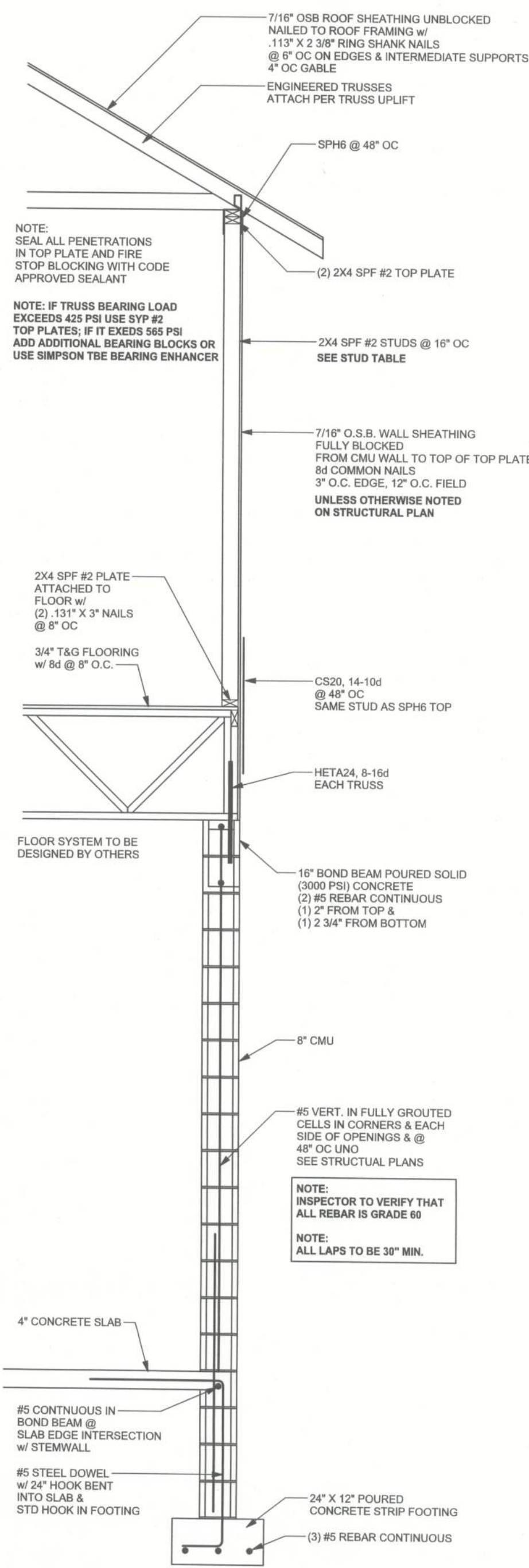
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



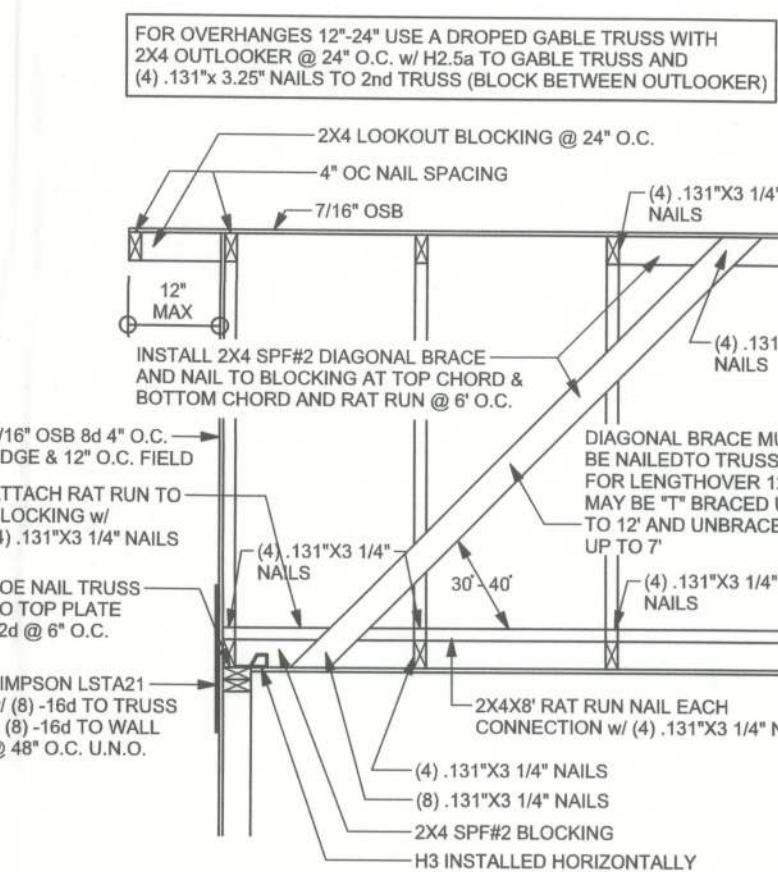
(TYP.) EXTERIOR WALL
ONE STORY CMU
SCALE: 1/2" = 1'-0"



GABLE END DETAIL (CMU)



(TYP.) EXTERIOR WALL
TWO STORY CMU / FRAME
SCALE: 1/2" = 1'-0"



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL
WOOD FRAME

REVISIONS

NO.	DESCRIPTION	DATE

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
PE No. 63915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disoway, P.E. hereby expressly reserves
its common law copyrights and property right in
these instruments of service. This document is
not to be reproduced, altered or copied in any
form or manner without the express written
permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section R301.2.1, 2010 Florida
Building Code Residential
to the best of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.



Adam's Construction
& Design

Duarte Residence

ADDRESS:
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
April 27, 2013

DRAWN BY: STRUCTURAL BY:

FINALS DATE:
27Apr13

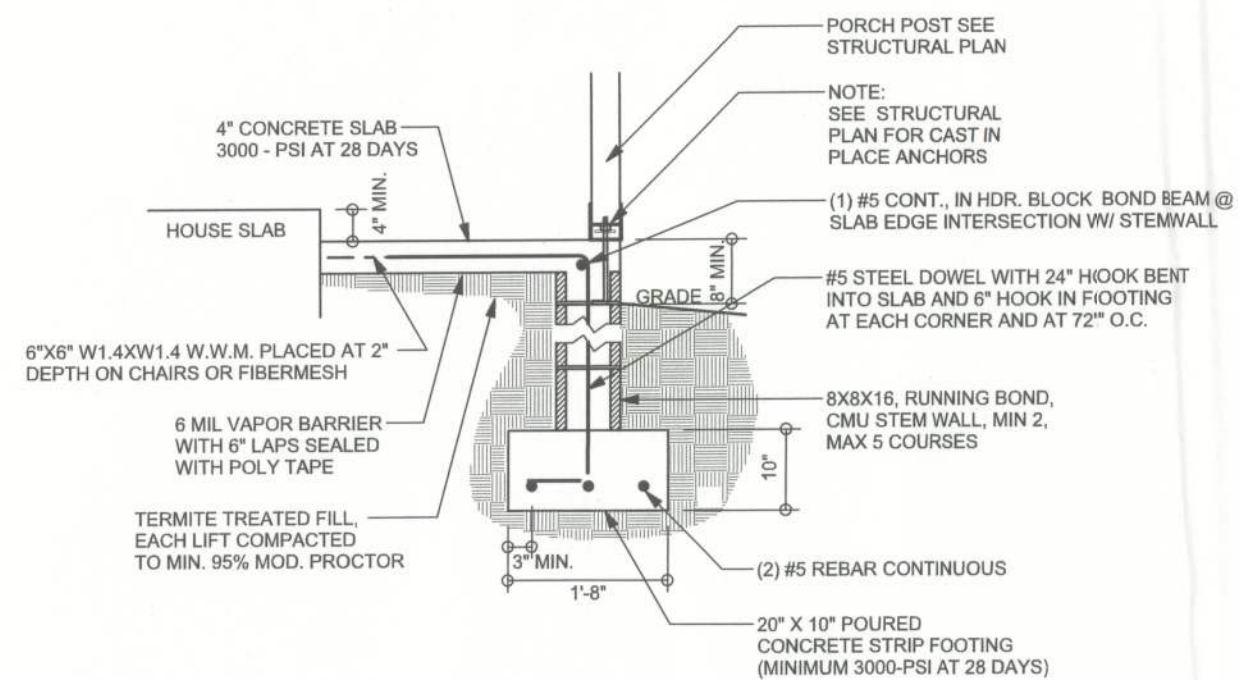
JOB NUMBER:
1304015

DRAWING NUMBER
S-1.1

OF 5 SHEETS

The table assumes 60 ksi reinforcing bars with # hook in the footing and bent 24" into the reinforcing slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure), within the extension of the wall. If the wall is over 8' high, add downward lateral reinforcement at 16"OC vertically on a horizontal bond with 186g, continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement shown in the table below.

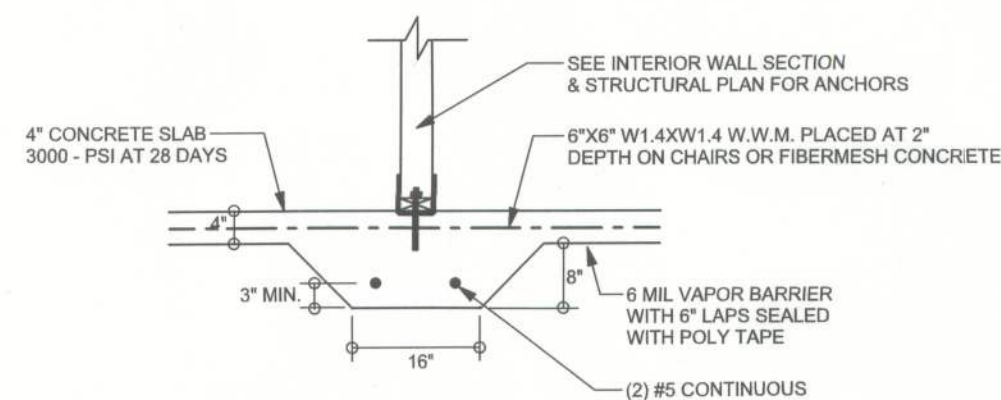
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)				VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)			
		#5	#7	#8		#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	96	
6.0	5.7	40	80	96	80	96	96	96	
6.7	6.3	32	56	80	56	96	96	96	
7.3	7.0	24	40	56	40	80	96	96	
8.0	7.7	16	32	48	32	64	80	96	
8.7	8.3	8	24	32	24	48	64	80	
9.3	9.0	8	16	24	16	40	48	64	



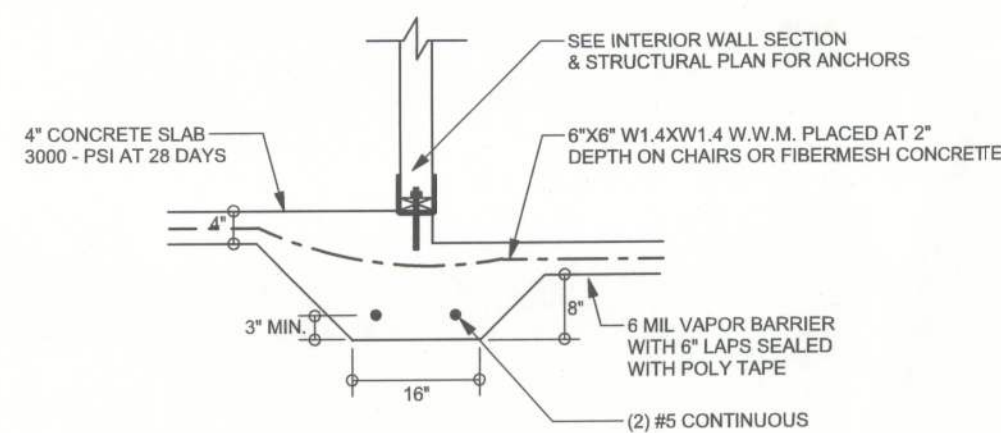
F1
S-2

STEM WALL PORCH FOOTING

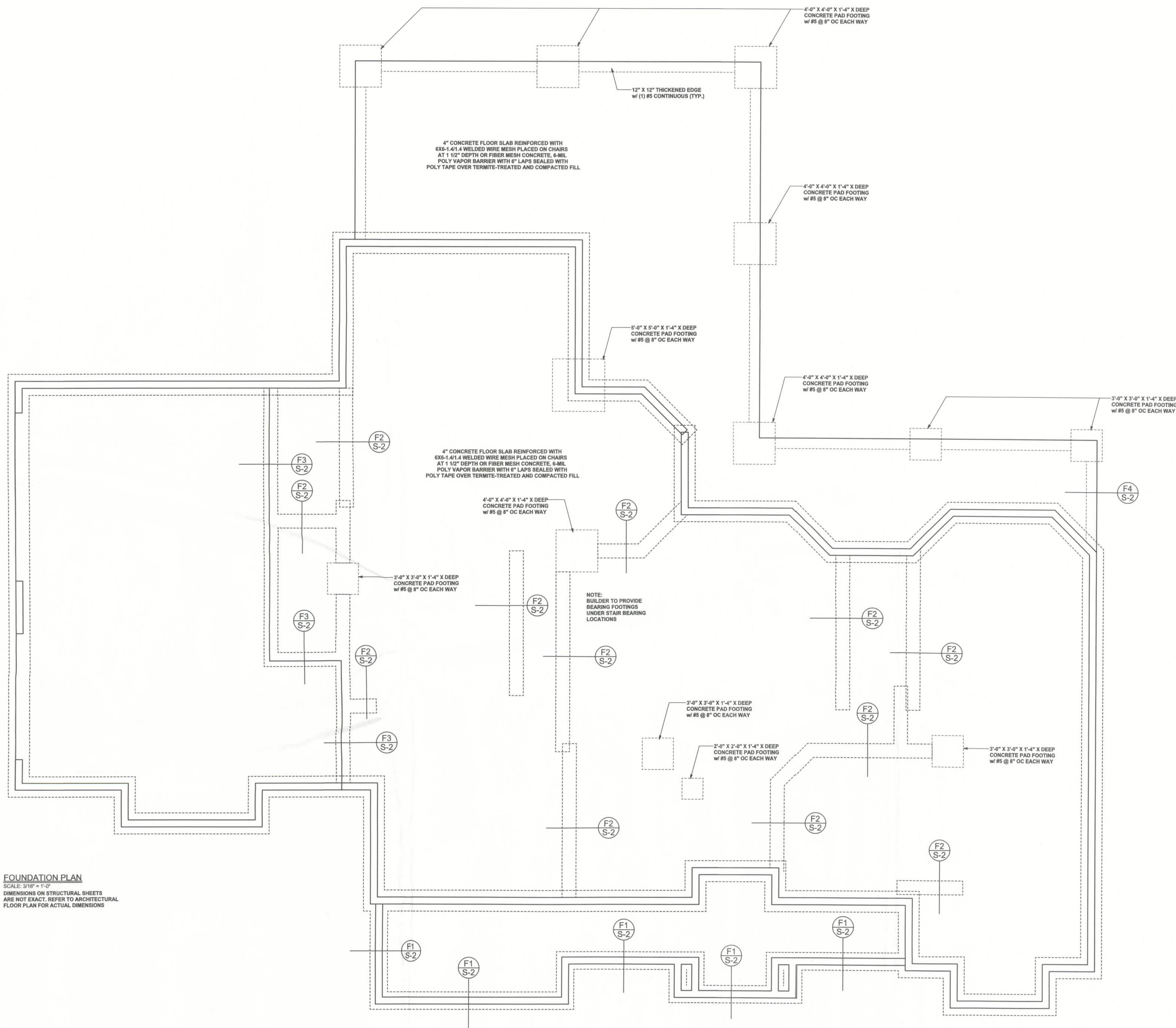
SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 3/16" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

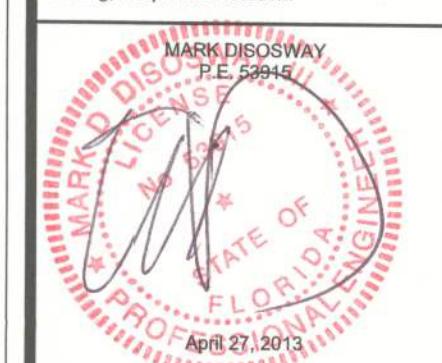
WINDLOAD ENGINEER: Mark Disosway,
PE No.53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Adam's Construction
& Design

Duarte Residence

ADDRESS:
Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
April 27, 2013

DRAWN BY:	STRUCTURAL BY:
FINALS DATE: 27Apr13	

JOB NUMBER:
1304015

DRAWING NUMBER

S-2
OF 5 SHEETS



SN-1	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
SN-2	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
SN-3	DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
SN-4	PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCST1-03, BCST1-01, BCST1-02, & BCST1-03. BCST1-01, BCST1-02, & BCST1-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

Diagram illustrating the components of a header call-out: (2) 2X12X10', 1J 1K.

- HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLYS IN HEADER

	EXTERIOR WALL (CMU)
	EXTERIOR WALL (WOOD FRAME)
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

	REQUIRED	ACTUAL
TRANSVERSE	65.0'	109.0'
LONGITUDINAL	58.0'	112.5'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB# 460382

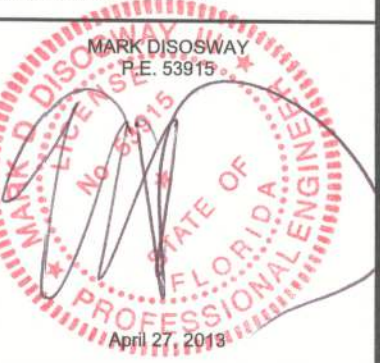
LOAD ENGINEER: Mark Disosway,
No.53915, POB 868, Lake City, FL
56, 386-754-5419

ENSIONS:
ed dimensions supercode scaled
ensions. Refer all questions to
k Disosway, P.E. for resolution.
not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly reserves common law copyrights and property right in the instruments of service. This document is to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable provisions of the plan, relating to wind engineering, comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

NOTATION: This design is valid for one
 day, at specified location.



Duarte Residence

ADDRESS:
Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
April 27, 2013

DRAWN BY:	STRUCTURAL BY:
ISSUANCE DATE: 27Apr13	

JOB NUMBER:
1304015

DRAWING NUMBER

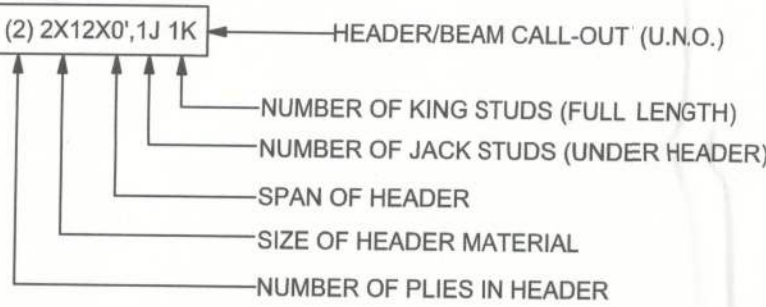
S-3

OF 5 SHEETS

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SPF #2 U.N.O.
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

2ND FLOOR TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	35.0'	117.5'
LONGITUDINAL	30.0'	72.5'

