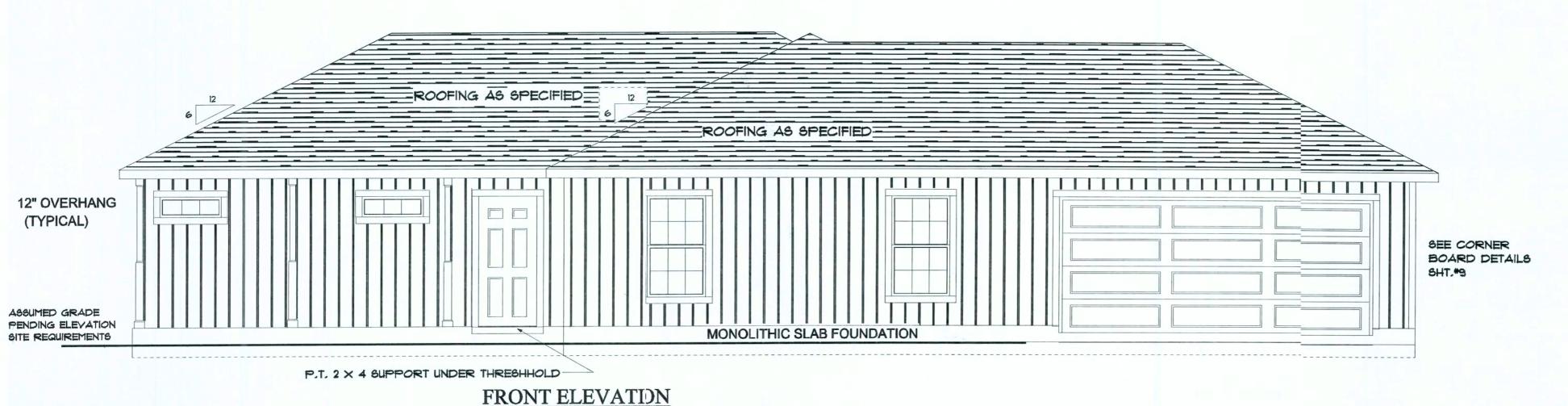
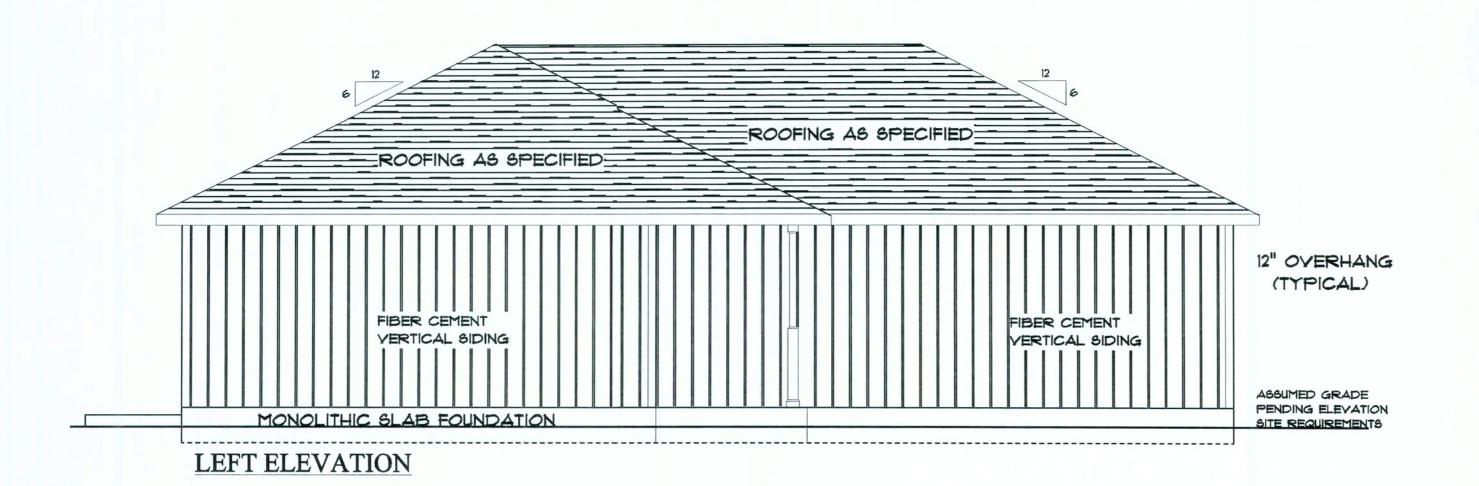
NOTE!:

REFER TO ENGINEERING FOR SPECIFICATIONS AND CALCULATIONS.



OFF RIDGE VENTS ARE REQUIRED
DO NOT PLACE ON FRONT OF HOUSE



SCALE: 1/4" = 1'-0"

DRAWN BY: PLAN DATE: PAGE
BAA July 27, 2006

1

REV.# REV. DATE: DRAWN BY: DISCRIPTION OF REVISION

JOB # 06-04-0053

OF 9

Pennyworth Homes

Got Land? Let's Buildl sc-010738 PennyworthHomes.com FL-CRC05

EXHIBIT "A"

PLAN: GRANBUS

PLAN: GRANBURY
CUSTOMER NAME:

MR. & MRS. LOUIS MUNOZ

HEATED LIVING AREA
GROUND FLOOR: 1496

SECOND FLOOR: OTHER:

TOTAL HEATED:

NON-HEATED LIVING AREA

NA

NA

1496

132

PORCH: GARAGE:

 GARAGE:
 400

 OTHER:
 N/A

 TOTAL U/R
 2028

IMPORTANT-PLEASE READ BELOW CAREFULLY

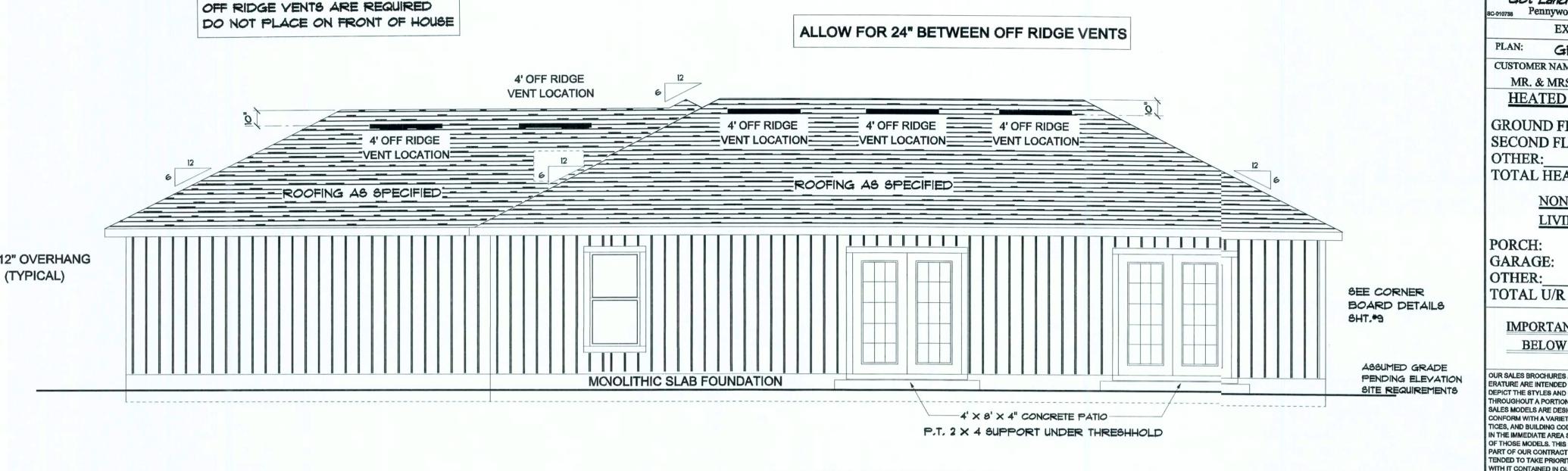
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NOTE

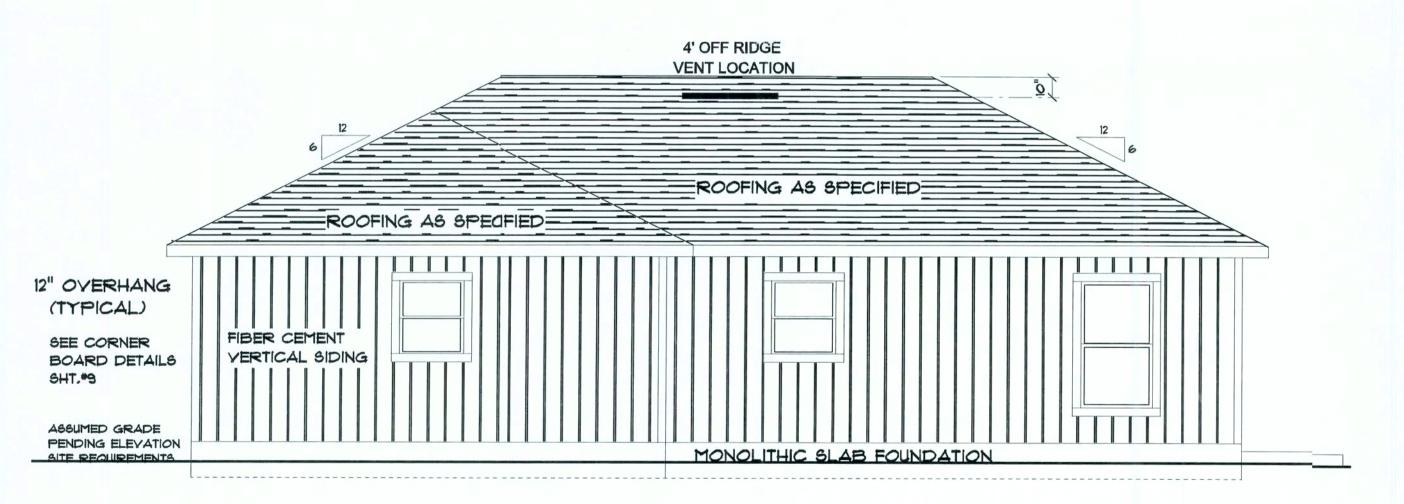
PENNYWORTH HOMES INC. RESERVES
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PLANS, REQUESTED BY BUYERS, THAT
PENNYWORTH HOMES INC. DEEMS TO BE
AESTHETICALLY, ARCHITECTURALLY,
AND STRUCTURALLY SOUND.

ANGES MADE TO REPLICATE
GINEERS NOTES AND
LCULATIONS.

CONSTRUCTION PLANS



REAR ELEVATION



RIGHT ELEVATION

NOTE!:

REFER TO ENGINEERING FOR SPECIFICATIONS AND CALCULATIONS.

> BAA REV.# REV. DAATE: DRAWN BY: DISCRIPTION OF REVISION

Pennyworth Homes

Got Land? Let's Buildle PennyworthHomes.com FL-CRC058

EXHIBIT "A"

GRANBURY

CUSTOMER NAME:

MR. & MRS. LOUIS MUNOZ HEATED LIVING AREA

1496 GROUND FLOOR: SECOND FLOOR: NA

N/A TOTAL HEATED: 1496

> **NON-HEATED** LIVING AREA

PORCH: GARAGE:

132 400 N/A 2028

IMPORTANT-PLEASE READ BELOW CAREFULLY

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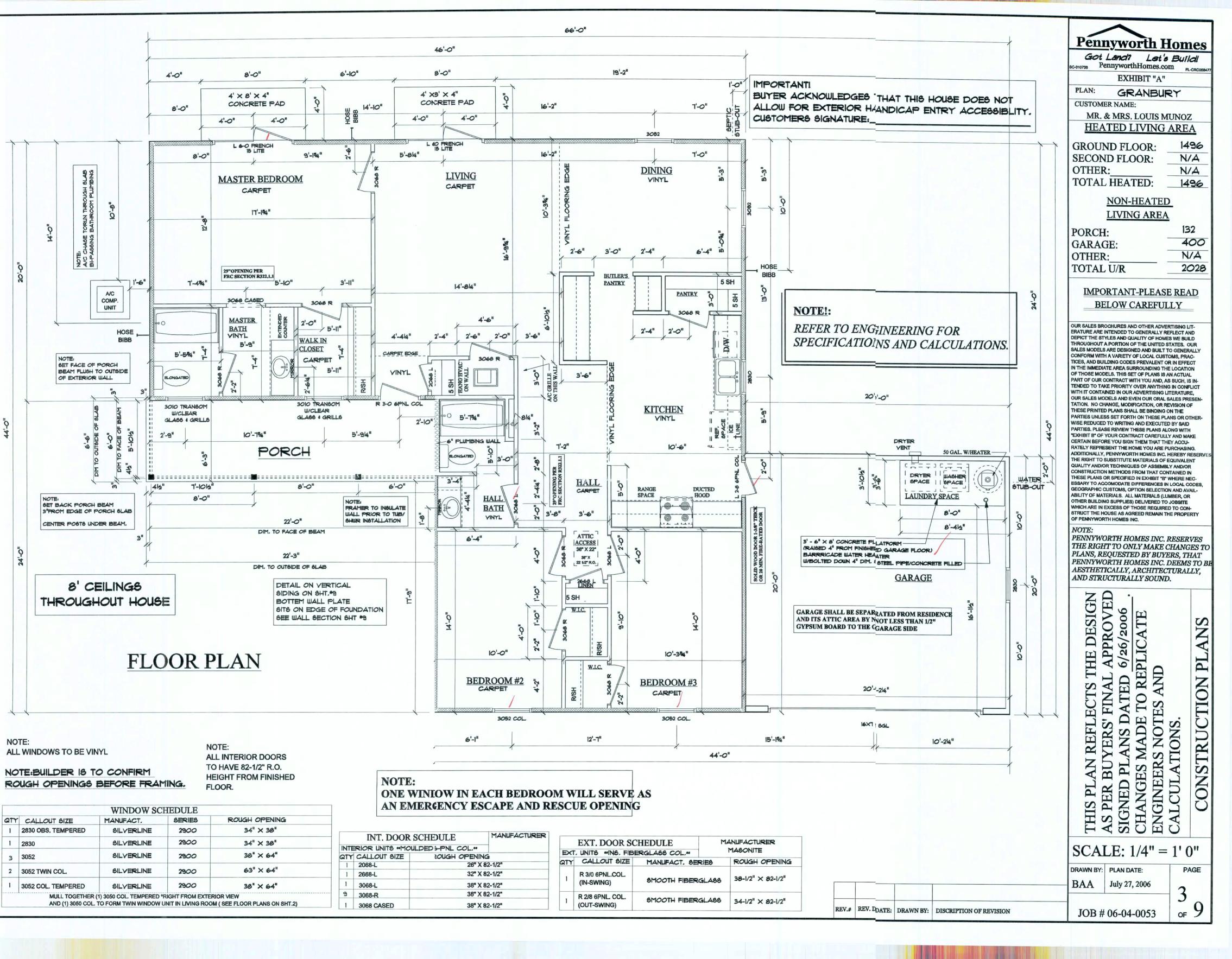
> PROVED 26/2006 ANS PL CONSTRUCTION

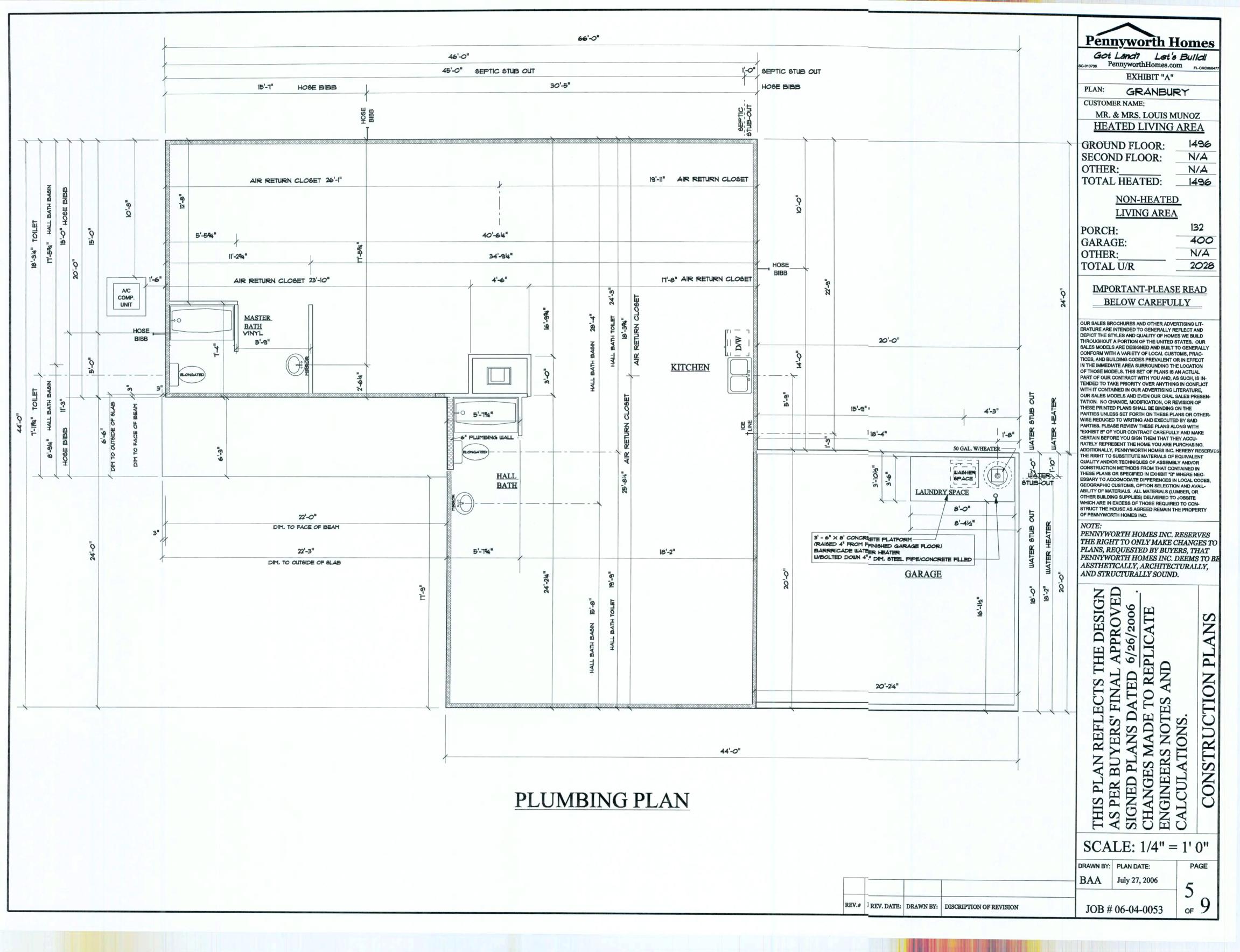
SCALE: 1/4" = 1'-0"

DRAWN BY: PLAN DATE: July 27, 2006

JOB # 06-04-0053

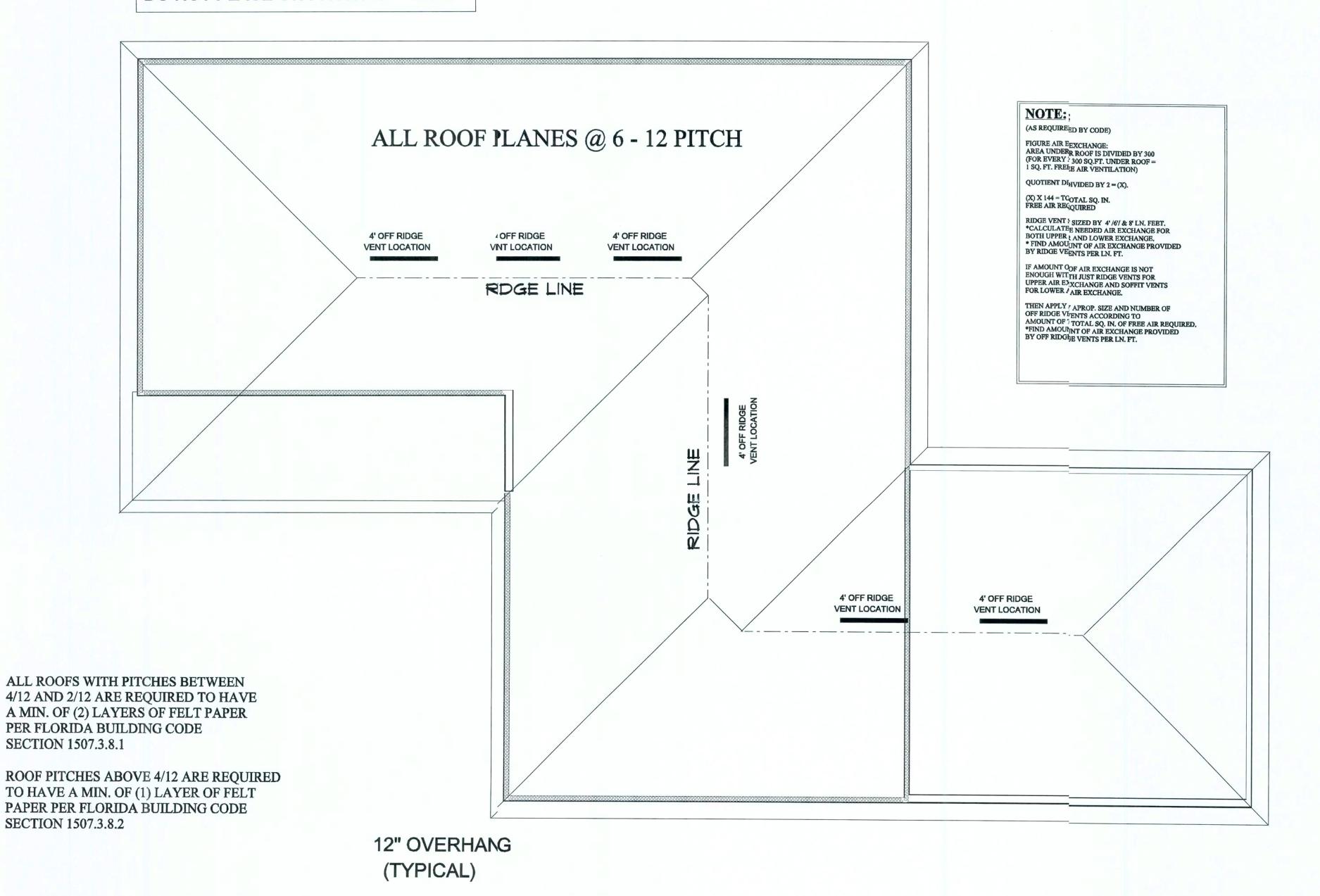
of 9





OFF RIDGE VENTS ARE REQUIRED DO NOT PLACE ON FRONT OF HOUSE

ALLOW FOR MIN. 24" BETWEEN OFF RIDGE VENTS



NOTE!:

REFER TO ENGINEERING FOR SPECIFICATIONS AND CALCULATIONS. **ROOF LAYOUT PLAN**

REV.# REV.J. DATE: DRAWN BY: DISCRIPTION OF REVISION

Pennyworth Homes

Got Land? Let's Build! PennyworthHomes.com FL-CRC

EXHIBIT "A"

GRANBURY

CUSTOMER NAME:

MR. & MRS. LOUIS MUNOZ HEATED LIVING AREA

GROUND FLOOR: SECOND FLOOR:

OTHER:

TOTAL HEATED: 1496

> NON-HEATED LIVING AREA

PORCH: GARAGE: OTHER:

TOTAL U/R

132 400 N/A 2028

1496

N/A

NA

IMPORTANT-PLEASE READ

BELOW CAREFULLY

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> DESIGN PROVED 26/2006 AS PER BUYERS' FINAL APSIGNED PLANS DATED 6/2 PL TO REPL CONSTRUCTION

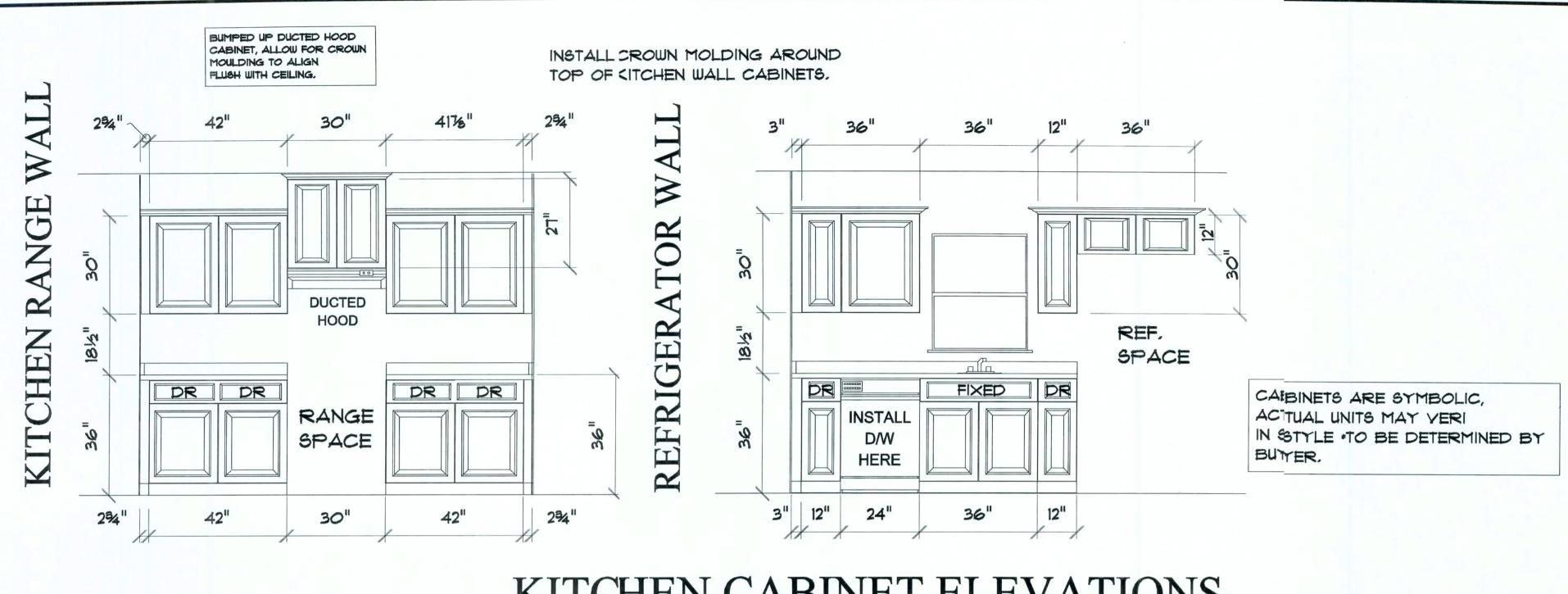
SCALE: 1/4" = 1'-0"

DRAWN BY: PLAN DATE: BAA July 27, 2006

JOB # 06-04-0053

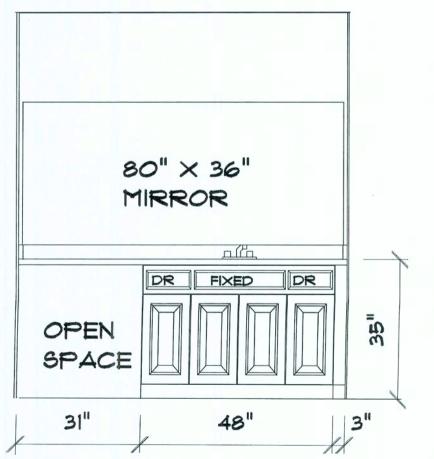
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PAGE



KITCHEN CABINET ELEVATIONS

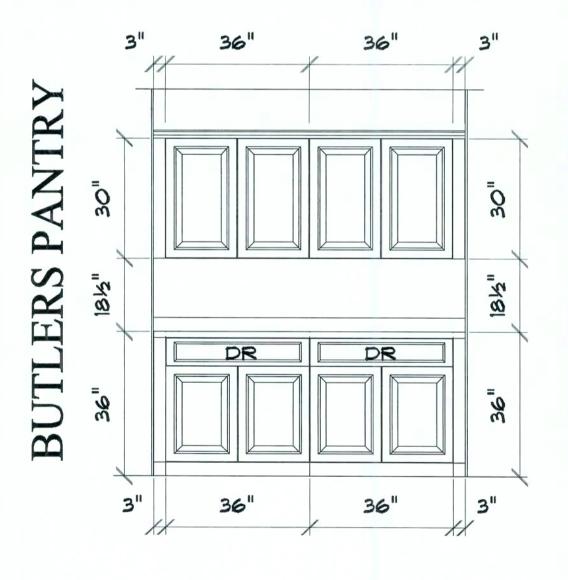
INSTALL QUARTER ROUND MOLDING AROUND BOTTOM OF ALL BASE CABINETS & YANITIES.

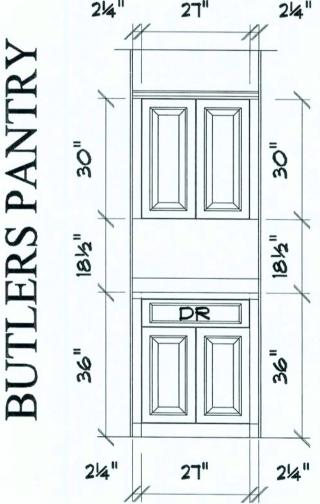


MASTER BATH



BATHROOM VANITY ELEVATIONS





THIS PLAN REFLECTS THE DESIGN AS PER BUYERS' FINAL APPROVED SIGNED PLANS DATED 6/26/2006 CATE CHANGES MADE TO REPLI **ENGINEERS NOTES AND** CALCULATIONS

DRAWN BY: PLAN DATE: July 27, 2006 of 9

JOB # 06-04-0053

PREV. DATE: DRAWN BY: DISCRIPTION OF REVISION

Pennyworth Homes Got Land? Let's Build!
PennyworthHomes.com FL-CRC0 EXHIBIT "A" GRANBURY

CUSTOMER NAME: MR. & MRS. LOUIS MUNOZ

PLAN:

HEATED LIVING AREA

GROUND FLOOR: 1496 SECOND FLOOR: N/A OTHER: N/A TOTAL HEATED: 1496

NON-HEATED

LIVING AREA PORCH:

132 400 GARAGE: OTHER: N/A TOTAL U/R 2028

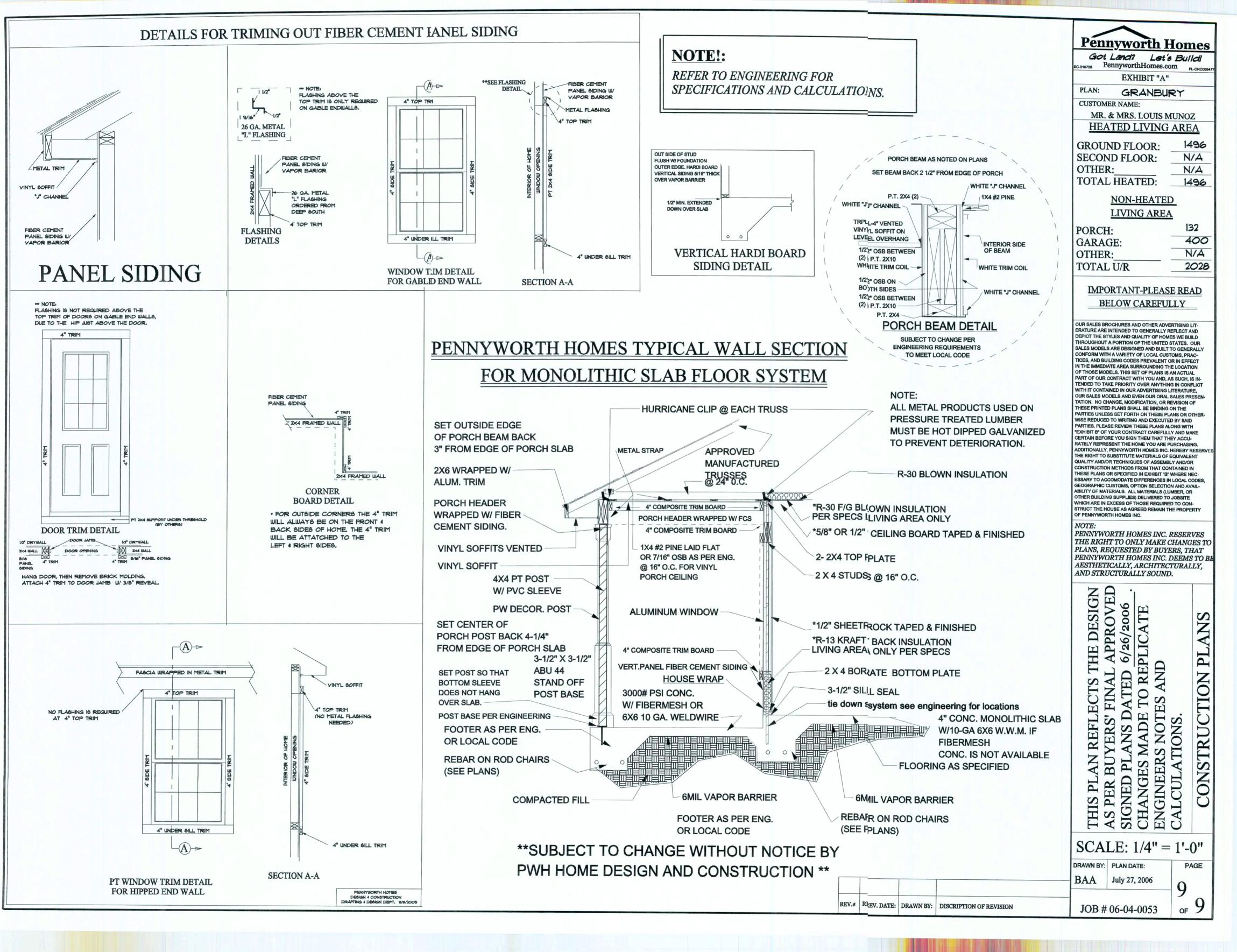
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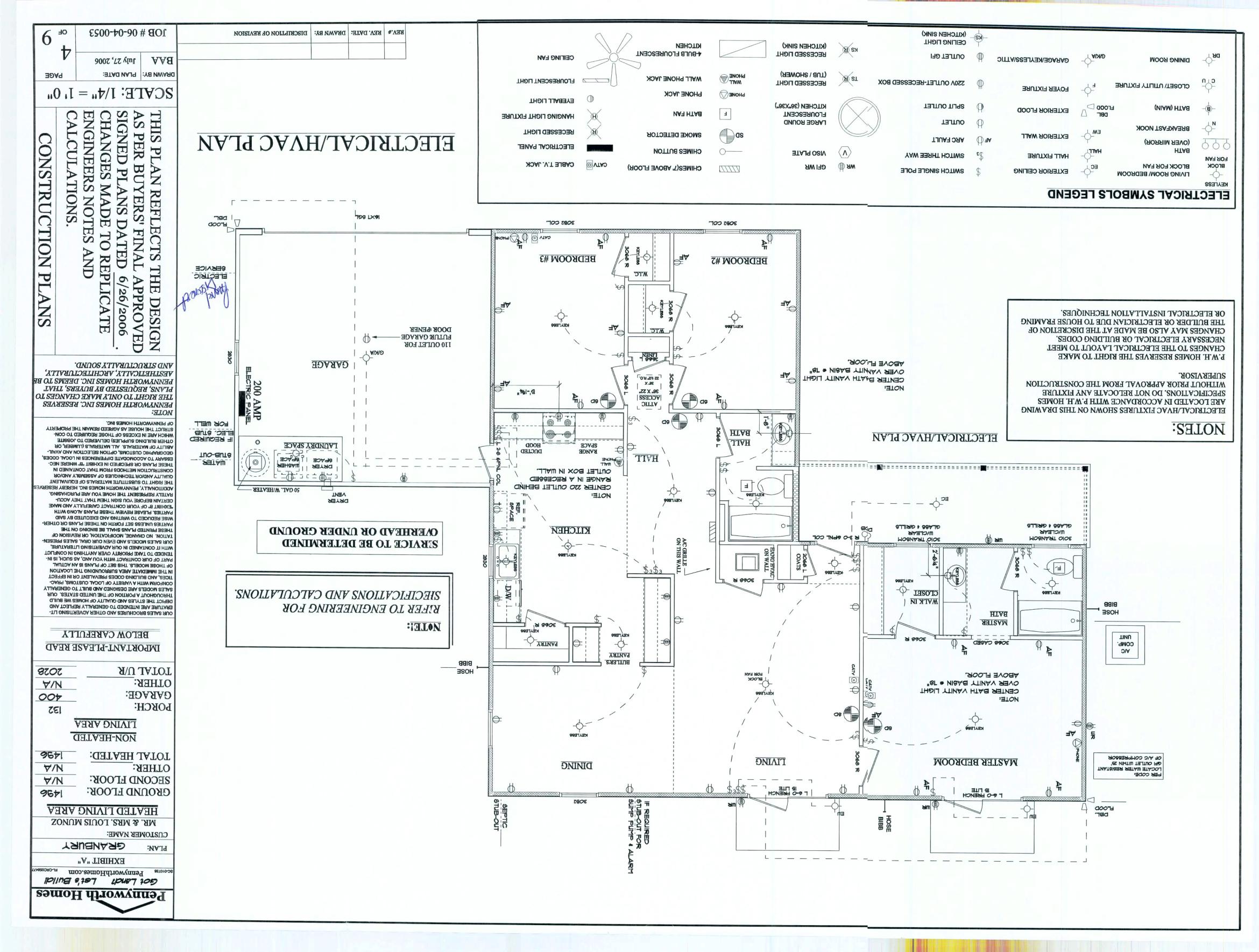
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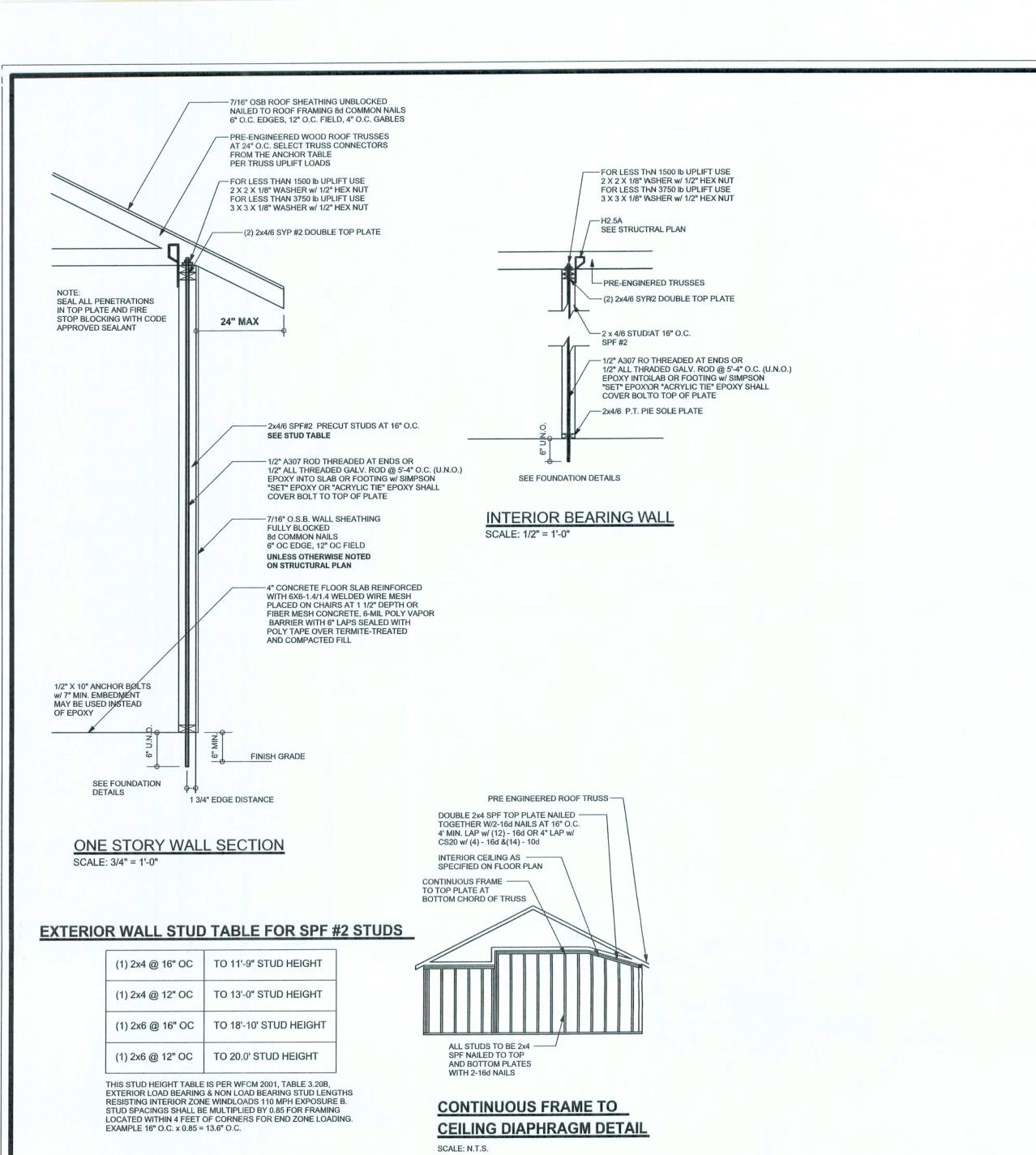
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CONSTRUCTION

SCALE: 1/2" = 1'-0"







- NON-SUPPORTIVE

SUPPORTIVE -

3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON -

NAILED WITH 14-10d

SCALE: N.T.S.

(2-ONE SIDE, 2-ON

OTHER SIDE)

SCALE: N.T.S.

IF BEAM JOINT IS AT -POST CONNECTION.

INSTALL ONE SIMPSON LSTA18 ON ONE SIDE

DETAIL FOR SINGLE BEAM

SUPPORTIVE BEAM -

(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN

> SIMPSON HUS412 MIN. -SEE STRUCTURAL PLAN

SCALE: N.T.S.

TOGETHER W/2-16d NAILS AT 16" O.C.

BEAM MID-WALL CONNECTION DETAIL

LSTA18

NAIL THRU 2x4 INTO

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

BEAM W/4-16d

SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

MIN. (SEE STRUCTURAL PLAN)

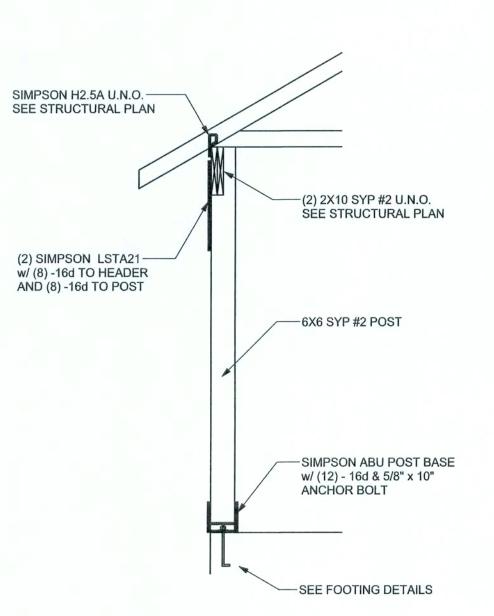
— (2) 2X12 SYP #2 MIN. —— SEE STRUCTURAL PLAN

2X4 LADDER BEAM

- SUPPORTIVE

SIMPSON H2.5A U.N.O.-SEE STRUCTURAL PLAN (2) 2X10 SYP #2 U.N.O. SEE STRUCTURAL PLAN SUPPORTIVE POST TO BEAM (2) SIMPSON LSTA21w/ (8) -16d TO HEADER AND (8) -16d TO POST -6X6 SYP #2 POST -SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT

TYPICAL PORCH POST DETAIL SUPPORTIVE CENTER POST TO BEAM DETAIL



ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 2300

< 2320

FOR LESS THAN 1500 Ib UPLIFT USE

FOR LESS THAN 3750 Ib UPLIFT USE

-NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT

—SP4/6 @ 48" O.C. (U.N.O.) /——(7) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU HEADER

INTO KING STUD

2 X 2 X 1/8" WASHER

3 X 3 X 1/8" WASHER

CRIPPLES IF REQUIRED

(5), .131 x 3 1/4" GUN NAILS

-TTOE NAILED THRU SILL-

IINTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

(SSEE STRUCTURAL PLAN)

(1) 2X66 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X44 SPF #2 SILL UP TO 7'-8" U.N.O. (1) 2X44 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 1200 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STO RY HEADER STRAPING DETAIL

IF TRUSS TO WALL STRAPS ARE NAILED

TO THE HEADER THE SP4/6 @ 48" O.C.

INTO KING STUD

(7) .131 x 3 1/4" GUN NAILS -

TOE NAILED THRU HEADER

ARE NOT REQUIRED

< 2300

< 2320

ABU66

12-16d

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	911
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	НЗ	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROU 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROE 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB

GENERAL NOTES:

(RUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC. UNC

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

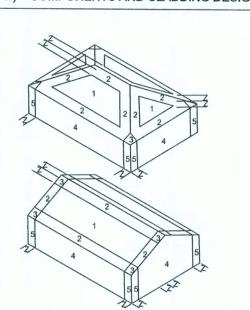
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRE LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

1/2" AB

2-5/8" AB

	50x HEIGHT OR 1 MILE WHICHEVER IS LE
BUILDING IS NOT IN THE HIGH VELOCITY HUP	
BUILDING IS NOT IN THE WIND-BORNE DEBR	IS REGION
1.) BASIC WIND SPEED = 110 MPH	
2.) WIND EXPOSURE = B	
3.) WIND IMPORTANCE FACTOR = 1.0	
4.) BUILDING CATEGORY = II	
5.) ROOF ANGLE = 10-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = N	/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN	WIND PRESSURES (TABLE R301.2(2))



NOT IN FLOOD ZONE (BUILDER TO VERIFY)

	10			100
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
	& Wind st Cas 5, 10	е	21.8	-29.1
8x7 Garage Door			19.5	-22.9
16x7 Ga	rage [Door	18.5	-21.0

ESIGN	LOADS
LOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
OOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
TAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
OIL BE	ARING CAPACITY 1000PSF

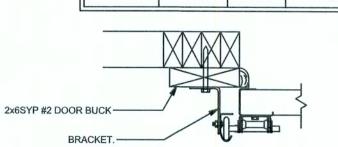
REVISIONS

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 86, Lake City, FL 32056, 386-754-5419 Stated dimensions supercede scaled dimensions. Refer all questions to

Mark Disosway, P.E. fir resolution

Do not proceed withou clarification.

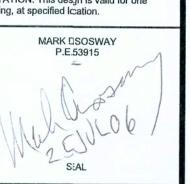
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code residential 2004, o the best of my

examined this plan, and that the applicable

portions of the plan, reating to wind engineer comply with section R301.2.1, florida building

LIMITATION: This desan is valid for one building, at specified loation.



Pennyworth Homes

Munoz Residence

ADDRESS: Columbia Coutny, Florida

Mark Disesway P.E. P.O. Eox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419

PRINTED DATE: July 25, 1006 STRUCTURAL BY DRAWN BY:

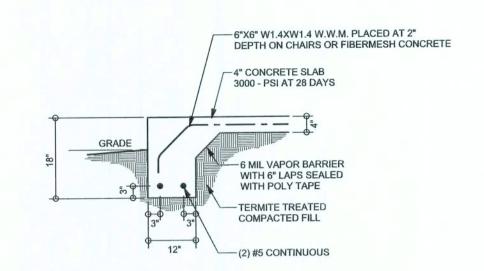
Fax: (386)269 - 4871

David Disosway

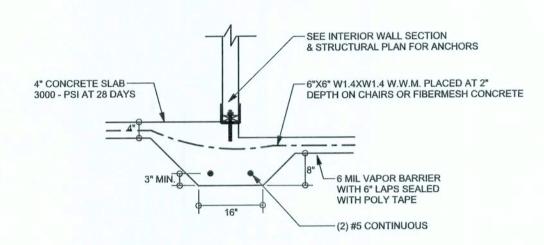
25 / Jul / 06

JOB NUMBER: 607241 DRAWING NUMBER

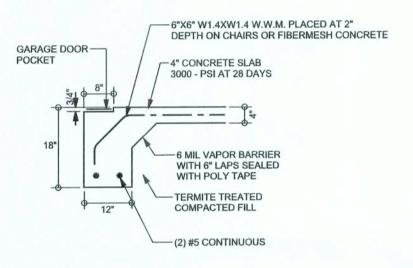
> **S-1** OF 3 SHEETS



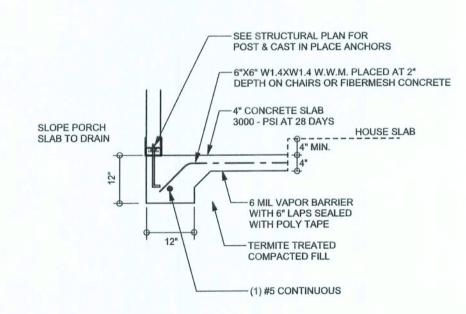
MONOLITHIC FOOTING S-2 SCALE: 1/2" = 1'-0"



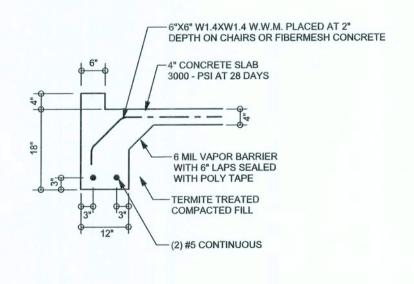
INTERIOR BEARING STEP FOOTING SCALE: 1/2" = 1'-0"



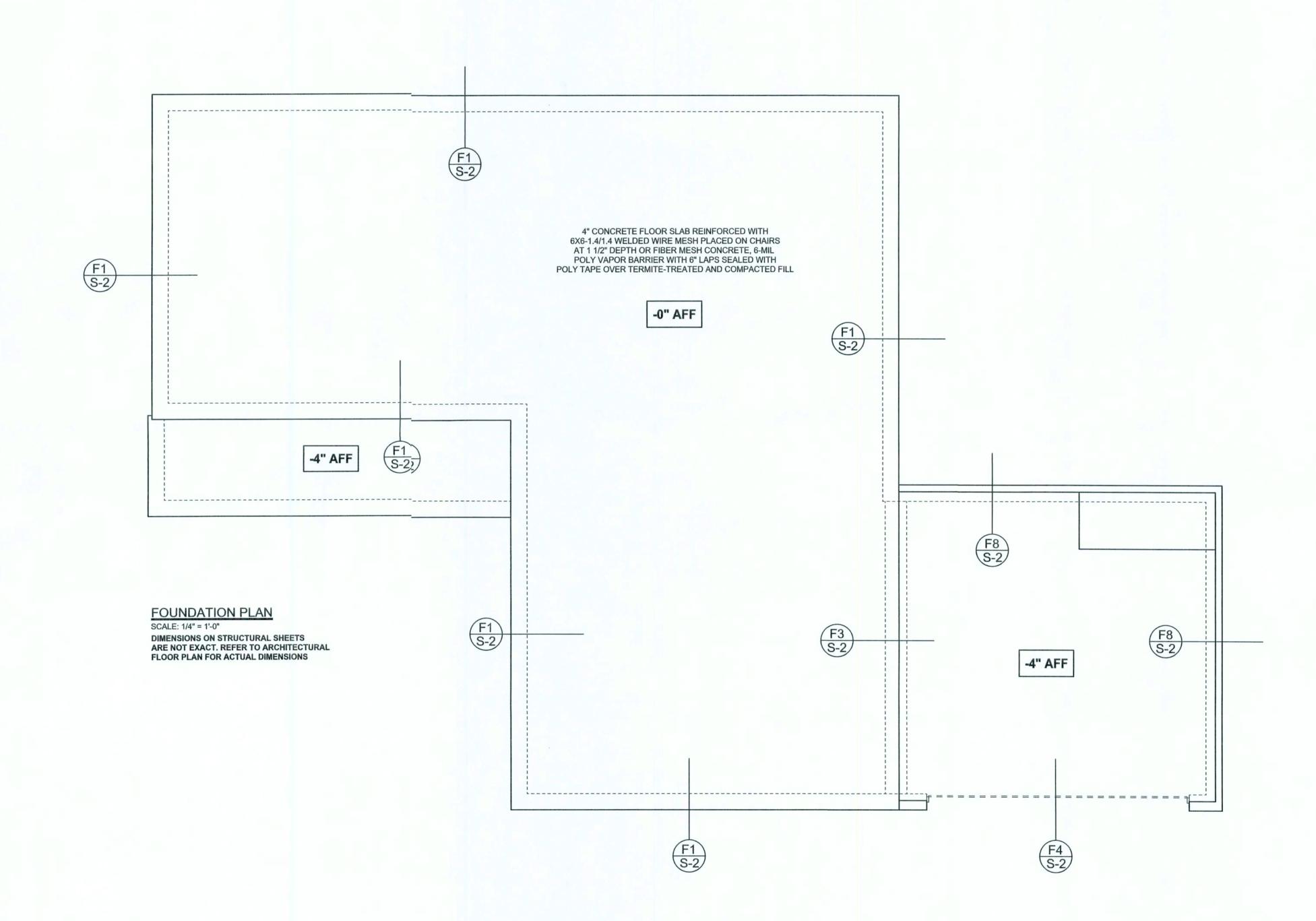
GARAGE DOOR FOOTING SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



GARAGE CURB FOOTING S-2 SCALE: 1/2" = 1'-0"



REVISIONS

SOFTPIAN

WINDLOAD ENGINEER Mark Disosway, PE No.53915, POB 868,Lake City, FL 32056, 386-754-5419

Stated dimensions supecede scaled dimensions. Refer all quistions to Mark Disosway, P.E. forresolution. Do not proceed without (arification. COPYRIGHTS AND PROPERTY RIGHTS:
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not to be reproduced, altred or copied in any form or manner without Irst the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and hat the applicable portions of the plan, relaing to wind engineering comply with section R30.2.1, florida building code residential 2004, to best of my knowledge.

LIMITATION: This design is valid for one building, at specified locition.

MARK DISOSWAY P.E. 5915

Pennyworth Homes

Munoz Residence

ADDIESS: Columbia Contny, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Fbrida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: July 25, 2)06 DRAWN BY: STRUCTURAL BY

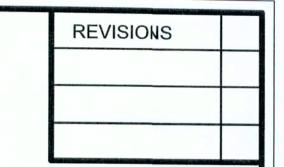
FINALS DATE: 25 / Jul / 06

> JOB NLMBER: 607241 DRAWINGNUMBER

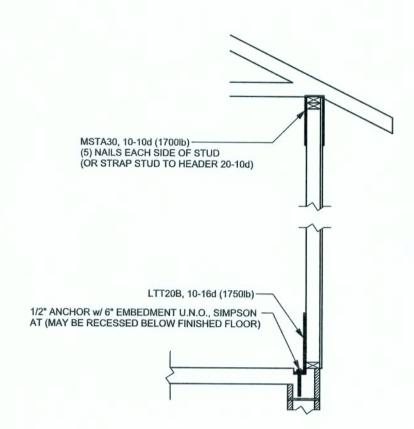
David Disosway

S-2

OF 3 SHEETS







ALTERNATE WALL TIE CONNECTION WHERE
THREADED ROD CANNOT BE PLACED IN WALL.
SCALE: 1/2" = 1'-0"

STRUCTURAL PLAN NOTES

-920 LB UPLIFT

8.00

(2) 2X12X6',2J 2K

USE H2.5A (480Ib) FOR ALLTRUSS TO WALL FRAME AND PORCH BEAM

T05

-2558 LB

UPLIFT

----847 LB UPLIFT

SWS = 4.5

T|6|

CJ5

CJ3

T13 (5)

CONNECTIONS UNLESS NCTED OTHERWISE

(3) 2X4 SPF #2 STUDS CENTERED UNDER TRUSS

(2) 2X4 SPF #2 STUDS CENTERED UNDER TRUSS

- ABU (TYP.)

SWS = 5.0'

1433 LB — UPLIFT

CJ3

CJ1

3

STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

(2) 2X 2X6',2J 2K

-652 LB

UPLIFT

SWS = 11.5'

T09 (6)

CJ3

SW\$ = 5.5'

2010 LB UPLIFT

> 1001 LB UPLIFT-

> > SWS = 5.0

EJ7 (9)

SW\$ = 9.5'

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

CJ1

SWS = 2.5

5

746 LB

UPLIFT

CLO

UPLIFT

(2) 1.755"X 2.00X16 LVL,2J 2K

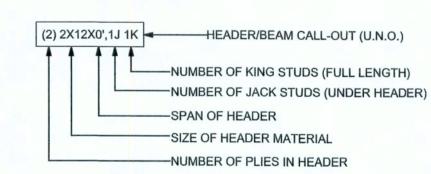
T04 (5)

sws = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW 20000001 = = = = 100000001	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

•	INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD
® —	

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

= 0.0' INDICATES	S SHEAR WA	LL SEGM
	REQUIRED	ACTUAL
TRANSVERSE	29.5'	78.5'
LONGITUDINAL	28.6'	75.0'

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. fo resolution.
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CERTIFICATION: I henby certify that I have examined this plan, anothat the applicable portions of the plan, relating to wind engineering comply with section R3I1.2.1, florida building code residential 2004, b the best of my knowledge.

LIMITATION: This design is valid for one building, at specified loation.

MARK DSOSWAY
P.E. i3915

WINDLOAD ENGINEEI: Mark Disosway, PE No.53915, POB 86ξ Lake City, FL 32056, 386-754-5419

Pennyworth Homes

Munoz Fesidence

ADDRESS: Columbia Cutny, Florida

Mark Discsway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386)269 - 4871

PRINTED DATE: July 25, 2006

DRAWN BY: STRUCTURAL BY
David Disosway

FINALS DATE: 25 / Jul / 06

JOB NUMBER: 607241

> S-3 OF 3 SHEETS

DRAWING NUMBER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L201610