

BOUNDARY SURVEY

IN  
THE NE 1/4 OF NW 1/4  
SECTION 13, TWP 4-S, RNG 15-E,  
COLUMBIA COUNTY, FLORIDA

DESCRIPTION

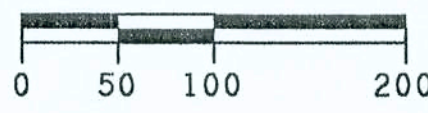
THE SOUTH 1/2 OF THE NORTHEAST 1/4 OF THE NORTHEAST 1/4 OF THE NORTHWEST 1/4;  
ALSO THE WEST 1/2 OF THE NORTHEAST 1/4 OF THE NORTHWEST 1/4, LESS AND EXCEPT  
THE SOUTH 60.00 FEET OF THE EAST 60.00 FEET OF SAID WEST 1/2 OF THE NORTHEAST 1/4  
OF THE NORTHWEST 1/4, ALL LYING IN SECTION 13, TOWNSHIP 4 SOUTH, RANGE 15 EAST,  
COLUMBIA COUNTY, FLORIDA. CONTAINING 25.11 ACRES, MORE OR LESS.

SAID LANDS BEING SUBJECT TO ROAD RIGHT OF WAYS.

NOTES:

1. BEARINGS ARE BASED ON THE NORTH LINE OF SECTION 13, TWP 4-S, RNG 15-S, BEING  
N 88°54'15" E.
2. SUBJECT PROPERTY LIES IN FLOOD ZONE "X" (UN-SHADED), AN AREA OUTSIDE OF THE 500-YEAR FLOOD PLAIN  
PER FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070 0175 B, LAST REVISION DATE JANUARY 6, 1988.  
FLOOD ZONE LINES, IF ANY, ARE SCALED FROM FLOOD INSURANCE RATE MAPS, PROVIDED BY FEMA.
3. ONLY THOSE VISIBLE INTERIOR IMPROVEMENTS AND IMPROVEMENTS PERTINENT TO THE SUBJECT PROPERTY  
HAVE BEEN LOCATED AS SHOWN HEREON. EXCEPTION IS MADE HEREON TO UNDERGROUND FACILITIES  
AND OTHER IMPROVEMENTS NOT VISIBLE OR KNOWN AT DATE OF SURVEY.
4. THIS SURVEY WAS PREPARED WITHOUT THE BENEFIT OF AN ABSTRACT OR TITLE POLICY. THEREFORE, EXCEPTION  
IS MADE HEREIN REGARDING EASEMENTS, RESERVATIONS AND RESTRICTIONS OF RECORD NOT PROVIDED BY THE CLIENT.
5. CLOSURE EXCEEDS 1 : 10,000.

LEGEND	
○	DENOTES 5/8" IRON ROD & CAP SET (LB6685)
●	DENOTES IRON PIPE OR REBAR FOUND
□	DENOTES 4"x4" CONCRETE MONUMENT SET (LB6685)
■	DENOTES 4"x4" CONCRETE MONUMENT FOUND
⊙	DENOTES NAIL & DISK FOUND
⊗	DENOTES 4"x4" CONCRETE MONUMENT (PRM); (LB6685) UNLESS OTHERWISE NOTED
—x—	DENOTES FENCE
—o—o—	DENOTES OVERHEAD ELECTRIC
—◇—	DENOTES POWER POLE
—>—	DENOTES GUY ANCHOR
CONCRETE	CONCRETE
±	MORE OR LESS
PC	POINT OF CURVATURE
PT	POINT OF TANGENCY
PI	POINT OF INTERSECTION
PRC	POINT OF REVERSE CURVATURE
PCC	POINT OF COMPOUND CURVATURE
R	RADIUS
T	TANGENT
L	ARC LENGTH
Δ	CENTRAL ANGLE
CH	CHORD BEARING & DISTANCE
PCP	PERMANENT CONTROL POINT
PRM	PERMANENT REFERENCE MONUMENT
R/W	RIGHT OF WAY
FOOT	FLORIDA DEPARTMENT OF TRANSPORTATION
N	NORTH
E	EAST
S	SOUTH
W	WEST
CL	CENTERLINE
(P)	PLAT
(D)	DEED
(C)	CALCULATED
(M)	MEASURED
O/S	OFFSET
NO ID	NO IDENTIFICATION
FND	FOUND
CM	CONCRETE MONUMENT
IP	IRON PIPE
IPC	IRON PIPE & CAP
RB	REBAR
RBC	REBAR & CAP
IR	IRON ROD
IRC	IRON ROD & CAP
NL	NAIL
NL&D	NAIL & DISK
ORB	OFFICIAL RECORDS BOOK
PG	PAGE(S)
POC	POINT OF COMMENCEMENT
PUB	POINT OF BEGINNING
SEC	SECTION
TWP	TOWNSHIP
RNG	RANGE



SCALE: 1" = 100'

SURVEY FOR: BRENDA K. AND RONALD D. MILLER  
FARM CREDIT  
TERRY McDAVID  
ATTORNEYS' TITLE INSURANCE FUND, INC.  
OLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY  
FIRST AMERICAN TITLE INSURANCE COMPANY

05-22-03

DATE OF SIGNATURE:

05/21/03

DATE OF FIELD SURVEY:

*Timothy B. Alcorn*  
TIMOTHY B. ALCORN  
PROFESSIONAL SURVEYOR AND MAPPER  
FLORIDA CERTIFICATE NO. 6332

SURVEY VALID ONLY TO THE DATE OF FIELD SURVEY SHOWN HEREON. NOT VALID WITHOUT  
THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND  
MAPPER. FLORIDA CERTIFICATE OF AUTHORIZATION NO. LB6685

DRAWN BY:

SD

JOB NUMBER

030502MIL

FIELD BOOK

157 : 5

EFB

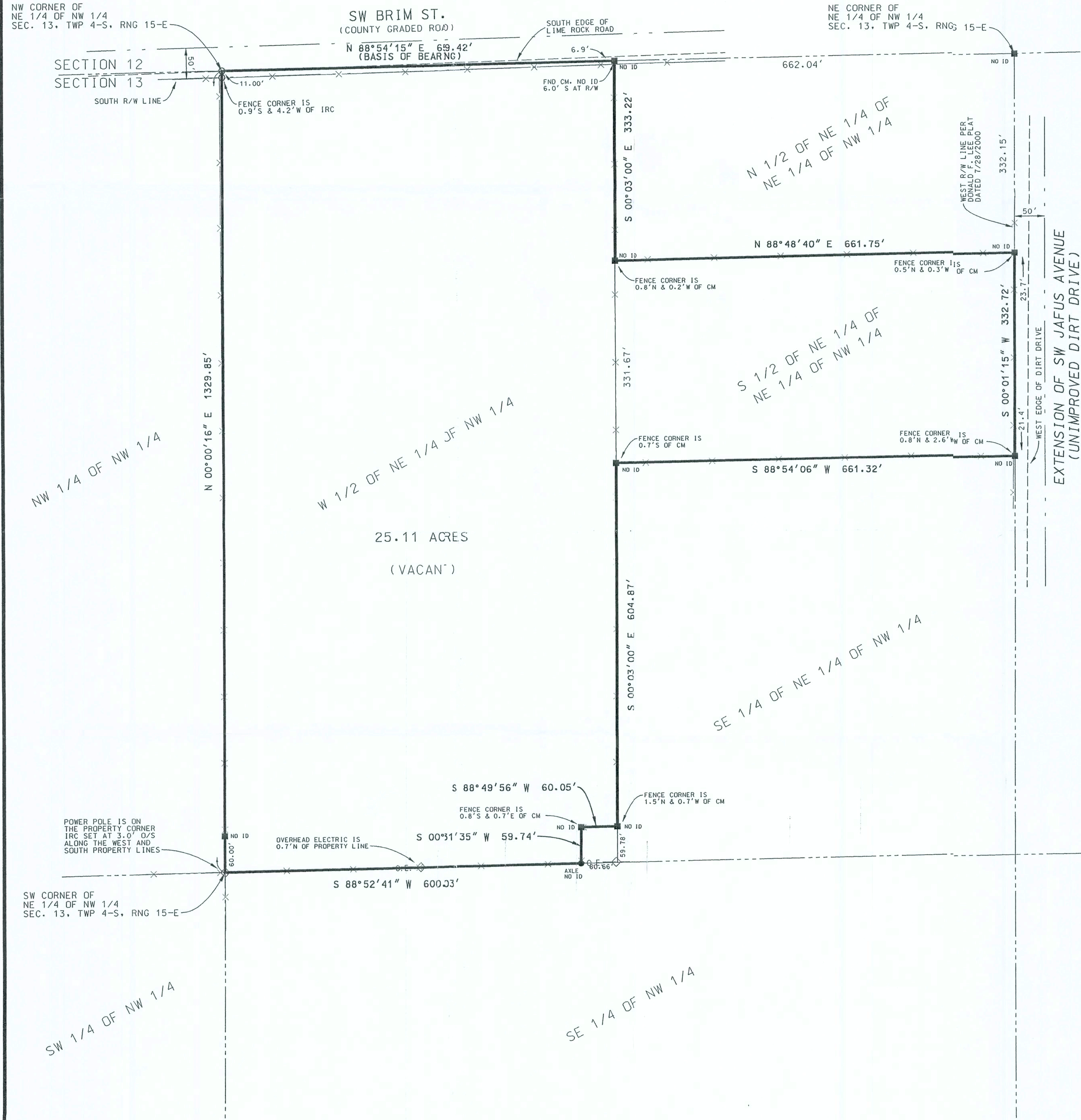
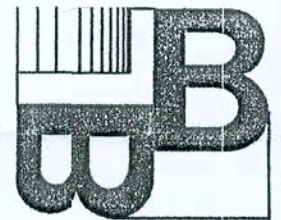
SHEET NO.

1 OF 1

BRENDA K. AND  
RONALD D. MILLER

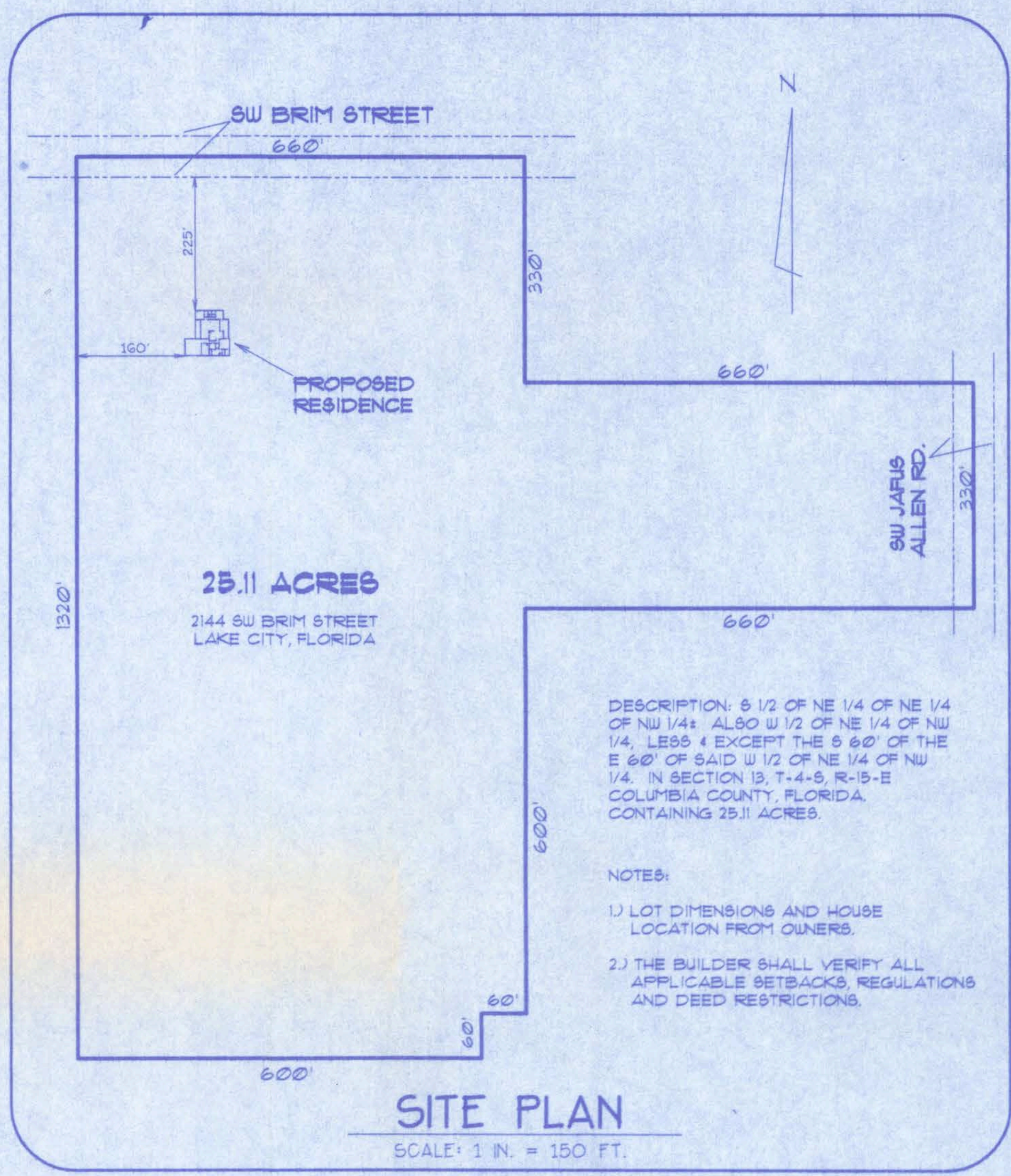
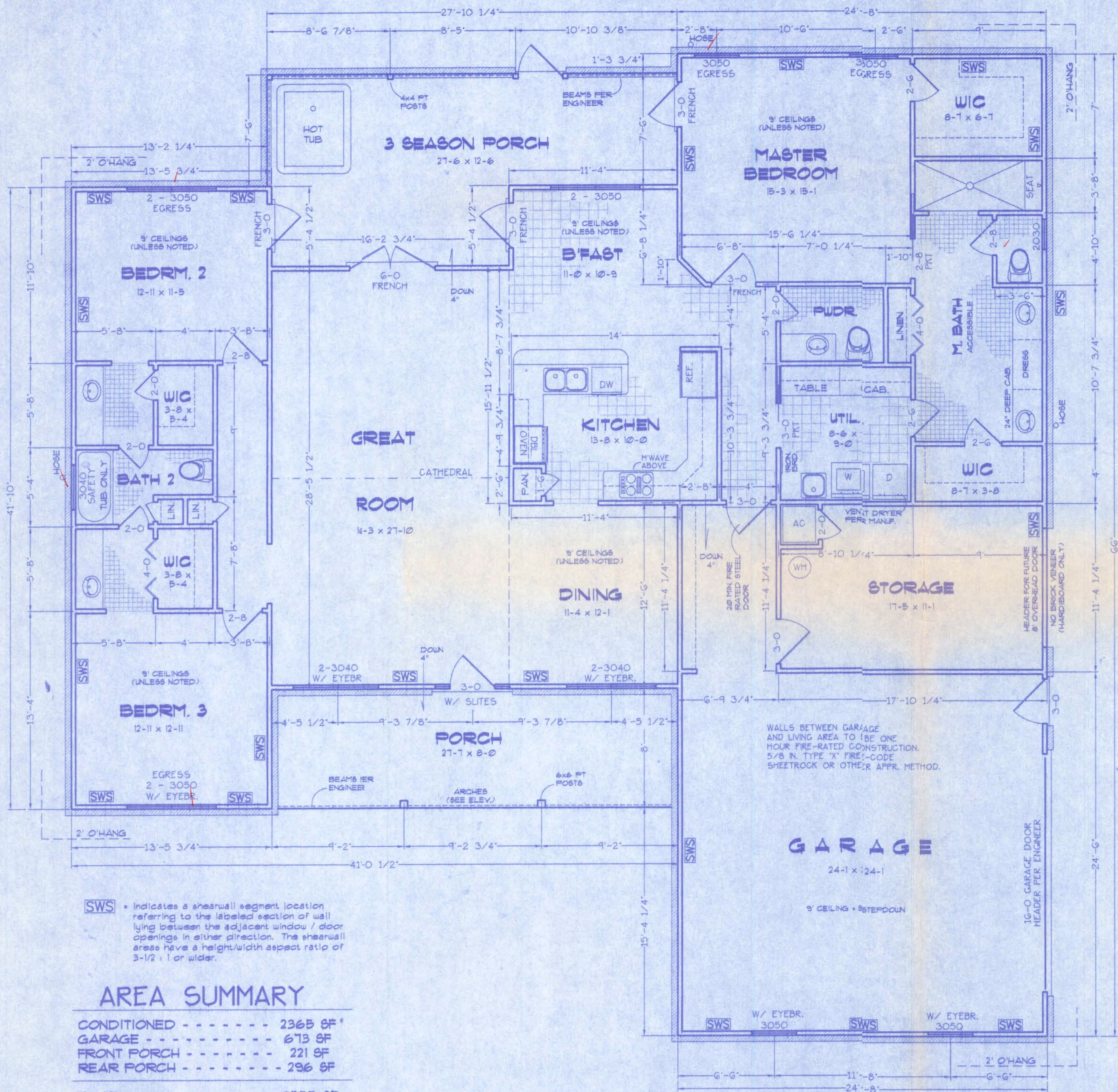
REVISIONS

BAILEY BISHOP & LANE, INC.  
484 SW COMMERCE DRIVE, SUITE 130  
P.O. BOX 3717  
LAKE CITY, FL 32056-3717  
PH: (386) 752-5640 FAX (386) 755-7771  
Eng. Lic. 7362 Survey Lic. LB-0006685





# Miller Residence



## Index to Sheets

SHEET A-1	-----	SITE PLAN + FLOOR PLAN
SHEET A-2	-----	ELEVATIONS + GEN. NOTES
SHEET A-3	-----	ELEVATIONS
SHEET A-4	-----	FOUNDATION + SECTIONS
SHEET A-5	-----	ELECTRICAL
SHEET S-1	-----	WIND ENGINEERING

**A-1**

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

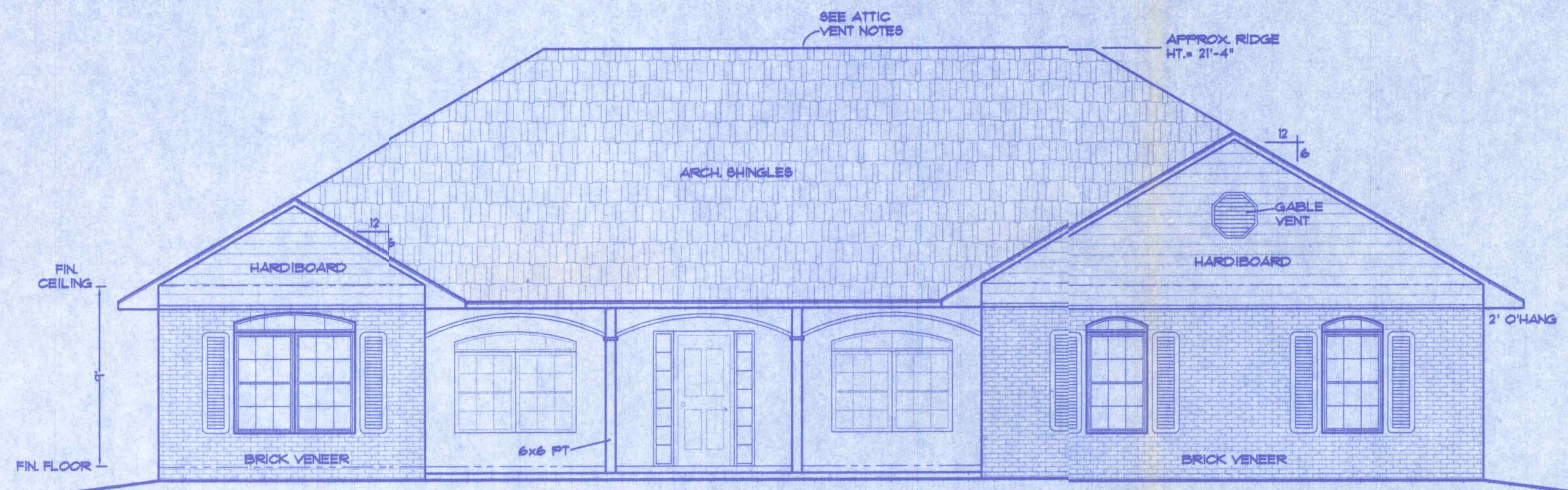
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

2144 SW BRIM STREET  
LAKE CITY, FLORIDA

Location: \_\_\_\_\_ Job No.: \_\_\_\_\_

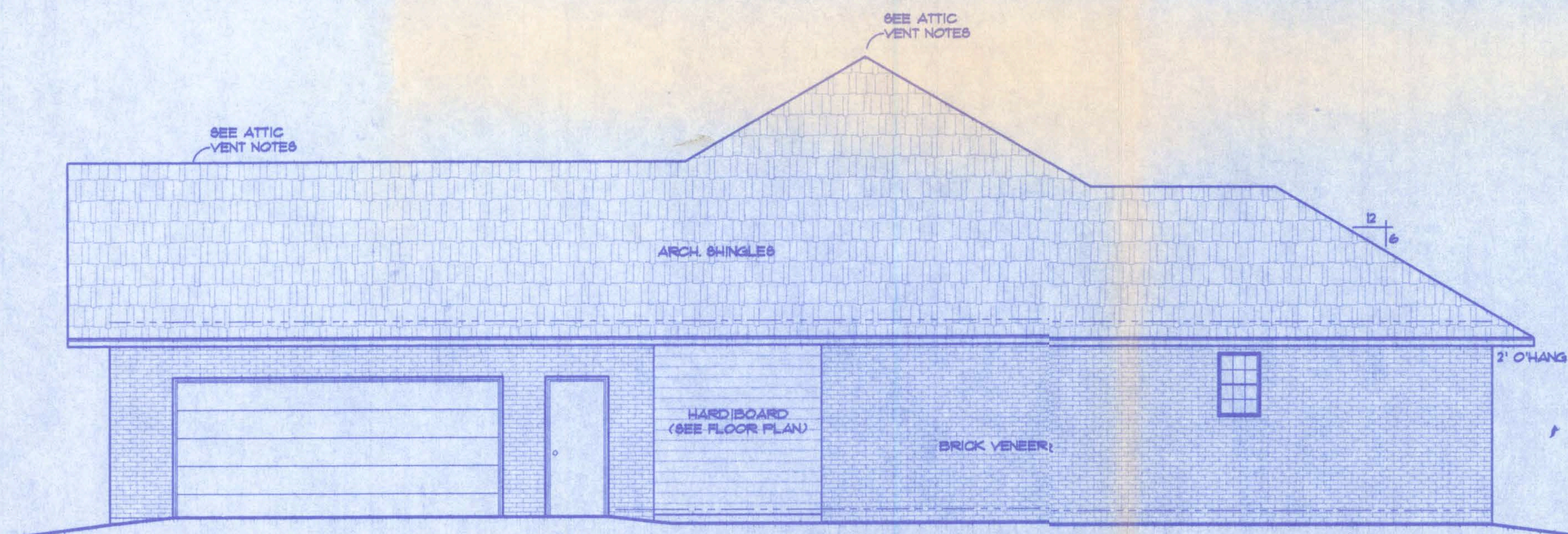
FILE: OG-002	<b>MILLER RESIDENCE</b>	SHEET: 1 OF 5
DATE: 2-27-06		CAD FILE: OG002
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln. Lake City, FL 32024 Phone: C 386 J 755-5891	REV:





### FRONT ELEVATION

SCALE: 1/4 IN. = 1 FT.



### RIGHT ELEVATION

SCALE: 1/4 IN. = 1 FT.

### GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

### ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

# A-2

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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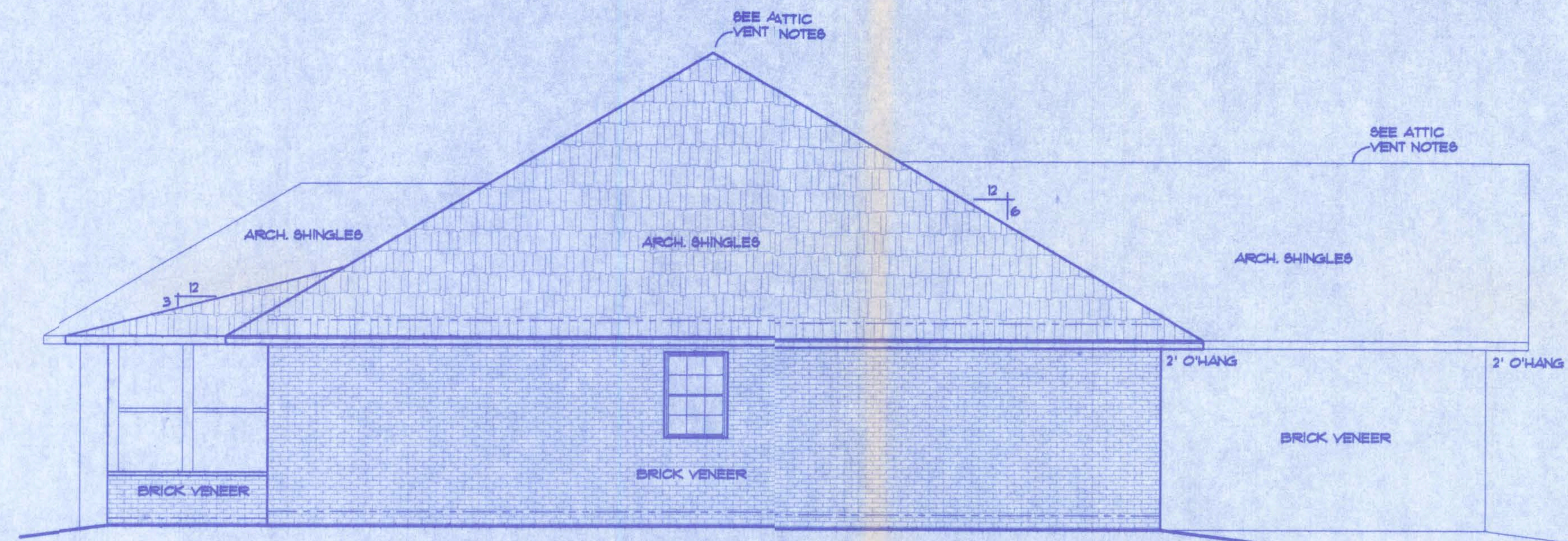
2144 SW BRIM STREET  
LAKE CITY, FLORIDA

Location: Job No.:

FILE: OG-002	MILLER RESIDENCE	SHEET: 2 OF 5
DATE: 2-27-06		CAD FILE: OG002
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln., Lake City, FL 32024 Phone (386) 755-5891	REV:

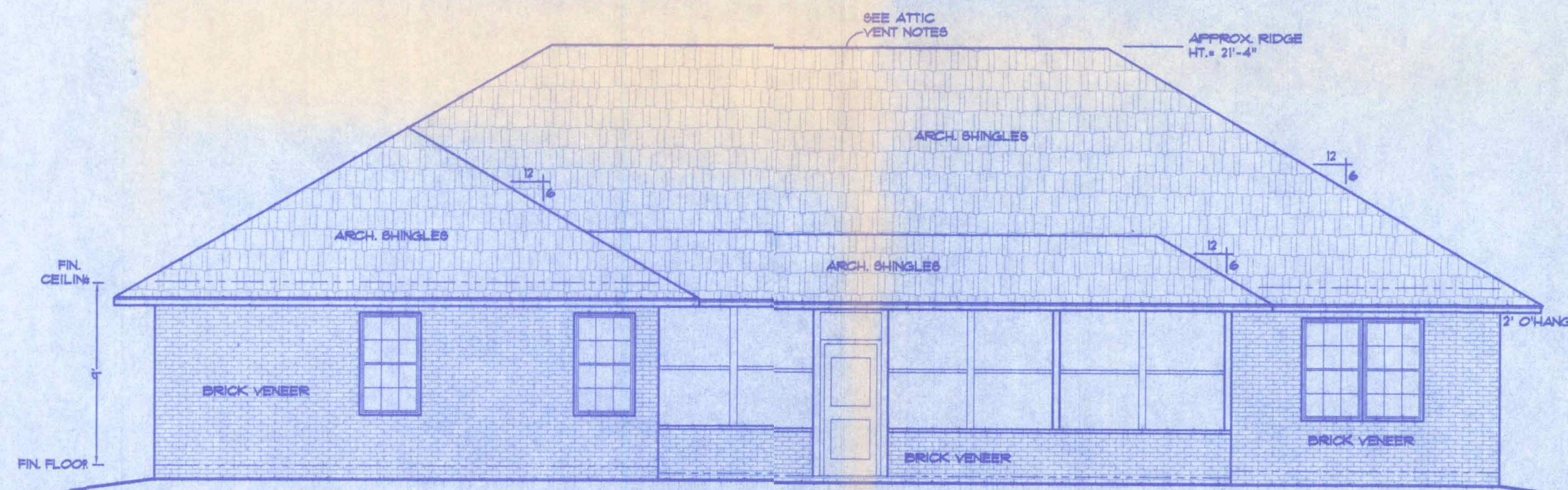
*Mark Disoway*  
Z9MAR06





LEFT ELEVATION

SCALE: 1/4" IN. = 1 FT.



REAR ELEVATION

SCALE: 1/4" IN. = 1 FT.

### ATTIC VENTILATION

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WINDLOAD ENGINEER: Mark Diasoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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2144 SW BRIM STREET  
LAKE CITY, FLORIDA

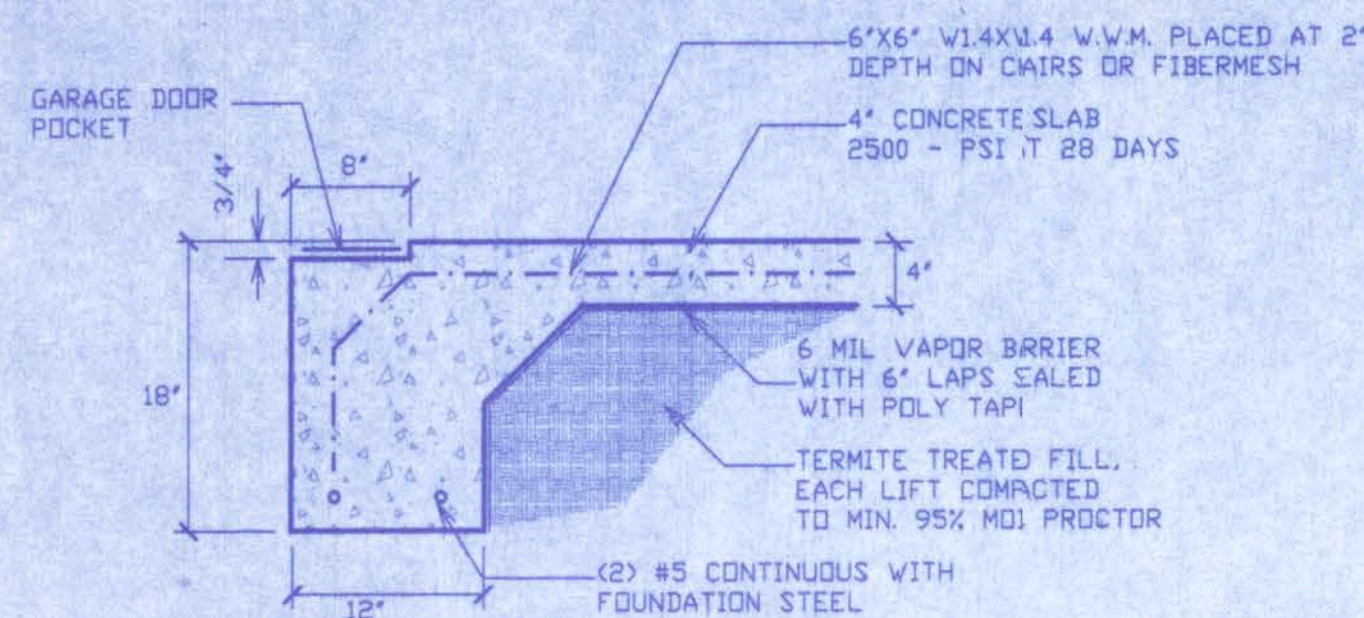
Location: Job No.:

A-3

FILE: OG-002	<b>MILLER RESIDENCE</b>	SHEET: 3 OF 5
DATE: 2-27-06		CAD FILE: OGG02
DRAWN: T A D	PREPARED BY: <b>TIM DELBENE</b> Drafting + Technical Services	REV:
CHECK: T A D	142 SW Segevood Cir., Lake City, FL 32024 Phone (386) 755-5891	REV:

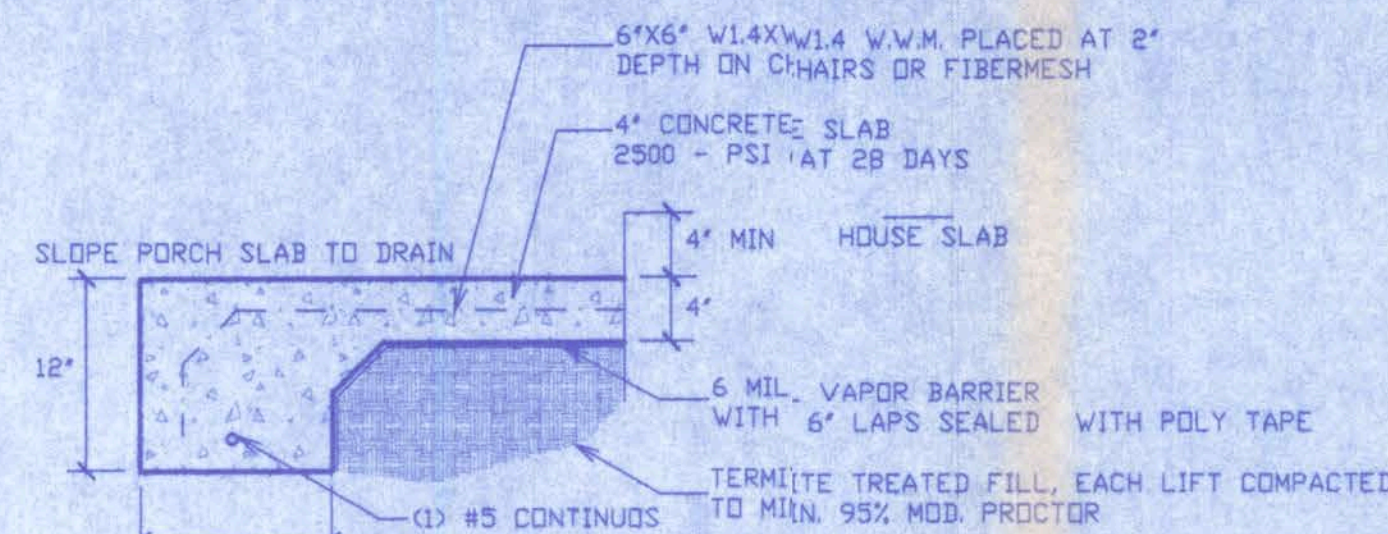
*Mark Diasoway*  
29 MAR 06





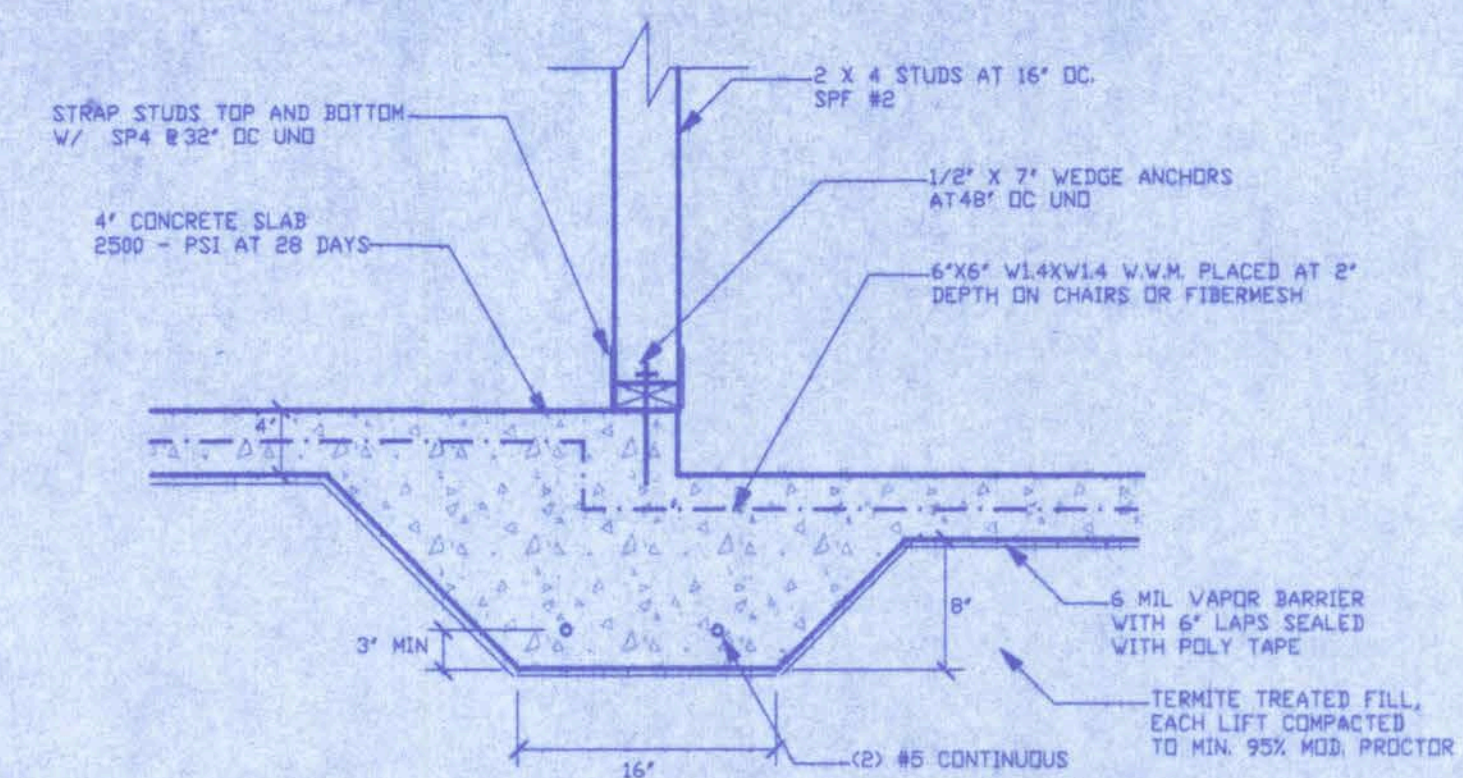
F4 - GARAGE DOOR POCKET

SCALE: 1" = 1'-0"



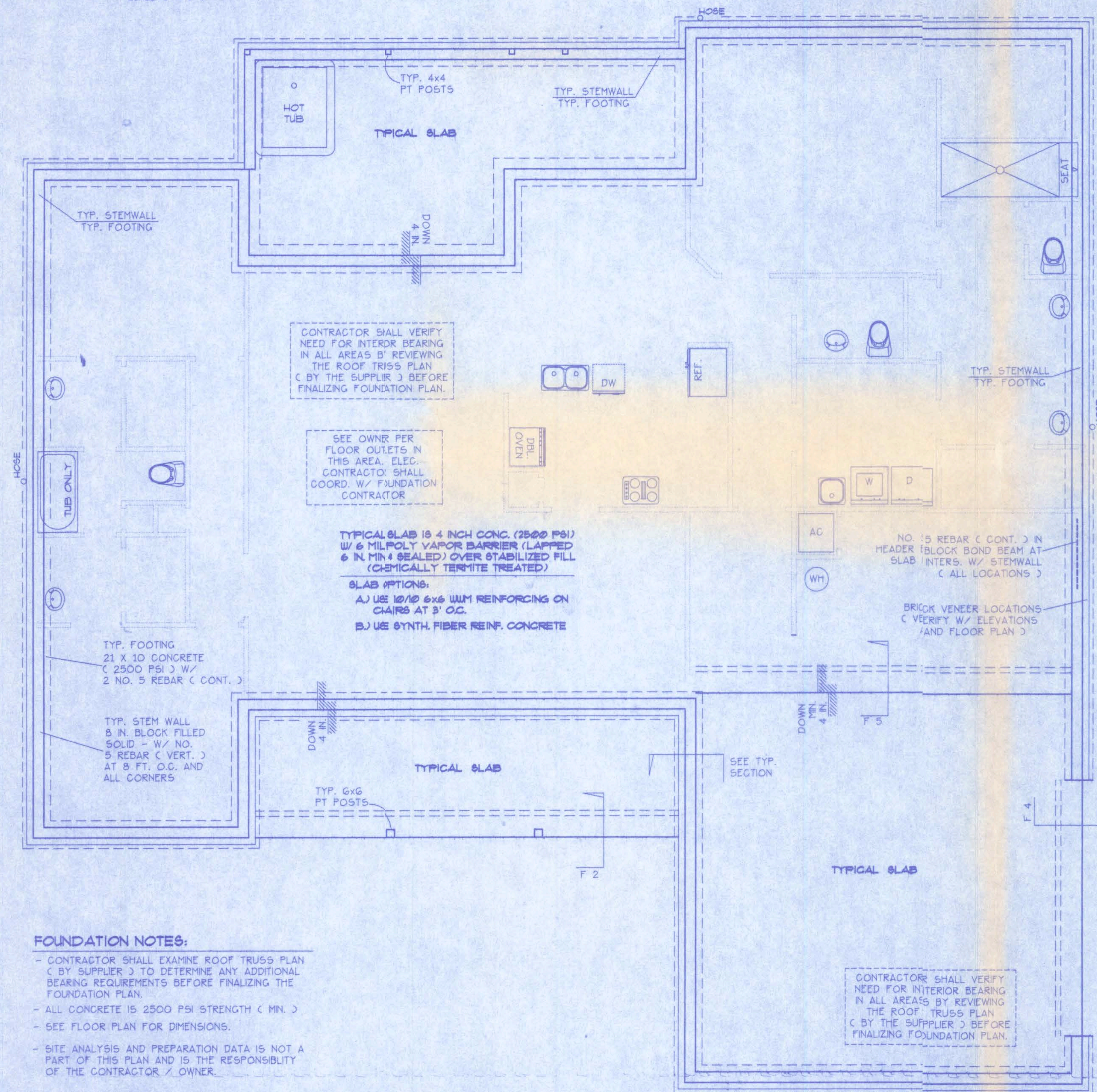
F2 - PORCH SLAB

SCALE: 1" = 1'-0"



F5 - INTERIOR BEARING STEP FOOTING

SCALE: 1" = 1'-0"

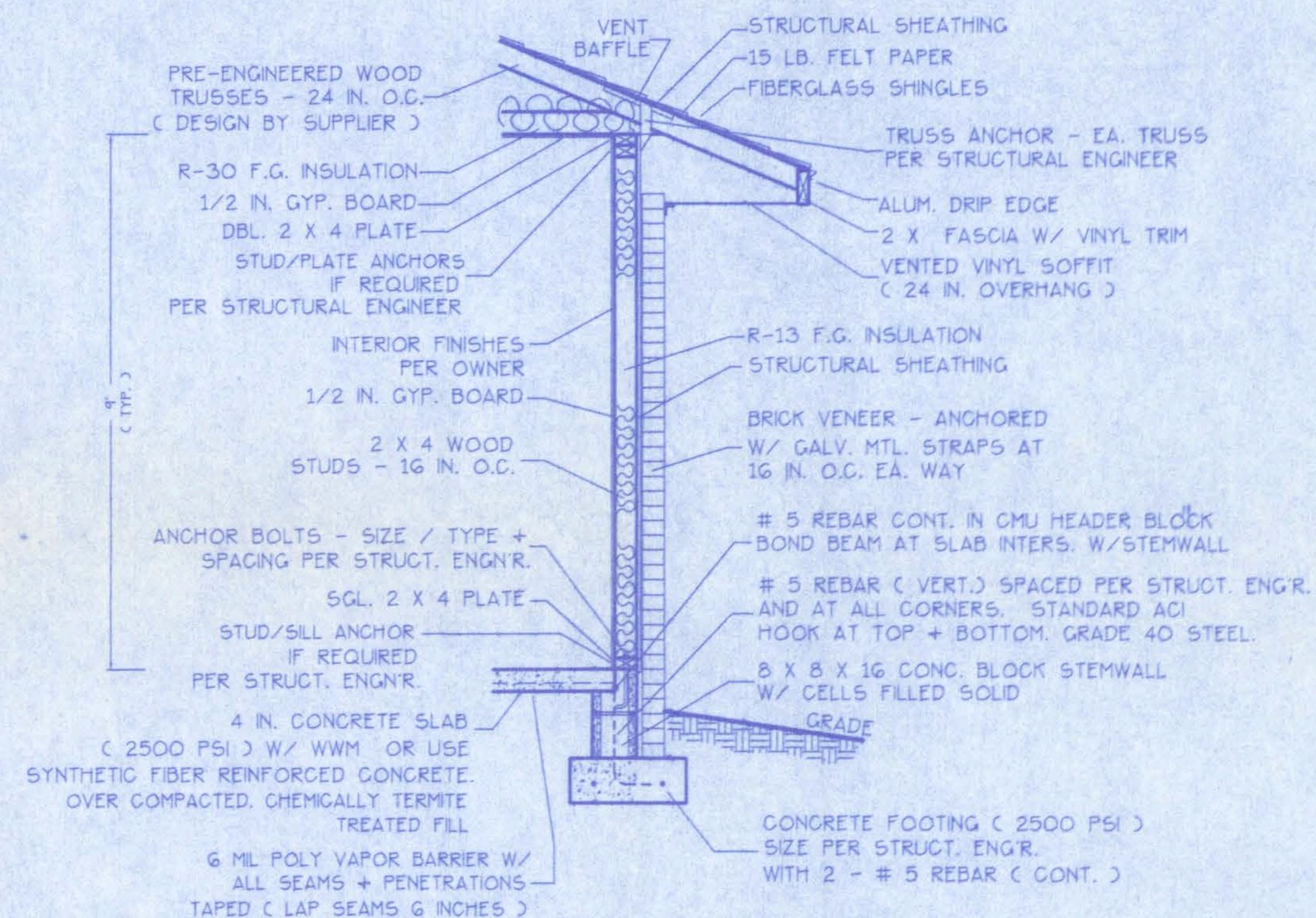


FOUNDATION PLAN

SCALE: 1/4 IN. = 1 FT.

FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- SEE FLOOR PLAN FOR DIMENSIONS.
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

DESIGN WALL SECTION

NON-STRUCTURAL DATA

SCALE: 1/2 IN. = 1 FT.

A-4

WINDLOAD ENGINEER: Mark Discey, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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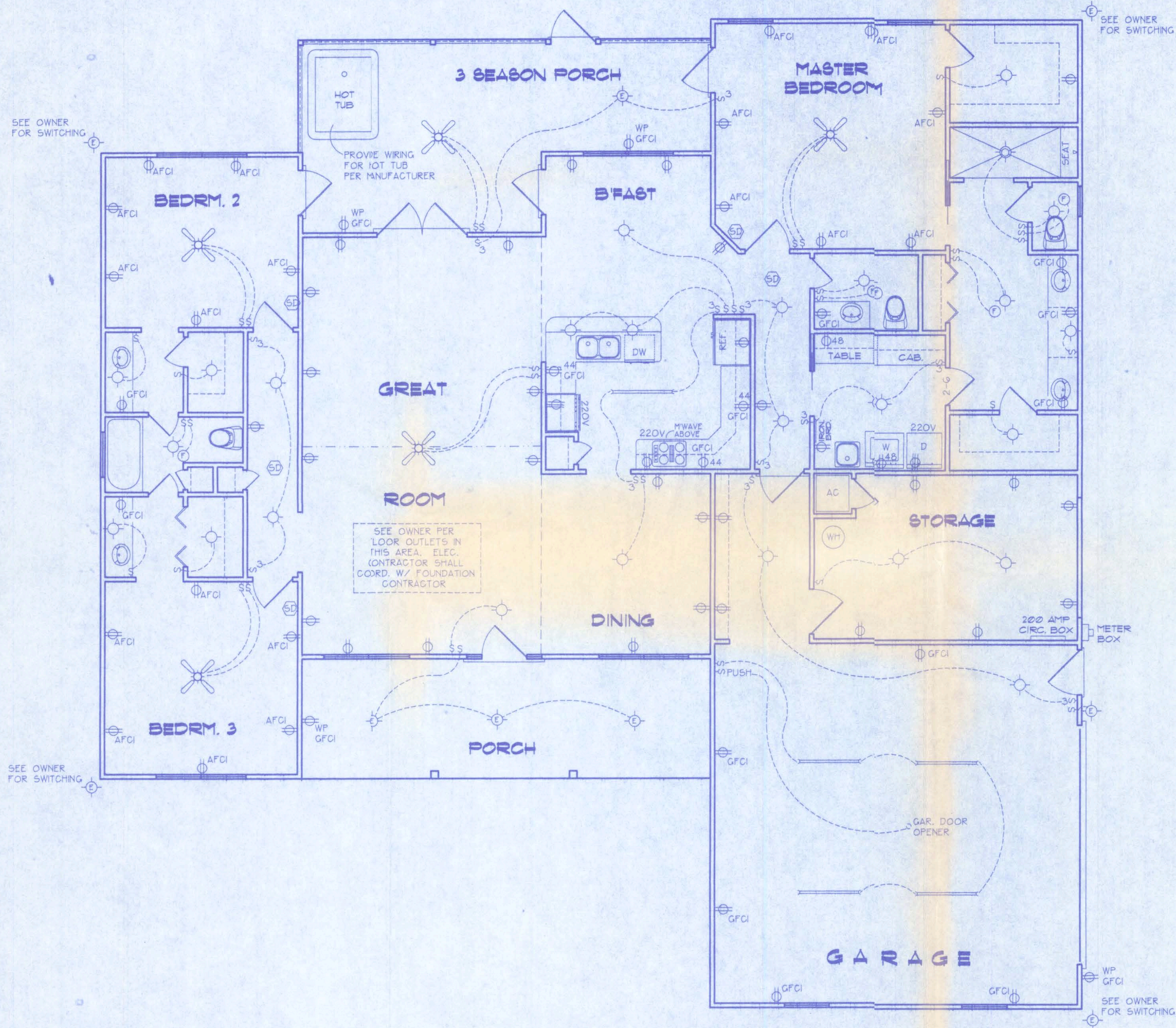
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2144 SW BRIM STREET LAKE CITY, FLORIDA Location: Job No.:

FILE: OG-002	MILLER RESIDENCE	SHEET: 4 OF 5
DATE: 2-27-06		CAD FILE: OG002
DRAWN: TAD	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: TAD	192 SW Sagewood Dr. Lake City, FL 32024 Phone (386) 755-5891	REV:

29 MAR 06





ELECTRICAL SYMBOL LEGEND	
	= FLOURESCENT LIGHTING FIXTURE.
	= CEILING LIGHT FIXTURE
	= EXTERIOR LIGHTING FIXTURE
	= LIGHT SWITCH.
	= THREE-WAY SWITCH.
	= 110 V. DUPLEX OUTLET.
	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
	= GROUND FAULT CIRC. OUTLET
	= ARC FAULT CIRC. OUTLET
	= 110 V. SINGLE RECEPTACLE OUTLET.
	= 220 VOLT OUTLET ( 4 WIRE )
	= FAN LOCATION ( CEILING )
	= FAN LOCATION ( EXHAUST )
	= SMOKE DETECTOR

#### ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD ) TO BE DETERMINED BY POWER COMPANY.

#### ELECTRICAL PLAN

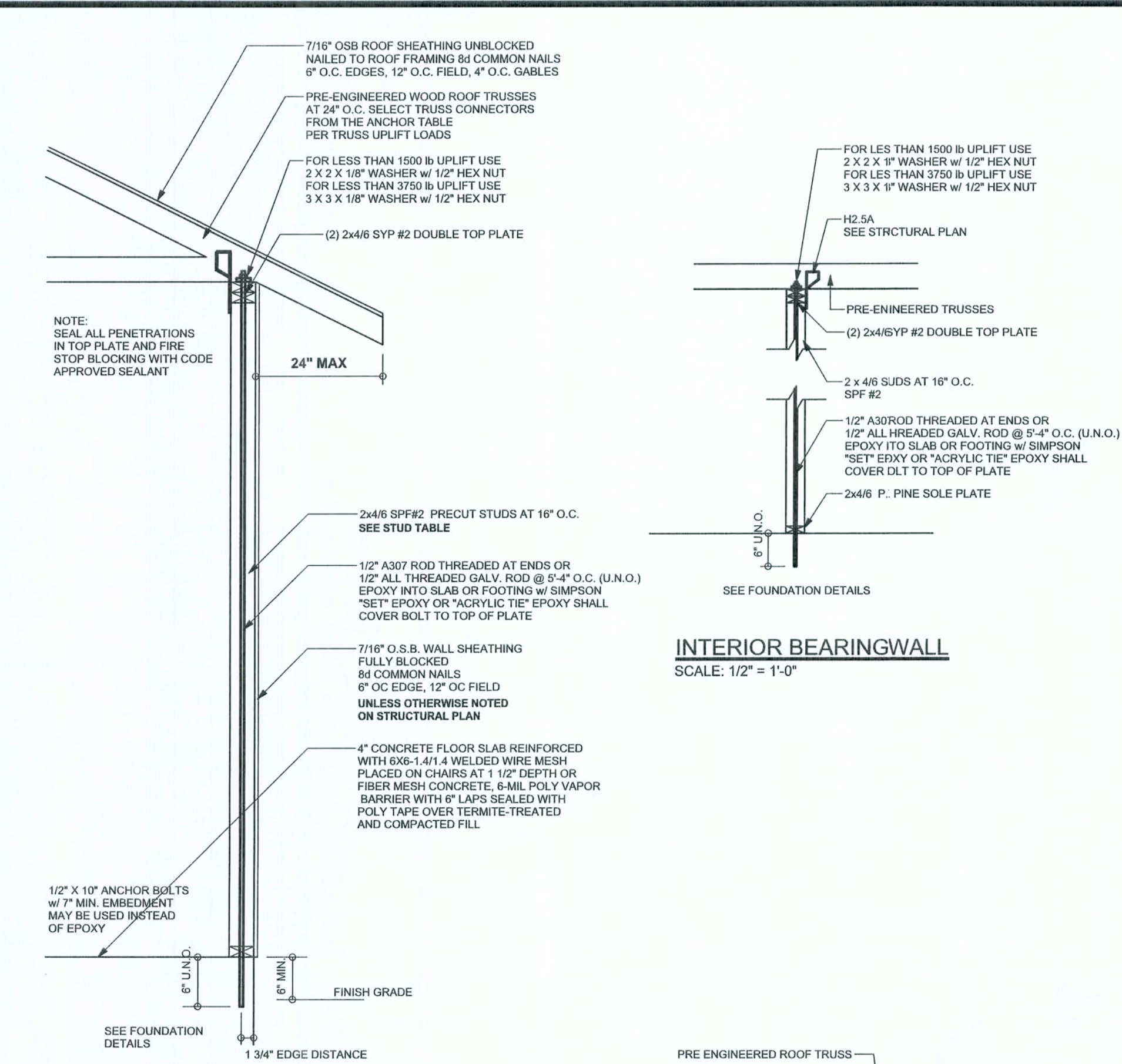
NOT TO SCALE

# A-5

2144 SW BRIM STREET  
LAKE CITY, FLORIDA

FILE: OG-002	<b>MILLER RESIDENCE</b>	SHEET: 5 OF 5
DATE: 2-27-06		CAD FILE: OG002
DRAWN: T A D	PREPARED BY: <b>TIM DELBENE</b> Drafting + Technical Services 192 SW Sagewood Cir., Lake City, FL 32024 Phone ( 386 ) 755-5891	REV:
CHECK: T A D		REV:



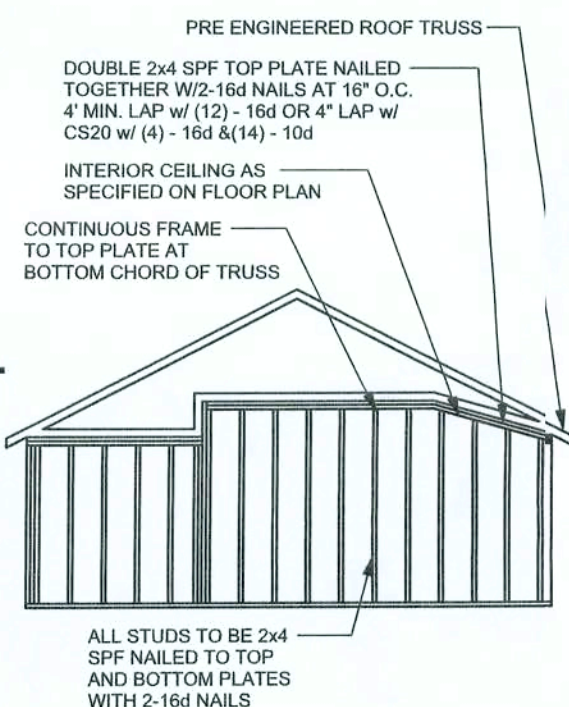


**ONE STORY WALL SECTION**  
SCALE: 3/4\" = 1'-0"

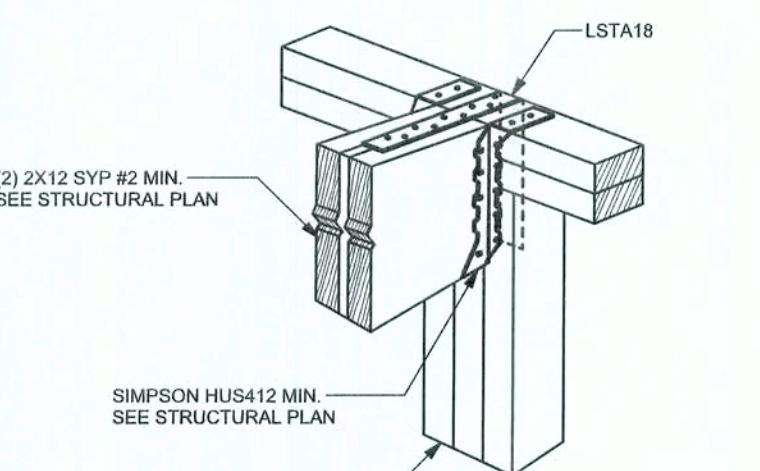
#### EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-0" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

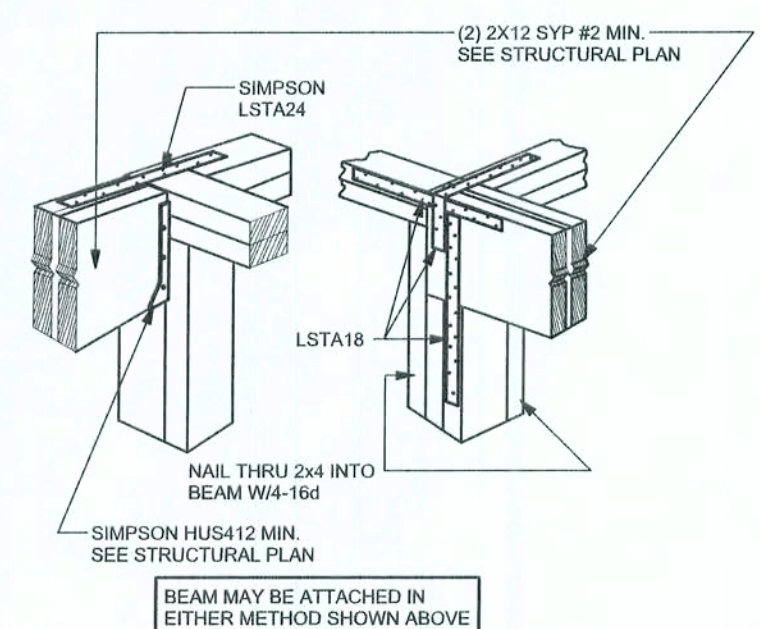
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



**CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL**  
SCALE: N.T.S.

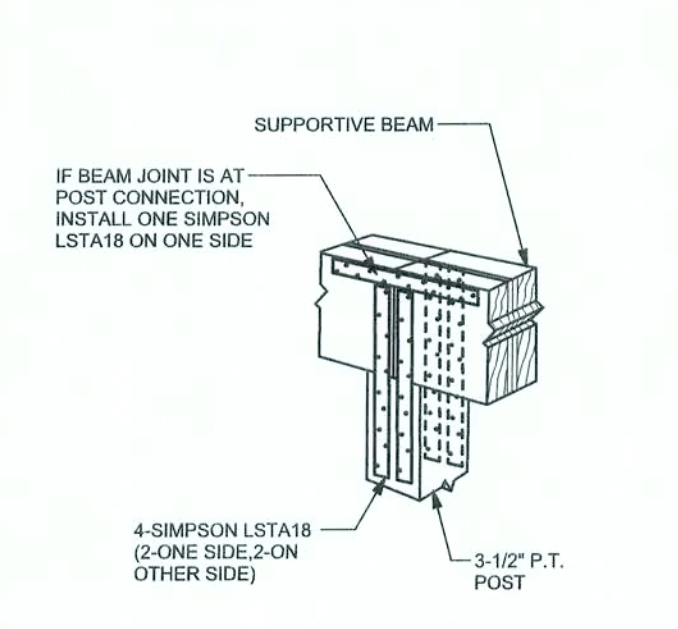


**BEAM MID-WALL CONNECTION DETAIL**  
SCALE: N.T.S.

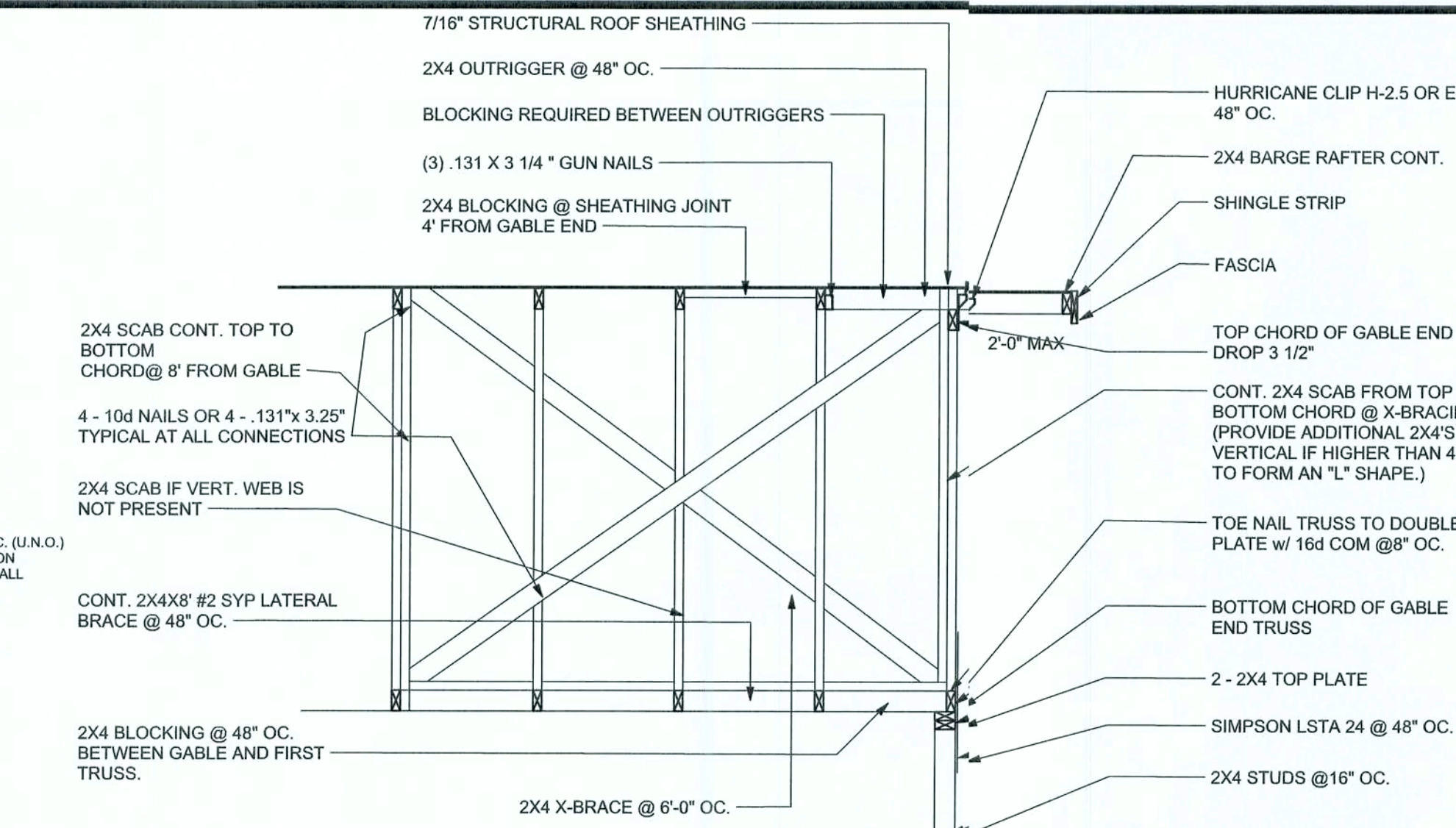


**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.

**SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.

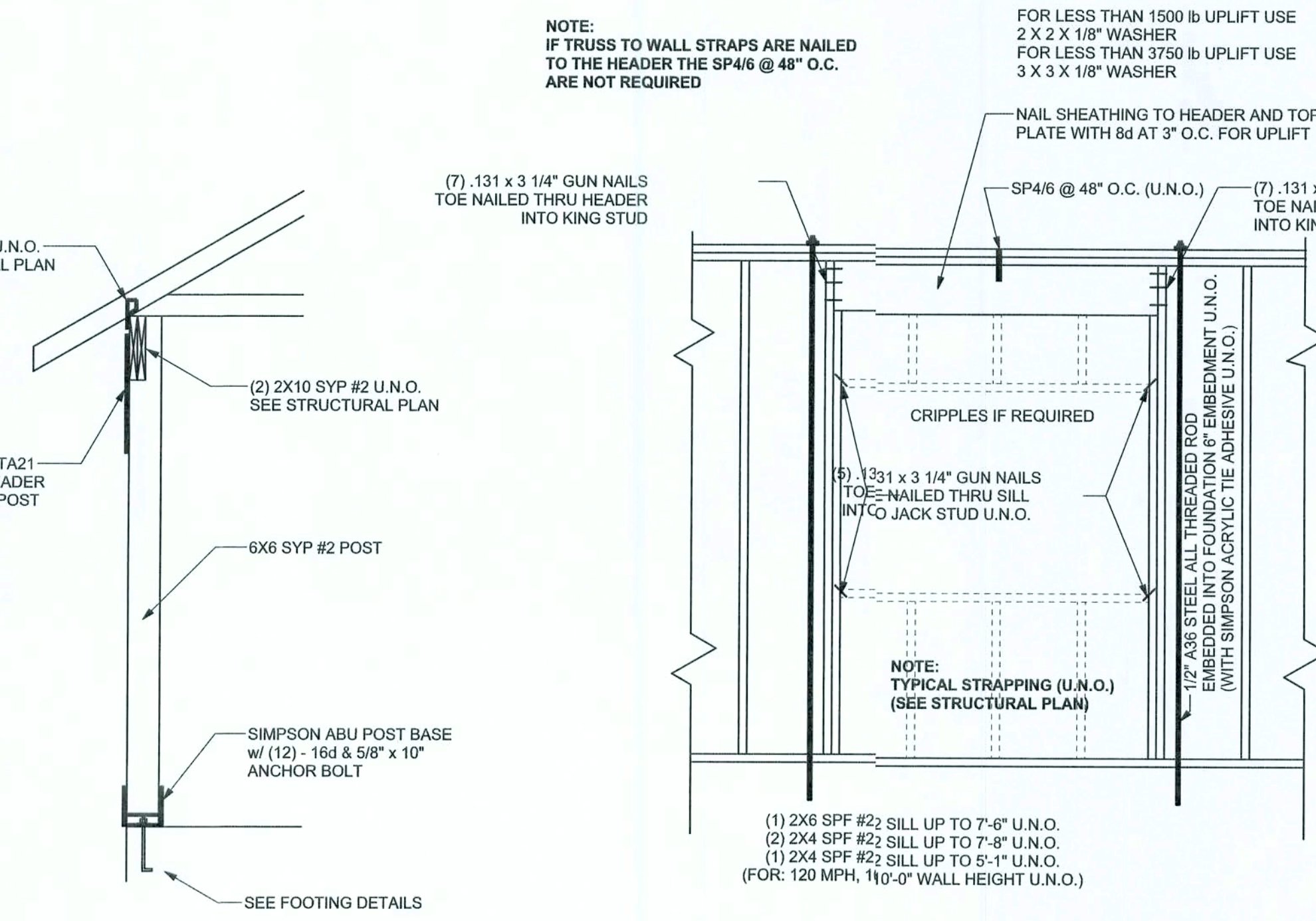


**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.

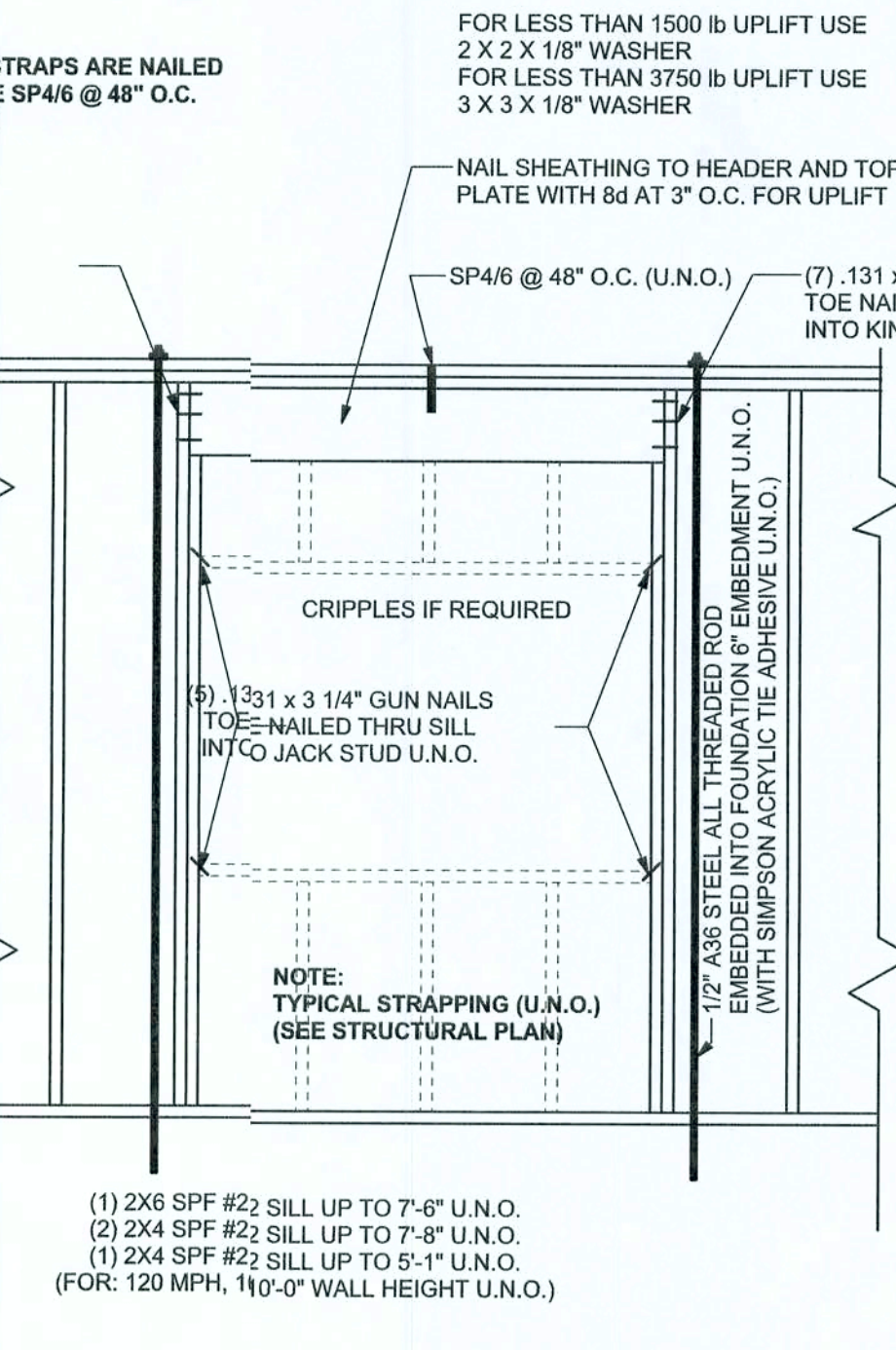


**TYPICAL GABLE END (X-BRACING)**

ALL MEMBERS SHALL BE SYP



**TYPICAL PORCH POST DETAIL**  
SCALE: 1/2\" = 1'-0"



**TYPICAL 1 STORY HEADER STRAPPING DETAIL**  
SCALE: 1/2\" = 1'-0"

#### ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-16d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-16d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2490	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 - 16d	14 - 16d	
<b>HEAVY GIRDER TIEDOWNS*</b>					<b>TO FOUNDATION</b>
< 3965	< 3330	MG1		22 - 10d	1-5/8\"
< 10980	< 6485	HGT-2		16 - 10d	2-5/8\"
< 10530	< 9035	HGT-3		16 - 10d	2-5/8\"
< 9250	< 9250	HGT-4		16 - 10d	2-5/8\"
<b>STUD STRAP CONNECTOR*</b>					<b>TO STUDS</b>
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 8d		
< 1705	< 1705	CS16	28 - 8d		
<b>STUD ANCHORS*</b>				<b>TO STUDS</b>	<b>TO FOUNDATION</b>
< 1350	< 1305	LTT19	8 - 16d		1/2\"
< 2310	< 2310	LTT101	18 - 10d, 1 1/2"		1/2\"
< 2775	< 2570	MD2A	2-6\"		5/8\"
< 4175	< 3695	HTT16	18 - 16d		5/8\"
< 1400	< 1400	PAHD42	16 - 16d		
< 3335	< 3335	HPAHD22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1/2\"
< 2300	< 2300	ABU66	12 - 16d		1/2\"
< 2320	< 2320	ABU88	18 - 16d		2-5/8\"

**GARAGE DOOR BUCK INSTALLATION DETAIL**  
SCALE: N.T.S.

#### GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING SHALL BE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE TRUSS STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 4x15.8 EACH END, 2X8 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $P_c = 3000$  PSI.

**WELDED WIRE REINFORCED SLAB:** 6\" x 6\" W14 x W1.4, FB = 85KSL WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3\".

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCED STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS,  $F_y = 60$  KSI. ALL LAP SPLICES @ 48\" (25\" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, UNO.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-VSP,  $F_b = 2.4ksi$ ,  $E = 1800ksi$ ; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16\" O.S.B SHEATHING, UNBLOKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4\" O.C. UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\" IN CONCRETE OR REINFORCED BOND BEAM OR 12\" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2\" BOLTS TO BE 2\" x 2\" x 9/64\"; WITH 5/8\" BOLTS TO BE 3\" x 3\" x 9/64\"; WITH 3/4\" BOLTS TO BE 3\" x 3\" x 9/64\"; WITH 7/8\" BOLTS TO BE 3\" x 3\" x 5/16\"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BUILDER'S RESPONSIBILITY

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

#### ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACINGS. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

#### DESIGN DATA

##### WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	19.9	-21.8	-18.1
2	19.9	-25.5	-18.1
2 Onq	-40.6	-40.6	-40.6
3	19.9	-25.5	-18.1
3 Onq	-68.3	-42.4	-42.4
4	21.8	-23.6	-18.5
5	21.8	-29.1	-18.5

Doors & Windows	21.8	-29.1
Worst Case (Zone 5, 10 ft2)		
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

#### DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

#### REVISIONS


**SOTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,  
P.E. No.53915, PCB 866, Lake City, FL  
32056, 386-754-919

**DIMENSIONS:** Stated dimension supersede scaled dimensions. Note all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

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PRINTED DATE:  
March 29, 2006

DRAWN BY: STRUCTURAL BY:  
David Disoway

FINALS DATE:  
29 / Mar / 06

JOB NUMBER:  
603271

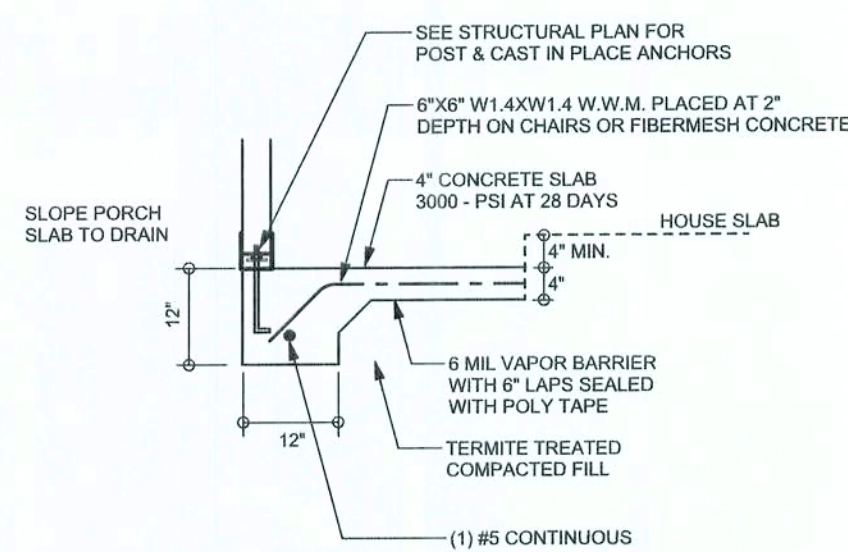
DRAWING NUMBER

**S-1**  
OF 3 SHEETS

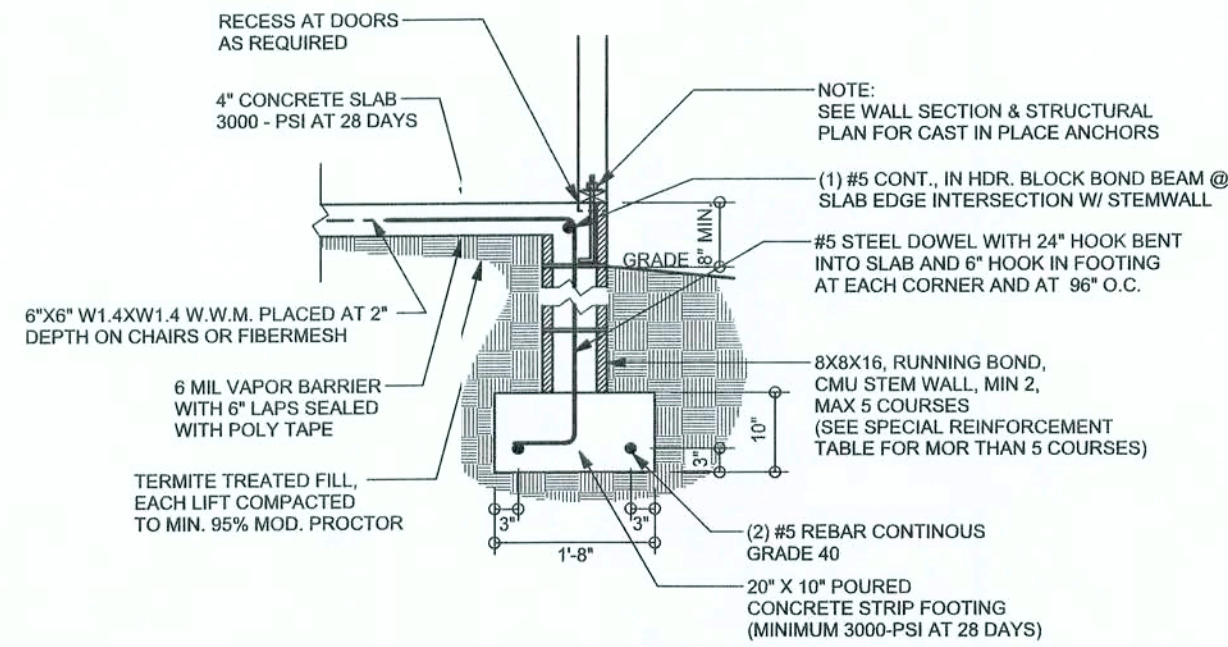


REVISIONS

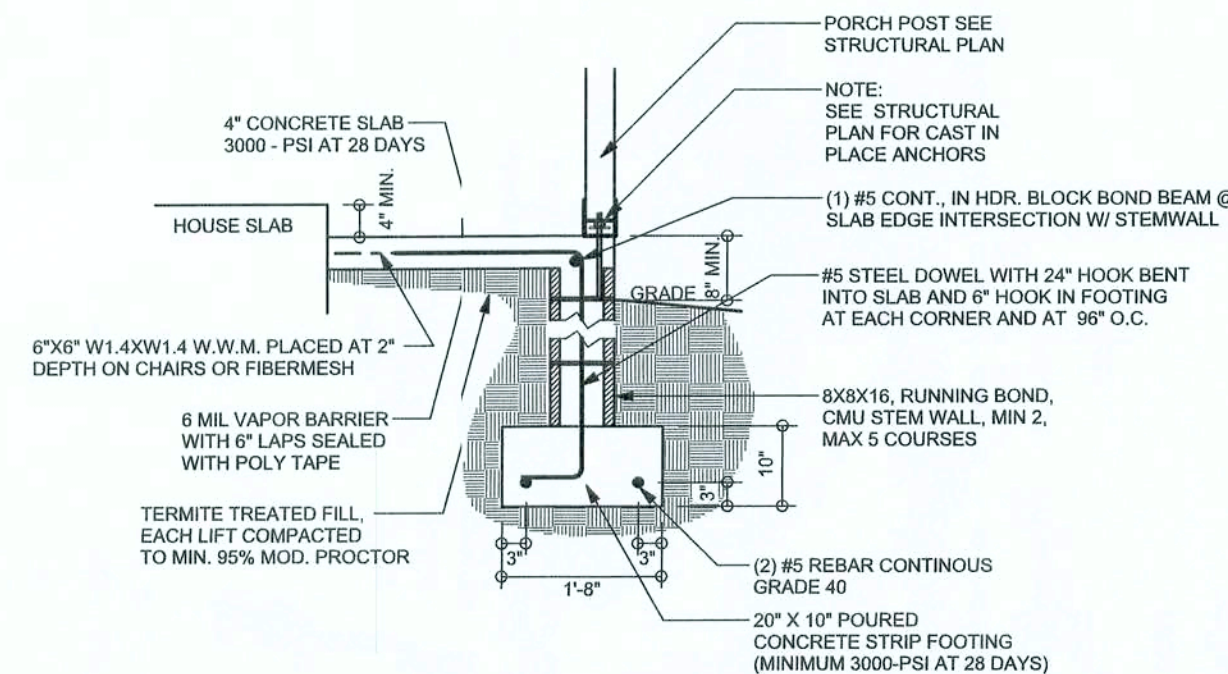
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



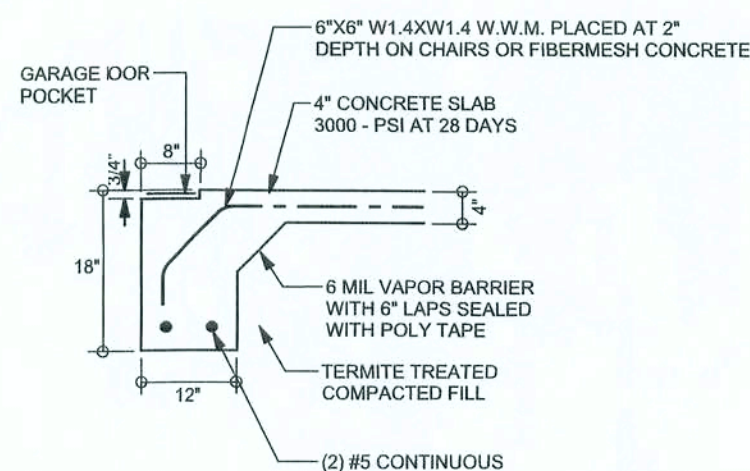
**F5**  
**S-2** PORCH FOOTING  
SCALE: 1/2" = 1'-0"



**F9**  
**S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



**F12**  
**S-2** ALT. STEM WALL PORCH FOOTING  
SCALE: 1/2" = 1'-0"



**F13**  
**S-2** ALT. STEM WALL GARAGE DOOR FOOTING  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE**

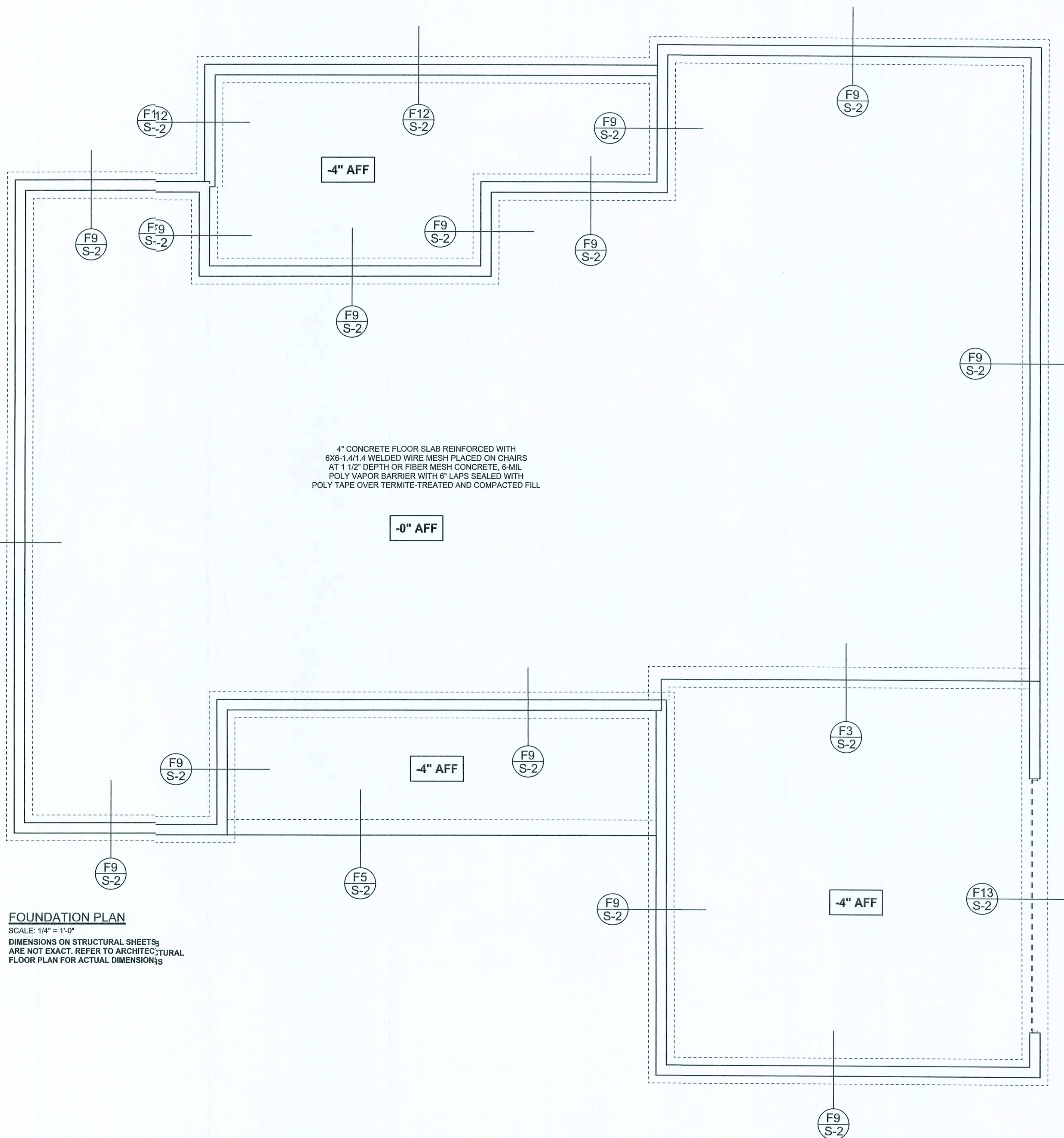
The table assumes 80 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 16S continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

**FOUNDATION PLAN**

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, F.O.B. 868, Lake City, FL  
32056, 386-54-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY  
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**Construction**

**Ron & Brenda**  
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March 29, 2006

**DRAWN BY:** David Disosway  
**STRUCTURAL BY:** David Disosway

**FINALS DATE:**  
29 / Mar / 06

**JOB NUMBER:**  
603271

**DRAWING NUMBER**

**S-2**

OF 3 SHEETS



REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS1-B1, BCS1-B2, & BCS1-B3, BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

MSTA30, 10-104 (1700lb)  
(5) NAILS EACH SIDE OF STUD  
(OR STRAP STUD TO HEADER 20-104)

LTT208, 10-164 (1750lb)  
1/2" ANCHOR w/ 6" EMBEDMENT U.N.O., SIMPSON  
AT (MAY BE RECESSED BELOW FINISHED FLOOR)

ALTERNATE WALL TIE CONNECTION WHERE  
THREADED ROD CANNOT BE PLACED IN WALL.  
SCALE: 1/2" = 1'-0"

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

- INDICATES LOCATION OF:  
1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF:  
2ND FLOOR 1/2" A307 ALL THREADED ROD

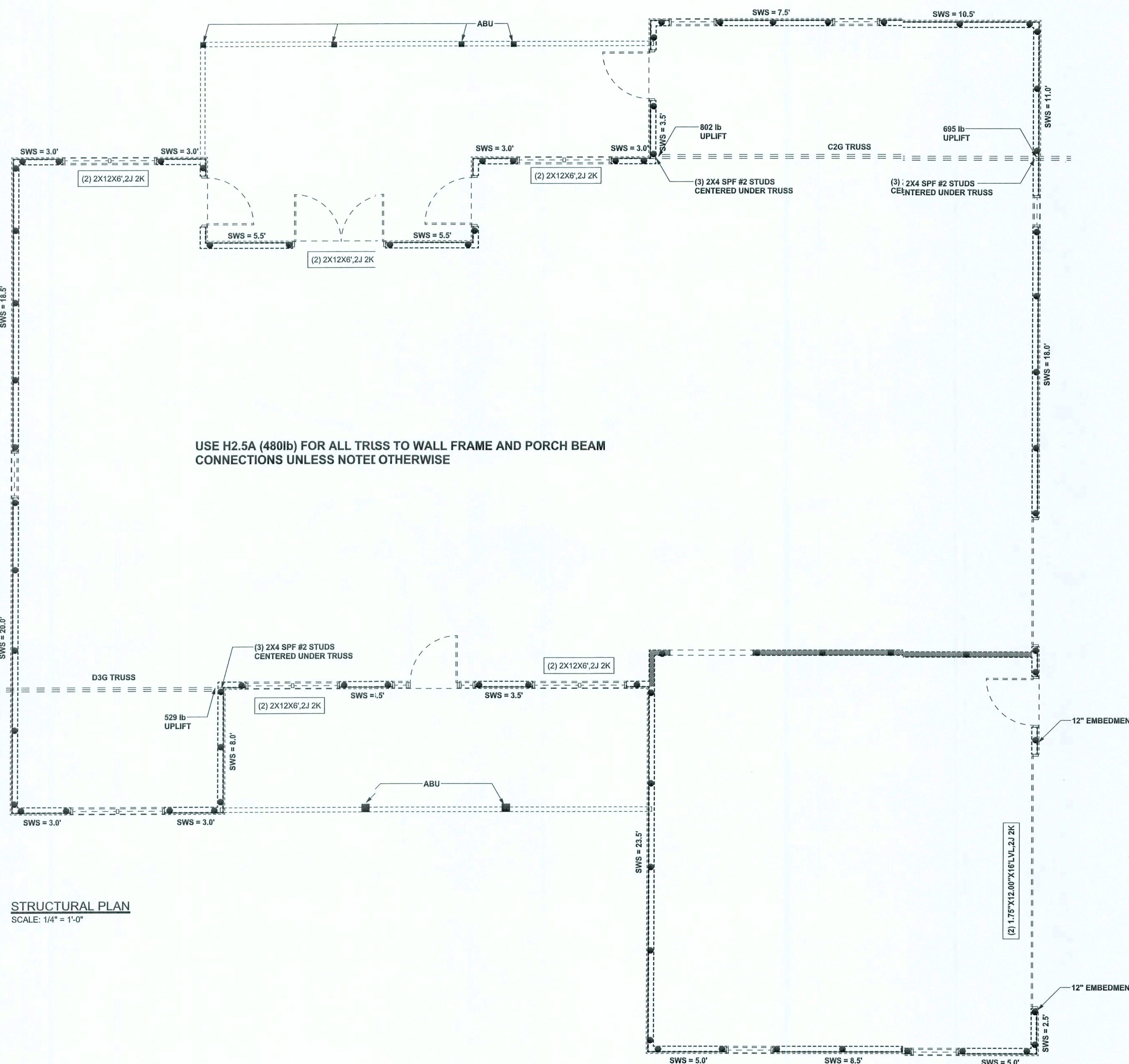
HEADER LEGEND

- (2) 2X12X0', 1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	32.5'	105.0'
LONGITUDINAL	29.5'	72.5'



USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM  
CONNECTIONS UNLESS NOTED OTHERWISE

STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, 706 868, Lake City, FL  
32056, 386-75-5419

DIMENSIONS  
Stated dimensions supercede scaled  
dimensions. Refer all questions to  
Mark Disosway, P.E. for resolution.  
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CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
portions of this plan, relating to wind engineering  
comply with section R301.2.1, Florida building  
code residential 2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
building, at specified location.

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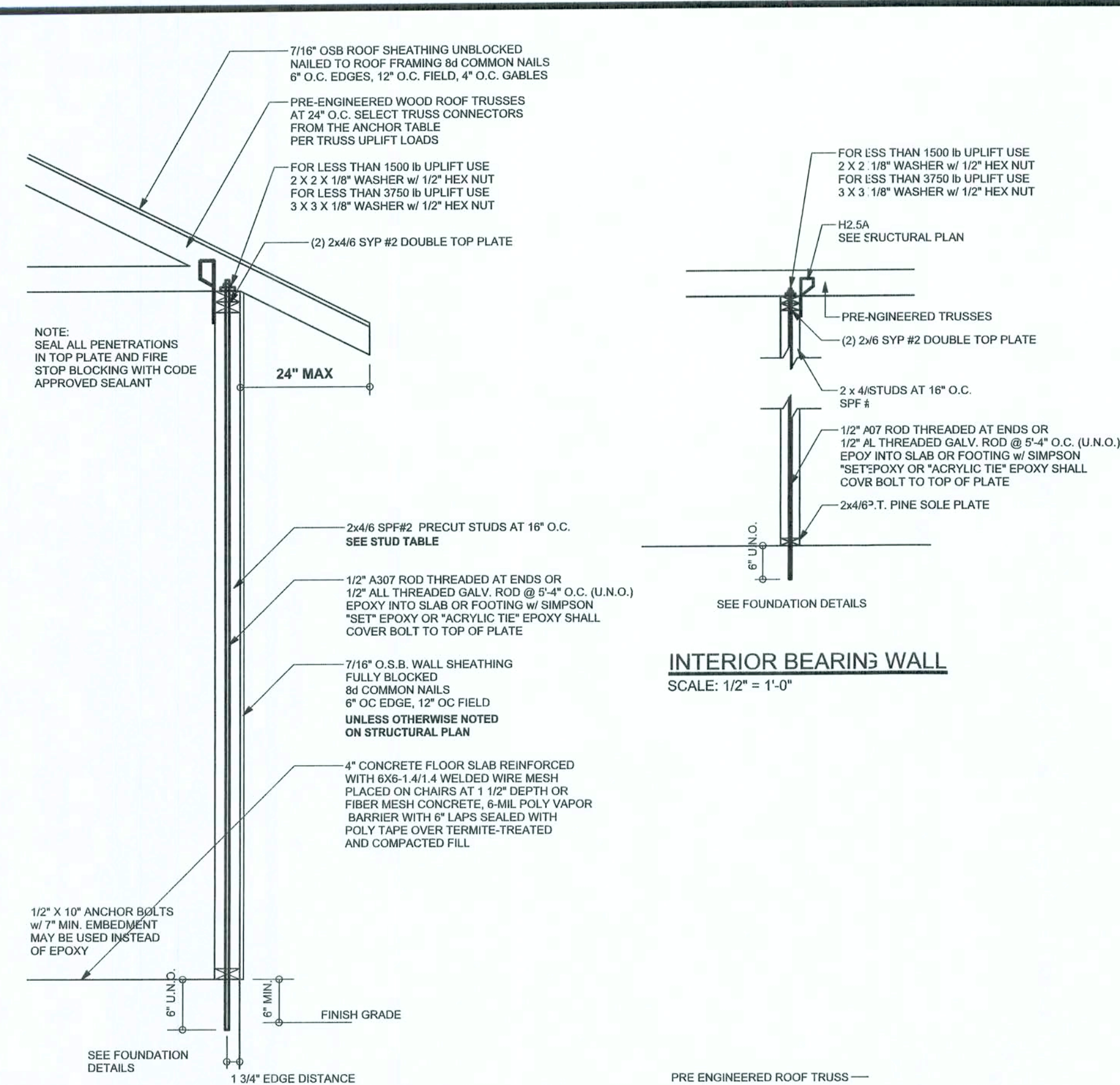
DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER, ANDERSON TRUSS CO.  
JOB #6-123



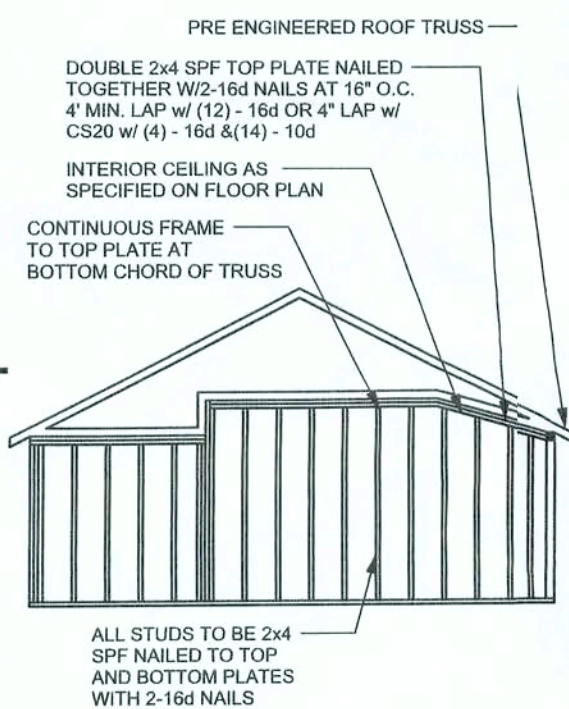


**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

#### EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

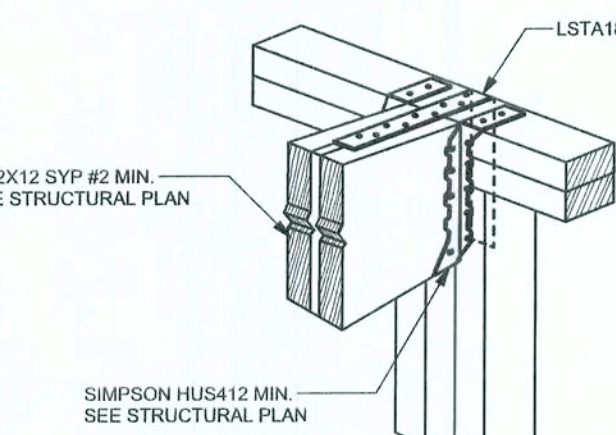
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.208. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13" O.C.



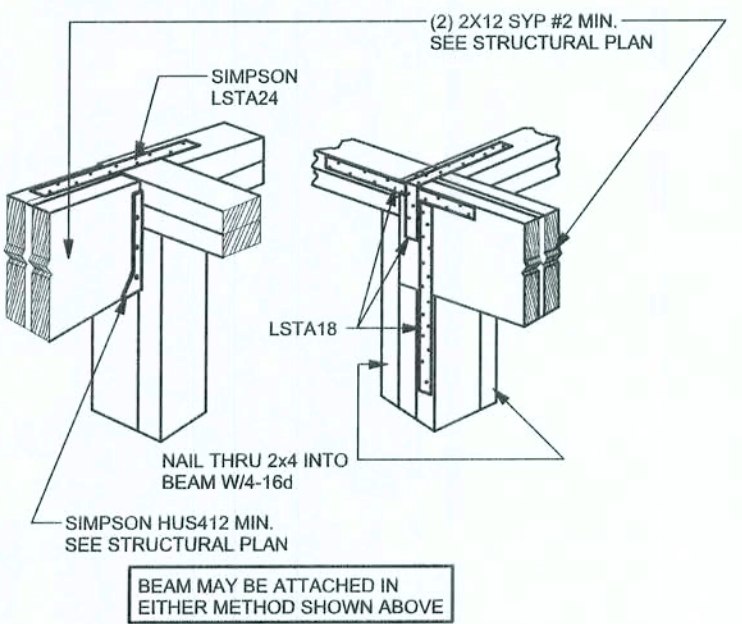
#### CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



#### BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.

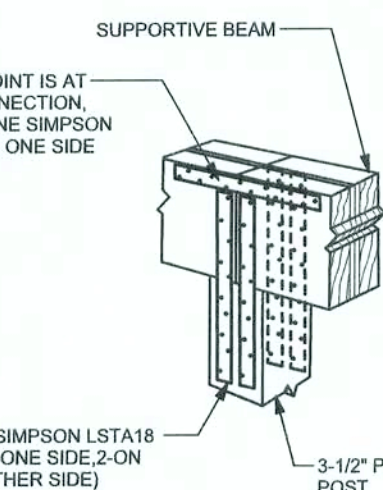


#### BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

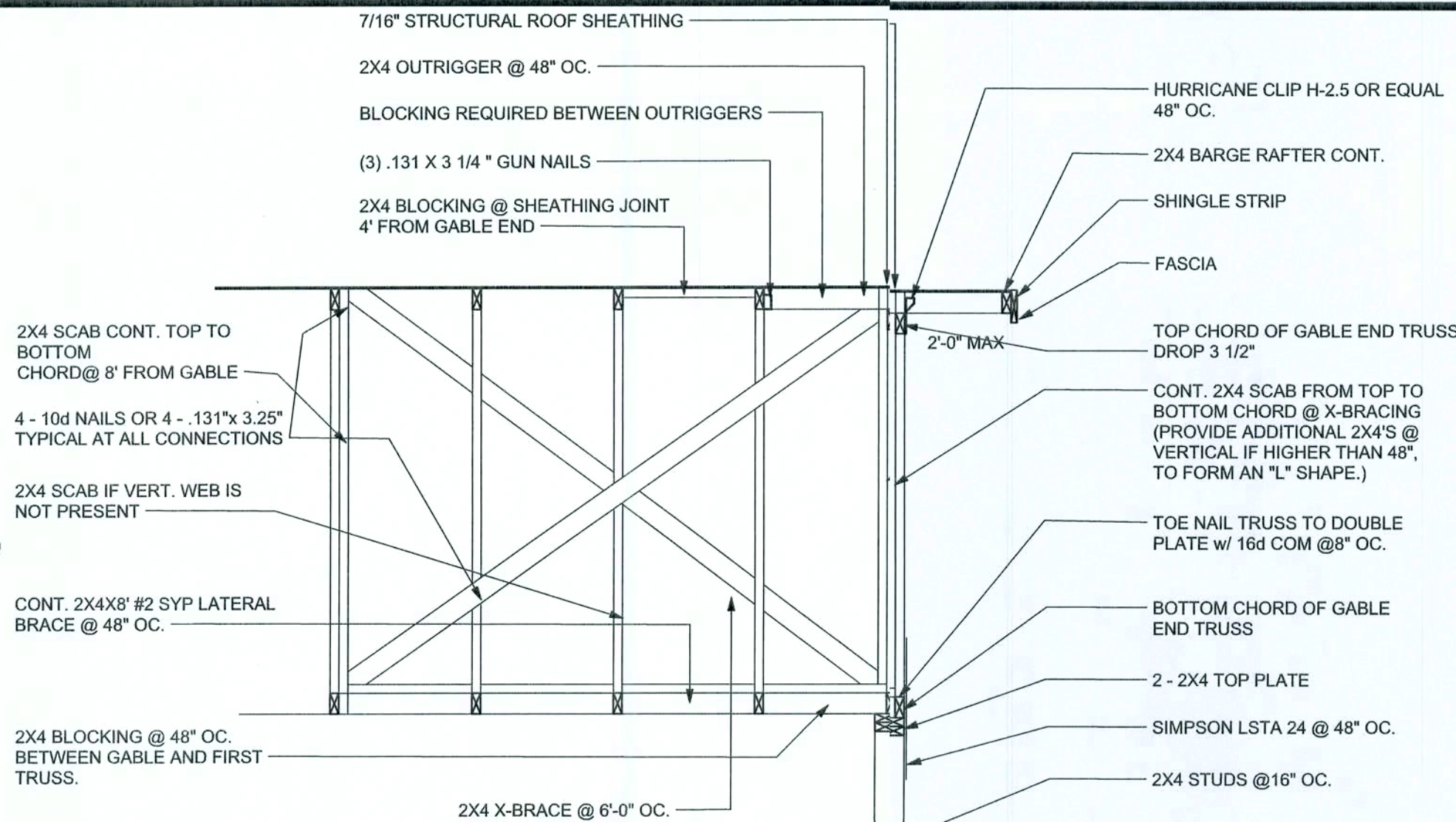
#### SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.



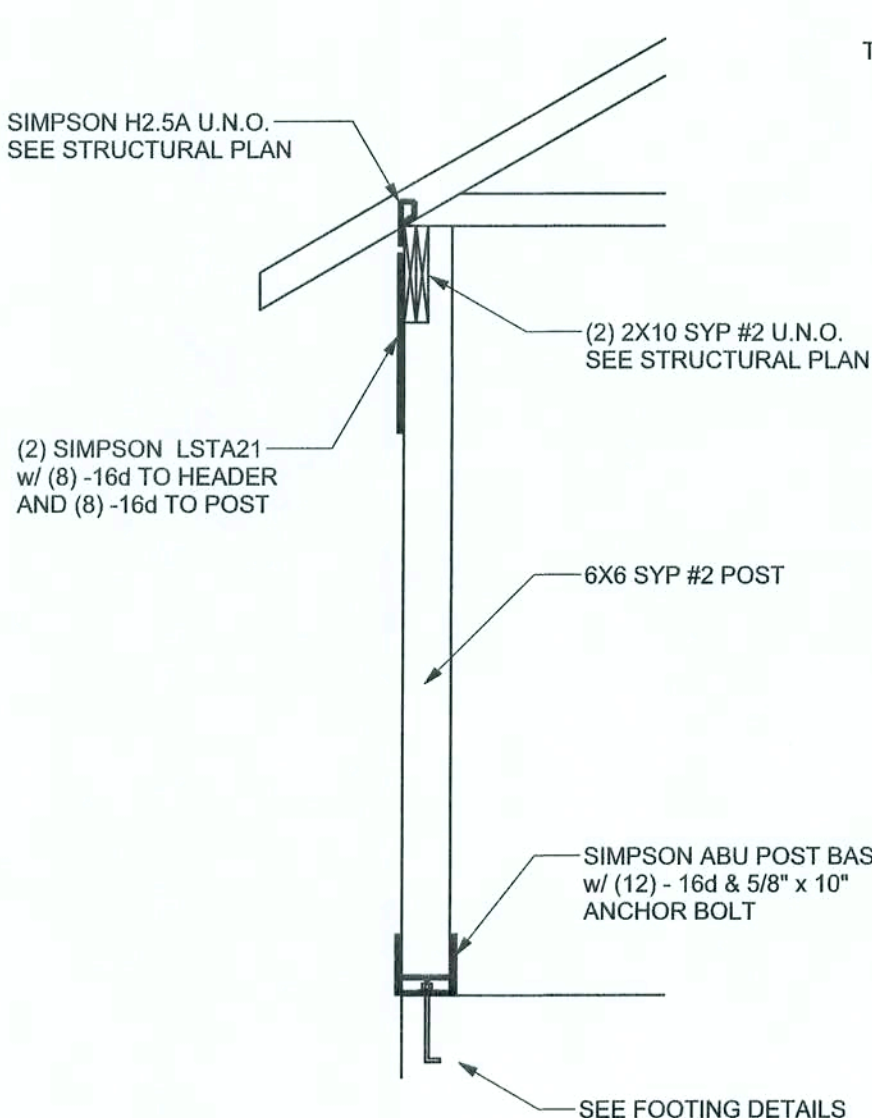
#### SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.



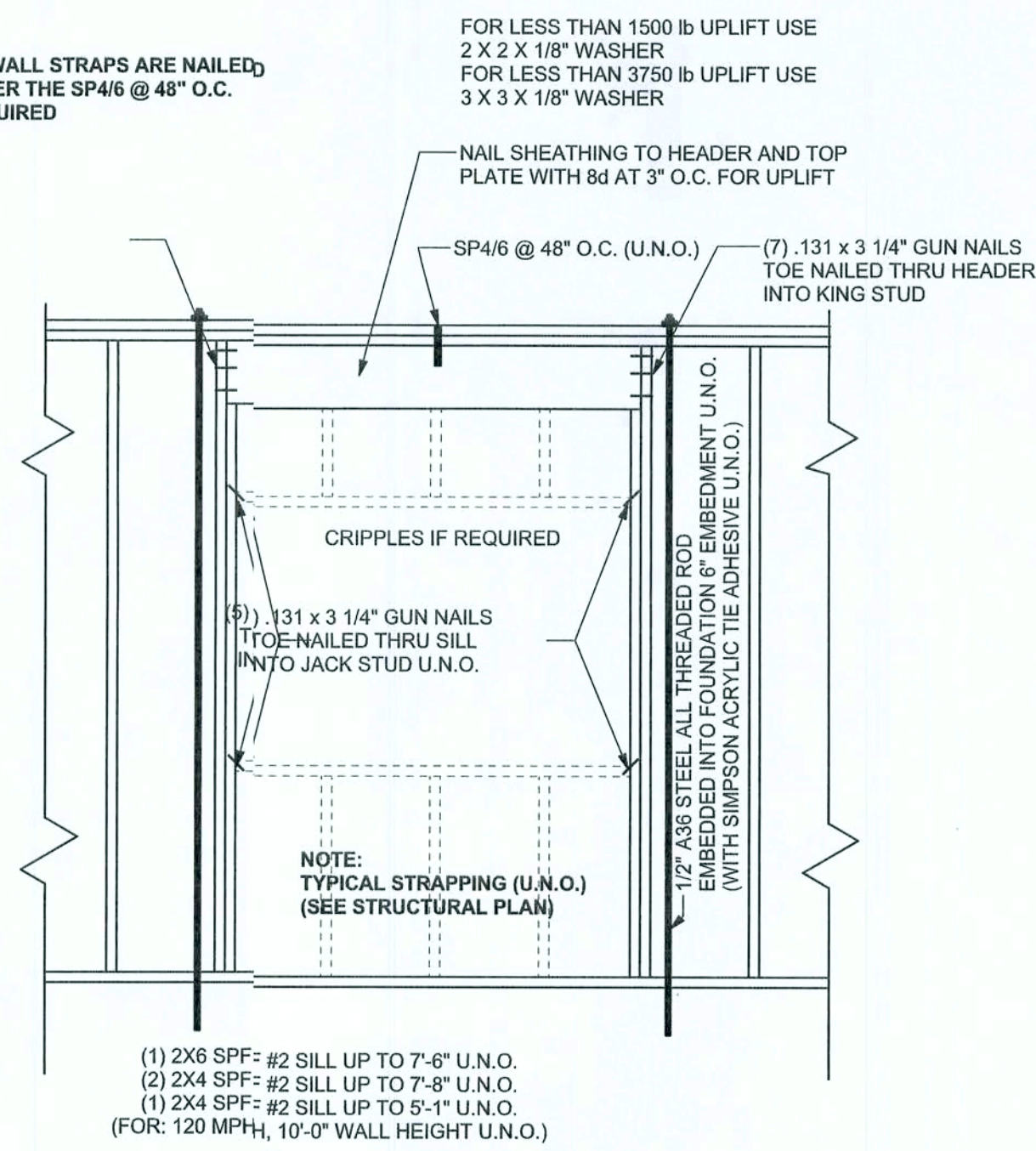
#### TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP



#### TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"



#### TYPICAL 1-STORY HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

#### ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1285	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1285	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1600	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MGT		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10880	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	16-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H20A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	16-16d		5/8" AB
< 1400	< 1400	PAID42	16-16d		
< 3335	< 3335	HPAH22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	16-16d		2-5/8" AB

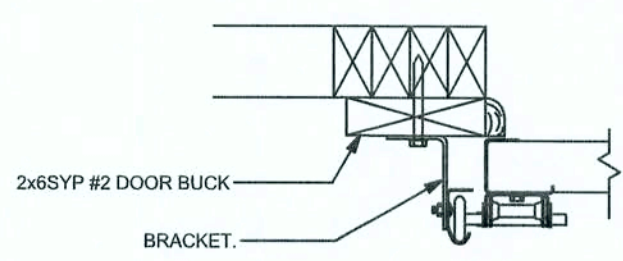
#### GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

#### 2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8\"/>

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF 131 x 3 1/4" ON
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



#### GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

#### GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X6 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W1.4 x W1.4, F<sub>y</sub> = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI. ALL LAP SPICES 40 x DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2,484, E = 1,800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. **ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY. 4"OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BUILDER'S RESPONSIBILITY

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

#### ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

#### DESIGN DATA

##### WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE  
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	
	10	100
1	19.9 -21.8	18.1 -18.1
2	19.9 -25.5	18.1 -21.8
2 Ohg	-40.6	-40.6
3	19.9 -25.5	18.1 -21.8
3 Ohg	-68.3	-42.4
4	21.8 -23.6	18.5 -20.4
5	21.8 -29.1	18.5 -22.6
Doors & Windows Worst Case (Zone 5, 10 ft <sup>2</sup> )	21.8	-29.1
6x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

##### DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

#### REVISIONS

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,  
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**DIMENSIONS:** Stated dimensions approximate scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

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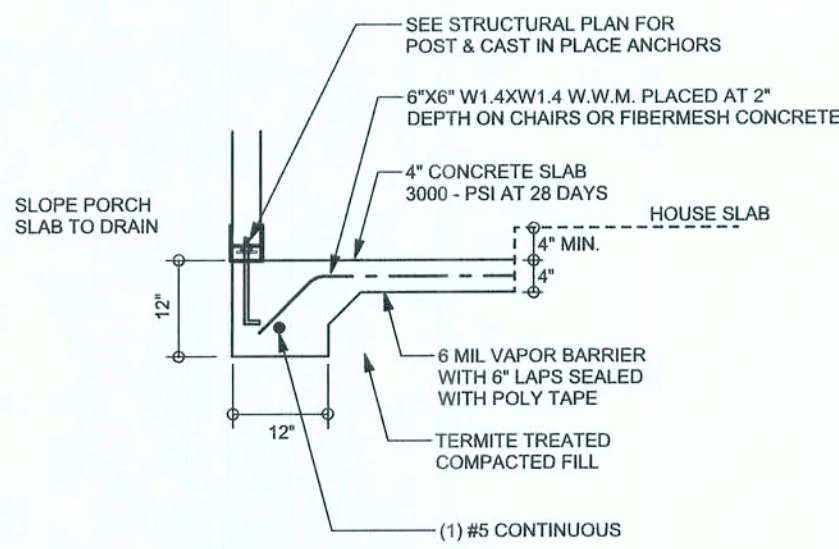
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603271  
**DRWING NUMBER**

**S-1**

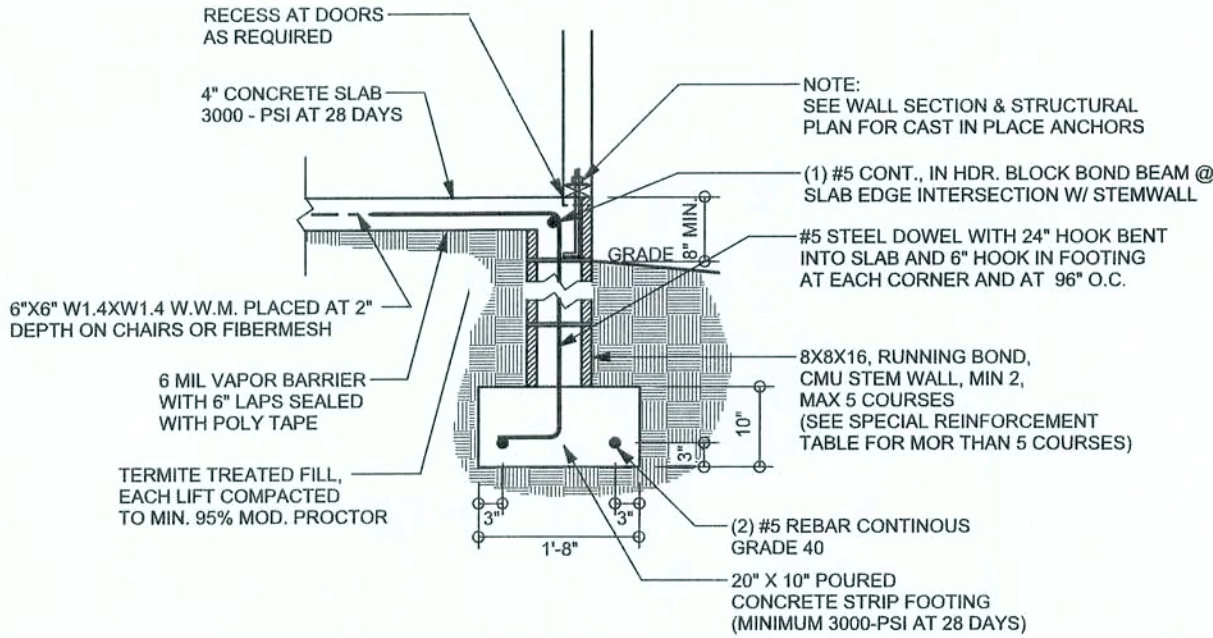
OF 3 SHEETS



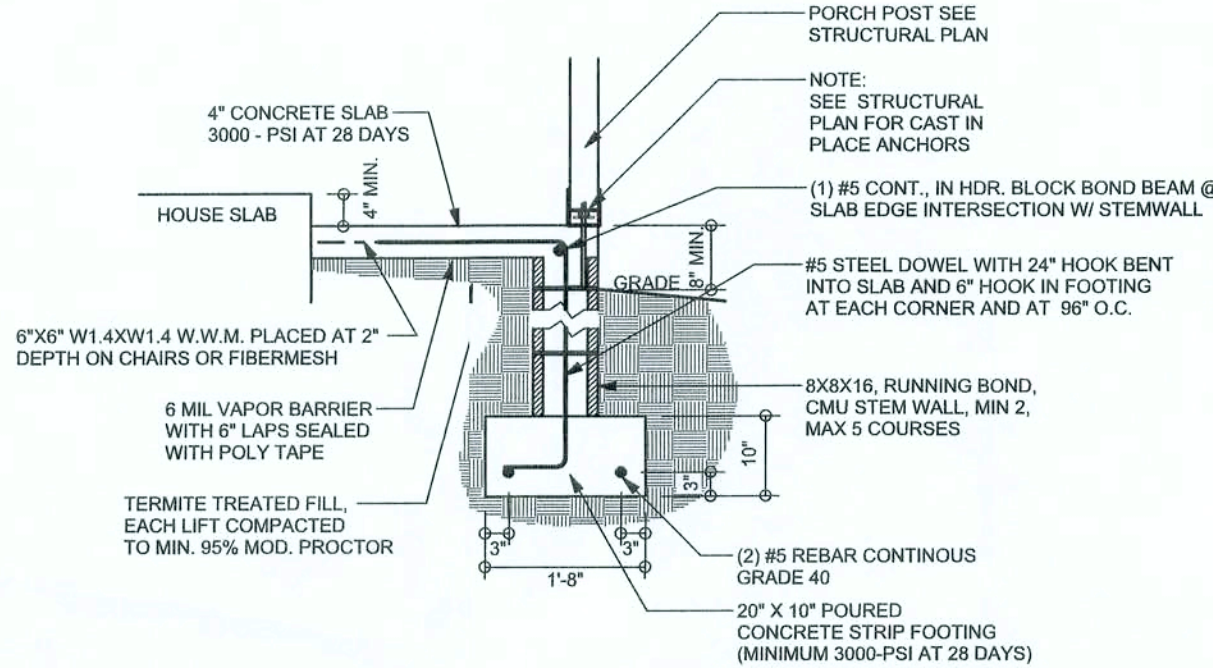
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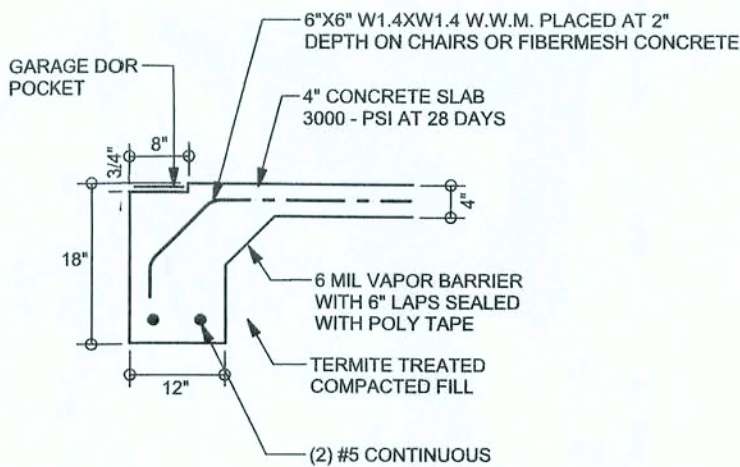
**F5**  
**S-2** PORCH FOOTING  
SCALE: 1/2" = 1'-0"



**F9**  
**S-2** STEM WALL FOOTING  
SCALE 1/2" = 1'-0"



**F12**  
**S-2** ALT STEM WALL PORCH FOOTING  
SCALE 1/2" = 1'-0"

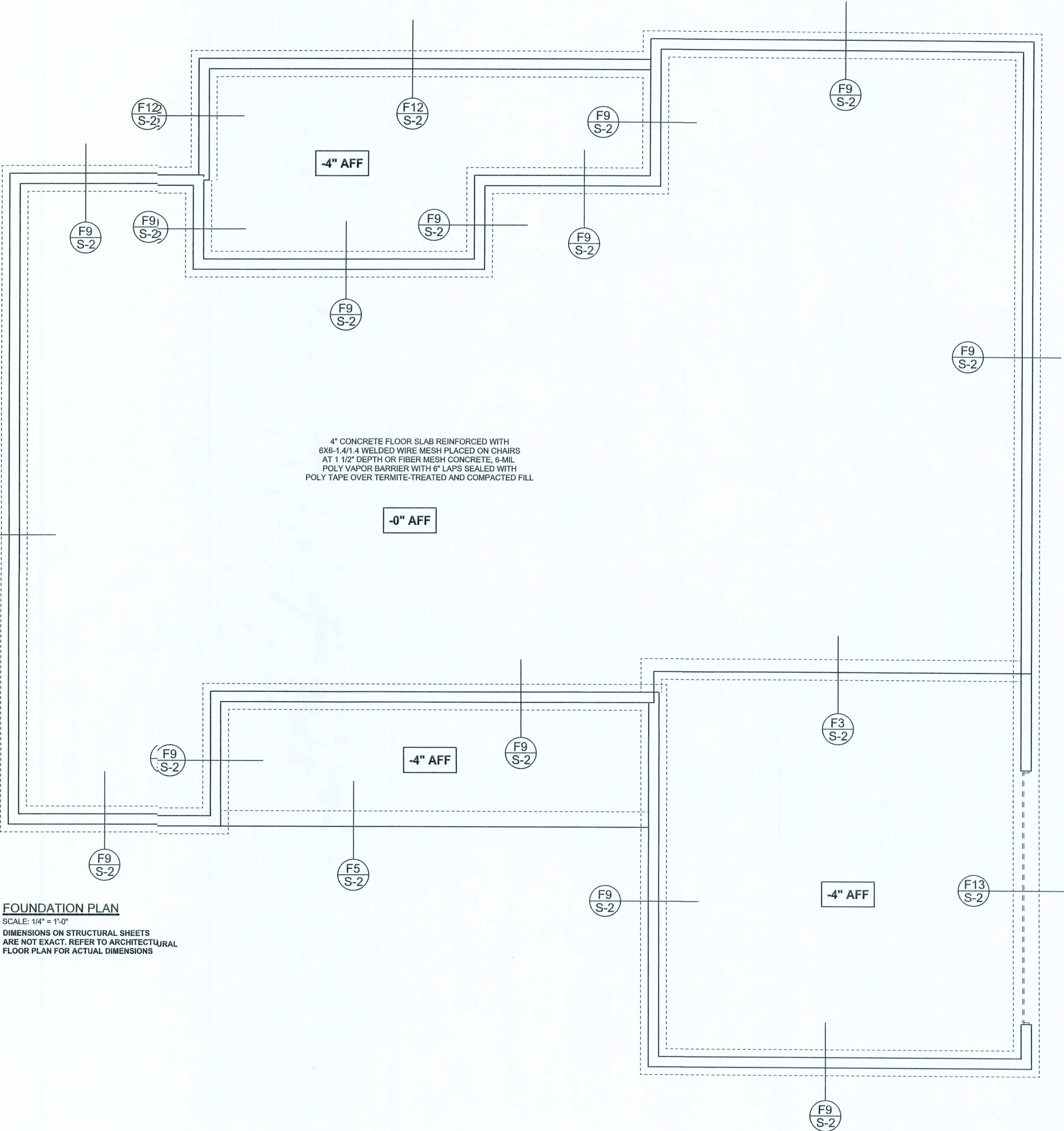


**F13**  
**S-2** ALT. STEM WALL GARAGE DOOR FOOTING  
SCALE: 1/2"= 1'-0"

#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcement bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duralwall ladderreinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)	
		#5	#8	#5	#8
3.3	3.0	5	96	96	96
4.0	3.7	5	96	96	96
4.7	4.3	8	96	96	96
5.3	5.0	5	96	96	96
6.0	5.7	0	80	96	96
6.7	6.3	2	56	80	96
7.3	7.0	3	40	56	80
8.0	7.7	5	32	48	64
8.7	8.3	1	24	32	48
9.3	9.0	1	16	24	40



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, P.E. No.53915, 808 868, Lake City, FL 32056, 386-7545415

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Rer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section FC001.2.1, Florida building code residential2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

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Mach 29, 2006

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29 / Mar / 06

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603271

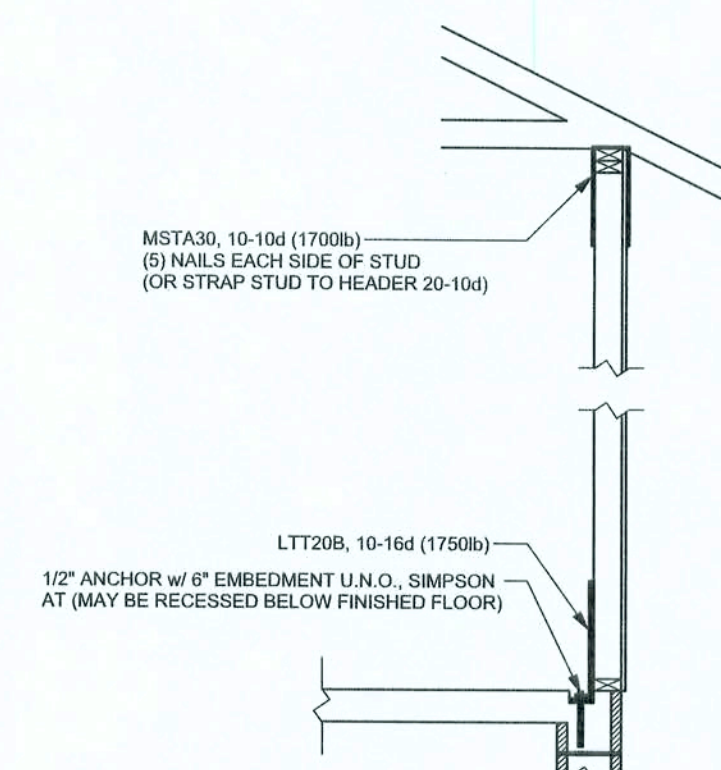
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**S-2**  
OF 3 SHEETS



# REVISIONS

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ALTERNATE WALL TIE CONNECTION WHERE  
THREADED ROD CANNOT BE PLACED IN WALL.  
SCALE: 1/2" = 1'-0"

## STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

## THREADED ROD LEGEND

- INDICATES LOCATION OF:  
1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF:  
2ND FLOOR 1/2" A307 ALL THREADED ROD

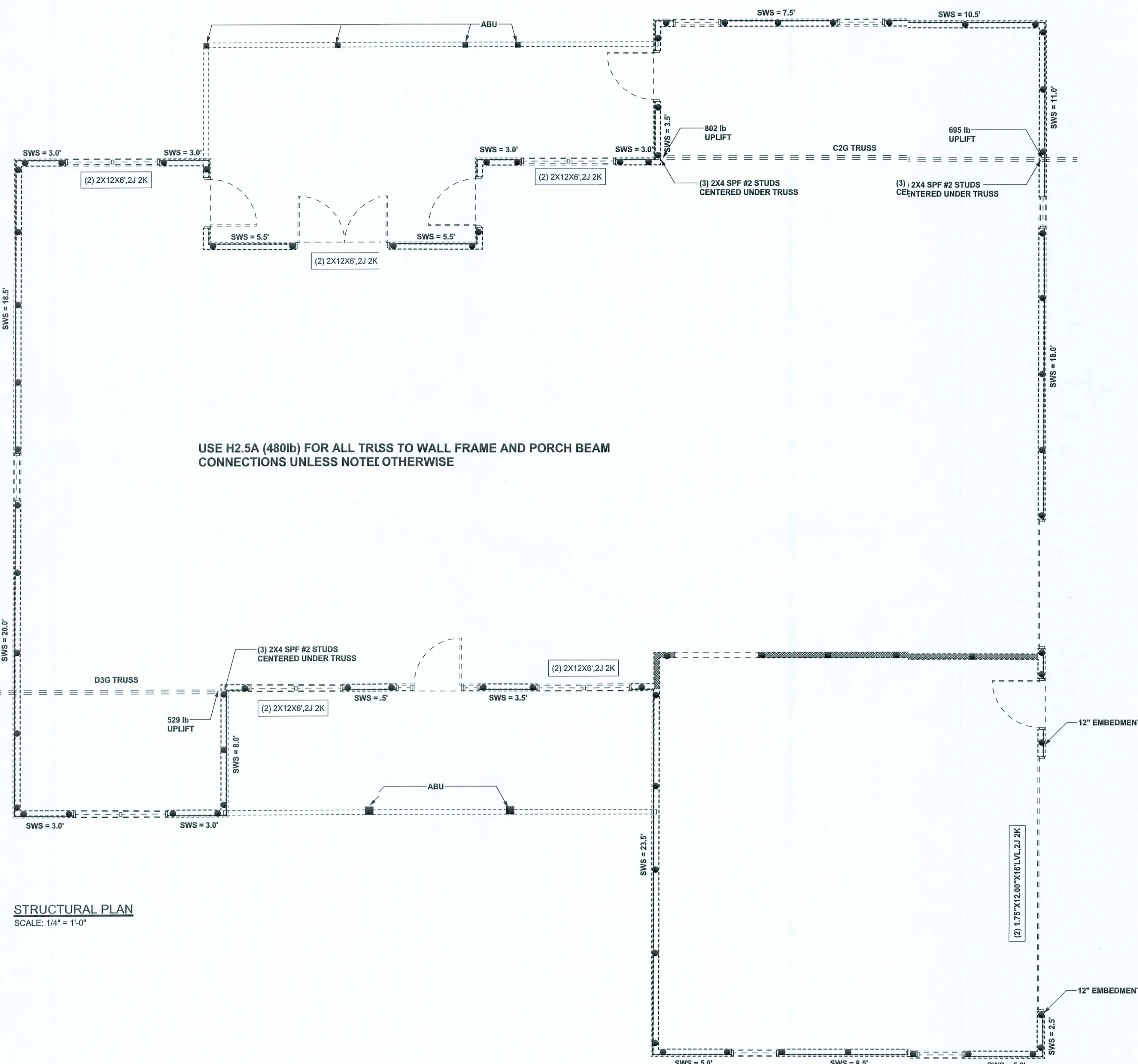
## HEADER LEGEND

- (2) 2X12X0', 1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

## TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	32.5'	105.0'
LONGITUDINAL	29.5'	72.5'



STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM  
CONNECTIONS UNLESS NOTED OTHERWISE

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, P.O. Box 888, Lake City, FL  
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DIMENSIONS  
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dimensions. Refer all questions to  
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portions of the plan, relating to wind engineering  
comply with section P301.2.1, Florida building  
code residential 2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
building, at specified location.

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PRINTED DATE:  
March 29, 2006

DRAWN BY: STRUCTURAL BY:  
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FINALS DATE:  
29 / Mar /06

JOB NUMBER:  
603271

DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER, ANDERSON TRUSS CO.  
JOB #6-123