

Floor Plan

AREA SUMMARY

Living Area	2307	S.F.
Front Porch Area	302	S.F.
Rear Porch Area	310	S.F.
Total Area	2819	S.F.

Note: Cantilever Front and Rear Porch Trusses



RESIDENCE

Wnnett

719 SW Jappy Jack Tr.
Lake City, FL 32024

ADDRESS:
Columbia County, Florida

Woodman Park Builders, Inc.
Lake City, Florida
Phone: (386) 755-2411
Fax: (386) 755-8684
Email:

PRINTED DATE:
Thursday, March 11, 2021

DESIGNED BY:
Mark Haddox

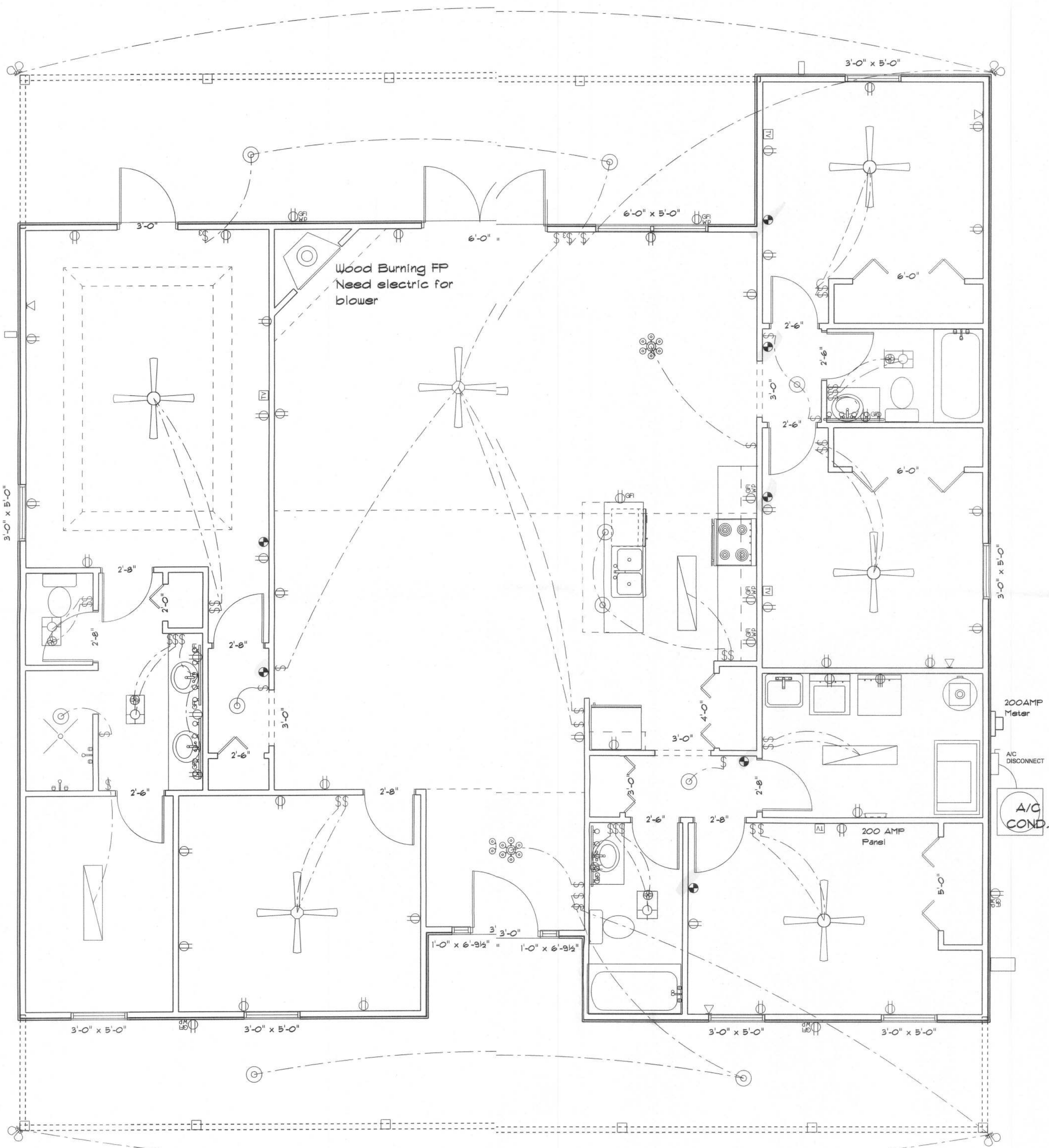
DRAWING NUMBER

A-1



REVISIONS	

- Electrical Plan Notes:
- E-1 Wire all appliances, HVAC units and other equipment per manufactures specifications.
 - E-2 Consult the owner for the number or separate telephone lines to be installed. Owner responsible for all overages not noted on plan.
 - E-3 All installations shall be per national cde 2008.
 - E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
 - E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Code latest edition. Owner is responsible for all overages not noted on plan.
 - E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
 - E-7 Entry of service (underground or overhead) to be determined by contractor agreement.
 - E-8 All outlets located in residential to be tamper-resistant per NEC.
 - E-9 All outlets to be located above baseboard elevation.
 - E-10 All exterior GFI outlets shall be weatherproof.
 - E-11 Overcurrent Protection device shall be installed on the exterior of structures at the load side of the meter to serve as a disconnecting means. Conductors fed from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as equipment ground.
 - E-12 All 120-VOLT, single phase, 15 and 20 ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sun rooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed arc-fault circuit interrupter, combination-type installed to provide protection of the branch circuit.
 - E-13 Carbon Monoxide alarms shall be required within 10' of all rooms for sleeping purposes in buildings having a fossil-fuel burning heater or appliance, a fireplace or attached garage.



Electrical pLAN

ELECTRICAL	SYMBOL
ceiling fan	
ceiling fan globe 1	
ceiling globe light	
chandelier	
double spotlight	
fluorescent fixture	
vanity bar light	
electrical panel	
AC Disconnect	
Outlet WP GFI	
cable tv outlet	
fan	
light	
outlet	
outlet 220v	
outlet gfi	
smoke detector	
switch	
switch 3 way	
telephone	

RESIDENCE

Vinnett

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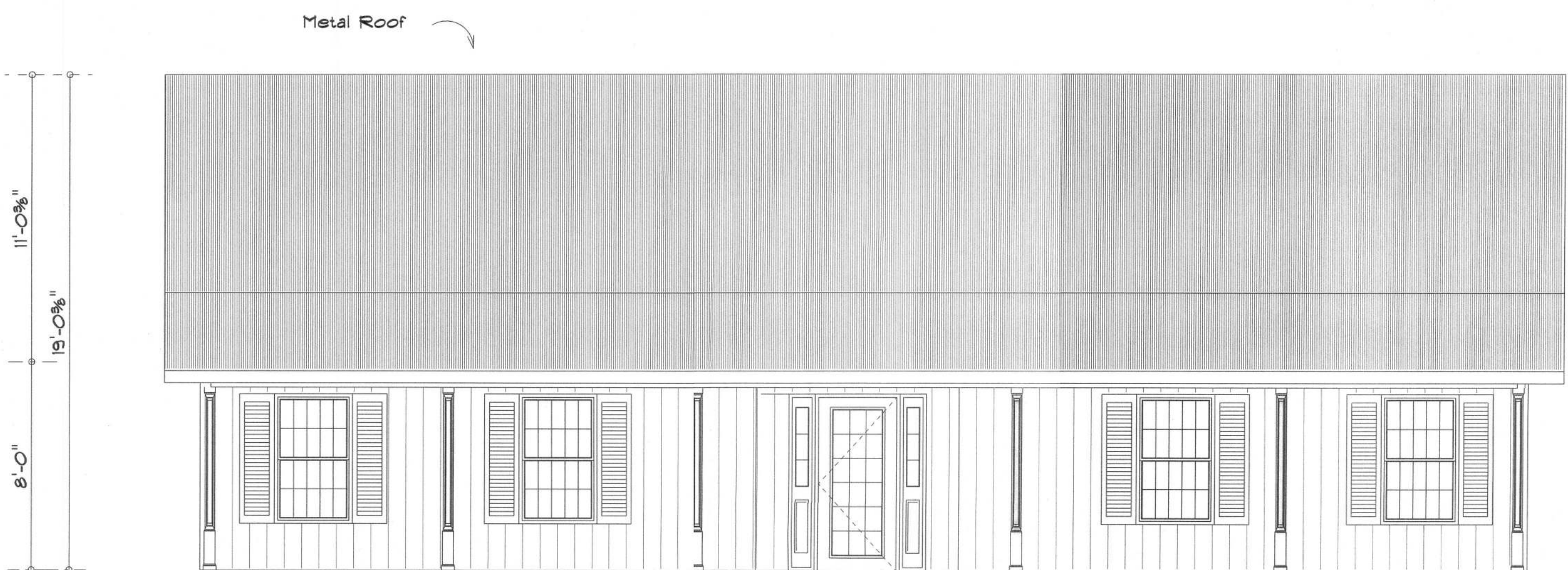
Mark Haddox

DRAWING NUMBER

A-2

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

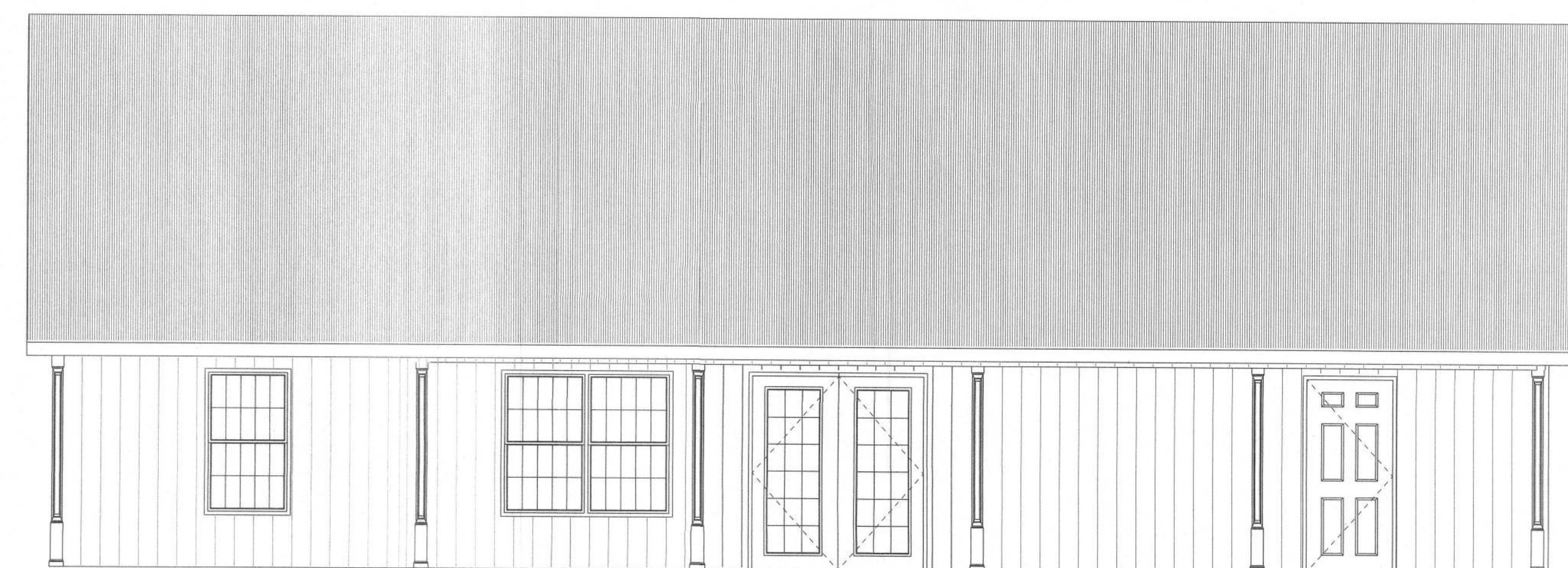


Board and Batton
Vinyl Siding

Front Elevation

Notes:

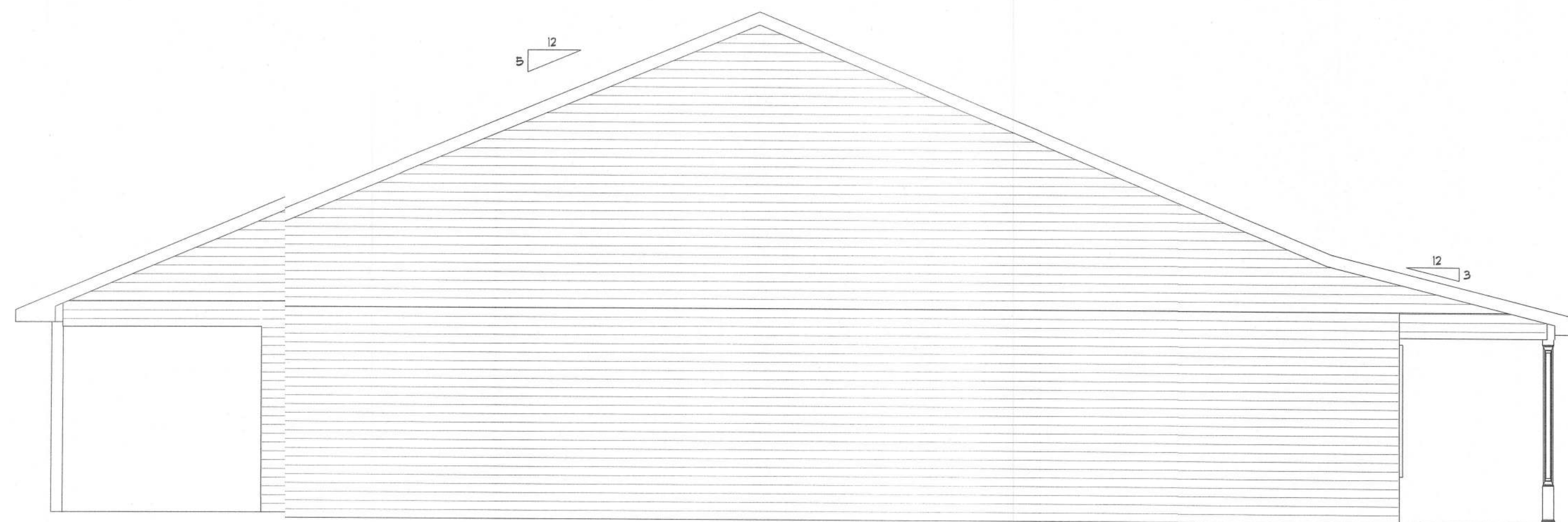
- R-1 All roof pitches shall be 6/12 unless otherwise noted.
- R-2 All overhangs shall be 24" except on gables 18".
- R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).



Rear Elevation



Right Elevation



Left Elevation

RESIDENCE

Vinnett

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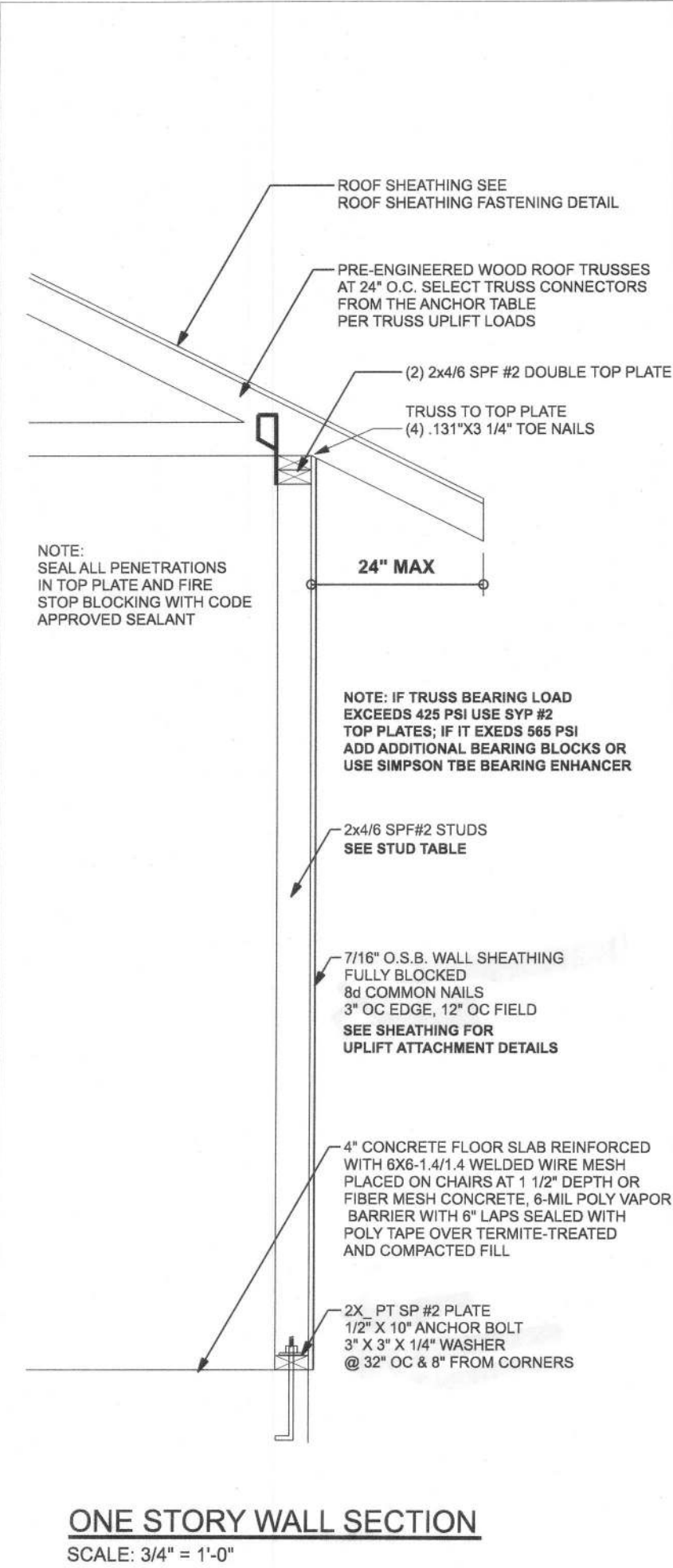
Thursday, March 11, 2021

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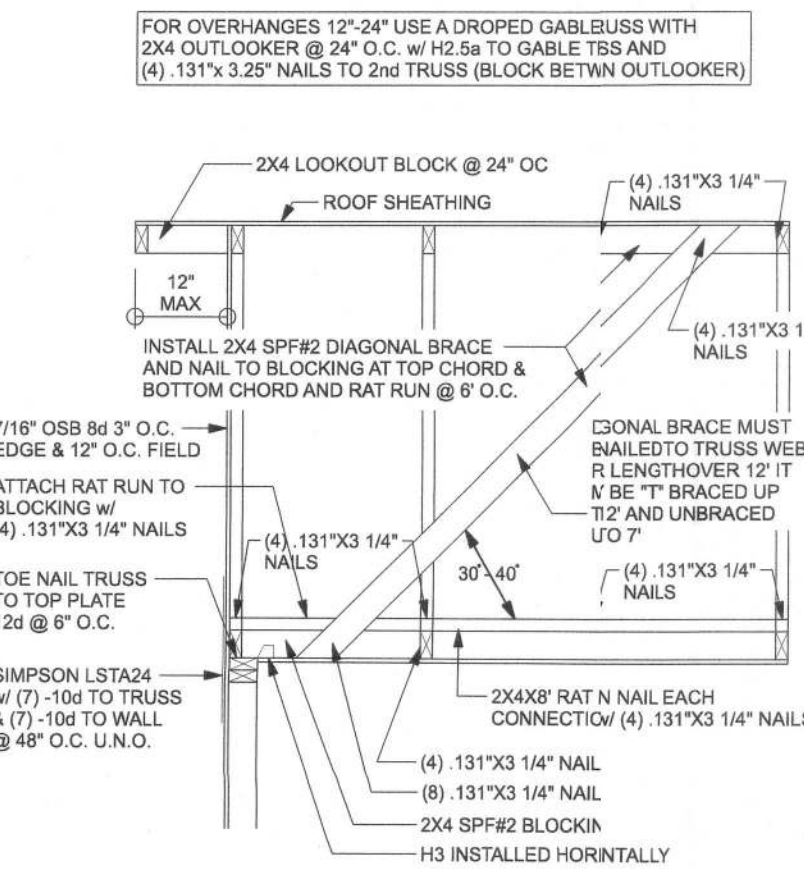
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A-3



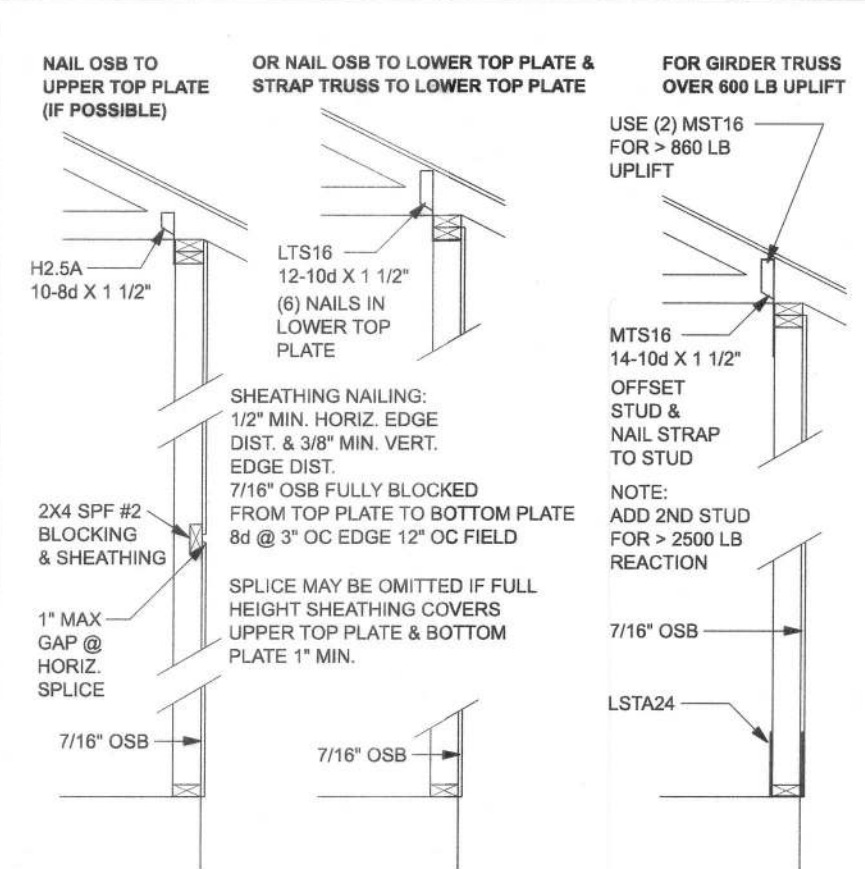
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc

Note: For sheathing located a minimum of 4 feet from perimeter edge of the roof, including 4 feet on each side of ridges and/or, nail spacing is permitted to be 6 inches on center along panel edge and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Flon product approval.

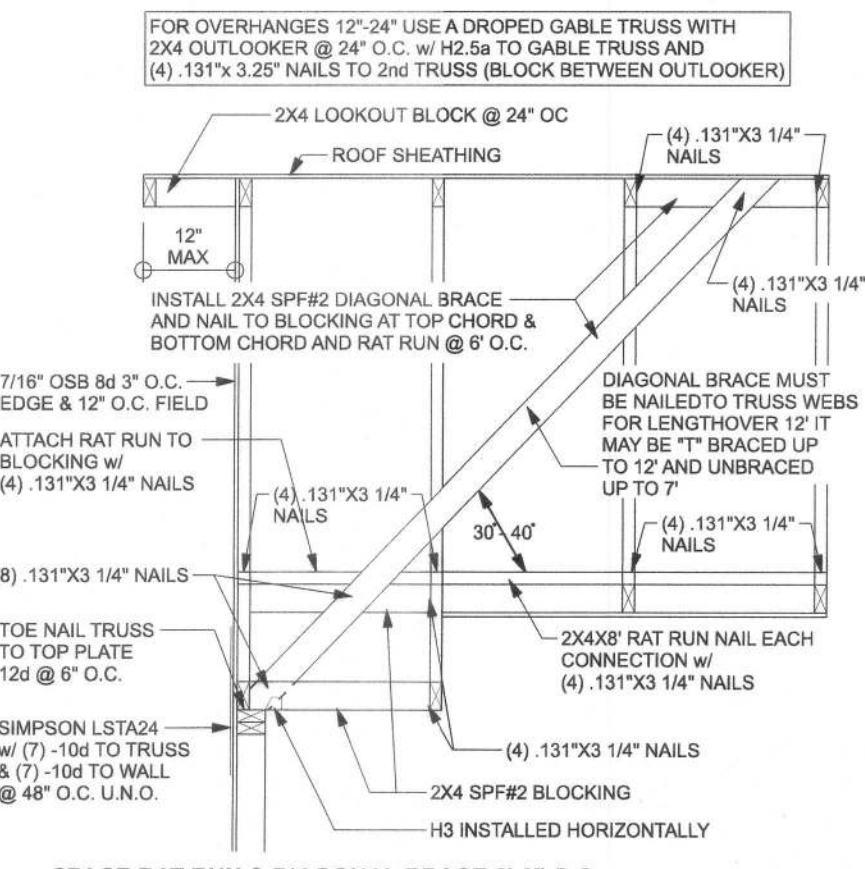


SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL (WOOD FRAME)

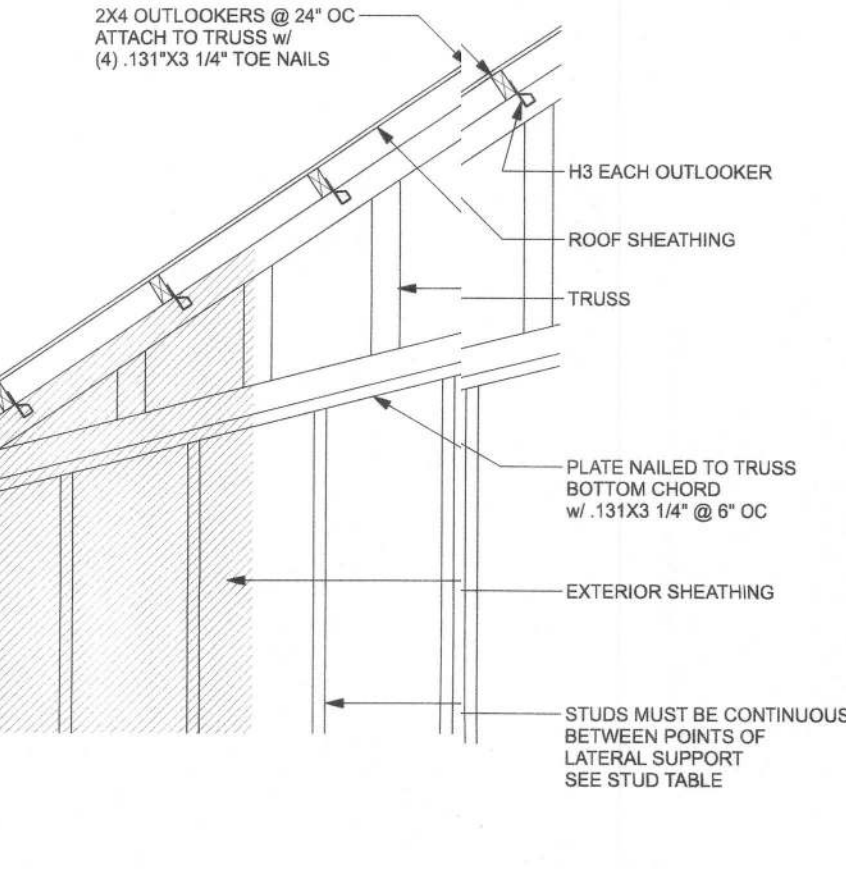


SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL (TRAY CEILING) WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600		
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
975	495	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"
720	630	LTS12-20	6-10d x 1 1/2"	6-10d x 1 1/2"
1000	860	MTS12-30	7-10d x 1 1/2"	7-10d x 1 1/2"
1450	1245	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stairwell	To Stud / Post	Anchor
1925	1800	DTT22	8-SDS 1/4" x 1 1/2"	1/2" x 12" Titan HD
4235	3640	HTT4	18-16d x 1 1/2"	1/2" x 12" Titan HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1925	1800	DTT22	8-SDS 1/4" x 1 1/2"	1/2" x 6" Titan HD
4235	3640	HTT4	18-16d x 1 1/2"	1/2" x 12" Titan HD
Uplift SP	Uplift SPF	Post Bases @ Stairwell	To Post	Anchor
2200	ABU44		12-16d	5/8" x 12" Drill & Epoxy
2300	ABU66		12-16d	5/8" x 12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44		12-16d	5/8" x 7" Drill & Epoxy
2300	ABU66		12-16d	5/8" x 7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

	Fb	E
2x8	SP #2	925 1.4
2x10	SP #2	800 1.4
2x12	SP #2	750 1.4
GLB	24F-V3 SP	2600 1.9
LSL	TIMBERSTRAND	1700 1.7
LVL	MICROLAM	2950 2.0
PSL	PARALAM	2900 2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415L EACH END, 2X6 RAFTERS 100 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1300 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB, 6" x 6" W1.4 x W1.4, FB = 65KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKING, BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPICES 40" x DB (25" FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 16" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

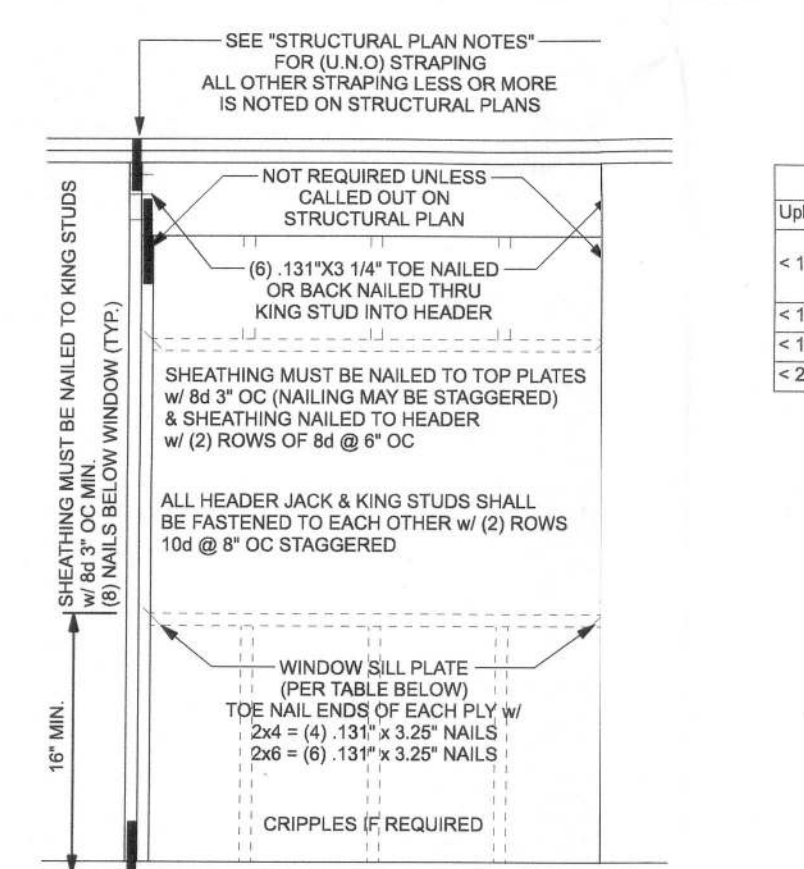
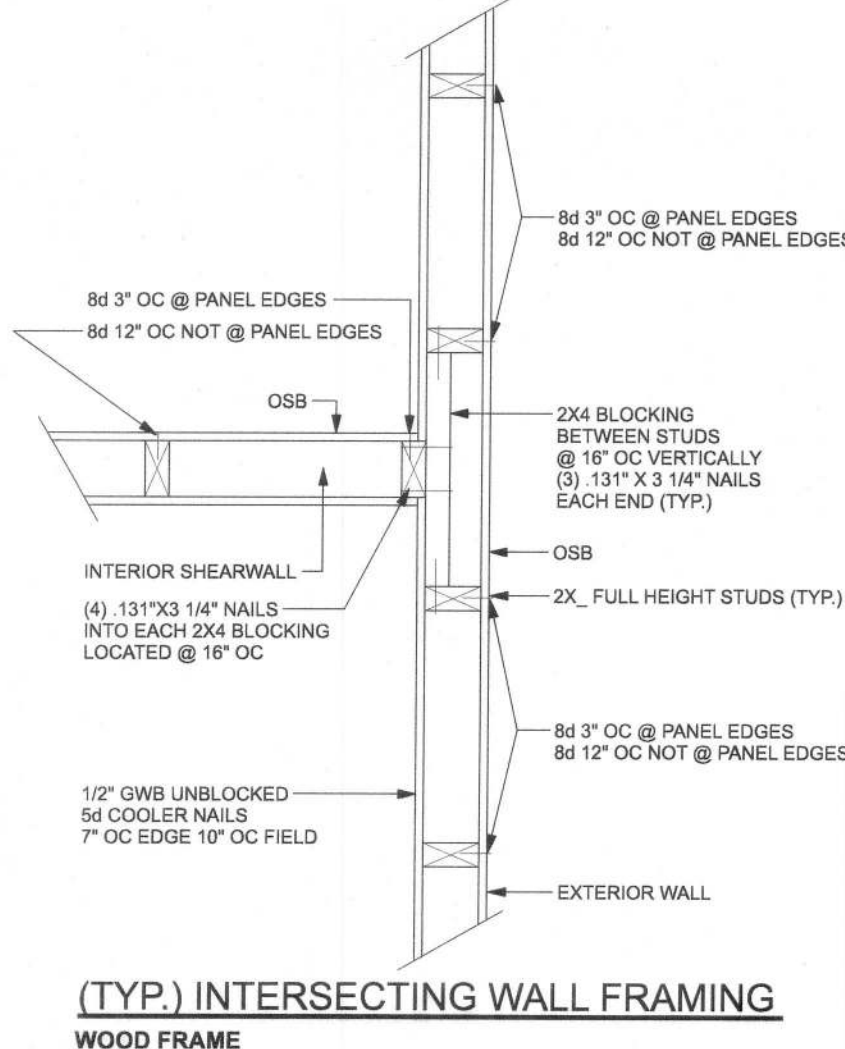
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN O.M.T.S. A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

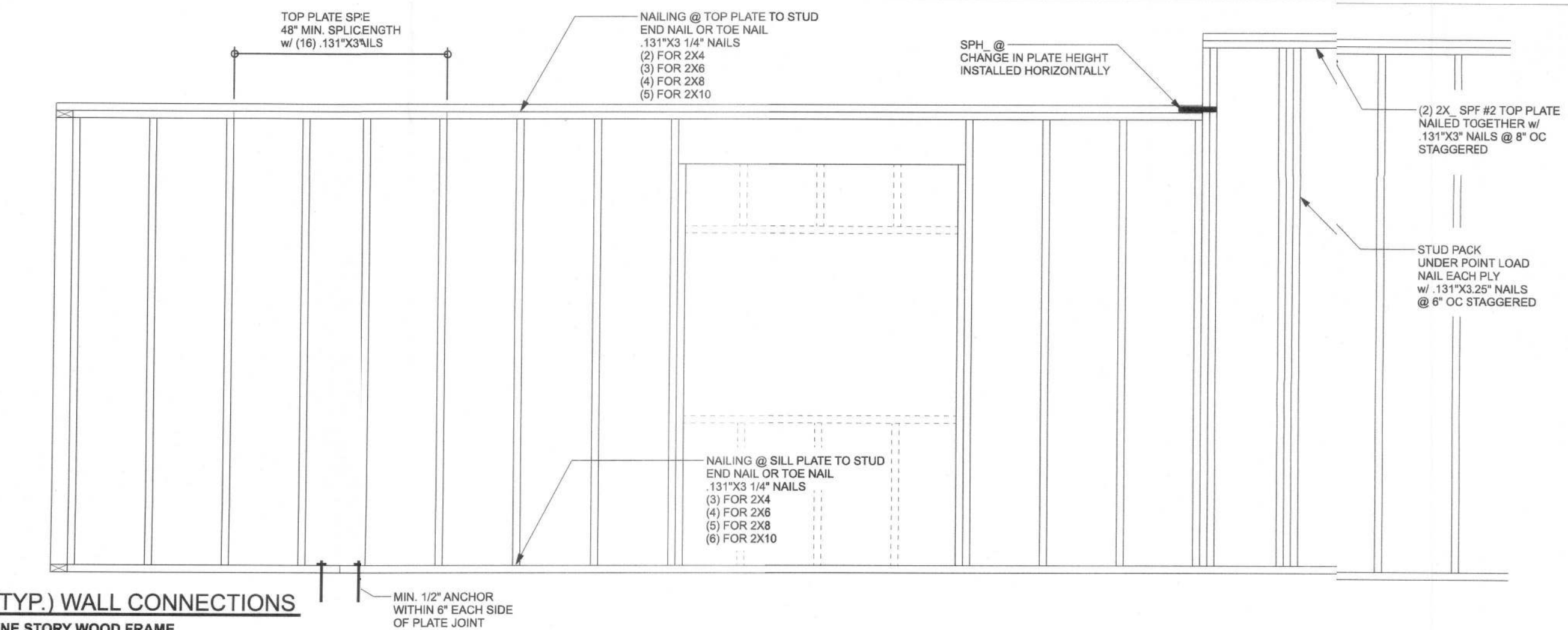
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



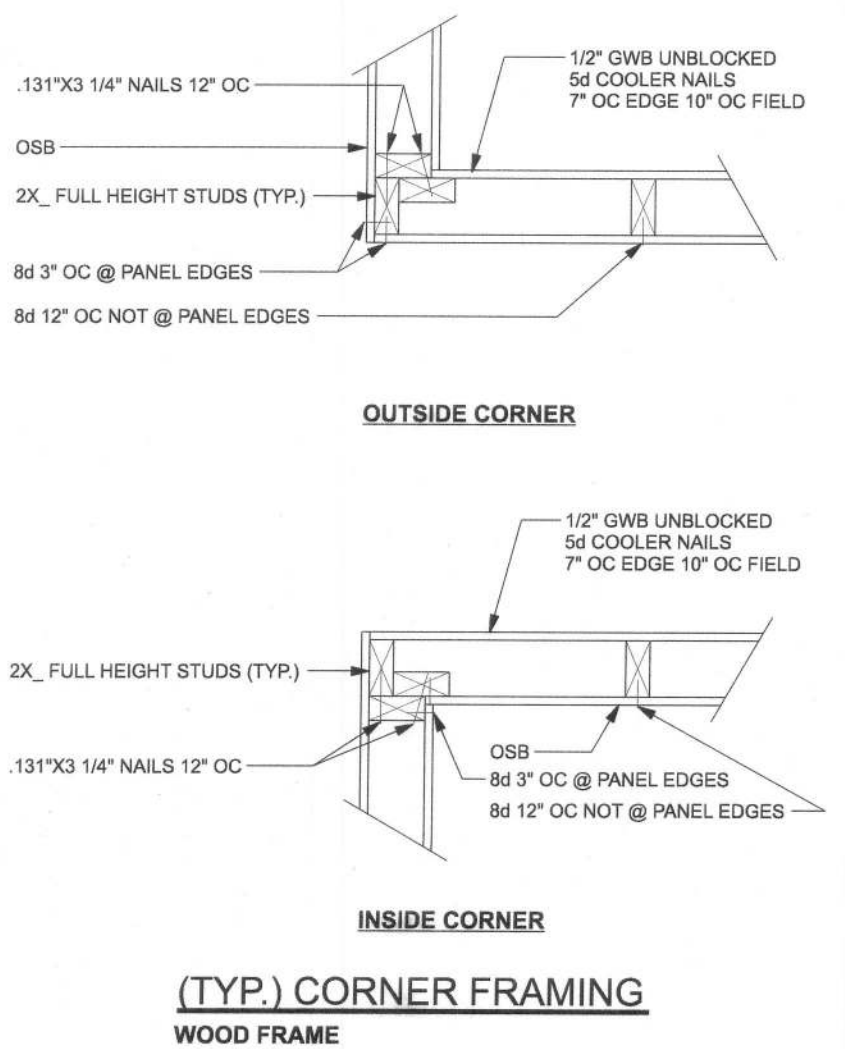
TYPICAL HEADER STRAPPING DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHOR

Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 18-10d header to jacks	DTT22
< 1800	(2) MSTA24, 18-10d header to jacks	DTT22
< 2910	(2) MSTA24, 18-10d header to jacks	HTT4

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.23B
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
90 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



(TYP.) CORNER FRAMING WOOD FRAME

Woodman Park Builders, Inc.

Winnett Res.

PROJECT ADDRESS:
719 SW Happy Jack Tr.
Lake City, FL 32824

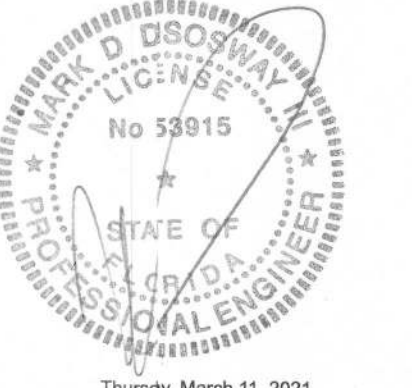
DIMENSIONS: Shaded dimension supercede scaled dimensions. Refs all questions to Mark Disosway, IE, for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



Thursday, March 11, 2021

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disoswayesign@gmail.com

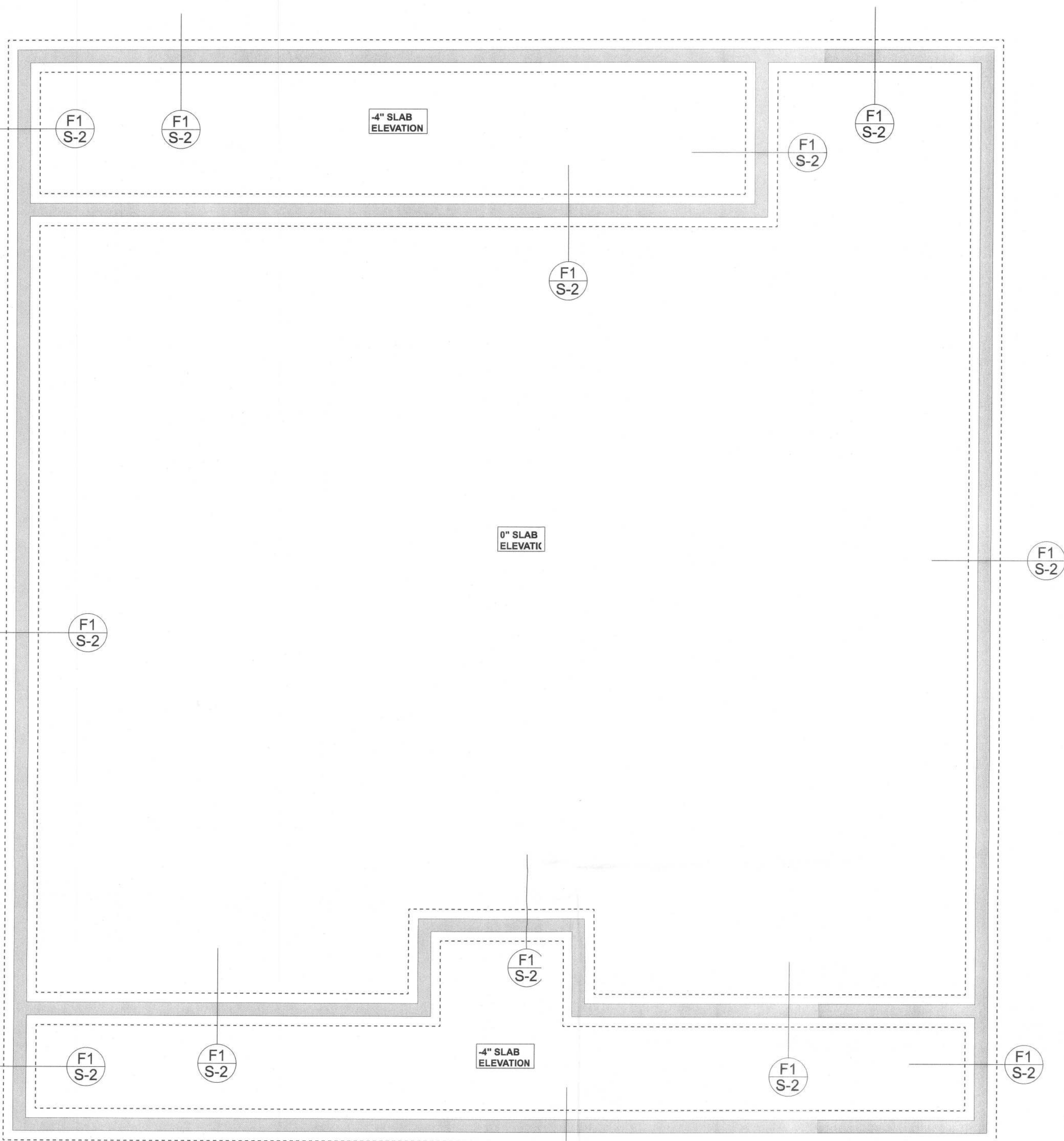
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10380

S-1

012 SHEETS

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WIND LOADS	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	18 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN FLOOD ZONE

EFFECTIVE WIND AREA (Ft ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vsust) -27.8(Vult)	+25.6(Vsust) -34.2(Vult)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)
9x7 GARAGE DOOR	+22.6(Vsust) -25.5(Vsust)	
16x7 GARAGE DOOR	+21.7(Vsust) -24.1(Vsust)	



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- FN-1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN-2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING WALLS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN-3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ 6X6-1/4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 8-MIL POLY VAPOR BARRIER W/ 1/4" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).

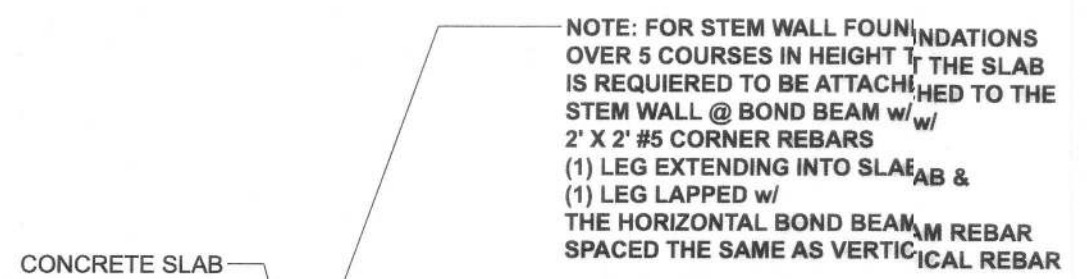
TALL STEM WALL TABLE:

The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

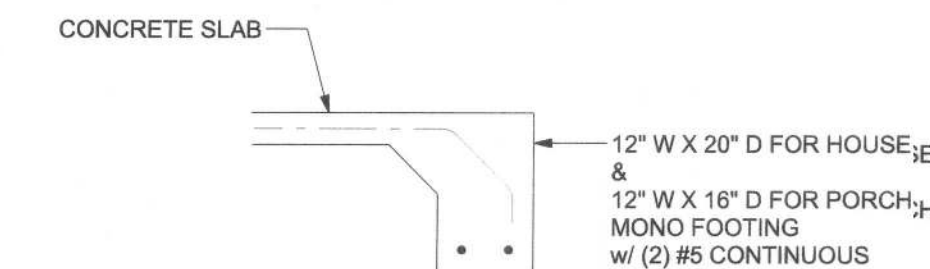
MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.2 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	Specific Requirements
1.4A Compressive strength	8" block bearing walls Fm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick stands	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars - #11	ASTM A 615, Grade 60, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #8)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 680, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



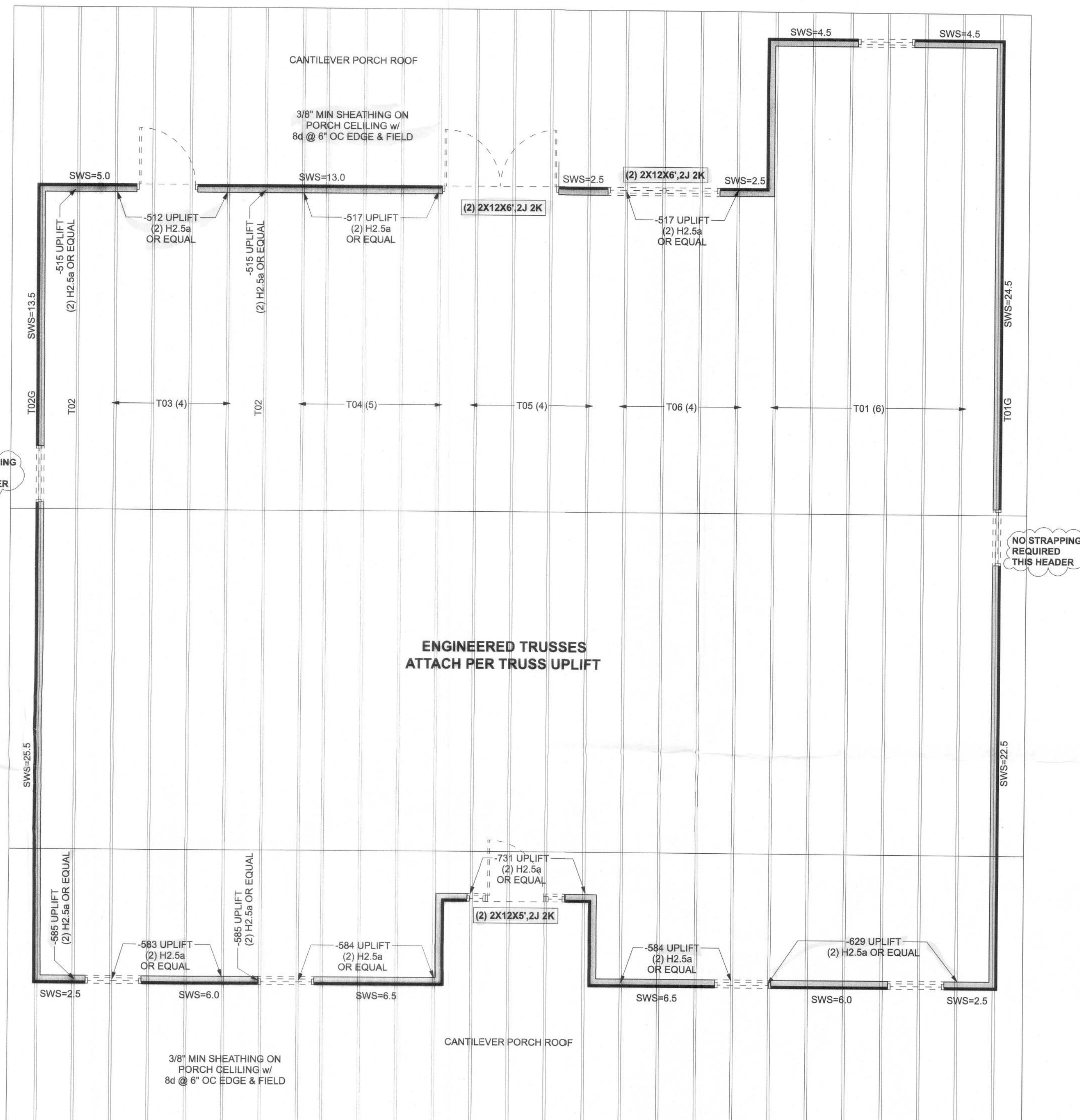
- NOTE: FOR STEM WALL FOUNDATIONS OVER 5 COURSES IN HEIGHT THE SLAB IS REQUIRED TO BE ATTACHED TO THE STEM WALL @ BOND BEAM W/ 2 X 2 #5 CORNER REBARS
- (1) LEG EXTENDING INTO SLAB & (1) LEG LAPPED W/ THE HORIZONTAL BOND BEAM REBAR SPACED THE SAME AS VERTICAL REBAR
- (1) #5 CONTINUOUS IN HEADER-BLOCK BOND BEAM @ SLAB EDGE INTERSECTION W/ STEM WALL
- #5 VERT. REBAR W/ STD. HOOK IN FOOTING @ EACH CORNER & 72" OC
- 8X8X16, RUNNING BOND, CMU STEM WALL, MAX 5 COURSES (SEE SPECIAL REINFORCEMENT TABLE FOR MORE THAN 5 COURSES)
- 20" W X 10" D POURED CONCRETE STRIP FOOTING W/ (2) #5 REBAR CONTINUOUS

STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



OPTIONAL MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS W/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 12" X 10" ANCHOR BOLT W/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND

- (2) 2X10X0', 1J 1K → HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	34056 LBF	24552 LBF
REQUIRED	15457 LBF	15371 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2652828

Woodman Park Builders, Inc.

Winnett Res.

PROJECT ADDRESS:
719 SW Happy Jack Tr.
Lake City, FL 32024

DIMENSIONS:
Stated dimension supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915



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JOB NUMBER:
210380

S-2

OF 2 SHEETS