Julius Lee Engineering

RE: 317548 - HOUSECRAFT - FARRIMOND RES.

1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Site Information:

6

Project Customer: HOUSECRAFT HOMES Project Name: 317548 Model: CUSTOM

Lot/Block: Subdivision:

Address: 5850 NW LAKE JEFFREY RD

City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: JOHN D. HARRINGTON License #: ĆGC038861

Address: 24113 NW OLD BELLAMY RD

City: HIGH SPRINGS, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1

Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf

Roof Load: 32.0 psf

This package includes 32 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	14143016	CJ1	10/29/09	18	14143033	T11	10/29/09
2	14143017	CJ3	10/29/09	19	14143034	T12	10/29/09
3	14143018	CJ5	10/29/09	20	14143035	T13	10/29/09
4	14143019	CJ5A	10/29/09	21	14143036	T14	10/29/09
5	14143020	EJ7	10/29/09	22	14143037	T15	10/29/09
6	14143021	HJ9	10/29/09	23	14143038	T16	10/29/09
7	14143022	T01	10/29/09	24	14143039	T17	10/29/09
8	14143023	T01G	10/29/09	25	14143040	T18	10/29/09
9	14143024	T02	10/29/09	26	14143041	T19	10/29/09
10	14143025	T03	10/29/09	27	14143042	T20	10/29/09
11	14143026	T04	10/29/09	28	14143043	T21G	10/29/09
12	14143027	T05	10/29/09	29	14143044	T22	10/29/09
13	14143028	T06	10/29/09	30	14143045	T22G	10/29/09
14	14143029	T07	10/29/09	31	14143046	T23	10/29/09
15	14143030	T08	10/29/09	32	14143047	T24	10/29/09
16	14143031	T09	10/29/09				
121200	\$ 12 Fin 19 Fin	1					



The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

10/29/09

Truss Design Engineer's Name: Julius Lee

T10

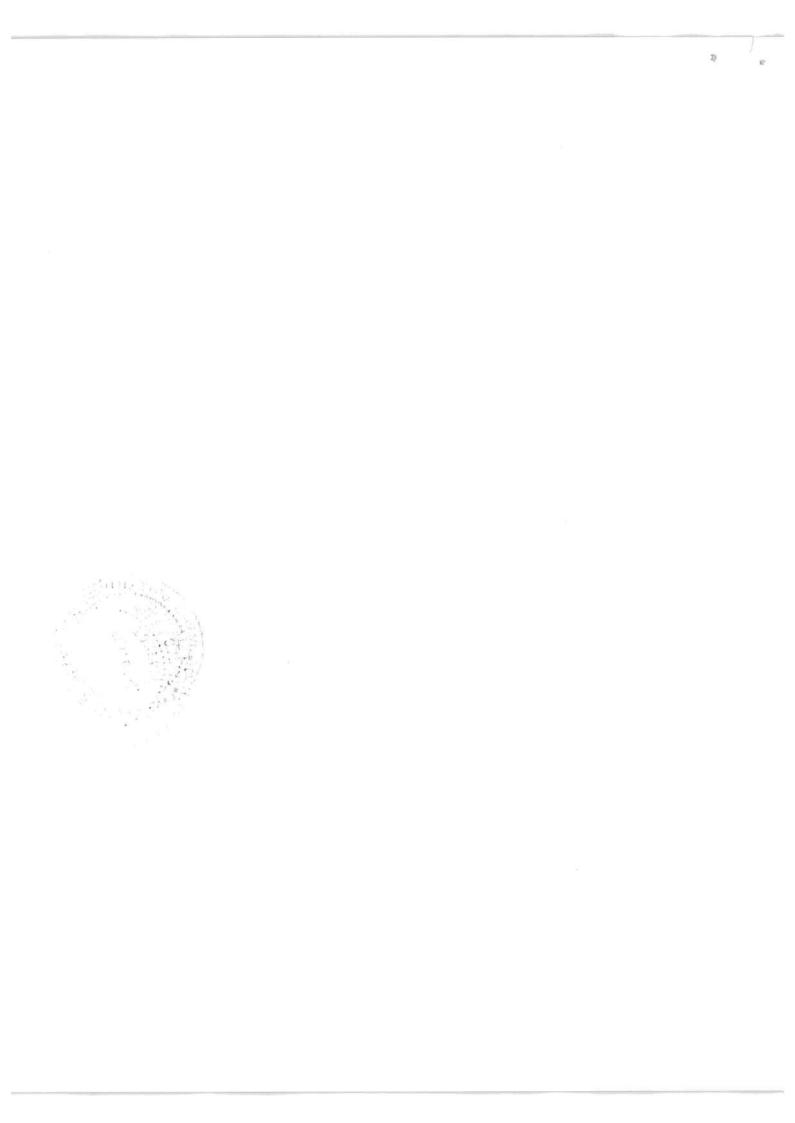
14143032

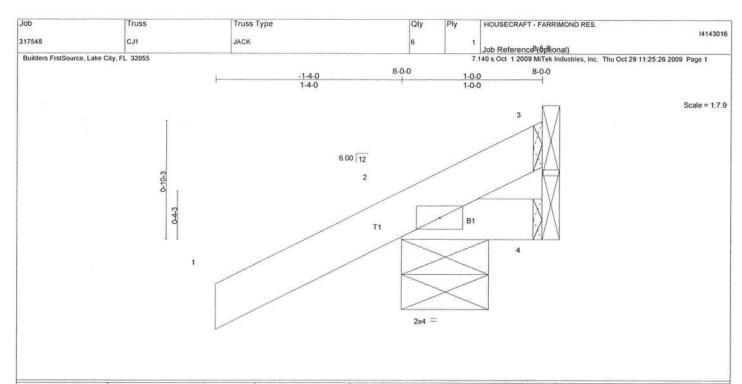
My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



1 of 1





LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	2	>999	360	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240	3810,077.0		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a			
BCDL	5.0	Code FBC2007/TF	PI2002	(Mate	rix)	Wind(LL)	0.00	2	****	240	Weight: 6 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 2=162/0-7-8, 4=5/Mechanical, 3=-32/Mechanical Max Horz 2=79(LC 6)

Max Uplift2=-198(LC 6), 3=-32(LC 1) Max Grav 2=162(LC 1), 4=14(LC 2), 3=60(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint 2 and 32 lb uplift at joint
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

October 29,2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer. For designer - not fuss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult — ANSI/TRIU Quality Criteria, DSB-89 and BCS11 Building Component Safety Intermation available from Truss Plate Institute. S83 D'Onofrio Drive, Madison, WI 53719.

HOUSECRAFT - FARRIMOND RES. Job Truss Truss Type Qtv 14143017 317548 CJ3 JACK Job Reference (optional)
7.140 s Oct 1 2009 MiTek Profusiries, Inc. Thu Oct 29 11:25:27 2009 Page 1 Builders FrstSource, Lake City, FL 32055 8-0-0 -1-4-0 3-0-0 Scale = 1:13.0 6.00 12 T1 0-4-3 **B**1

LOADIN	IG (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.20	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.00	2-4	>999	240		
BCLL	0.0	.	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	PI2002	(Mate	ix)	Wind(LL)	0.01	2-4	>999	240	Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=43/Mechanical, 2=201/0-7-8, 4=13/Mechanical Max Horz 2=133(LC 6)

Max Uplift3=-45(LC 6), 2=-227(LC 6), 4=-32(LC 4) Max Grav 3=43(LC 1), 2=201(LC 1), 4=39(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 3, 227 lb uplift at joint 2 and 32 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heets" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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Job Truss Truss Type Qty HOUSECRAFT - FARRIMOND RES. 14143018 CJ5 317548 Job Reference (optional) Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:25:27 2009 Page 1 8-0-0 5-0-0 1-4-0 Scale = 1:18.0 6.00 12 0-4-3

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.02	2-4	>999	360	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.04	2-4	>999	240	10.000		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a			
BCDL	5.0	Code FBC2007/TPI2	2002	(Matr	rix)	Wind(LL)	0.09	2-4	>608	240	Weight: 18 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (Ib/size) 3=109/Mechanical, 2=253/0-7-8, 4=23/Mechanical Max Horz 2=189(LC 6)

Max Uplift3=-124(LC 6), 2=-267(LC 6), 4=-56(LC 4) Max Grav 3=109(LC 1), 2=253(LC 1), 4=69(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3, 267 lb uplift at joint 2 and 56 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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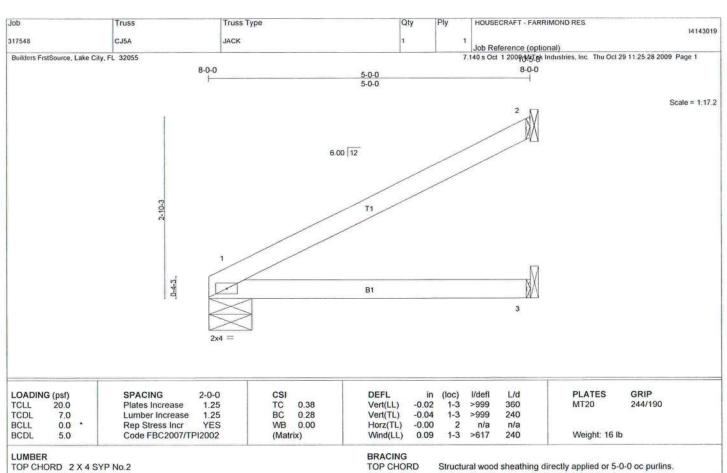
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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

REACTIONS (lb/size) 1=147/0-8-0, 2=124/Mechanical, 3=23/Mechanical Max Horz 1=140(LC 6)

Max Uplift 1=-127(LC 6), 2=-147(LC 6), 3=-56(LC 4) Max Grav 1=147(LC 1), 2=124(LC 1), 3=69(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2 X 4 SYP No.2

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 1, 147 lb uplift at joint 2 and 56 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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Octo October 29,2009

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control, storage, delivery, erection and bracing, consult — ANSI/TPI. ANSI/TPI. SPI. 3 PRIOR ST. 4 PRIOR ST.

Job Truss russ Type Qty HOUSECRAFT - FARRIMOND RES. MONO TRUSS 30 317548 EJ7 Job Reference (optional) Builders FrstSource, Lake City, FL 32055 7.140 s Jun 24 2009 MiTek Industries, Inc. Thu Oct 29 13:04:15 2009 Page 1 7-0-0 1-4-0 7-0-0 Scale = 1:22.5 6.00 12 0-4-3

Plate Off	sets (X,	Y): [2	2:0-2-10,0-1-8]										
LOADING	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	Vdefl	L/d	PLATES	GRIP
TCLL	20.0	- 1	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.08	2-4	>992	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.16	2-4	>500	240		
BCLL	0.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	PI2002	(Matr	ix)	Wind(LL)	0.39	2-4	>205	240	Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

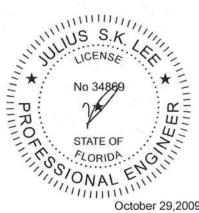
(lb/size) 3=157/Mechanical, 2=312/0-7-8, 4=43/Mechanical REACTIONS Max Horz 2=175(LC 6)

Max Uplift 3=-123(LC 6), 2=-245(LC 6), 4=-82(LC 6) Max Grav 3=157(LC 1), 2=312(LC 1), 4=94(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 123 lb uplift at joint 3, 245 lb uplift at joint 2 and 82 lb uplift at joint 4.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any
 particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL

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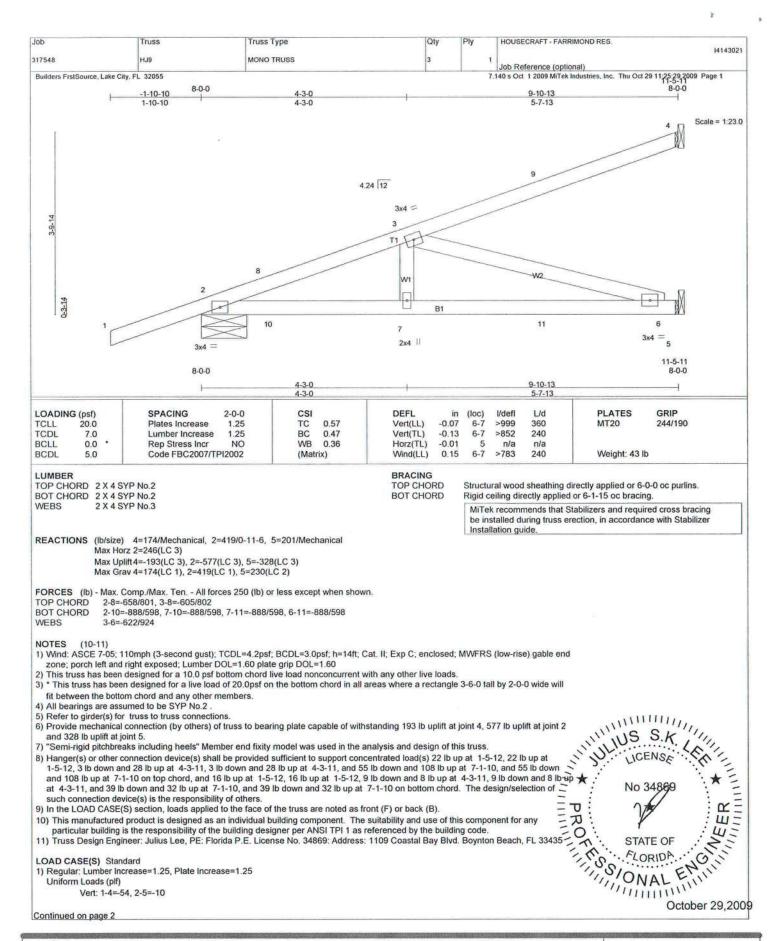


October 29,2009

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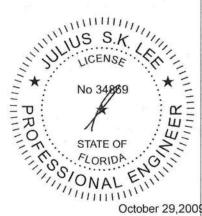
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Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	
317548	HJ9	MONO TRUSS	3	1		14143021
					Job Reference (optional)	
Builders FrstSource, Lake City,	FL 32055			7.	140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:	25:29 2009 Page 2

LOAD CASE(S) Standard

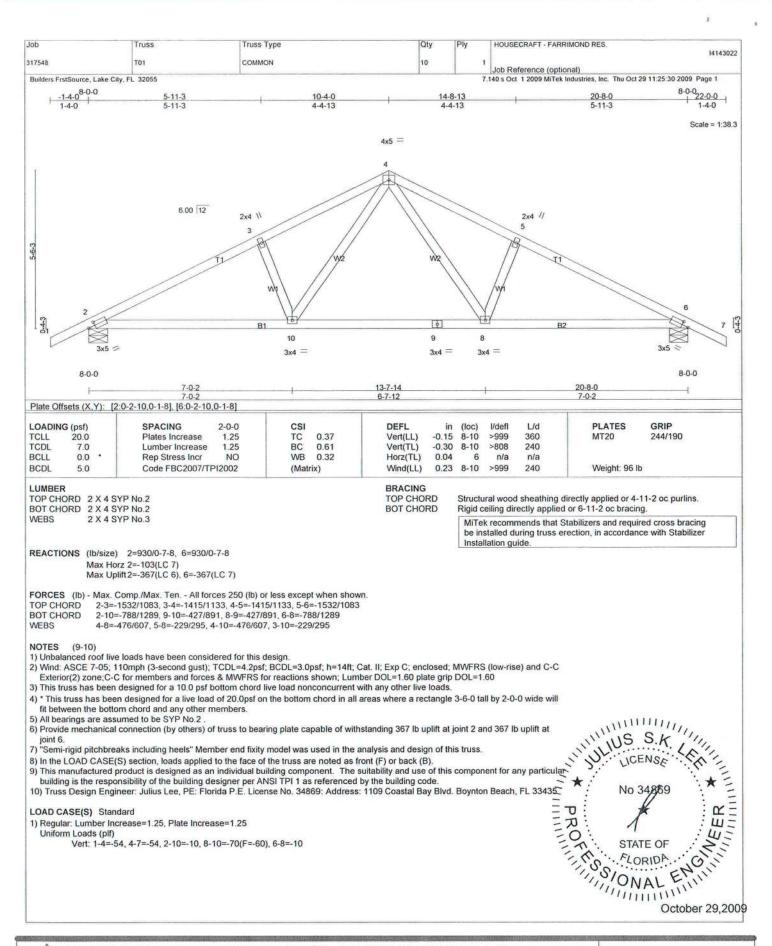
Concentrated Loads (Ib)
Vert: 3=22(F=11, B=11) 7=-6(F=-3, B=-3) 8=45(F=22, B=22) 9=-110(F=-55, B=-55) 10=11(F=5, B=5) 11=-26(F=-13, B=-13)



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Job Truss Truss Type Qty Ply HOUSECRAFT - FARRIMOND RES. GABLE Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:25:31 2009 Page 1 Builders FrstSource, Lake City, FL 32055 -1-4-0⁸⁻⁰⁻⁰ 22-0-0 10-4-0 20-8-0 Scale = 1:38.3 4x5 = 8 6 6.00 12 5 3x4 < 3x4 > 11 0-4-3 13 4 5x7 || 5x7 || 3x4 = 21 20 19 18 17 16 15 14 3x4 = 3x4 = 8-0-0 20-8-0 20-8-0 Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-2-8,Edge], [12:0-3-8,Edge], [12:0-2-8,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL **PLATES** GRIP I/defl 120 90 TCLL 20.0 Plates Increase 1.25 TC 0.19 Vert(LL) 0.00 13 n/r MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.10 0.01 13 Vert(TL) n/r BCLL 0.0 Rep Stress Incr WB 0.08 Horz(TL) NO 12 n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Weight: 106 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. OTHERS 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS All bearings 20-8-0. (lb) - Max Horz 2=-113(LC 7) Max Uplift All uplift 100 lb or less at joint(s) 18 except 2=-195(LC 6), 12=-213(LC 7), 19=-158(LC 6), 20=-108(LC 6), 21=-238(LC 6), 16=-157(LC 7), 15=-107(LC 7), 14=-243(LC 7) Max Grav All reactions 250 lb or less at joint(s) 18, 19, 20, 16, 15 except 2=273(LC 10), 12=273(LC 11), 21=340(LC 10), 14=340(LC 11) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 4-21=-290/290, 10-14=-290/290 NOTES (13-14)1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 THINITUM S.K. 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002. 4) All plates are 2x4 MT20 unless otherwise indicated. Gable requires continuous bottom chord bearing. 6) Gable studs spaced at 2-0-0 oc. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) All bearings are assumed to be SYP No.2 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18 except (jt=lb) 2=195, 12=213, 19=158, 20=108, 21=238, 16=157, 15=107, 14=243. No 34869 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. NOWEF 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 October 29,2009 Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
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Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	14143023
317548	T01G	GABLE	1		Job Reference (optional)	14143023
Builders FrstSource,	Lake City, FL 32055				7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:25:31 20	009 Page 2

LOAD CASE(S) Standard

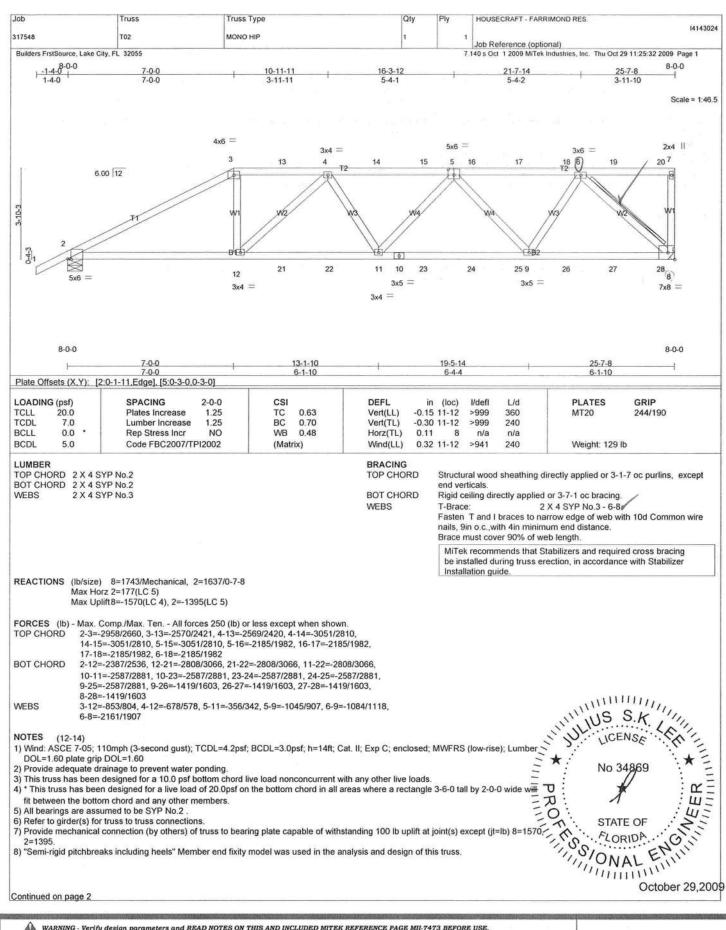
Uniform Loads (plf) Vert: 1-7=-79(F=-25), 7-13=-79(F=-25), 2-12=-10



October 29,2009

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Inus designer. Riracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TP1 Quality Criteria, DSS-89 and BCS11 Building Component Sately Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	14143024
317548	T02	MONO HIP	1	1	Job Reference (optional)	14143024

Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:25:32 2009 Page 2

NOTES (12-14)

- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 223 lb down and 283 lb up at 7-0-0, 103 lb down and 107 lb up at 9-0-12, 103 lb down and 107 lb up at 11-0-12, 103 lb down and 107 lb up at 15-0-12, 103 lb down and 107 lb up at 17-0-12, 103 lb down and 107 lb up at 19-0-12, 103 lb down and 107 lb up at 19-0-12, 103 lb down and 107 lb up at 25-0-12 on top chord, and 264 lb down and 423 lb up at 7-0-0, 64 lb down and 88 lb up at 9-0-12, 64 lb down and 88 lb up at 11-0-12, 64 lb down and 88 lb up at 13-0-12, 64 lb down and 88 lb up at 15-0-12, 64 lb down and 88 lb up at 17-0-12, 64 lb down and 88 lb up at 19-0-12, 64 lb down and 88 lb up at 23-0-12, and 64 lb down and 88 lb up at 23-0-12, and 64 lb down and 88 lb up at 25-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-3=-54, 3-7=-54, 2-8=-10

Concentrated Loads (lb)

Vert: 3=-223(B) 12=-224(B) 4=-103(B) 11=-33(B) 13=-103(B) 14=-103(B) 15=-103(B) 16=-103(B) 17=-103(B) 18=-103(B) 19=-103(B) 20=-103(B) 21=-33(B) 23=-33(B) 24=-33(B) 25=-33(B) 2

No 34869

No 34869

STATE OF

FLORIDA

October 29,200

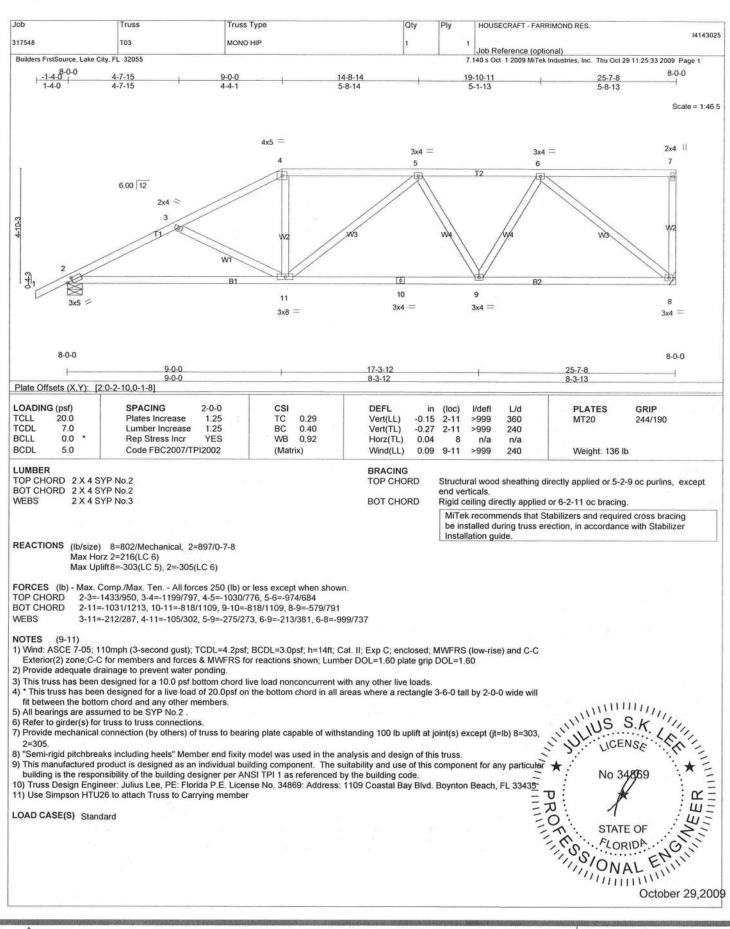
October 29,2009

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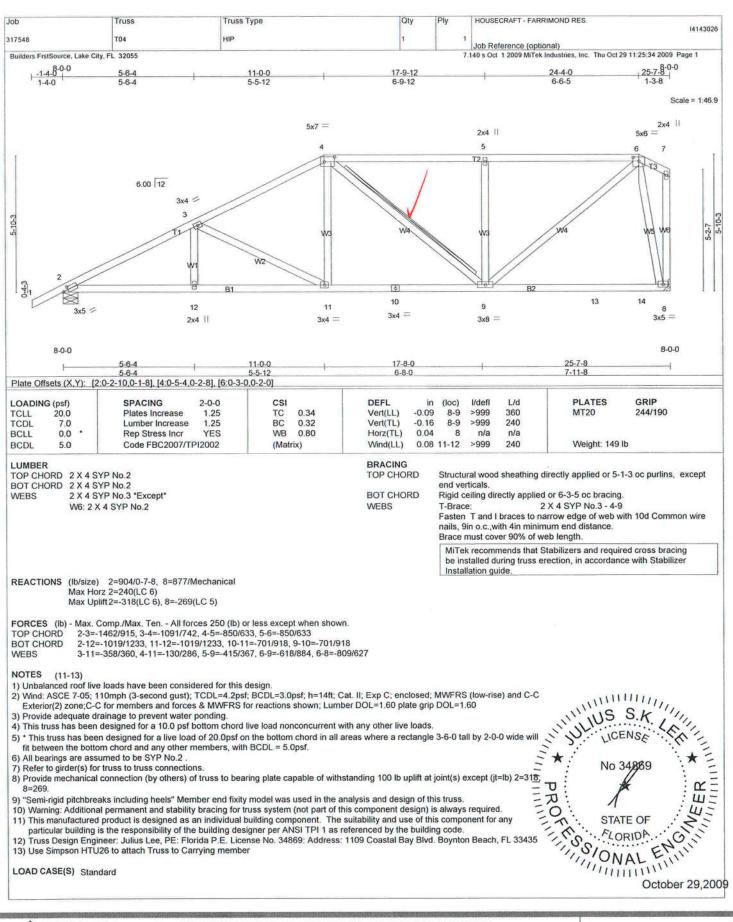
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing show is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, qualify control, storage, delivery, erection and bracing, consult.

AMSI/TRUSH CRIBING STABLES AND STABLES



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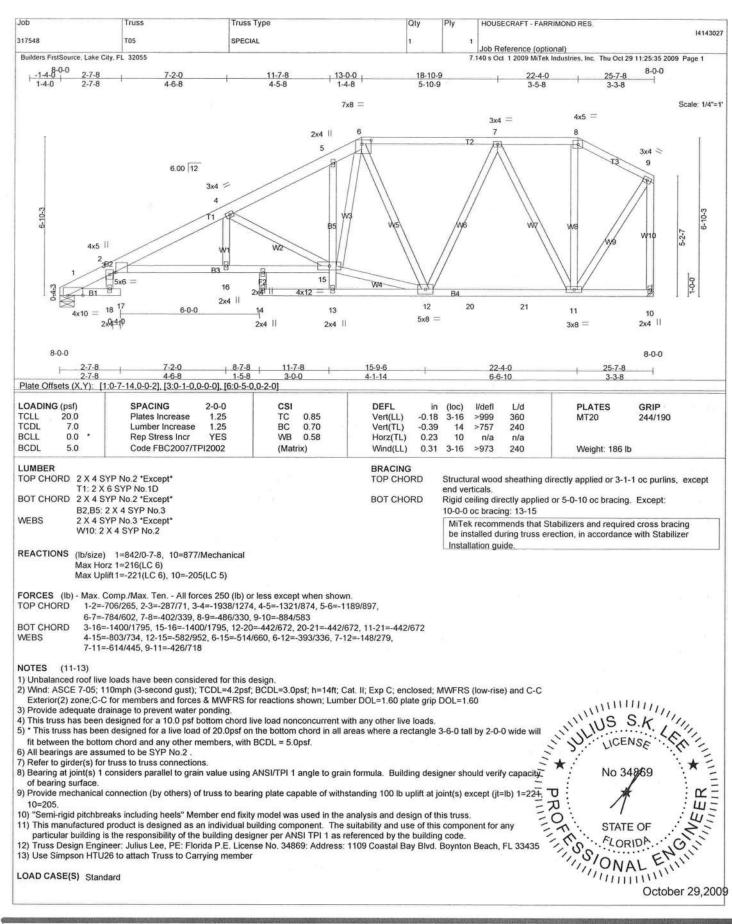
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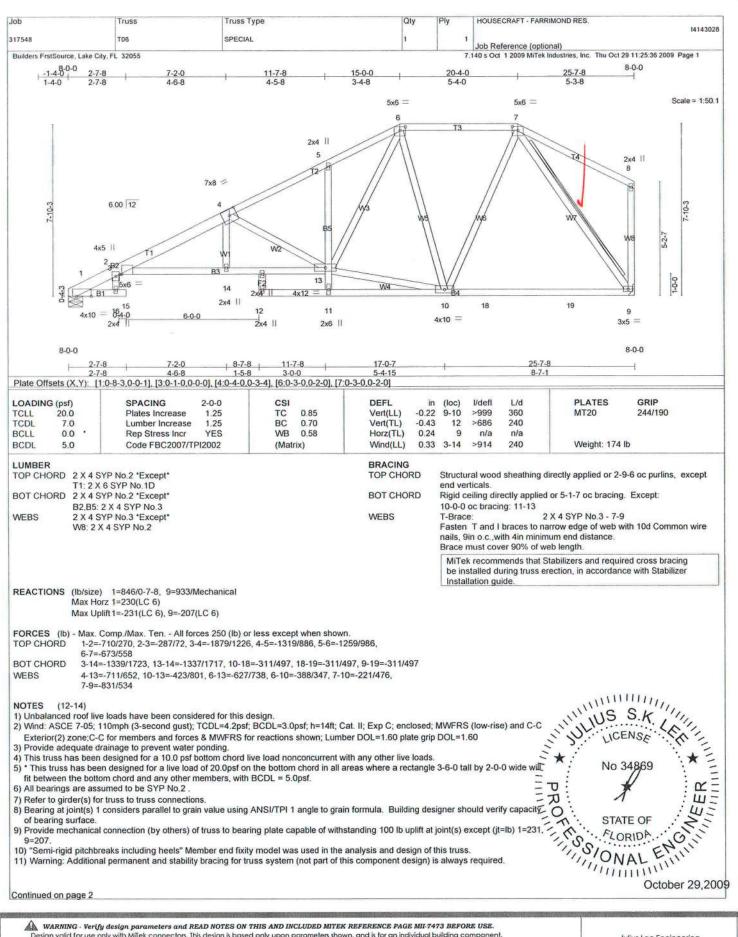
Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer- not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, exection and bracing, consult

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Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	
217548		SPECIAL				14143028
317340	T06	SPECIAL			Job Reference (optional)	
Builders FrstSource L	ake City FL 32055			7	140 c Oct 1 2000 MiTek Industries Inc. The Oct 20 11	26-26 2000 Dags 2

12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

No 34869

No 34869

STATE OF

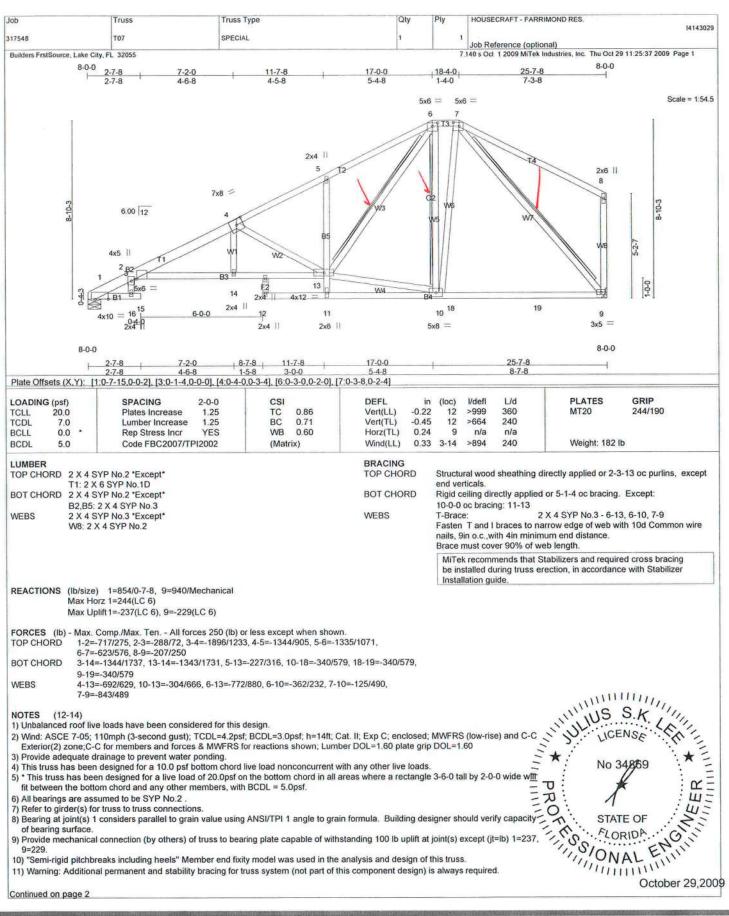
FLORIDA

October 29,

October 29,2009

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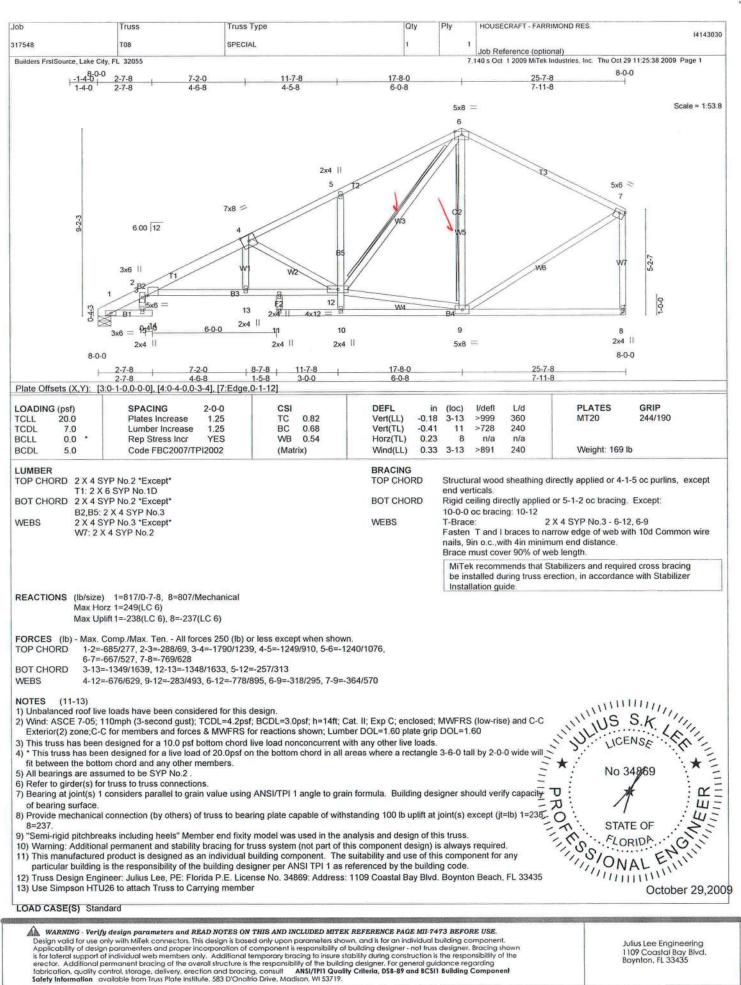
Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	/oversemous
317548	Т07	SPECIAL	1	1		14143029
Builders FrstSource, Lake City, Ft	L 32055			7.	Job Reference (optional) 140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:25:37	2009 Page 2
building designer per A 13) Truss Design Engineer	NSI TPI 1 as referenced by	the building code. E. License No. 34869: Address: 1109 Coas			ent for any particular building is the responsibility Beach, FL 33435	of the
LOAD CASE(S) Standard	ı					
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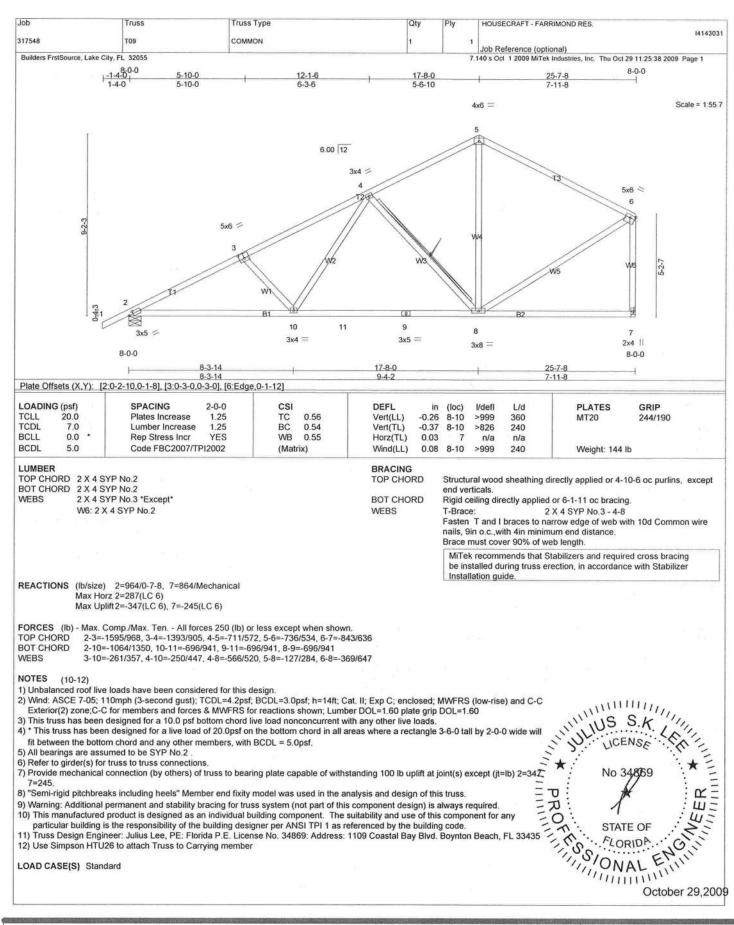
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available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



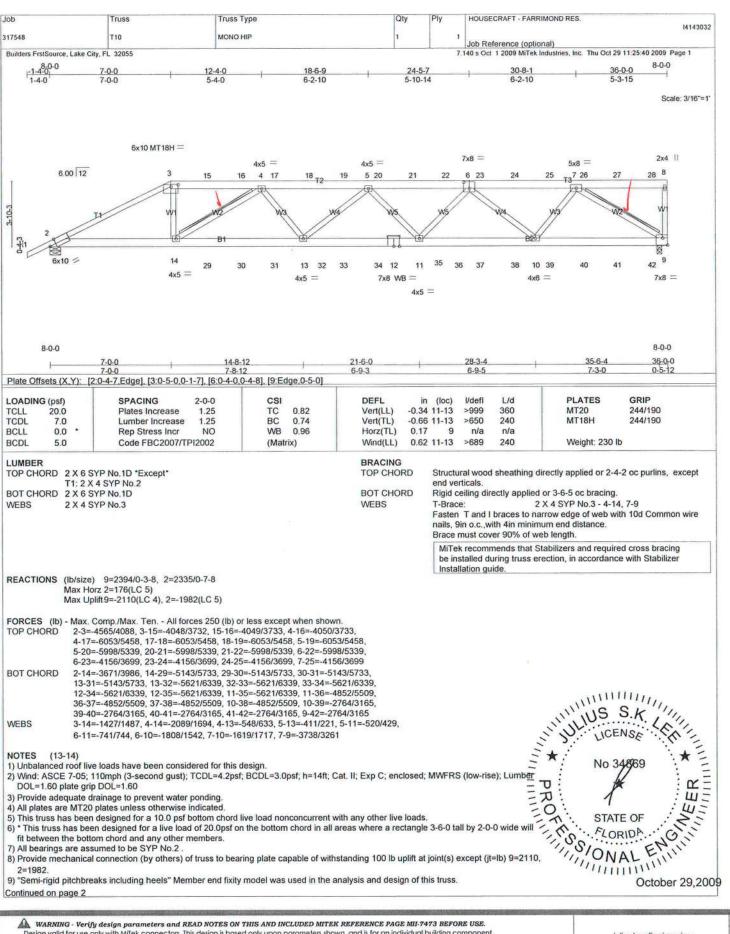
1109 Coastal Bay Blvd. Boynton, FL 33435



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Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	
317548	T10	MONO HIP		1		14143032
	10.000		120	100	Job Reference (optional)	
Builders ErstSource Lake	City El 22066			7	110 - O-1 1 2000 HT-1 I-1 - I T- O-100 H	25 to 2000 B

- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 303 lb down and 283 lb up at 7-0-0, 143 lb down and 107 lb up at 9-0-12, 143 lb down and 107 lb up at 11-0-12, 143 lb down and 107 lb up at 15-0-12, 143 lb down and 107 lb up at 17-0-12, 143 lb down and 107 lb up at 19-0-12, 143 lb down and 107 lb up at 21-0-12, 143 lb down and 107 lb up at 25-0-12, 143 lb 27-0-12, 143 lb down and 107 lb up at 29-0-12, 143 lb down and 107 lb up at 31-0-12, and 143 lb down and 107 lb up at 33-0-12, and 143 lb down and 107 lb up at 35-0-12 on top chord, and 264 lb down and 423 lb up at 7-0-0, 64 lb down and 88 lb up at 9-0-12, 64 lb down and 88 lb up at 11-0-12, 64 lb down and 88 lb up at 13-0-12, 64 lb down and 88 lb up at 15-0-12, 64 lb down and 88 lb up at 17-0-12, 64 lb down and 88 lb up at 19-0-12, 64 lb down and 88 lb up at 21-0-12, 64 lb down and 88 lb up at 23-0-12, 64 lb down and 88 lb up at 25-0-12, 64 lb down and 88 lb up at 27-0-12, 64 lb down and 88 lb up at 29-0-12, 64 lb down and 88 lb up at 31-0-12, and 64 lb down and 88 lb up at 33-0-12, and 64 lb down and 88 lb up at 35-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
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- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-8=-54, 2-9=-10

Concentrated Loads (lb)

Vert: 3=-223(F) 14=-224(F) 15=-103(F) 16=-103(F) 17=-103(F) 18=-103(F) 19=-103(F) 20=-103(F) 21=-103(F) 22=-103(F) 23=-103(F) 24=-103(F) 25=-103(F) 26=-103(F) 26=-103(F) 28=-103(F) 28=-103(F) 30=-33(F) 31=-33(F) 32=-33(F) 33=-33(F) 34=-33(F) 35=-33(F) 36=-33(F) 37=-33(F) 38=-33(F) 39=-33(F) 40=-33(F)

No 34 NONEF October 29,2009

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

HOUSECRAFT - FARRIMOND RES Joh Truss Truss Type Oty Ply 14143033 T11 317548 MONO HIP Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:25:40 2009 Page 1 Builders FrstSource, Lake City, FL 32055 1-4-0 1-4-0 8-0-0 9-0-0 28-8-11 36-0-0 15-11-9 22-4-2 6-4-10 Scale: 3/16"=1" 5x6 = 5x6 = 3x4 || 3x4 = 3x4 = 8 T2 T3 6.00 12 2x4 > 4-10-3 12 13 4x5 3x4 = 3x6 = 6x8 = 3x4 = 3x8 = 8-0-0 8-0-0 36-0-0 0-5-12 9-0-0 18-4-15 9-0-0 9-4-15 7-10-5 9-3-0 Plate Offsets (X,Y): [2:0-1-4,0-0-2], [4:0-3-0,0-2-0], [6:0-3-0,0-3-0] PLATES GRIP LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d 244/190 TC 0.41 Vert(LL) -0.19 9-10 >999 360 MT20 TCLL 20.0 Plates Increase 1.25 TCDL 7.0 Lumber Increase 1.25 BC 0.58 Vert(TL) -0.37 11-13 >999 240 0.0 Rep Stress Incr YES WB 0.68 Horz(TL) 0.11 9 n/a n/a BCLL BCDL Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.24 11 >999 240 Weight: 193 lb 5.0 LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins, except BOT CHORD 2 X 4 SYP No.2 end verticals 2 X 4 SYP No.3 *Except* BOT CHORD Rigid ceiling directly applied or 5-0-5 oc bracing. WEBS WEBS 2 X 4 SYP No.3 - 5-13, 7-9 W6: 2 X 8 SYP No.1D T-Brace: Fasten T and I braces to narrow edge of web with 10d Common wire nails. 9in o.c. with 4in minimum end distance. Brace must cover 90% of web length. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 9=1130/0-3-8, 2=1223/0-7-8 Max Horz 2=216(LC 6) Max Uplift 9=-429(LC 5), 2=-367(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2108/1415, 3-4=-1901/1281, 4-5=-1671/1217, 5-6=-2187/1522, 6-7=-1714/1165 2-13=-1438/1803, 12-13=-1580/2204, 11-12=-1580/2204, 10-11=-1469/2089, 9-10=-1007/1431 WEBS 3-13=-167/256, 4-13=-293/530, 5-13=-642/436, 6-10=-574/465, 7-10=-335/595, 7-9=-1665/1179 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be SYP No.2.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstending 2-367.

7) "Semi-rioid nitebhroads. 5) All bearings are assumed to 22
367.

6) Provide mechanical connection (by others) of truss to bearing 2=367.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular STATE OF building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

FLORIDA:

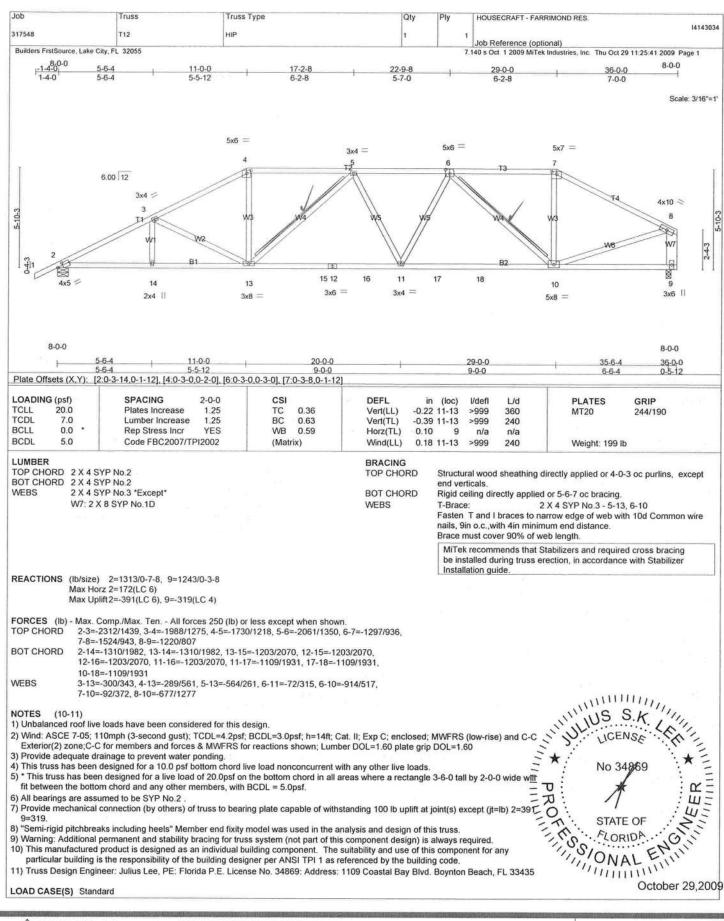
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OC October 29,2009

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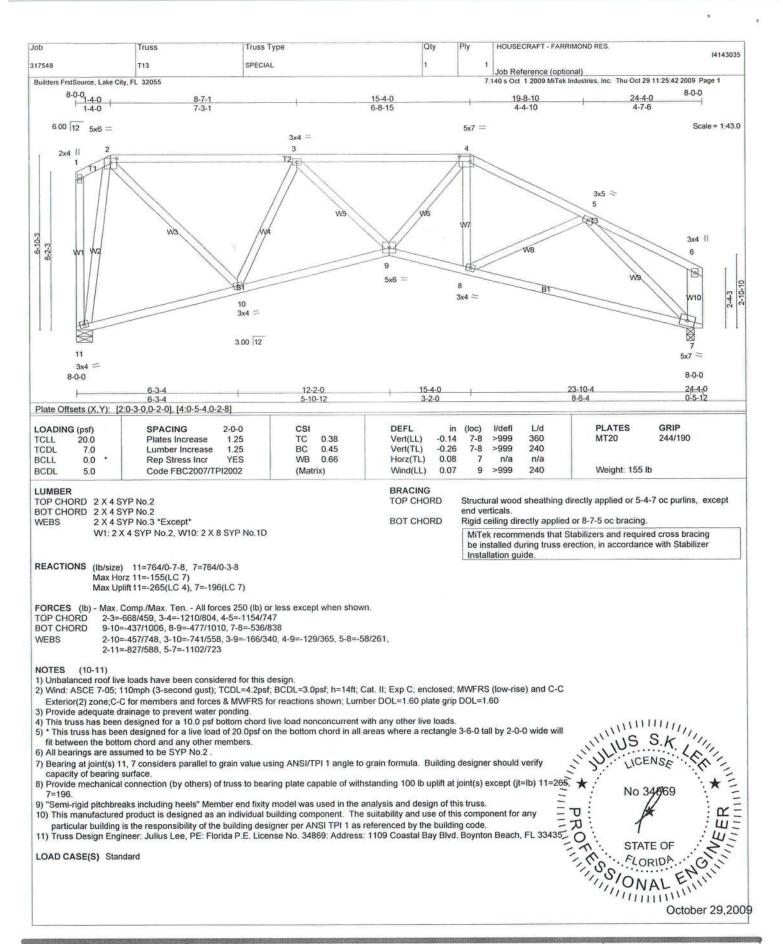
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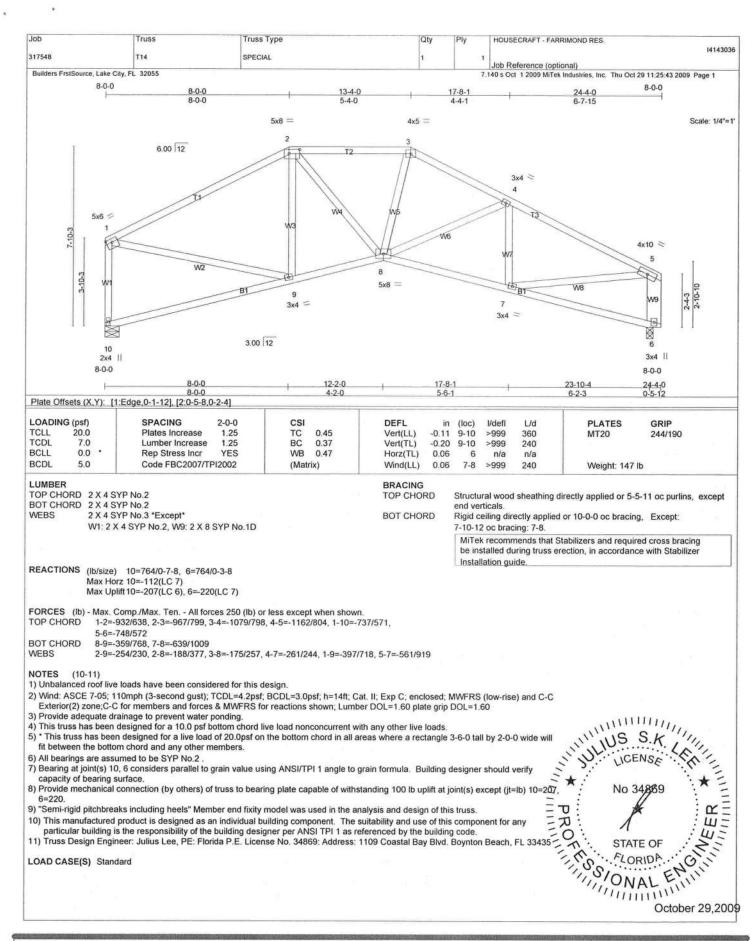
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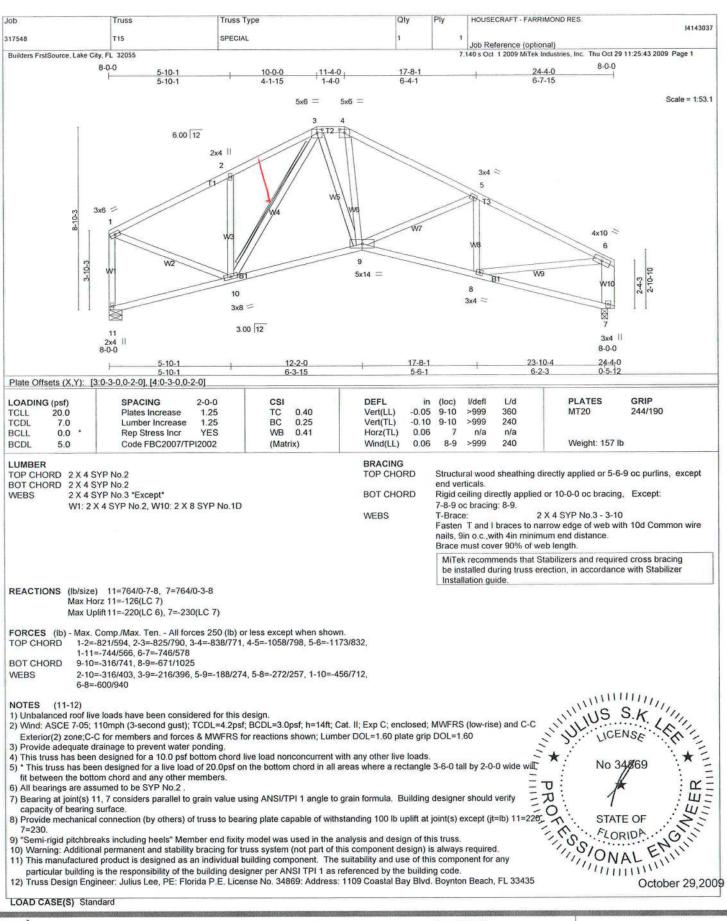


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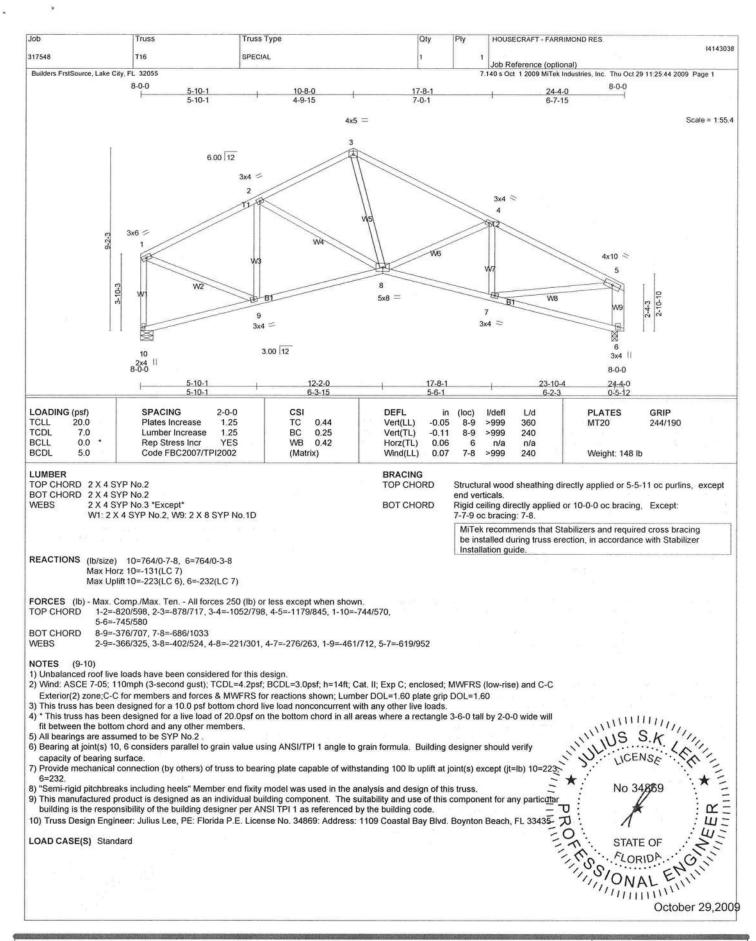


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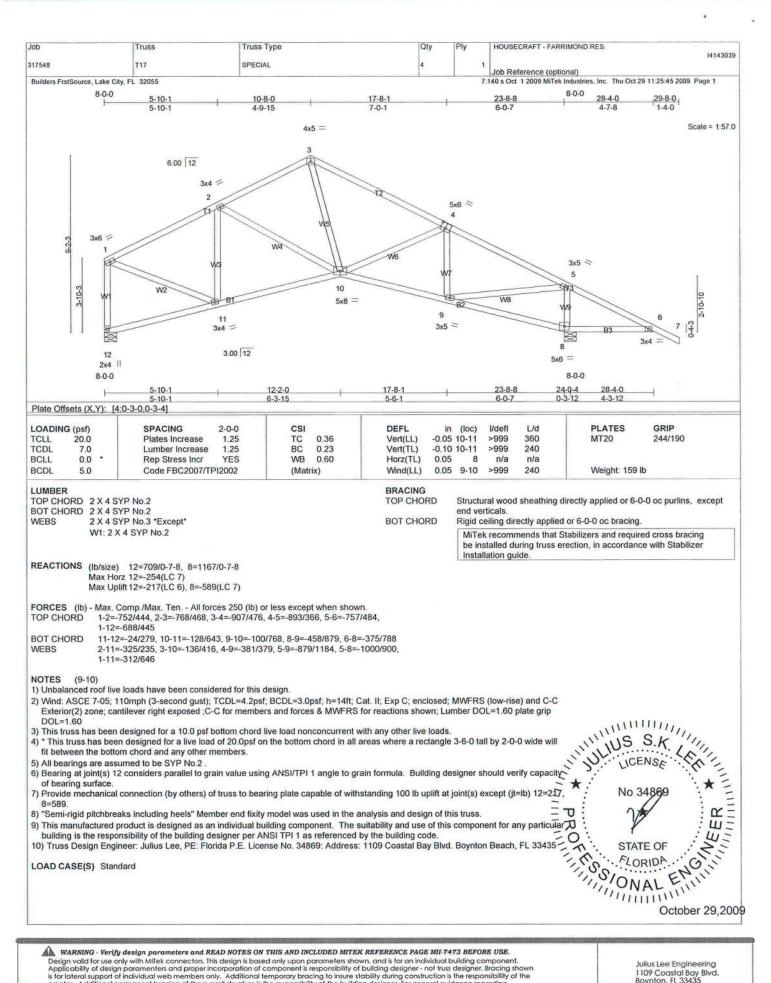
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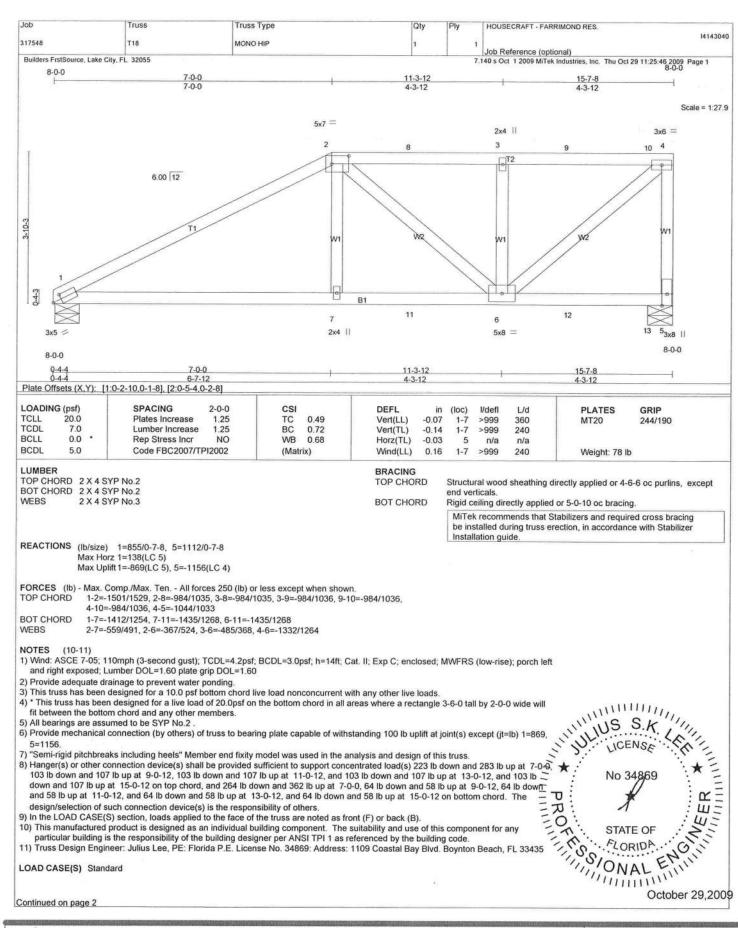
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1109 Coastal Bay Blvd. Boynton, FL 33435



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Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	14143040
317548	T18	MONO HIP	1	1	Job Reference (optional)	200 A T.T. TARRING WARNING
Builders FrstSource, Lake City, F	L 32055			7.	140 s Oct 1 2009 MiTek Industries, Inc. Thu Oct 29 11:25:46	6 2009 Page 2

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 1-5=-10

Concentrated Loads (lb)
Vert: 2=-223(F) 7=-224(F) 3=-103(F) 6=-33(F) 8=-103(F) 9=-103(F) 10=-103(F) 11=-33(F) 12=-33(F) 13=-33(F)

No 34869

No 34869

No 34869

STATE OF

FLORIDA

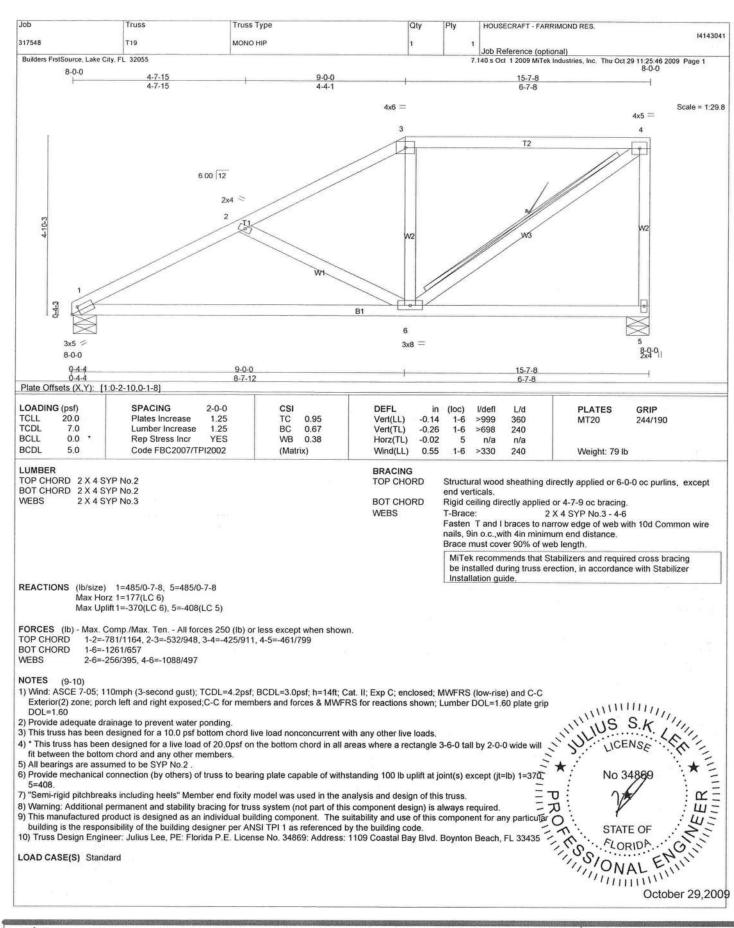
October 29,200

October 29,2009

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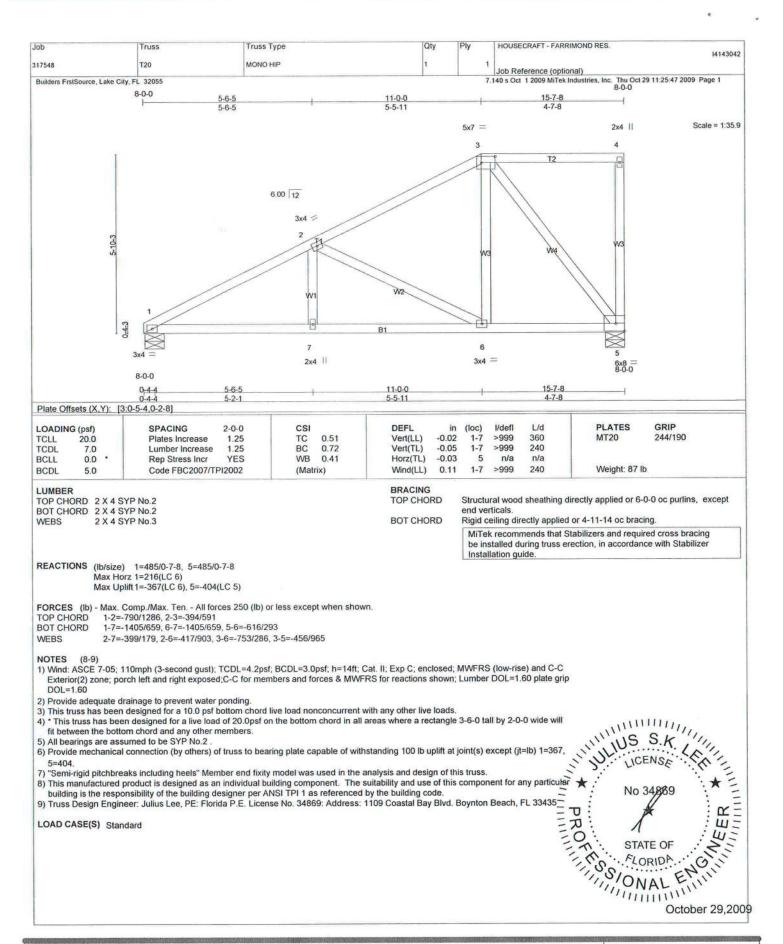
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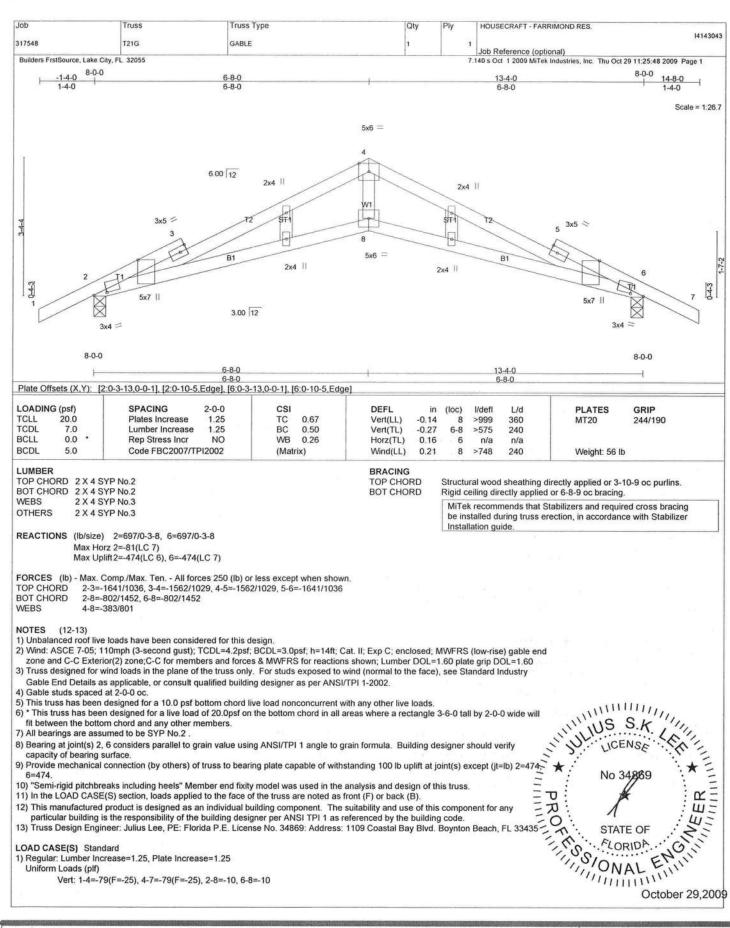
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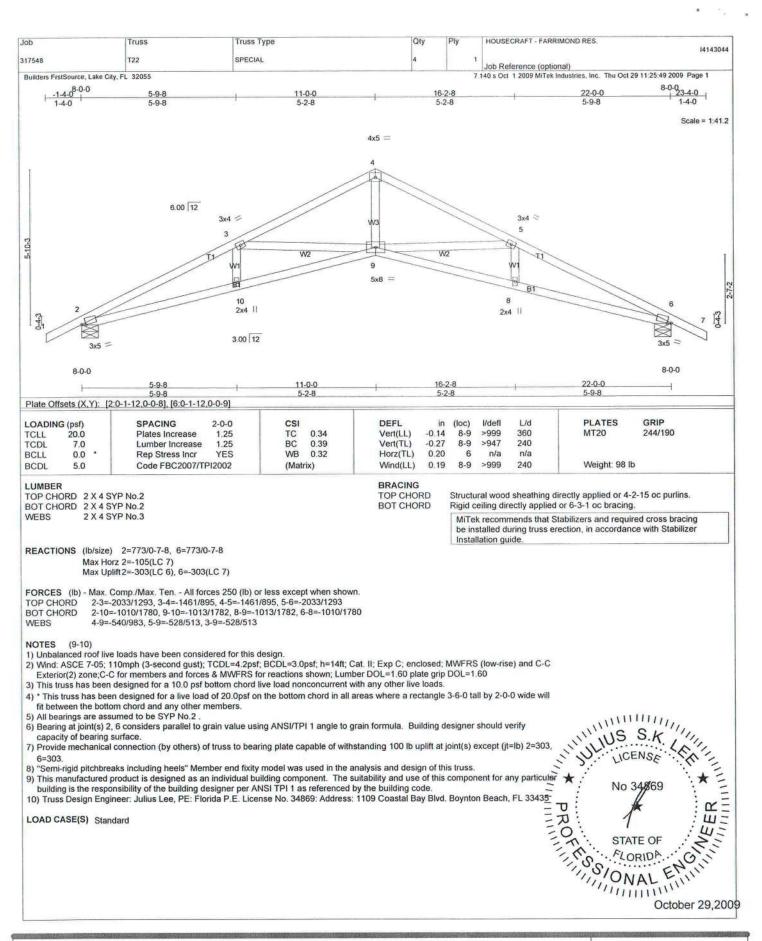
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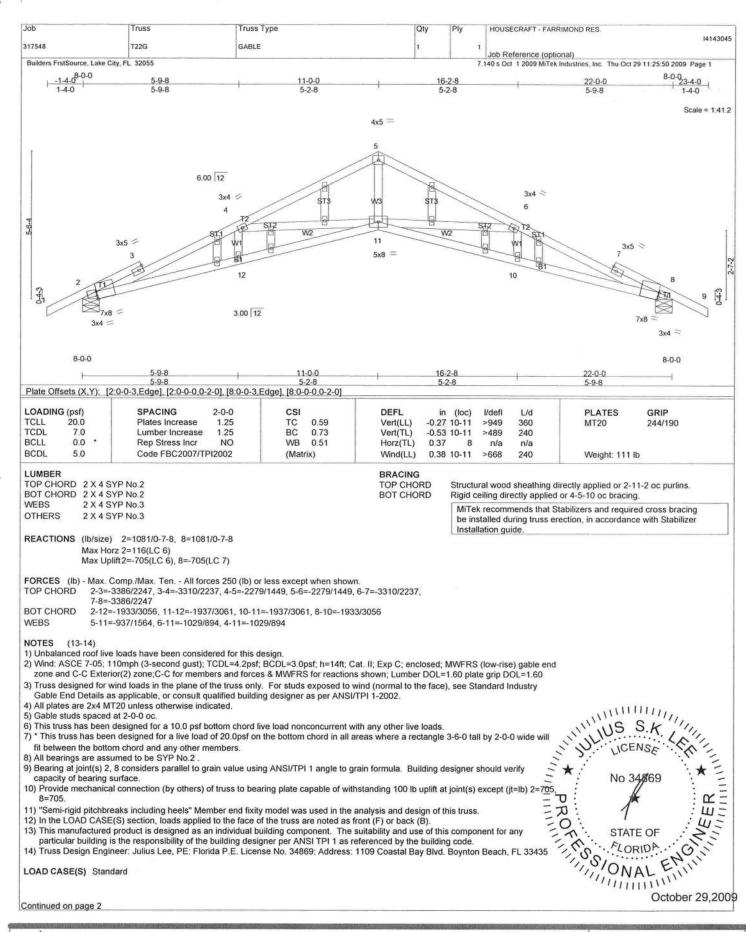


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Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.	11607 0200 102
317548	T22G	GABLE	1	1	Lab Professore (actional)	14143045
P. lide - France Lake City I	72055			7	Job Reference (optional)	nne 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-5=-79(F=-25), 5-9=-79(F=-25), 2-11=-10, 8-11=-10

No 34869

No 34869

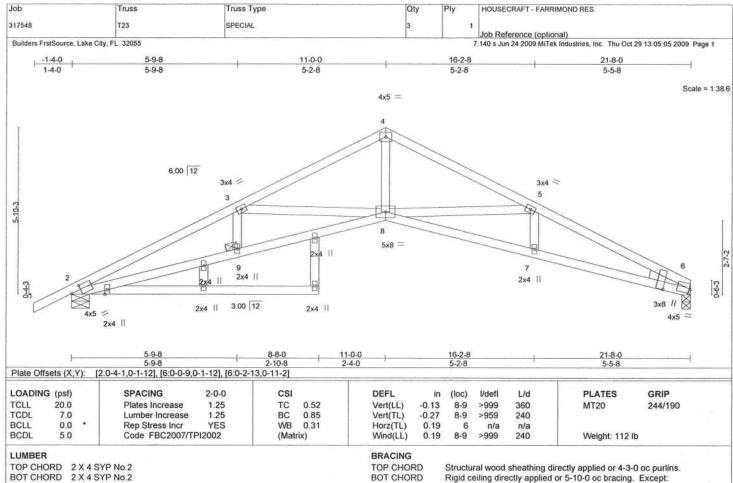
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5-11-0 oc bracing: 2-8

Installation guide.

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

WEDGE

Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 2=771/0-7-8, 6=675/0-3-8

Max Horz 2=115(LC 6)

Max Uplift 2=-303(LC 6), 6=-210(LC 7)

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-2026/1355, 3-4=-1453/963, 4-5=-1453/964, 5-6=-2007/1387

BOT CHORD 2-9=-1125/1774, 8-9=-1128/1776, 7-8=-1161/1771, 6-7=-1160/1770 WEBS 4-8=-604/978, 5-8=-529/540, 3-8=-529/507

NOTES (8-9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=14ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 210 lb uplift at "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL

LOAD CASE(S) Standard

No 34669

No 34669

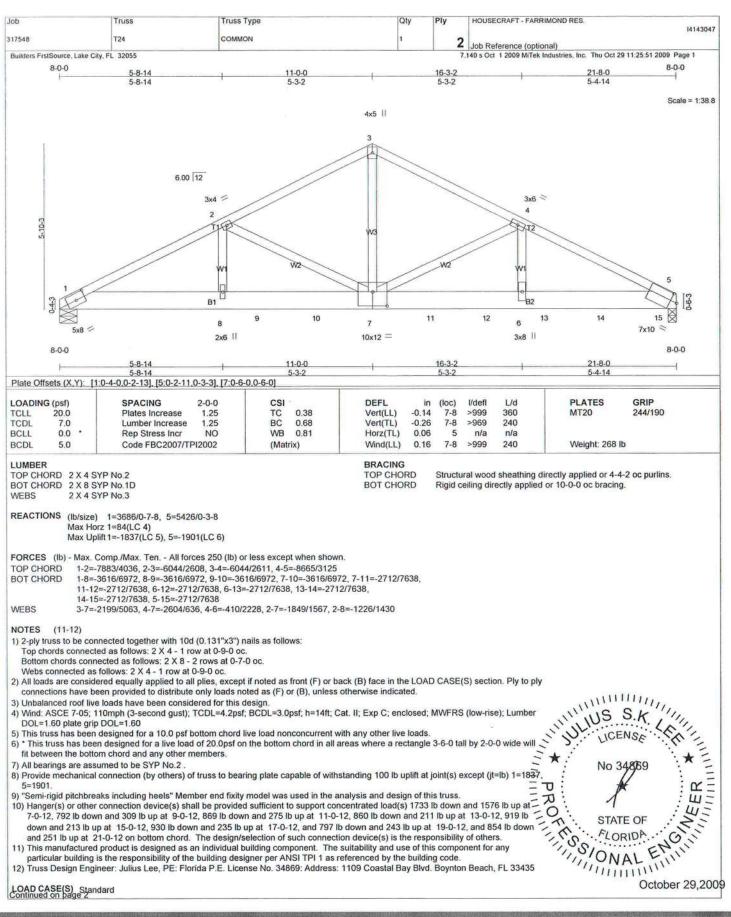
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ob	Truss	Truss Type		Qty	Ply	HOUSECRA	FT - FARRIMOND RES.	141430
7548	T24	COMMON		1	2	Inh Defect	nce (optional)	141430
uilders FrstSource, Lake City, I	FL 32055				7	1.140 s Oct 1 20	nce (optional) 009 MiTek Industries, Inc. Thu Oct 2	9 11:25:51 2009 Page 2
Uniform Loads (plf)	d ase=1.25, Plate Increase= 3-5=-54, 1-5=-10	1.25						
Concentrated Loads (Ib))) 9=-1733(F) 10=-792(F)	1=-860(F) 12=-919(F) 1	3=-930(F) 14=-797(F	F) 15=-854(F	=)			
	1							
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Julius Lee Engineering 1109 Coastal Bay Blvd. Boynton, FL 33435

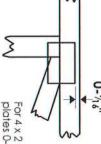
October 29,2009

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth offsets are indicated.



plates 0- 1/16" from outside For 4 x 2 orientation, locate edge of truss.

11

required direction of slots in connector plates. This symbol indicates the

00

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

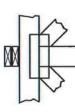
the length parallel to slots. width measured perpendicular to slots. Second dimension is The first dimension is the plate

LATERAL BRACING LOCATION



if indicated. output. Use T, I or Eliminator bracing Indicated by symbol shown and/or by text in the bracing section of the

BEARING



reaction section indicates joint Indicates location where bearings number where bearings occur (supports) occur. Icons vary but

ANSI/TPII: Industry Standards:

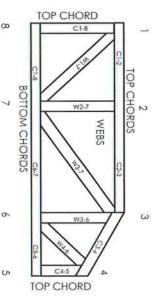
Building Component Safety Information. Design Standard for Bracing. Plate Connected Wood Truss Construction National Design Specification for Metal

Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling,

BCSI1: DSB-89

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

NUMBERS/LETTERS CHORDS AND WEBS ARE IDENTIFIED BY END JOINT

PRODUCT CODE APPROVALS

ICC-ES Reports:

9730, 95-43, 96-31, 9667A NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A ESR-1311, ESR-1352, ER-5243, 9604B

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Boynton, FL 33435 Julius Lee Engineering 1 109 Coastal Bay Blvd



General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative $\mathsf{I},\mathsf{I},\mathsf{or}$ Eliminator bracing should be considered.

2

Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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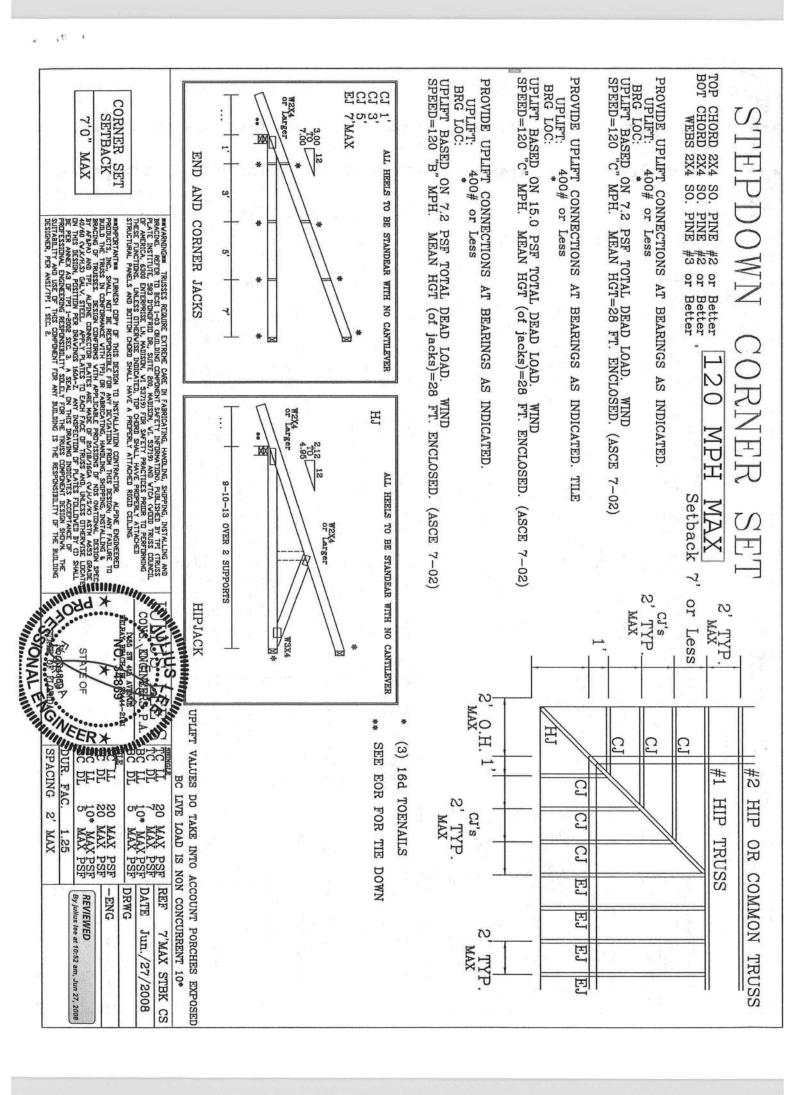
all other interested parties. Provide copies of this truss design to the building

4

Cut members to bear tightly against each other

CT

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP11.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or after truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



CONNECT DIAGONAL AT THE MARGING TERM THE SET REDUCE EXTREME CARE IN FARSICATION, HANDLING, SHEPPING, INSTALLING AND REAL MARGING, REFER TO CHART ABOVE FOR INFORMATION, PUBLISHED BY 191 CRASS CHART AND STALLING AND SHAPPING, INSTALLING AND SHAPPIN DIAGONAL BEACE OPTION:
VERTICAL LENGTE MAY BE
DOUBLED WICH DIAGONAL
HRACE IS USED, CONNECT
HRACENAL BEACE TUR SAG!
AT EACH END. MAX WEB VERTICAL TOTAL MANGIN IS 14". MAX GABLE LENGTH IN TABLE ABOVE. SPACING SPECIES 24" 16" 12" O.C. O.C. O.C. GABLE VERTICAL SPF SPF SPF DFL DFI DFL ASCE NACHB STANDARD STANDARD #1 / #2 STANDARD GRADE STANDARD STANDARD STANDARD CLLIS BRACE 7-02: GABLE THUSS No m 6 6 6 6 130 SPF #/#Z, DF-L #Z,
SPF #/#Z, DR BETTEH
DIAGONAL BRACE;
SINGLE OR DOUBLE
CUT (AS SHIPN) AT GROUP (1) 1X4 "L" BRACE * MPH UPPER END. GROUP H WIND D 10 12 GROUP A (1) 2X4 "L" BRACE . SPEED BRACE GROUP B 15 MAKYE 18 (2) 2X4 "L" BRACE ** GROUP A MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH EX4 #EN OR BETTER CONLINGOR BRYKING GROUP 10 2° HEIGHT, · (D) 5 യയ B CONS. GROUP (1) 2X8 DELRAY HEACH, FL. 33444-2161 12. 9. 21. 0. 10. 10' 10" STATE OF FLORIDA US LEE'S P.A. A ENCLOSED, "L" BRACE . GROUP B 12' 6" 13,10 12 13 12 (2) GROUP A 12 BXZ H MAX MAX. HRACE GROUP B II 12 11 8 11 13' 3" 13' 7" 12' 0" 14' D' TOT. 1.00, SPACING 1 B ATTACE EACH "L" ERACE WITH 104 NAIS.

* FOR (1) "L" BRACE; SPACE NAIS AT 2" O.C.

* FUR (2) "L" BRACES; SPACE NAIS AT 3" O.C.

** FUR (2) "L" BRACES; SPACE NAIS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. PROVIDE UPLIFT CONNECTIONS FOR 136 FLF OVER CONVINCIOUS BEARING (6 PSF TC BEAD LOAD). T. BRACING MUST BE A MINIMUM OF BOX OF WEB CABLE END SUPPORTS LOAD FROM 4' 0" LIVE LOAD DEPLECTION CRITERIA IS L/240. MEMBER LENGTH. DOUGLAS FIR-LARCH SPRUCE-PINE-ITB

41 / 42 | STANDARD

43 | STUD PLYWOOD OVERHANG. BRACING EXPOSURE VERTICAL LENGTH
LESS THAN 4 0 GREATER THAN 11 8 CAHLE BOUTHDAY PINE 60 GREATER THAN 11 6 24.0 PEAK, SPLICE, AND HEEL FLATES. CABLE VERTICAL PLATE SIZES STANDARD PSF GROUP SPECIES TRUSS DATE REF -ENG DRWG #1 & BIB GROUP GROUP DETAIL TUB MITEX SID GABLE 16 E HI DOUGLAS FIR-LARCH 11/26/03 ASCB7-02-GAB13015 ₿; SOUTHERN PORE A: £3 1X4 OR EXS STANDARD AND GRADES: STANDARD NOTES: 2.5X4 7XZ

2.

ASCE 7-02: 130 MPH WIND SPEED, 30 MEAN HEIGHT, ENCLOSED, 11 1.00, EXPOSURE a

OUP SPECIES

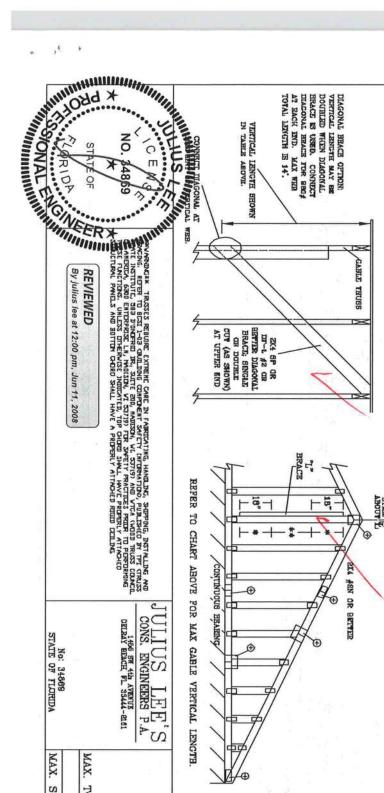
AND GRADES:

GROUP A:

STAUDARD

STANDARD GIVE

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			M	A	X		C	i I	I	3]		E		V	E	R	T	Ί	C	A	L		L	E	N	10	[1	Ή	
		1	2	33	10	0	.(٦,			1	6	51	9	0	.(ζ.			2	4	31		0	.(C		SPACING	CARI
			1	7)	TIT	I I	ילט	בובים		L L	1	7)	TIT	디 디	ひてり	בובם		1	1		j	TIL	ij	ひてっ	בובה	SPACING SPECIES	ZX4
	STANDARD	STUD	哲	#22	#1	STANDARD	STUD	#3	£1 / #8	STANDARD	CUIS	t 3	#22	+1	STANDARD	CUIS	#3	£1 / #2	STANDARD	STUD	£4	#13	11	STANDARD	STUD	₽\$	£1 / #2	GRADE	BRACE
	4' 0"	4.	4. 23.	4' 4"	4' 5"	3' 11"	3' 11"	3' 11"	4. 0.	3' 8"		3' 9"	8' 11"	4. 0.		3' 7"	3' 7"	1.3	3' 0"	3' 3"		3' 6"	3' 6"	2. 11.	3' 1"	3' 1"	si,	BRACES	Z 5
	5, 6,	6 4	B. 63	6' 11"	B' 11°	5' 4"	8 3	8 3,	6 11	4' 9"	5 6	5° 7°	8' 4"	8 4"		B' 6"		6 4		4' 8"		5' 6"		3, 8,	4' 6"	4' 5	5' 6',	GROUP A	(1) 1X4
	5' 6"	8' 4"	6' 5'		7' 8"	5' 4"	6. 3.	8 3	7. 8.	4' 9"		6. 7.	8' 10"		4. 8.	6, 6,	5, 5,	6. 6.	3' 10"	4' 6"	4' 6"	5' 11.°		3, 9,		4' 50	6′ 8"	GROUP H	T BRACE .
	7' 3"	8 3		B' 3"	8 3		A 3"	8 a	8. 3.				7' 8'		6. 5.		7. 2.	7. 8.	5' 1"	5' 11"		6, 9,				6. 10.	6' 6"	GROUP A	(1) 2X4 "L"
KAS	7' 3"	8 6		8' 11"	B' 11°		8 ² 3 ²		B. 6.				8' 1"		6. 2.				6 1.	5' 11"	8. 0.	7' 0"		5. 0.	5' 10°	5' 10"	6. 8.	GROUP B	L" BRACE .
3 PULLS	8. 8.	9, 10,		8, 10,	8, 10,		9' 10"		9. 10.			8. 11	B' 11"	8, 11,			8' 11"		8' 11"	7' 10"	7' 10"	7' 10"	7' 10"	6, 9,	7' 10"	7' 10"	7' 10"	GROUP	(2) 2X4 "L"
\	8, 8,	10' 4"	10' 4"	10' 7"	10' 7"	9, 8,	9' 10"	9' 10"	10. 1.	8' 5"	8, 2,	9. 6.	8, 2,	-	8. 3.	0.00	8' 11"		6, 11,	8) 0"	8, 1,	e' 5°	7	e, 9.	7' 10"	24553	8, 0.	A GROUP B	L" BRACE **
	11' 4"	12' 11"	12' 11"	12 11	12' 11"	11' 1"	12' 10"	12' 11"	12' 11"		11' 4"	11. 6.	11. 9.	11, 8,	9. 7.	11. 1	11' 2"	11. 9.	B' 0*	8, 3,	9. 4.	10′ 3°	10' 3"	7' 10"	9' 1°	9' 1"	10' 3"	GROUP A	(1) 2Xe
	11' 4"	13. 1.	18' 3"	13' 11°	13' 11"	11' 1"	12' 10"	12' 11"	18' 4"	8, 8,,	11' 4"	11' 6'	12' B"	12, 8,	8. 2.	11, 1,,	11, 5,	12' 1"	8. 0.	8, 3,	e: 4.	11, 1,	11. 1.	7' 10"	9, 1,	9' 1"	10' 7"		"L" BRACE .
	14' 0"	14 O"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14 00	T4. 0"	13' 9"	14 0	14. 0.	14' 0"	14, 0,	18. 11.	14' 0"	14' 0"	14. 0	10' 10"	12' 3"	12. 3.	12' 3"	12' 3"	10' 7"	12' 3*	12' 3"	12, 3,	GROUP B GROUP A GROUP B	.T. 8XZ (2)
	14' 0"	14. 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14. 0	13' 3"	14' 0"	14. 0	14' D"	14' 0"	18. 11.	14' 0"	14' 0"	14. 0.	10' 10"	12' 6"	12, 8.	13' 2"	13' 2"	10, 7,	12' 3"	12' 3"	12' 7"	GROUP B	HRACE .
DULITONGENS ALTH S. O.	CASE STORES OF THE LOCAL PROPERTY OF THE PARTY OF THE PAR	CONTINUOUS BEARING (6	PROVIDE THEIR CHARGE	LIVE LOAD DEPLECTION CR	Over Two or T	CADIE TOILES I			*22	15	SOUTHERN PINE		#I 00	TEK-	OI OI	iloan.			CHARTAN		DOUGLAS FIR-LARCH		TO THE CONTROL	AI / HZ STANDARD	ONO O	Iloan	BRACING GROUP SPE		



DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR SEC! AT EACH END. MAX WEB

DIAGONAL BRACE OFTION:

GABLE TRUSS

RUSS DETAIL NOTES: ECTION CHATERIA IS L/240.

HEM-PIR GROUP B:

DOUGLAS FIR-LARCH

PLYWOOD OVERHANG. PORTS LOAD FROM 4 0" GEARING (5 PSF TC DEAD LOAD).

ATIMCE EACH 'L' ERACE WITH 104 NAILS.

* FOR (1) 'L' BEACES, SPACE NAILS AF 2 O.C.

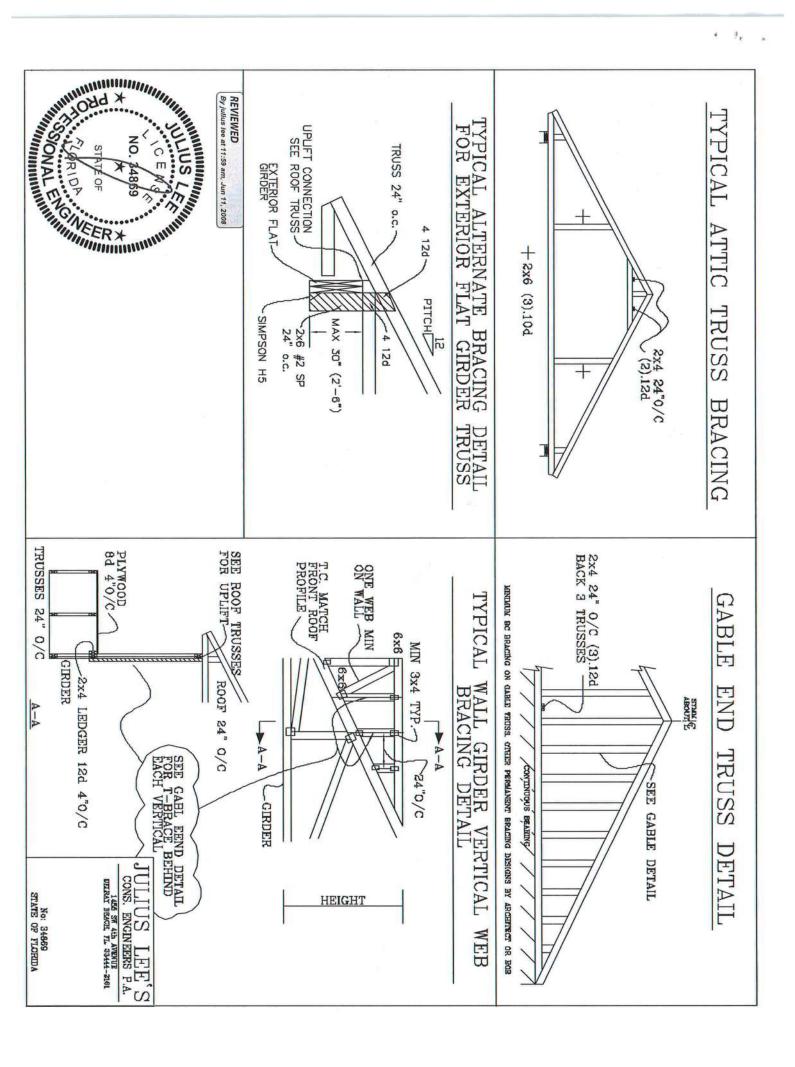
* FOR (2) 'L' BEACES; EFACE NAILS AT 3° O.C.
IN 18° END ZONES AND 6° O.C. BETWEEN ZONES. MEMBER LENGTH. T. BRACING MUST BE A MINIMUM OF BOX OF WEB

PLATES.	PEAK, SPLICE, AND HEEL
2.5X4	GREATER THAN 11' 6"
2X4	GREATER THAN 4' D', BUT
TX4 DR BX3	IPSS THAN 4' 0"
ND SPLICE	VEHINCAL LENGTH
E SIZES	GABLE VERTICAL PLAN

	24.0"	MAX. SPACING 24.0"	MAX. S	No: 34869 STATE OF FLORIDA
11	MAX. TOT. LD. 60 PSF	OT. LD.	MAX. T	
-ENG				
DWG MEEK SED GABLE SO' E HT				PLRAY BEACH, FL 33444-8161
DATE 11/26/09	la U			ONS. ENGINEERS P.A.
REF ASCE7-02-GAB13030				S, HHI SIIII

CONS.

DELRAY BIACH, FI



BOT CHORD 284 444 经路路 BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

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5

30'

34

88

2.5X4

2.6X4

3X5 52

REFER TO SEALED DESIGN FOR DASHED PLATES.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER. SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-03, CLOSED HIDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL-5 PSF 110 MPH WIND, 30' MBAN HGT, FBG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND HC DL=6 PSF

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5X4 EX3

5X6

53

BX6

OR SX6 TRULOX AT 4'

BC,

C

1.5X4

1.5X4

1.5X4

ш

4XB 284

5X6

5X8

9X9

FRONT FACE (8,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. EITHER PLATE LOCATION IS ACCEPTABLE ZX ZX 12 20' FLAT TOP CHORD MAX SPAN B MAX SIZE OF ZXIZ #2 OR BETTER 要 ш J-TYP D-SPLICE

0

* PIGGYBACK SPECIAL PLATE

D C

ATTACH TRULOX EQUAL, PER FA BE CONNECTED INFORMATION.
C PLATES WITH (8) 0.120" X 1.375" NAILS, OR CE PER PLY. (4) NAILS IN EACH MEMBER TO REFER TO DRAWING 160 TL FOR TRULOX

WEB LENGIH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB
9 9	AJICS AT 4" OC.
10' TO 14'	ZX4 "T" BRACE. SAME GRADE, SPECIES AS WEB
	MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

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THIS DRAWING REPLACES DRAWINGS 634,018 634,017 & 847,045

PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

C

STATE OF FLORIDA			DIARAY BEACH, FL. 33444 2161	CONS. ENGINEERS P.A.	A, 보면 I SIII IIII
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	50 PSF AT 1.25 DUR. FAC.	1.33 DUR. FAC.	55 PSF AT	MAX LOADING
	=	-ENG JL	DRWGMITEK STD PIGGY	DATE 09/12/07	REF PIGGYBACK

NO. 34869

TOP CHORD CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR 2X4 SP #3 OR BETTER. BETTER

Di

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE)
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENC BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF. FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR OR (3) 16d FOR ENCLOSED

UNIESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VALLEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

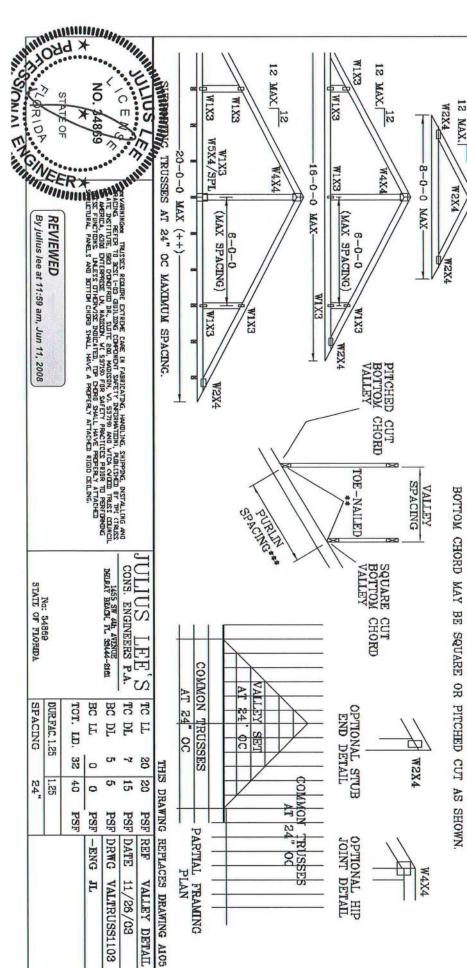
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED

ENGINEERS' SEALED DESIGN.

++ LARGER SPANS NAY BE NOT EXCEED 12'0". BUILT AS LONG AS THE VERTICAL HEIGHT DOES

BOTTOM CHORD MAY BE



CONS.

DELRAY BEACH, I'L 35444-8101

BC LL BC TC

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DATE

11/26/09 VALTRUSS1103

TOT.

E.

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PSF PSF PSF PSF

STATE OF FLORIDA

SPACING DUR.FAC. 1.25

24 1.25 *** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

LARGER AS REQ'D

4-0-0

MAX

SQUARE OR PITCHED CUT AS SHOWN

TOE-NAIL \mathtt{DETAIL}

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

5	4	ಎ	N	TOE-NAILS	NUMBER OF
5 493 # 639 # 452 # 585# 390 # 507# 384	394#	296#	197#	1 PLY	
639#	511#	383#	256#	2 PLIES	SOUTHERN PINE
452#	361#	271#	181#	1 PLY	DOUGLAS
585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
390#	312#	234#	156#	1 PLY	
507#	406#	304#	203#	2 PLIES	HEM-FIR
384#	307#	230#	154#	1 PLY	SPRUCE
496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR

1/8" OPTIONAL GIRDER (2) PLY

\	30°-60°	
JACK ALTERNATIVE CONDITION	1 1/8"	(2) PLY GIRDER
NDITION		NAL LY R

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×	,).	•••				
Sill Control of the C	11. A.	STATE OF	***	NO. 34869	CEMA	WALL TO THE PROPERTY OF THE PARTY OF THE PAR	WHITE STATES
***	By julius lee at	REVIEWED	SEDICTURAL PANELS AND B	MA	WARNING TRUSSES REQUIRE EXTREM		JACK
	By Julius lee at 11:59 am. Jun 11, 2008		L PANELS AND BUTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CELLING	ITTE 200, NADISON, WL 337(9) AND VICA (WOLD IT	DURE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND LING COMPONENT SAFETY (MITRIALING, PRINTERS IN FARRICATING, PRINTERS IN FARRICATIONS AND LING COMPONENT SAFETY (MITRIALING, PRINTERS IN FARRICATIONS).		- 50
STATE OF FLORIDA	No: 34889	2.1	5.0	DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	S'HHI SULIUL	
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL	THE
	1.00	PSF	PSF	PSF	PSF	PSF	DRAWING RE
			-ENG JL	DRWG	DATE	REF	PLACES
			JL.	DRWG CNTONALL103	09/12/07	TOE-NAIL	THIS DRAWING REPLACES DRAWING 784040

DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO

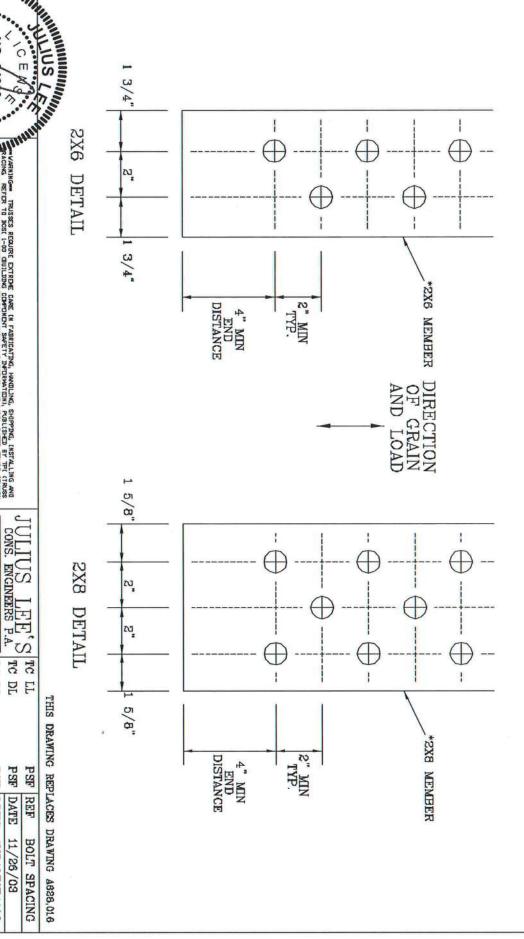
* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

4 h

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PAITERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



LARIOR AND HARRING THE NAME OF THE PARTY OF

REVIEWED

By julius lee at 11:59 am, Jun 11, 2008

CONS.

TC BC DL

DELRAY BEACH, FL 33444-2161

No: 34869 STATE OF FLORIDA

SPACING

DUR. FAC.

TOT. BC

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> CNBOLTSP1103 11/26/03

DATE

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TRULOX CONNECTION DETAI

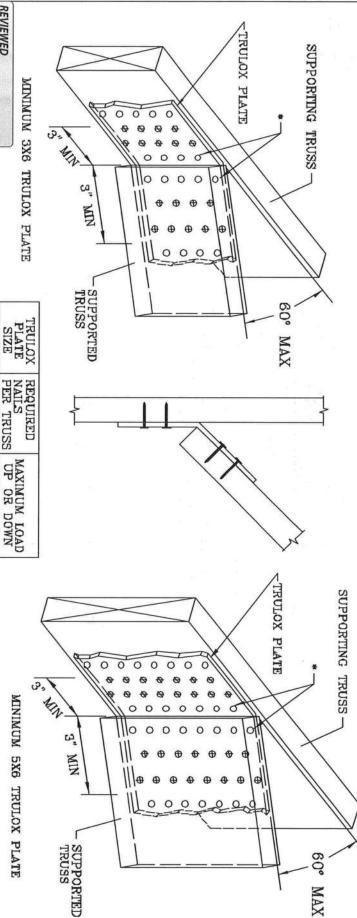
II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\(\phi \)).

NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

INFORMATION NOT SHOWN. REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER



NO. 24889

STATE OF THE REAL PROPERTY OF THE PARTY OF THE

UNGAM TRUSSES REQUIRE EXTREME CARE IN FABRICATIVA, HAVILLING, SHIPPING, INSTALLING AND GERERA TO 3531-04 (BUILLING COAPONENT SAFETY MEGRAMITION, PUBLISSED BY TP) (TRUSS ONSTITUTE, 1933 DYDNORTHE MR, SUITE BO, MAISSON, VI. 33739) AND VICA VOIDH TRUSS COUNTIESCA, 4300 ENTERPRISE LIV, MAISSIN, VI. 33739 FOR SAFETY PRACTICES PRICE TO PEFFENNING FUNCTIONS. VALESS DIFFERVING MEGRAMINATIONED SHALL HAVE PROPERLY ATTACKET TO PEFFENNING FUNCTIONS. VALESS DIFFERVING MEGRAMINATIONED SHALL HAVE A PROPERLY ATTACKET RIGHT CECLLING.

By julius lee at 11:58 am, Jun 11, 2008

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#088 350#

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CONS. ENGINEERS PA. DELEAT BEACH, IL 30444-2161

DRWG DATE REF

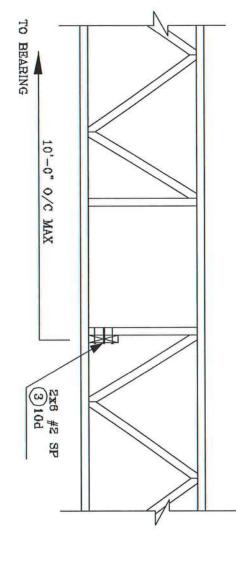
CNTRULOX1103 11/26/03 TRULOX 1,154,844

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,152,217 1,152,017 1,159,154 & 1,151,524

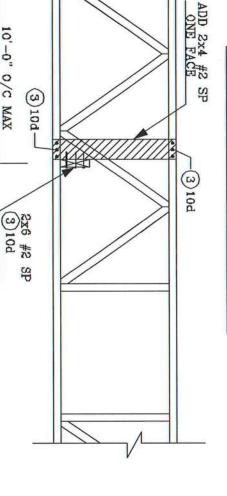
MINIMUM 5X6 TRULOX PLATE

No: 34869 STATE OF FLORIDA

REVIEWED



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



By julius lee at 11:58 am, Jun 11, 2008

REVIEWED

ULIUS LEE'S cons. ENGINEERS P.A. DELRAY BEACH, TL 33444-2161

No: 34869 STATE OF FLORIDA

NO. 4889 NO.

TO BEARING

10'-0" 0/C MAX

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern						
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F	
			2" 134"	-114"	11/4" 31/2"				
			31/2" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply	
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245			
	3	12"	555	415	415	370		University of the last	
1/2" A307 Through Bolts(2)(4)	2	24"	505	380	520	465	860	340	
		19.2"	635	475	655	580	1,075	425	
iniough boits		16"	760	570	785	695	1,290	505	
	2	24"	680	510	510	455			
SDS 1/4" x 31/2"(4)		19.2"	850	640	640	565			
		16"	1,020	765	765	680		more from the least of the later of	
SDS 1/4" x 6"(3)(4)	2	24"				455	465	455	
		19.2"				565	580	565	
		16"				680	695	680	
USP WS35 (4)	2	24"	480	360	360	320			
		19.2"	600	450	450	400			
		16"	715	540	540	480			
USP WS6 (3)(4)	2	24"				350	525	350	
		19.2"				440	660	440	
		16"				525	790	525	
22/8	2	24"	635	475	475	425			
33/e" TrussLok ⁽⁴⁾		19.2"	795	595	595	530		No.	
		16"	955	715	715	635		THE WORLD	
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445	
		19.2"		625	625	555	600	555	
		16"		750	750	665	725	665	
6¾" TrussLok ⁽⁴⁾	2	24"				445	620	445	
		19.2"				555	770	555	
		16"			MEALINE TO S	665	925	665	

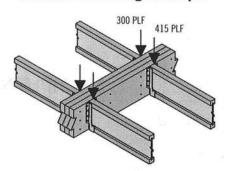
Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

- (2) Washers required. Bolt holes to be 9/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
 Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16-33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
 of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16-33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 134" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.



MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

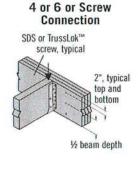
		Connector Pattern								
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F			
Connector Type	Number of Connectors	1 2" 1 31/2" 31/2"		1½" 3½"	134" 31½" 134"	1 2" 31/2"	134"			
	6	2-ply 1,110	3-ply 835	2-ply 835	3-ply 740	2-ply	4-ply			
10d (0.128" x 3") Nail	12	2,225	1,670	1,670	1,485					
	18	3,335	2,505	2,505	2,225	Control of the second second				
	24	4,450	3,335	3,335	2,965					
SDS Screws 1/4" x 31/2" or WS35 1/4" x 6" or WS6(1)	4	1,915	1.435(4)	1,435	1,275	1,860(2)	1,405(2)			
	6	2,870	2,150 (4)	2,150	1,915	2,785(2)	2,110(2)			
	8	3,825	2,870 (4)	2,870	2,550	3,715(2)	2,810(2)			
3³⁄a" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	1,695	1,925(3)	1,775(3)			
	6	3,815	2,860 (4)	2,860	2,545	2,890(3)	2,665(3)			
	8	5,090	3,815 (4)	3,815	3,390	3,855(3)	3,550(3)			

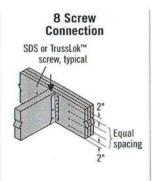
(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

- (2) 6" long screws required.
- (3) 5" long screws required.
- (4) 3½" and 3½" long screws must be installed on both sides.

Connections





Nail Connection

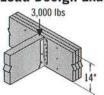
10d (0.128" x 3") nails, typical. Stagger to prevent splitting.

8"-10"
2" spacing, typical

2"
minimum spacing,

There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3¾" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

13/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d-16d (0.148"-0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3¾" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed
- on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

31/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

