

APPLICANTMIKE HERLONG

PHONE497-3991

ADDRESS545SE HUGO LNFORT WHITEFL32038

OWNERGERALD HUETT

PHONE497-3991

ADDRESS492SW FIELDING WAYFORT WHITEFL32038

CONTRACTORMIKE HERLONG

PHONE497-3991

LOCATION OF PROPERTY47 S, L 27, L FIELDING WAY, BARE RIGT AT EASMENT TO  
CABLE AND LET ONTO PROPERTY

TYPE DEVELOPMENTDETACHED GARAGE ,UTI

ESTIMATED COST OF CONSTRUCTION25000.00

HEATED FLOOR AREATOTAL AREA800.00

HEIGHT0.00

STORIES1

FOUNDATIONCONCRETE

WALLSFRAMED

ROOF PITCH6/12

FLOORSLAB

LAND USE & ZONINGA-3

MAX. HEIGHT35

Minimum Set Back Requirments:

STREET-FRONT30.00

REAR25.00

SIDE25.00

NO. EX.D.U.0

FLOOD ZONEX

DEVELOPMENT PERMIT NO.

PARCEL ID03-7S-16-04118-013

SUBDIVISION

LOTBLOCKPHASEUNIT

TOTAL ACRES5.00

RB0029433

Culvert Permit No.

Culvert Waiver

Contractor's License Number

Applicant/Owner/Contractor

EXISITNG PRIVAE05-1213-NBKJHN

Driveway Connection

Septic Tank Number

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS:SFD PERMIT #24152

Check # or Cash1003

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power

Foundation

Monolithic

date/app. by

date/app. by

date/app. by

Under slab rough-in plumbing

Slab

Sheathing/Nailing

date/app. by

date/app. by

date/app. by

Framing

Rough-in plumbing above slab and below wood floor

date/app. by

date/app. by

Electrical rough-in

Heat & Air Duct

Peri. beam (Lintel)

date/app. by

date/app. by

date/app. by

Permanent power

C.O. Final

Culvert

date/app. by

date/app. by

date/app. by

M/H tie downs, blocking, electricity and plumbing

09/07/2005HD

Pool

date/app. by

date/app. by

date/app. by

Reconnection

Pump pole

Utility Pole

date/app. by

date/app. by

date/app. by

M/H Pole

Travel Trailer

Re-roof

date/app. by

date/app. by

date/app. by

BUILDING PERMIT FEE \$125.00

CERTIFICATION FEE \$4.00

SURCHARGE FEE \$4.00

MISC. FEES \$0.00

ZONING CERT. FEE \$

FIRE FEE \$0.00

WASTE FEE \$

FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$

CULVERT FEE \$

TOTAL FEE133.00

INSPECTORS OFFICE

CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

**This Permit Must Be Prominently Posted on Premises During Construction**

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

# Columbia County Building Permit Application

Revised 9-23-04

**For Office Use Only** Application # 060212 Date Received 2-6-06 By LH Permit # 24153  
 Application Approved by - Zoning Official BLK Date 16-02-06 Plans Examiner OK JTH Date 2-13-06  
 Flood Zone X Development Permit NIA Zoning A-3 Land Use Plan Map Category A-3  
 Comments NOC  
Only 1 set of plans turned in

Applicants Name Huey Hawkins Phone call# 961-1727 497-3991  
 Address 6855 SW Elm Church Rd. Fort White, Florida 32038  
 Owners Name Gerald Huettt Phone \_\_\_\_\_  
 911 Address 8672 La Lista Ct. Orlando, Florida 32825  
 Contractors Name Columbia Home Builders, Inc Phone 752-4071  
 Address 545 SE Hugo Ln., Lake City, Florida 32025  
 Fee Simple Owner Name & Address Gerald Huettt, 8672 La Lista Ct, Orlando, Florida 32825  
 Bonding Co. Name & Address \_\_\_\_\_  
 Architect/Engineer Name & Address Huey Hawkins, 6855 SW Elm Church Rd. Fort White  
 Mortgage Lenders Name & Address \_\_\_\_\_  
 Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy  
 Property ID Number 03-75-16 04118-013 Estimated Cost of Construction 25,000  
 Subdivision Name N/A Lot \_\_\_\_\_ Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase \_\_\_\_\_  
 Driving Directions 1 mile SE of Fort White on 4527, Left onto Fielding Way.

Type of Construction Frame, Hardboard Garage Number of Existing Dwellings on Property 0  
 Total Acreage 5 Lot Size \_\_\_\_\_ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive  
 Actual Distance of Structure from Property Lines - Front 195 Side 50 Side 50 Rear 200  
 Total Building Height 15' Number of Stories 1 Heated Floor Area \_\_\_\_\_ Roof Pitch 6/12  
Porch 270 TOTAL 2753

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

**OWNERS AFFIDAVIT:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Huey Hawkins  
 Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA  
 COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me  
 this 6 day of FEB 2006.  
 Personally known ✓ or Produced Identification \_\_\_\_\_

Mike Harley  
 Contractor Signature  
 Contractors License Number RB0029432  
 Competency Card Number \_\_\_\_\_  
**NOTARY STAMP/SEAL**  
 DALE R. BURD  
 Commission # DD0134008  
 Expires 7/18/2008  
 Bonded through \_\_\_\_\_  
 (800-432-4254) Florida Notary Assn., Inc.  
 Notary Signature \_\_\_\_\_

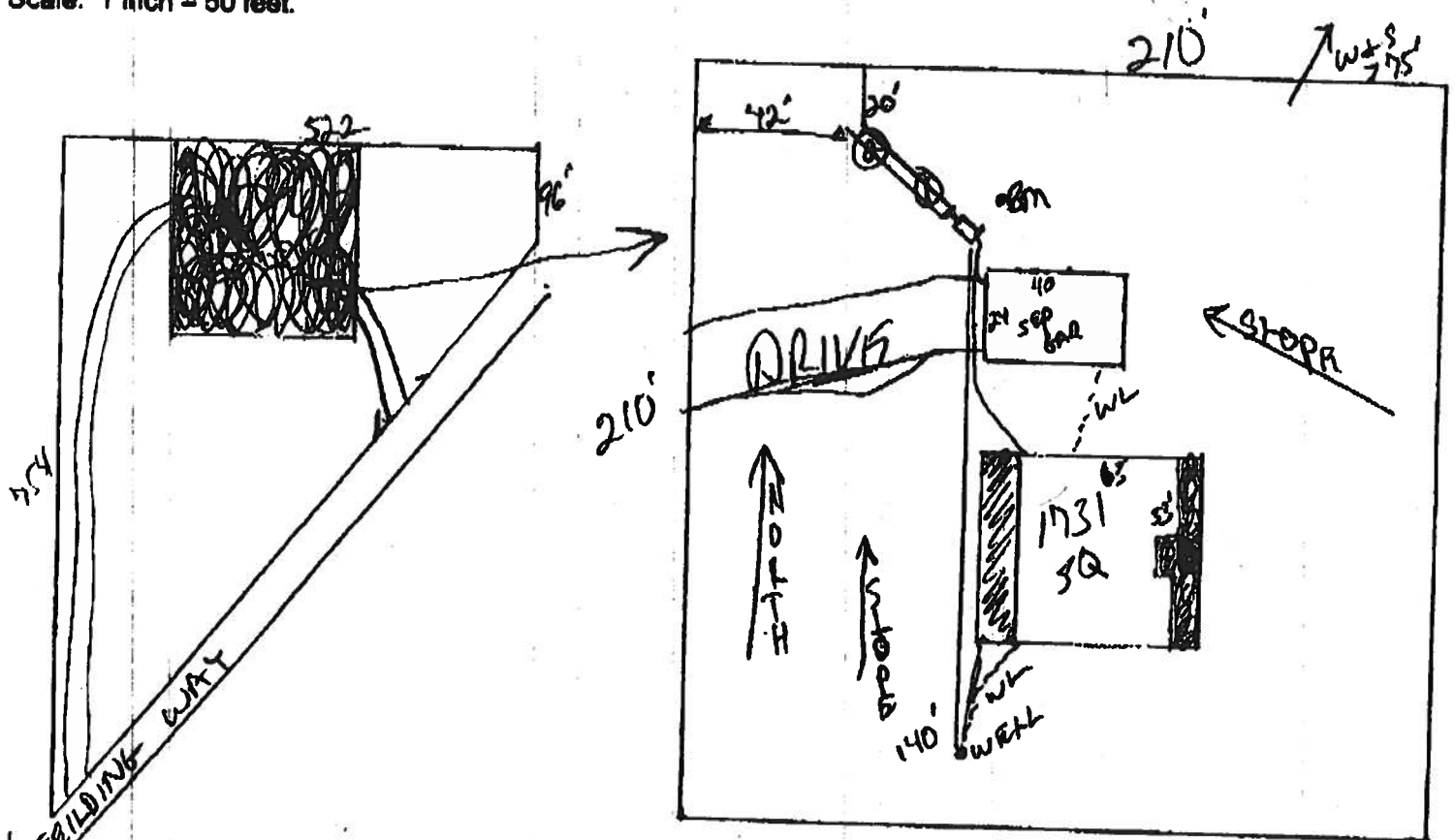
To left margin 2.10.06

STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1213N

PART II - SITEPLAN

Scale: 1 inch = 50 feet.



Notes:

1 of 5 ACRES

Site Plan submitted by:

Plan Approved

By

Not Approved

MASTER CONTRACTOR

Date

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

# Columbia County Property Appraiser

DB Last Updated: 9/16/2005

## 2005 Proposed Values

Parcel: 03-7S-16-04118-013

Tax Record

Property Card

Interactive GIS Map

Print

### Owner & Property Info

Search Result: 1 of 1

<b>Owner's Name</b>	HUETT GERALD L & DELORES S
<b>Site Address</b>	
<b>Mailing Address</b>	WILSON (JTWRS) 8672 LA LISTA CT ORLANDO, FL 32825
<b>Brief Legal</b>	COMM AT NW COR OF NE1/4 OF NE 1/4 OF SEC, RUN N 108 FT, E 598.58 FT, S 335 FT FOR POB,

<b>Use Desc. (code)</b>	NO AG ACRE (009900)
<b>Neighborhood</b>	3716.00
<b>Tax District</b>	3
<b>UD Codes</b>	MKTA02
<b>Market Area</b>	02
<b>Total Land Area</b>	4.900 ACRES

### Property & Assessment Values

<b>Mkt Land Value</b>	cnt: (1)	\$36,750.00
<b>Ag Land Value</b>	cnt: (0)	\$0.00
<b>Building Value</b>	cnt: (0)	\$0.00
<b>XFOB Value</b>	cnt: (0)	\$0.00
<b>Total Appraised Value</b>		\$36,750.00

<b>Just Value</b>	\$36,750.00
<b>Class Value</b>	\$0.00
<b>Assessed Value</b>	\$36,750.00
<b>Exempt Value</b>	\$0.00
<b>Total Taxable Value</b>	\$36,750.00

### Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
7/29/2005	1055/1051	WD	V	U	06	\$100.00
7/29/2005	1053/2951	WD	V	Q		\$58,000.00

### Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

### Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

### Land Breakdown

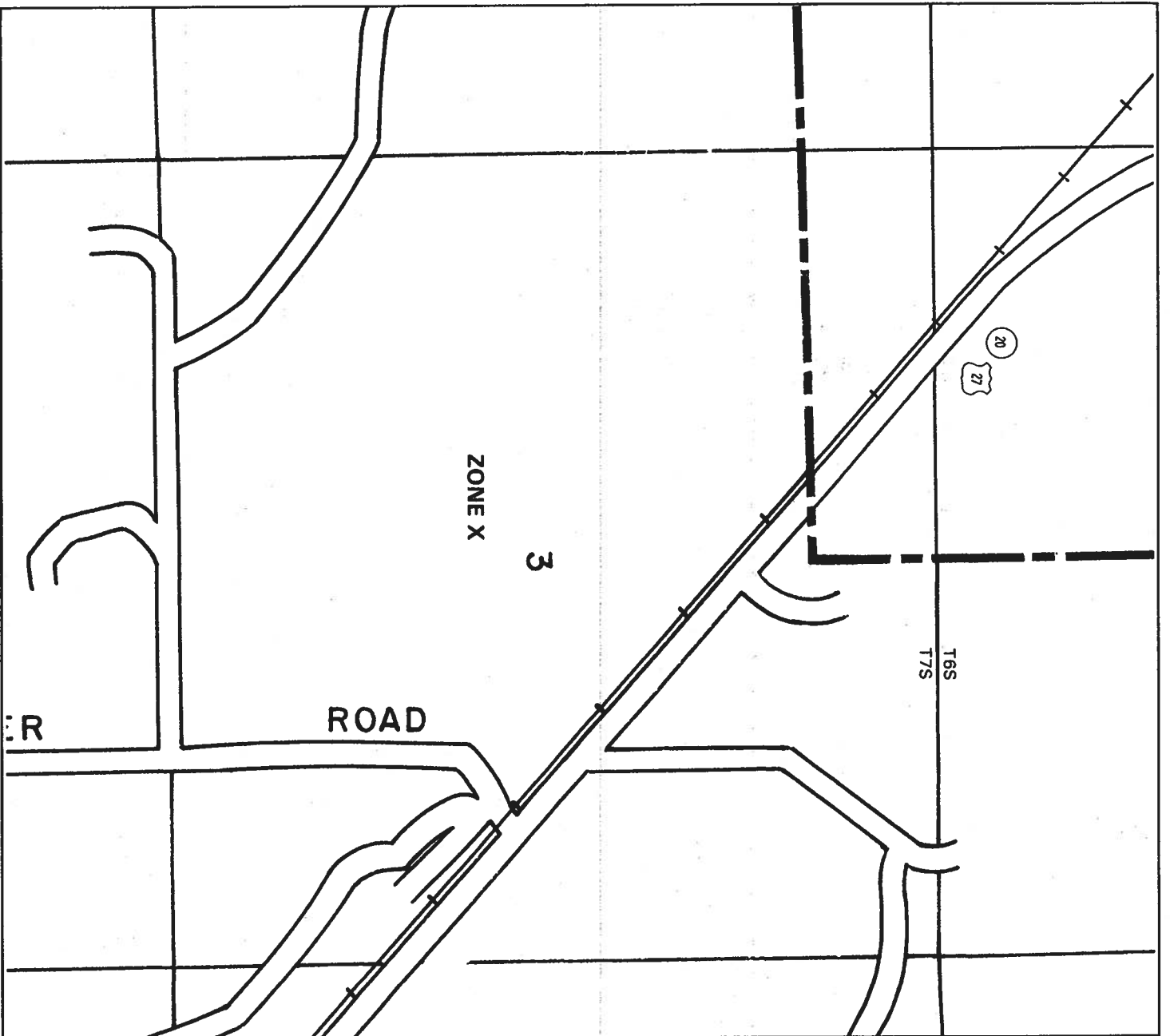
Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	4.900 AC	1.00/1.00/1.00/1.00	\$7,500.00	\$36,750.00

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

1 of 1





APPROXIMATE SCALE IN FEET



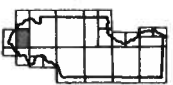
NATIONAL FLOOD INSURANCE PROGRAM

**FIRM**  
FLOOD INSURANCE RATE MAP

COLUMBIA  
COUNTY,  
FLORIDA  
(UNINCORPORATED AREAS)

PANEL 260 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER  
120070 0260 B  
EFFECTIVE DATE:  
JANUARY 6, 1988



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Further information about National Flood Insurance Program flood hazard maps is available at [www.fema.gov/nifsdc](http://www.fema.gov/nifsdc).

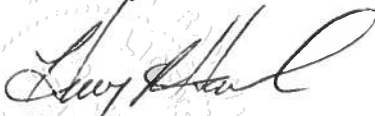
February 12, 2006

Columbia County Building and Zoning Department  
Lake City, Florida

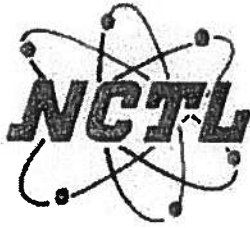
Dear Mr. Haltiwanger

Please accept this letter as a revision to the plans for Application No. 0602-12 and 0602-13. The plans were developed prior to the effective date for 2004 specification. However no change would be required since the wind loading would remain the same for both 2002 and 2004 specifications.

Sincerely

A handwritten signature in black ink, appearing to read "Huey Hawkins", written over a faint circular official seal.

Huey R. Hawkins, PE  
PE No. 33665



## **NATIONAL CERTIFIED TESTING LABORATORIES**

1464 GEMINI BOULEVARD • ORLANDO, FLORIDA 32837  
PHONE (407) 240-1356 • FAX (407) 240-8882

### **STRUCTURAL PERFORMANCE TEST REPORT**

**REPORT NO.:** NCTL-210-2065-4  
**TEST DATE:** 07-16-98  
**REPORT DATE:** 07-31-98  
**EXPIRATION DATE:** 07-31-02

**CLIENT:** Better Bilt Aluminum Products  
704 12<sup>th</sup> Avenue  
Smyrna, TN 37167

**TEST SPECIMEN:** Better Bilt Aluminum Product's Series "420" (Type "OXX") Aluminum Sliding Glass Door. (SGD-C35) (Insulated Glazed) (with and without sill riser)

**TEST SPECIFICATION:** AAMA/NWWDA/101/I.S. 2-97, "Voluntary Specifications for Aluminum Vinyl (PVC) and Wood Windows and Sliding Glass Doors."

### **TEST SPECIMEN DESCRIPTION**

**GENERAL:** The sample tested was a three panel type "OXX" aluminum sliding glass door measuring 15'1-3/4" wide by 6'10-1/8" high overall. The active panels measured 5'0-1/2" wide by 6'9-1/8" high; the fixed panel measured 5'0-7/8" wide by 6'9-1/8" high. Frame and panel members were not thermally broken. A plastic spacer/guide was used at each panel head/stile corner. The fixed panel was secured to the jamb with two (2) 3" long aluminum angle retainers each fastened to the jamb stile with two (2) (# 8 x 3/4") pan head screws. One claw-type door lock assembly was located at 40" from the bottom of each active panel lock stile each with the keeper fastened and secured to the fixed meeting stile and the right jamb stile at lock position with two (2) screws. One adjustable metal double roller assembly was used at each end of the active bottom rails. The frame was of double screw coped corner construction. Panel corners were of single screw at the bottom rail and double screw at the top rail coped corner construction. The interior vertical sill leg employed an extruded aluminum 1-1/8" high extension; an overall height of 2.031. One (1) aluminum panel retainer was fastened at 2" from the end of each active panel bottom rail. One (1) extruded aluminum female panel adapter was fastened to the fixed panel butt stile with five (5) (# 8 x 1/2") screws. One (1) extruded aluminum screen adapter was fastened to the butt stile using five (5) (# 8 x 1/2") screws.

**INSTALLATION:** The main frame was fastened to the wood test buck using forty-eight (48) (# 8 x 1-1/2"), FHS. (See fastener diagram)

**GLAZING:** All panels were channel glazed using sealed insulated glass. The overall insulating glass thickness was 5/8" consisting of two (2) pieces of 3/16" clear tempered glass and one (1) air space created by desiccant-filled aluminum spacer system. One (1) extruded aluminum female panel adapter was located at the fixed panel.

**WSTP:** Double strips of centerfin weatherstrip (0.270" high) were located at each jamb, stile, lock stile. A double strip of centerfin weatherstrip (0.180" high) was located at each interlock stile. A double strip of centerfin weatherstrip (0.250" high) was located at each panel top rail. A double strip of side fin weatherstrip (.430" high) was located at each panel bottom rail. An adhesive back polypile dust plug measuring 1-3/16" x 13/16" x 0.420" was located on the head and sill at each end of vertical stile exterior track

**WEEPS:** One weep notch measuring 1-1/2" x leg height was located at each end of each of the interior sill roller leg, exterior sill roller leg and the screen sill roller leg.

**INTERIOR & EXTERIOR SURFACE FINISH:** Non painted aluminum.

**SEALANT:** Frame and panel bottom rail corners were sealed with a small-joint sealant.

**SCREEN:** Two (2) insect screens, one center insect screen measuring 5'0-1/4" wide by 7'11" high; Both were of coped type corner construction. The screen employed fiberglass mesh cloth with a hollow vinyl spline. One roller assembly was located at each end of the bottom rails. One (1) claw-type lock assembly.

### **TEST RESULTS**

<b><u>PARAGRAPH NO.</u></b>	<b><u>TITLE OF TEST</u></b>	<b><u>MEASURED</u></b>	<b><u>ALLOWED</u></b>
** 2.2.19.5.	Operating Force		
	Center Active Panel		
	To Open	20# max.	30#
	In Motion	5 # max.	20#
	Right Active Panel		
	To Open	18 # max.	30 #
	In Motion	3 # max.	20#
** 2.1.2	Air Infiltration		
	1.57 psf (15 mph)	Pass	0.30 CFM/FT <sup>2</sup>
** 2.1.3	* Water Resistance - 5.0 GPH/FT <sup>2</sup>		
	WTP = 4.50 psf	No Entry	No Entry
2.1.4.2	Uniform Load Structural		
	45.0 psf exterior	0.251"	0.328"
	45.0 psf interior	0.267"	0.328"
**2.2.19.5.2	Deglazing		
	Center Active Panel		
	Top Rail (50#)	10.2% (0.051")	< 100%
	Bottom Rail (50#)	7.8% (0.039")	< 100%
	Left Stile (70#)	6.0% (0.030")	< 100%
	Right Stile (70#)	5.4% (0.027")	< 100%
	Right Active Panel		
	Top Rail (50#)	8.4% (0.042")	< 100%
	Bottom Rail (50#)	8.4% (0.042")	< 100%
	Left Stile (70#)	8.0% (0.040")	< 100%
	Right Stile (70#)	6.2% (0.031")	< 100%



**OPTIONAL PERFORMANCE**

\* Water Resistance - (5.0 GPH/FT<sup>2</sup>)  
WTP = 5.25 psf

No Entry

No Entry

Note: At this point in testing an additional sill riser was attached to the existing main sill's interior vertical leg.

The following results were obtained:

** 4.3	* Water Resistance - (5.0 GPH/FT <sup>2</sup> ) WTP - 6.00 psf	No Entry	No Entry
4.4.2	Uniform Structural Load		
	52.5 psf exterior	0.301"	0.328"
	52.5 psf interior	0.317"	0.328"

**TEST COMPLETED: 07-16-98**

\*Test performed with and without insect screen.

\*\* Reference parent test report no. NCTL-210-2065-1 for test results and qualifications.

In addition, Better Bilt Aluminum Products' Series "430" and "440" also receive an SGD-C35 rating being identical in panel construction and interior sill heights.

This test specimen meets the performance criteria level of SGD-C35 of the AAMA/NWDA/101/I.S. 2-97 specification.

Detailed drawings were available for laboratory records and compared to the test specimen at the time of this report. A copy of this report along with representative sections of the test specimen will be retained by NCTL for a period of four (4) years. The results obtained apply only to the specimen tested.

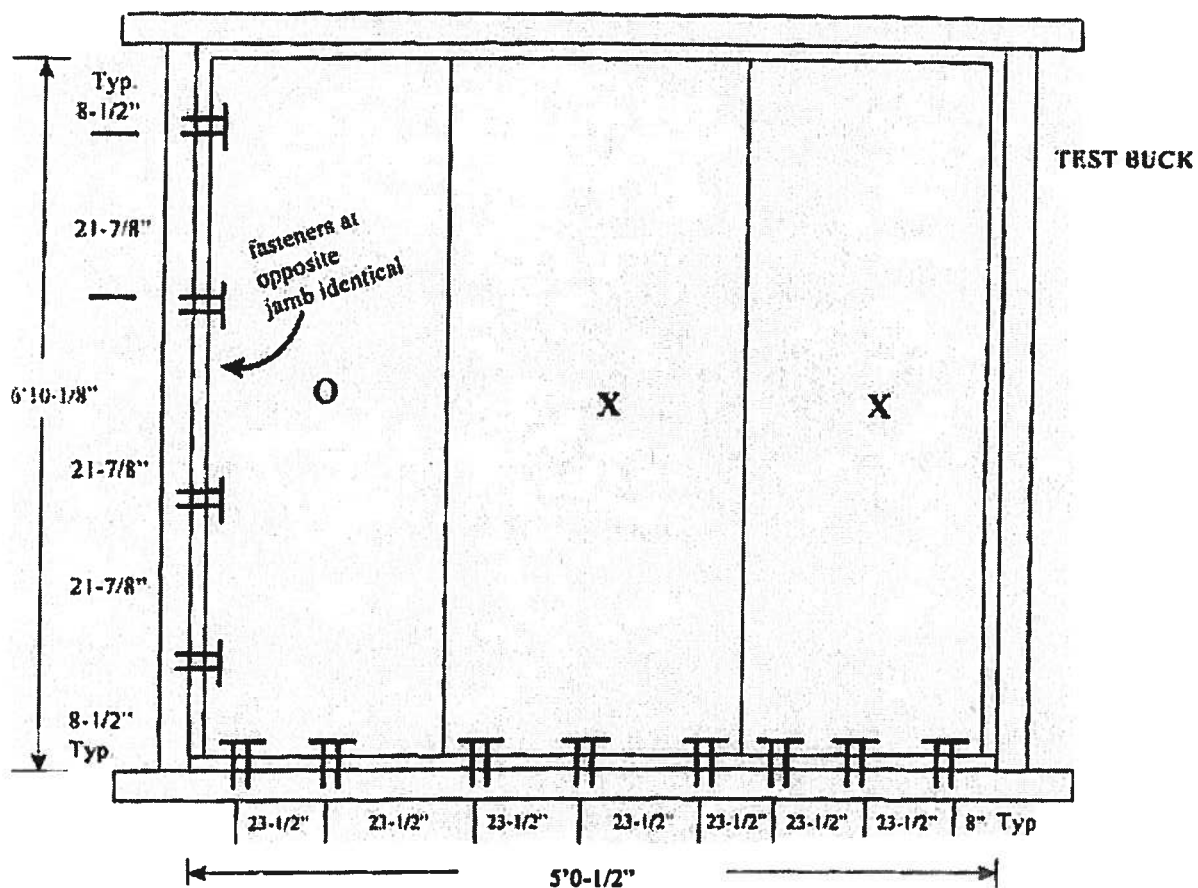
**NATIONAL CERTIFIED TESTING  
LABORATORIES, INC.**

**MICHAEL E. LANE**  
Division Manager

MEL/M

8/5/98

# FASTENER LOCATIONS



The test specimen was fin mounted to the test buck using forty-eight (48) (# 8 x 1-1/2") screws at locations shown.

⏏ - Denotes double row (2) at each location.

NOTE: Head fasteners identical to sill both jamb fastening identical

METRO DADE COUNTY REQUIRED

NATIONAL CERTIFIED TESTING LABORATORIES INC.

JOB NO.: NCTL-210-2065-3 & 4

COMPANY: BETTER BILT

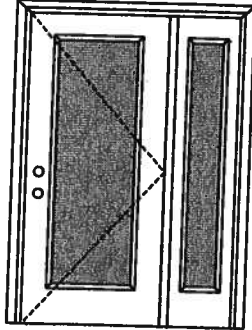
TEST DATE: 07-15-98

**XO or OX**  
Glazed Inswing Unit

COP-WL-JH4143-02

## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itssemko.com](http://www.itssemko.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Note:**  
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Single Door with 1 Sidelite  
Maximum unit size = 6'0" x 6'8"

#### Design Pressure

**+40.5/-40.5**

Limited water unless special threshold design is used.

#### Large Missile Impact Resistance

**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0003-02 or MAD-WL-MA0006-02 and MAD-WL-MA0041-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0003-02.

### APPROVED DOOR STYLES:

#### 1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

#### 1/2 GLASS:



105 Series\*



106, 160 Series\*



129 Series\*



200 Series\*



12 RA, 23 RA, 24 RA Series\*



167 Series\*



188 Series



304 Series

\*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

**Johnson™**  
**Entry Systems**

June 17, 2002  
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from  
**Masonite**  
Masonite International Corporation

## WOOD-EDGE STEEL DOORS

### APPROVED DOOR STYLES:

#### 3/4 GLASS:



404 Series



410 Series



450 Series

#### FULL GLASS:



109 Series



114, 120, 122  
Series



152 Series



149 Series



300 Series

### APPROVED SIDELITE STYLES:



680 Series



129 Series



200 Series



12R, 12L, 23R,  
23L, 24R, 24L  
Series



450 Series



152 Series



149 Series



109 Series



120, 122 Series



300 Series

### CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

*Kurt L. Balthazor*

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Johnson**  
EntrySystems

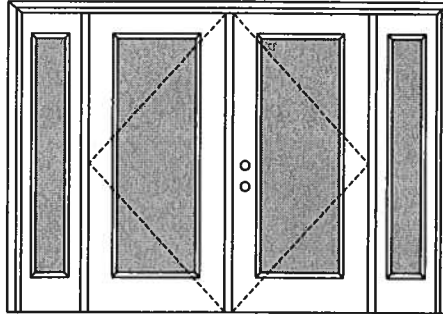
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## WOOD-EDGE STEEL DOORS

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**Note:**  
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door with 2 Sidelites  
Maximum unit size = 12'0" x 6'8"

**Design Pressure**  
**+40.5/-40.5**  
Limited water unless special threshold design is used.

**Large Missile Impact Resistance**  
**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

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#### 1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

#### 1/2 GLASS:



105 Series\*



106, 160 Series\*



129 Series\*



200 Series\*



12 R/L, 23 R/L, 24 R/L  
Series\*



107 Series\*



108 Series



304 Series

\*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

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## WOOD-EDGE STEEL DOORS

### APPROVED DOOR STYLES:

#### 3/4 GLASS:



404 Series



410 Series

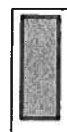


450 Series

#### FULL GLASS:



109 Series



114, 120, 122  
Series



152 Series



149 Series



300 Series

### APPROVED SIDELITE STYLES:



680 Series



129 Series



200 Series



12R, 12L, 23R,  
23L, 24R, 24L  
Series



450 Series



152 Series



149 Series



109 Series



120, 122 Series



300 Series

### CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Johnson**  
**EntrySystems**

June 17, 2002  
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from  
**Masonite**

Masonite International Corporation





**AAMA/NWDA 101/I.S.2-97  
TEST REPORT**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

**SERIES/MODEL: 650**

**TYPE: Aluminum Triple Single Hung Window**

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft <sup>2</sup>
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete description and data.

*Allen N. Reeves*  
7 JUNE 2002





Architectural Testing

**AAMA/NWWDA 101/I.S.2-97 TEST REPORT**

Rendered to

MI HOME PRODUCTS, INC.  
P.O. Box 370  
650 West Market Street  
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01  
Test Date: 05/13/02  
And: 05/16/02  
Report Date: 06/05/02  
Expiration Date: 05/16/06

**Project Summary:** Architectural Testing, Inc. (ATT) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

**Test Specimen Description:**

**Series/Model:** 650

**Type:** Aluminum Triple Single Hung Window

**Overall Size:** 9' 3-1/2" wide by 5' 11-11/16" high

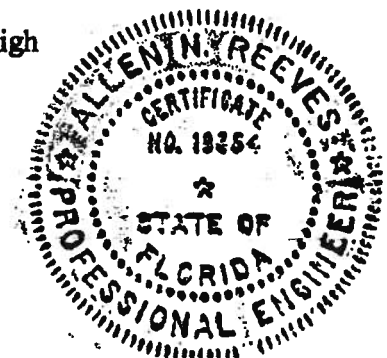
**Active Sash Size (3):** 3' 0-1/4" wide by 2' 10-3/4" high

**Fixed Daylight Opening Size (3):** 2' 8-1/4" wide by 2' 9-1/8" high

**Screen Size (3):** 2' 9-1/8" wide by 2' 11" high

**Finish:** All aluminum was painted white.

130 Derry Court  
York, PA 17402-9405  
phone: 717.764.7700  
fax: 717.764.4129  
www.archtest.com



*Allen N. Reeves*  
7 JUNE 2002

**Test Specimen Description: (Continued)**

**Glazing Details:** The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

**Weatherstripping:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



*Allen N. Reeves*  
7 JUN 2002

**Test Specimen Description: (Continued)****Hardware:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper	1	Midspace of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

**Drainage:** Sloped sill**Reinforcement:** No reinforcement was utilized.

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

**Test Results:**

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft <sup>2</sup>	0.3 cfm/ft <sup>2</sup> max.

**Note #1:** The tested specimen meets the performance levels specified in AAMA/NWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00)  
(with and without screen)  
WTP = 2.86 psf

No leakage

*Allen N. Reeves*  
7 JUNE 2002



**Test Results: (Continued)**


<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"	0.41" max
	@ 47.2 psf (negative)	0.67"	0.41" max

*\*Exceeds L/175 for deflection, but meets all other test requirements.*


Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
@ 53.0 psf (positive)	0.03"	0.29" max
@ 52.5 psf (negative)	0.02"	0.29" max

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC.

  
Mark A. Hess  
Technician

MAH:nfb  
01-41641.01

  
Allen N. Reeves, P.E.  
Director - Engineering Services  
7 June 2002



**Test Results: (Continued)**

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.15" 0.29"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" 0.01"	0.29" max. 0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs  Right sash, meeting rail Right sash, bottom rail Middle sash, meeting rail Middle sash, bottom rail Left sash, meeting rail Left sash, bottom rail  In remaining direction at 50 lbs  Right sash, right stile Right sash, left stile Middle sash, right stile Middle sash, left stile Left sash, right stile Left sash, left stile	0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25%  0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%  0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)  Type: A Grade: 10  Lock Manipulation Test  Test A1 through A5 Test A7  Lock Manipulation Test	No entry  No entry No entry  No entry	No entry  No entry No entry  No entry

*Allen N. Reeves*  
7 JUNE 2002



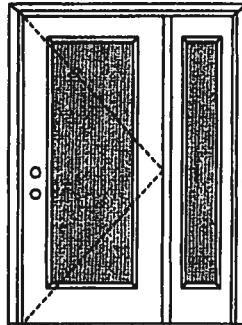


**XO or OX**  
Glazed Inswing Unit

COP-WL-JH4143-02

## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



Single Door with 1 Sidelite  
Maximum unit size = 6'0" x 6'8"

**Design Pressure**  
**+40.5/-40.5**

Limited water unless special threshold design is used.

**Large Missile Impact Resistance**

**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Note:**  
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0003-02 or MAD-WL-MA0006-02 and MAD-WL-MA0041-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0003-02.

### APPROVED DOOR STYLES:

#### 1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

#### 1/2 GLASS:



105 Series\*



106, 160 Series\*



129 Series\*



200 Series\*



12 R/L, 23 R/L, 24 R/L  
Series\*



107 Series\*



108 Series



304 Series

\*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

**Johnson**  
**EntrySystems**

June 17, 2002  
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## WOOD-EDGE STEEL DOORS

### APPROVED DOOR STYLES:

#### 3/4 GLASS:



404 Series

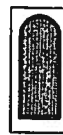


410 Series



450 Series

#### FULL GLASS:



109 Series



114, 120, 122  
Series



152 Series



149 Series



300 Series

### APPROVED SIDELITE STYLES:



680 Series



129 Series



200 Series



12R, 12L, 23R,  
23L, 24R, 24L  
Series



450 Series



152 Series



149 Series



109 Series



120, 122 Series



300 Series

### CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

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Frame constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Johnson**  
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June 17, 2002  
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# ELK



**PRESTIQUE®  
HIGH DEFINITION®**



**RAISED PROFILE®**

## Prestique Plus High Definition and Prestique Gallery Collection\*\*

Product size	13 1/4" x 39 1/4"	50-year limited warranty period:
Exposure	5 1/2"	5-7** years non-prorated coverage for
Pieces/Bundle	18	shingles and application labor with
Bundles/Square	4/98.5 sq.ft.	prorated coverage for remainder of
Squares/Pallet	11	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 80 mph, extended
		110 mph***

## Raised Profile

Product size	13 1/4" x 39 1/4"	30-year limited warranty period:
Exposure	6"	5-7** years non-prorated coverage for
Pieces/Bundle	22	shingles and application labor with
Bundles/Square	3/100 sq.ft.	prorated coverage for remainder of
Squares/Pallet	18	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 70 mph.

## Prestique I High Definition

Product size	13 1/4" x 39 1/4"	40-year limited warranty period:
Exposure	5 1/2"	5-7** years non-prorated coverage for
Pieces/Bundle	18	shingles and application labor with
Bundles/Square	4/98.5 sq.ft.	prorated coverage for remainder of
Squares/Pallet	14	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 80 mph, extended
		90 mph***

## HIP AND RIDGE SHINGLES

### Seal-A-Ridge® w/FLX™

Size: 12" x 12"  
Exposure: 6 1/2"  
Pieces/Bundle: 45  
Coverage: 4 Bundles =  
100 linear feet

### Vented RidgeCrest™ w/FLX™

Size: 13" x 13 1/4"  
Exposure: 9 1/4"  
Pieces/Box: 28  
Coverage: 5 boxes =  
100 linear feet

## Prestique High Definition

Product size	13 1/4" x 39 1/4"	30-year limited warranty period:
Exposure	5 1/2"	5-7** years non-prorated coverage for
Pieces/Bundle	22	shingles and application labor with
Bundles/Square	3/100 sq.ft.	prorated coverage for remainder of
Squares/Pallet	18	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 80 mph.

## Elk Starter Strip

52 Bundles/Pallet  
18 Pallets/Truck  
936 Bundles/Truck  
18 Pieces/Bundle  
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakeswood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood, Gallery Collection: Balsam Forest®, Weathered Sage®, Sierra Sunset®.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL: Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

\*See actual limited warranty for conditions and limitations.

\*\* Effective January 1, 2004, the seven year non-prorated Underlath Coverage Period applies only when a full Elk Ro of System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for each product. A full Elk Ro of System includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all eaves and gable edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, NY, NJ, CT, CO, UT, WY, & OR.

\*\*\*For a Limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 80 mph for Prestique I or Grand, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

## SPECIFICATIONS

**Scope:** Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

**PREPARATION OF ROOF DECK:** Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.52mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

**Materials:** Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes (4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)), use two piles of underlayment overlapped a minimum of 18". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

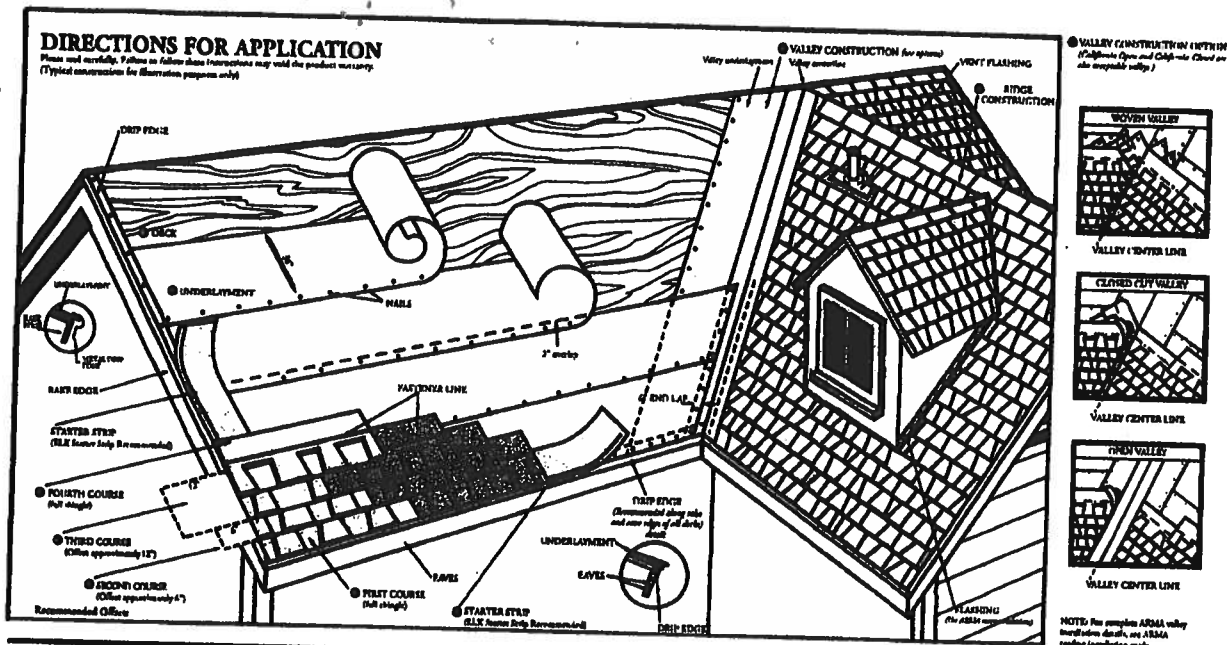
For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

**SOUTHEAST &  
ATLANTIC OFFICE:**  
800.945.5551

**CORPORATE HEADQUARTERS:**  
800.354.7732

**PLANT LOCATION:**  
800.945.5545

**ELK**  
The Premium Choice®  
www.elkcorp.com  
5500T 06/04



## DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

### 1 DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

### 2 UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt, Elk VeroShield® or self-adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 18". Begin by fastening a 18" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

### EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

### 3 STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

### 4 FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

### 5 SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 6".

### 6 THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

### 7 FOURTH COURSE

Start at the rake and continue with full shingles across roof.

### FIFTH AND SUCCEEDING COURSES

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

### 8 VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturers Association (ARMA) recommended procedures. For metal valleys, use 38" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

### 9 RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" 2" Ridge or Seal-A-Ridge® with formula FLX® or RidgeCrest® with FLX® (See ridge packages for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

### FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common head area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

**NAILS:** Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for re-roofs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

**STAPLES:** Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

### MANUARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Loosen fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

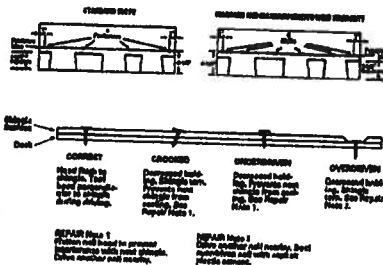
### LIMITED WIND WARRANTY

For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of manuard applications, 6 properly placed fasteners per shingle.

For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 80 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4" of an inch.

### HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along - and through - the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.

All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

**CAUTION TO WHOLESALE:** Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

**ELK**  
The Premium Choice  
www.elkcorp.com

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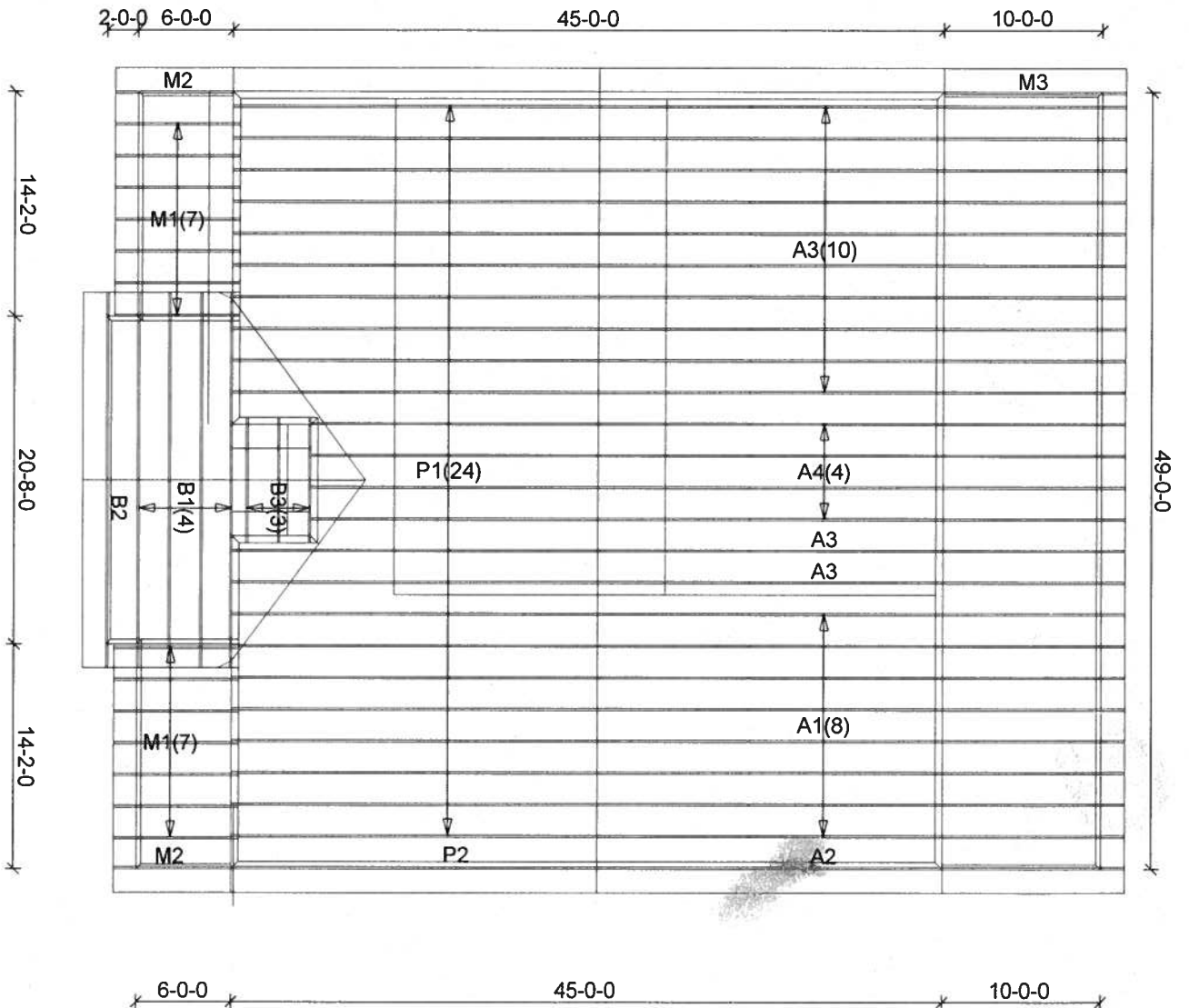
Mayo Truss Co. Inc.

362 NE CLYDE AVE.  
MAYO, FL 32066  
(386)294-3988  
(877)-558-6262

FORT WHITE DEVELOPERS

HUETT RESIDENCE

110 MPH ASCE WIND LOAD



Roof Loading  
TC Live: 20.00 psf  
TC Dead: 10.00 psf  
BC Live: 0.00 psf  
BC Dead: 10.00 psf  
TC Stress Inc: 25.00  
BC Stress Inc: 25.00  
Spacing: 2'-0" o.c.

Account: CONTRACTORS  
Job: FWDEV-HUETT  
Designer: M.MURRAY  
Checker: M.MURRAY  
Date: 02-01-06

Permit Number: \_\_\_\_\_ Lot Number: \_\_\_\_\_  
 Miscellaneous: \_\_\_\_\_ Address: \_\_\_\_\_

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: FWDEV-HUETT - HUETT RESIDENCE

### Standard Loading:

T.C. Live 20 psf  
 T.C. Dead 10 psf  
 B.C. Live 0 psf  
 B.C. Dead 10 psf  
 Total 40 psf

**ROBBINS  
ENGINEERING, INC.**

P.O. Box 280055  
 Tampa, FL 33682-0055  
 Phone: (813) 972-1135

## Engineering Index Sheet

Index Page 1 of 1

ANSI/ASCE 7-02  
 Wind Speed - 110 mph  
 Mean Roof Ht. - 15 ft  
 Exposure Category - B  
 Occupancy Factor - 1.00  
 MWFRS  
 Enclosed

Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06012415	01/26/2006		11

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-1995, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Software - Online Plus)

Notes: Refer to individual truss design drawings for special loading conditions.

### Date Mark

1	01/26/06	A1
5	01/26/06	B1
9	01/26/06	M2

### Date Mark

2	01/26/06	A2
6	01/26/06	B2
10	01/26/06	P1

### Date Mark

3	01/26/06	A3
7	01/26/06	B3
11	01/26/06	P2

### Date Mark

4	01/26/06	A4
8	01/26/06	M1

Truss Design Engineer: Philip J. O'Regan  
 License #: 58126  
 Address: P.O. Box 280055, Tampa, FL 33682



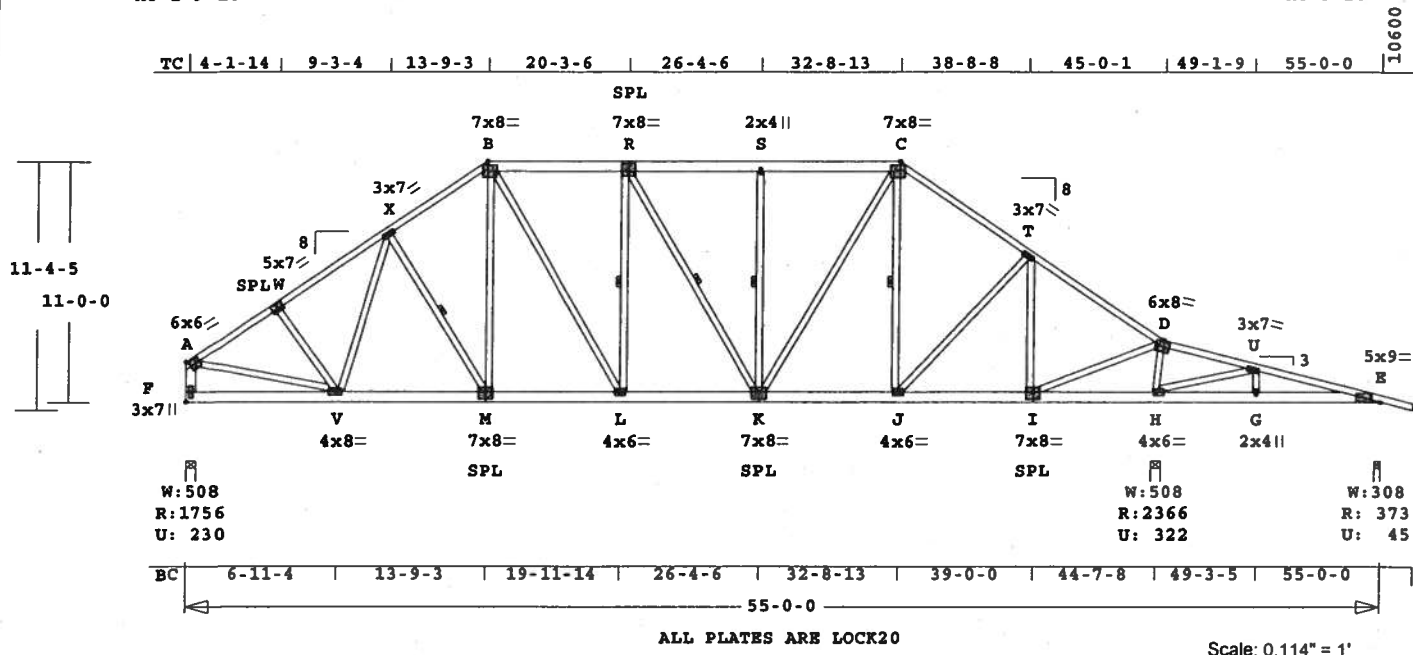


Job	Mark	Quar	Type	Span	Pl-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A1	8	SP	550000	3	0	1- 6- 0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE

HO 1-9-15

HO 3-14



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 543.9 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---  
TC 0.38 2x 4 SP-#2  
EX B -R 2x 6 SP-#2  
EX R -C 2x 6 SP-#2  
BC 0.27 2x 6 SP-#2  
WB 0.38 2x 4 SP-#2  
EX F -A 2x 6 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 13- 9- 3  
TC 2- 0- 0 13- 9- 3 32- 8- 13  
TC Cont. 32- 8- 13 55- 0- 0  
BC Cont. 0- 0- 0 55- 0- 0  
WB 1 rows CLB on X -M  
WB 1 rows CLB on L -R  
WB 1 rows CLB on R -K  
WB 1 rows CLB on K -S  
WB 1 rows CLB on J -C  
Attach CLB with (2)-10d nails  
at each web.

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
F	1756	231	5- 8	2- 1
			Hz =	-268
H	2366	322	5- 8	2-10
E	373	46	3- 8	1- 8
			Hz =	223

Membr	CSI	P	Lbs	Ax1-CSI-Bnd
A -W	0.23	2122	C	0.02 0.21
W -X	0.25	1959	C	0.05 0.20
X -B	0.26	1851	C	0.01 0.25
B -R	0.22	1806	C	0.01 0.21
R -S	0.21	1739	C	0.00 0.21
S -C	0.19	1739	C	0.01 0.18
C -T	0.30	1645	C	0.01 0.29
T -D	0.38	1407	C	0.01 0.37
D -U	0.36	547	T	0.08 0.28
U -E	0.28	301	C	0.00 0.28

Bottom Chords-----  
F -V 0.12 249 T 0.00 0.12  
V -M 0.24 1644 T 0.19 0.05  
M -L 0.24 1542 T 0.18 0.06  
L -K 0.27 1806 T 0.21 0.06  
K -J 0.22 1367 T 0.16 0.06  
J -I 0.20 1167 T 0.14 0.06  
I -H 0.16 744 C 0.00 0.16  
H -G 0.19 302 T 0.03 0.16  
G -E 0.17 302 T 0.03 0.14  
Webs-----  
F -A 0.10 1690 C WindLd  
A -V 0.33 1825 T  
W -V 0.08 253 C  
V -X 0.04 81 T  
X -M 0.05 190 C 1 Br  
M -B 0.06 349 T  
B -L 0.19 516 T  
L -R 0.11 313 C 1 Br  
R -K 0.06 132 C 1 Br  
K -S 0.14 393 C 1 Br  
K -C 0.18 726 T  
J -C 0.02 93 T 1 Br  
J -T 0.05 287 T  
I -T 0.36 690 C  
I -D 0.38 2059 T  
H -D 0.22 1962 C  
H -U 0.25 883 C  
G -U 0.02 189 T

LL Defl -0.04" in M -L L/999  
TL Defl -0.17" in L -K L/999  
Shear // Grain in D -U 0.23

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 6.0x 6.0-0.4-0.3 0.97  
W LOCK 5.0x 7.0-0.3 0.5 0.85  
X LOCK 3.0x 7.0 Ctr Ctr 0.64  
B LOCK 7.0x 8.0-0.5 Ctr 0.92  
R LOCK 7.0x 8.0 Ctr 0.8 0.76  
S LOCK 2.0x 4.0 Ctr Ctr 0.57  
C LOCK 7.0x 8.0 0.5 Ctr 0.92  
T LOCK 3.0x 7.0 Ctr Ctr 0.50  
D LOCK 6.0x 8.0-1.0 0.2 0.87  
U LOCK 3.0x 7.0 Ctr Ctr 0.48  
E LOCK 5.0x 9.0-9.0 2.2 0.88  
F LOCK 3.0x 7.0 Ctr Ctr 0.81  
V LOCK 4.0x 8.0-1.0 Ctr 0.87  
M LOCK 7.0x 8.0 Ctr-0.8 0.76  
L LOCK 4.0x 6.0 Ctr Ctr 0.53  
K LOCK 7.0x 8.0 Ctr Ctr 0.8 0.76  
J LOCK 4.0x 6.0 Ctr Ctr 0.48  
I LOCK 7.0x 8.0 Ctr-0.8 0.76

H LOCK 4.0x 6.0 Ctr Ctr 0.76  
G LOCK 2.0x 4.0 Ctr Ctr 0.57

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

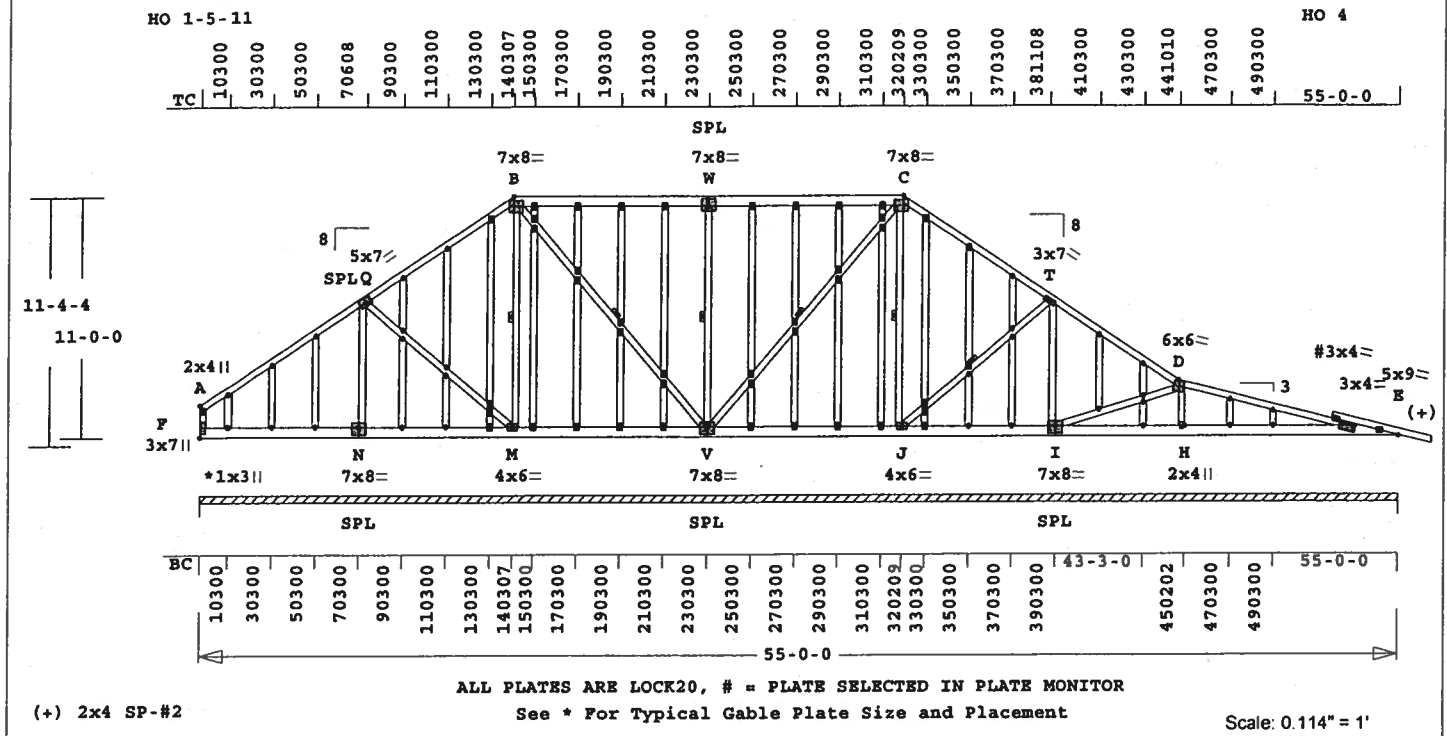
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Prevent truss rotation at all  
bearing locations.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 2122 Lbs

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A2	1	SP	550000	3	0	0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 746.7 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---

TC	0.65	2x 4	SP-#2
EX B -W	2x 6	SP-#2	
EX W -C	2x 6	SP-#2	
BC	0.30	2x 6	SP-#2
WB	0.29	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	55- 0- 0
BC Cont.	0- 0- 0	55- 0- 0
WB 1 rows CLB on M -B		
WB 1 rows CLB on B -V		
WB 1 rows CLB on V -W		
WB 1 rows CLB on V -C		
WB 1 rows CLB on J -C		
WB 1 rows CLB on J -T		

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 55- 0- 0		
	4400	582	Hz =	263

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -Q	0.65	89	T	0.02	0.63
Q -B	0.63	125	T	0.00	0.63
B -W	0.43	120	T	0.00	0.43
W -C	0.43	120	T	0.00	0.43
C -T	0.44	151	C	0.00	0.44
T -D	0.44	272	C	0.00	0.44
D -E	0.41	824	C	0.01	0.40
-----Bottom Chords-----					
F -N	0.15	0	T	0.00	0.15
N -M	0.15	0	T	0.00	0.15

Web	Size	Length	Area	Weight
F -A	0.01	178	C	WindLd
N -Q	0.25	565	C	
Q -M	0.09	177	T	
M -B	0.10	302	C	1 Br
B -V	0.06	105	C	1 Br
V -W	0.21	609	C	1 Br
W -C	0.13	221	C	1 Br
C -J	0.06	176	C	1 Br
J -T	0.04	161	C	1 Br
I -T	0.10	236	C	
I -D	0.29	608	C	
H -D	0.01	172	C	

LL Defl -0.03" in H -E2 L/999  
TL Defl -0.09" in H -E2 L/999  
Shear // Grain in A -Q 0.28

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga.	Gross Area
Plate - RHS	20 Ga. <td>Gross Area</td> <td></td>	Gross Area	
Jt Type	Plt Size	X	Y
A LOCK	2.0x 4.0	Ctr	Ctr
Q LOCK	5.0x 7.0	0.3	0.5
B LOCK	7.0x 8.0	0.5	0.92
W LOCK	7.0x 8.0	Ctr	0.8
C LOCK	7.0x 8.0	0.5	Ctr
T LOCK	3.0x 7.0	Ctr	Ctr
D LOCK	6.0x 6.0	Ctr	Ctr
E LOCK	5.0x 9.0	Ctr	0.8
F LOCK	3.0x 7.0	Ctr	Ctr
N LOCK	7.0x 8.0	Ctr	0.8
M LOCK	4.0x 6.0	Ctr	Ctr
V LOCK	7.0x 8.0	Ctr	0.8
J LOCK	4.0x 6.0	Ctr	Ctr
I LOCK	7.0x 8.0	Ctr	0.8
H LOCK	2.0x 4.0	Ctr	Ctr

36 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.

Design checked for 10 psf non-concurrent LL on BC.

Prevent truss rotation at all bearing locations.

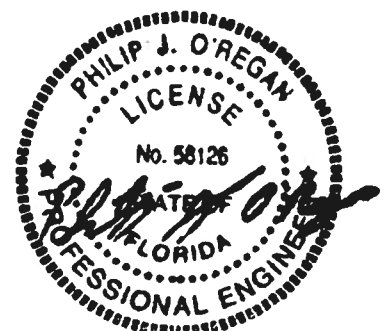
Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main Wind-Force Resistance System.

Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 824 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A1	8	SP	550000	3	0	1- 6- 0	T06012415
U# J#FWDEV-HUETT HUETT RESIDENCE								

Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A3	12	SP	550000	3	0	1- 6- 0	T06012415

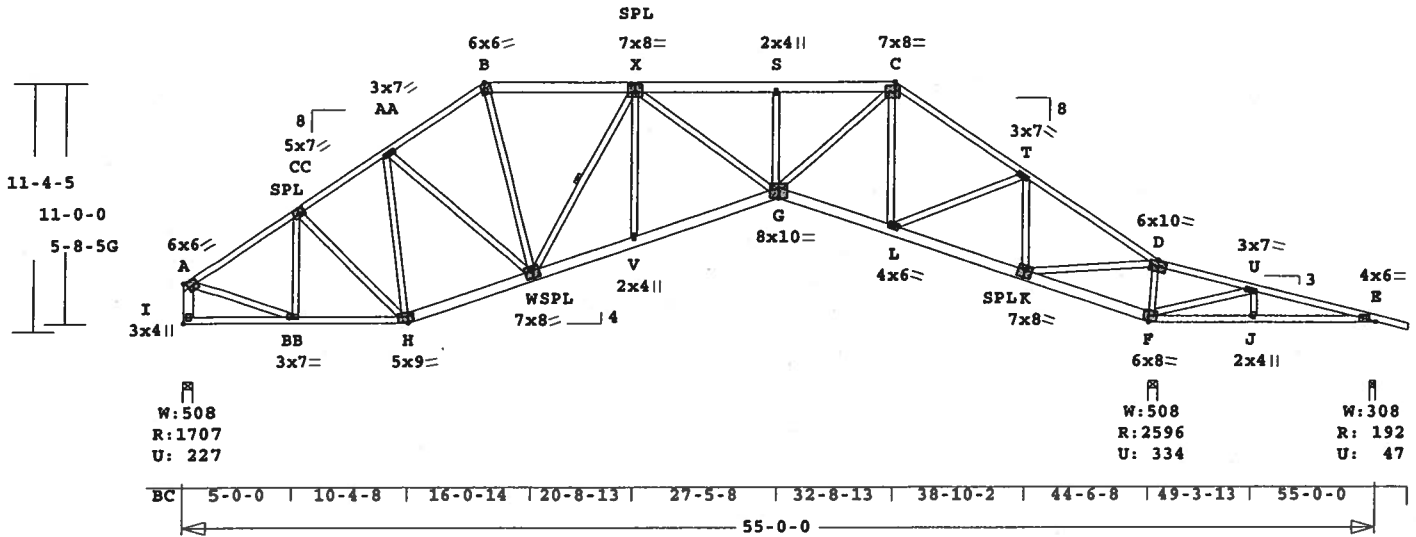
U# J#FWDEV-HUETT HUETT RESIDENCE

HO 1-9-15

HO 3-14

10600

TC 5-3-8 | 9-3-4 | 13-9-3 | 20-8-13 | 27-3-12 | 32-8-13 | 38-10-2 | 45-0-1 | 49-5-9 | 55-0-0



W: 508  
R: 1707  
U: 227

W: 508  
R: 2596  
U: 334

W: 308  
R: 192  
U: 47

ALL PLATES ARE LOCK20

Scale: 0.114" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 483.0 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---  
TC 0.66 2x 4 SP-#2  
EX B -X 2x 6 SP-#2  
EX X -C 2x 6 SP-#2  
BC 0.47 2x 6 SP-#2  
EX I -H 2x 4 SP-#2  
EX F -E 2x 4 SP-#2  
WB 0.50 2x 4 SP-#2  
EX I -A 2x 6 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 55- 0- 0  
BC Cont. 0- 0- 0 55- 0- 0  
WB 1 rows CLB on W -X  
Attach CLB with (2)-10d nails  
at each web.

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
I	1707	228	5- 8	2- 0
			Hx =	-271
F	2596	334	5- 8	2-14
E	192	48	3- 8	1- 8
			Hx =	224

Membr	CSI	P	Lbs	Ax1	CSI	Bnd
-----Top Chords-----						
A -CC	0.29	1811	C	0.04	0.25	
CC-AA	0.25	1906	C	0.02	0.23	
AA-B	0.28	2042	C	0.02	0.26	
B -X	0.25	1921	C	0.01	0.24	
X -S	0.27	3454	C	0.03	0.24	
S -C	0.14	3454	C	0.04	0.10	
C -T	0.44	2350	C	0.10	0.34	
T -D	0.46	1466	C	0.01	0.45	
D -U	0.66	1516	T	0.24	0.42	
U -E	0.46	247	T	0.04	0.42	
-----Bottom Chords-----						
I -BB	0.14	250	T	0.00	0.14	
BB-H	0.33	1519	T	0.25	0.08	
H -W	0.24	1611	T	0.19	0.05	

W -V	0.37	2684	T	0.32	0.05
V -G	0.47	2699	T	0.33	0.14
G -L	0.41	2041	T	0.25	0.16
L -K	0.29	1293	T	0.15	0.14
K -F	0.35	1604	C	0.00	0.35
F -J	0.40	227	C	0.00	0.40
J -E	0.29	227	C	0.00	0.29
-----Webs-----					
I -A	0.10	1660	C	WindLd	
A -BB	0.29	1603	T		
BB-CC	0.12	409	C		
CC-H	0.03	79	T		
AA-H	0.35	458	C		
AA-W	0.04	237	T		
B -W	0.15	839	T		
W -X	0.40	1335	C	1 Br	
V -X	0.03	199	T		
X -G	0.20	1102	T		
G -S	0.08	310	C		
G -C	0.36	1960	T		
L -C	0.16	320	C		
L -T	0.14	796	T		
K -T	0.31	1137	C		
K -D	0.50	2715	T		
F -D	0.19	1654	C		
F -U	0.39	1305	C		
J -U	0.04	292	T		

LL Defl -0.19" in G -L L/999  
TL Defl -0.39" in V -G L/999  
Shear // Grain in D -U 0.27  
Hz Disp LL DL TL  
Jt F 0.13" 0.13" 0.26"

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 6.0x 6.0-0.4-0.3 0.97  
CC LOCK 5.0x 7.0-0.3 0.5 0.85  
AA LOCK 3.0x 7.0 Ctr Ctr 0.50  
B LOCK 6.0x 6.0 1.1-3.7 0.75  
X LOCK 7.0x 8.0 Ctr 0.7 0.76  
S LOCK 2.0x 4.0 Ctr Ctr 0.57  
C LOCK 7.0x 8.0 0.5 Ctr 0.92  
T LOCK 3.0x 7.0 Ctr Ctr 0.50  
D LOCK 6.0x10.0-0.5 0.1 0.94  
U LOCK 3.0x 7.0 Ctr Ctr 0.47  
E LOCK 4.0x 6.0 Ctr Ctr 0.1 1.00  
I LOCK 3.0x 4.0 Ctr Ctr 0.81  
BB LOCK 3.0x 7.0 Ctr Ctr 0.58  
H LOCK 5.0x 9.0-1.5 3.2 0.75  
V LOCK 7.0x 8.0 0.3-0.8 0.74  
W LOCK 2.0x 4.0 Ctr Ctr 0.57  
G LOCK 8.0x10.0 Ctr-1.8 0.70  
L LOCK 4.0x 6.0 Ctr Ctr 0.51

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Prevent truss rotation at all  
bearing locations.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682

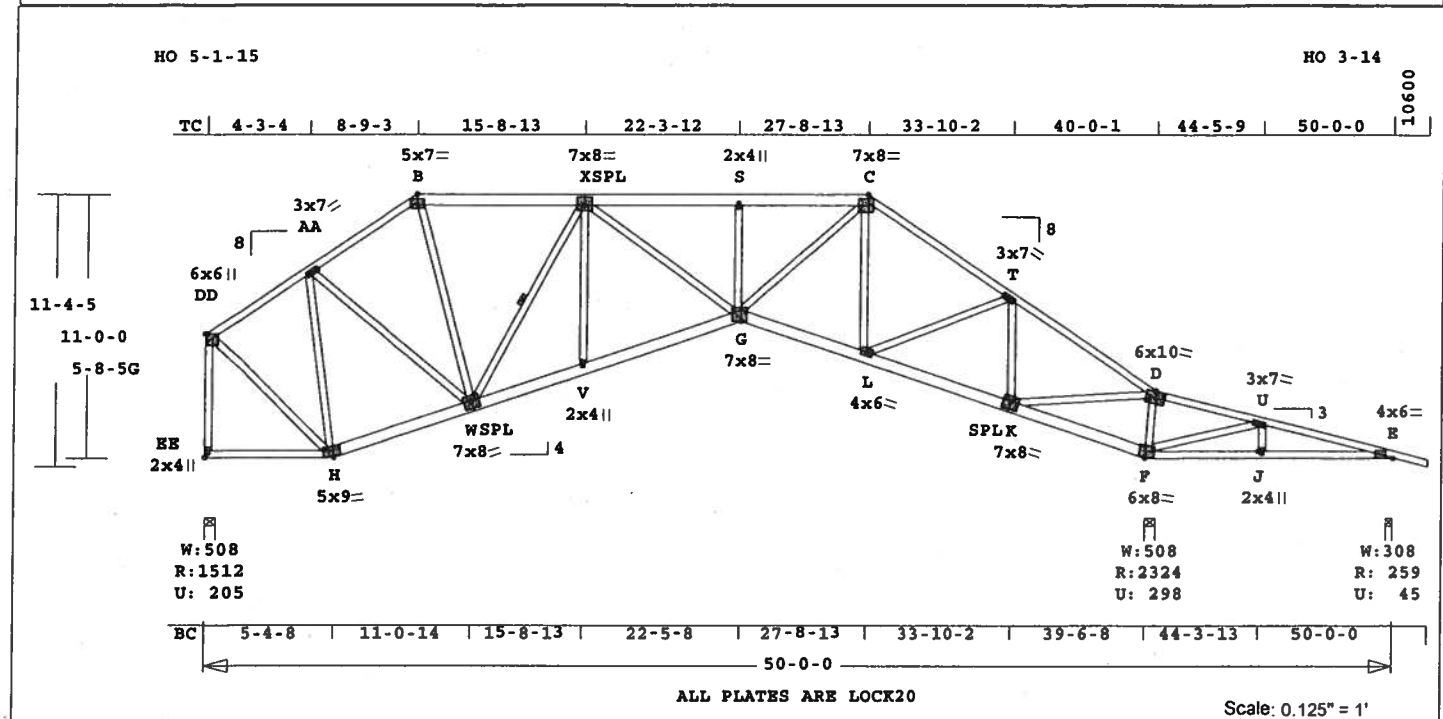


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A3	12	SP	550000	3	0	1- 6- 0	T06012415
U# J#FWDEV-HUETT HUETT RESIDENCE								

Max comp. force 3454 Lbs  
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A4	4	SP	500000	3	0	1- 6- 0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 449.0 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI	-Size-	----	Lumber----
TC	0.57	2x 4	SP-#2
EX B -X	2x 6	SP-#2	
EX X -C	2x 6	SP-#2	
BC	0.35	2x 6	SP-#2
EX EE-H	2x 4	SP-#2	
EX F -E	2x 4	SP-#2	
WB	0.70	2x 4	SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 50- 0- 0  
BC Cont. 0- 0- 0 50- 0- 0  
WB 1 rows CLB on W -X  
Attach CLB with (2)-10d nails  
at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
Lbs	Lbs	In-Sx	In-Sx	
EE	1512	206	5- 8	1-13
			Hx =	-305
F	2324	298	5- 8	2- 9
E	260	46	3- 8	1- 8
			Hx =	195

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----	Top	Chords			
DD-AA	0.17	983	C	0.00	0.17
AA-B	0.21	1441	C	0.01	0.20
B -X	0.24	1328	C	0.00	0.24
X -S	0.26	2875	C	0.02	0.24
S -C	0.11	2875	C	0.02	0.09
C -T	0.39	2038	C	0.08	0.31
T -D	0.42	1358	C	0.01	0.41
D -U	0.57	1216	T	0.19	0.38
U -E	0.38	127	T	0.00	0.38
-----	Bottom	Chords			
EE-H	0.17	241	T	0.00	0.17
H -W	0.15	761	T	0.09	0.06
W -V	0.28	2074	T	0.25	0.03

V -G	0.35	2081	T	0.25	0.10
G -L	0.33	1769	T	0.21	0.12
L -K	0.26	1197	T	0.14	0.12
K -F	0.31	1292	C	0.00	0.31
F -J	0.34	72	T	0.00	0.34
J -E	0.25	72	T	0.00	0.25
-----Webs-----					
EE-DD	0.46	1455	C	WindLd	
DD-H	0.21	1147	T		
AA-H	0.70	924	C		
AA-W	0.11	619	T		
B -W	0.09	503	T		
W -X	0.41	1357	C		1 Br
V -X	0.03	218	T		
X -G	0.20	1105	T		
G -S	0.09	321	C		
G -C	0.28	1545	T		
L -C	0.11	217	C		
L -T	0.11	609	T		
K -T	0.26	958	C		
K -D	0.43	2333	T		
F -D	0.17	1497	C		
F -U	0.38	1242	C		
J -U	0.04	280	T		

LL Defl	-0.14"	in G -L	L/999
TL Defl	-0.29"	in G -L	L/999
Shear //	Grain	in D -U	0.26
Hx Disp	LL	DL	TL
Jt F	0.11"	0.10"	0.21"

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

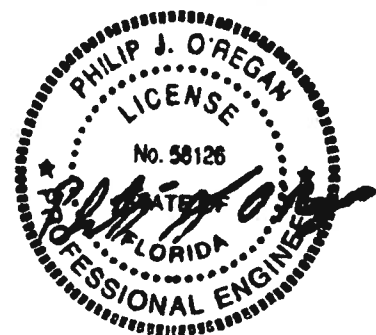
Plate - LOCK	20 Ga, Gross Area			
Plate - RHS	20 Ga, Gross Area			
Jt Type	Plt Size	X	Y	JSI
DD LOCK	6.0x 6.0	Ctr	0.68	0.68
AA LOCK	3.0x 7.0	Ctr	Ctr	0.49
B LOCK	5.0x 7.0	Ctr	-0.6	0.95
X LOCK	7.0x 8.0	Ctr	0.7	0.71
S LOCK	2.0x 4.0	Ctr	Ctr	0.54
C LOCK	7.0x 8.0	0.5	Ctr	0.87
T LOCK	3.0x 7.0	Ctr	Ctr	0.48
D LOCK	6.0x10.0	-0.5	0.1	0.88
U LOCK	3.0x 7.0	Ctr	Ctr	0.46
E LOCK	4.0x 6.0	Ctr	0.1	1.00
EE LOCK	2.0x 4.0	Ctr	Ctr	0.68
H LOCK	5.0x 9.0	-1.0	3.2	0.78
W LOCK	7.0x 8.0	0.3	-0.8	0.70
V LOCK	2.0x 4.0	Ctr	Ctr	0.54
G LOCK	7.0x 8.0	-1.0	-2.3	0.76
L LOCK	4.0x 6.0	Ctr	Ctr	0.49
K LOCK	7.0x 8.0	-0.3	-0.8	0.73
F LOCK	6.0x 8.0	1.1	3.6	0.77
J LOCK	2.0x 4.0	Ctr	Ctr	0.54

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Provide connection to bearing  
for 305 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.  
Prevent truss rotation at all  
bearing locations.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 2875 Lbs  
Quality Control Factor 1.25

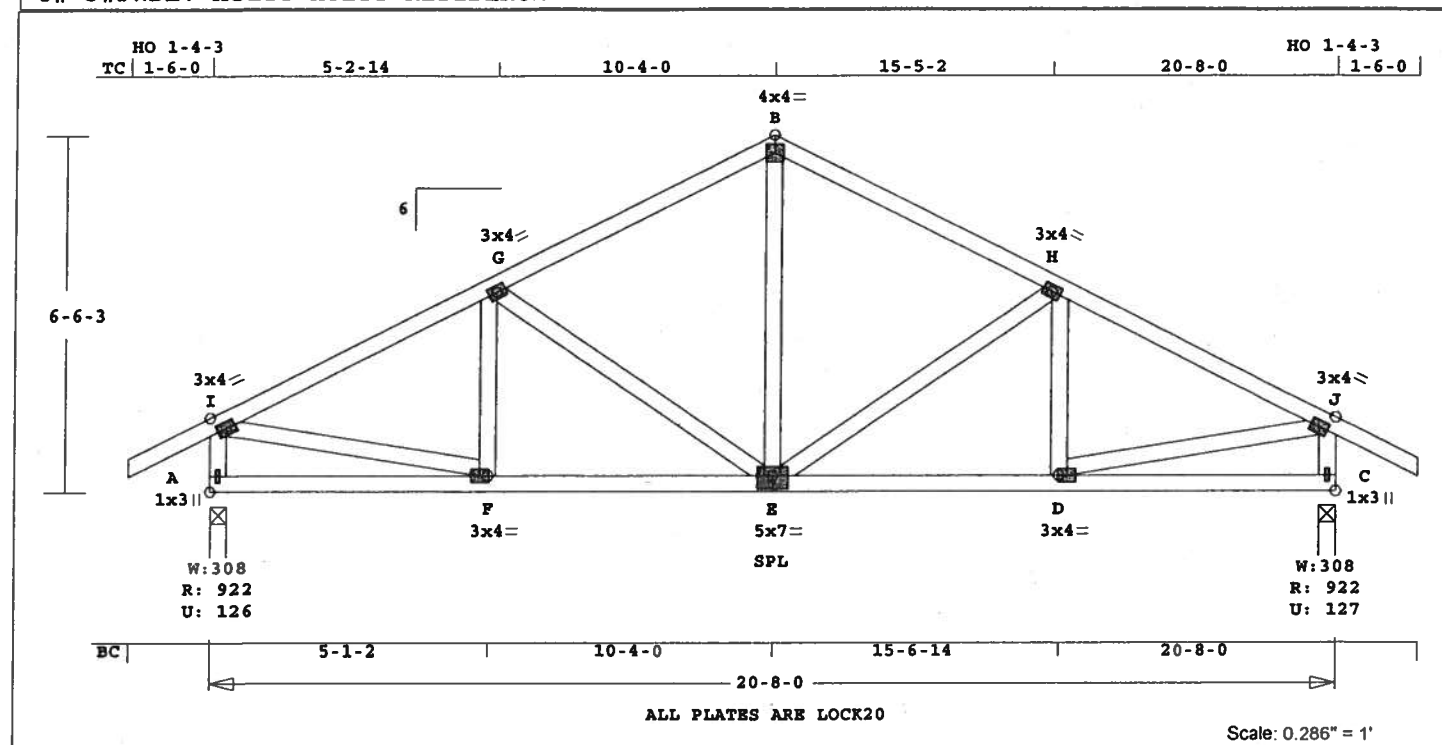
Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	B1	4	TR	200800	6	1- 6- 0	1- 6- 0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 157.1 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ----Lumber----

TC	0.28	2x 4	SP-#2
BC	0.24	2x 4	SP-#2
WB	0.17	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 0
BC Cont.	0- 0- 0	20- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	923	127	3- 8	1- 8
			Hz =	-140
C	923	127	3- 8	1- 8
			Hz =	119

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
I -G	0.28	1000	C	0.00	0.28
G -B	0.28	779	C	0.00	0.28
B -H	0.28	779	C	0.00	0.28
H -J	0.28	1000	C	0.00	0.28
-----Bottom Chords-----					
A -F	0.15	126	T	0.00	0.15
F -E	0.24	907	T	0.09	0.15
E -D	0.24	907	T	0.09	0.15

D	-C	0.15	110	T	0.00	0.15
-----Webs-----						
A	-I	0.07	782	C	WindLd	
I	-F	0.17	930	T		
F	-G	0.01	93	C		
G	-E	0.12	262	C		
E	-B	0.07	421	T		
D	-H	0.12	262	C		
H	-J	0.01	93	C		
J	-C	0.17	930	T		
C	-J	0.07	782	C	WindLd	

LL Defl -0.02" in A -F L/999  
TL Defl -0.05" in E -D L/999  
Shear // Grain in I -G 0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	- LOCK	20	Ga,	Gross	Area
Plate	- RHS	20	Ga,	Gross	Area
Jt	Type	Plt	Size	X	Y
I	LOCK	3.0x	4.0	Ctr	Ctr
G	LOCK	3.0x	4.0	Ctr	Ctr
B	LOCK	4.0x	4.0	Ctr	Ctr
H	LOCK	3.0x	4.0	Ctr	Ctr
J	LOCK	3.0x	4.0	Ctr	Ctr
A	LOCK	1.0x	3.0	Ctr	Ctr
F	LOCK	3.0x	4.0	Ctr	Ctr
E	LOCK	5.0x	7.0	Ctr	-0.5
D	LOCK	3.0x	4.0	Ctr	Ctr
C	LOCK	1.0x	3.0	Ctr	Ctr

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.

Analysis Conforms To:  
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main  
Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 1000 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



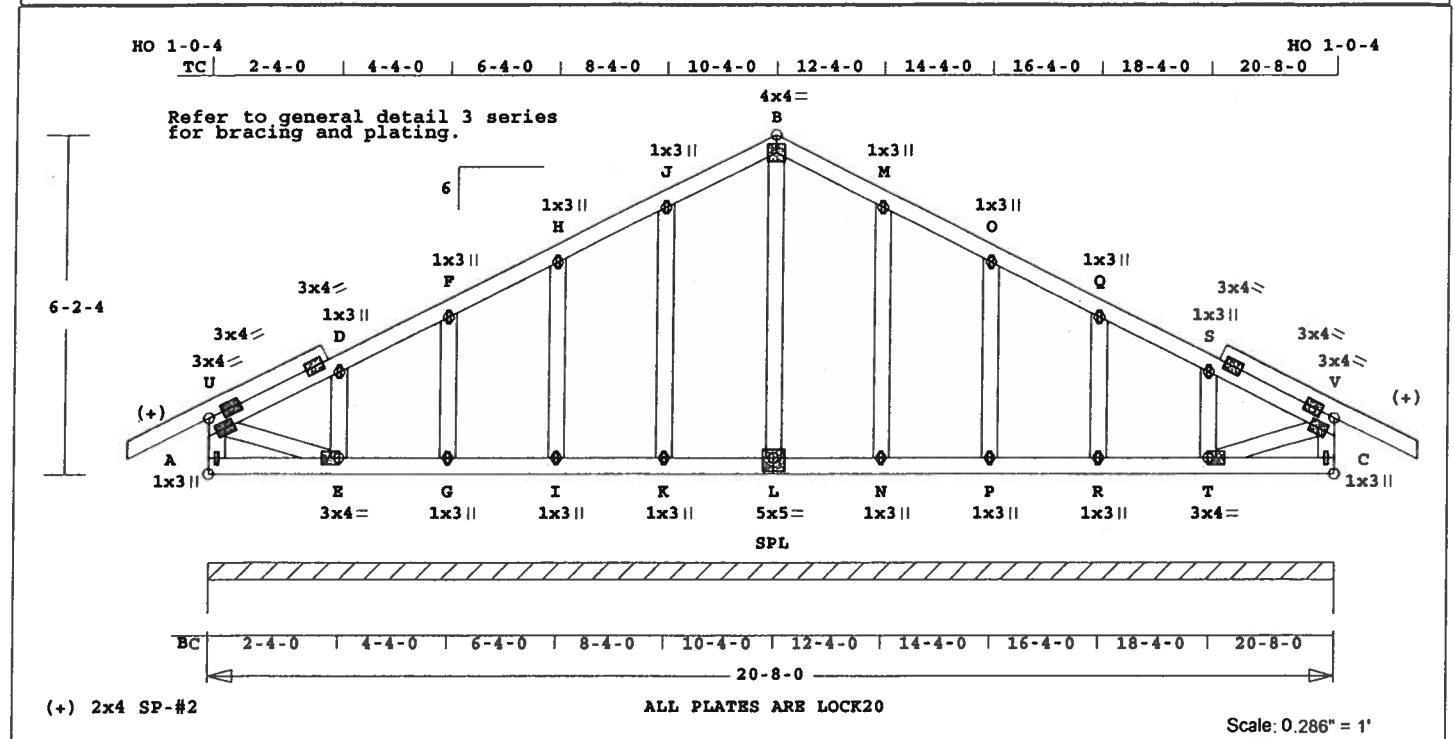
REVIEWED BY:

Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	B2	1	TR	200800	6	0	0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 166.1 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI	Size	Lumber
TC	0.04	2x 4 SP-#2
BC	0.02	2x 4 SP-#2
WB	0.01	2x 4 SP-#2
GW	0.04	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 0
BC Cont.	0- 0- 0	20- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd
Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 20- 8- 0	
1653	220	Hz =	126

Membr	CSI	P	Lbs	Ax1-CSI-Bnd
<b>Top Chords</b>				
U -D	0.04	61	C	0.00 0.04
D -F	0.04	39	C	0.00 0.04
F -H	0.03	28	C	0.00 0.03
H -J	0.03	46	T	0.00 0.03
J -B	0.03	79	T	0.00 0.03
B -M	0.03	79	T	0.00 0.03
M -O	0.03	46	T	0.00 0.03
O -Q	0.03	28	C	0.00 0.03
Q -S	0.04	39	C	0.00 0.04
S -V	0.04	61	C	0.00 0.04
<b>Bottom Chords</b>				
A -E	0.02	0	T	0.00 0.02
E -G	0.02	0	T	0.00 0.02
G -I	0.02	0	T	0.00 0.02
I -K	0.02	0	T	0.00 0.02
K -L	0.02	0	T	0.00 0.02
L -N	0.02	0	T	0.00 0.02
N -P	0.02	0	T	0.00 0.02

P	-R	0.02	0	T	0.00	0.02
R	-T	0.02	0	T	0.00 <td>0.02</td>	0.02
T	-C	0.02	0	T	0.00 <td>0.02</td>	0.02

-----Webs-----

A -U	0.00	76	C	WindLd
U -E	0.01	82	T	
T -V	0.01	79	T	
C -V	0.00	76	C	WindLd

-----Gable Webs-----

E -D	0.01	141	C
G -F	0.01	115	C
I -H	0.02	120	C
K -J	0.03	124	C
L -B	0.04	95	C
N -M	0.03	124	C
P -O	0.02	120	C
R -Q	0.01	115	C
T -S	0.01	141	C

LL Defl 0.00" in A -E L/999  
TL Defl 0.00" in T -C L/999  
Shear // Grain in U -D 0.09

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga	Gross Area
Plate - RHS	20 Ga <td>Gross Area</td> <td></td>	Gross Area	
Jt Type	Plt Size	X	Y JSI
U LOCK	3.0x 4.0	Ctr	Ctr 0.69
D LOCK	1.0x 3.0	Ctr	Ctr 0.75
F LOCK	1.0x 3.0	Ctr	Ctr 0.75
H LOCK	1.0x 3.0	Ctr	Ctr 0.75
J LOCK	1.0x 3.0	Ctr	Ctr 0.75
B LOCK	4.0x 4.0	Ctr	Ctr 0.65
M LOCK	1.0x 3.0	Ctr	Ctr 0.75
O LOCK	1.0x 3.0	Ctr	Ctr 0.75
Q LOCK	1.0x 3.0	Ctr	Ctr 0.75
S LOCK	1.0x 3.0	Ctr	Ctr 0.75
V LOCK	3.0x 4.0	Ctr	Ctr 0.69
A LOCK	1.0x 3.0	Ctr	Ctr 0.81
E LOCK	3.0x 4.0	Ctr	Ctr 0.52
G LOCK	1.0x 3.0	Ctr	Ctr 0.81
I LOCK	1.0x 3.0	Ctr	Ctr 0.81
K LOCK	1.0x 3.0	Ctr	Ctr 0.81
L LOCK	5.0x 5.0	Ctr	Ctr 0.58
N LOCK	1.0x 3.0	Ctr	Ctr 0.81
P LOCK	1.0x 3.0	Ctr	Ctr 0.81
R LOCK	1.0x 3.0	Ctr	Ctr 0.81
T LOCK	3.0x 4.0	Ctr	Ctr 0.52
C LOCK	1.0x 3.0	Ctr	Ctr 0.81

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

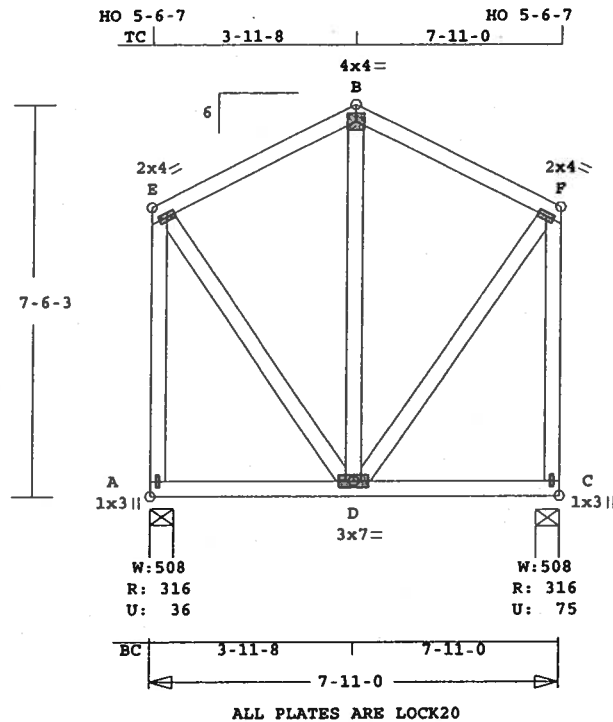
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 141 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	B3	3	TR	71100	6	0	0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 87.7 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI	Size	Lumber
TC	0.14	2x 4 SP-#2
BC	0.10	2x 4 SP-#2
WB	0.10	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7-11- 0
BC Cont.	0- 0- 0	7-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	317	37	5- 8	1- 8
			Hz =	-261
C	317	75	5- 8	1- 8
			Hz =	237

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
E	-B	0.14	99	C	0.00 0.14
B	-F	0.14	99	C	0.00 0.14

-----Bottom Chords-----

A	-D	0.10	193	T	0.00 0.10
D	-C	0.10	168	T	0.00 0.10

-----Webs-----

A	-E	0.10	283	C	WindLd
E	-D	0.04	160	T	
D	-B	0.10	168	C	
D	-F	0.04	160	T	
C	-F	0.10	283	C	WindLd

LL Defl -0.01" in A -D L/999  
TL Defl -0.01" in A -D L/999  
Shear // Grain in E -B 0.14

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga	Gross Area
Jt Type	Plt Size	X	Y
E	LOCK	2.0x 4.0	Ctr Ctr 0.76
B	LOCK	4.0x 4.0	Ctr Ctr 0.47
F	LOCK	2.0x 4.0	Ctr Ctr 0.76
A	LOCK	1.0x 3.0	Ctr Ctr 0.75
D	LOCK	3.0x 7.0	Ctr Ctr 0.33
C	LOCK	1.0x 3.0	Ctr Ctr 0.75

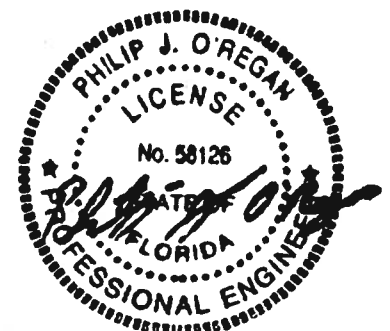
REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

#### NOTES:

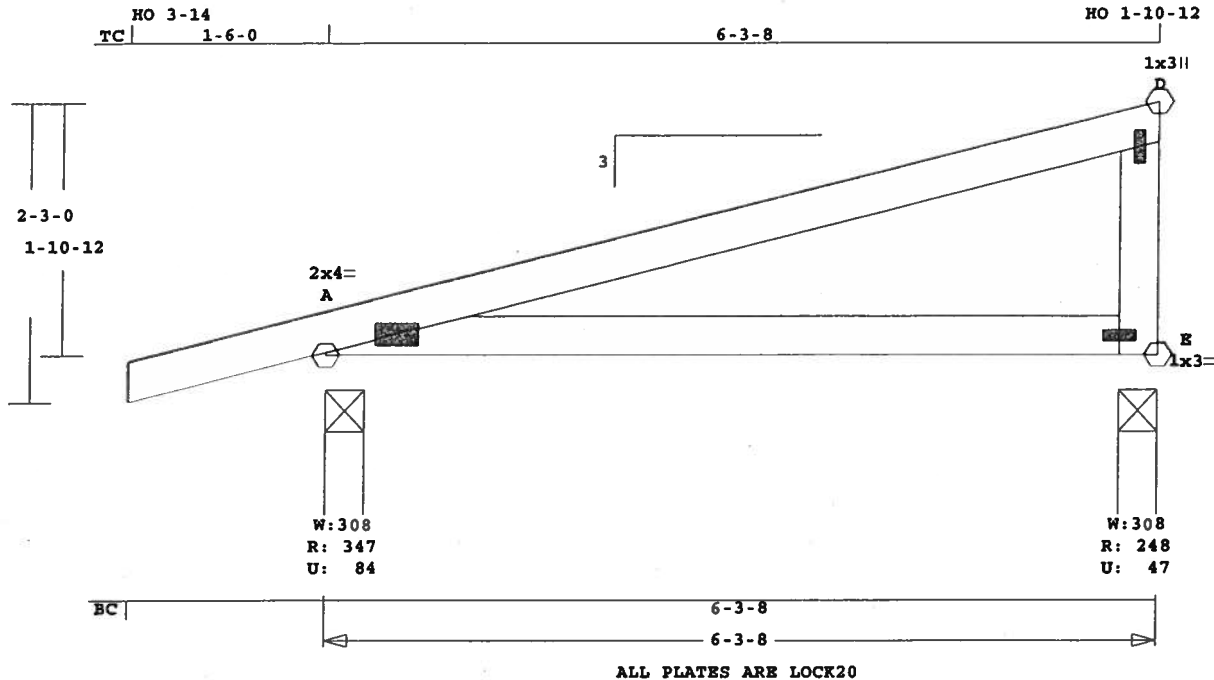
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 283 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	M1	14	MONO.DD	60308	3	1- 6- 0	0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 29.7 LBS

A -E 0.24 182 T 0.03 0.21

E -D 0.36 153 C 0.00 0.36

LL Defl -0.02" in A -E L/999

TL Defl -0.06" in A -E L/999

Shear // Grain in A -D 0.24

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORT: NER 691

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.83

D LOCK 1.0x 3.0 Ctr Ctr 0.75

E LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main

Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load : 5.0 psf

BC Dead Load : 5.0 psf

Max comp. force 179 Lbs

Quality Control Factor 1.25

Online Plus -- Version 18.5.027

RUN DATE: 26-JAN-06

CSI -Size- ----Lumber----

TC 0.31 2x 4 SP-#2

BC 0.24 2x 4 SP-#2

WB 0.36 2x 4 SP-#2

Brace truss as follows:

O.C. From To

TC Cont. 0- 0- 0 6- 3- 8

BC Cont. 0- 0- 0 6- 3- 8

Loading Live Dead (psf)

TC 20.0 10.0

BC 0.0 10.0

Total 20.0 20.0 40.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd

Lbs Lbs In-Sx In-Sx

A 348 85 3- 8 1- 8

Hz = 39

E 249 47 3- 8 1- 8

Hz = 69

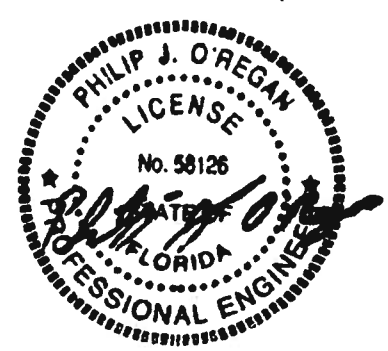
Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

A -D 0.31 179 C 0.00 0.31

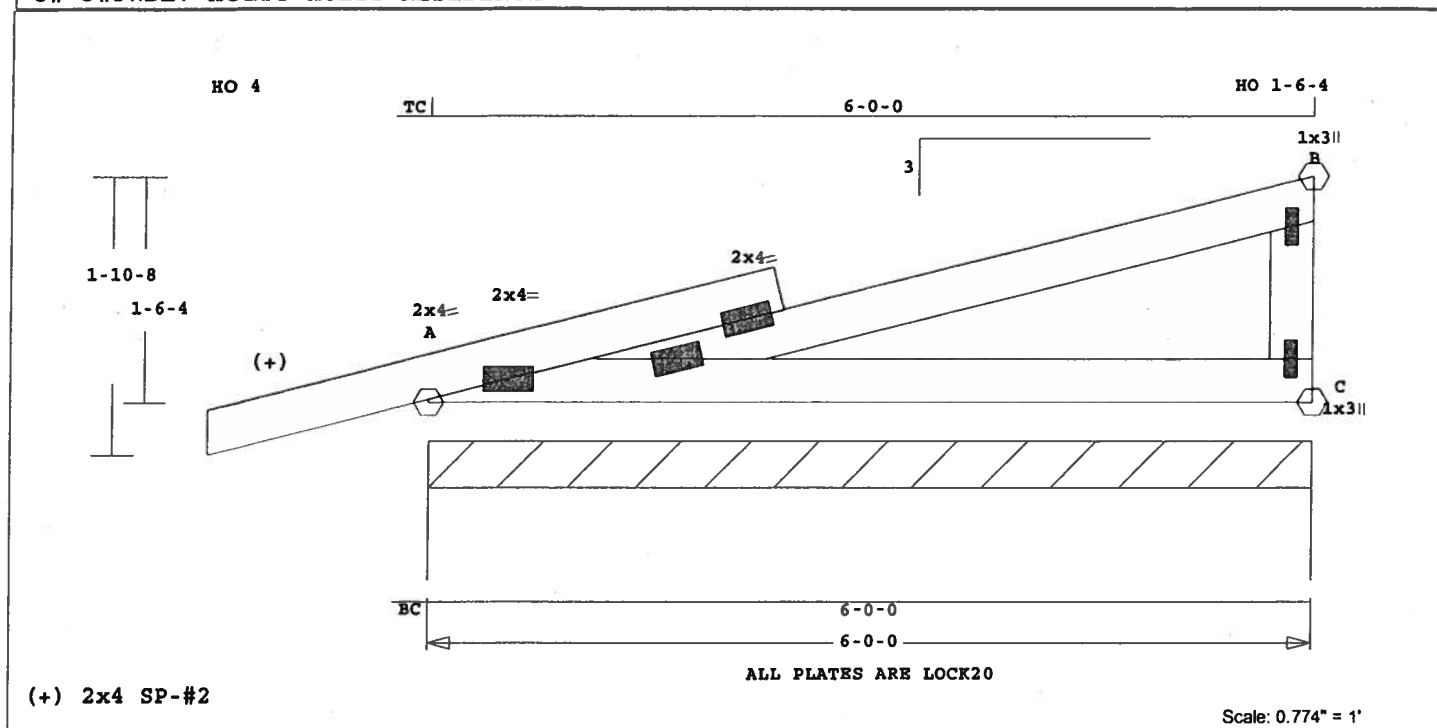
-----Bottom Chords-----

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	M2	2	MONO.DD	60000	3	0	0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 30.3 LBS

LL Defl -0.01" in A -C L/999

TL Defl -0.02" in A -C L/999

Shear // Grain in A -B 0.18

Truss is designed as a Main Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load : 5.0 psf

BC Dead Load : 5.0 psf

Max comp. force 149 Lbs

Quality Control Factor 1.25

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI	-Size-	----	Lumber----
TC	0.19	2x 4	SP-#2
BC	0.14	2x 4	SP-#2
WB	0.22	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 0- 0
BC Cont.	0- 0- 0	6- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing		24.0"	
Lumber Duration Factor		1.25	
Plate Duration Factor		1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate -	LOCK 20 Ga,	Gross Area
Plate -	RHS 20 Ga,	Gross Area
Jt Type	Plt Size	X Y JSI
A	LOCK 2.0x 4.0	Ctr Ctr 0.82
B	LOCK 1.0x 3.0	Ctr Ctr 0.75
C	LOCK 1.0x 3.0	Ctr Ctr 0.75

REVIEWED BY:

Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

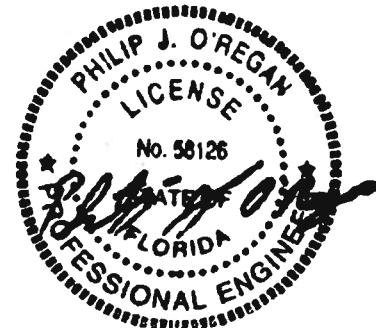
WARNING Do Not Cut overframe  
member between outside of  
truss and first tie-plate  
to inside of heel plate.

Design checked for 10 psf non-  
concurrent LL on BC.

Refer to Gen Det 3 series for  
web bracing and plating.

Wind Loads - ANSI / ASCE 7-02

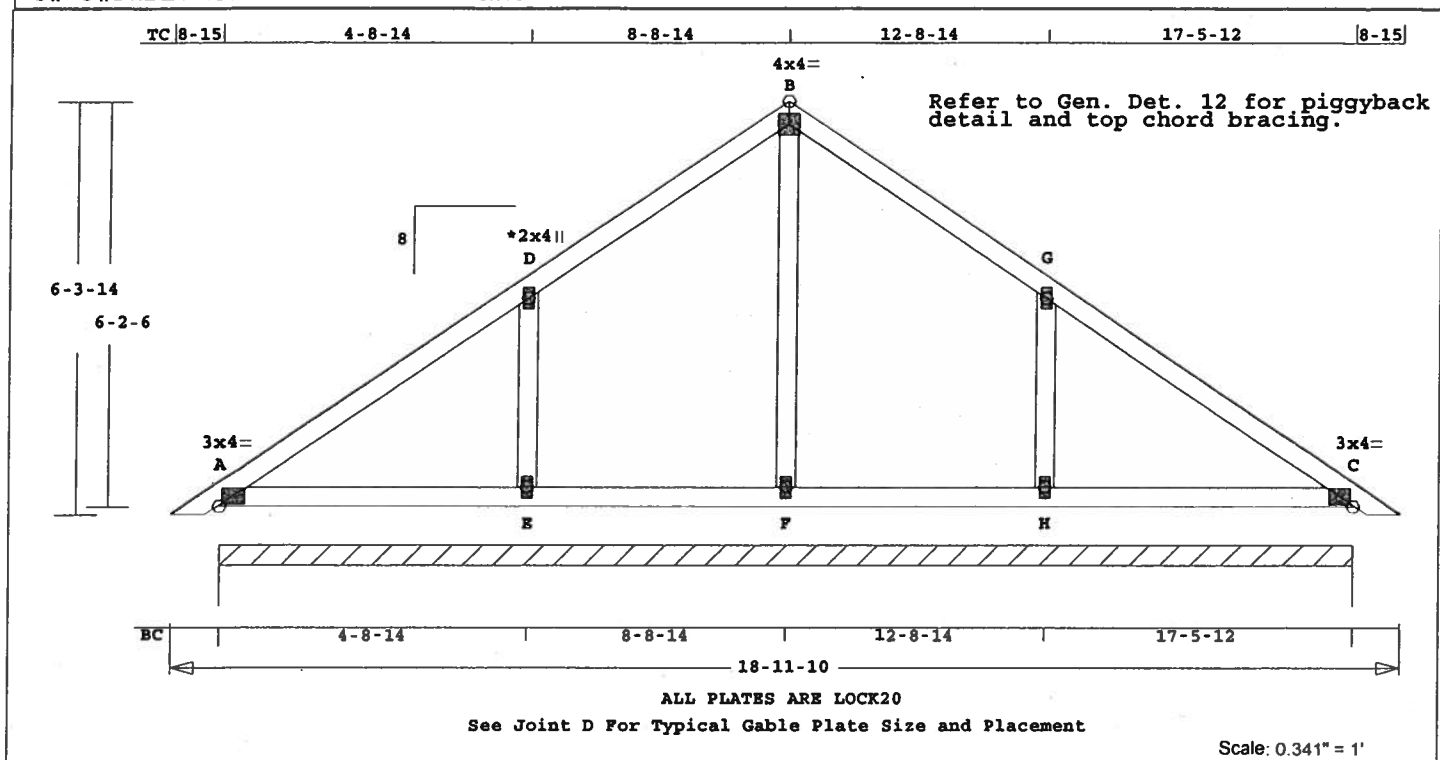
Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 1/26/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	P1	24	TR	181110	8	8-15	8-15	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 97.8 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

-----Gable Webs-----  
E -D 0.04 287 C  
F -B 0.02 53 T  
H -G 0.04 287 C

concurrent LL on BC.  
Refer to Gen Det 3 series for web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 287 Lbs  
Quality Control Factor 1.25

CSI -Size- ----Lumber----

TC	0.20	2x 4	SP-#2
BC	0.12	2x 4	SP-#2
GW	0.04	2x 4	SP-#2

LL Defl -0.01" in H -C L/999  
TL Defl -0.02" in H -C L/999  
Shear // Grain in A -D 0.16

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18-11-10
BC Cont.	0- 0- 0	18-11-10

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.69  
D LOCK 2.0x 4.0 Ctr Ctr 0.00  
B LOCK 4.0x 4.0 Ctr Ctr 0.58  
G LOCK 2.0x 4.0 Ctr Ctr 0.00  
C LOCK 3.0x 4.0 Ctr Ctr 0.69  
E LOCK 2.0x 4.0 Ctr Ctr 0.00  
F LOCK 2.0x 4.0 Ctr Ctr 0.00  
H LOCK 2.0x 4.0 Ctr Ctr 0.00

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 17- 5-12		
	1494	203	Hz =	122

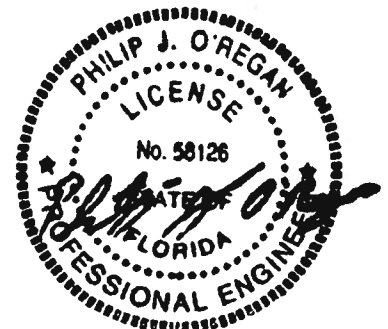
Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -D	0.20		160 C	0.00	0.20
D -B	0.20		181 C	0.00	0.20
B -G	0.20		181 C	0.00	0.20
G -C	0.20		160 C	0.00	0.20
-----Bottom Chords-----					
A -E	0.12		1 T	0.00	0.12
E -F	0.12		0 T	0.00	0.12
F -H	0.12		0 T	0.00	0.12
H -C	0.12		1 T	0.00	0.12

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

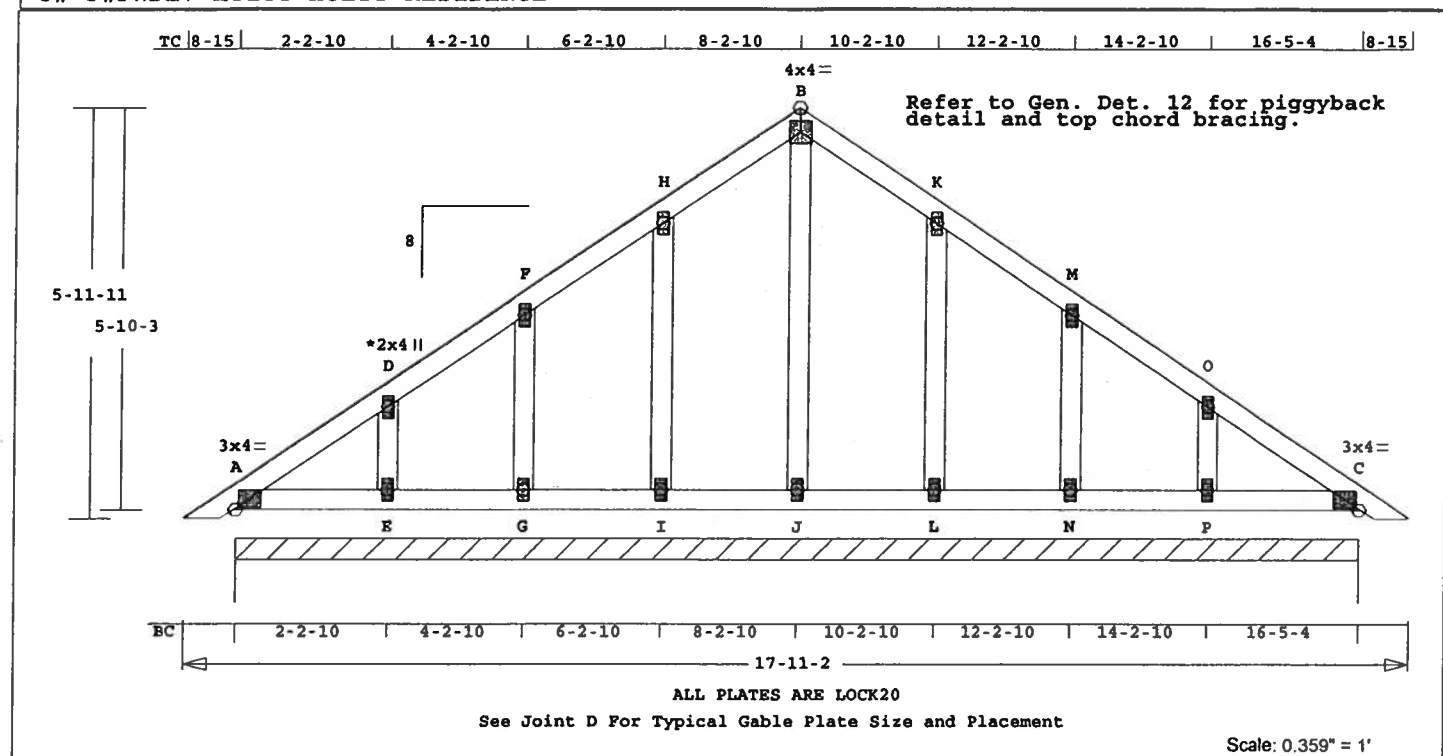
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job FWDEV-HUETT	Mark P2	Quan 1	Type TR	Span 171102	P1-H1 8	Left OH 8-15	Right OH 8-15	Engineering T06012415
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U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 111.4 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---  
TC 0.04 2x 4 SP-#2  
BC 0.02 2x 4 SP-#2  
GW 0.02 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	17-11- 2
BC Cont.	0- 0- 0	17-11- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 16- 5- 4		
	1410	192	Hz =	114

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -D	0.04		75 C	0.00	0.04
D -F	0.04		48 C	0.00	0.04
F -H	0.03		41 C	0.00	0.03
H -B	0.03		74 T	0.00	0.03
B -K	0.03		74 T	0.00	0.03
K -M	0.03		41 C	0.00	0.03
M -O	0.04		48 C	0.00	0.04
O -C	0.04		75 C	0.00	0.04

-----Bottom Chords-----					
A -E	0.02		2 T	0.00	0.02
E -G	0.02		0 T	0.00	0.02
G -I	0.02		0 T	0.00	0.02
I -J	0.02		0 T	0.00	0.02

J	-L	0.02	0 T	0.00	0.02
L	-N	0.02	0 T	0.00	0.02
N	-P	0.02	0 T	0.00	0.02
P	-C	0.02	2 T	0.00	0.02

-----Gable Webs-----		
E -D	0.01	127 C
G -F	0.01	118 C
I -H	0.02	124 C
J -B	0.02	66 C
L -K	0.02	124 C
N -M	0.01	118 C
P -O	0.01	127 C

LL Defl 0.00" in A -E L/999  
TL Defl 0.00" in A -E L/999  
Shear // Grain in A -D 0.07

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	- LOCK	20 Ga,	Gross Area
Plate - RHS	20 Ga, <td>Gross Area</td> <td></td>	Gross Area	
Jt Type	Plt Size	X	Y JSI
A LOCK	3.0x 4.0	Ctr	Ctr 0.67
D LOCK	2.0x 4.0	Ctr	Ctr 0.00
F LOCK	2.0x 4.0	Ctr	Ctr 0.00
H LOCK	2.0x 4.0	Ctr	Ctr 0.00
B LOCK	4.0x 4.0	Ctr	Ctr 0.57
K LOCK	2.0x 4.0	Ctr	Ctr 0.00
M LOCK	2.0x 4.0	Ctr	Ctr 0.00
O LOCK	2.0x 4.0	Ctr	Ctr 0.00
C LOCK	3.0x 4.0	Ctr	Ctr 0.67
E LOCK	2.0x 4.0	Ctr	Ctr 0.00
G LOCK	2.0x 4.0	Ctr	Ctr 0.00
I LOCK	2.0x 4.0	Ctr	Ctr 0.00
J LOCK	2.0x 4.0	Ctr	Ctr 0.00
L LOCK	2.0x 4.0	Ctr	Ctr 0.00
N LOCK	2.0x 4.0	Ctr	Ctr 0.00
P LOCK	2.0x 4.0	Ctr	Ctr 0.00

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.

Refer to Gen Det 3 series for  
web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph

Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00

Building Type: Enclosed  
Zone location: Exterior

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 127 Lbs

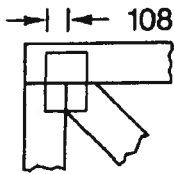
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



# ROBBINS ENG. GENERAL NOTES & SYMBOLS

## PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

## PLATE SIZE

6 x 8

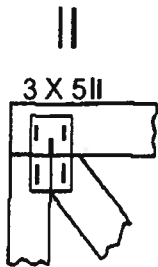
The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.



## LATERAL BRACING

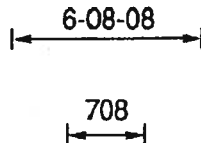
Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

## PLATE ORIENTATION



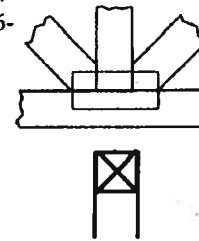
Shown next to plate size, indicates direction of slots in connector plate.

## DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

## BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. These designs were prepared in accordance with "National Design Specifications for Wood Construction" (AF & PA), "National Design Standard for Metal Plate Connected Wood Truss Construction" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by the Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THESE DESIGNS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE TRUSS DESIGN DRAWINGS & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS PLACEMENT DIAGRAM.



Corporate Headquarters

6904 Parke East Blvd  
Tampa, FL 33610-4115  
813-972-1135 Fax: 813-971-6117



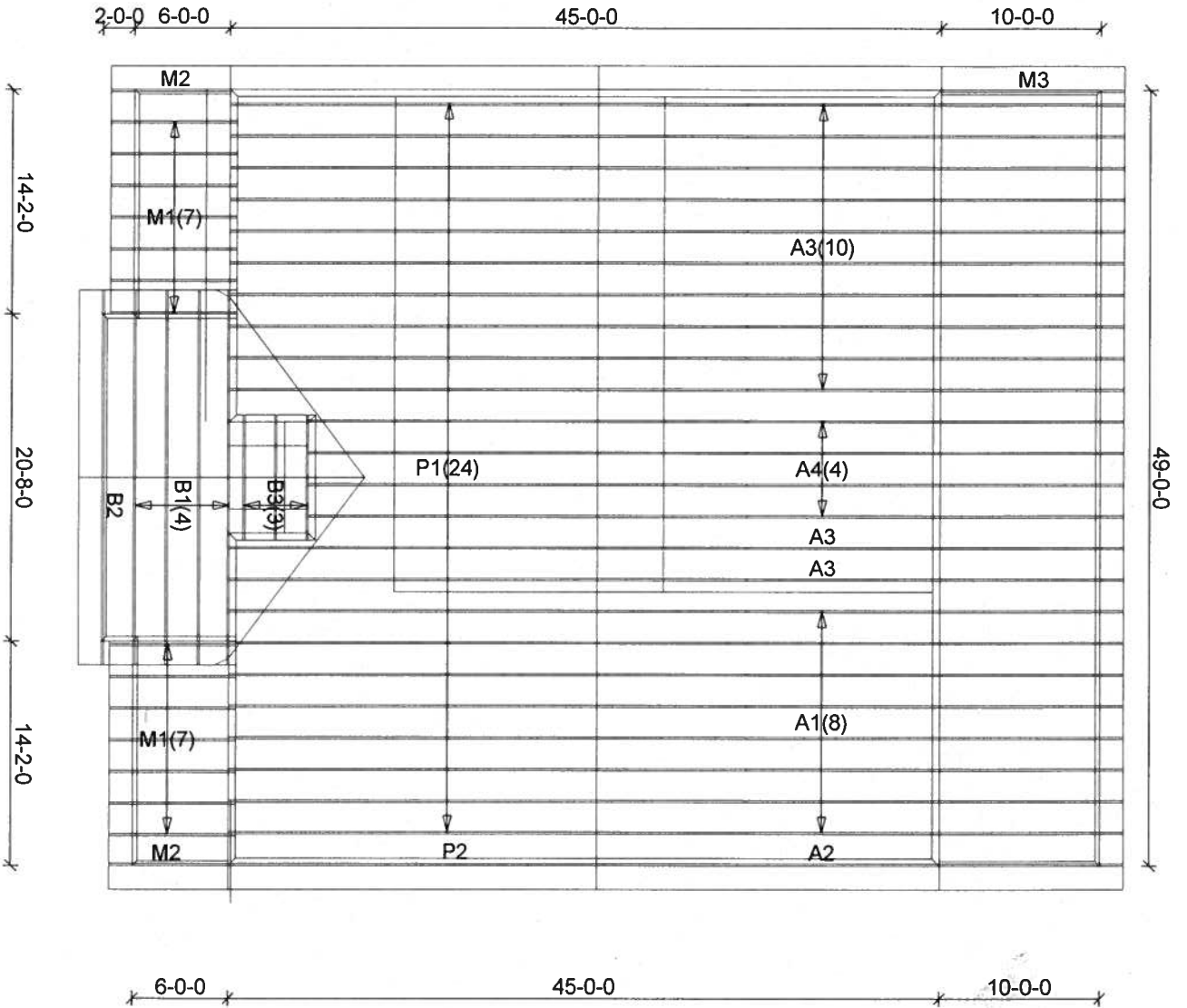
Mayo Truss Co. Inc.

362 NE CLYDE AVE.  
MAYO, FL 32066  
(386) 294-3988  
(877)-558-6262

# FORT WHITE DEVELOPERS

HUETT RESIDENCE

110 MPH ASCE WIND LOAD



Roof Loading  
TC Live: 20.00 psf  
TC Dead: 10.00 psf  
BC Live: 0.00 psf  
BC Dead: 10.00 psf  
TC Stress Inc: 25.00  
BC Stress Inc: 25.00  
Spacing: 2'-0" o.c.

Account: CONTRACTORS  
Job: FWDEV-HUETT  
Designer: M.MURRAY  
Checker: M.MURRAY  
Date: 02-01-06

Permit Number: \_\_\_\_\_ Lot Number: \_\_\_\_\_

Miscellaneous: \_\_\_\_\_ Address: \_\_\_\_\_

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: FWDEV-HUETT - HUETT RESIDENCE

ROBBINS  
ENGINEERING, INC.P.O. Box 280055  
Tampa, FL 33682-0055  
Phone: (813) 972-1135

## Engineering Index Sheet

Index Page 1 of 1

## Standard Loading:

T.C. Live	20 psf
T.C. Dead	10 psf
B.C. Live	0 psf
B.C. Dead	10 psf
Total	40 psf

ANSI/ASCE 7-02  
Wind Speed - 110 mph  
Mean Roof Ht. - 15 ft.  
Exposure Category - B  
Occupancy Factor - 1.00  
MWFRS  
Enclosed

Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06012415	01/26/2006		11

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-1995, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Software - Online Plus)

Notes: Refer to individual  
truss design drawings for  
special loading  
conditions.

Date Mark			Date Mark			Date Mark			Date Mark		
1	01/26/06	A1	2	01/26/06	A2	3	01/26/06	A3	4	01/26/06	A4
5	01/26/06	B1	6	01/26/06	B2	7	01/26/06	B3	8	01/26/06	M1
9	01/26/06	M2	10	01/26/06	P1	11	01/26/06	P2			

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 1/26/2006

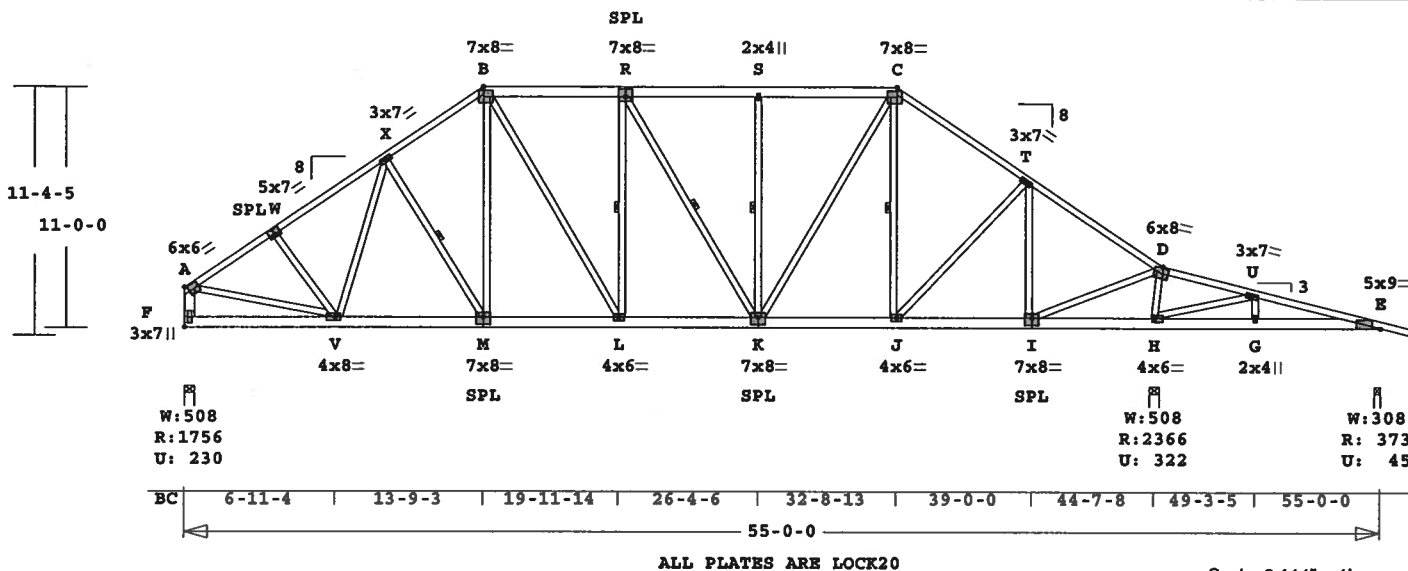
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A1	8	SP	550000	3	0	1- 6- 0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE

HO 1-9-15

HO 3-14

TC| 4-1-14 | 9-3-4 | 13-9-3 | 20-3-6 | 26-4-6 | 32-8-13 | 38-8-8 | 45-0-1 | 49-1-9 | 55-0-0 | 10600



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 543.9 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---  
TC 0.38 2x 4 SP-#2  
EX B -R 2x 6 SP-#2  
EX R -C 2x 6 SP-#2  
BC 0.27 2x 6 SP-#2  
WB 0.38 2x 4 SP-#2  
EX F -A 2x 6 SP-#2

Brace truss as follows:

O.C. From To  
TC Cont. 0- 0- 0 13- 9- 3  
TC 2- 0- 0 13- 9- 3 32- 8-13  
TC Cont. 32- 8-13 55- 0- 0  
BC Cont. 0- 0- 0 55- 0- 0  
WB 1 rows CLB on X -M  
WB 1 rows CLB on L -R  
WB 1 rows CLB on R -K  
WB 1 rows CLB on K -S  
WB 1 rows CLB on J -C  
Attach CLB with (2)-10d nails  
at each web.

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
F	1756	231	5- 8	2- 1
			Hx =	-268
H	2366	322	5- 8	2-10
E	373	46	3- 8	1- 8
			Hx =	223

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -W	0.23	2122	C 0.02	0.21
W -X	0.25	1959	C 0.05	0.20
X -B	0.26	1851	C 0.01	0.25
B -R	0.22	1806	C 0.01	0.21
R -S	0.21	1739	C 0.00	0.21
S -C	0.19	1739	C 0.01	0.18
C -T	0.30	1645	C 0.01	0.29
T -D	0.38	1407	C 0.01	0.37
D -U	0.36	547	T 0.08	0.28
U -E	0.28	301	C 0.00	0.28

Bottom Chords  
F -V 0.12 249 T 0.00 0.12  
V -M 0.24 1644 T 0.19 0.05  
M -L 0.24 1542 T 0.18 0.06  
L -K 0.27 1806 T 0.21 0.06  
K -J 0.22 1367 T 0.16 0.06  
J -I 0.20 1167 T 0.14 0.06  
I -H 0.16 744 C 0.00 0.16  
H -G 0.19 302 T 0.03 0.16  
G -E 0.17 302 T 0.03 0.14

Webs  
F -A 0.10 1690 C WindLd  
A -V 0.33 1825 T  
V -W 0.08 253 C  
W -X 0.04 81 T  
X -M 0.05 190 C 1 Br  
M -B 0.06 349 T  
B -L 0.19 516 T  
L -R 0.11 313 C 1 Br  
R -K 0.06 132 C 1 Br  
K -S 0.14 393 C 1 Br  
K -C 0.18 726 T  
J -C 0.02 93 T 1 Br  
J -T 0.05 287 T  
I -T 0.36 690 C  
I -D 0.38 2059 T  
H -D 0.22 1962 C  
H -U 0.25 883 C  
G -U 0.02 189 T

LL Defl -0.04" in M -L L/999  
TL Defl -0.17" in L -K L/999  
Shear // Grain in D -U 0.23

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 6.0x 6.0-0.4-0.3 0.97  
W LOCK 5.0x 7.0-0.3 0.5 0.85  
X LOCK 3.0x 7.0 Ctr Ctr 0.64  
B LOCK 7.0x 8.0-0.5 Ctr 0.92  
R LOCK 7.0x 8.0 Ctr 0.8 0.76  
S LOCK 2.0x 4.0 Ctr Ctr 0.57  
C LOCK 7.0x 8.0 0.5 Ctr 0.92  
T LOCK 3.0x 7.0 Ctr Ctr 0.50  
D LOCK 6.0x 8.0-1.0 0.2 0.87  
U LOCK 3.0x 7.0 Ctr Ctr 0.48  
E LOCK 5.0x 9.0-9.0 2.2 0.88  
F LOCK 3.0x 7.0 Ctr Ctr 0.81  
V LOCK 4.0x 8.0-1.0 Ctr 0.87  
M LOCK 7.0x 8.0 Ctr-0.8 0.76  
L LOCK 4.0x 6.0 Ctr Ctr 0.53  
K LOCK 7.0x 8.0 Ctr-0.8 0.76  
J LOCK 4.0x 6.0 Ctr Ctr 0.48  
I LOCK 7.0x 8.0 Ctr-0.8 0.76

H LOCK 4.0x 6.0 Ctr Ctr 0.76  
G LOCK 2.0x 4.0 Ctr Ctr 0.57

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Prevent truss rotation at all  
bearing locations.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 2122 Lbs

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



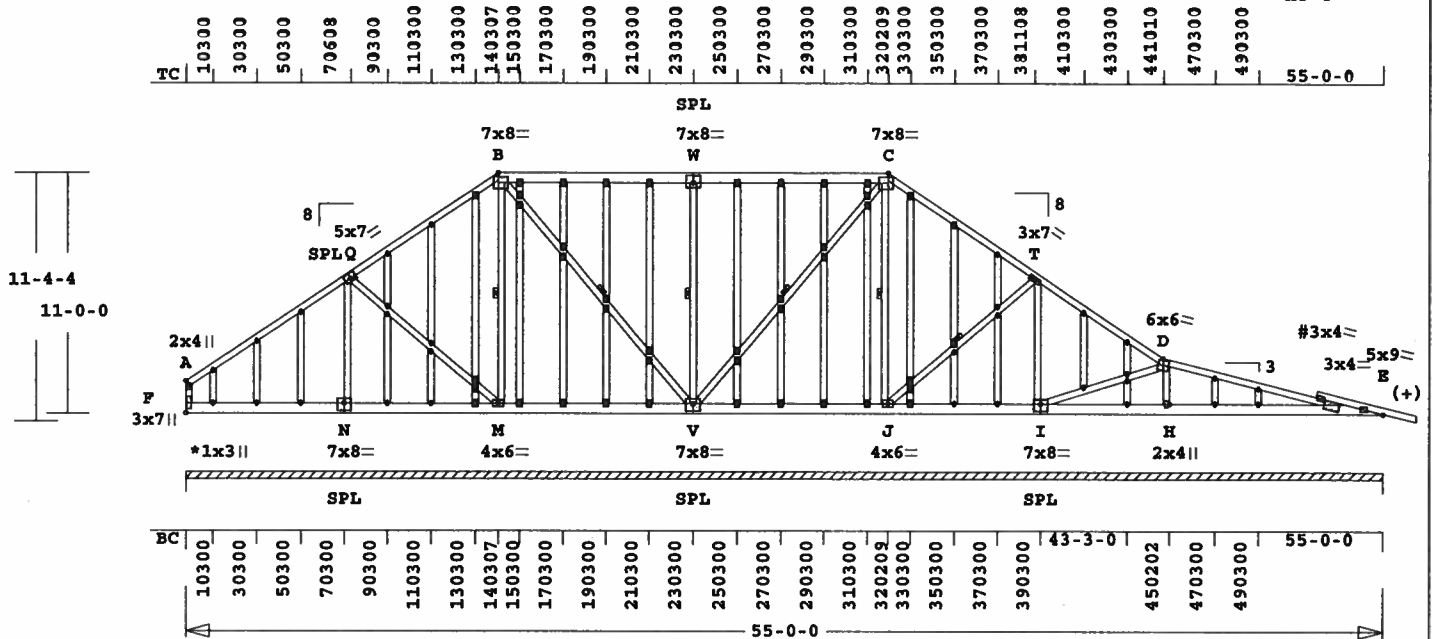
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A1	8	SP	550000	3	0	1- 6- 0	T06012415
U# J#FWDEV-HUETT HUETT RESIDENCE								

Quality Control Factor 1.25

**U# J#FWDEV-HUETT HUETT RESIDENCE**

HO 1-5-11

HO 4



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

See \* For Typical Gable Plate Size and Placement

Scale: 0.114" = 1'

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI	-Size-	---	Lumber---
TC	0.65	2x 4	SP-#2
EX B -W	2x 6	SP-#2	
EX W -C	2x 6	SP-#2	
BC	0.30	2x 6	SP-#2
WB	0.29	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	55- 0- 0
BC Cont.	0- 0- 0	55- 0- 0
WB 1 rows CLB	on M -B	
WB 1 rows CLB	on B -V	
WB 1 rows CLB	on V -W	
WB 1 rows CLB	on V -C	
WB 1 rows CLB	on J -C	
WB 1 rows CLB	on J -T	

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead (psf)
TC	20.0	10.0
BC	0.0	10.0
Total	20.0	20.0
Spacing		24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0- 0- 0	55- 0- 0	
	4400	582	Hx	= 263

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -Q	0.65	89 T	0.02	0.63
Q -B	0.63	125 T	0.00	0.63
B -W	0.43	120 T	0.00	0.43
W -C	0.43	120 T	0.00	0.43
C -T	0.44	151 C	0.00	0.44
T -D	0.44	272 C	0.00	0.44
D -E	0.41	824 C	0.01	0.40
-----Bottom Chords-----				
F -N	0.15	0 T	0.00	0.15
N -M	0.15	0 T	0.00	0.15

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 746.7 LBS

M -V	0.21	0 T	0.00	0.21
V -J	0.21	0 T	0.00	0.21
J -I	0.17	0 T	0.00	0.17
I -H	0.21	0 T	0.00	0.21
H -E	0.30	28 T	0.00	0.30

F	-A	0.01	178 C	WindLd
N	-Q	0.25	565 C	
Q	-M	0.09	177 T	
M	-B	0.10	302 C	1 Br
B	-V	0.06	105 C	1 Br
V	-W	0.21	609 C	1 Br
W	-C	0.13	221 C	1 Br
C	-J	0.06	176 C	1 Br
J	-T	0.04	161 C	1 Br
T	-I	0.10	236 C	
I	-D	0.29	608 C	
H	-D	0.01	172 C	

LL Defl -0.03" in H -E2 L/999  
TL Defl -0.09" in H -E2 L/999  
Shear // Grain in A -Q 0.28

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.74  
Q LOCK 5.0x 7.0-0.3 0.5 0.85  
B LOCK 7.0x 8.0-0.5 Ctr 0.92  
W LOCK 7.0x 8.0 Ctr 0.8 0.76  
C LOCK 7.0x 8.0 0.5 Ctr 0.92  
T LOCK 3.0x 7.0 Ctr Ctr 0.50  
D LOCK 6.0x 6.0 Ctr Ctr 0.87  
E LOCK 5.0x 9.0 Ctr-0.8 0.93  
F LOCK 3.0x 7.0 Ctr Ctr 0.69  
N LOCK 7.0x 8.0 Ctr-0.8 0.76  
M LOCK 4.0x 6.0 Ctr Ctr 0.48  
V LOCK 7.0x 8.0 Ctr-0.8 0.76  
J LOCK 4.0x 6.0 Ctr Ctr 0.48  
I LOCK 7.0x 8.0 Ctr-0.8 0.76  
H LOCK 2.0x 4.0 Ctr Ctr 0.57

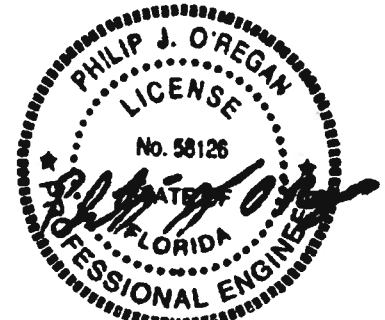
36 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.  
Design checked for 10 psf non-concurrent LL on BC.  
Prevent truss rotation at all bearing locations.  
Refer to Gen Det 3 series for web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 824 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A3	12	SP	550000	3	0	1- 6- 0	T06012415

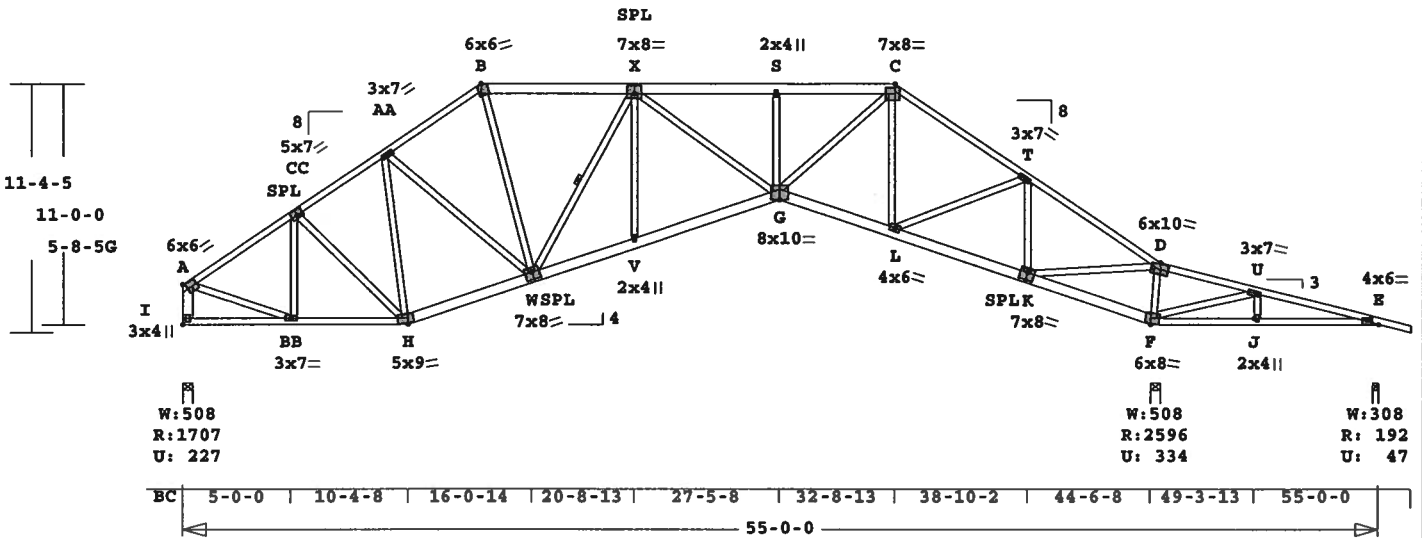
U# J#FWDEV-HUETT HUETT RESIDENCE

HO 1-9-15

HO 3-14

10601

TC 5-3-8 | 9-3-4 | 13-9-3 | 20-8-13 | 27-3-12 | 32-8-13 | 38-10-2 | 45-0-1 | 49-5-9 | 55-0-0



W:508  
R:1707  
U: 227

W:508  
R:2596  
U: 334

W:308  
R: 192  
U: 47

BC 5-0-0 | 10-4-8 | 16-0-14 | 20-8-13 | 27-5-8 | 32-8-13 | 38-10-2 | 44-6-8 | 49-3-13 | 55-0-0

ALL PLATES ARE LOCK20

Scale: 0.114" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 483.0 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---  
TC 0.66 2x 4 SP-#2  
EX B -X 2x 6 SP-#2  
EX X -C 2x 6 SP-#2  
BC 0.47 2x 6 SP-#2  
EX I -H 2x 4 SP-#2  
EX F -E 2x 4 SP-#2  
WB 0.50 2x 4 SP-#2  
EX I -A 2x 6 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 55- 0- 0  
BC Cont. 0- 0- 0 55- 0- 0  
WB 1 rows CLB on W-X  
Attach CLB with (2)-10d nails  
at each web.

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
I	1707	228	5- 8	2- 0
			Hx =	-271
F	2596	334	5- 8	2-14
E	192	48	3- 8	1- 8
			Hx =	224

Membr	CSI	P	Lbs	Ax1	CSI	Bnd
-----Top Chords-----						
A	-CC	0.29	1811	C	0.04	0.25
CC-AA	0.25	1906	C	0.02	0.23	
AA-B	0.28	2042	C	0.02	0.26	
B-X	0.25	1921	C	0.01	0.24	
X-S	0.27	3454	C	0.03	0.24	
S-C	0.14	3454	C	0.04	0.10	
C-T	0.44	2350	C	0.10	0.34	
T-D	0.46	1466	C	0.01	0.45	
D-U	0.66	1516	T	0.24	0.42	
U-E	0.46	247	T	0.04	0.42	
-----Bottom Chords-----						
I	-BB	0.14	250	T	0.00	0.14
BB-H	0.33	1519	T	0.25	0.08	
H-W	0.24	1611	T	0.19	0.05	

W-V	0.37	2684	T	0.32	0.05
V-G	0.47	2699	T	0.33	0.14
G-L	0.41	2041	T	0.25	0.16
L-K	0.29	1293	T	0.15	0.14
K-F	0.35	1604	C	0.00	0.35
F-J	0.40	227	C	0.00	0.40
J-E	0.29	227	C	0.00	0.29
-----Webs-----					
I-A	0.10	1660	C	WindLd	
A-BB	0.29	1603	T		
BB-CC	0.12	409	C		
CC-H	0.03	79	T		
AA-H	0.35	458	C		
AA-W	0.04	237	T		
B-W	0.15	839	T		
W-X	0.40	1335	C	1 Br	
V-X	0.03	199	T		
X-G	0.20	1102	T		
G-S	0.08	310	C		
G-C	0.36	1960	T		
L-C	0.16	320	C		
L-T	0.14	796	T		
K-T	0.31	1137	C		
K-D	0.50	2715	T		
F-D	0.19	1654	C		
F-U	0.39	1305	C		
J-U	0.04	292	T		

LL Defl -0.19" in G -L L/999  
TL Defl -0.39" in V -G L/999  
Shear // Grain in D -U 0.27  
Hz Disp LL DL TL  
Jt F 0.13" 0.13" 0.26"

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 6.0x 6.0-0.4-0.3 0.97  
CC LOCK 5.0x 7.0-0.3 0.5 0.85  
AA LOCK 3.0x 7.0 Ctr Ctr 0.50  
B LOCK 6.0x 6.0 1.1-3.7 0.75  
X LOCK 7.0x 8.0 Ctr 0.7 0.76  
S LOCK 2.0x 4.0 Ctr Ctr 0.57  
C LOCK 7.0x 8.0 0.5 Ctr 0.92  
T LOCK 3.0x 7.0 Ctr Ctr 0.50  
D LOCK 6.0x10.0-0.5 0.1 0.94  
U LOCK 3.0x 7.0 Ctr Ctr 0.47  
E LOCK 4.0x 6.0 Ctr 0.1 1.00  
I LOCK 3.0x 4.0 Ctr Ctr 0.81  
BB LOCK 3.0x 7.0 Ctr Ctr 0.58  
H LOCK 5.0x 9.0-1.5 3.2 0.75  
W LOCK 7.0x 8.0 0.3-0.8 0.74  
V LOCK 2.0x 4.0 Ctr Ctr 0.57  
G LOCK 8.0x10.0 Ctr-1.8 0.70  
L LOCK 4.0x 6.0 Ctr Ctr 0.51

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Prevent truss rotation at all  
bearing locations.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682

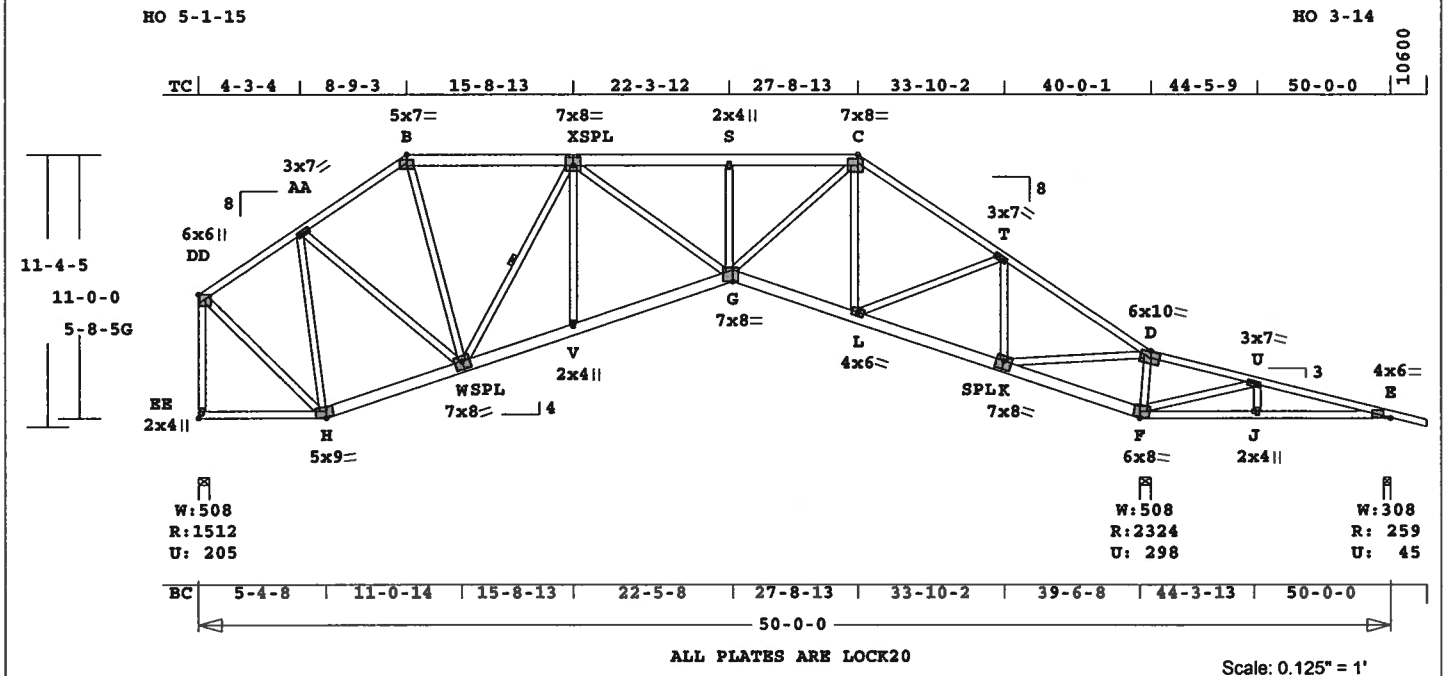


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A3	12	SP	550000	3	0	1- 6- 0	T06012415
U# J#FWDEV-HUETT HUETT RESIDENCE								

Max comp. force 3454 Lbs  
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	A4	4	SP	500000	3	0	1- 6- 0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 449.0 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ----Lumber----

TC	0.57	2x 4	SP-#2
EX B -X	2x 6	SP-#2	
EX X -C	2x 6	SP-#2	
BC	0.35	2x 6	SP-#2
EX EE-H	2x 4	SP-#2	
EX F -E	2x 4	SP-#2	
WB	0.70	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50- 0- 0
BC Cont.	0- 0- 0	50- 0- 0

WB 1 rows CLB on W -X  
Attach CLB with (2)-10d nails  
at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
Lbs	Lbs	In-Sx	In-Sx	
EE	1512	206	5- 8	1-13
			Hx =	-305
F	2324	298	5- 8	2- 9
E	260	46	3- 8	1- 8
			Hx =	195

Membr	CSI	P	Lbs	Ax1-CSI-Bnd
-----Top Chords-----				
DD-AA	0.17		983 C	0.00 0.17
AA-B	0.21		1441 C	0.01 0.20
B -X	0.24		1328 C	0.00 0.24
X -S	0.26		2875 C	0.02 0.24
S -C	0.11		2875 C	0.02 0.09
C -T	0.39		2038 C	0.08 0.31
T -D	0.42		1358 C	0.01 0.41
D -U	0.57		1216 T	0.19 0.38
U -E	0.38		127 T	0.00 0.38
-----Bottom Chords-----				
EE-H	0.17		241 T	0.00 0.17
H -W	0.15		761 T	0.09 0.06
W -V	0.28		2074 T	0.25 0.03

V -G	0.35	2081 T	0.25	0.10
G -L	0.33	1769 T	0.21	0.12
L -K	0.26	1197 T	0.14	0.12
K -F	0.31	1292 C	0.00	0.31
F -J	0.34	72 T	0.00	0.34
J -E	0.25	72 T	0.00	0.25

-----Webs-----

EE-DD	0.46	1455 C	WindLd
DD-H	0.21	1147 T	
AA-H	0.70	924 C	
AA-W	0.11	619 T	
B -W	0.09	503 T	
W -X	0.41	1357 C	1 Br
V -X	0.03	218 T	
X -G	0.20	1105 T	
G -S	0.09	321 C	
G -C	0.28	1545 T	
L -C	0.11	217 C	
L -T	0.11	609 T	
K -T	0.26	958 C	
K -D	0.43	2333 T	
F -D	0.17	1497 C	
F -U	0.38	1242 C	
J -U	0.04	280 T	

LL Defl -0.14" in G -L L/999  
TL Defl -0.29" in G -L L/999  
Shear // Grain in D -U 0.26  
Hz Disp LL DL TL  
Jt F 0.11" 0.10" 0.21"

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga,	Gross Area
Plate - RHS	20 Ga,	Gross Area	
Jt Type	Plt Size	X	Y
DD LOCK	6.0x 6.0	Ctr	0.8
AA LOCK	3.0x 7.0	Ctr	0.49
B LOCK	5.0x 7.0	Ctr	0.95
X LOCK	7.0x 8.0	Ctr	0.7
S LOCK	2.0x 4.0	Ctr	0.54
C LOCK	7.0x 8.0	0.5	0.87
T LOCK	3.0x 7.0	Ctr	0.48
D LOCK	6.0x10.0	0.5	0.1
U LOCK	3.0x 7.0	Ctr	0.46
E LOCK	4.0x 6.0	Ctr	0.1
EE LOCK	2.0x 4.0	Ctr	0.68
H LOCK	5.0x 9.0	1.0	3.2
W LOCK	7.0x 8.0	0.3	0.8
V LOCK	2.0x 4.0	Ctr	0.54
G LOCK	7.0x 8.0	1.0	2.3
L LOCK	4.0x 6.0	Ctr	0.49
K LOCK	7.0x 8.0	0.3	0.8
F LOCK	6.0x 8.0	1.1	3.6
J LOCK	2.0x 4.0	Ctr	0.54

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Provide connection to bearing  
for 305 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.  
Prevent truss rotation at all  
bearing locations.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 2875 Lbs  
Quality Control Factor 1.25

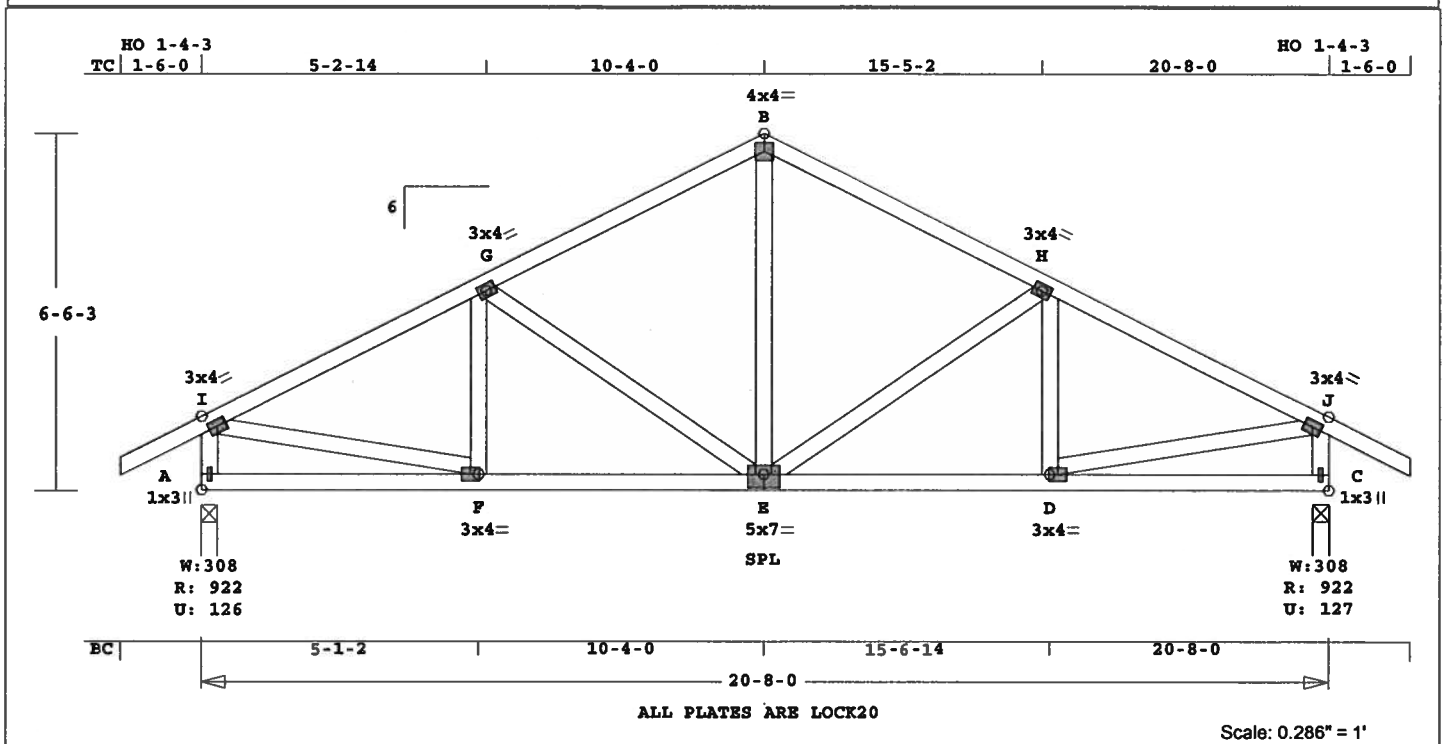
Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Job <b>FWDEV-HUETT</b>	Mark <b>B1</b>	Quan <b>4</b>	Type <b>TR</b>	Span <b>200800</b>	P1-H1 <b>6</b>	Left OH <b>1- 6- 0</b>	Right OH <b>1- 6- 0</b>	Engineering <b>T06012415</b>
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**U# J#FWDEV-HUETT HUETT RESIDENCE**



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 157.1 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ----Lumber-----  
TC 0.28 2x 4 SP-#2  
BC 0.24 2x 4 SP-#2  
WB 0.17 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 0
BC Cont.	0- 0- 0	20- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	923	127	3- 8	1- 8
			Hx =	-140
C	923	127	3- 8	1- 8
			Hx =	119

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
I -G	0.28	1000	C	0.00	0.28
G -B	0.28	779	C	0.00	0.28
B -H	0.28	779	C	0.00	0.28
H -J	0.28	1000	C	0.00	0.28
-----Bottom Chords-----					
A -F	0.15	126	T	0.00	0.15
F -E	0.24	907	T	0.09	0.15
E -D	0.24	907	T	0.09	0.15

D -C	0.15	110	T	0.00	0.15
-----Webs-----					
A -I	0.07	782	C	WindLd	
I -F	0.17	930	T		
F -G	0.01	93	C		
G -E	0.12	262	C		
E -B	0.07	421	T		
E -H	0.12	262	C		
D -H	0.01	93	C		
D -J	0.17	930	T		
C -J	0.07	782	C	WindLd	

LL Defl -0.02" in A -F L/999  
TL Defl -0.05" in E -D L/999  
Shear // Grain in I -G 0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
I LOCK 3.0x 4.0 Ctr Ctr 0.69  
G LOCK 3.0x 4.0 Ctr Ctr 0.58  
B LOCK 4.0x 4.0 Ctr Ctr 0.65  
H LOCK 3.0x 4.0 Ctr Ctr 0.58  
J LOCK 3.0x 4.0 Ctr Ctr 0.69  
A LOCK 1.0x 3.0 Ctr Ctr 0.81  
F LOCK 3.0x 4.0 Ctr Ctr 0.70  
E LOCK 5.0x 7.0 Ctr-0.5 0.58  
D LOCK 3.0x 4.0 Ctr Ctr 0.70  
C LOCK 1.0x 3.0 Ctr Ctr 0.81

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 1000 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



FWDEV-HUETT

**Mark**  
**B2**

Quant

Type  
TR

**Span**  
**200800**

**P1-H1**  
**6**

Left  
0

Right OH  
0

Engineering  
T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



ALL PLATES ARE LOCK20

Scale: 0 286" = 1'

P	-R	0.02	0	T	0.00	0.02
R	-T	0.02	0	T	0.00	0.02
T	-C	0.02	0	T	0.00	0.02

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

	CSI	-Size-	----Lumber----
TC	0.04	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
WB	0.01	2x 4	SP-#2
GW	0.04	2x 4	SP-#2

Brace truss as follows:		
O.C.	From	To
TC Cont.	0- 0- 0	20- 8- 0
BC Cont.	0- 0- 0	20- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber	Duration	Factor	1.25
Plate	Duration	Factor	1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

-----Webs-----			
A	-U	0.00	76 C WindLd
U	-E	0.01	82 T
T	-V	0.01	79 T
C	-V	0.00	76 C WindLd
-----Gable Webs-----			
E	-D	0.01	141 C
G	-F	0.01	115 C
I	-H	0.02	120 C
K	-J	0.03	124 C
L	-B	0.04	95 C
N	-M	0.03	124 C
P	-O	0.02	120 C
R	-Q	0.01	115 C
T	-S	0.01	141 C

```
LL Defl  0.00" in A -E  L/999
TL Defl  0.00" in T -C  L/999
Shear // Grain in U -D   0.09
```

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 141 Lbs  
Quality Control Factor 1.25

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0-	0 to	20- 8-	0
	1653	220	Hz =	126

Membr	CSI	P	Lbs	Ax1-CSI-Bnd
-----Top Chords-----				
U -D	0.04		61 C	0.00
D -F	0.04		39 C	0.00
F -H	0.03		28 C	0.00
H -J	0.03		46 T	0.00
J -B	0.03		79 T	0.00
B -M	0.03		79 T	0.00
M -O	0.03		46 T	0.00
O -Q	0.03		28 C	0.00
Q -S	0.04		39 C	0.00
S -V	0.04		61 C	0.00
-----Bottom Chords-----				
A -E	0.02		0 T	0.00
E -G	0.02		0 T	0.00
G -I	0.02		0 T	0.00
I -K	0.02		0 T	0.00
K -L	0.02		0 T	0.00
L -N	0.02		0 T	0.00
N -P	0.02		0 T	0.00

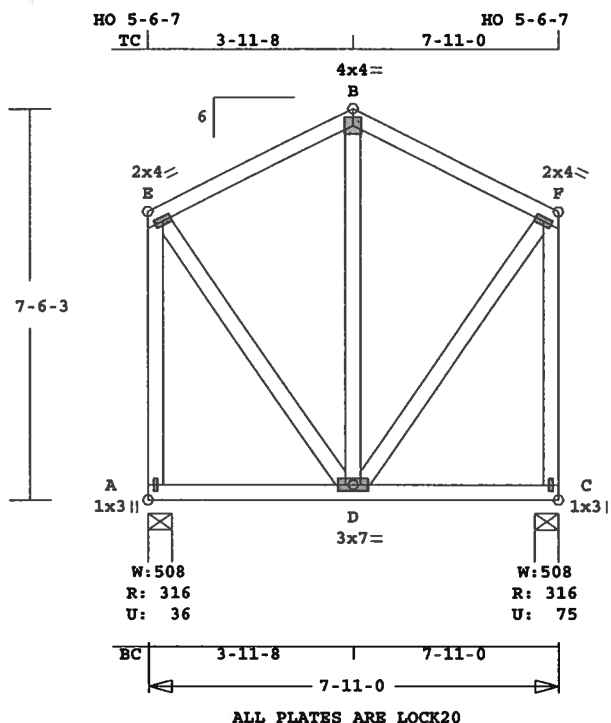
Plate	-	LOCK	20 Ga,	Gross	Area
Plate	-	RHS	20 Ga,	Gross	Area
Jt	Type	Plt	Size	X	Y
U	LOCK	3.0x	4.0	Ctr	Ctr
D	LOCK	1.0x	3.0	Ctr	Ctr
F	LOCK	1.0x	3.0	Ctr	Ctr
H	LOCK	1.0x	3.0	Ctr	Ctr
J	LOCK	1.0x	3.0	Ctr	Ctr
B	LOCK	4.0x	4.0	Ctr	Ctr
M	LOCK	1.0x	3.0	Ctr	Ctr
O	LOCK	1.0x	3.0	Ctr	Ctr
Q	LOCK	1.0x	3.0	Ctr	Ctr
S	LOCK	1.0x	3.0	Ctr	Ctr
V	LOCK	3.0x	4.0	Ctr	Ctr
A	LOCK	1.0x	3.0	Ctr	Ctr
E	LOCK	3.0x	4.0	Ctr	Ctr
G	LOCK	1.0x	3.0	Ctr	Ctr
I	LOCK	1.0x	3.0	Ctr	Ctr
K	LOCK	1.0x	3.0	Ctr	Ctr
L	LOCK	5.0x	5.0	Ctr	-0.5
N	LOCK	1.0x	3.0	Ctr	Ctr
P	LOCK	1.0x	3.0	Ctr	Ctr
R	LOCK	1.0x	3.0	Ctr	Ctr
T	LOCK	3.0x	4.0	Ctr	Ctr
C	LOCK	1.0x	3.0	Ctr	Ctr

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	B3	3	TR	71100	6	0	0	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 87.7 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

TC	BC	WB	CSI	Size	Lumber
0.14	0.10	0.10	2x 4	SP-#2	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7-11- 0
BC Cont.	0- 0- 0	7-11- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	317	37	5- 8	1- 8
			Hz =	-261
C	317	75	5- 8	1- 8
			Hz =	237

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
E -B	0.14	99	C	0.00	0.14
B -F	0.14	99	C	0.00	0.14

-----Bottom Chords-----  
A -D 0.10 193 T 0.00 0.10  
D -C 0.10 168 T 0.00 0.10  
-----Webs-----  
A -E 0.10 283 C WindLd  
E -D 0.04 160 T  
D -B 0.10 168 C  
D -F 0.04 160 T  
C -F 0.10 283 C WindLd

LL Defl -0.01" in A -D L/999  
TL Defl -0.01" in A -D L/999  
Shear // Grain in E -B 0.14

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
E LOCK 2.0x 4.0 Ctr Ctr 0.76  
B LOCK 4.0x 4.0 Ctr Ctr 0.47  
F LOCK 2.0x 4.0 Ctr Ctr 0.76  
A LOCK 1.0x 3.0 Ctr Ctr 0.75  
D LOCK 3.0x 7.0 Ctr Ctr 0.33  
C LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.

Analysis Conforms To:  
FBC2004

Design checked for 10 psf non-  
concurrent LL on BC.

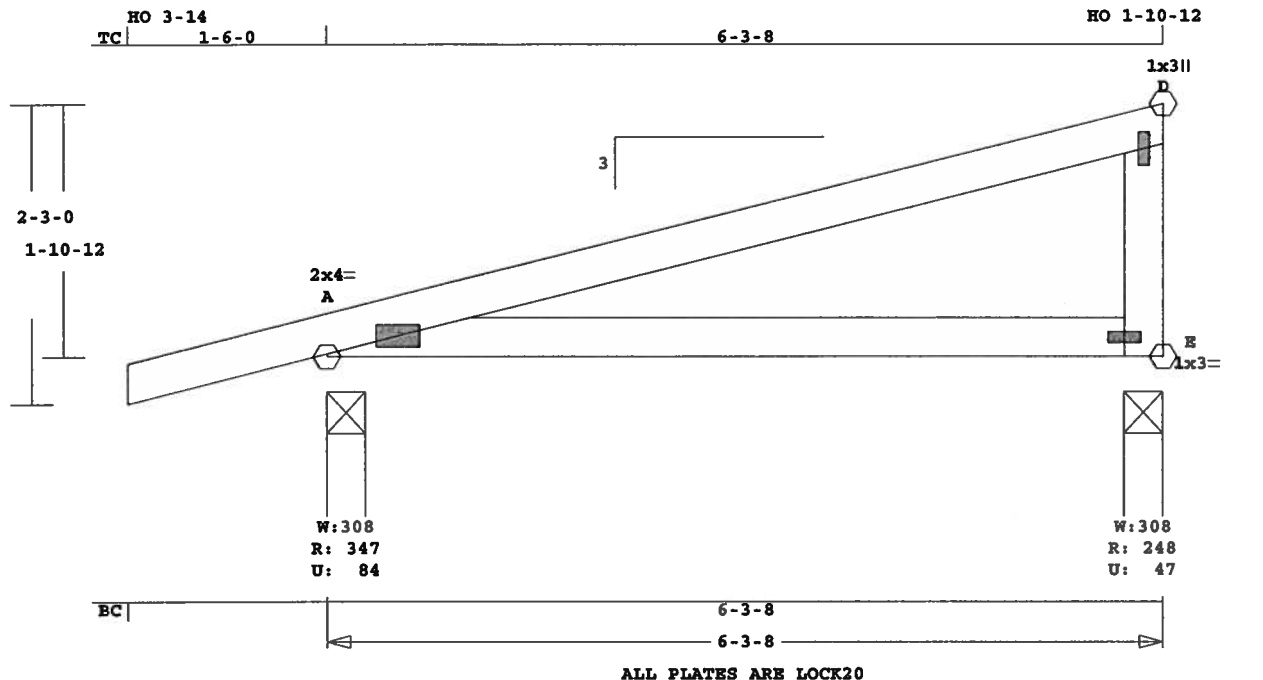
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.

Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 283 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	M1	14	MONO.DD	60308	3	1- 6- 0	0	T06012415
U# J#FWDEV-HUETT HUETT RESIDENCE								



Scale: 0.697" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 29.7 LBS

Online Plus -- Version 18.5.027  
 RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---  
 TC 0.31 2x 4 SP-#2  
 BC 0.24 2x 4 SP-#2  
 WB 0.36 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 3- 8
BC Cont.	0- 0- 0	6- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 5 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	348	85	3- 8	1- 8
			Hx =	39
E	249	47	3- 8	1- 8
			Hx =	69

Membr CSI P Lbs Ax1-CSI-Bnd  
 -----Top Chords-----  
 A -D 0.31 179 C 0.00 0.31  
 -----Bottom Chords-----

A -E 0.24 182 T 0.03 0.21  
 -----Webs-----  
 E -D 0.36 153 C 0.00 0.36

LL Defl -0.02" in A -E L/999  
 TL Defl -0.06" in A -E L/999  
 Shear // Grain in A -D 0.24

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.

REPORT: NER 691  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 2.0x 4.0 Ctr Ctr 0.83  
 D LOCK 1.0x 3.0 Ctr Ctr 0.75  
 E LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY:  
 Robbins Engineering, Inc.  
 PO Box 280055  
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 OH Loading  
 Soffit psf 2.0  
 Design checked for 10 psf non-  
 concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as a Main  
 Wind-Force Resistance System.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 Zone location: Exterior  
 TC Dead Load : 5.0 psf  
 BC Dead Load : 5.0 psf  
 Max comp. force 179 Lbs  
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
 License #: 58126  
 Address: P.O. Box 280055, Tampa, FL 33682



[illegible]

Robbins Engineering, Inc./Online Plus™				APPROX. TRUSS WEIGHT: 30.3 LBS
LL Defl -0.01" in A -C L/999				Truss is designed as a Main
TL Defl -0.02" in A -C L/999				Wind-Force Resistance System.
Online Plus -- Version 18.5.027				Wind Speed: 110 mph
RUN DATE: 26-JAN-06				Mean Roof Height: 15-0
CSI -Size- ----Lumber----				Exposure Category: B
Plates for each ply each face.				Occupancy Factor : 1.00
PLATING CONFORMS TO TPI.				Building Type: Enclosed
REPORT: NER 691				Zone location: Exterior
ROBBINS ENGINEERING, INC.				TC Dead Load : 5.0 psf
BASED ON SP LUMBER				BC Dead Load : 5.0 psf
USING GROSS AREA TEST.				Max comp. force 149 Lbs
Brace truss as follows:				Quality Control Factor 1.25
O.C.	From	To	Plate - LOCK 20 Ga, Gross Area	
			Plate - RHS 20 Ga, Gross Area	

Brace truss as follows:		
O.C.	From	To
TC Cont.	0- 0- 0	6- 0- 0
BC Cont.	0- 0- 0	6- 0- 0

<b>Loading</b>	<b>Live</b>	<b>Dead</b>	<b>(psf)</b>
TC	20.0	10.0	
BC	0.0	10.0	
<b>Total</b>	<b>20.0</b>	<b>20.0</b>	<b>40.0</b>
<b>Spacing</b>			<b>24.0"</b>
<b>Lumber</b>	<b>Duration</b>	<b>Factor</b>	<b>1.25</b>
<b>Plate</b>	<b>Duration</b>	<b>Factor</b>	<b>1.25</b>
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.82  
B LOCK 1.0x 3.0 Ctr Ctr 0.75  
C LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

Plus 5 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont.	Brg	0- 0-	0 to	6- 0- 0
	480	76	Hz =	52

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.19		149 C	0.00	0.19
-----Bottom Chords-----					
A -C	0.14		3 T	0.00	0.14
-----Webs-----					
C -B	0.22		128 C	0.00	0.22

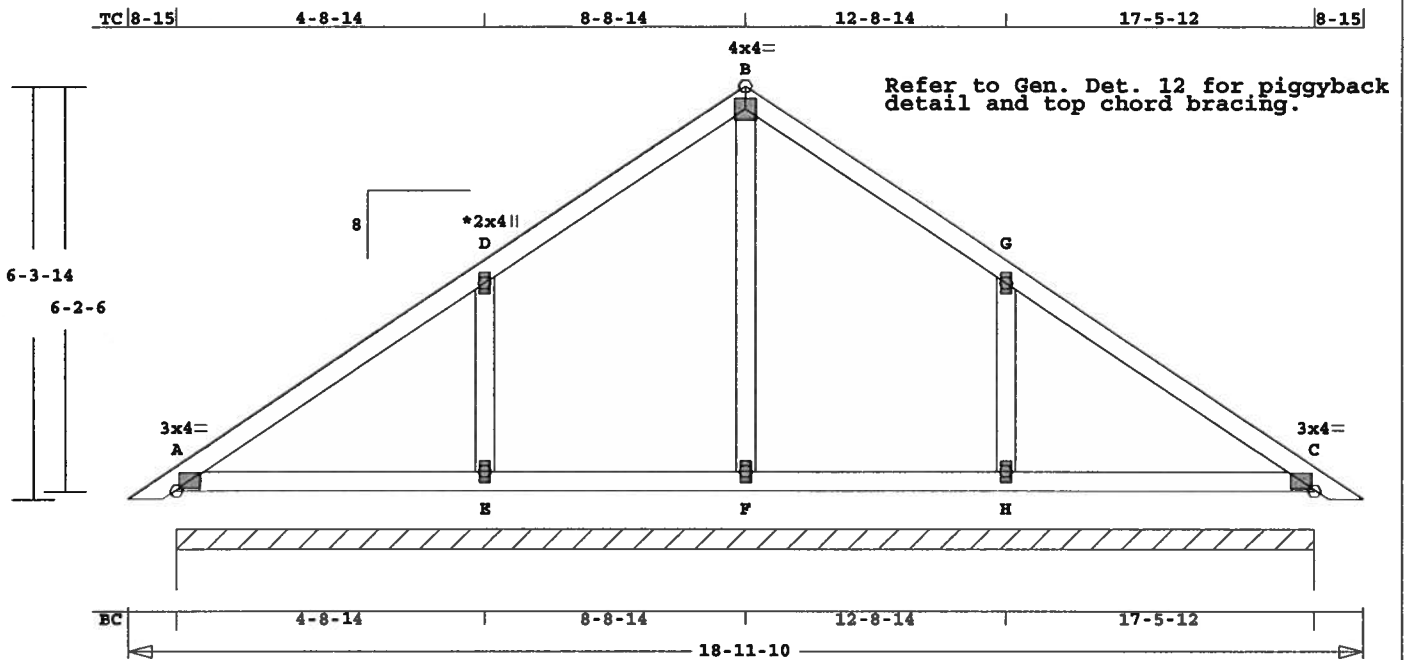
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
WARNING Do Not Cut overframe  
member between outside of  
truss and first tie-plate  
to inside of heel plate.  
Design checked for 10 psf non-  
concurrent LL on BC.  
Refer to Gen Det 3 series for  
web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
FWDEV-HUETT	P1	24	TR	181110	8	8-15	8-15	T06012415

U# J#FWDEV-HUETT HUETT RESIDENCE



ALL PLATES ARE LOCK20  
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.341" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 97.8 LBS

Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI	Size	Lumber
TC	0.20	2x 4 SP-#2
BC	0.12	2x 4 SP-#2
GW	0.04	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18-11-10
BC Cont.	0- 0- 0	18-11-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 17- 5-12		
	1494	203	Hz =	122

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -D	0.20	160 C	0.00	0.20
D -B	0.20	181 C	0.00	0.20
B -G	0.20	181 C	0.00	0.20
G -C	0.20	160 C	0.00	0.20
-----Bottom Chords-----				
A -E	0.12	1 T	0.00	0.12
E -F	0.12	0 T	0.00	0.12
F -H	0.12	0 T	0.00	0.12
H -C	0.12	1 T	0.00	0.12

-----Gable Webs-----  
E -D 0.04 287 C  
F -B 0.02 53 T  
H -G 0.04 287 C

LL Defl -0.01" in H -C L/999  
TL Defl -0.02" in H -C L/999  
Shear // Grain in A -D 0.16

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.69  
D LOCK 2.0x 4.0 Ctr Ctr 0.00  
B LOCK 4.0x 4.0 Ctr Ctr 0.58  
G LOCK 2.0x 4.0 Ctr Ctr 0.00  
C LOCK 3.0x 4.0 Ctr Ctr 0.69  
E LOCK 2.0x 4.0 Ctr Ctr 0.00  
F LOCK 2.0x 4.0 Ctr Ctr 0.00  
H LOCK 2.0x 4.0 Ctr Ctr 0.00

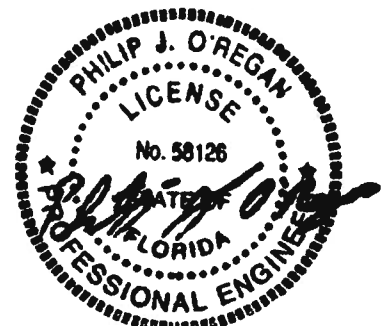
REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-

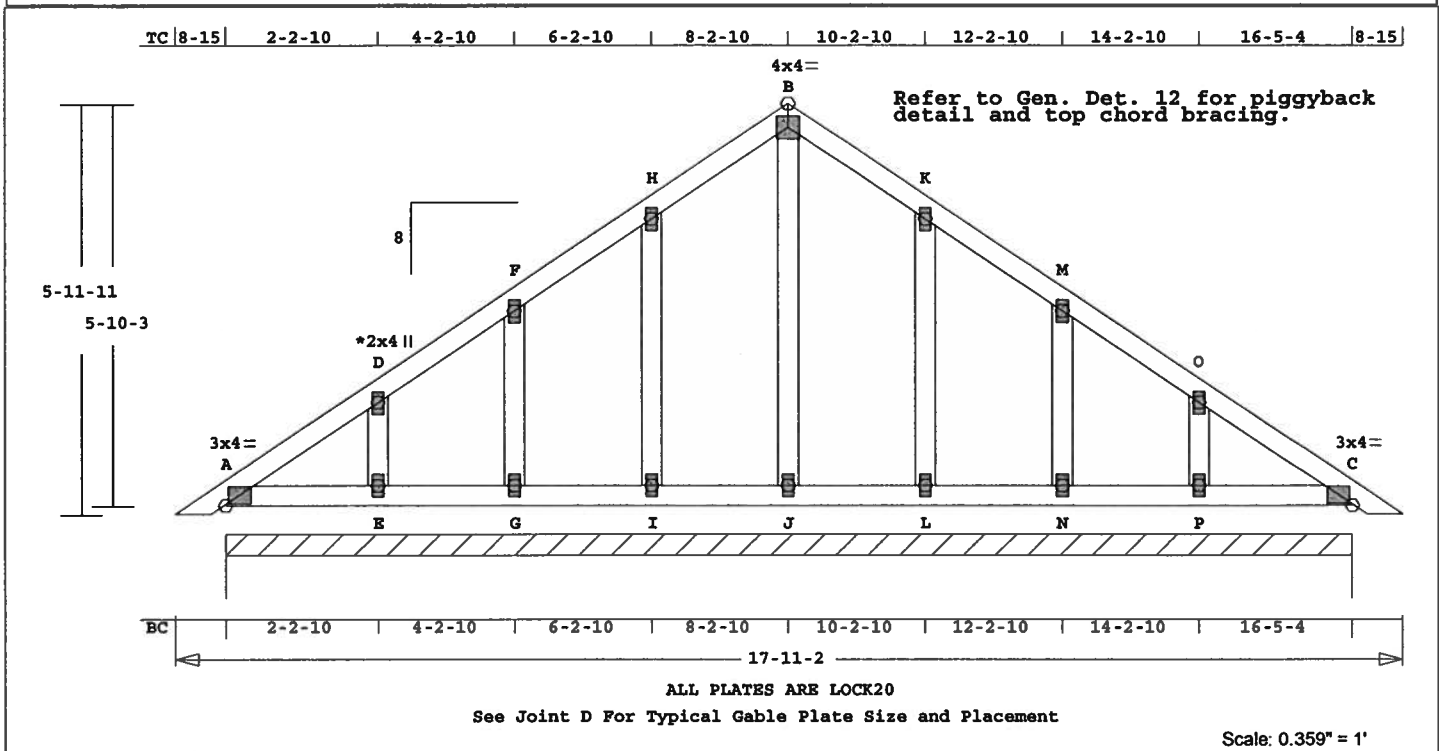
concurrent LL on BC.  
Refer to Gen Det 3 series for  
web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 287 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job FWDEV-HUETT	Mark P2	Quan 1	Type TR	Span 171102	P1-H1 8	Left OH 8-15	Right OH 8-15	Engineering T06012415
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U# J#FWDEV-HUETT HUETT RESIDENCE



Online Plus -- Version 18.5.027  
RUN DATE: 26-JAN-06

CSI -Size- ---Lumber---  
TC 0.04 2x 4 SP-#2  
BC 0.02 2x 4 SP-#2  
GW 0.02 2x 4 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 17-11- 2  
BC Cont. 0- 0- 0 17-11- 2

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd  
Lbs Lbs In-Sx In-Sx  
Cont. Brg 0- 0- 0 to 16- 5- 4  
1410 192 Hz = 114

Membr CSI P Lbs Axl-Csi-Bnd  
-----Top Chords-----  
A -D 0.04 75 C 0.00 0.04  
D -F 0.04 48 C 0.00 0.04  
F -H 0.03 41 C 0.00 0.03  
H -B 0.03 74 T 0.00 0.03  
B -K 0.03 74 T 0.00 0.03  
K -M 0.03 41 C 0.00 0.03  
M -O 0.04 48 C 0.00 0.04  
O -C 0.04 75 C 0.00 0.04  
-----Bottom Chords-----  
A -E 0.02 2 T 0.00 0.02  
E -G 0.02 0 T 0.00 0.02  
G -I 0.02 0 T 0.00 0.02  
I -J 0.02 0 T 0.00 0.02

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 111.4 LBS  
J -L 0.02 0 T 0.00 0.02  
L -N 0.02 0 T 0.00 0.02  
N -P 0.02 0 T 0.00 0.02  
P -C 0.02 2 T 0.00 0.02

-----Gable Webs-----  
E -D 0.01 127 C  
G -F 0.01 118 C  
I -H 0.02 124 C  
J -B 0.02 66 C  
L -K 0.02 124 C  
N -M 0.01 118 C  
P -O 0.01 127 C

LL Defl 0.00" in A -E L/999  
TL Defl 0.00" in A -E L/999  
Shear // Grain in A -D 0.07

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORT: NER 691  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.67  
D LOCK 2.0x 4.0 Ctr Ctr 0.00  
F LOCK 2.0x 4.0 Ctr Ctr 0.00  
H LOCK 2.0x 4.0 Ctr Ctr 0.00  
B LOCK 4.0x 4.0 Ctr Ctr 0.57  
K LOCK 2.0x 4.0 Ctr Ctr 0.00  
M LOCK 2.0x 4.0 Ctr Ctr 0.00  
O LOCK 2.0x 4.0 Ctr Ctr 0.00  
C LOCK 3.0x 4.0 Ctr Ctr 0.67  
E LOCK 2.0x 4.0 Ctr Ctr 0.00  
G LOCK 2.0x 4.0 Ctr Ctr 0.00  
I LOCK 2.0x 4.0 Ctr Ctr 0.00  
J LOCK 2.0x 4.0 Ctr Ctr 0.00  
L LOCK 2.0x 4.0 Ctr Ctr 0.00  
N LOCK 2.0x 4.0 Ctr Ctr 0.00  
P LOCK 2.0x 4.0 Ctr Ctr 0.00

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

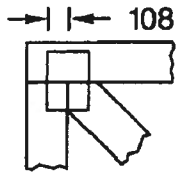
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Refer to Gen Det 3 series for  
web bracing and plating.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as a Main  
Wind-Force Resistance System.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
Zone location: Exterior  
TC Dead Load : 5.0 psf  
BC Dead Load : 5.0 psf  
Max comp. force 127 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



# ROBBINS ENG. GENERAL NOTES & SYMBOLS

## PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

## PLATE SIZE

6 x 8

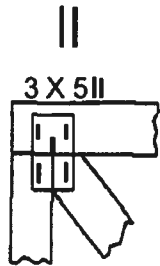
The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

## LATERAL BRACING



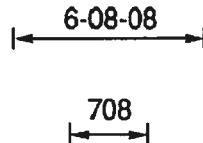
Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

## PLATE ORIENTATION



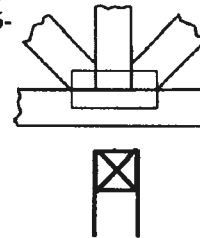
Shown next to plate size, indicates direction of slots in connector plate.

## DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

## BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. These designs were prepared in accordance with "National Design Specifications for Wood Construction" (AF & PA), "National Design Standard for Metal Plate Connected Wood Truss Construction" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by the Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THESE DESIGNS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE TRUSS DESIGN DRAWINGS & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS PLACEMENT DIAGRAM.



Corporate Headquarters

6904 Parke East Blvd  
Tampa, FL 33610-4115  
813-972-1135 Fax: 813-971-6117



**RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR  
FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004  
WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS**

**ALL REQUIREMENTS ARE SUBJECT TO CHANGE**  
**EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ----- 110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

**APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL**

**GENERAL REQUIREMENTS:** Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<b><u>Site Plan including:</u></b> a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<b><u>Wind-load Engineering Summary, calculations and any details required</u></b> Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, $I_w$ , and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf ( $kN/m^2$ ) to be used for the design of exterior component and cladding materials not specfally designed by the registered design professional.
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<b><u>Elevations including:</u></b> a) All sides b) Roof pitch c) Overhang dimensions and detail with attic ventilation
<input checked="" type="checkbox"/>	<input type="checkbox"/>	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	

- d) Location, size and height above roof of chimneys.
- e) Location and size of skylights
- f) Building height
- e) Number of stories
- Floor Plan including:**
  - a) Rooms labeled and dimensioned.
  - b) Shear walls identified.
  - c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
  - d) Show safety glazing of glass, where required by code.
  - e) Identify egress windows in bedrooms, and size.
  - f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
  - g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
  - h) Must show and identify accessibility requirements (accessible bathroom)
- Foundation Plan including:**
  - a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
  - b) All posts and/or column footing including size and reinforcing
  - c) Any special support required by soil analysis such as piling
  - d) Location of any vertical steel.
- Roof System:**
  - a) Truss package including:
    - 1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
    - 2. Roof assembly (FBC 106.1.1.2 )Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
  - b) Conventional Framing Layout including:
    - 1. Rafter size, species and spacing
    - 2. Attachment to wall and uplift
    - 3. Ridge beam sized and valley framing and support details
    - 4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- Wall Sections including:**
  - a) Masonry wall
    - 1. All materials making up wall
    - 2. Block size and mortar type with size and spacing of reinforcement
    - 3. Lintel, tie-beam sizes and reinforcement
    - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
    - 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
    - 6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
    - 7. Fire resistant construction (if required)
    - 8. Fireproofing requirements
    - 9. Shoe type of termite treatment (termicide or alternative method)
    - 10. Slab on grade
      - a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
    - 11. Indicate where pressure treated wood will be placed
    - 12. Provide insulation R value for the following:

- a. Attic space
- b. Exterior wall cavity
- c. Crawl space (if applicable)

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b) Wood frame wall

- 1. All materials making up wall
- 2. Size and species of studs
- 3. Sheathing size, type and nailing schedule
- 4. Headers sized
- 5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
- 6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
- 7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- 8. Fire resistant construction (if applicable)
- 9. Fireproofing requirements
- 10. Show type of termite treatment (termiticide or alternative method)
- 11. Slab on grade
  - a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
  - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
- 12. Indicate where pressure treated wood will be placed
- 13. Provide insulation R value for the following:
  - a. Attic space
  - b. Exterior wall cavity
  - c. Crawl space (if applicable)

☐ N/A

☐

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

**N/A Floor Framing System:**

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

☐ N/A

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**Plumbing Fixture layout**

**Electrical layout including:**

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms
- h) Exhaust fans in bathroom

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**HVAC information**

- a) Energy Calculations (dimensions shall match plans)
- b) Manual J sizing equipment or equivalent computation
- c) Gas System Type (LP or Natural) Location and BTU demand of equipment

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**Disclosure Statement for Owner Builders**

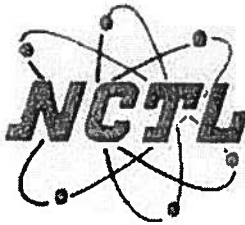
**\*\*\*Notice Of Commencement Required Before Any Inspections Will Be Done Private Potable Water**

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

**THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS**

1. ☒ **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.
2. ☒ **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
3. ☒ **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- N/A 4. ☒ **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- N/A 5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.** A development permit will also be required. Development permit cost is \$50.00
- N/A 6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**
7. ☒ **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

**ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS – PLEASE DO NOT ASK**



## **NATIONAL CERTIFIED TESTING LABORATORIES**

1464 GEMINI BOULEVARD • ORLANDO, FLORIDA 32837  
PHONE (407) 240-1356 • FAX (407) 240-8882

### **STRUCTURAL PERFORMANCE TEST REPORT**

**REPORT NO.:** NCTL-210-2065-4

**TEST DATE:** 07-16-98

**REPORT DATE:** 07-31-98

**EXPIRATION DATE:** 07-31-02

**CLIENT:** Better Bilt Aluminum Products  
704 12<sup>th</sup> Avenue  
Smyrna, TN 37167

**TEST SPECIMEN:** Better Bilt Aluminum Product's Series "420" (Type "OXX") Aluminum Sliding Glass Door. (SGD-C35) (Insulated Glazed) (with and without sill riser)

**TEST SPECIFICATION:** AAMA/NWWDA/101/I.S. 2-97, "Voluntary Specifications for Aluminum Vinyl (PVC) and Wood Windows and Sliding Glass Doors."

### **TEST SPECIMEN DESCRIPTION**

**GENERAL:** The sample tested was a three panel type "OXX" aluminum sliding glass door measuring 15'1-3/4" wide by 6'10-1/8" high overall. The active panels measured 5'0-1/2" wide by 6'9-1/8" high; the fixed panel measured 5'0-7/8" wide by 6'9-1/8" high. Frame and panel members were not thermally broken. A plastic spacer/guide was used at each panel head/stile corner. The fixed panel was secured to the jamb with two (2) 3" long aluminum angle retainers each fastened to the jamb stile with two (2) (# 8 x 3/4") pan head screws. One claw-type door lock assembly was located at 40" from the bottom of each active panel lock stile each with the keeper fastened and secured to the fixed meeting stile and the right jamb stile at lock position with two (2) screws. One adjustable metal double roller assembly was used at each end of the active bottom rails. The frame was of double screw coped corner construction. Panel corners were of single screw at the bottom rail and double screw at the top rail coped corner construction. The interior vertical sill leg employed an extruded aluminum 1-1/8" high extension; an overall height of 2.031. One (1) aluminum panel retainer was fastened at 2" from the end of each active panel bottom rail. One (1) extruded aluminum female panel adapter was fastened to the fixed panel butt stile with five (5) (# 8 x 1/2") screws. One (1) extruded aluminum screen adapter was fastened to the butt stile using five (5) (# 8 x 1/2") screws.

**INSTALLATION:** The main frame was fastened to the wood test buck using forty-eight (48) (# 8 x 1-1/2") FHS. (See fastener diagram)

**GLAZING:** All panels were channel glazed using sealed insulated glass. The overall insulating glass thickness was 5/8" consisting of two (2) pieces of 3/16" clear tempered glass and one (1) air space created by desiccant-filled aluminum spacer system. One (1) extruded aluminum female panel adapter was located at the fixed panel.

**WSTP:** Double strips of centerfin weatherstrip (0.270" high) were located at each jamb, stile, lock stile. A double strip of centerfin weatherstrip (0.180" high) was located at each interlock stile. A double strip of centerfin weatherstrip (0.250" high) was located at each panel top rail. A double strip of side fin weatherstrip (.430" high) was located at each panel bottom rail. An adhesive back polypile dust plug measuring 1-3/16" x 13/16" x 0.420" was located on the head and sill at each end of vertical stile exterior track

**WEEPS:** One weep notch measuring 1-1/2" x leg height was located at each end of each of the interior sill roller leg, exterior sill roller leg and the screen sill roller leg.

**INTERIOR & EXTERIOR SURFACE FINISH:** Non painted aluminum.

**SEALANT:** Frame and panel bottom rail corners were sealed with a small-joint sealant.

**SCREEN:** Two (2) insect screens, one center insect screen measuring 5'0-1/4" wide by 7'11" high; Both were of coped type corner construction. The screen employed fiberglass mesh cloth with a hollow vinyl spline. One roller assembly was located at each end of the bottom rails. One (1) claw-type lock assembly.

### **TEST RESULTS**

<b><u>PARAGRAPH NO.</u></b>	<b><u>TITLE OF TEST</u></b>	<b><u>MEASURED</u></b>	<b><u>ALLOWED</u></b>
** 2.2.19.5.	Operating Force		
	Center Active Panel		
	To Open	20# max.	30#
	In Motion	5 # max.	20#
	Right Active Panel		
	To Open	18 # max.	30 #
	In Motion	3 # max.	20#
** 2.1.2	Air Infiltration 1.57 psf (15 mph)	Pass	0.30 CFM/FT <sup>2</sup>
** 2.1.3	* Water Resistance - 5.0 GPH/FT <sup>2</sup> WTP = 4.50 psf	No Entry	No Entry
2.1.4.2	Uniform Load Structural		
	45.0 psf exterior	0.251"	0.328"
	45.0 psf interior	0.267"	0.328"
**2.2.19.5.2	Deglazing		
	Center Active Panel		
	Top Rail (50#)	10.2% (0.051")	< 100%
	Bottom Rail (50#)	7.8% (0.039")	< 100%
	Left Stile (70#)	6.0% (0.030")	< 100%
	Right Stile (70#)	5.4% (0.027")	< 100%
	Right Active Panel		
	Top Rail (50#)	8.4% (0.042")	< 100%
	Bottom Rail (50#)	8.4% (0.042")	< 100%
	Left Stile (70#)	8.0% (0.040")	< 100%
	Right Stile (70#)	6.2% (0.031")	< 100%



**OPTIONAL PERFORMANCE****\* Water Resistance - (5.0 GPH/FT<sup>2</sup>)**

WTP = 5.25 psf

No Entry

No Entry

Note: At this point in testing an additional sill riser was attached to the existing main sill's interior vertical leg.

The following results were obtained:

**\*\* 4.3****\* Water Resistance - (5.0 GPH/FT<sup>2</sup>)**

WTP - 6.00 psf

No Entry

No Entry

**4.4.2****Uniform Structural Load**

52.5 psf exterior

0.301"

0.328"

52.5 psf interior

0.317"

0.328"

**TEST COMPLETED: 07-16-98**

\*Test performed with and without insect screen.

\*\* Reference parent test report no. NCTL-210-2065-1 for test results and qualifications.

In addition, Better Bilt Aluminum Products' Series "430" and "440" also receive an SGD-C35 rating being identical in panel construction and interior sill heights.

This test specimen meets the performance criteria level of SGD-C35 of the AAMA/NWWDA/101/I.S. 2-97 specification.

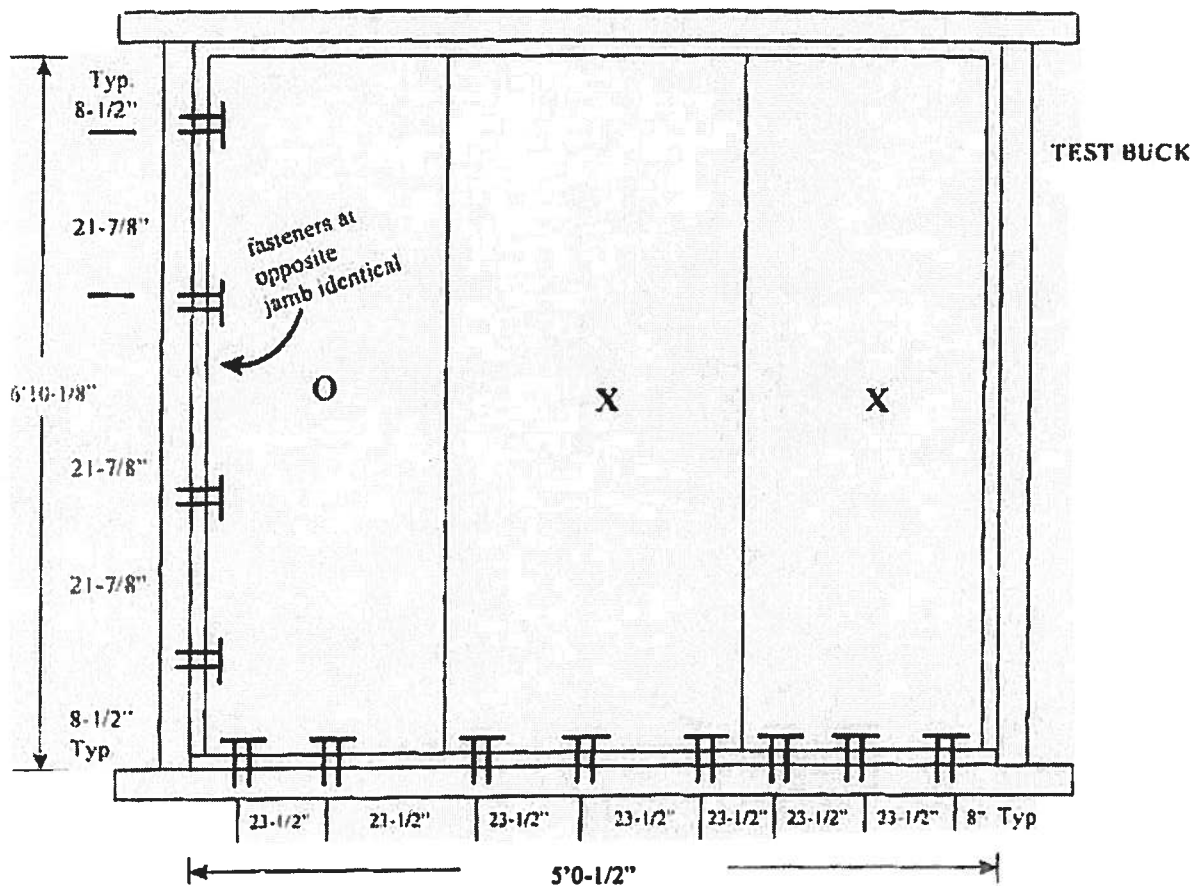
Detailed drawings were available for laboratory records and compared to the test specimen at the time of this report. A copy of this report along with representative sections of the test specimen will be retained by NCTL for a period of four (4) years. The results obtained apply only to the specimen tested.

NATIONAL CERTIFIED TESTING  
LABORATORIES, INC.

MICHAEL E. LANE  
Division Manager

MEL/K  
8/5/98

# FASTENER LOCATIONS



The test specimen was fin mounted to the test buck using forty-eight (48) (# 8 x 1-1/2") screws at locations shown.

II - Denotes double row (2) at each location.

NOTE: Head fasteners identical to sill both jamb fastening identical

## **METRO DADE COUNTY REQUIRED**

**NATIONAL CERTIFIED TESTING LABORATORIES INC.**

**JOB NO.: NCTL-210-2065-3 & 4**

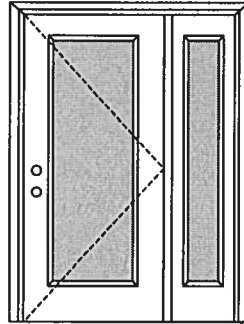
**COMPANY: BETTER BILT**

**TEST DATE: 07-15-98**



## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



Single Door with 1 Sidelite  
Maximum unit size = 6'0" x 6'8"

**Design Pressure**  
**+40.5/-40.5**

Limited water unless special threshold design is used.

**Large Missile Impact Resistance**

**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447A  
and COP/Test Report Validation Matrix  
#3026447A-001 provides additional  
information - available from the ITS/WH  
website ([www.itswh.com](http://www.itswh.com)), the  
Masonite website ([www.masonite.com](http://www.masonite.com))  
or the Masonite technical center.

**Note:**

Units of other sizes are covered by this  
report as long as the panels used do not  
exceed 3'0" x 6'8".

### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0003-02 or  
MAD-WL-MA0006-02 and MAD-WL-MA0041-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0003-02.

### APPROVED DOOR STYLES:

#### 1/4 GLASS:



100 Series



133, 135 Series



136 Series

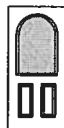


680 Series



822 Series

#### 1/2 GLASS:



105 Series\*



106, 160 Series\*



129 Series\*



200 Series\*



12 R/L, 23 R/L, 24 R/L  
Series\*



107 Series\*



108 Series



304 Series

\*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

**Johnson**  
**EntrySystems**

June 17, 2002  
Our continuing program of product improvement makes specifications, design and product  
detail subject to change without notice.



Exclusively from

**Masonite**  
Masonite International Corporation

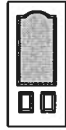
## WOOD-EDGE STEEL DOORS

### APPROVED DOOR STYLES:

#### 3/4 GLASS:



404 Series



410 Series

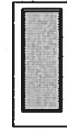


450 Series

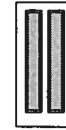
#### FULL GLASS:



109 Series



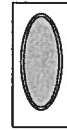
114, 120, 122  
Series



152 Series



149 Series



300 Series

### APPROVED SIDELITE STYLES:



680 Series



129 Series



200 Series



12R, 12L, 23R,  
23L, 24R, 24L  
Series



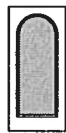
450 Series



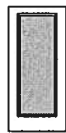
152 Series



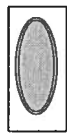
149 Series



109 Series



120, 122 Series



300 Series

### CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.elisemko.com](http://www.elisemko.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

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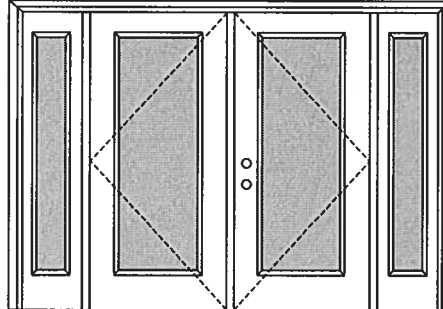


Exclusively from

Masonite International Corporation

## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Note:**  
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door with 2 Sidelites  
Maximum unit size = 12'0" x 6'8"

**Design Pressure**  
**+40.5/-40.5**

Limited water unless special threshold design is used.

**Large Missile Impact Resistance**

**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0005-02 or MAD-WL-MA0008-02 and MAD-WL-MA0041-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0005-02.

### APPROVED DOOR STYLES:

#### 1/4 GLASS:



100 Series



133, 135 Series



136 Series

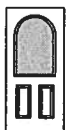


680 Series



822 Series

#### 1/2 GLASS:



105 Series\*



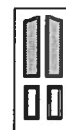
106, 160 Series\*



129 Series\*



200 Series\*



12 R/L, 23 R/L, 24 R/L Series\*



107 Series\*



108 Series



304 Series

\*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

## WOOD-EDGE STEEL DOORS

### APPROVED DOOR STYLES: 3/4 GLASS:



404 Series



410 Series



450 Series

### FULL GLASS:



109 Series



114, 120, 122  
Series



152 Series



149 Series



300 Series

### APPROVED SIDELITE STYLES:



680 Series



129 Series



200 Series



12R, 12L, 23R,  
23L, 24R, 24L  
Series



450 Series



152 Series



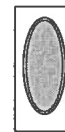
149 Series



109 Series



120, 122 Series



300 Series

### CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

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Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. – License Number 56533



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June 17, 2002  
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Masonite International Corporation



**AAMA/NWWDA 101/I.S.2-97  
TEST REPORT**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

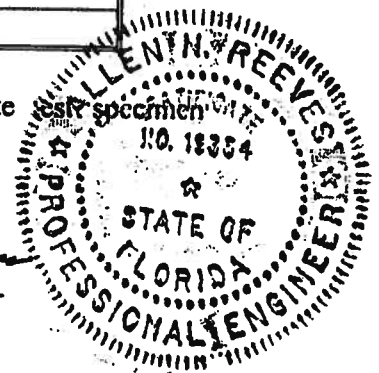
**SERIES/MODEL: 650**

**TYPE: Aluminum Triple Single Hung Window**

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft <sup>2</sup>
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

*Allen H. Reeves*  
7 JUNE 2002





## **AAMA/NWWDA 101/I.S.2-97 TEST REPORT**

Rendered to

MI HOME PRODUCTS, INC.  
P.O. Box 370  
650 West Market Street  
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01  
Test Date: 05/13/02  
And: 05/16/02  
Report Date: 06/05/02  
Expiration Date: 05/16/06

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

### **Test Specimen Description:**

**Series/Model:** 650

**Type:** Aluminum Triple Single Hung Window

**Overall Size:** 9' 3-1/2" wide by 5' 11-11/16" high

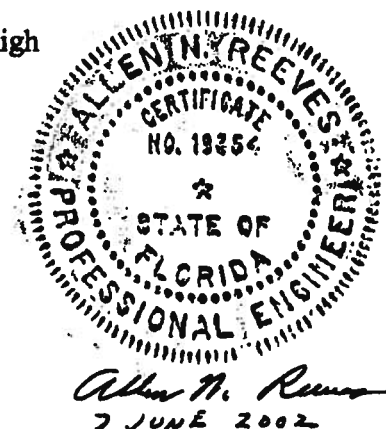
**Active Sash Size (3):** 3' 0-1/4" wide by 2' 10-3/4" high

**Fixed Daylight Opening Size (3):** 2' 8-1/4" wide by 2' 9-1/8" high

**Screen Size (3):** 2' 9-1/8" wide by 2' 11" high

**Finish:** All aluminum was painted white.

130 Derry Court  
York, PA 17402-9405  
phone: 717.764.7700  
fax: 717.764.4129  
www.archtest.com



**Test Specimen Description: (Continued)**

**Glazing Details:** The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

**Weatherstripping:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



*Allen N. Reeves*  
7 JUN 2002

**Test Specimen Description: (Continued)**

**Hardware:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

**Drainage:** Sloped sill

**Reinforcement:** No reinforcement was utilized.

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

**Test Results:**

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft <sup>2</sup>	0.3 cfm/ft <sup>2</sup> max.

**Note #1:** The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00)  
(with and without screen)  
WTP = 2.86 psf

No leakage

No leakage

*Allen N. Reeves*  
7 JUNE 2002






**Test Results: (Continued)**


<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
*Exceeds L/175 for deflection, but meets all other test requirements.			
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

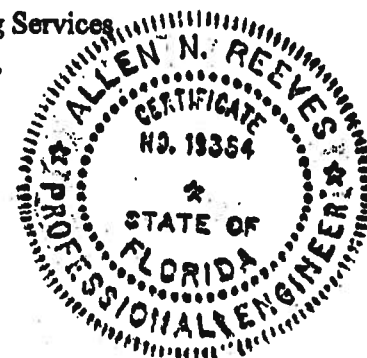
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

  
Mark A. Hess  
Technician

MAH:nlb  
01-41641.01

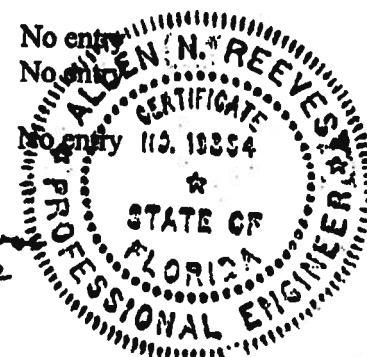
  
Allen N. Reeves, P.E.  
Director - Engineering Services  
7 JUNE 2002



**Test Results: (Continued)**

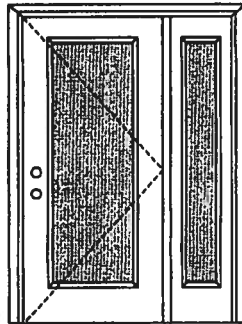
<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.15" 0.29"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" 0.01"	0.29" max. 0.29" max.
2.2. .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs  Right sash, meeting rail Right sash, bottom rail Middle sash, meeting rail Middle sash, bottom rail Left sash, meeting rail Left sash, bottom rail  In remaining direction at 50 lbs  Right sash, right stile Right sash, left stile Middle sash, right stile Middle sash, left stile Left sash, right stile Left sash, left stile	  0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25%  0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12%	  0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%  0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)  Type: A Grade: 10  Lock Manipulation Test  Test A1 through A5 Test A7  Lock Manipulation Test	    No entry No entry No entry  No entry	    No entry No entry No entry  No entry

*Allen N. Reeves*  
7 JUNE 2002



## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



Single Door with 1 Sidelite  
Maximum unit size = 6'0" x 6'8"

#### Design Pressure

**+40.5/-40.5**

Limited water unless special threshold design is used.

#### Large Missile Impact Resistance

**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

#### Note:

Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0003-02 or MAD-WL-MA0006-02 and MAD-WL-MA0041-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0003-02.

### APPROVED DOOR STYLES:

#### 1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

#### 1/2 GLASS:



105 Series\*



106, 180 Series\*



129 Series\*



200 Series\*



12 R/L, 23 R/L, 24 R/L Series\*



107 Series\*



108 Series



304 Series

\*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

**Johnson**  
**EntrySystems™**

June 17, 2002  
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

**PREMDOR Collection**  
Premium Quality Doors

Exclusively from  
**Masonite**  
Masonite International Corporation

## WOOD-EDGE STEEL DOORS

### APPROVED DOOR STYLES:

#### 3/4 GLASS:



404 Series



410 Series



450 Series

#### FULL GLASS:



109 Series



114, 120, 122  
Series



152 Series



149 Series



300 Series

### APPROVED SIDELITE STYLES:



660 Series



129 Series



200 Series



12R, 12L, 23R,  
23L, 24R, 24L  
Series



450 Series



152 Series



149 Series



109 Series



120, 122 Series



300 Series

### CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Johnson**  
**EntrySystems**

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# ELK



**PRESTIQUE®  
HIGH DEFINITION®**



**RAISED PROFILE®**

## Prestique Plus High Definition and Prestique Gallery Collection¹

Product size	134" x 39"	30-year limited warranty period:
Exposure	36"	5-7 years non-prorated coverage for
Pieces/Bundle	16	shingles and application labor with
Bundles/Square	4/98.5 sq.ft.	prorated coverage for remainder of
Squares/Pallet	11	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 80 mph, extended
		110 mph***

## Raised Profile

Product size	134" x 39"	30-year limited warranty period:
Exposure	36"	5-7 years non-prorated coverage for
Pieces/Bundle	22	shingles and application labor with
Bundles/Square	3/100 sq.ft.	prorated coverage for remainder of
Squares/Pallet	16	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 70 mph.

## Prestique I High Definition

Product size	134" x 39"	40-year limited warranty period:
Exposure	36"	5-7 years non-prorated coverage for
Pieces/Bundle	16	shingles and application labor with
Bundles/Square	4/98.5 sq.ft.	prorated coverage for remainder of
Squares/Pallet	14	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 80 mph, extended
		90 mph***

## HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/FLX™	Vented RidgeCrest™ w/FLX™
Size: 12" x 12"	Size: 13" x 13"
Exposure: 6"	Exposure: 9"
Pieces/Bundle: 48	Pieces/Box: 28
Coverage: 4 Bundles = 100 linear feet	Coverage: 5 boxes = 100 linear feet

## Prestique High Definition

Product size	134" x 39"	30-year limited warranty period:
Exposure	36"	5-7 years non-prorated coverage for
Pieces/Bundle	22	shingles and application labor with
Bundles/Square	3/100 sq.ft.	prorated coverage for remainder of
Squares/Pallet	16	limited warranty period, plus an
		option for transferability*. 5-year
		limited wind warranty*. Wind
		Coverage: standard 80 mph.

## Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
18 Pieces/Bundle
1 Bundle = 120.50 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shetwood, Sablewood, Hickory, Barwood, Forest Green, Wedgewood, Birchwood, Sandalwood, Gallery Collection: Balsam Forest™, Weathered Sage™, Stone Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard™ treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 587) and Class "A" Fire Ratings (UL 780); and ASTM Specifications D 3078, Type-I; D 3161, Type-C; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

\*See actual limited warranty for conditions and limitations.

\*\*Shingles meeting UL 587, the seven year non-prorated Unlabeled Coverage Period apply only when a full Elk Kit of System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for each product. A full Elk Kit and system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rakes and eave edges, an Elk ventilation system, and Elk AD-Climate Self-Adhesive Underlayment in all valleys. Additionally, Elk AD-Climate Self-Adhesive Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, NC, SC, GA, FL, and HI. \*\*\*For a Limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grand, at least six (6) properly placed nails and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

## SPECIFICATIONS

**Basic Work** includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

**Preparation or Rear Deck:** Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/4" (19.0mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.3mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NDT approved substrates for Elk shingles. Consult Elk Field Service for application recommendations near other decks and other decks.

**Materials:** Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 20 asphalt-saturated felt underlayment. For Low slopes/4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm), use two pieces of underlayment overlapped a minimum of 18". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Turquoise plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.384.SPEC (7732) or e-mail specinfo@elkcorp.com.

(Typical consumption for American passenger car)

(Typical consumption for American passenger car)



These application instructions are the only

These application instructions are the minimum required to meet EPA's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will EPA accept application requirements that are less than those printed here. Stringers should not be jammed tightly together. All entries should be properly fastened. Note: It is not necessary to remove tape on back of stringer.

Good dentists should do this:

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 2 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

**Drake Anderson**

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt, Elk Varnashield® or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two rolls of underlayment overlapping a minimum of 18". Begin by starting a 18" wide strip of underlayment placed along the eaves. Place a 3rd 24" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

or standard slope (4/12 to less than 21/12), use coated roll roofing.

no less than 80 pounds over the felt underlayment extending on the eave edge to a point at least 24" beyond the inside wall of a living space below or one layer of a self-adhered eave and string membrane.

low slope (2/12 up to 4/12), use a continuous layer of asphalt side cement between the two piles of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of living space below or one layer of a self-adhered membrane and

submit the Ed Technical Services Department for application  
efficiency over other decks and other areas.

### AN ELK STARTER COULD

AN ELK STARTER STRIP ON THE HEADLAP OF A STRIP  
ABLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE  
E. With at least 3" turned from the end of the first shingle,  
at the rake edge overhanging the eave and rake edges 1/2  
". Fasten 2" from the lower edge and 1" from each side.

at rate and a

at rake and eave course with full shingles laid flush with starter course. Shingles may be applied with a course break of 48" on the roof.

the recent

the second course of shingles with respect to the first by  
exactly 6". Other effects are approved if greater than 4".

the next con-

the next course by 5" with respect to the second course, consistent with the original offset.

the city and

the rule and continue with full shingles across roof.

application as shown for

application as shown for second, third, and fourth  
Do not rack shingles straight up the roof. Offsets may be  
around valleys and penetrations.

**Open waves and closed**

Open, woven and closed end valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

For ridge constructs—see

For ridge construction with recommendations Class "A" Z-Ridge or See-A-Ridge® with formula *FLX* or RidgeCrest® with *FLX* (See ridge package for installation instructions). Vented RidgeCrest or tab shingles are also approved.

On the other

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the string to the double thickness common hand arm. For strings without fastener line, nails or staples must be placed between and/or on the content data.

Min. 24" Corrosion resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for re-roofs. In cases where you are applying shingles to a roof that has accepted overhang, for new roofs only, 2 1/2" ring shank nails are used to be used from the eave's edge to a point up the roof that past the outside wall line. 1" ring shank nails allowed for re-roof.

PLEB; Corrosive resisters, 10-gauge minimum, crown width minimum of 18/18. Note: An improperly adjusted staple gun can break in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

eners should be long enough to obtain 3/4" deck penetration  
concentration through deck, whichever is less. This product  
to the requirements of the IRC 2003 code when fastened with  
la.

not faster than is with

fastening is critical to the performance of the roof. For a reroofing job (or 21/12) use six fasteners per shingle. Use fasteners in the fastener area 1" from each side edge with minimum four fasteners equally spaced along the length of shingle thickness (laminated) area. Only fastening methods following the above instructions are acceptable.

## United Way of America

1. **Limited Wind Warranty, all Prestige and Raised Profile™** shingles must be applied with 4 properly placed fasteners, or in case of standard applications, 8 properly placed fasteners single.
2. **Limited Wind Warranty up to 110 MPH for Practice**ry Collection or Practice Plus or 80 MPH for Practice I, shingles must be applied with 6 properly placed nails per sq. SHINGLES APPLIED WITH STAPLES WILL NOT APPLY FOR THIS ENHANCED LIMITED WIND WARRANTY.
3. **Eik Starter Strip** shingles must be applied at the eaves and edges to qualify Prestige Plus, Practice Gallery and Practice I shingles for this enhanced Limited Warranty. Under no circumstances should the Eik or the Eik Starter Strip overhang the eaves or rake more than 3/4 of an inch.

A minimum of four fasteners must be used.

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along - and through - the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Eik has specified. All Prestique and Raised Profile shingles have a U.L.C. Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

**CAUTION TO VEHICLELEASER:** Careless and improper storage or handling can harm fiberglass chingies. Keep these chingies completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. **DO NOT DOUBLE STACK.** Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

**ELK**   
The Premium Choice.  
[www.elkcorp.com](http://www.elkcorp.com)

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NOTICE OF COMMENCEMENT FORM  
COLUMBIA COUNTY, FLORIDA

\*\*\*THIS DOCUMENT MUST BE RECORDED AT THE COUNTY  
CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.\*\*\*

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 03-75-16-04118-013

1. Description of property: (legal description of the property and street address or 911 address)  
492 SW Fielding Way, Fort White, Fla 32038  
NE 1/4 of Section 3, Township 6 South, Range 16 East
2. General description of improvement: Framed Detached Garage
3. Owner Name & Address Gerald C. & Delores S. Hyett  
8672 Calista Ct Orlando Fla 32825 Interest In Property 100%
4. Name & Address of Fee Simple Owner (if other than owner): Same
5. Contractor Name Columbia Home Builders, Inc. Phone Number 752-4071  
Address 545 SW Hugo Ln, Lake City, Fla. 32026
6. Surety Holders Name None Phone Number \_\_\_\_\_  
Address \_\_\_\_\_  
Amount of Bond \_\_\_\_\_
7. Lender Name None Phone Number \_\_\_\_\_  
Address \_\_\_\_\_
8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 713.13 (1)(a) 7; Florida Statutes:  
Name Hay R. Hawkins Phone Number 407-3991  
Address 6855 SW Elm Church Rd Fort White, Fla 32038
9. In addition to himself/herself the owner designates Hay R. Hawkins of \_\_\_\_\_  
to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -  
(a) 7. Phone Number of the designee 316-497-3991
10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) 1 year

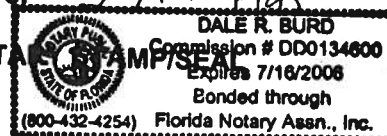
NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

[Signature]  
Signature of Owner

Sworn to (or affirmed) and subscribed before  
day of 27th Feb, 2006

NOTARY



[Signature]  
Signature of Notary

Inst: 200604666 Date: 02/27/2006 Time: 09:46  
A. J. DC, P. Dewitt Cason, Columbia County B: 1075 P: 830

29153





**Consultants In: Geotechnical Engineering •  
Environmental Sciences • Construction Materials Testing**

# REPORT ON IN-PLACE DENSITY TESTS

Permit #000024153

Huey job

DEPTH OF TEST: 0-1'

DATE TESTED: 277-06

REMARKS:

TECH. ED



# New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525  
(exp. 10/31/2005)

This form is completed by the licensed Pest Control Company.

**Public reporting burden** for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

21453

## Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.  
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055  
Company Business License No. JB109476 Company Phone No. 386-755-3611  
FHA/VA Case No. (if any) \_\_\_\_\_

## Section 2: Builder Information

Company Name: Ft. White Developments Company Phone No. \_\_\_\_\_

## Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 4925 W. Fielding Way  
Ft. White, FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other \_\_\_\_\_  
Approximate Depth of Footing: Outside 0 Inside 0 Type of Fill Dir

## Section 4: Treatment Information

Date(s) of Treatment(s) 2-28-06  
Brand Name of Product(s) Used Expro 1.2  
EPA Registration No. 53453-92  
Approximate Final Mix Solution % 0.25%  
Approximate Size of Treatment Area: Sq. ft. 960 Linear ft. 0 Linear ft. of Masonry Voids 0  
Approximate Total Gallons of Solution Applied 96  
Was treatment completed on exterior? ☐ Yes ☒ No  
Service Agreement Available? ☒ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) \_\_\_\_\_

Comments \_\_\_\_\_

Name of Applicator(s) Steve Brennan Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature [Signature] Date 2-28-06

**Warning:** HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPCA-99-B (04/2003)