

WIND ANALYSIS -- 120MPH Wind Velocity or as interpolated

2023 8th edition Florida Building Code

Calculations as per Section 1609ASCE 7-22

Prepared By
James Zaleski PE 51544

Prepared by (print legibly): James Zaleski
Design Professional FL Lic. #: 51544

Importance factor: 1.0 Building Category: ENCLOSED
Wind Exposure (s): B Risk Category II
Internal Pressure Coefficient +/- .18

THE:	PLEASANTVIEW A
FOR:	FOYE AND KELLY JACKSON
	FL COLUMBIA - FL

DESIGN CRITERIA:

2023 FLORIDA BUILDING CODE -- BUILDING, 8TH EDITION

2018 WFCM (WOOD FRAME CONSTRUCTION MANUAL)

ASCE 7-22: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.

BUILDING OCCUPANCY CATEGORY= II

POSTS WILL UTILIZE SIMPSON MODEL ABU FOR THE POST BASES AND AN AC OR LCE4 AS APPLICABLE OR BETTER, UNLESS OTHERWISE NOTED.

Mean Roof Height 13.67 **Zone Length** 6.0 feet

MAX OVERHANG 1.5 FT MAX

SEE ATTACHED

Roof Slope =7/12



TRUSS SPAN/LOCATION HURRICANE CLIPS

HC MODEL-1 Simpson H-10A IN ALL AREAS 2 SDWC15600 SCREWS EQUAL 1-H10A

ROOF SHEATHING MATERIAL 7/16" OSB

NAILING -8D RING SHANK

NAILING PATTERN

EDGES-

6" O.C **FIELD - 6" O.C.**

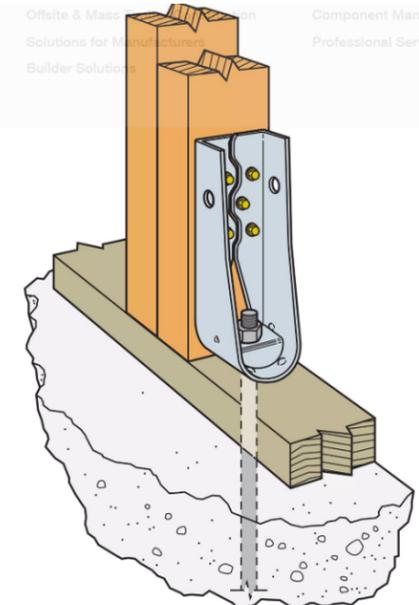
Plan May Be Mirrored at Contractors Option

James Zaleski P.E. #51544 2305 haverhill rd tall fl 32312 ph 850-766-7778

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Offsite & Mass Production
Solutions for Manufacturers
Builder Solutions

Component Manufacture
Professional Services



Typical HDU2-SDS2.5 Installation

Wall Exterior Panel – Sheath with 7/16 OSB

– 2 X 4 STUDS AT 16” O.C. UP TO 10 FEET

– 2 X 4 STUDS AT 12” O.C. UP TO 12 FEET

ALL WALLS OVER 10 FEET TO HAVE 2 ROWS OF BLOCKING

SIMPSON SP1 AT BASE OF STUDS AND SIMPSON SP2 AT TOP OF STUDS AT 32” O.C (SPH STRAPS MAY ALSO BE USED)

MIN NAIL PENETRATION – 1-1/2”

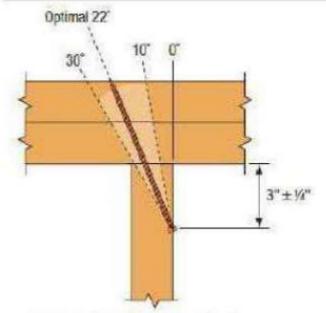
Nail Type 8 D

Edge Nail Spacing 6” o.c

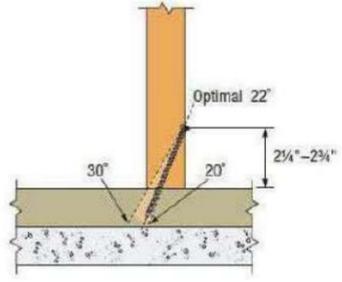
Intermediate Nail Spacing 6” o.c

BEAM TO WALL/CORNER CONNECTION – POCKET AND NAIL INTO WALL W/ (10) 16 PENNY NAILS, STRAP W/ SIMPSON H7Z.

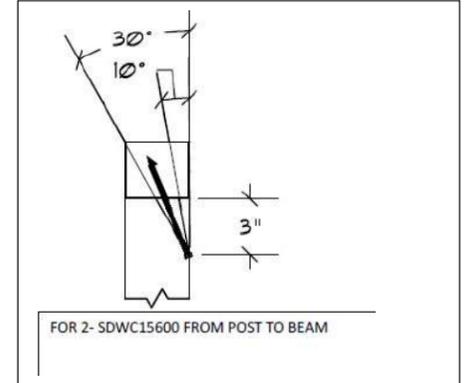
10” ‘ANCHOR’ BOLT MINIMUM 32” O.C. w/ 3 GA. x 2” x 2” BEARING PLATE And 6” from each corner TO ATTACH SILL PLATE TO WALL



Stud-to-Top Plate Connection
(This application requires SDWC15600)



Stud-to-Bottom Plate Connection Over Concrete/Masonry Foundation
(This application requires SDWC15450)



FOR 2- SDWC15600 FROM POST TO BEAM



IN LIEU OF STRAPPING STUDS SDWC SCREWS MAY BE USED



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COMPONENTS AND CLADDING PRESSURES: (WORST CASE LOADS MAY BE USED)

COMPONENTS AND CLADDING

ZONE per

SEE ATTACHED

MAIN WIND FORCE RESISTING SYSTEMS (MWFRS) (WORST CASE LOADS MAY BE USED)

SEE ATTACHED

All Load Bearing and Shear Walls To be Framed as per FBC

Alternative Hurricane Clips are acceptable as long as they meet the requirements shown

See Attached header schedule

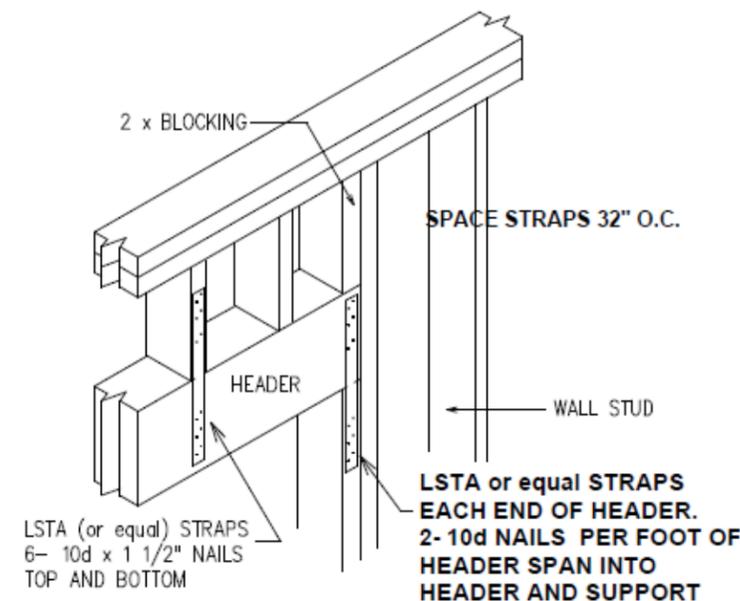
PROVIDE GABLE END BRACING DETAIL, all vaulted or high ceilings shall be balloon framed to the ceiling diaphragm.

NOTES: PLEASE READ & complete all blanks!!!!

1. See floor plan for wall bracing locations or circle 100% if structural sheathing is required on all exterior walls, with the nailing pattern indicated above.
2. There are , there are not interior shear walls, locate interior shear walls on plan.
3. Gable ends required to be sheathed with same material as shear wall? Yes or **(No)** circle one)
4. Wall sheathing used in lieu of vertical straps: Nailing @ o.c. along top & bottom plates
5. Provide detail for 2 story bldgs showing continuous load path between 2nd floor stud & 1st floor studs.
6. Provide additional information for column base & column/beam connection if required for porches.
7. Provide calculations or documentation to substantiate method used as an attachment to this form(SEE PLANS)

Instructions:

1. The form should be completed & signed, sealed & dated by a Fla. licensed engineer or architect.
2. Since more than one methodology for determination of wind forces is permitted under Section 1609ASCE7-22, to comply with State Building Codes a space has been provided to indicate method used.
3. Wind Analysis Forms submitted & permitted to be used as Master Plans will be for identical plans only, minor deviations such as door swings. Any deviation from the exterior form, opening sizes or locations will not be permitted unless noted by the design professional.

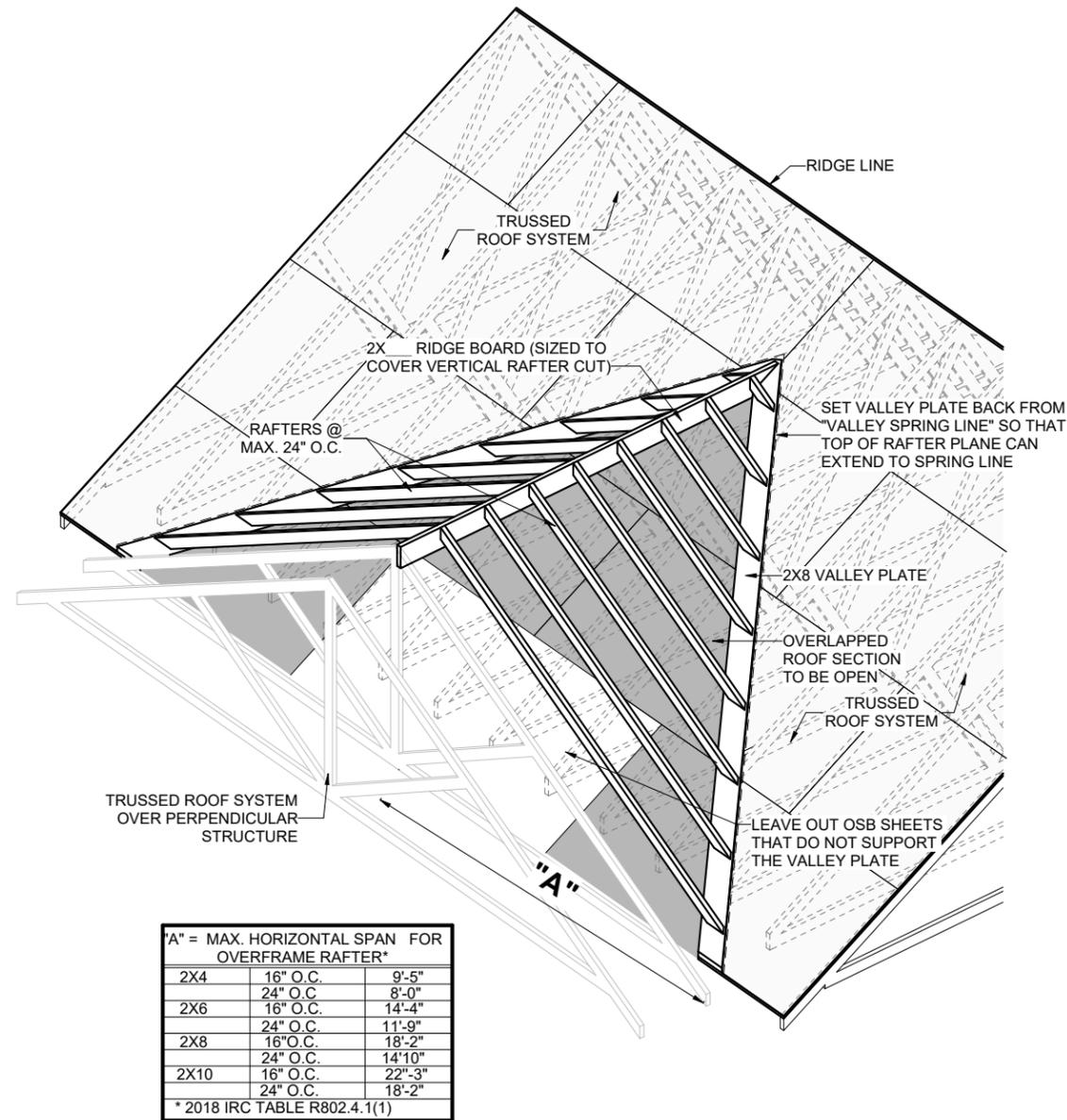


HEADER CONN.



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2 ROOF OVERFRAMING DETAIL @ INTERSECTION
1/4" = 1'-0"

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

PRELIMINARY CONSTRUCTION PLANS

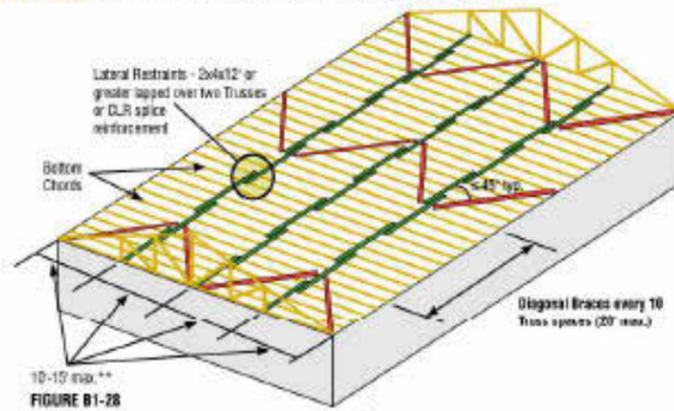
PROPERTY OF: © COPYRIGHT - 2025	SHEET # S-3	THE: PLEASANTVIEW A	REVISION SCHEDULE
		FOR: FOYE AND KELLY JACKSON	REV # DESCRIPTION DATE
DRAWN BY: LUCAS N.	JOB # 66-25-031	AREAS: FIRST FLOOR HEATED 1,667 SF	1,667 SF
CHECKED BY: K.S.	2X6 EXTERIOR WALLS	FRONT PORCH 144 SF	144 SF
PRINTED: 12/3/2025 9:56:33 AM	FOUNDATION TYPE: STEMWALL SLAB	SCREEN PORCH 176 SF	176 SF
		TOTAL UNDER ROOF 1,987 SF	1,987 SF
	OFFICE: VALDOSTA SALES		
	FL COLUMBIA - FL		
	SOLD BY: B. DIMICK		
DETAILS & SECTIONS			

TRUSS BRACING SUMMARY

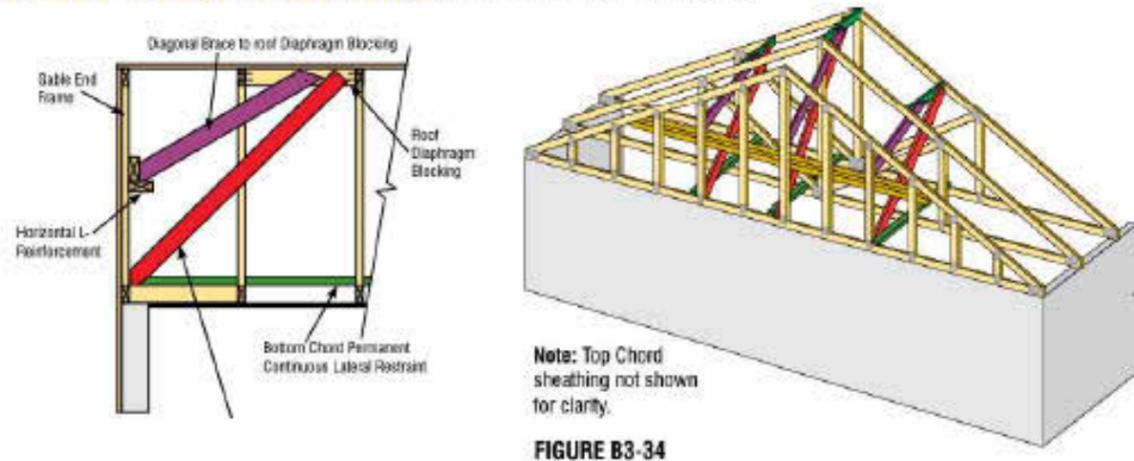
Five to Thrive

#1 CLR - Continuous Lateral Bracing on bottom chord 10' on center

#2 DLR - Diagonal Lateral Bracing on bottom chord - No more than 20' in between

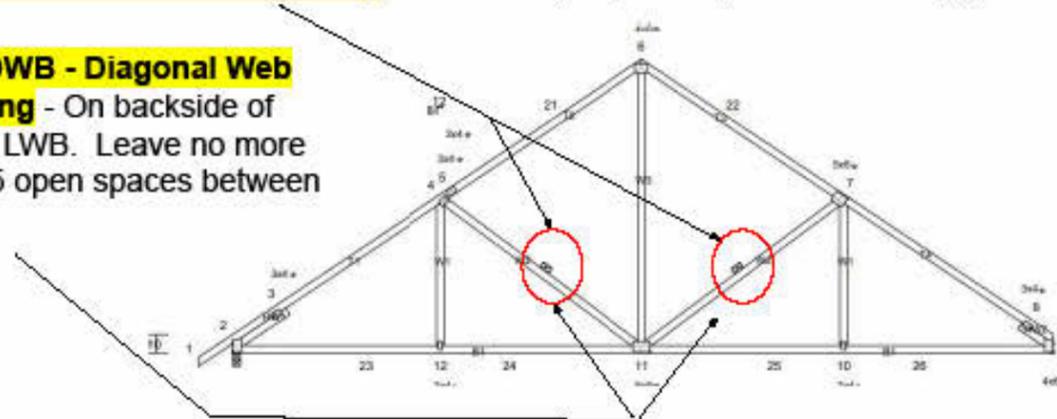


#3 GEB - Gable End Bracing up to ridge or top chord



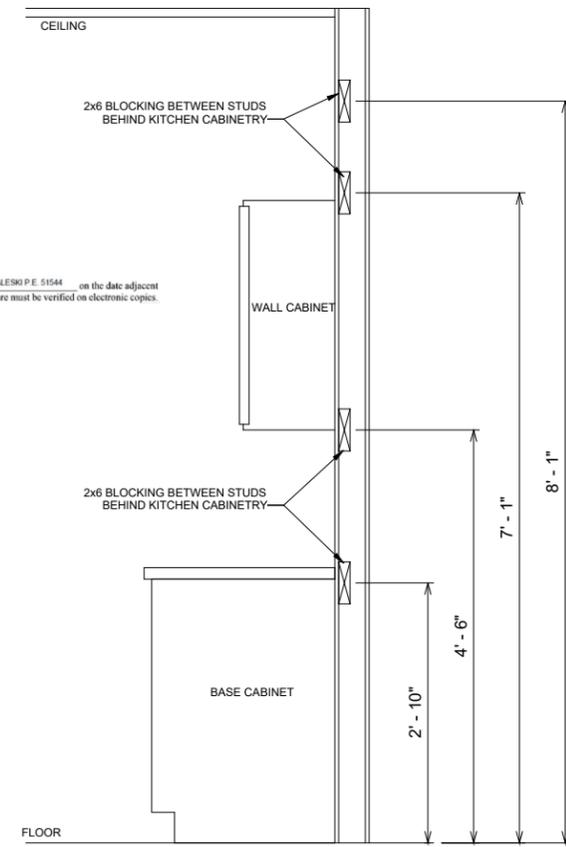
#4 - LWB - Lateral Web Bracing - Truss Design Specific (See Truss Design)

#5 - DWB - Diagonal Web Bracing - On backside of every LWB. Leave no more than 5 open spaces between DWB

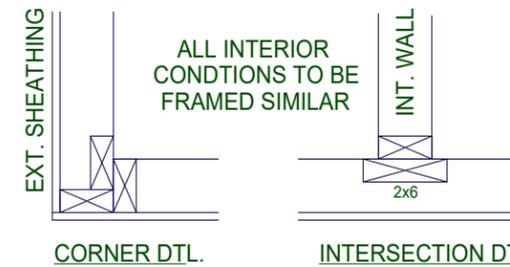


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1 **KITCHEN CABINET WALL BLOCKING**
1/2" = 1'-0"



TYPICAL FRAMING DETAILS

2 **43.2 CORNER T FRAMING DETAIL**
1" = 1'-0"

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

PRELIMINARY CONSTRUCTION PLANS

REVISION SCHEDULE		REV #	DESCRIPTION	DATE
AREAS:	FIRST FLOOR HEATED	1,667 SF		
	FRONT PORCH	144 SF		
	SCREEN PORCH	176 SF		
	TOTAL UNDER ROOF	320 SF		
		1,987 SF		

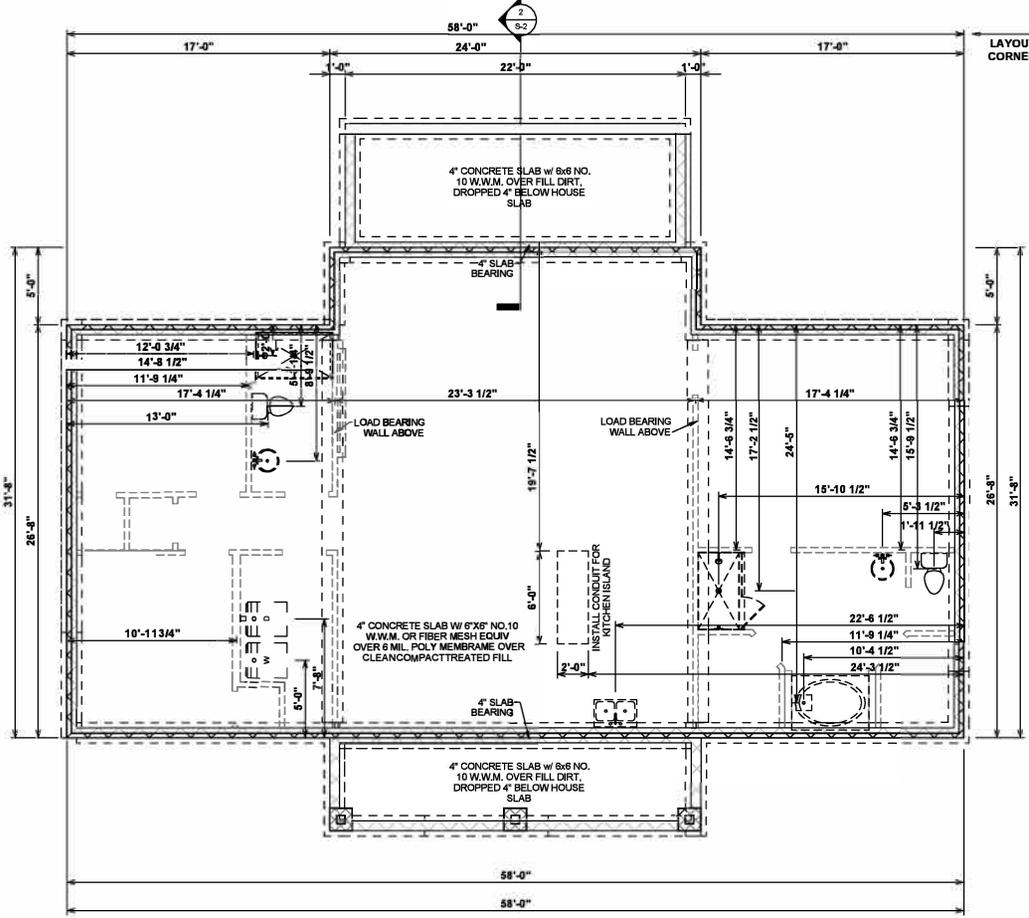
THE:	PLEASANTVIEW A
FOR:	FOYE AND KELLY JACKSON
OFFICE:	COLUMBIA - FL
SOLD BY:	B. DIMICK
OFFICE:	VALDOSTA SALES

JOB #	66-25-031
FOUNDATION TYPE:	STEMWALL SLAB
FOUNDATION:	2x6 EXTERIOR WALLS

PROPERTY OF:	AMERICA'S HOME PLACE
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DRAWN BY:	LUCAS N.
CHECKED BY:	K.S.
PRINTED:	12/31/2025 9:56:33 AM

SHEET #	S-7
SCALE:	As indicated



1 FOUNDATION PLAN - STEMWALL
 1/8" = 1'-0"



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FOUNDATION NOTES:

- TURN OUT REBAR AT THE POWER METER FOR ELECTRICAL GROUNDING AS REQUIRED BY CODE
- WATER INLET & SEWER OUTLET LOCATIONS TO BE FIELD LOCATED BY AHP
- PORCH, STOOP & DECK HANDRAILS NOT INCLUDED WITH SLAB FOUNDATION AND RAILING IS A FORCED OPTION WHEN THE PORCH IS OVER 30" HIGH
- SEE ENGINEERING PLAN DETAIL FOR FINAL CONSTRUCTION METHOD

PORCH, STOOP, & DECK HANDRAILS NOT INCLUDED w/ SLAB FOUNDATION

NOTE: RAILINGS ARE A FORCED OPTION WHEN PORCH IS OVER 30" HIGH

TERMITE PROTECTION

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION.

FOUNDATION/CONCRETE NOTES

1. THE FOUNDATION PLAN INCLUDED HEREIN IS FOR SUITABLE SOILS. IF BUILDER FINDS UNSUITABLE SOILS NOTIFY ENGINEER.
2. SHOULD A HIGH WATER TABLE BE ENCOUNTERED THE ENGINEER SHALL BE NOTIFIED.
3. ALL PLUMBING SHALL COMPLY WITH PFC AND ALL LOCAL CODES. WATER INLET AND SEWER OUTLET LOCATIONS TO BE FIELD LOCATED BY BUILDER.
4. PORCH, STOOP, & DECK HANDRAILS NOT INCLUDED WITH SLAB FOUNDATION. RAILING IS REQUIRED WHEN THE PORCH IS OVER 30" HIGH.
5. SEE ENGINEERING PLAN BY OTHERS FOR FINAL CONSTRUCTION METHODS AND REQUIREMENTS.
6. FOOTING AND GRADE BEAMS AS DETAILED ON THIS PLAN ARE DESIGNED FOR TYPICAL FRAME CONSTRUCTION WITH NO HEAVY APPLIANCES, SUCH AS A MASONRY FIREPLACE, HOT TUB, SAUNA, ETC.
7. CONTRACTOR TO VERIFY LOAD BEARING REQUIREMENTS WITH TRUSS MAN. BEFORE CONSTRUCTION OF FOUNDATION. IN SOME CASES SHALL NOT BE NOTED AS LOAD BEARING WILL BE BY SO. ADDITIONAL ENGINEERING WILL BE REQUIRED.
8. THESE FOUNDATIONS HAVE BEEN DESIGNED TO INSURE STRUCTURAL INTEGRITY OF THE FOUNDATION. SOME DETAILS SUCH AS REINFORCED CONCRETE, SEE BARR REQUIREMENTS, SLOPING FORMS, ETC. MAY HAVE BEEN OMITTED. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND REQUIREMENTS WITH BUILDER.
9. CONTRACTOR SHALL VERIFY ALL DIMENSIONS WITH ARCHITECTURAL PLANS BEFORE CONSTRUCTION BEGINS.
10. SITE UNDERMINING
- 10.1. CLEAR BUILDING FOOTPRINT AREA PLUS 10 FEET TO REMOVE ALL SURFACE VEGETATION, ROOTS, BRANCHES, AND ANY OTHER UNSUITABLE MATERIALS.
- 10.2. GRADE SITE AND SOIL CONTACT EXISTING SOIL TO A FIRM CONDITION.
- 10.3. ALL FILL SHALL BE A CLEAN SAND COMPACTED IN LAYERS NOT EXCEEDING 10" THICK, LOOSE MEASURING TO A MINIMUM OF 95% MAXIMUM DENSITY, MODIFIED PROCTOR.
- 10.4. DO NOT PERMIT WATER TO STAND OR POOL ON OR AROUND FOUNDATION AREA DURING OR AFTER CONSTRUCTION. SAFE GRADING SHALL BE SUCH TO PROVIDE POSITIVE DRAINAGE OF SURFACE WATER RUNOFF AWAY AND AWAY FROM SLAB AREA.
11. ALL CONCRETE SHALL HAVE A MINIMUM STRENGTH OF 3000 PSI AT 28 DAYS.
12. REINFORCING BARS: ASTM A-615, GRADE 60
13. WELDED WIRE FABRIC: ASTM A-185, TYPE III SHALL BE SUPPORTED AND CAST IN PLACE SO THAT THE WIRE REMAINS IN THE MIDDLE ONE-THIRD OF THE SLAB. FIBER MESH MAY BE SUBSTITUTED FOR WFF.
14. MASONRY JOINT REINFORCEMENT: ASTM A-82, 9 GAUGE
15. BRICK LATH: C-10, TYPE B
16. MORTAR: ASTM C-270, TYPE M
17. GROUT: ASTM C-1100, LARGO PSI MIX
18. REINFORCING BAR SPICES SHALL BE LAPPED 36" MIN. AND TIED SECURELY. DO NOT FIELD. WFF SHALL BE LAPPED 12" MINIMUM AND SHALL BE CONTINUOUS OVER INTERNAL GRADE BEAMS.
19. REINFORCING BAR SHALL RUN CONTINUOUS AROUND CORNERS AND INTERSECTIONS.
20. TURN OUT REBAR AT THE METER AND PANEL FOR ELECTRICAL GROUNDING AS REQUIRED BY CODE.
21. MINIMUM PROTECTIVE CONCRETE COVER FOR REINFORCING BARS SHALL BE AS FOLLOWS:
 - 21.1. CAST, CURVED, AND PERMANENTLY EXPOSED TO WEATHER - 2"
 - 21.2. EXPOSED TO EARTH AND WEATHER - 3"
 - 21.3. ALL LOAD BEARING WALLS, COLUMNS, AND PIERS SHALL BE FOUNDED ON CONTINUOUS PROFILING OR GRADE BEAMS AS DETAILED.
 - 21.4. ALL LOAD BEARING WALLS MAY NOT BE SUBST. REFER TO TRUSS W/RS PLAN FOR ALL LOAD BEARING WALLS. ANY INTERIOR WALLS NOT SHOWN THAT ARE LOAD BEARING SHALL

REVISION SCHEDULE		DATE	
REV #	DESCRIPTION	REV #	DATE
1,667 SF	FIRST FLOOR HEATED	1,667 SF	
144 SF	FRONT PORCH	144 SF	
176 SF	SCREEN PORCH	176 SF	
320 SF	TOTAL UNDER ROOF	320 SF	
1,967 SF		1,967 SF	

AREAS:	TIME: PLEASANTVIEW A	FOR: FOYE AND KELLY JACKSON	FL: COLUMBIA - FL	SOLD BY: B. DIMICK
JOB #: 66-25-031	FOUNDATION TYPE: 2x6 EXTERIOR WALLS	FOUNDATION TYPE: STEMWALL SLAB	OFFICE: VALDOSTA SALES	
PROPERTY OF: LUCAS N. K.S.	CHECKED BY: K.S.	PRINTED: 1/29/2025 9:56:28 AM	© COPYRIGHT - 2025:	
SHEET #: F-1	FOUNDATION PLAN			

OPT. PAPER SIZE: for 14" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

PRELIMINARY CONSTRUCTION PLANS

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PAPER SIZE: 11" x 17"

SCALE: 1/8" = 1'-0"

STEM WALL CHART

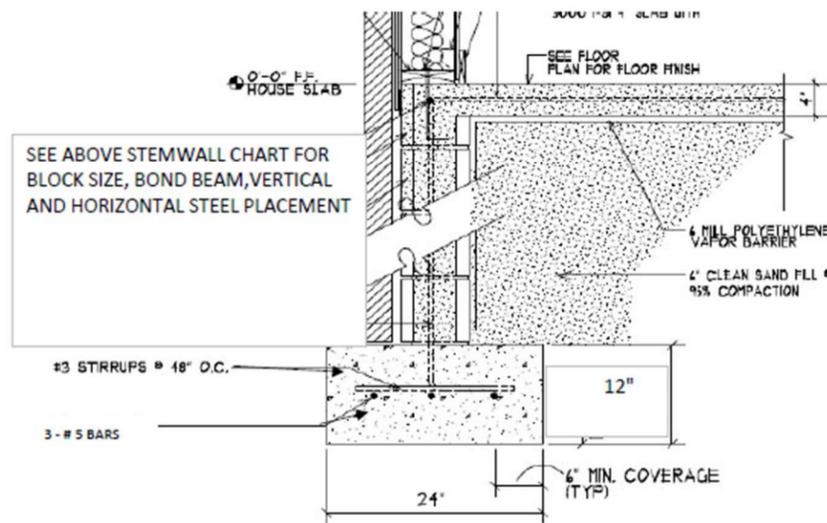
WALL HEIGHT	BOND BEAM	VERTICAL STEEL PLACEMENT	BLOCK SIZE	OTHER
0" -56"	NONE	# 5 Bar 36" o.c. AND All corners	12"	1 # 5 Bar Continous in Top Course IF WALL DOES NOT EXCEED 56 - "8" BLOCK MAY BE USED
56"-120"	BOND BEAM CONTINUOUS WITH 1- # 5 BAR AT 56" AND TOP COURSE	# 5 Bar 24" o.c.	12"	1 # 5 Bar Continous in Top Course

- 1 IN ALL CONFIGURATIONS, VERTICAL REBAR SHALL BE PLACED AT EITHER SIDE OF OPENINGS IN FOUNDATION WALL AND AT EACH CORNER OF THE FOUNDATION AND THE CELLS POURED WITH 3,000 PSI CONCRETE. ADDITIONALLY, ONE #5 REINFORCING BAR WITH THE CELL POURED FULL WITH 3,000 PSI CONCRETE SHALL BE PLACED UNDER ANY POINT LOADS THAT ARE LOCATED ON THE MASONRY WALL.
- 2 BACKFILL PLACEMENT: BACKFILL SHALL NOT BE PLACED AGAINST THE WALL UNTIL THE WALL HAS SUFFICIENT STRENGTH AND HAS BEEN ANCHORED TO THE FLOOR ABOVE, OR HAS BEEN SUFFICIENTLY BRACED TO PREVENT DAMAGE BY BACKFILLING
- 3 MORTAR: MORTAR SHALL BE TYPE M OR TYPE S; (28 DAY STRENGTH OF 2000 PSI). MASONRY SHALL BE LAID IN A RUNNING BOND.
- 4 THE MINIMUM COMPRESSIVE STRENGTH OF THE MASONRY SHALL BE F'M = 1500 PSI

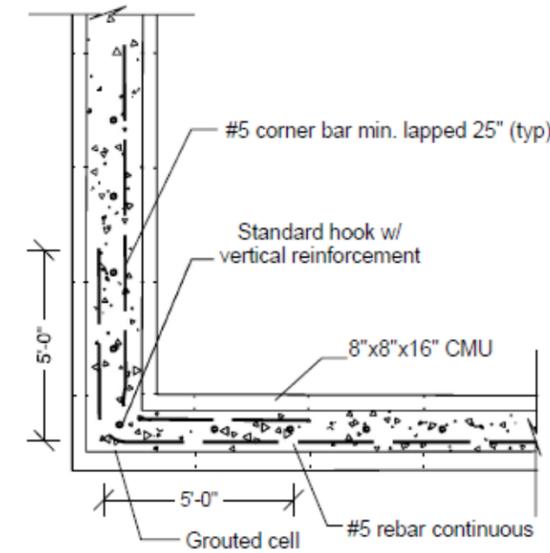
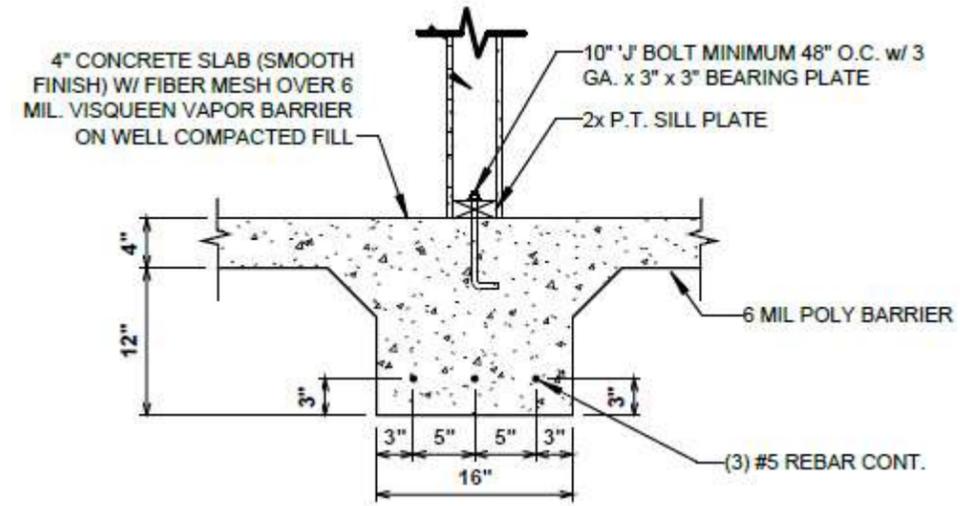
SATISFACTORY FILL MATERIAL SHALL BE FREE OF VEGETATION AND ORGANIC MATTER, WITH NOT MORE THAN 20 PERCENT BY WEIGHT PASSING THE 200 SIEVE. FILL LIFTS SHALL BE 12 INCHES MAXIMUM. ALL TOP SOIL CONTAINING UNSUITABLE MATERIAL SHALL BE REMOVED PRIOR TO THE PLACEMENT OF BACKFILL

EACH LAYER OF CLEAN FILL SHALL BE ADEQUATELY COMPACTED TO AT LEAST 95% IN THE TOP 3 FEET OF OPTIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR TEST.

SOIL BENEATH SLAB SHALL BE CHEMICALLY TREATED FOR TERMITES.



FOOTER SECTION USE MIN 3000 PSI CONCRETE
OPTIONAL STEMWALL SECTION



CORNER CONTINUITY OF BOND BEAM

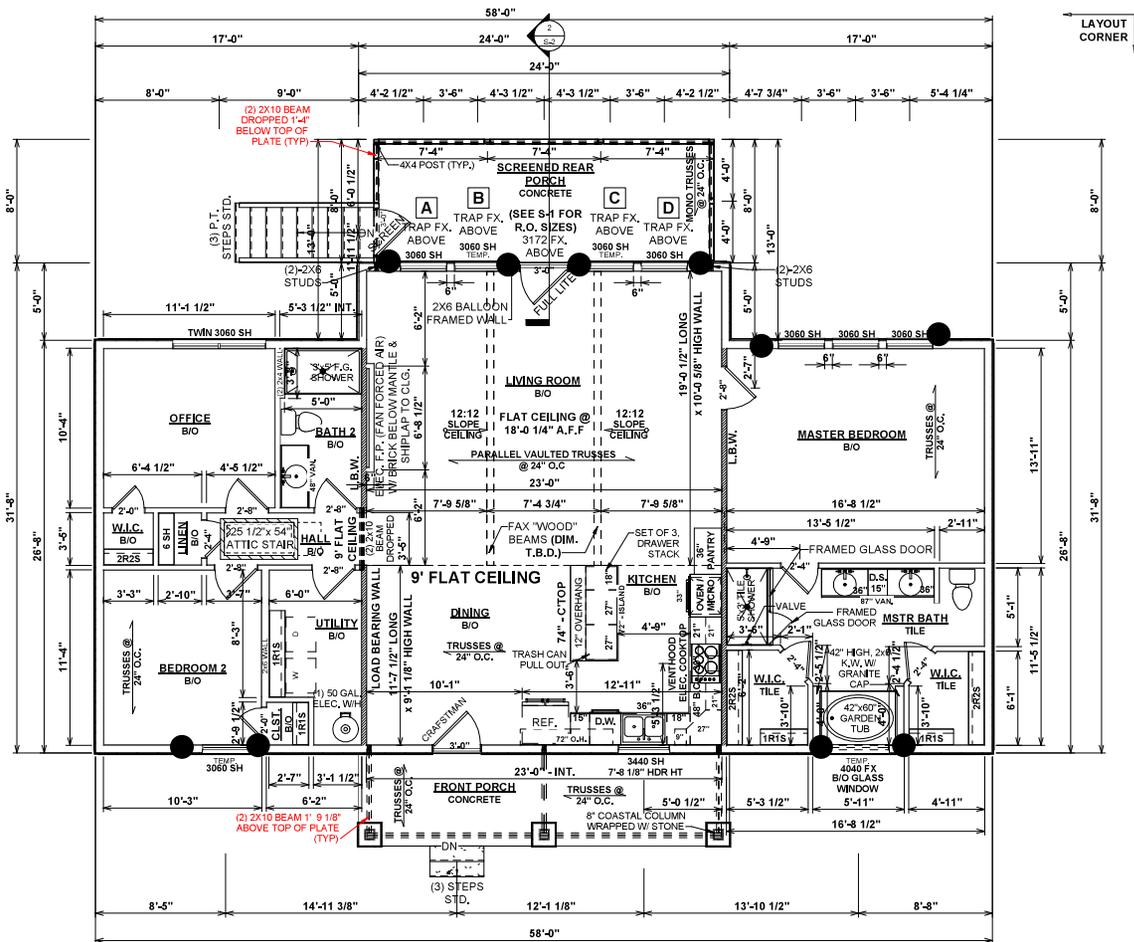
NOT TO SCALE

THE:	PLEASANTVIEW A
FOR:	FOYE AND KELLY JACKSON
	FL COLUMBIA - FL



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HDU2 HOLD DOWN



1 FIRST FLOOR PLAN
1/8" = 1'-0"



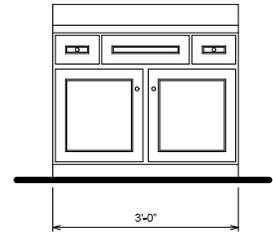
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GENERAL NOTES:

- MIN 9'-1 1/8" CEILING HEIGHT ON FIRST FLOOR
- ALL LOAD BEARING WALLS & EXT. OPENINGS TO HAVE (2) 2X10 HEADERS UNLESS OTHERWISE NOTED
- STANDARD FIRST FLOOR WINDOW HEADERS TO BE FRAMED DOWN 2'-2" FROM T.O.P, EXCEPT AS NOTED
- STANDARD SECOND FLOOR WINDOW HEADERS TO BE FRAMED DOWN 1'-2" FROM T.O.P, EXCEPT AS NOTED
- 7/16" O.S.B. AND HOUSEWRAP REQUIRED
- DIMENSIONS ARE TO SHEATHING EXTERIOR; SUBTRACT 1/2" FROM DIMENSIONS FOR EXTERIOR WINDOW AND DOOR FRAMING LOCATION IF OPENINGS ARE FRAMED BEFORE SHEATHING INSTALLATION
- ALL INTERIOR DOORS ARE EITHER CENTERED ON WALLS OR R.O. STARTED MIN OF 4" FROM ADJOINING WALL UNLESS OTHERWISE DIMENSIONED
- NUMBER OF STAIR TREADS & RISERS MAY VARY AS A RESULT OF LOCAL BUILDING CODES, STANDARDS AND FINAL GRADE
- CLOSET SHELF HEIGHT OFF FLOOR:
SINGLE-58"
DOUBLE 42" & 84"
- ALL PLUMBING FIXTURES SHOWN ARE A REPRESENTATION OF SIZE AN LOCATION ONLY. ACTUAL STYLE AND BRAND OF FIXTURES MAY VARY PER OFFICE LOCATION
- ALL TUBS/SHOWERS ARE TO HAVE NAILERS AT FLANGE
- INSTALL A 24" WIDE WALKWAY FROM ATTIC ACCESS TO FURNACE PLATFORM
- PORCH, STOOP, & DECK HANDRAILS NOT INCLUDED W/ SLAB FOUNDATION
- RAILINGS ARE A FORCED OPTION WHEN PORCH IS OVER 30" HIGH
- SCHLUTER DITRA UNCOUPLING AND WATERPROOFING MEMBRANE TO BE USED ON ALL TILE SHOWER INSTALLATIONS

SPECIAL NOTES:

- TILE BACK-SPLASH LEVEL 2 @ KITCHEN
- CERAMIC TILE LEVEL 2 I/O CARPET @ MSTR BATH
- T.B.D. FLUSH MOUNTS ON REAR PORCH
- B/O FLOORING THROUGHOUT EXCEPT MASTER BATH
- TINTED UPPER WINDOWS ON REAR OR HOME
- GENERATOR DISCONNECT/TRANSFER SWITCH T.B.D.
- ESTIMATE ONLY-RADIANT BARRIER OSB IN ROOF
- LEVEL 2 COUNTERTOPS THROUGH OUT HOME-BLACK PEARL AND OYSTER WHITE
- TILE SHOWER IN BATH 2



2 891.1 36" VANITY ELEVATION
1/2" = 1'-0"

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

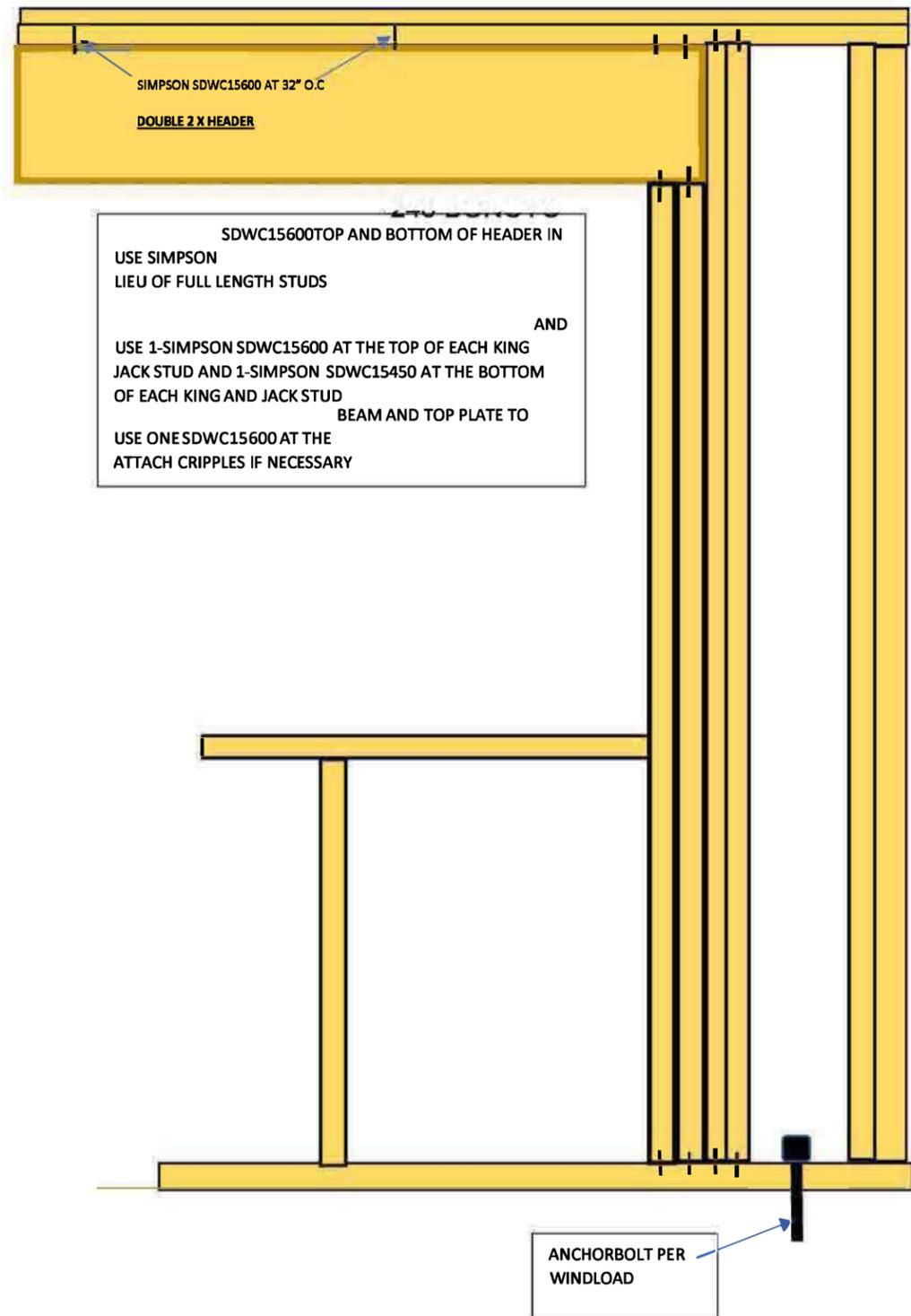
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PAPER SIZE: 11" x 17"

SCALE: As indicated

SHEET #	PROPERTY OF:	DRAWN BY:	JOB #	TITLE:	REVISION SCHEDULE	
					REV #	DATE
F-2	AMERICAN HOME PLACE	LUCAS N. K.S.	66-25-031	PLEASANTVIEW A	1	1,687 SF
					2	1,687 SF
FIRST FLOOR PLAN	AMERICAN HOME PLACE	LUCAS N. K.S.	2x6 EXTERIOR WALLS	FOYE AND KELLY JACKSON	1	144 SF
					2	144 SF
FIRST FLOOR PLAN	AMERICAN HOME PLACE	LUCAS N. K.S.	FOUNDATION TYPE:	COLUMBIA - FL	1	320 SF
					2	320 SF
FIRST FLOOR PLAN	AMERICAN HOME PLACE	LUCAS N. K.S.	STEMWALL SLAB	SOLD BY:	1	1,367 SF
					2	1,367 SF
FIRST FLOOR PLAN	AMERICAN HOME PLACE	LUCAS N. K.S.	OFFICE:	VALDOSTA SALES	1	
					2	
FIRST FLOOR PLAN	AMERICAN HOME PLACE	LUCAS N. K.S.	OFFICE:	B. DIMICK	1	
					2	



HEADER SIZE AND STRAPPING CHART

SPAN	HEADER SIZE	QUANTITY OF JACK STUDS AT EACH END	QUANTITY OF KING STUDS AT EACH END	STRAPPING TO JACK STUDS AT EACH END TOP AND BOTTOM	STRAPPING TO KING STUDS AT EACH END TOP AND BOTTOM
0'-0" TO 7'-6"	2 - 2X10" WITH ½" PLATE	1	1	1 SIMPSON MSTA24	1 SIMPSON SPH4
7'-6" – 11'-3"	2 - 2X12" WITH ½" PLATE OR 4-2 X 10" WITH ½" PLATE	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4
11'-3" – 14'-0"	2 - 1 ¼" X 9 ¼" LVL	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4

IN LIEU OF STRAPPING USE A SDWC15600 AT THE TOP OF EACH JACK AND KING STUD AND ONE SDWC15450 AT THE BASE OF EACH JACK AND KING STUD

AT ALL CORNERS USE 3- SIMPSON SDWC1540 SCREWS AT BASE AND 3- SIMPSON SDWC15600 AT TOP – IF NECESSARY, USE 2-SCREWS IN ONE STUD AND ADD ONE MORE STUD MIDSPAN



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MecaWind v2545

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Calculations Prepared by:

Date: Dec 04, 2025

File Location: Current Project Not Saved



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General:

Reference Abbreviations: T: Table, F: Figure, E: Equation, S: Section, ¶: Paragraph

Wind Load Standard	= ASCE 7-22	Basic Wind Speed	= 120.0 mph
Exposure Classification	= B	Risk Category	= II
Structure Type	= Building	Basis for Wind Pressures	= ASD
MWFRS Analysis Method	= Ch 27	C&C Analysis Method	= Ch 30 Pt 1
Dynamic Type of Structure	= Rigid	Advanced Options	= False

Building:

Roof = Roof Type	= Gabled	Encl = Enclosure Classification	= Enclosed
IsCust = Custom Roof	= False	W = Building Width	= 32.0000 ft
L = Building Length	= 58.0000 ft	R _{ht} = Ridge Height	= 18.333 ft
E _{ht} = Eave Height	= 9.0000 ft	Pitch = Pitch of Roof	= 6.4 :12
θ = Slope of Roof	= 28.07 °	OH = Overhang Configuration	= All Soffit
Par = Parapet	= None	z _i = Highest Opening Elevation	= 6.0000 ft
HT _{over} = Override Mean Roof Height	= False	Ht _{man} = Mean Roof Height	= 13.667 ft
RA _{over} = Override Roof Area	= False	GC _{pi_o} = Override GC _{pi} value	= False
IsElev = Building is Elevated	= False		

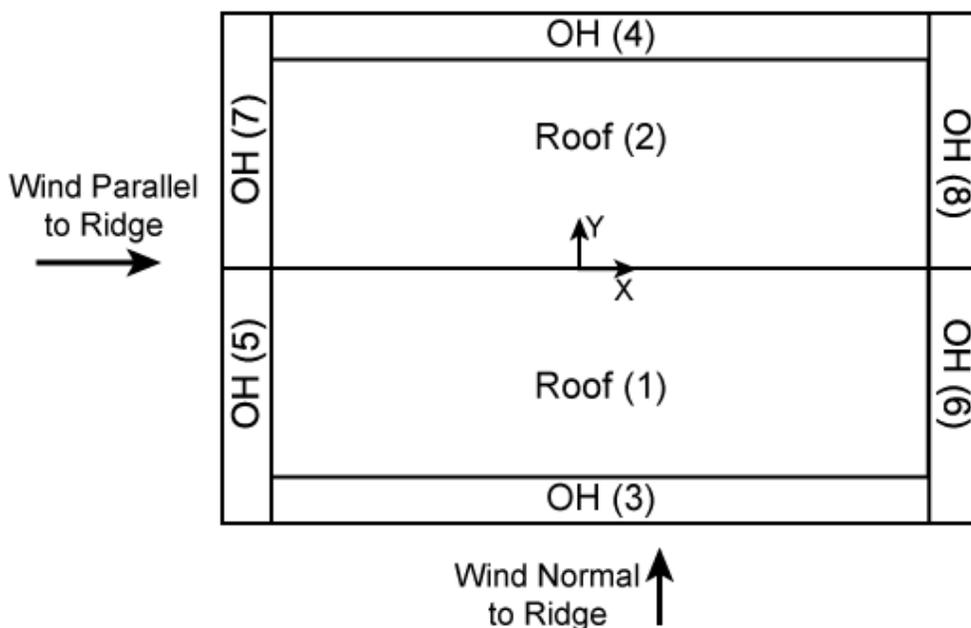
Exposure Constants T:26.11-1:

α = 3-s Gust-speed exponent	= 7.500	Z _g = Nominal Ht of Boundary Layer	= 3280.000 ft
â = Reciprocal of α	= 0.133	b = 3 sec gust speed factor	= 0.840
α _m = Mean hourly Wind-Speed Exponent	= 0.222	b _m = Mean hourly Windspeed Exponent	= 0.470
c = Turbulence Intensity Factor	= 0.300	ε = Integral Length Scale Exponent	= 0.3333

Overhang Inputs:

Std	= Overhangs on all sides are the same	= True
OHType	= Type of Roof Wall Intersections	= Soffit
OH	= Overhang of Roof Beyond Wall	= 1.500 ft

Main Wind Force Resisting System (MWFRS) Wind Calculations per Ch 27



h = Mean structure height	= 13.667 ft	K _h = 2.41 • (15/Z _g) ^{2/α} T:26.10-1	= 0.573
K _{zt} = No Topographic Feature	= 1.000	K _d = Directionality Factor T:26.6-1	= 0.85
GC _{pi} = ± Internal Press Coef T:26.13-1	= ±0.18	LF = ASD Load Factor	= 0.60
K _e = Ground Elev Factor T:26.10-1	= 1.000	q _h = .00256 • K _h • K _{zt} • K _e • V ² • LF _{E:26.10-1}	= 12.67 psf
q _{in} = - Internal Press: q _h	= 12.67 psf	q _{ip} = For +GC _{pi} use q _h	= 12.67 psf



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MWFRS Wind Loads [Parallel to Ridge]

h = Mean Roof Height of Building	= 13.6667 ft	R _{ht} = Ridge Height Of Roof	= 18.3333 ft
B = Building Width Normal To Wind	= 32.0000 ft	L = Building Width Parallel To Wind	= 58.0000 ft
L/B = Ratio: L/B	= 1.813	h/L = Ratio: h/L	= 0.236
θ = Slope of Roof	= 28.07 °	C _{pW} = Windward Wall Coefficient	= 0.800
C _{pLW} = Leeward Wall Coefficient	= -0.338	C _{pSW} = Side Wall Coefficient	= -0.700

Gust Factor Calculation for Wind: [Parallel to Ridge]

Gust Factor Category I Rigid Structures - Simplified Method
G₁ = Simplified: For Rigid Structures can use 0.85 = 0.85

Gust Factor Category II Rigid Structures - Complete Analysis
Z_m = Equiv Struc Height: Max(0.6•h, Z_{min}) = 30.000 ft
I_{zm} = Turbulence Intensity: c•(33/Z_m)^{1/6} E:26.11-7 = 0.305
L_{zm} = Turbulence Integral Length Scale: l•(Z_m/33)^ε E:26.11-9 = 309.993 ft
B = Building Width Normal to Wind Direction = 32.000 ft
Q = [1/(1+0.63•[(B+h)/L_{zm}]^{0.63})]^{0.5} E:26.11-8 = 0.917
G₂ = Detailed: 0.925•[(1+1.7•g_q•I_{zm}•Q)/(1+1.7•g_v•I_{zm})] E:26.11-6 = 0.876

Gust Factor Used in Analysis
G = Gust Factor: Min(G₁, G₂) = 0.850

Wall Wind Pressures [Parallel to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC _{pi}	q _i psf	K _z	K _{zt}	q _z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
18.333	+0.18	12.67	0.604	1.000	13.37	5.79	-5.03	-8.35	10.82	9.60
13.667	+0.18	12.67	0.573	1.000	12.67	5.39	-5.03	-8.35	10.41	9.60
9.000	+0.18	12.67	0.573	1.000	12.67	5.39	-5.03	-8.35	10.41	9.60
18.333	-0.18	12.67	0.604	1.000	13.37	9.67	-1.15	-4.47	10.82	9.60
13.667	-0.18	12.67	0.573	1.000	12.67	9.26	-1.15	-4.47	10.41	9.60
9.000	-0.18	12.67	0.573	1.000	12.67	9.26	-1.15	-4.47	10.41	9.60

K_z = 2.41•(15/Z_g)^{2/α}
GC_{pi} = +Internal Coef E:26.13-1
q_{ip} = For +GC_{pi} use q_h
Side = q_h•K_d•G•C_{pSW}-q_{ip}•K_d•(GC_{pi+}) E:27.3-1
Windward = q_z•K_d•G•C_{pW}-q_{ip}•K_d•(GC_{pi+}) E:27.3-1
+Press = Pressure Acting Toward Surface
s27.1.5 = MWFRS Min Wall Pressure = 9.60 psf

K_{zt} = No Topographic Feature
q_z = .00256•K_z•K_{zt}•K_e•V²•LF E:26.10-1
q_{in} = - Internal Press: q_h
Leeward = q_h•K_d•G•C_{pLW}-q_{ip}•K_d•(GC_{pi+}) E:27.3-1
Total = Windward - Leeward
-Press = Pressure Acting Away from Surface

Roof Wind Pressures [Parallel to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC _{pi}	C _{pMin}	C _{pMax}	P _{min} psf	P _{max} psf	P _{min} psf
Overhang	Overhang 0 to h/2	5,7	0.000	1.500	0.0	P	0	-0.9	-0.18	-8.24	-1.65	4.80
Overhang	Overhang ≥ 2•h	6,8	59.500	61.000	0.0	P	0	-0.3	-0.18	-2.75	-1.65	4.80
Overhang Bottom	Bottom	5,7	All	All	0.0	N/A	0	0.8	0.8	7.32	7.32	4.80
Overhang	Overhang 0 to h	3,4	1.500	13.667	0.0	P	+0.1800	-0.9	-0.18	-10.18	-3.59	4.80
Overhang	Overhang h to 2•h	3,4	13.667	27.333	0.0	P	+0.1800	-0.5	-0.18	-6.52	-3.59	4.80
Overhang	Overhang ≥ 2•h	3,4	27.333	59.500	0.0	P	+0.1800	-0.3	-0.18	-4.69	-3.59	4.80
Roof	Roof 0 to h	1,2	1.500	13.667	0.0	P	+0.1800	-0.9	-0.18	-10.18	-3.59	4.80
Roof	Roof h to 2•h	1,2	13.667	27.333	0.0	P	+0.1800	-0.5	-0.18	-6.52	-3.59	4.80
Roof	Roof ≥ 2•h	1,2	27.333	59.500	0.0	P	+0.1800	-0.3	-0.18	-4.69	-3.59	4.80
Overhang	Overhang 0 to h	3,4	1.500	13.667	0.0	P	-0.1800	-0.9	-0.18	-6.30	0.29	4.80
Overhang	Overhang h to 2•h	3,4	13.667	27.333	0.0	P	-0.1800	-0.5	-0.18	-2.64	0.29	4.80
Overhang	Overhang ≥ 2•h	3,4	27.333	59.500	0.0	P	-0.1800	-0.3	-0.18	-0.81	0.29	4.80
Roof	Roof 0 to h	1,2	1.500	13.667	0.0	P	-0.1800	-0.9	-0.18	-6.30	0.29	4.80
Roof	Roof h to 2•h	1,2	13.667	27.333	0.0	P	-0.1800	-0.5	-0.18	-2.64	0.29	4.80
Roof	Roof ≥ 2•h	1,2	27.333	59.500	0.0	P	-0.1800	-0.3	-0.18	-0.81	0.29	4.80

Roof Pressures based upon ch 27:

Component = The building component for pressures
Start = Start Dist from Windward Edge
C_{pMin} = Smallest Coefficient Magnitude
P_{min} = q_h•K_d•G•C_{pMin}-q_{ip}•K_d•GC_{piE:27.3-1}
GC_{pi} = +Internal Coef E:26.13-1
P_{min} = Min Press projected on vertical plane s27.1.5

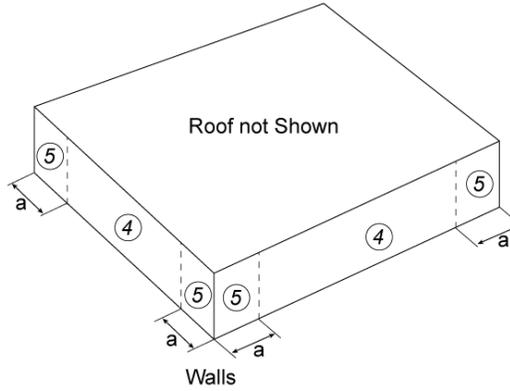
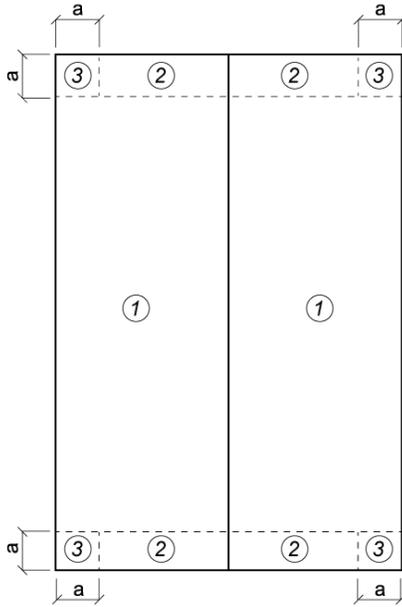
Location = Reference Graphic in Output for Values
End = End Dist from Windward Edge
C_{pMax} = Largest Coefficient Magnitude
P_{max} = q_h•K_d•G•C_{pMax}-q_{ip}•K_d•GC_{piE:27.3-1}
Basis = P=Parallel to Ridge; N=Normal to Ridge
θ = Roof Slope Relative to Wind

§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf
 -Press = Pressure Acting Away from Surface

+Press = Pressure Acting Toward Surface

• The smaller uplift pressures due to C_{pmin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

Components and Cladding (C&C) Wind Loads per Ch 30 Pt 1 Roof & Wall



h = Mean structure height = 13.667 ft
 K_{zt} = No Topographic Feature = 1.000
 GC_{pi} = ± Internal Press Coef $T_{26.13-1}$ = ±0.18
 K_e = Ground Elev Factor $T_{26.10-1}$ = 1.000
 θ = Slope of Roof = 28.07 °
 a = Max(a_1 , $0.04 \cdot B$, 3 ft [0.9 m]) = 3.200 ft

K_h = $2.41 \cdot (15/Z_g)^{2/\alpha}$ = 0.573
 K_d = Directionality Factor $T_{26.6-1}$ = 0.85
 LF = ASD Load Factor = 0.60
 q_h = $.00256 \cdot K_h \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF$ $E_{26.10-1}$ = 12.67 psf
 a_1 = Min($0.1 \cdot B$, $0.4 \cdot h$) = 3.200 ft

C&C Wind Roof & Wall Detailed per Ch 30 Pt 1 All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft ²	1/3 Rule	Reference	a ft	GC _{pi}	GC _{pd}	GC _{pu}	P _{max} psf	P _{min} psf
Zone 1	1	1.0000	1.0000	1.00	No	F:30.3-2D	3.2	±0.18	0.90	-1.80	11.63	-21.33
Zone 2	2	1.0000	1.0000	1.00	No	F:30.3-2D	3.2	±0.18	0.90	-2.00	11.63	-23.48
Zone 3	3	1.0000	1.0000	1.00	No	F:30.3-2D	3.2	±0.18	0.90	-2.50	11.63	-28.87
Zone 4	4	1.0000	1.0000	1.00	No	F:30.3-1	3.2	±0.18	1.00	-1.10	12.71	-13.79
Zone 5	5	1.0000	1.0000	1.00	No	F:30.3-1	3.2	±0.18	1.00	-1.40	12.71	-17.02

GC_{pd} = Down (+) External Coefficient
 P_{max} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $E_{30.3-1}$
 +Press = Pressure Acting Toward Surface
 §30.2.2 = C&C Min Pressure = 9.60 psf
 Width = Width of Component
 Area = Span • Width
 a = Max[Min($.1 \cdot B$, $.4 \cdot h$), $.04 \cdot B$, 3] $F_{30.3-2D}$
 GC_{pi} = Internal Coef $T_{26.13-1}$

GC_{pu} = Uplift (-) External Coefficient
 P_{min} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $E_{30.3-1}$
 -Press = Pressure Acting Away from Surface
 Zone = Applicable Zone per Figure
 Span = Span of Component
 1/3 Rule = Width limited to Span/3
 a = Max[Min($.1 \cdot B$, $.4 \cdot h$), $.04 \cdot B$, 3] $F_{30.3-1}$
 Reference = Applicable Reference from Standard

C&C Wind Roof & Wall Overhangs Detailed per Ch 30 Pt 4 All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft ²	1/3 Rule	Reference	a ft	GC _{pi}	GC _{pd}	GC _{pu}	P _{max} psf	P _{min} psf
Zone 1_OHS	1_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	3.2	±0.18	0.00	-2.80	9.60	-32.10
Zone 2_OHS	2_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	3.2	±0.18	0.00	-3.00	9.60	-34.25
Zone 3_OHS	3_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	3.2	±0.18	0.00	-3.50	9.60	-39.64

GC_{pd} = Down (+) External Coefficient
 P_{max} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $E_{30.7-1}$
 +Press = Pressure Acting Toward Surface
 §30.2.2 = C&C Min Pressure = 9.60 psf
 Width = Width of Component

GC_{pu} = Uplift (-) External Coefficient
 P_{min} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $E_{30.7-1}$
 -Press = Pressure Acting Away from Surface
 Zone = Applicable Zone per Figure
 Span = Span of Component

Area = Span • Width
a = Max[Min(.1•B,.4•h),.04•B,3] F:30.3-2D
Reference = Applicable Reference from Standard
Soffit = Soffit present so use building G_{Cpi}

1/3 Rule = Width limited to Span/3
 G_{Cpi} = Internal Coef T:26.13-1
#_OHS = Roof Zone # on Overhang Soffit

C&C Wind Roof & Wall Summary per Ch 30 Pt 1 (Table 1 of 2)
All wind pressures include a Load Factor (LF) of 0.6

Zone	Reference	P_{max}	P_{min}	P_{max}	P_{min}	P_{max}	P_{min}
		A ≤ 10 ft ² psf	A ≤ 10 ft ² psf	A = 20 ft ² psf	A = 20 ft ² psf	A = 50 ft ² psf	A = 50 ft ² psf
1	F:30.3-2D	11.63	-21.33	10.64	-18.08	9.60	-13.80
2	F:30.3-2D	11.63	-23.48	10.64	-20.99	9.60	-17.69
3	F:30.3-2D	11.63	-28.87	10.64	-25.13	9.60	-20.19
4	F:30.3-1	12.71	-13.79	12.14	-13.21	11.38	-12.46
5	F:30.3-1	12.71	-17.02	12.14	-15.87	11.38	-14.36
1_OHS	F:30.3-2D/F:30.3-1	9.60	-32.10	9.60	-28.28	9.60	-23.24
2_OHS	F:30.3-2D/F:30.3-1	9.60	-34.25	9.60	-31.19	9.60	-27.14
3_OHS	F:30.3-2D/F:30.3-1	9.60	-39.64	9.60	-35.33	9.60	-29.63

C&C Wind Roof & Wall Summary per Ch 30 Pt 1 (Table 2 of 2)
All wind pressures include a Load Factor (LF) of 0.6

Zone	Reference	P_{max}	P_{min}	P_{max}	P_{min}	P_{max}	P_{min}
		A = 100 ft ² psf	A = 100 ft ² psf	A = 200 ft ² psf	A = 200 ft ² psf	A > 500 ft ² psf	A > 500 ft ² psf
1	F:30.3-2D	9.60	-10.56	9.60	-10.56	9.60	-10.56
2	F:30.3-2D	9.60	-15.20	9.60	-12.71	9.60	-12.71
3	F:30.3-2D	9.60	-16.45	9.60	-12.71	9.60	-12.71
4	F:30.3-1	10.81	-11.88	10.24	-11.31	9.60	-10.56
5	F:30.3-1	10.81	-13.21	10.24	-12.07	9.60	-10.56
1_OHS	F:30.3-2D/F:30.3-1	9.60	-19.42	9.60	-18.85	9.60	-18.09
2_OHS	F:30.3-2D/F:30.3-1	9.60	-24.07	9.60	-21.01	9.60	-20.25
3_OHS	F:30.3-2D/F:30.3-1	9.60	-25.32	9.60	-21.01	9.60	-20.25

P_{max} = Maximum Pressure
Area = Span • Width
Span = Span of Component
 $s_{30.2.2}$ = C&C Min Pressure = 9.60 psf

P_{min} = Minimum Pressure
Width = Width of Component
Reference = Applicable Reference from Standard
Interpolate = Interpolate for Areas between columns



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