

REVISIONS	
21Feb06	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

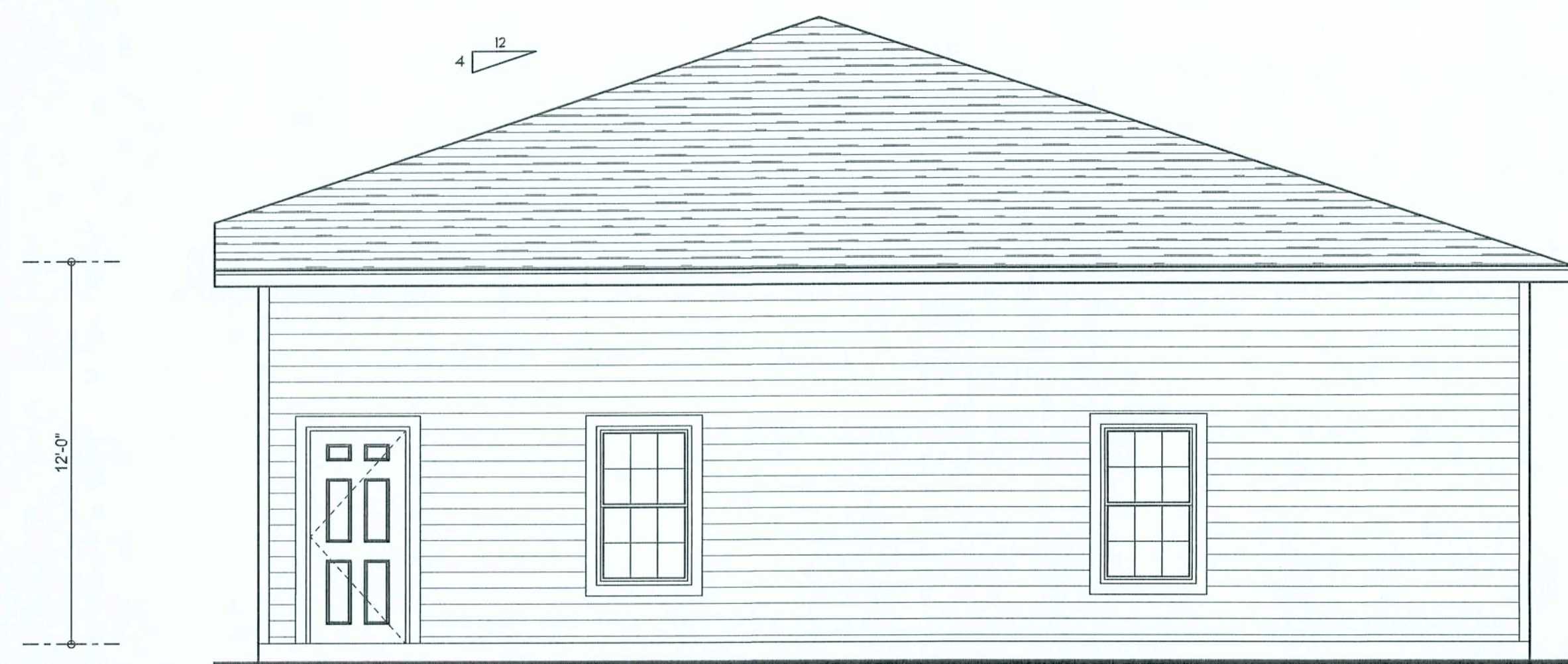
REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2305.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)
1840 S.F. / 300 x 50% = 3.06 S.F. RIDGE VENT AREA REQUIRED
27.8 FEET OF RIDGE VENT REQUIRED

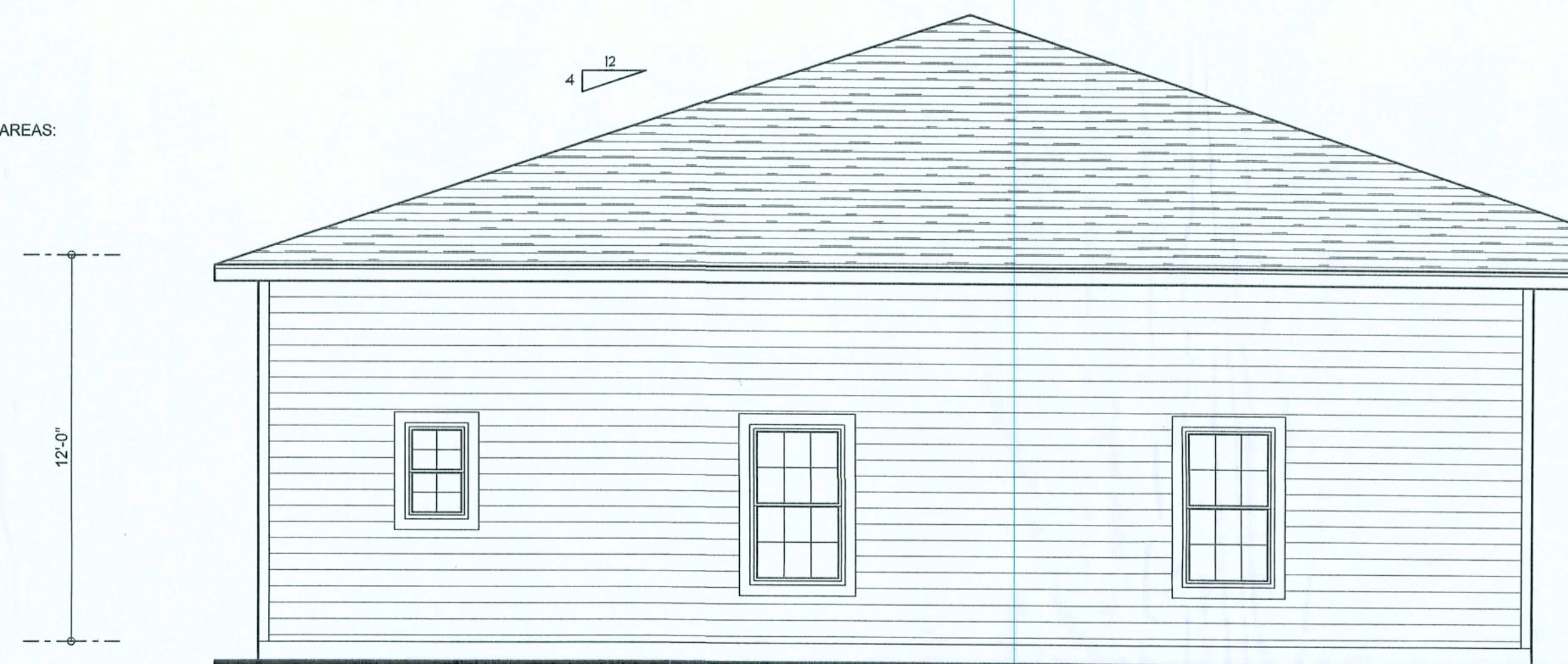
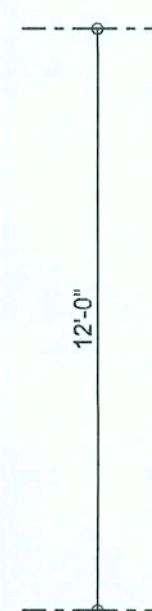
SOFFIT VENT
1840 S.F. / 300 x 50% = 3.06 S.F. SOFFIT VENT AREA REQUIRED
102 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

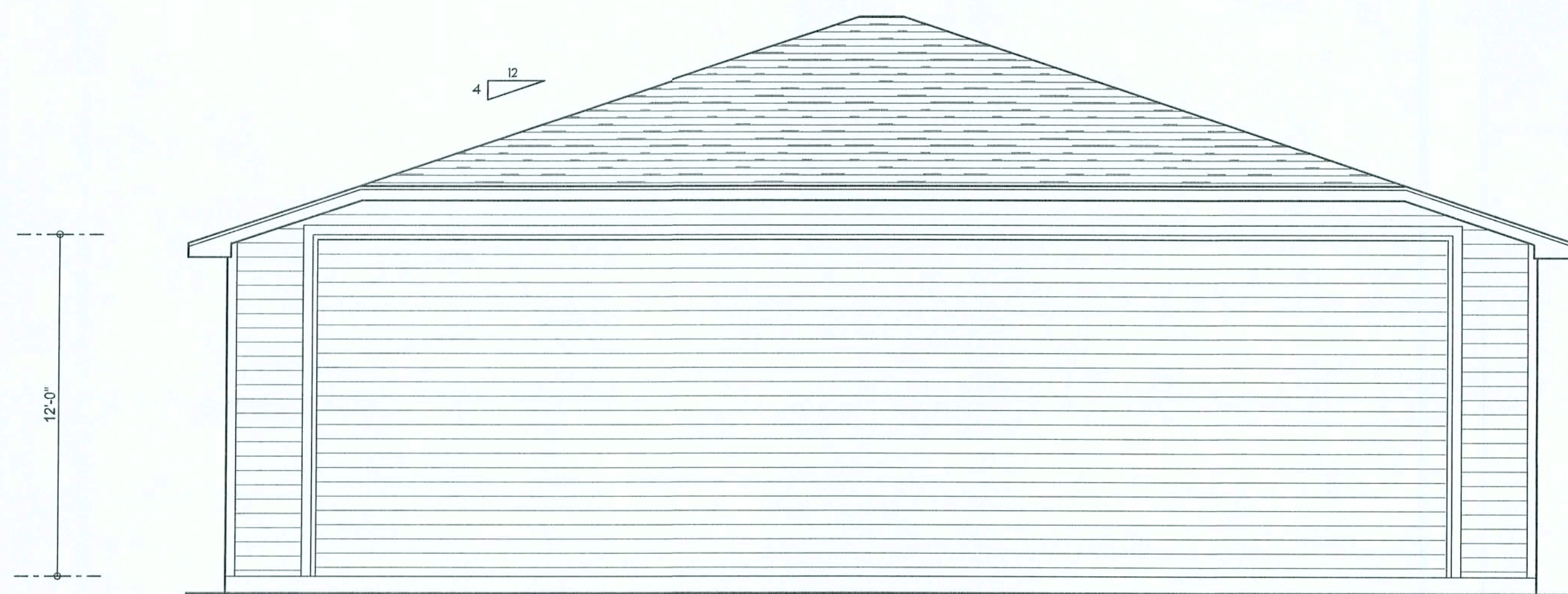
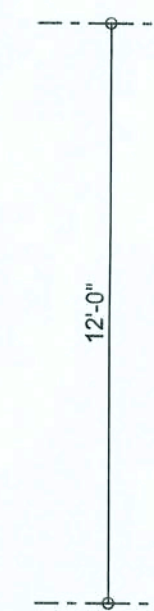
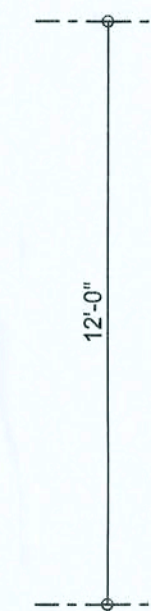
1. RIDGE VENTS = 18 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 41.3 IN2/FT (.03 FT2/FT)



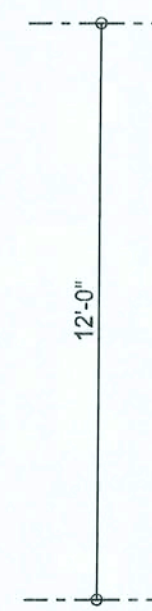
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



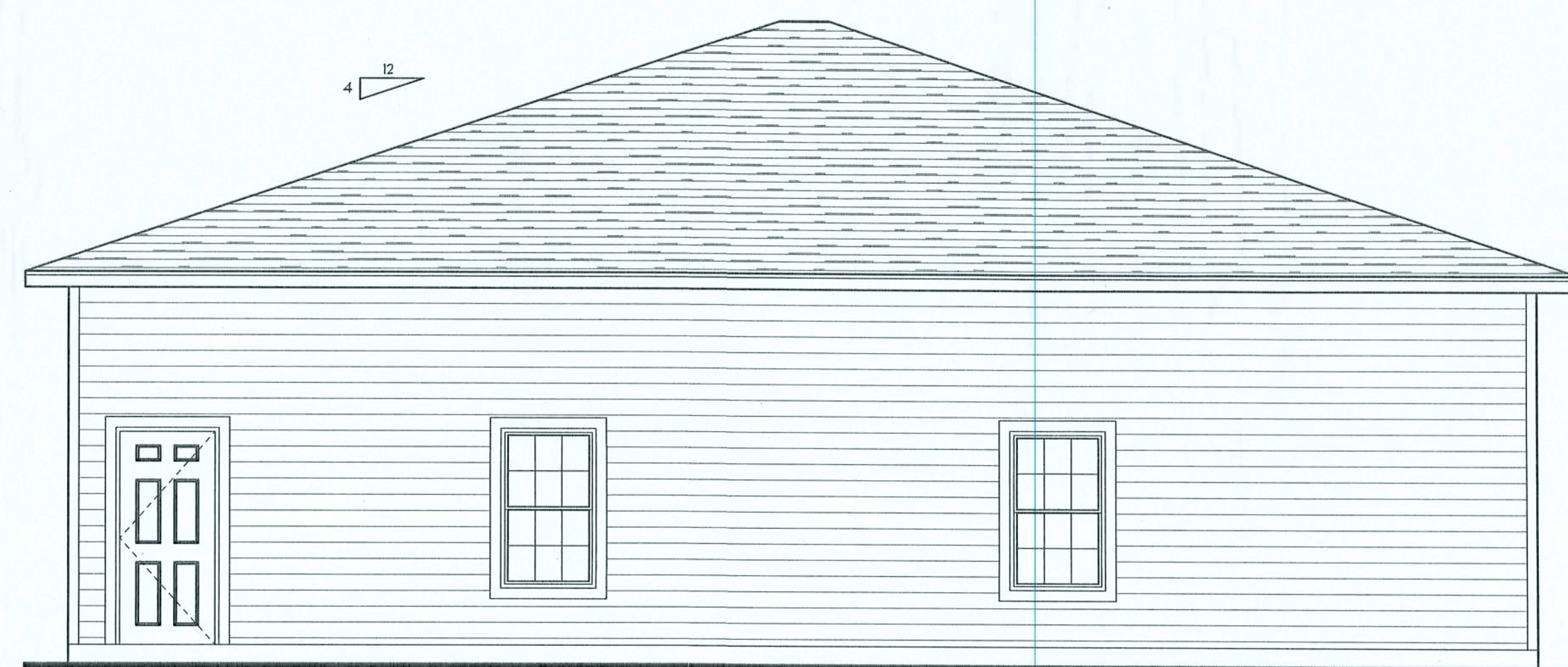
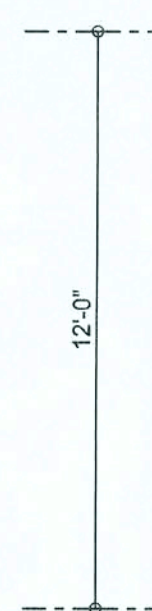
REAR ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



NOTE:
13'-0" HEIGHT FROM FINISHED FLOOR
TO THE INTERSECTION OF THE ROOF
TRUSS WITH THE HANGER DOOR WALL
IS NEEDED TO ATTACH DOOR



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 53915, P.O. Box 868, Lake City, FL
32066, 386-754-5419

DIMENSIONS:
Scaled dimensions supercede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section 1609, Florida building code
2004, to the best of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOWAY
P.E. 53915

Mark Disoway
25 Sep 06
SEAL

Robert Foti
Hanger

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PRINTED DATE:
September 25, 2006

DRAWN BY: David Disoway CHECKED BY:

FINALS DATE:
25 / Sep / 06

JOB NUMBER:
608291

DRAWING NUMBER

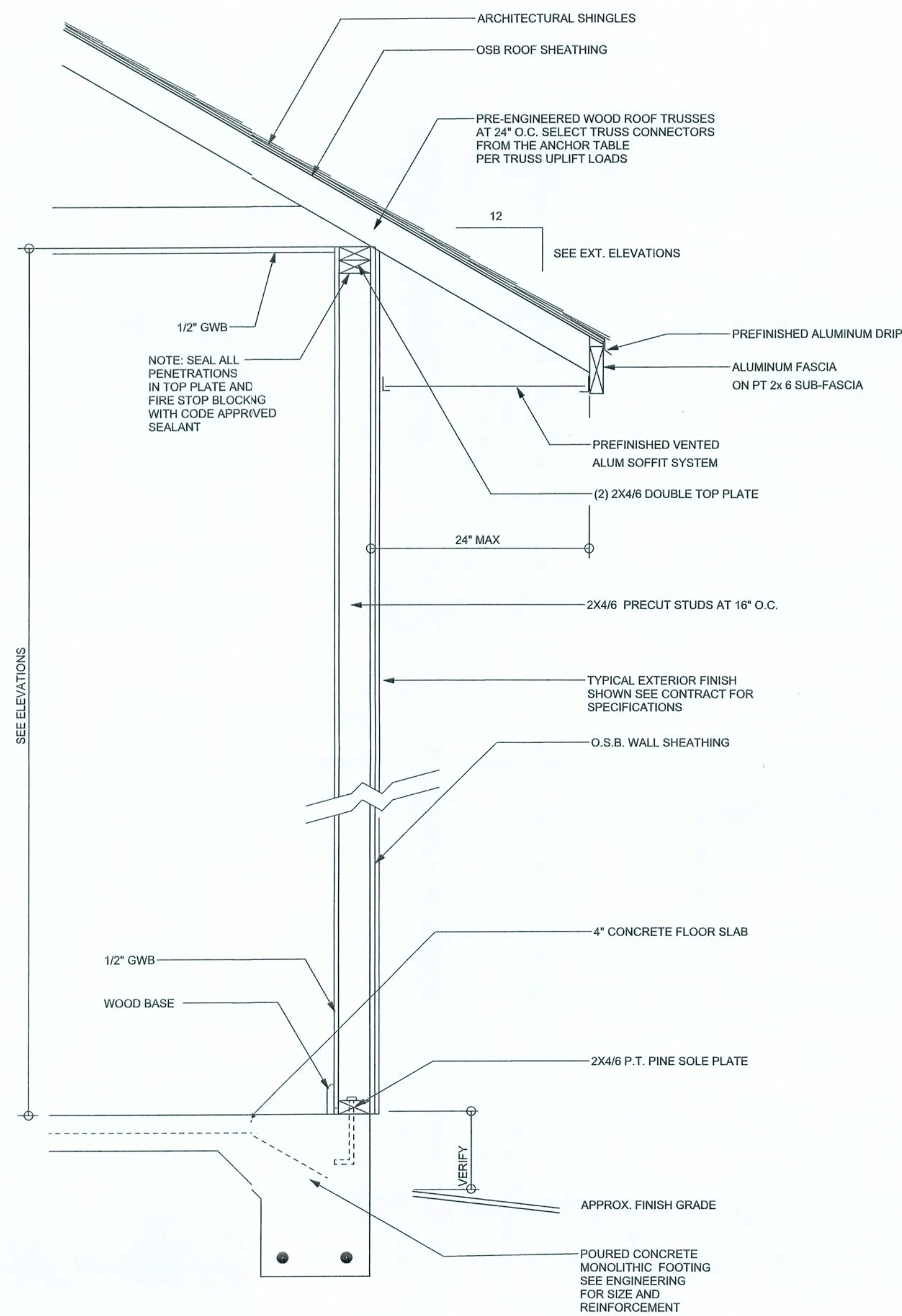
A-1

OF 6 SHEETS

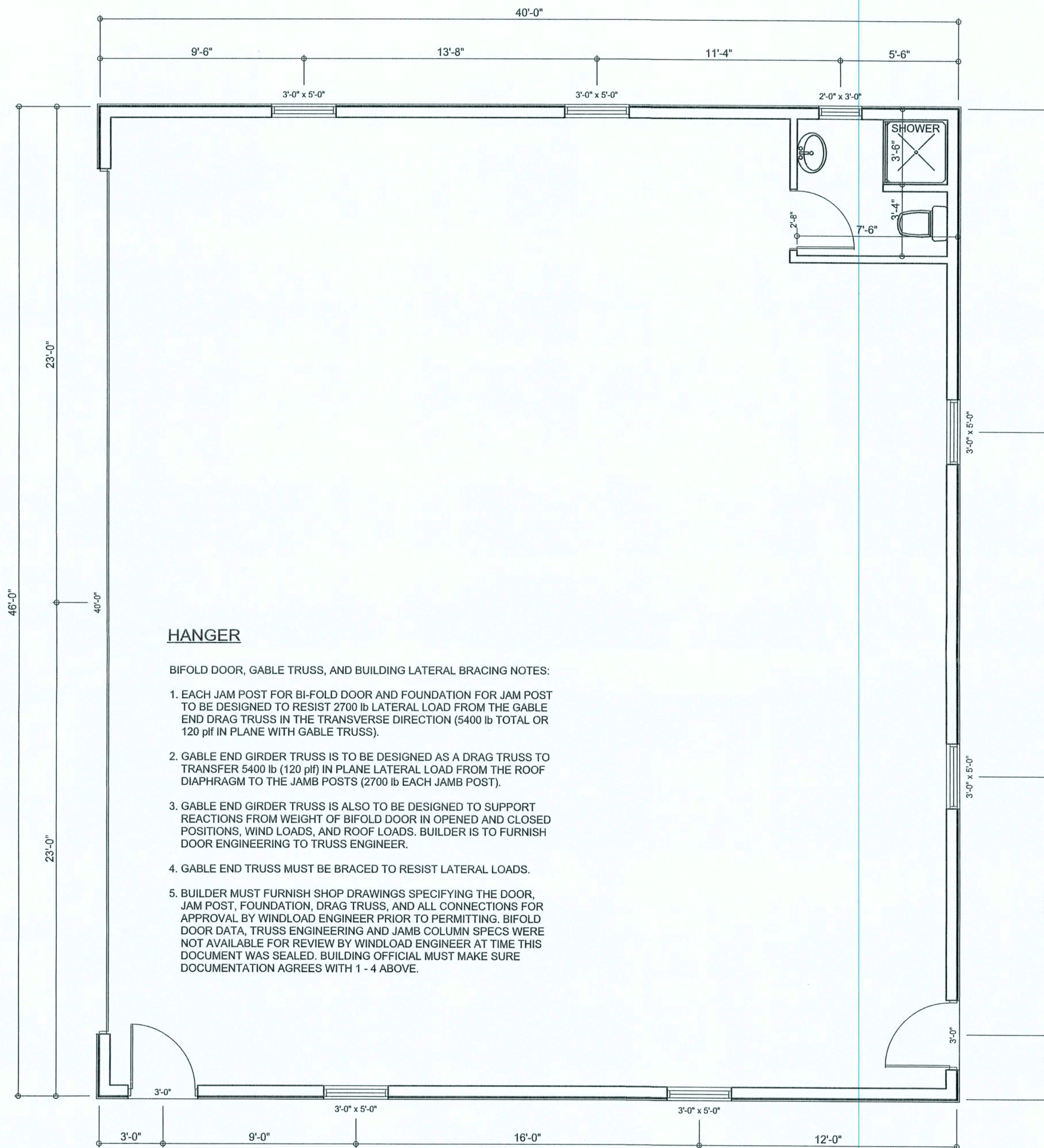
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TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1\"/>



HANGER

BIFOLD DOOR, GABLE TRUSS, AND BUILDING LATERAL BRACING NOTES:

1. EACH JAM POST FOR BIFOLD DOOR AND FOUNDATION FOR JAM POST TO BE DESIGNED TO RESIST 2700 lb LATERAL LOAD FROM THE GABLE END DRAG TRUSS IN THE TRANSVERSE DIRECTION (5400 lb TOTAL OR 120 pif IN PLANE WITH GABLE TRUSS).
2. GABLE END GIRDER TRUSS IS TO BE DESIGNED AS A DRAG TRUSS TO TRANSFER 5400 lb (120 pif) IN PLANE LATERAL LOAD FROM THE ROOF DIAPHRAGM TO THE JAMB POSTS (2700 lb EACH JAMB POST).
3. GABLE END GIRDER TRUSS IS ALSO TO BE DESIGNED TO SUPPORT REACTIONS FROM WEIGHT OF BIFOLD DOOR IN OPENED AND CLOSED POSITIONS, WIND LOADS, AND ROOF LOADS. BUILDER IS TO FURNISH DOOR ENGINEERING TO TRUSS ENGINEER.
4. GABLE END TRUSS MUST BE BRACED TO RESIST LATERAL LOADS.
5. BUILDER MUST FURNISH SHOP DRAWINGS SPECIFYING THE DOOR, JAM POST, FOUNDATION, DRAG TRUSS, AND ALL CONNECTIONS FOR APPROVAL BY WINDLOAD ENGINEER PRIOR TO PERMITTING. BIFOLD DOOR DATA, TRUSS ENGINEERING AND JAMB COLUMN SPECS WERE NOT AVAILABLE FOR REVIEW BY WINDLOAD ENGINEER AT TIME THIS DOCUMENT WAS SEALED. BUILDING OFFICIAL MUST MAKE SURE DOCUMENTATION AGREES WITH 1 - 4 ABOVE.

FLOOR PLAN
SCALE: 1/4\"/>

AREA SUMMARY

HANGER AREA	1840	S . F .
TOTAL AREA	1840	S . F .

WINDLOAD ENGINEER: Mark Disosway,
PE No. 53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1609, Florida building code 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

Mark Disosway
25SEP06
SEAL

Robert Foti
Hanger

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OF 6 SHEETS

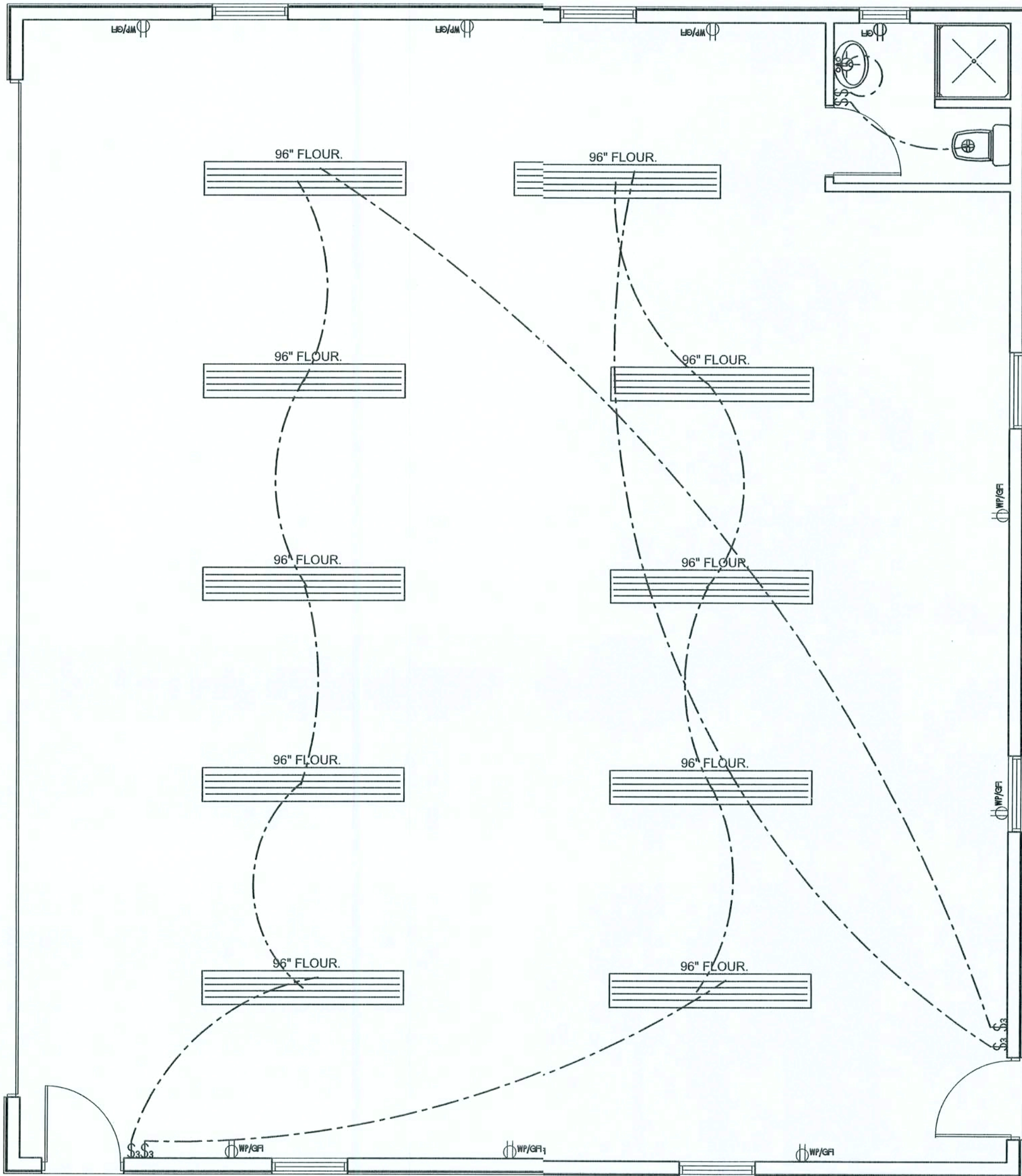
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21Feb06	



ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING.
SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

WINDL OAD ENGINEER: Mark Disosway,
PE No.53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
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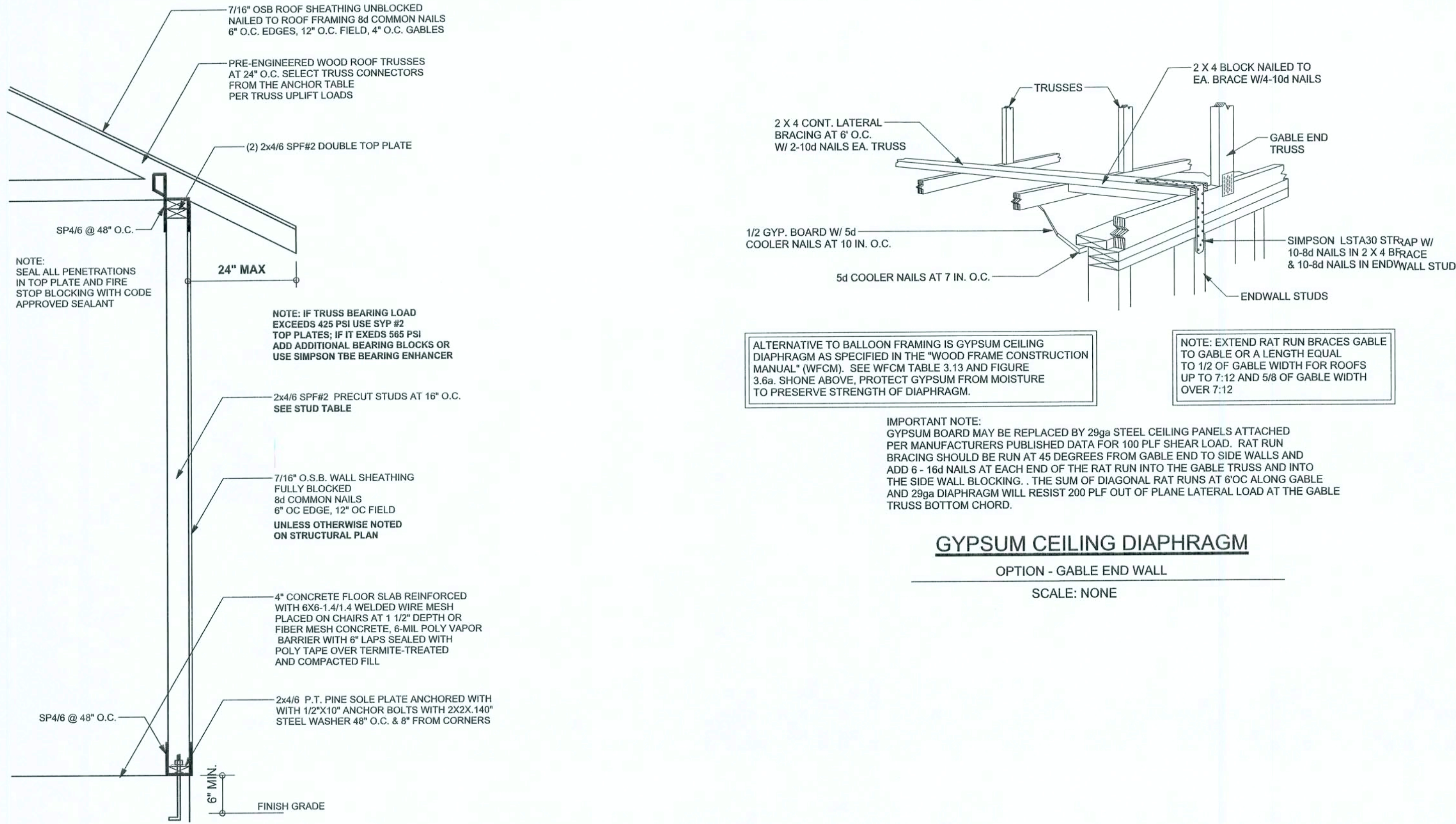
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OF 6 SHEETS



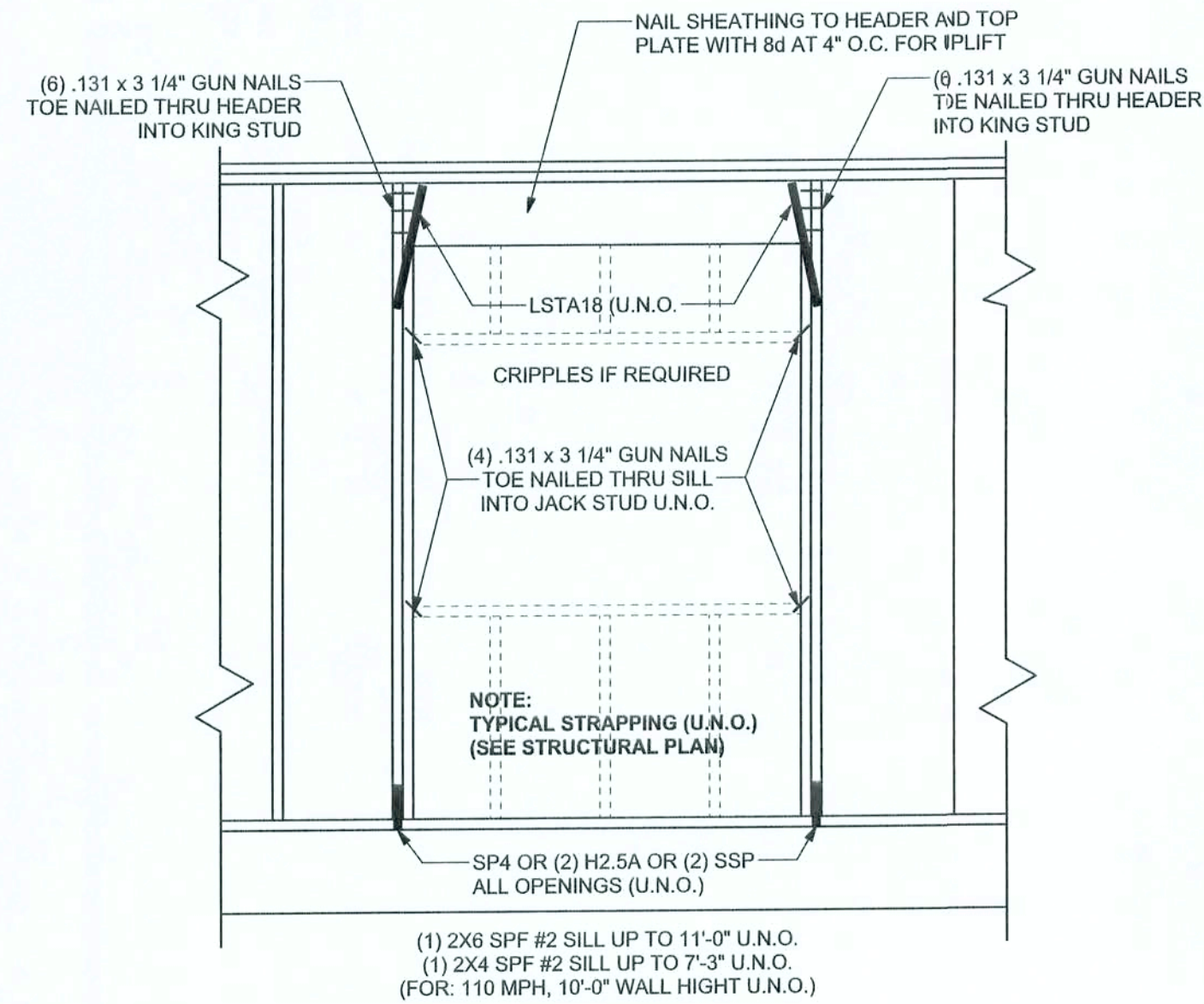
ONE STORY WALL SECTION

SCALE: 3/4\" = 1'-0"

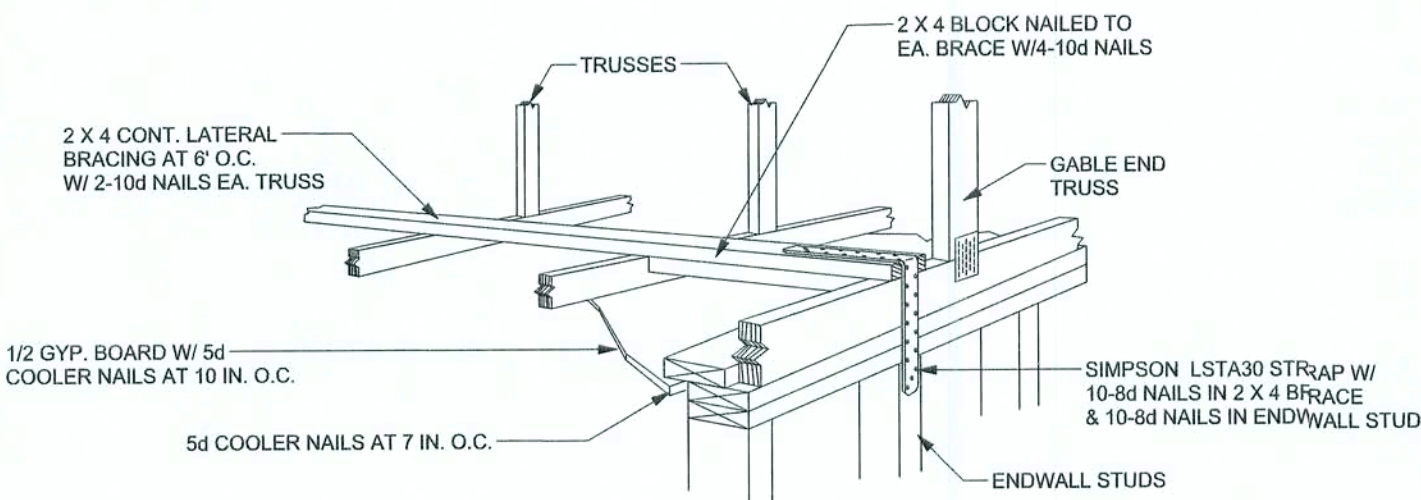
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16\" OC	TO 11'-9\" STUD HEIGHT
(1) 2x4 @ 12\" OC	TO 13'-0\" STUD HEIGHT
(1) 2x6 @ 16\" OC	TO 18'-10\" STUD HEIGHT
(1) 2x6 @ 12\" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MAINTAINED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16\" O.C. 0.85 = 13.6\" O.C.



TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2\" = 1'-0"



ALTERNATIVE TO BALLOON FRAMING IS GYPSUM CEILING DIAPHRAGM AS SPECIFIED IN THE "WOOD FRAME CONSTRUCTION MANUAL" (WFCM). SEE WFCM TABLE 3.19 AND FIGURE 3.9a. SHOWN ABOVE, PROTECT GYPSUM FROM MOISTURE TO PRESERVE STRENGTH OF DIAPHRAGM.

NOTE: EXTEND RAT RUN BRACES GABLE TO GABLE OR A LENGTH EQUAL TO 1/2 OF GABLE WIDTH FOR ROOFS UP TO 7:12 AND 5/8 OF GABLE WIDTH OVER 7:12

IMPORTANT NOTE:
GYPSUM BOARD MAY BE REPLACED BY 29ga STEEL CEILING PANELS ATTACHED PER MANUFACTURERS PUBLISHED DATA FOR 100 PLF SHEAR LOAD. RAT RUN BRACING SHOULD BE RUN AT 45 DEGREES FROM GABLE END TO SIDE WALLS AND ADD 6 - 16d NAILS AT EACH END OF THE RAT RUN INTO THE GABLE TRUSS AND INTO THE SIDE WALL BRACING. THE SUM OF DIAGONAL RAT RUNS AT 60C ALONG GABLE AND 29ga DIAPHRAGM WILL RESIST 200 PLF OUT OF PLANE LATERAL LOAD AT THE GABLE TRUSS BOTTOM CHORD.

GYPSUM CEILING DIAPHRAGM

OPTION - GABLE END WALL

SCALE: NONE

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGN FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSL

WELDED WIRE REINFORCED SLAB: 12\" x 12\" W/4 x W/4, Fy = 60KSI, WELDED WIRE REINFORCEMENT FABRIC (W/ W/4) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 18FT. DO NOT CUT W/WM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI. ALL LAP SPLICES 40\" DB (25\" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2,400 psi, E = 1,800,000 psi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16\" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6\" OC PANEL EDGES, 12\" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4\" OC UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\" IN CONCRETE OR REINFORCED BOND BEAM OR 10\" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2\" BOLTS TO BE 2\" x 2\" x 9/64\", WITH 5/8\" BOLTS TO BE 3\" x 3\" x 9/64\"; WITH 3/4\" BOLTS TO BE 3\" x 3\" x 9/64\", WITH 1\" BOLTS TO BE 3\" x 3\" x 9/64\", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS

MASONRY NOTES:

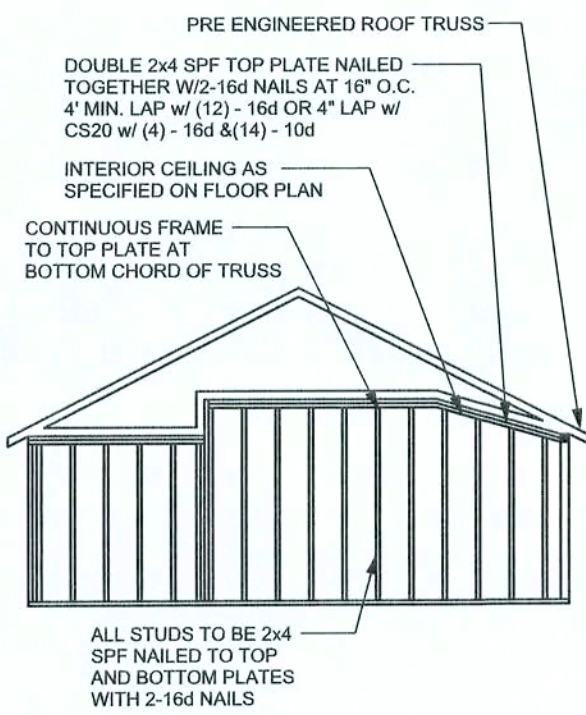
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

		Fb (psi)	E (10 ³ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

		Specific Requirements
1.4A	Compressive strength	8\" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 475, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8\"x8\"x16\" running bond and 12\"x12\" or 16\"x16\" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5\"x2.75\"x11.5\"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30\" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 800, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ³ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 305	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 880	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG12	14 - 16d	14 - 16d	
		HEAVY ORDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	NGT		22 - 10d	1-5/8\" THREADED ROD 12\" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8\" THREADED ROD 12\" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8\" THREADED ROD 12\" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8\" THREADED ROD 12\" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 8d		
< 1705	< 1705	CS16	28 - 8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8 - 16d		1/2\" AB
< 2310	< 2310	LTT131	18 - 10d, 1 1/2"		1/2\" AB
< 2775	< 2570	H02A	2-5/8\" BOLTS		5/8\" AB
< 4175	< 3695	HTT16	18 - 16d		5/8\" AB
< 1400	< 1400	PAHD42	16 - 16d		
< 3335	< 3335	HPHD22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1/2\" AB
< 2300	< 2300	ABU66	12 - 16d		1/2\" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8\" AB

DESIGN DATA

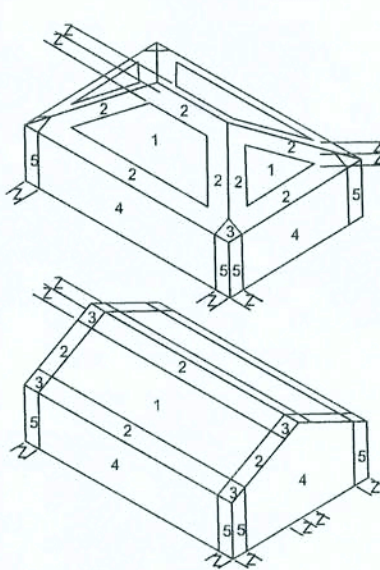
WIND LOADS PER FLORIDA BUILDING CODE 2004, SECTION 1609

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND > 10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING, 1609)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (FBC TABLE 1609 B&C)



Zone	Effective Wind Area (ft ²)		
	10	100	
1	19.9	-21.8	18.1 -18.1
2	19.9	-25.5	18.1 -21.8
2 O'Hg		-40.6	-40.6
3	19.9	-25.5	18.1 -21.8
3 O'Hg		-48.3	-42.4
4	21.8	-23.6	18.5 -20.4
5	21.8	-29.1	18.5 -22.6
Doors & Windows Worst Case (Zone 5, 10 ft ²)		21.8	-29.1
8x7 Garage Door		19.5	-22.9
16x7 Garage Door		18.5	-21.0

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SLAB BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

21Feb06

SOFTMAN
ARCHITECTURAL DESIGN & ENGINEERING

WINDLOAD ENGINEER: Mark Disoway, PE No. 53915, POB 868, Lake City, FL 33556, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1609, Florida building code 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

SEAL

Robert Foti
Hanger

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PRINTED DATE:
September 25, 2006

DRAWN BY: David Disoway

CHECKED BY:

FINALS DATE:
25 / Sep / 06

JOB NUMBER:
608291

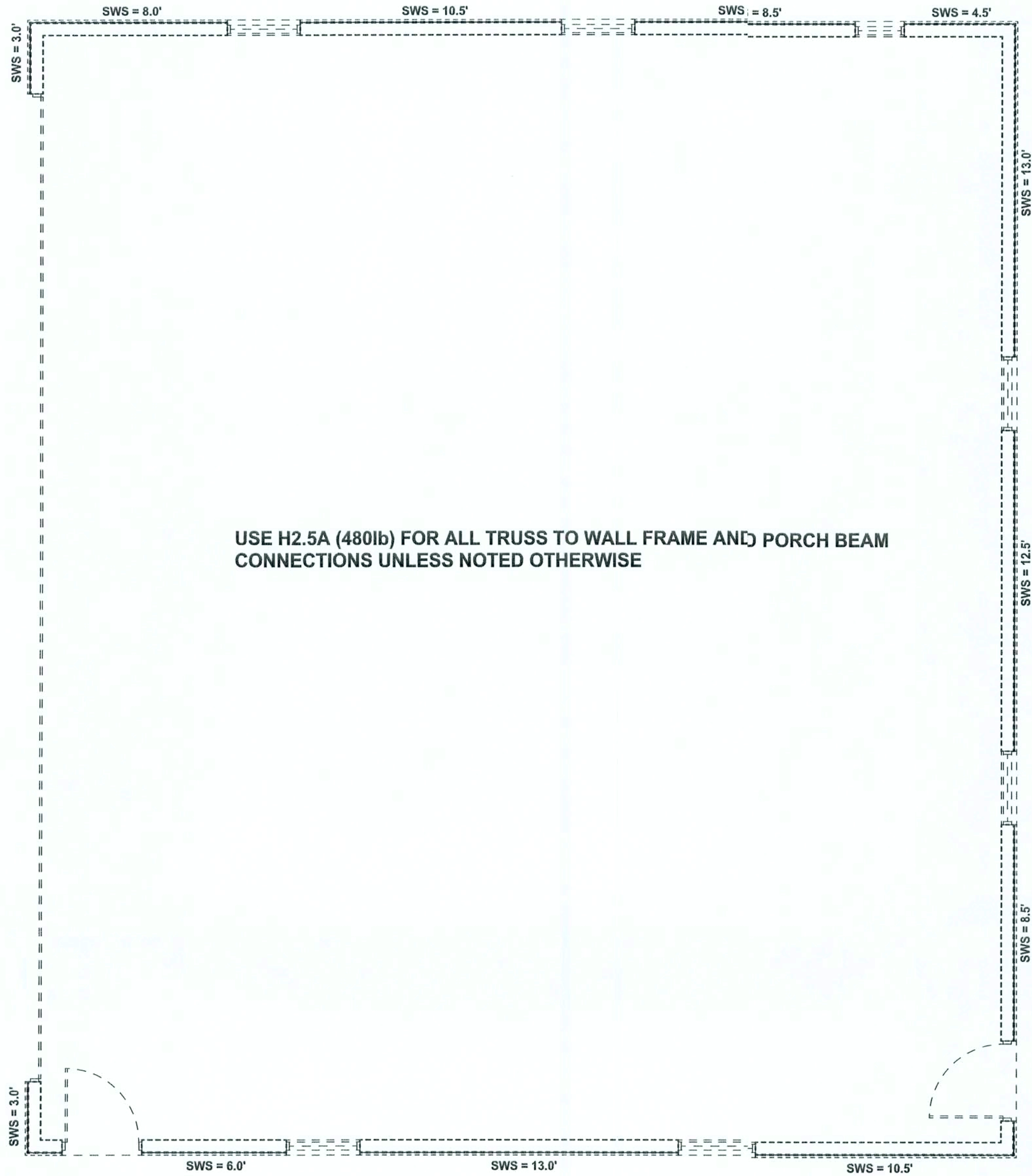
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S-1

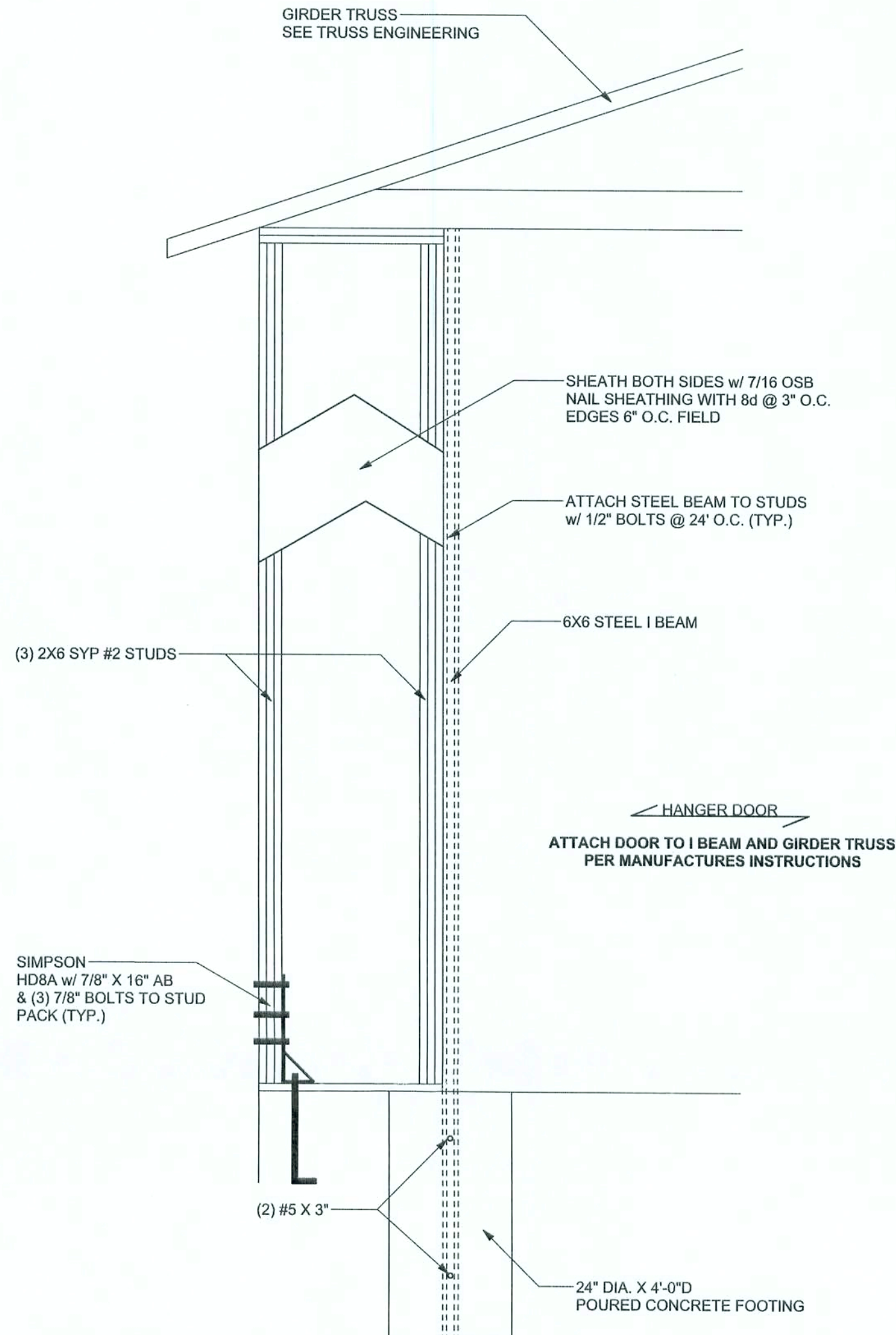
OF 6 SHEETS

REVISIONS	
21Feb06	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE



HANGER DOOR WALL DETAIL
SCALE: 1/2" = 1'-0"

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND

(2) 2X12X0', 1J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
NUMBER OF KING STUDS (FULL LENGTH)	
NUMBER OF JACK STUDS (UNDER HEADER)	
SPAN OF HEADER	
SIZE OF HEADER MATERIAL	
NUMBER OF PLIES IN HEADER	

TOTAL SHEAR WALL SEGMENTS
SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	26.5'	61.0'
LONGITUDINAL	23.5'	40.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER: W.B. HOWLAND JOB #3865

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Set/dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1609, Florida building code 204, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

Robert Foti
Hanger

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DRAWING NUMBER
S-3
OF 6 SHEETS