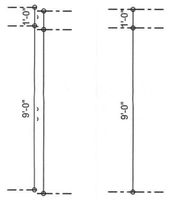
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

REQUIRED ROOF VENTILATION:

AS PER FLORIDA BUILDING CODE 2309.7

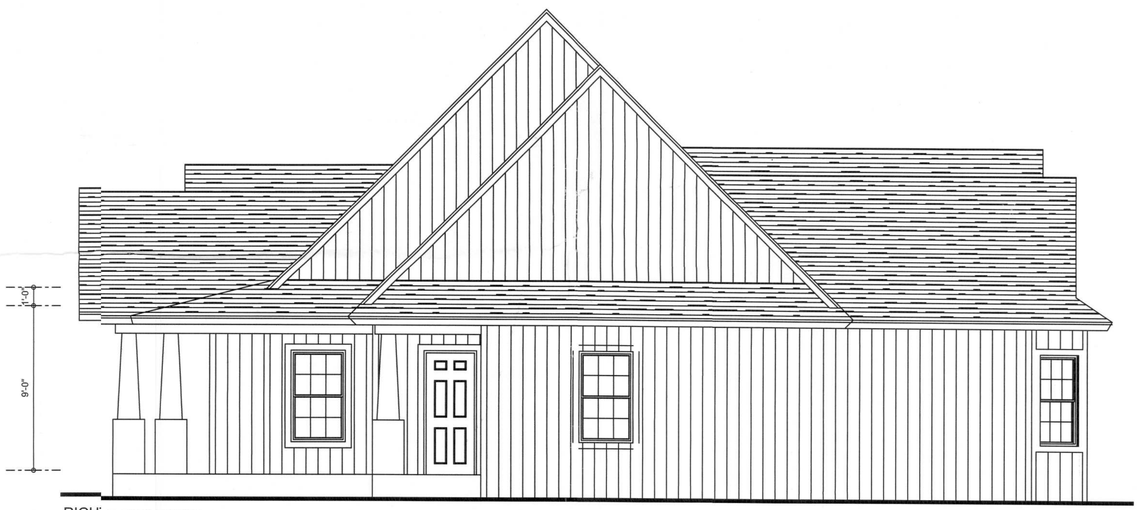
RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
1418 S.F. / 300 x 50% = 2.36 S.F. & RIDGE VENT AREA REQUIRED
22 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
1418 S.F. / 300 x 50% = 2.36 S.F. & SOFFIT VENT AREA REQUIRED
79 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 & PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)

REAR ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

JoshSparks
Consruction

Ben & Anne
Sparks

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Lake City,Florida 32024

PRINTED DATE:
October 13, 2010

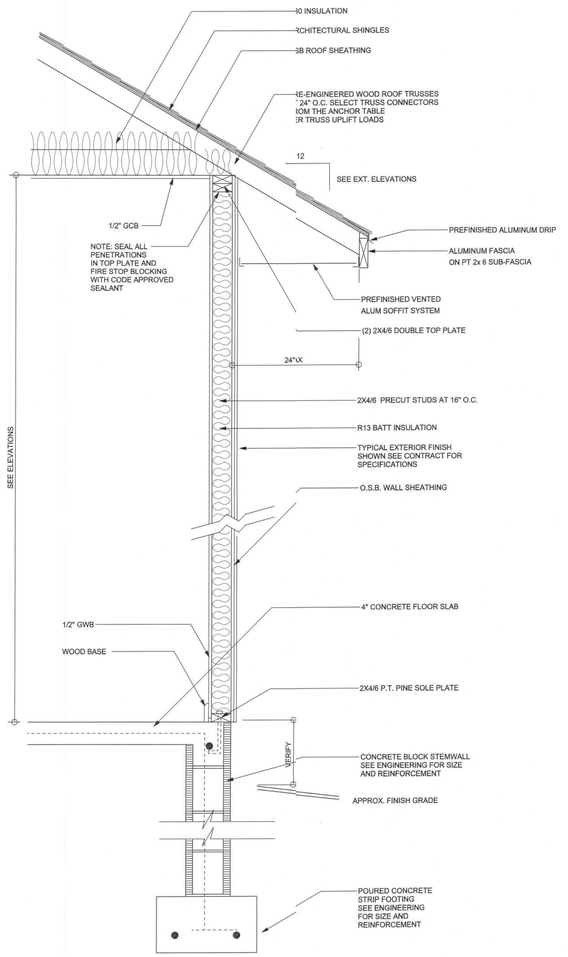
DRAWN BY: Ben Sparks
CHECKED BY:

DESIGNED BY:

FINALS DATE:
31 / Jan / 08

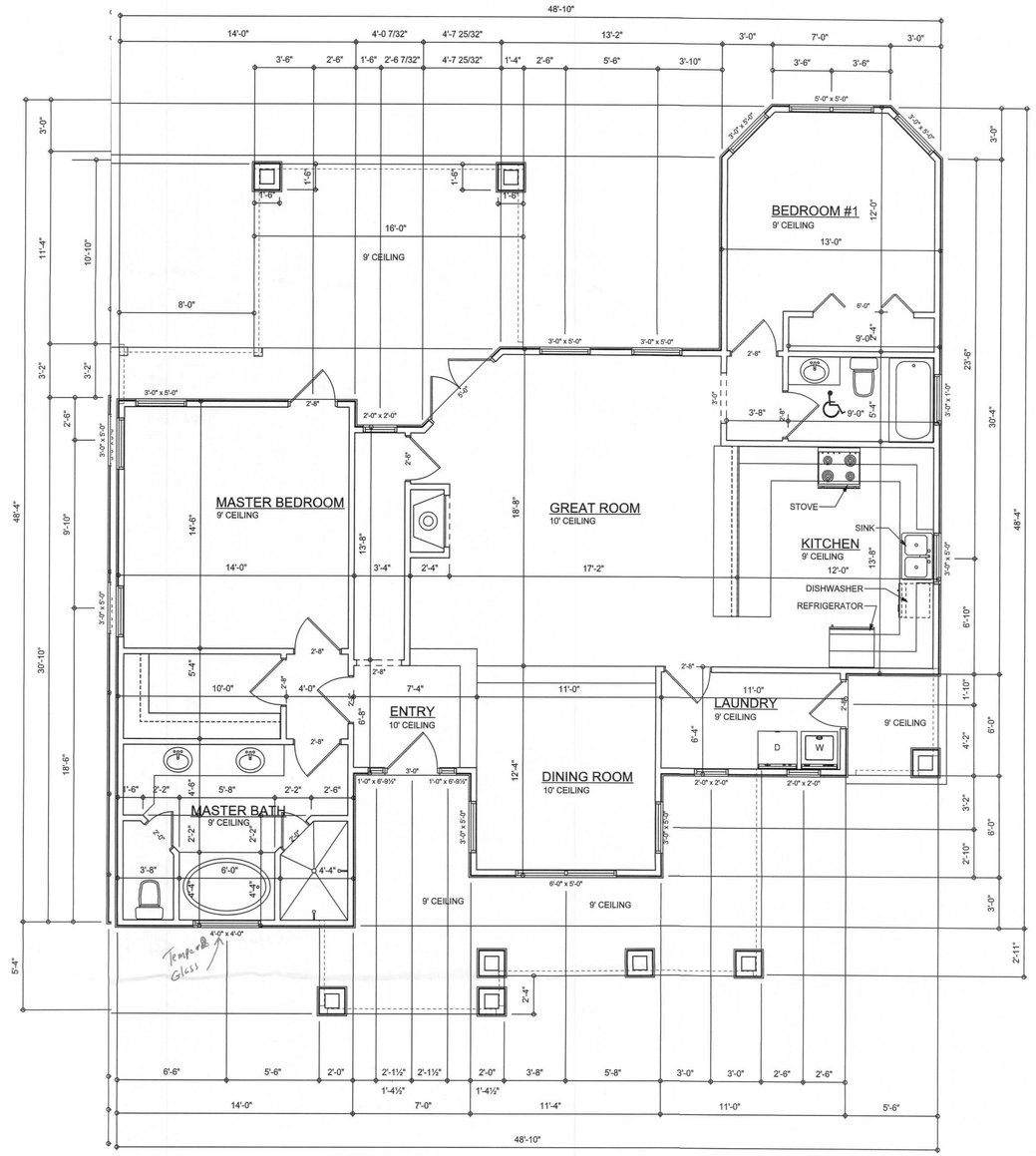
JOB NUMBER:
108031

DRAWING NUMBER
1
OF 3SHEETS



TYPICAL DESIGN WALL SCTION
NON - STRUCTURAL DAT.

SCALE: 1" = 1'- 0"



AREA SUMMARY

LIVING AREA	1418	S . F .
PORCH AREA	557	S . F .
TOTAL AREA	1975	S . F .

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

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2
OF 3 SHEETS

REVISIONS	

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ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN NOTES

E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.

E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

Also All others Rec. per 2008 NEC. Receptacles must be Tamper Proof

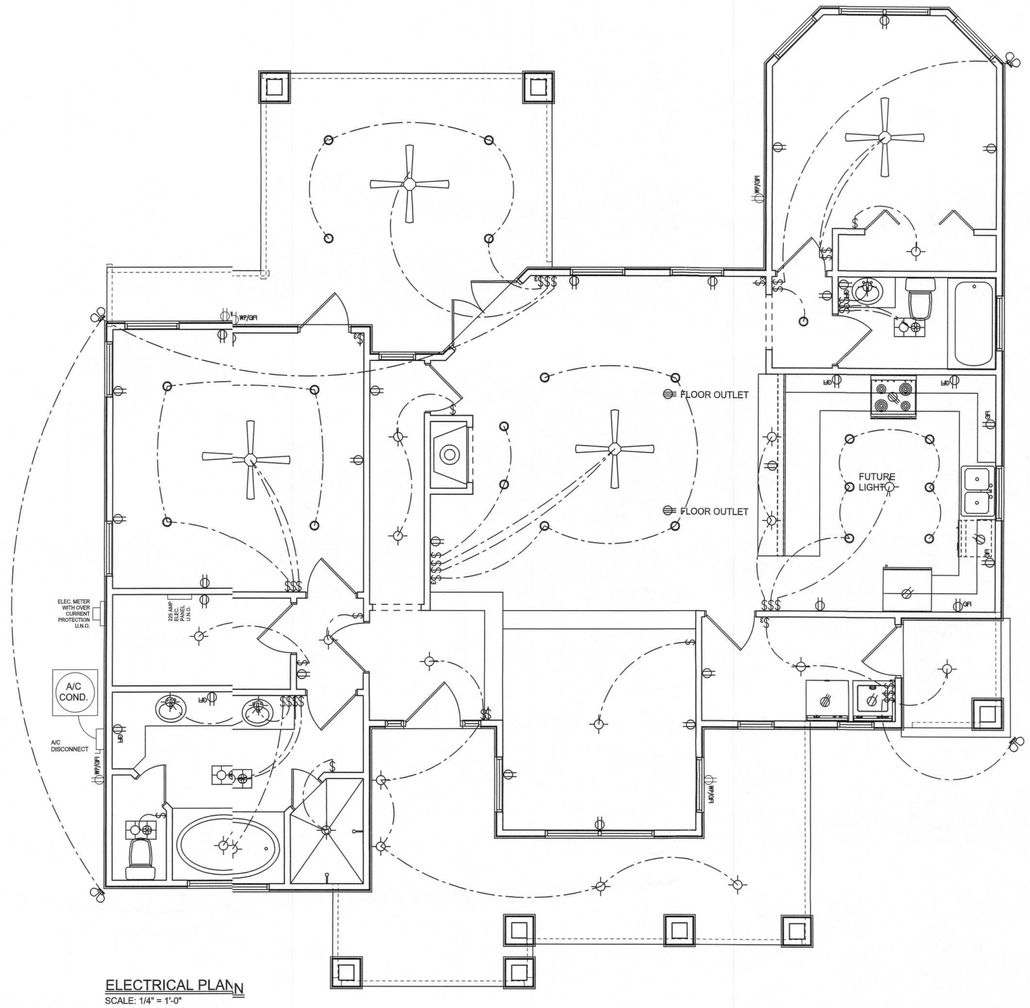
E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

Ground to Meter In Footer

E -11 CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXAUST FAN WITH LIGHT
	BATH EXAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
GFI	GFI DUPLEX OUTLET
	SMOKE DETECTOR
$	WALL SWITCH
$3	3 WAY WALL SWITCH
$4	4 WAY WALL SWITCH
WP/GFI	WATER PROOF GFI OUTLET
	PHONE JACK
TV	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM



ELECTRICAL PLANN
SCALE: 1/4" = 1'-0"

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31 / Jan / 08

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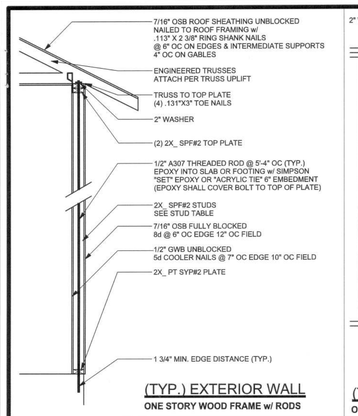
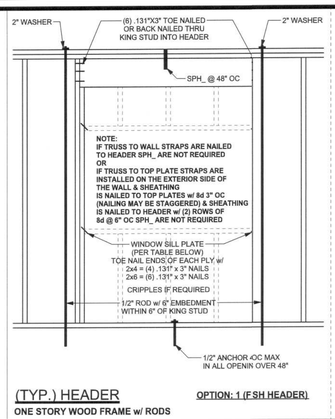
108131

DRAWING NUMBER
3
OF 3 SHEETS



(TYP.) EXTERIOR WALL

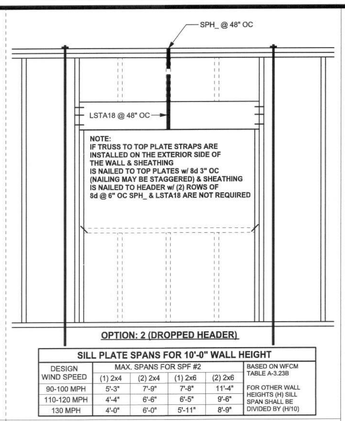
ONE STORY WOOD FRAME w/ RODS



(TYP.) HEADER

ONE STORY WOOD FRAME w/ RODS

OPTION: 1 (FSH HEADER)



OPTION: 2 (DROPPED HEADER)

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.23B
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
90-100 MPH	5'-3"	7'-9"	7'-8"	11'-4"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)
110-120 MPH	4'-4"	6'-6"	6'-5"	9'-6"	
130 MPH	4'-0"	6'-0"	5'-11"	8'-9"	

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF
H5	455	265
H3	415	290
H2.5	415	365
H2.5A	480	480
H6	950	820
H8	745	565
H14-1	1465	1050
H14-2	1465	1050
H10	990	850
H10-2	760	655
H16	1470	1265
H16-2	1470	1265
LTS12 - LTS20	1000	620
MTS12 - MTS30	1000	860
HTS16 - HTS30	1450	1245
HEAVY GIRDER TIEDOWNS		
LGT2	2050	1785
LGT3-SDS2.5	3685	2655
LGT4-SDS3	4060	3860
MGT	3965	3330
HGT-2	10980	6485
HGT-3	10530	9035
HGT-4	9250	9250
STUD STRAP CONNECTOR		
SSP DOUBLE TOP PLATE	435	435
SSP SINGLE SILL PLATE	455	420
DSP DOUBLE TOP PLATE	825	825
DSP SINGLE SILL PLATE	825	600
SP1	585	535
SP2	1065	605
SP4	885	760
SPH4	1240	1065
SP6	885	760
SPH6	1240	1065
LSTA18	1235	1110
LSTA21	1235	1235
CS20	1030	1030
CS16	1705	1705
STUD ANCHORS		
LTT19	1350	1305
LTT31	2310	2310
HD2A	2775	2570
HTT16	4175	3695
HTT22	5260	5250
ABU44	2200	2200
ABU66	2300	2300
ABU88	2320	2320

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES	
115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"	
125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"	
150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"	
110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"	
				8-8d	8-8d	
				5-10d x 1 1/2"	5-10d x 1 1/2"	
515	265	480	245	12-8d x 1 1/2"	13-8d	
515	265	480	245	12-8d x 1 1/2"	15-8d	
585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"	
455	395	390	340	6-10d	6-10d	
				2-10d x 1 1/2"	10-10d x 1 1/2"	
				2-10d x 1 1/2"	10-10d x 1 1/2"	
				6-10d x 1 1/2"	6-10d x 1 1/2"	
				7-10d x 1 1/2"	7-10d x 1 1/2"	
				12-10d x 1 1/2"	12-10d x 1 1/2"	
						TO FOUNDATION
700	170	700	170	14-16d	14-16d	
795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS	
2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS	
				22 -10d		5/8" ANCHOR
				16 -10d		2-5/8" ANCHOR
				16 -10d		2-5/8" ANCHOR
				16 -10d		2-5/8" ANCHOR
						TO STUDS
					3-10d	4 -10d
					1-10d	4 -10d
					6 -10d	8 -10d
					2 -10d	8 -10d
					4 -10d	6 -10d
					6 -10d	6 -10d
						6-10d x 1 1/2"
						10-10d x 1 1/2"
						6-10d x 1 1/2"
						10-10d x 1 1/2"
						14-10d
						16-10d
						14-10d
						22-10d
				TO STUDS		TO FOUNDATION
				8-16d		1/2" ANCHOR
				18-10d x 1 1/2"		5/8" ANCHOR
				2-5/8" BOLTS		5/8" ANCHOR
				18-16d		5/8" ANCHOR
				32-16d		5/8" ANCHOR
				12-16d		5/8" ANCHOR
				12-16d		5/8" ANCHOR
				18-16d		2-5/8" ANCHOR

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131\), @"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 5/16"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

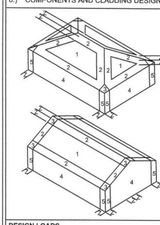
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

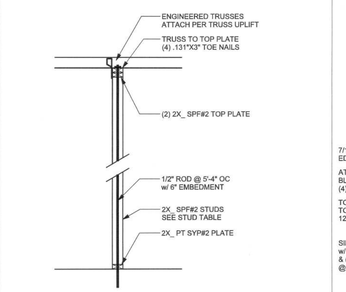
DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1	
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED = 110 MPH	
2.) WIND EXPOSURE = C	
3.) WIND IMPORTANCE FACTOR = 1.0	
4.) BUILDING CATEGORY = II	
5.) ROOF ANGLE = 10-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	



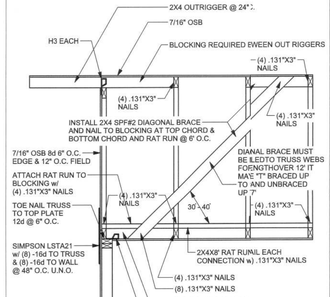
Zone	Effective Wind Area (ft2)			
	10		100	
1	27.8	-30.5	25.3	-25.3
2	27.8	-35.7	25.3	-30.5
2 O'hg		-56.8		-56.8
3	27.8	-35.7	25.3	-30.5
3 O'hg		-95.6		-59.3
4	30.5	-33.0	25.9	-28.5
5	30.5	-40.7	25.9	-31.6
Doors & Windows Worst Case (Zone 5, 10 ft2)	30.5	-40.7		
8x7 Garage Door	27.3	-32.0		
16x7 Garage Door	25.9	-29.4		

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	



(TYP.) INTERIOR BEARING WALL

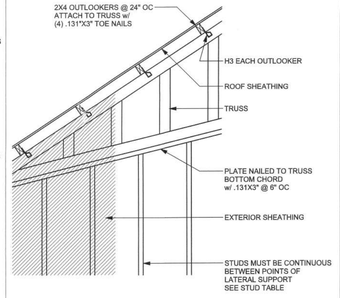
ONE STORY WOOD FRAME w/ RODS



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. CNCLOSED

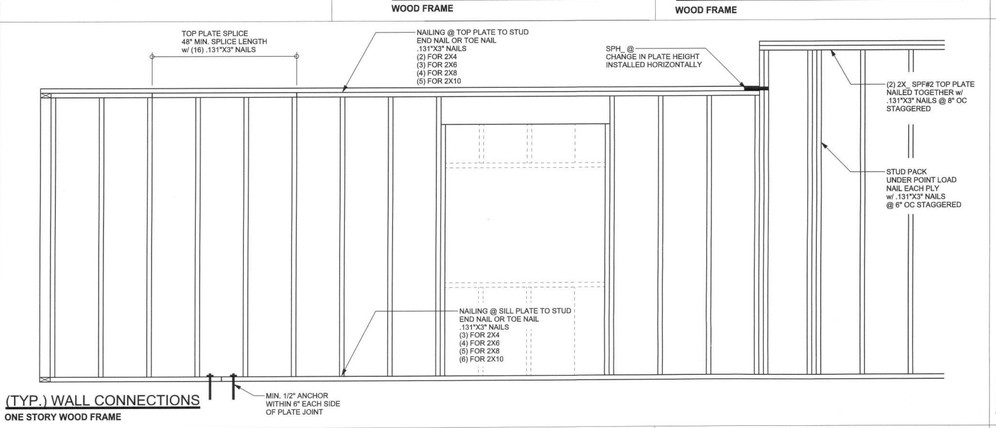
(TYP.) GABLE BRACING DEAIL

WOOD FRAME



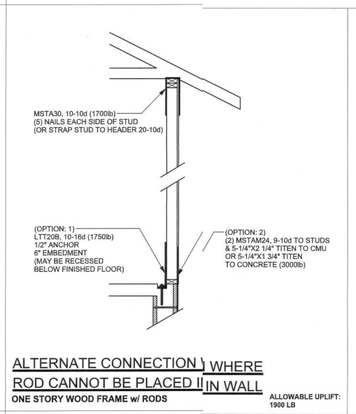
(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME



(TYP.) WALL CONNECTIONS

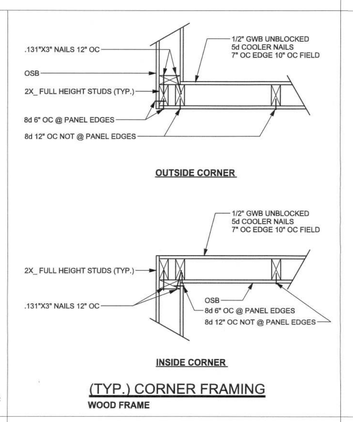
ONE STORY WOOD FRAME



ALTERNATE CONNECTION WHERE ROD CANNOT BE PLACED IN WALL

ONE STORY WOOD FRAME w/ RODS

ALLOWABLE UPLIFT: 1900 LB

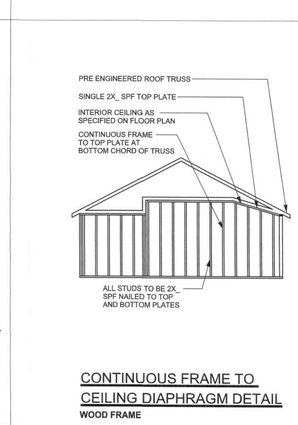


OUTSIDE CORNER

INSIDE CORNER

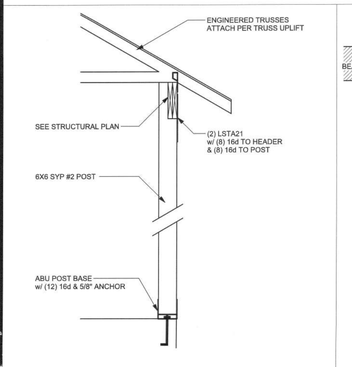
(TYP.) CORNER FRAMING

WOOD FRAME



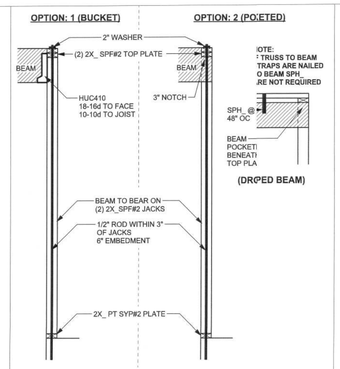
CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

WOOD FRAME



(TYP.) PORCH POST

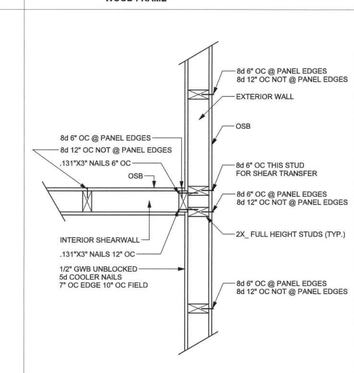
ONE STORY WOOD



(TYP.) BEAM TO WALL

WOOD FRAME w/ RODS

ALLOWABLE UPLIFT: 1770 LB



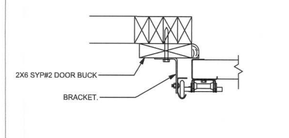
(TYP.) INTERSECTING WALL FRAMING

WOOD FRAME

2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC
20'	12" OC	2.5" OC	2.5" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION

WOOD FRAME

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoswav, PE No.53915, POB868, Lake City, FL 32056, 386-754-549

DIMENSIONS:
Stated dimensions upercade scaled dimensions. Refer il questions to Mark Disoswav, P.I. for resolution. Do not proceed without clarification.

CERTIFICATION: hereby certify that I have examined this plan,and that the applicable portions of the plannitating to wind engineering comply with sectior R301.2.1, florida building code residential 207, to the best of my knowledge.

LIMITATION: This esign is valid for one building, at specifie location.

MARK DISOSWAY
PE 53915
STATE OF FLORIDA
No. 53915
STATE OF
October 12, 2010
PROFESSIONAL ENGINEER

Sparks Construction

Ben& Anne
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AIDRESS:
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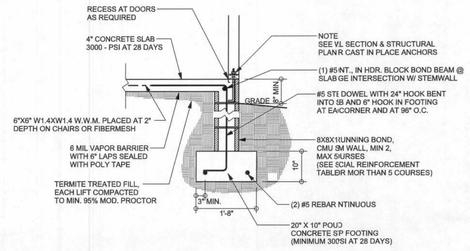
PRIITED DATE:
Octobr 12, 2010

DRAWN BY:	STRUCTURAL BY:
David Disosway	David Disosway

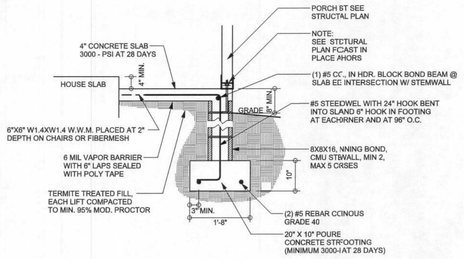
FINALS DATE:
12Oct10

JOB NUMBER:
1C10018

DRAWNG NUMBER
S-1
OF SHEETS



F9 / S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

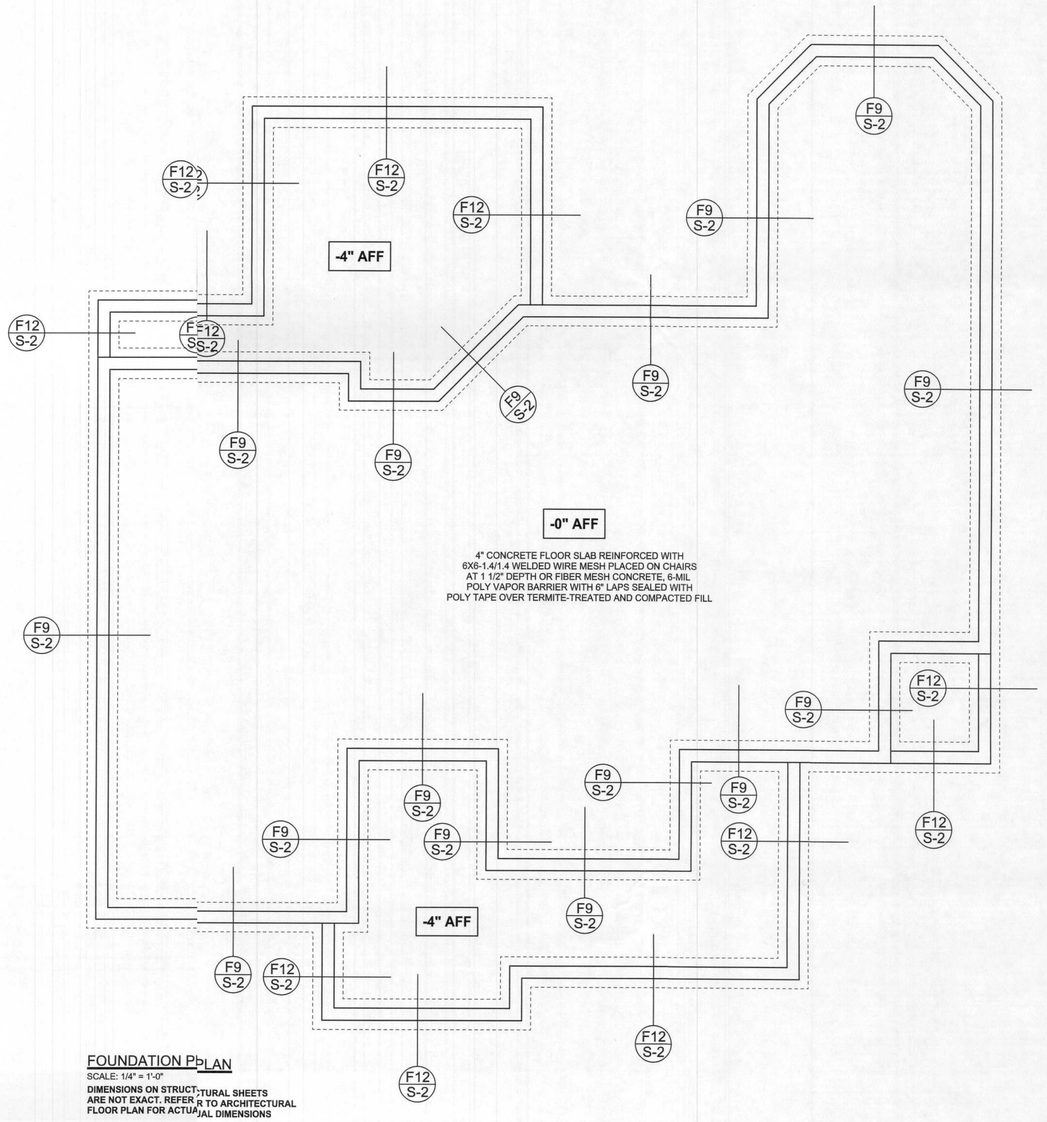


F12 / S-2 STEM WALL PORCH FOOTIG
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFOEMENT FOR 12" CMU STWALL (INCHES O.		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



4" CONCRETE FLOOR SLAB REINFORCED WITH 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS AT 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER WITH 6" LAPS SEALED WITH POLY TAPE OVER TERMITE-TREATED AND COMPACTED FILL

FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEERMark Disoway, PE No.53915, POB 868,,ake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all quetions to Mark Disoway, P.E. for esolution. Do not proceed without cnification.

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CERTIFICATION: I herely certify that I have examined this plan, and sat the applicable portions of the plan, relatng to wind engineering comply with section R30.2.1, florida building code residential 2007, tohe best of my knowledge.

LIMITATION: This desigis valid for one building, at specified locion.

MARK DISOWAY PE 53915 STATE OF FLORIDA LICENSED No. 53915 STATE OF STONAL ENGINEER October 1, 2010

Sparks Construction

Ben & Anne Sparks Residence

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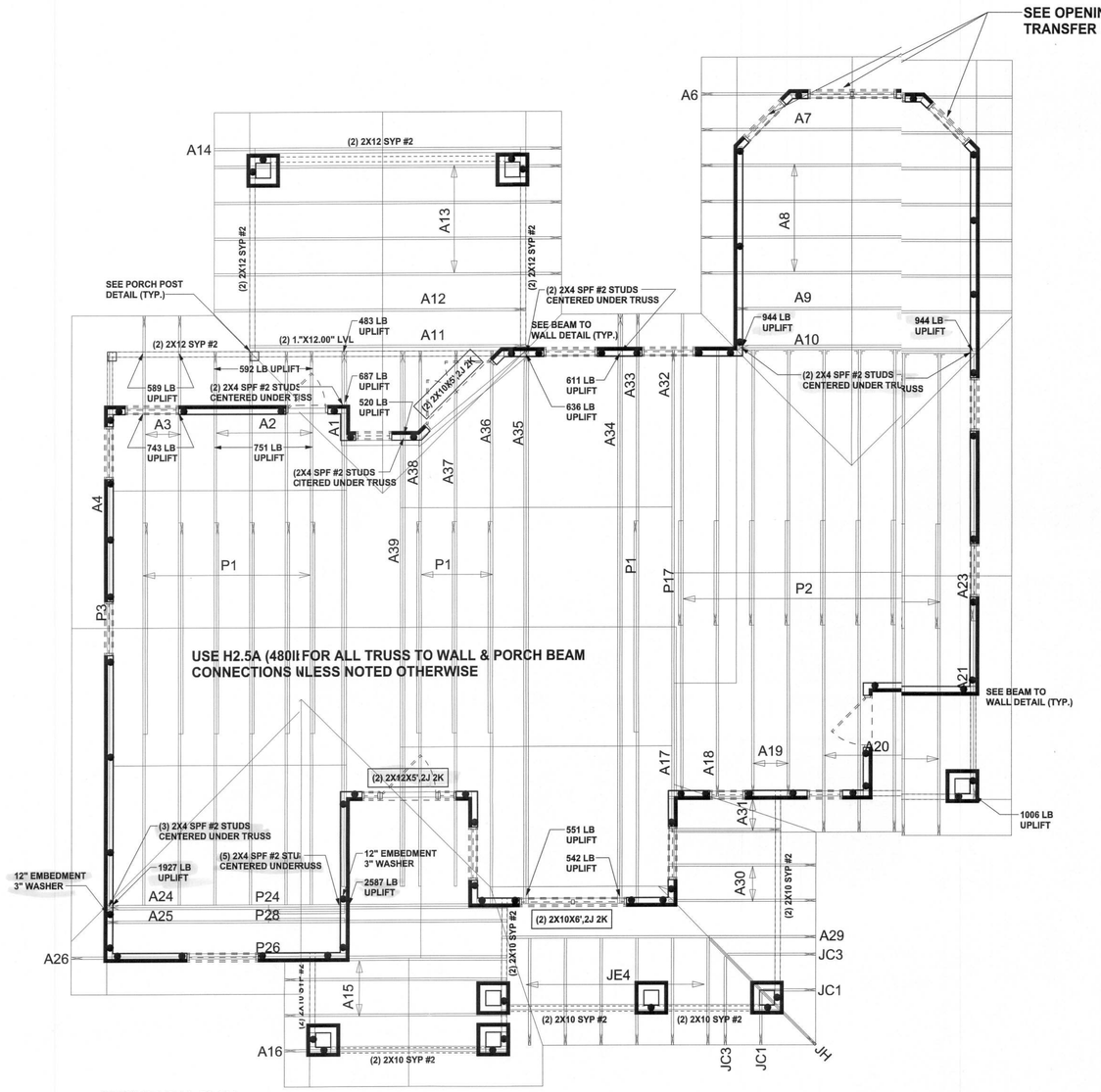
PRINTEDDATE: October 1 2010

DRAWN BY: David Disoway
STRUCTURAL BY: David Disoway

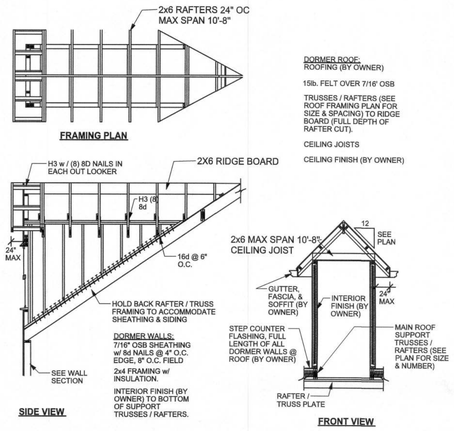
FINALS DATE: 12Oct10

JOB NUMBER: 1010)18

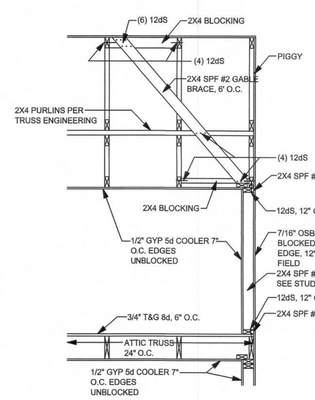
DRAWING IUMBER
S-2
OF 3 SHETS



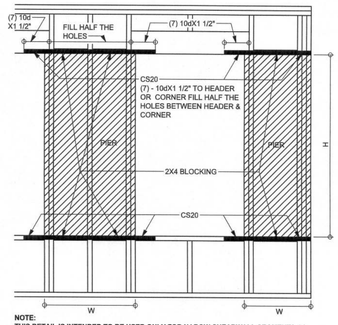
STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"



DORMER ANCHORING DETAIL (ON ROOF)
SCALE: N.T.S.



BONUS ROOM / GABLE END BRACING
SCALE: 1/2"=1'-0"



NOTE:
THIS DETAIL IS INTENDED TO BE USED ONLY FOR NAROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT H/W < 3.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGHT OR 5' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

OPENING FORCE TRANSFER
WOOD FRAME

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL ?ORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 S'¥2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL IADERS SHALL HAVE (1) JACK STUD & (1) K3 STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHITS ARE NOT EXACT, REFER TO ARCHICTURAL FLOOR PLAN FOR ACTUAL DIMENSNS

SN-4 PERMANENT TRUSS BRACING IS T3E INSTALLED AT LOCATIONS AS SHOWN ON THE SLED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTIINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSII, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SIPLIER, WITH THE SEALED TRUSS PACKAGE

THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

(2) 2X6X0',1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	46.8'	71.1'
LONGITUDINAL	33.2'	38.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND JOB # 16970B

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: lark Discsway, PE No.53915, POB 868, Lxe City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercee scaled dimensions. Refer all questns to Mark Discsway, P.E. for reolution. Do not proceed without clafication.

CERTIFICATION: I hereby ertify that I have examined this plan, and tha the applicable portions of the plan, relatin to wind engineering comply with section R301.1, florida building code residential 2007, to th best of my knowledge.

LIMITATION: This design ivalid for one building, at specified locatin.

MARK DISOSWAY P.E. 53915 STATE OF FLORIDA PROFESSIONAL ENGINEER LICENSED No. 53915 October 12,2010

Sparks Construction

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PRINTED DATE: October 12, 2010

DRAWN BY: David Disosway | STRUCTURAL BY: David Disosway

FINALS DATE: 12Oct10

JOB NUMBER: 1010018

DRAWING NUMBER **S-3**

OF 3 SHETS