

SOFTPIAN APCHECIBAL CERNADE

REVISIONS

### STRUCTURAL PLAN NOTES

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

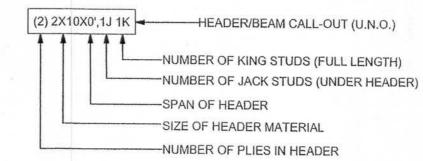
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3
ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

#### WALL LEGEND

\$\frac{1}{2} \frac{1}{2} \frac	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

### HEADER LEGEND



### TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.5'	60.0'
LONGITUDINAL	29.6'	48.0'

COPYRIGHT: AND PROPERTY RIGHTS:
Mark Disoswa, P.E. hereby expressly reserves
its common law copyrights and property right in
these instruments of service. This document is
not to be reproduced, altered or copied in any
form or manner without first the express written
permission and consent of Mark Disosway. CERTIFICATDN: I hereby certify that I have examined thisplan, and that the applicable portions of theplan, relating to wind engineerin comply with section R301.2.1, florida building code residental 2004, to the best of my knowledge. LIMITATION: This design is valid for one building, at specified location.

PE No.53915,POB 868, Lake City, FL 32056, 386-74-5419

DIMENSIONS
Stated dimensions supercede scaled dimensions. Fefer all questions to Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

John & Alice Webb Residence

ADDRESS: W Infiniti Place Columbia County, Florida rear half of 245S- 16-03707-009

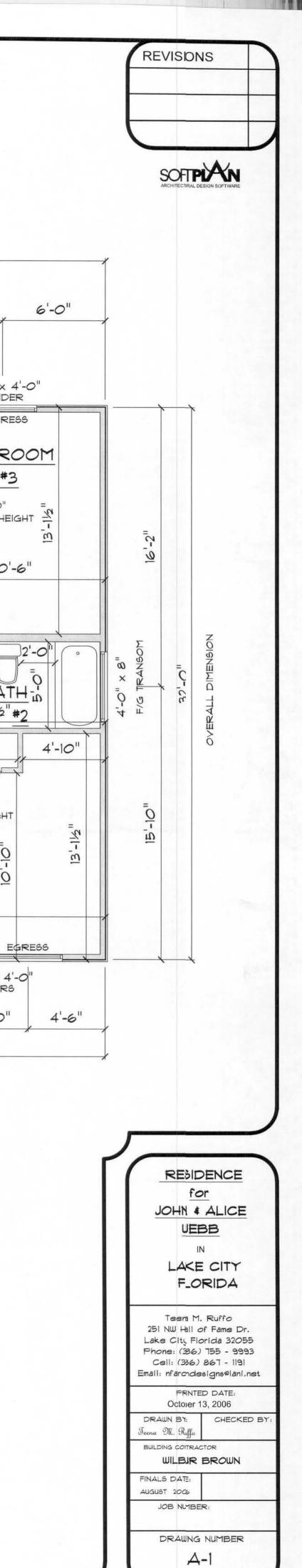
Marc Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: September 14, 2006 DRAWN BY: CHECKED BY: David Discway

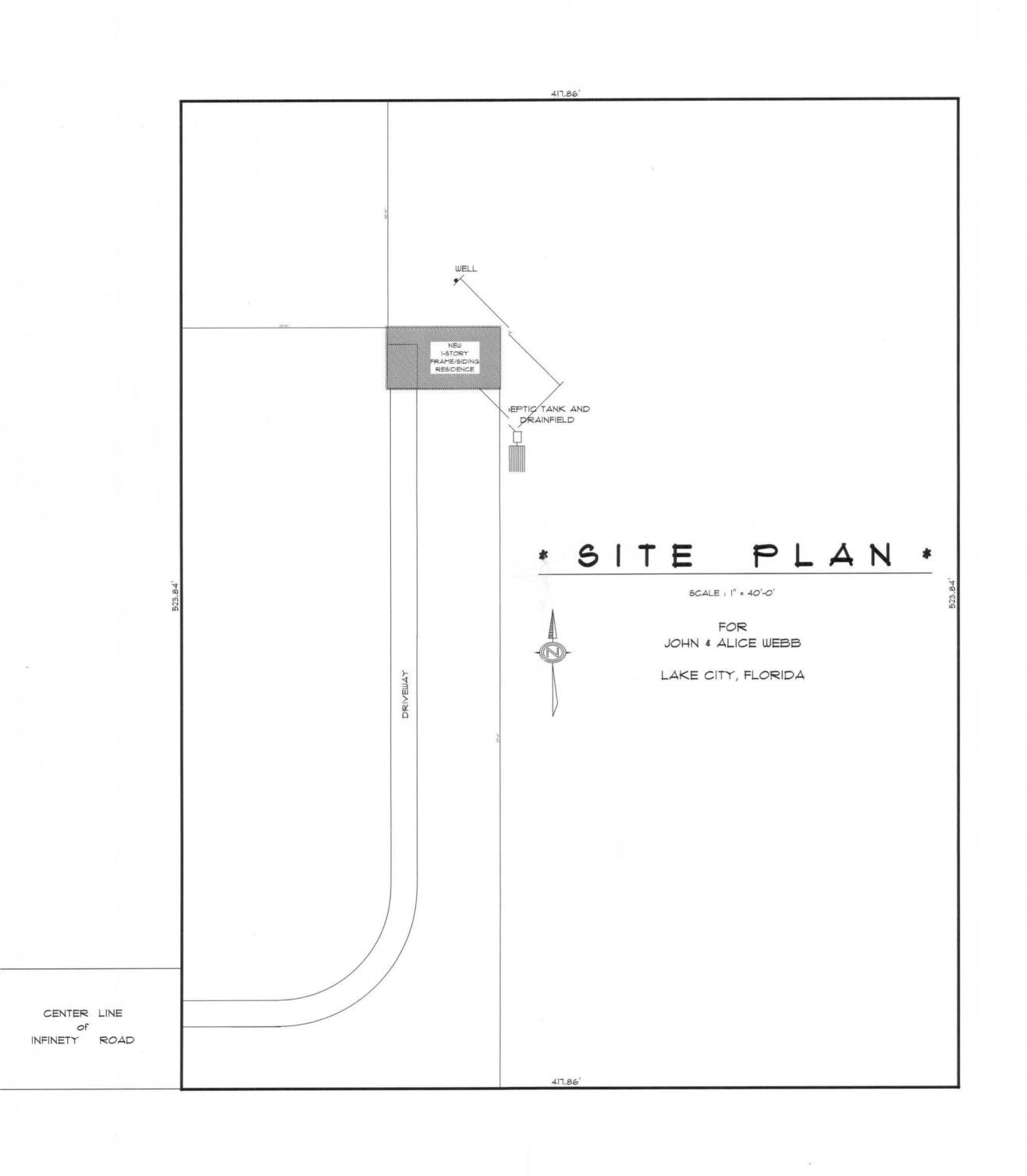
FINALS DATE: 14 / Sep '06

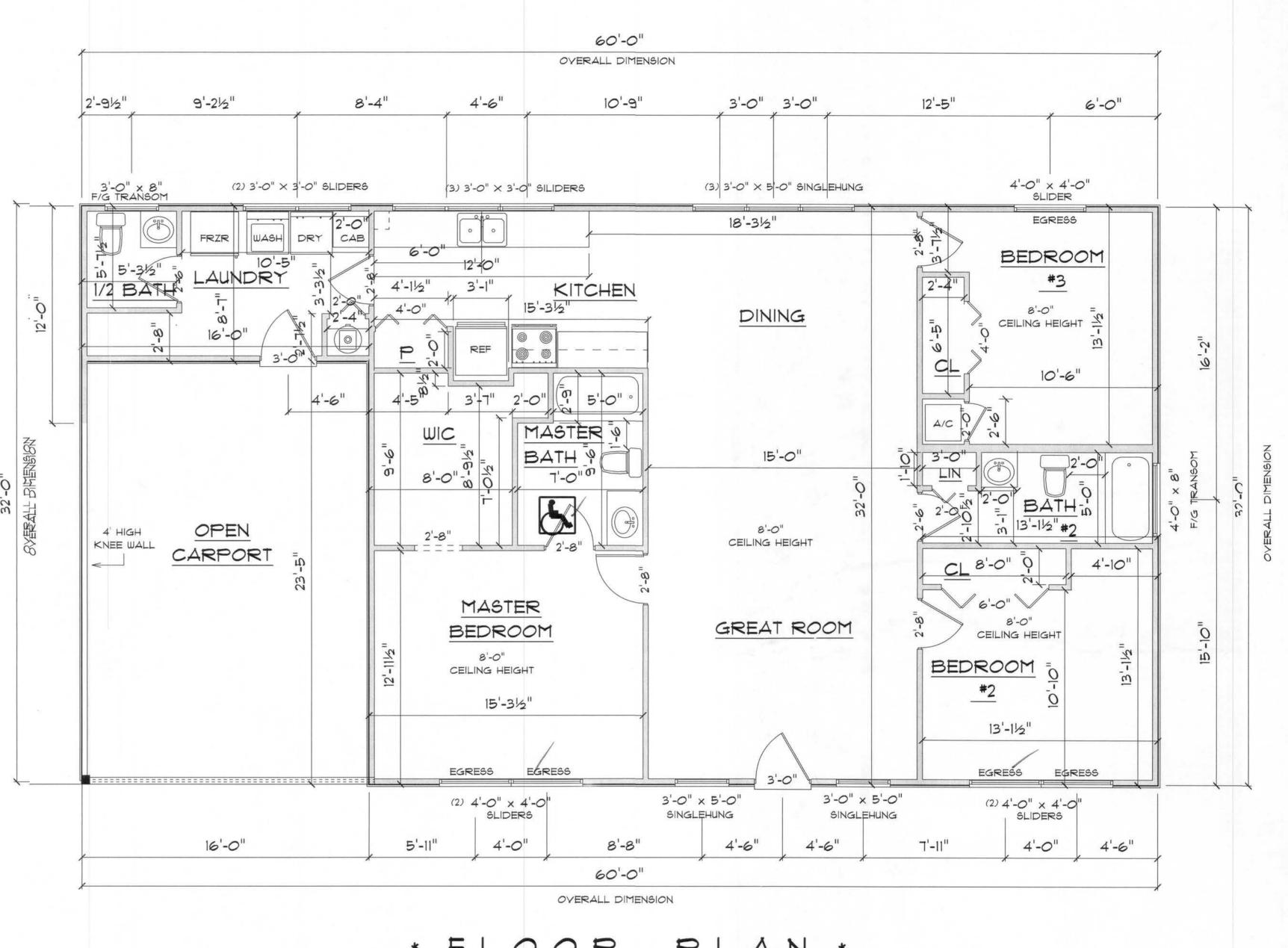
JOB NUMBER: 609052 CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING DRAWING NUMBER FURNISHED BY BUILDER. ANDERSON TRUSS CO. JOB #6-312

S-3 OF 3 SHEETS



OF 3 SHEETS





# \* FLOOR PLAN \*

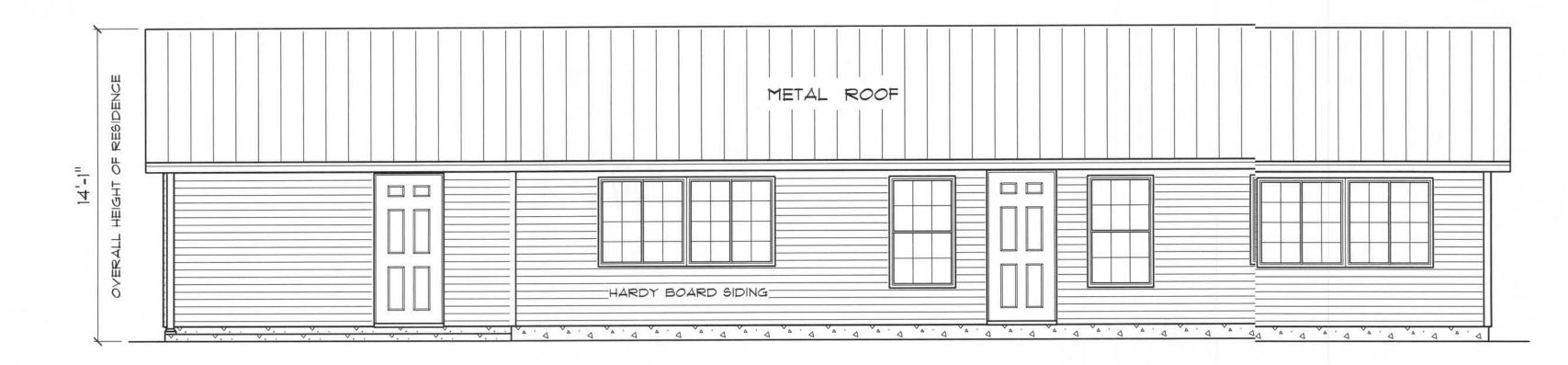
SCALE : 1/4" = 1'-0"

A/C LIVING AREA = 1408 S.F CARPORT = 519 S.F.

TOTAL AREA UNDER ROOF = 1927 S.F.

IMPORTANT NOTE:

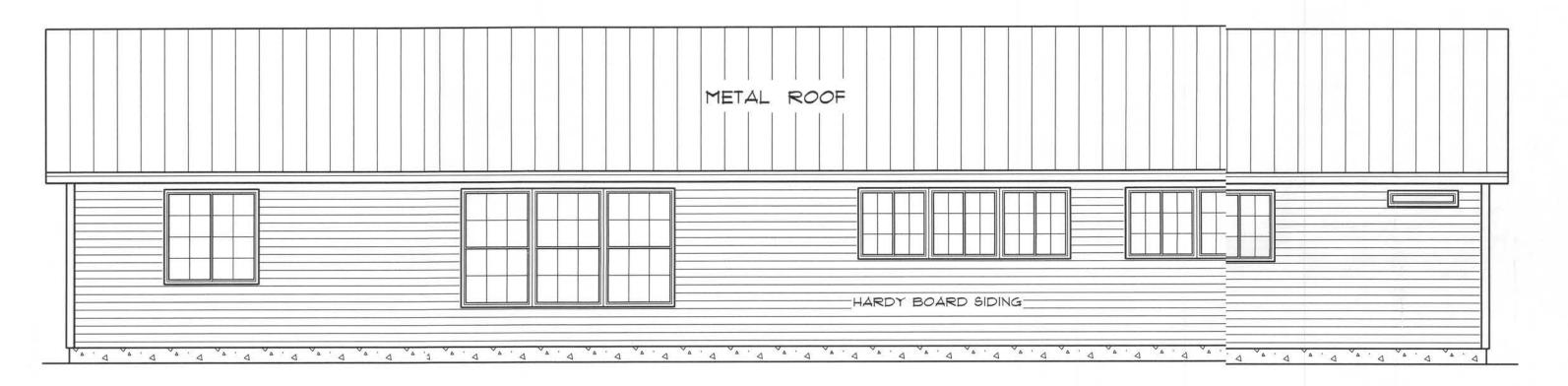
- 1. IT IS THE BUILDERS RESPONSIBILITY TO VERIFY ALL GEOMETRY AND DIMENSIONS BEFORE ORDERING ANY MATERIALS OR BEGINNING CONSTRUCTION.
- 2. THESE PLANS ARE FOR LAYOUT AND DESIGN PURPOSES ONLY ALL STRUCTURAL NOTES AND DETAILS TO BE PROVIDED BY AN ENGINEER.



# \* FRONT ELEVATION \*

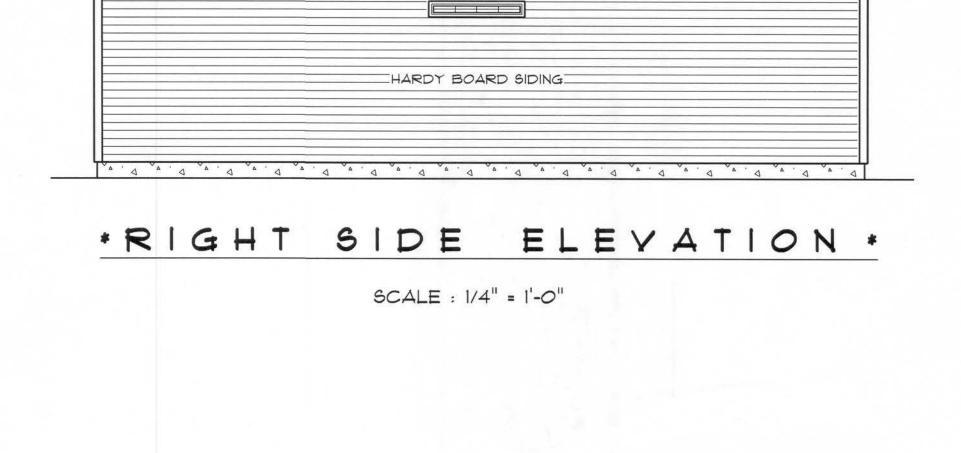
SCALE : 1/4" = 1'-0"

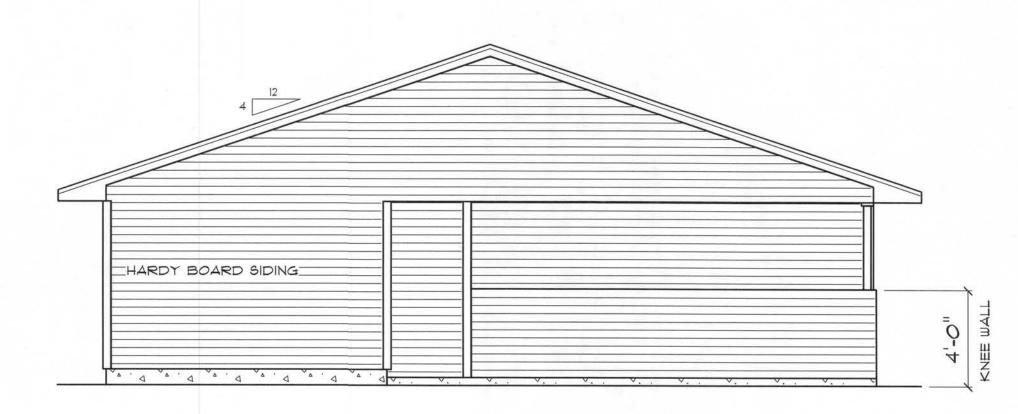
NOTE: 24" OVERHANG ON EAVES 12" OVERHANG: ON GABLE ENDS



# \* REAR ELEVATION \*

SCALE : 1/4" = 1'-0"





# \*LEFT SIDE ELEVATION \*

SCALE : 1/4" = 1'-0"

RESIDENCE for JOHN & ALICE

> LAKE CITY FLORIDA

Teena M. Ruffo 251 NW Hall of Fame Dr. Lake City, Florida 32055 Phone: (386) 755 - 9993 Cell: (386) 867 - 1191 Imail: nfarchdesigns@lani.net

PRINTED DATE:
October 13, 2006

DRAWN BY: CHECKED BY

BUILDING CONTRACTOR

WILBUR BROWN

AUGUST 2006

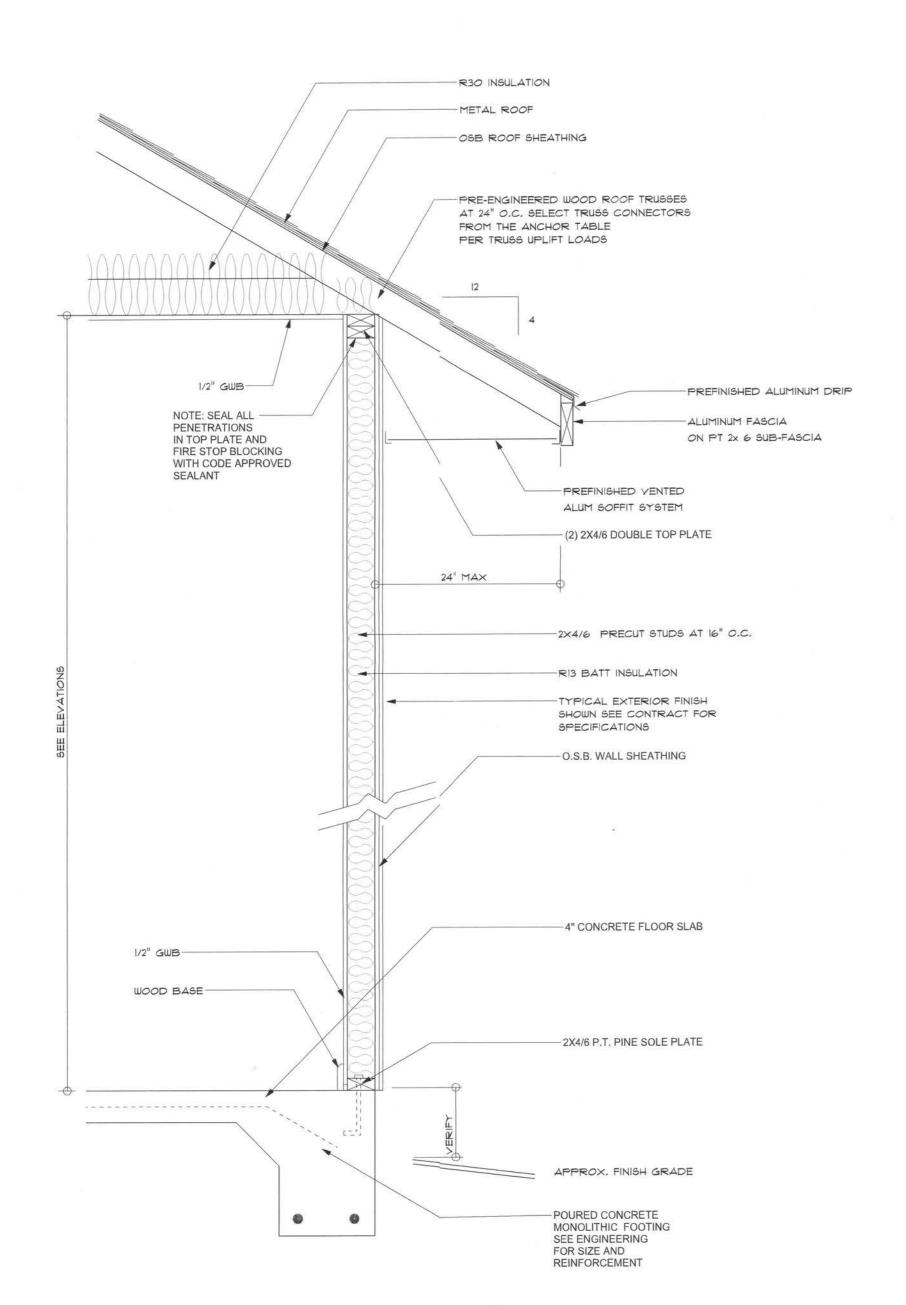
JOB NUMBER:

A-2

DRAWING NUMBER

REVISIONS

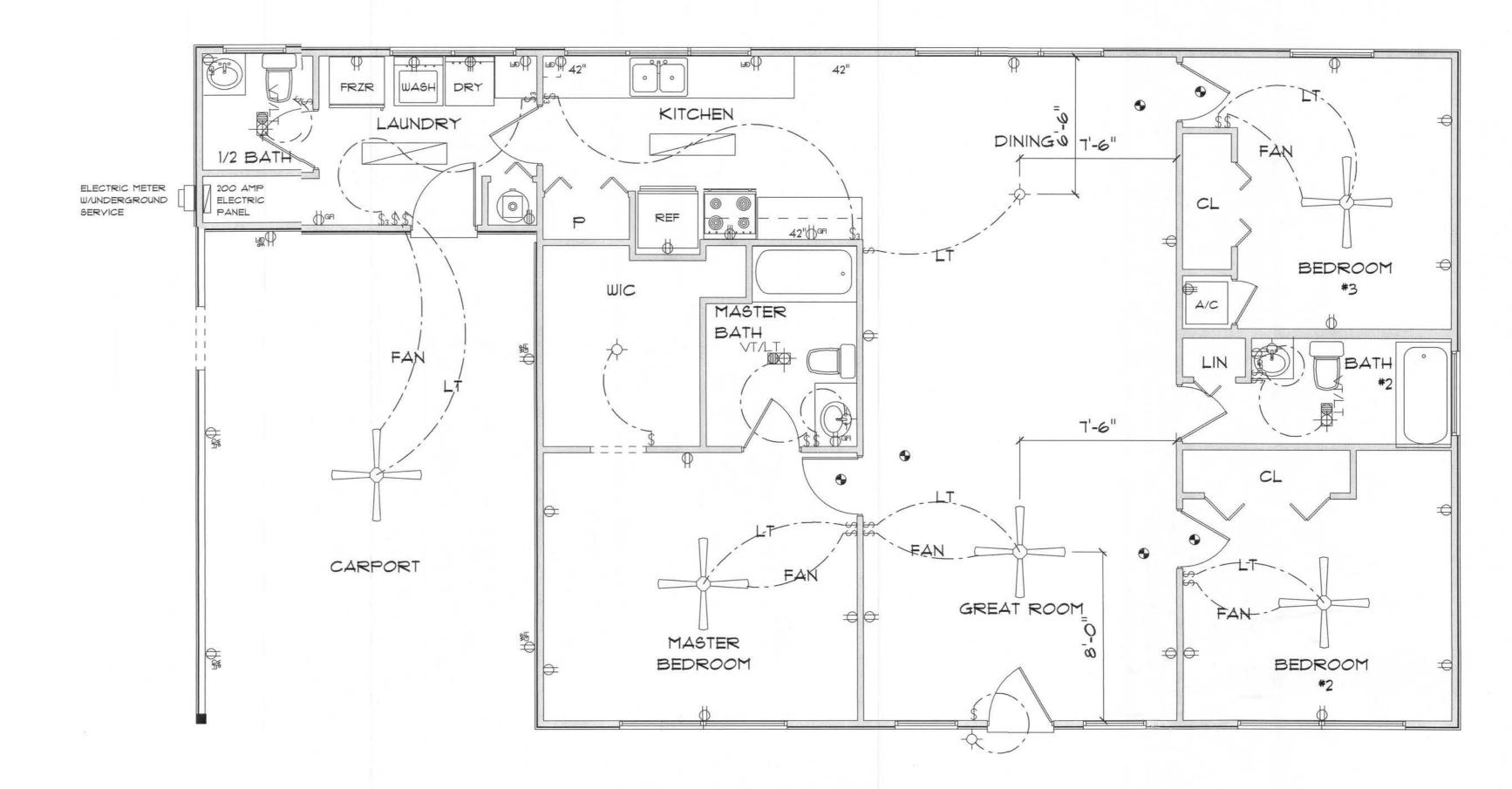
SOFTFIXN



TYPICAL DESIGN WALL SECTION

NON - STRUCTURAL DATA

SCALE = N.T.S.



# \* ELECTRIC PLAN \*

### SCALE : 1/4" = 1'-0"

ELECTFRICAL	COUNT	SYMBOL
ceiling fan	5	
200 amp serryice panel	1	200 AMP ELECTRIC PANEL
electric metter	1	ELECTRIC METER W/UNDERGROUND SERVICE
fluorescent It fixture	2	
gfi waterpropof outlet	5	<b>₽</b>
light	5	<b>+</b>
outlet	21	Ф
outlet 220v	4	₩
outlet gfi	8	⊕an
smoke detesctor	6	•
switch	18	\$
switch 3 way,	4	\$3
vent light combo	3	VT/LT

#### ELECTRICAL PLAN NOTES

- E-I WIRE ALL APPLIANCES, HYAC UNITS AND OTHER EQUIPMENT PER MANUFACTORS SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E-4 ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.
- E-3 ALL SMOKE DETECTORS SHALL BE 120V W/BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER, INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVYISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNERS DIRECTION AND IN ACCORDANCE WITH APPLICABLE SECTIONS OF NATIONAL ELCT. CODE LATEST EDITION.
- E-6 ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE UNDERGROUND OR OVERHEAD) IS TO BE DETERMINED BY THE POWER COMPANY.
- E-8 ALL BEDROOM RECEPTICALS ARE TO BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

RESIDENCE

JOHN 4 ALICE WE3B

> LAKE CITY FLORIDA

Teena M. Ruffo
251 NW Hall of Fame Dr.
Lake City, Florida 32055
Phone: (386: 755 - 9993
Cell: (386: 867 - 1191
Email: nfarchdesigns@lani.net

PRINTID DATE:
October 3, 2006

DRAWN BY: CHECKED BY:

Jeena M. Ruffa

BUILDING CONTRACTOR

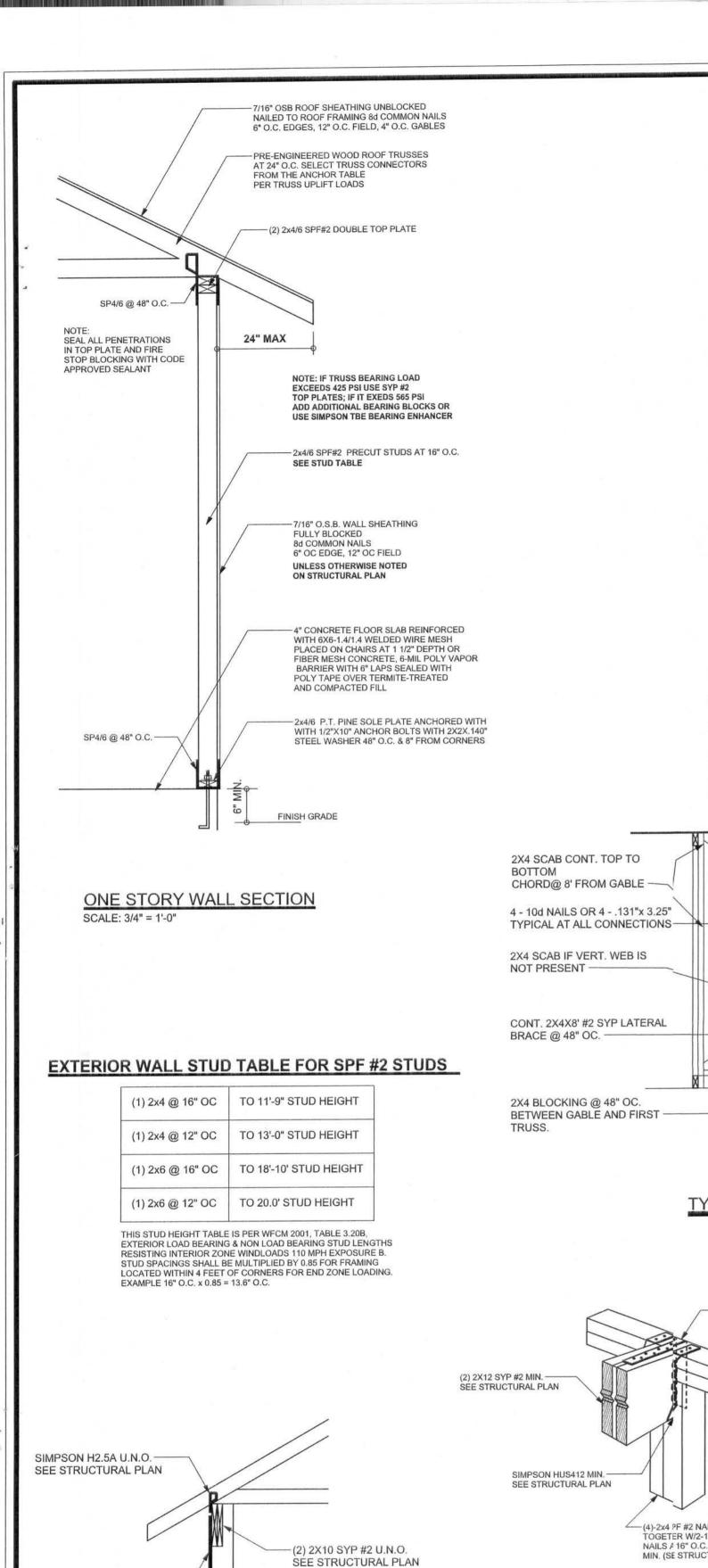
WILBUR BROWN

FINALS DATE: AUGUST 2006

JOB NUMBIR:

**4-3**OF 3 SHEETS

DRAWING NUMBER



(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

-6X6 SYP #2 POST

-SIMPSON ABU POST BASE

SEE STRUCTURAL PLAN

BEAM MAY BE ATTACHED IN EITHER METHOD SHOWN ABOVE

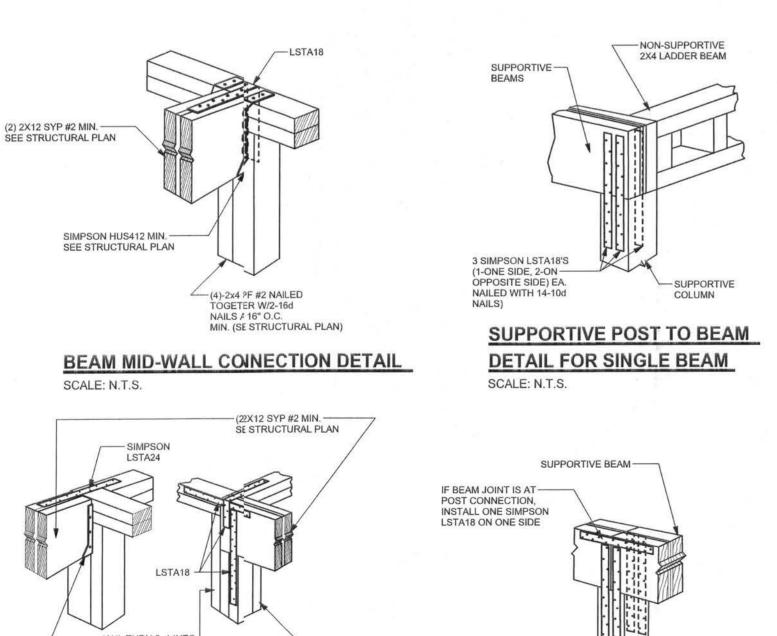
BEAM CORNER CONNECTON. DETAIL

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

ANCHOR BOLT

AND (8) -16d TO POST



7/16" STRUCTURAL ROOF SHEATHING

BLOCKING REQUIRED BETWEEN OUTRIGGERS —

2X4 X-BRACE @ 6'-0" OC. —

TYPICAL GABLE END (X-BRACING

ALL MEMBERS SHALL BE SYP

2X4 OUTRIGGER @ 48" OC. —

(3) .131 X 3 1/4 " GUN NAILS ---

4' FROM GABLE END -

2X4 BLOCKING @ SHEATHING JOINT

## **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

Y LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN RDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO WNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); JNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION NSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BUILDER'S RESPONSIBILITY

	R AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE Y NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND ATT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	RIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE P	TINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU AN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ENGINEER IMMEDIATELY.
DESIGN, PLACE	USS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS MENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, SS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL TIONS.

#### ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

Grout

3.3.E.7 | Movement joints

CMU standard

Clay brick standard

Reinforcing bars, #3 - #11

Coating for corrosion protection

ACI530.1-02 Section

IN WRITING.

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

Specific Requirements

5.5"x2.75"x11.5"

Coating for corrosion protection Anchors, sheet metal ties completely

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

or 304SS

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

Joint reinforcement in walls exposed to noisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

equire engineering approval.

detailed on project drawings.

ASTM C 476, admixtures require approval

medium surface finish, 8"x8"x16" running

**ANCHOR TABLE** 

< 420

< 455

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2300

< 2320

MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP UPLIFT LBS. SPF

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 600

< 760

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2300

< 2320

**DESIGN DATA** 

TRUSS CONNECTOR\*

H2.5

H2.5A

H14-1

H14-2

H10-1

H10-2

H16-1

H16-2

MTS24C

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWNS

HGT-2

HGT-3

HGT-4

TUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

OSP SINGLE SILL PLATE

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS

LTT19

HD2A

PAHD42

HPAHD22

ABU88

HTS24

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2

13-8d

15-8d

6-10d

10-10d, 1 1/2

10-10d, 1 1/2

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

6-10d

14-10d

16-10d

18-8d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

3-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

16 -10d

TO FOUNDATION

1-5/8" THREADED ROD

2-5/8" THREADED ROD

-5/8" THREADED ROD

2-5/8" THREADED ROD

12" EMBEDMENT

12" EMBEDMENT

12" EMBEDMENT

TO STUDS

4 -10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

12" EMBEDMENT

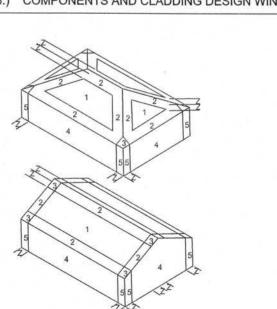
WIN	ID LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
ME, ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT</p> INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY

Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6	01.00=0.00	-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
1355	& Wind st Cas 5, 10	е	21.8	-29.1
8x7 Gar	age D	oor	19.5	-22.9
16x7 Garage Door		18.5	-21.0	

DESIGN	LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	1100 0 200
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF	20 PSF (FLAT OR <4:12)	
	16 PSF (4:12 TO <12:12)	141
	12 PSF (12:12 AND GREATER)	

JOB NUMBER: STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

> **S-1** OF 3 SHEETS

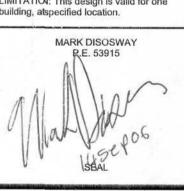
**REVISIONS** 

VINDLOAI ENGINEER: Mark Disosway, PE No.5395, POB 868, Lake City, FL 32056, 386754-5419 Stated diminsions supercede scaled imensions Refer all questions to Mark Disosvay, P.E. for resolution. Do not proceed without clarification.

COPYRIGITS AND PROPERTY RIGHTS: Mark Disosvay, P.E. hereby expressly reserve ts commor law copyrights and property right hese instruments of service. This document is not to be reroduced, altered or copied in any form or mainer without first the express writte ermissionand consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined his plan, and that the applicable portions of he plan, relating to wind engineer comply witl section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, atspecified location.



John & Alice Webb Residence

ADDRESS: SW Infiniti Place Cdumbia County, Florida rear half of 24-5S-16-03707-009

Mark Disosway P.E. P.O. Box 868 Lak∈ City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: September 14, 2006 CHECKED BY: David Dsosway

FINALSDATE: 14 / Sep / 06

> 609052 TRAWING NUMBER

SUPPORTIVE CENTER POST TO BEAM DETAIL TYPICAL PORCH POST DETAIL SCALE: N.T.S. SCALE: 1/2" = 1'-0"

(2-ONE SIDE, 2-ON

OTHER SIDE)

-NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 4" O.C. FOR UPLIFT (6) .131 x 3 1/4" GUN NAILS ----(6) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD

CONTINUOUS FRAME TO

**CEILING DIAPHRAGM DETAIL** 

**GRADE & SPECIES TABLE** 

SYP#2

SYP#2

SYP#2

24F-V3 SP

MICROLAM

PARALAM

PRE ENGINEERED ROOF TRUSS

LSL TIMBERSTRAND

TOGETHER W/2-16d NAILS AT 16" O.C. 4' MIN. LAP w/ (12) - 16d OR 4" LAP w/

CS20 w/ (4) - 16d &(14) - 10d

SPECIFIED ON FLOOR PLAN

ALL STUDS TO BE 2x4 -

SPF NAILED TO TOP AND BOTTOM PLATES

WITH 2-16d NAILS

SCALE: N.T.S.

INTERIOR CEILING AS -

O TTOP PLATE AT

BOTITOM CHORD OF TRUSS

2x10

2x12

Fb (psi) E (10<sup>6</sup> psi)

1.6

1.6

1.9

1200

1050

975

2400

1700

1600

2900

HURRICANE CLIP H-2.5 OR EQUAL

TOP CHORD OF GABLE END TRUSS

CONT. 2X4 SCAB FROM TOP TO

BOTTOM CHORD @ X-BRACING

(PROVIDE ADDITIONAL 2X4'S @

**VERTICAL IF HIGHER THAN 48"** 

TO FORM AN "L" SHAPE.)

TOE NAIL TRUSS TO DOUBLE

PLATE w/ 16d COM @8" OC.

BOTTOM CHORD OF GABLE

SIMPSON LSTA 24 @ 48" OC.

2X4 STUDS @16" OC.

2X4 BARGE RAFTER CONT.

48" OC.

- FASCIA

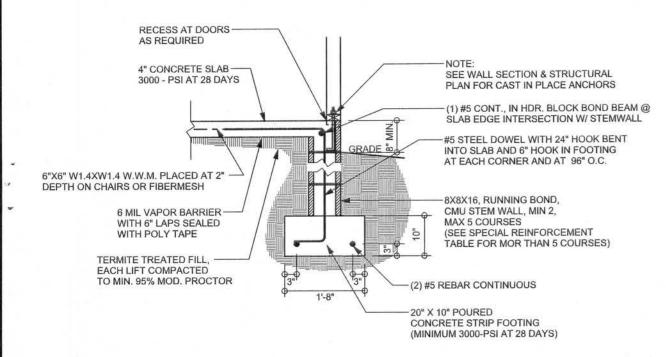
SHINGLE STRIP

DROP 3 1/2"

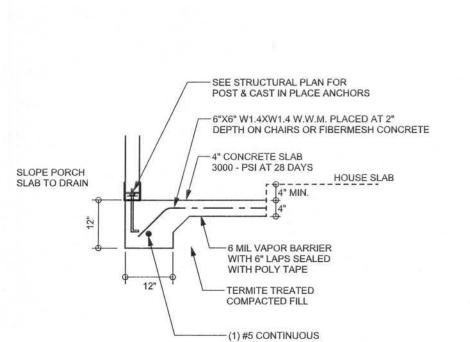
-LSTA18 (U.N.O.-CRIPPLES IF REQUIRED (4) .131 x 3 1/4" GUN NAILS - TOE NAILED THRU SILL -INTO JACK STUD U.N.O. TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN)

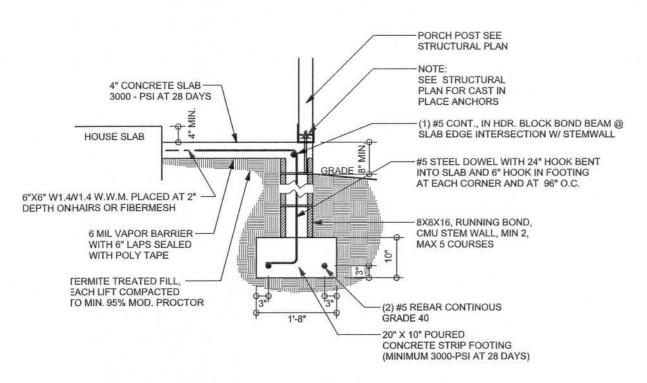
-SP4 OR (2) H2.5A OR (2) SSP---ALL OPENINGS (U.N.O.) (1) 22X6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 22X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

TYPICAL, HEADER STRAPING DETAIL



F9 STEM WALL FOOTING SCALE: 1/2" = 1'-0"



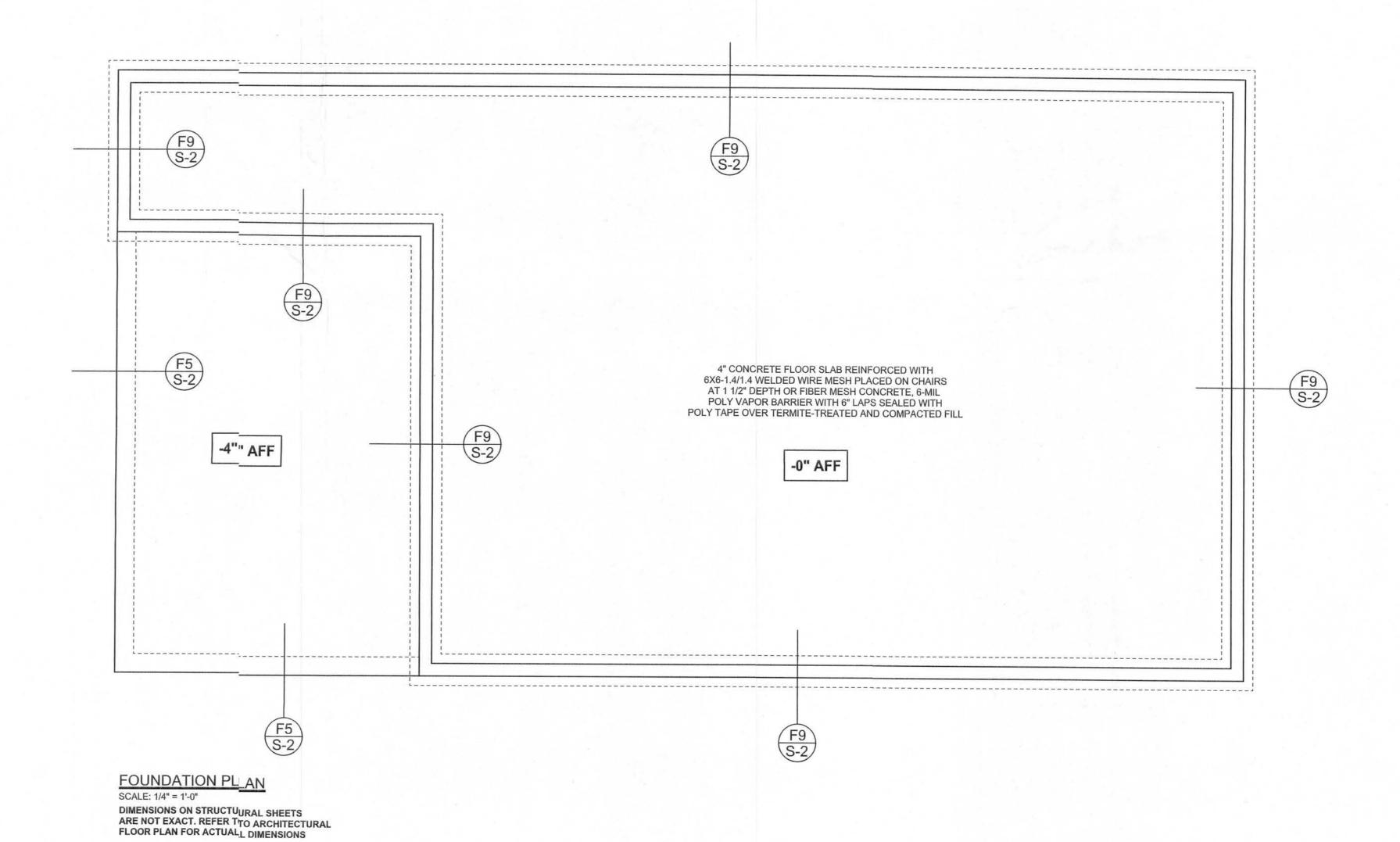


ALT. STEM WALL PORCH FOOTING

#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	FOR 8	AL REINFOR 5" CMU STEN INCHES O.C	<b>WALL</b>	FOR 1	AL REINFOR 2" CMU STEI INCHES O.C	MWALL
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



VINDLOAD ENGINEER: Mark Disosway, PE No.53915,POB 868, Lake City, FL 32056, 386-75-5419

REVISIONS

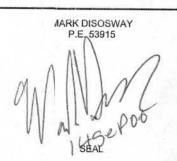
SOFTPIXN

Stated dimensons supercede scaled dimensions. Refer all questions to Mark Disoswar, P.E. for resolution. Do not proceel without clarification. COPYRIGHTS AND PROPERTY RIGHTS:

Mark Disoswa, P.E. hereby expressly reserve its common lay copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manne without first the express written ermission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have

examined this lan, and that the applicable portions of theplan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: 'his design is valid for one building, at spicified location.



John & Alice Webb Residence

ADDRESS: SW Infiniti Place Colunbia County, Florida rear half of 245S-16-03707-009

Marł Disosway P.E. F.O. Box 868 Lake City, Florida 32056 Phone (386) 754 - 5419 Fax: 386) 269 - 4871

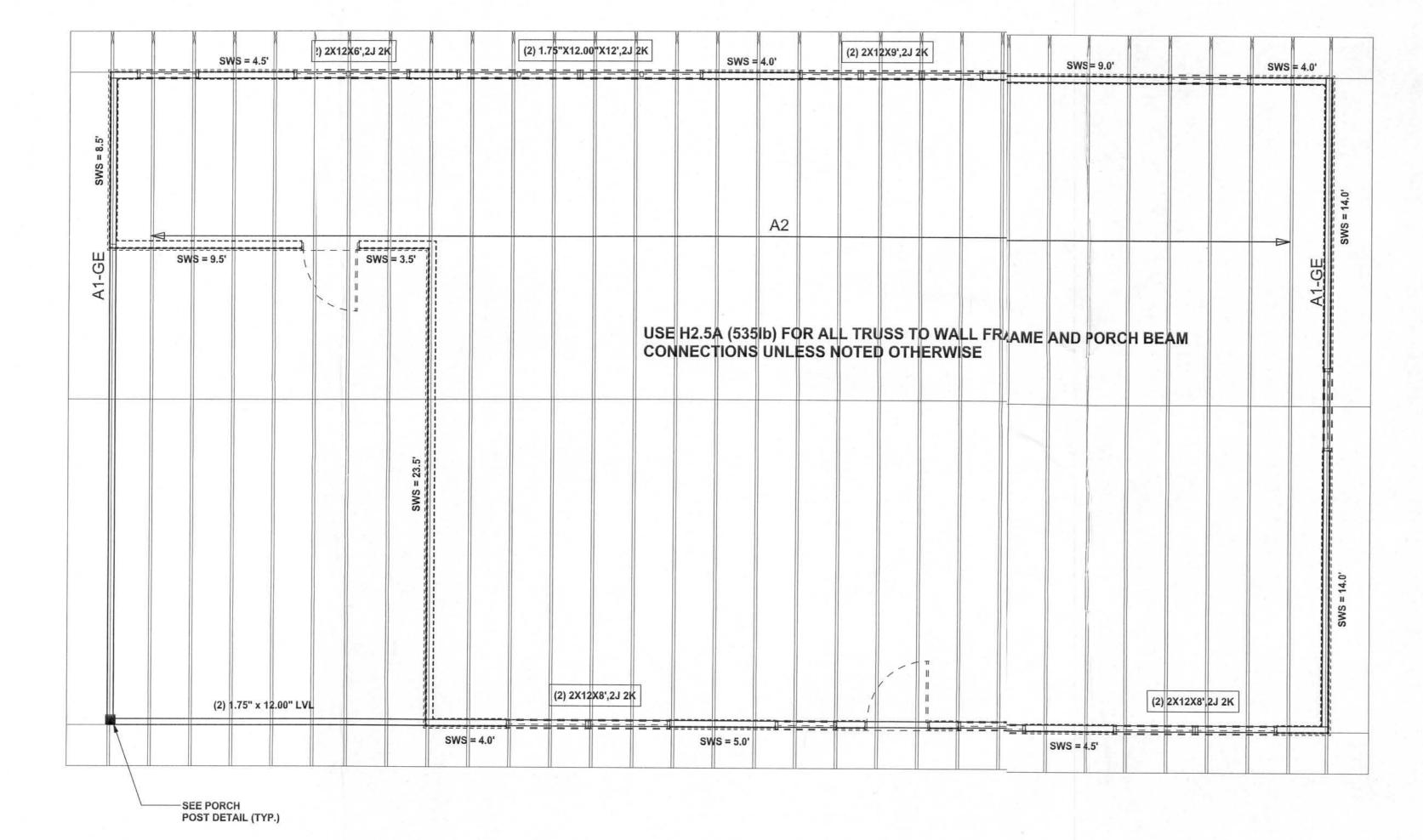
PRINTED DATE: September 14, 2006 DRAWN B': CHECKED BY: David Disosway

FINALS DATE: 14 / Sep /06

JOB NUMBER: 609052 DRAWING NUMBER

**S-2** 

**0F 3 SHEETS** 



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

### STRUCTURAL PLAN NOTES

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

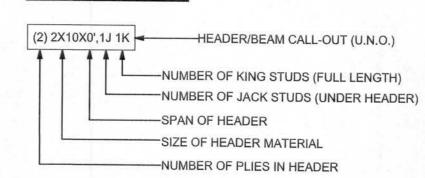
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3
ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED
TRUSS PACKAGE

#### WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

### **HEADER LEGEND**



# TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.5'	60.0'
LONGITUDINAL	29.6'	48.0'

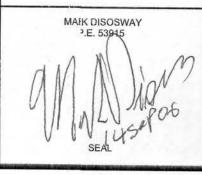
WINDLOAD ENGNEER: Mark Disosway, PE No.53915, PO3 868, Lake City, FL 32056, 386-754-519 DIMENSIONS: Stated dimensions supercede scaled dimensions. Referall questions to Mark Disosway, PE. for resolution. Do not proceed whout clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, PE. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is

REVISIONS

SO-TPIXAN ARCHITETURAL DESIGN SOFTWARE

not to be reprodued, altered or copied in any form or manner whout first the express written permission and cosent of Mark Disosway. CERTIFICATION: hereby certify that I have examined this plar, and that the applicable portions of the plar, relating to wind engineering comply with section R301.2.1, florida building code residential 204, to the best of my knowledge.

LIMITATION: Thisdesign is valid for one building, at specified location.



John & Alice Webl Residence

ADDRESS: SW[nfiniti Place Columbia County, Florida rar half of 24-5S-16-03707-009

Mark Dsosway P.E. P.C. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: September 14, 2006 DRAWN BY: CHECKED BY: David Disosway

14 / Sep / 06

CONNECTIONS, WALL, & HEADER DESIGN IS BASED

JOB #6-312

ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO.

JOB NUMBER: 609052

**S-3** 

DRAWING NUMBER

OF I SHEETS