MiTek®

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 1124-060 - Streeter

MiTek, Inc.
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Site Information:
Customer Info: SCCI Project Name: . Model: .
Lot/Block: . Subdivision: .
Address: ., .
City: Columbia County State: Fl

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

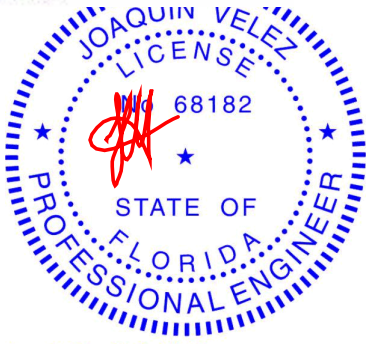
General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2023/TPI2014 Design Program: MiTek 20/20 8.7
Wind Code: ASCE 7-22 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 7 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T35700072	A01	12/3/24
2	T35700073	A02	12/3/24
3	T35700074	A03	12/3/24
4	T35700075	A04	12/3/24
5	T35700076	B01	12/3/24
6	T35700077	B02	12/3/24
7	T35700078	B03	12/3/24

The truss drawing(s) referenced above have been prepared by under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Velez, Joaquin
My license renewal date for the state of Florida is February 28, 2025.



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

December 3,2024

Job	Truss	Truss Type	Qty	Ply	Streeter
1124-060	A01	Roof Special Supported Gable	1	1	T35700072
					Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Mon Dec 02 14:37:52 Page: 1
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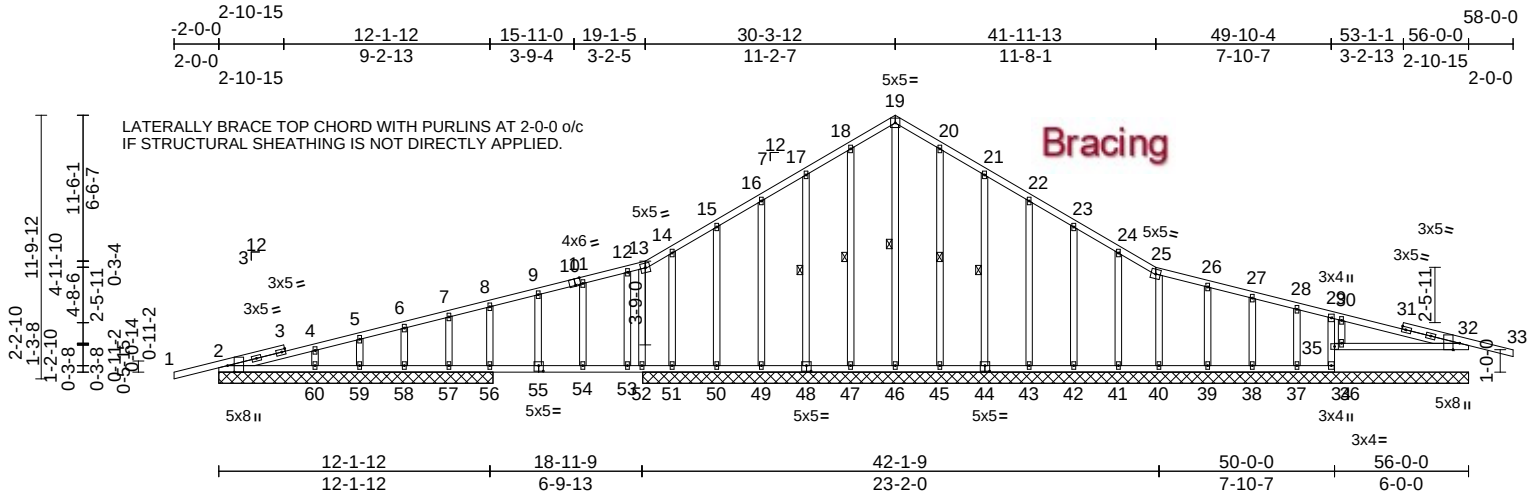


Scale = 1:103.2

Plate Offsets (X, Y): [2:0-3-8,Edge], [10:0-2-14,Edge], [32:0-3-8,Edge], [44:0-2-8,0-3-0], [48:0-2-8,0-3-0], [55:0-2-8,0-3-0]

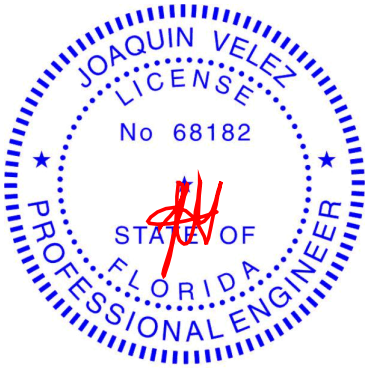
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.28	Vert(LL)	-0.04	54-55	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.37	Vert(CT)	-0.08	54-55	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.01	32	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 375 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 19-46, 18-47, 17-48, 20-45, 21-44

REACTIONS (size) 2=12-3-8, 32=37-0-0, 34=37-0-0, 35=37-0-0, 36=37-0-0, 37=37-0-0, 38=37-0-0, 39=37-0-0, 40=37-0-0, 41=37-0-0, 42=37-0-0, 43=37-0-0, 44=37-0-0, 45=37-0-0, 46=37-0-0, 47=37-0-0, 48=37-0-0, 49=37-0-0, 50=37-0-0, 51=37-0-0, 52=37-0-0, 56=12-3-8, 57=12-3-8, 58=12-3-8, 59=12-3-8, 60=12-3-8, 61=12-3-8
Max Horiz 2=169 (LC 11), 61=169 (LC 11)
Max Uplift 2=-48 (LC 12), 32=-50 (LC 12), 35=-405 (LC 1), 37=-23 (LC 12), 39=-7 (LC 12), 40=-14 (LC 12), 41=-17 (LC 12), 42=-16 (LC 12), 43=-15 (LC 12), 44=-21 (LC 12), 45=-3 (LC 12), 47=-3 (LC 12), 48=-21 (LC 12), 49=-15 (LC 12), 50=-17 (LC 12), 51=-134 (LC 23), 52=-24 (LC 12), 56=-9 (LC 12), 57=-57 (LC 23), 58=-1 (LC 12), 59=-12 (LC 12), 61=-48 (LC 12)
Max Grav 2=303 (LC 1), 32=330 (LC 1), 34=739 (LC 1), 35=-119 (LC 9), 36=26 (LC 12), 37=138 (LC 24), 38=160 (LC 1), 39=180 (LC 24), 40=159 (LC 1), 41=149 (LC 18), 42=165 (LC 18), 43=161 (LC 18), 44=163 (LC 18), 45=166 (LC 24), 46=189 (LC 12), 47=167 (LC 17), 48=163 (LC 17), 49=153 (LC 17), 50=203 (LC 23), 51=-1 (LC 8), 52=563 (LC 1), 56=516 (LC 1), 57=0 (LC 12), 58=220 (LC 23), 59=104 (LC 1), 60=281 (LC 1), 61=303 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/29, 2-4=-167/132, 4-5=-134/121, 5-6=-126/123, 6-7=-127/117, 7-8=-114/129, 8-9=-130/105, 9-11=-117/112, 11-12=-106/123, 12-13=-107/112, 25-26=-39/48, 26-27=-37/33, 27-28=-39/35, 28-29=-41/35, 29-30=-77/10, 30-32=-156/71, 32-33=0/29, 13-14=-140/118, 14-15=-118/164, 15-16=-110/234, 16-17=-108/300, 17-18=-132/372, 18-19=-150/427, 19-20=-150/427, 20-21=-132/372, 21-22=-108/300, 22-23=-86/233, 23-24=-64/165, 24-25=-46/100
BOT CHORD 2-60=-74/153, 59-60=-38/133, 58-59=-38/133, 57-58=-38/133, 56-57=-38/133, 54-56=-38/134, 53-54=-38/134, 52-53=-38/134, 51-52=-35/131, 50-51=-35/131, 49-50=-35/131, 47-49=-35/131, 46-47=-35/131, 45-46=-35/131, 43-45=-36/131, 42-43=-36/131, 41-42=-36/131, 40-41=-36/131, 39-40=-38/135, 38-39=-38/135, 37-38=-38/135, 36-37=-38/135, 35-36=0/0, 29-35=-74/197, 34-35=-47/150, 32-34=-56/171



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

December 3,2024

Continued on page 2

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

WEBS 19-46=-322/80, 18-47=-128/84, 17-48=-121/121, 16-49=-121/107, 15-50=-132/116, 14-51=-25/67, 12-53=-142/86, 11-54=-29/49, 9-55=-46/58, 7-57=-13/41, 6-58=-149/93, 5-59=-91/77, 4-60=-195/105, 20-45=-126/84, 21-44=-123/121, 22-43=-122/108, 23-42=-124/111, 24-41=-112/105, 26-39=-138/96, 27-38=-120/81, 28-37=-98/71, 30-34=-446/223, 8-56=-294/146, 13-52=-154/91, 25-40=-119/106

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=56ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 .
10) Bearing at joint(s) 35 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 2, 405 lb uplift at joint 35, 50 lb uplift at joint 32, 3 lb uplift at joint 47, 21 lb uplift at joint 48, 15 lb uplift at joint 49, 17 lb uplift at joint 50, 134 lb uplift at joint 51, 57 lb uplift at joint 57, 1 lb uplift at joint 58, 12 lb uplift at joint 59, 3 lb uplift at joint 45, 21 lb uplift at joint 44, 15 lb uplift at joint 43, 16 lb uplift at joint 42, 17 lb uplift at joint 41, 7 lb uplift at joint 39, 23 lb uplift at joint 37, 9 lb uplift at joint 56, 24 lb uplift at joint 52, 14 lb uplift at joint 40 and 48 lb uplift at joint 2.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Streeter
1124-060	A02	Roof Special	16	1	T35700073
					Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

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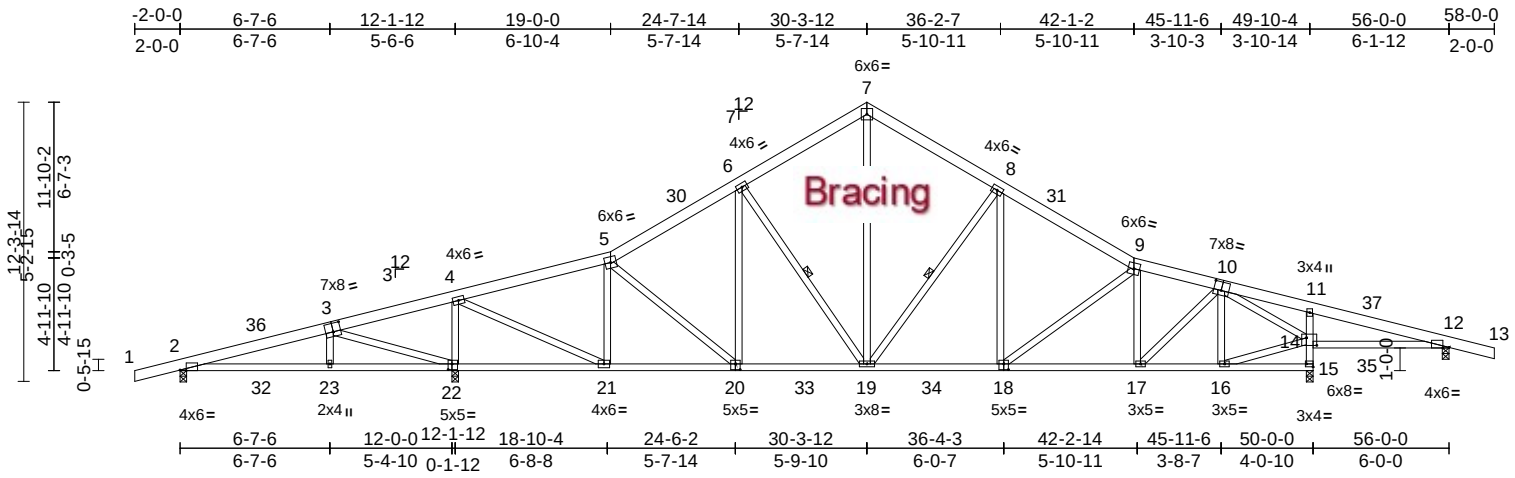


Scale = 1:101.7

Plate Offsets (X, Y): [2:0-3-4,Edge], [3:0-4-0,0-4-8], [10:0-4-0,0-4-8], [12:0-3-4,Edge], [14:0-5-8,0-4-0], [15:Edge,0-1-8], [18:0-2-8,0-3-0], [20:0-2-8,0-3-0], [22:0-2-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.35	Vert(LL)	0.06	23-26	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.59	Vert(CT)	-0.22	18-19	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.74	Horz(CT)	0.06	15	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 390 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

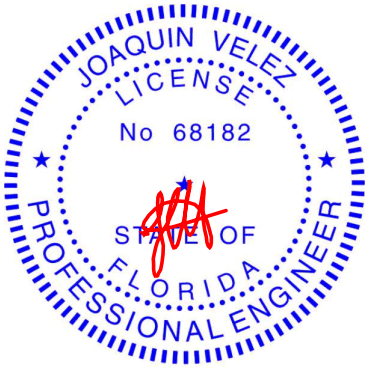
BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 6-19, 8-19

REACTIONS (size) 2=0-3-8, 12=0-3-8, 15=0-3-8, 22=0-3-8
Max Horiz 2=190 (LC 11)
Max Uplift 2=-132 (LC 12), 12=-108 (LC 12), 15=-37 (LC 12), 22=-127 (LC 12)
Max Grav 2=392 (LC 23), 12=234 (LC 24), 15=2061 (LC 18), 22=2492 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 5-6=-1796/136, 6-7=-1527/212, 7-8=-1510/212, 8-9=-1998/165, 1-2=0/29, 2-4=-218/1053, 4-5=-1535/22, 9-11=-2022/619, 11-12=-25/645, 12-13=0/29
BOT CHORD 2-23=-290/86, 21-23=-859/257, 19-21=0/1541, 17-19=-28/1945, 16-17=-7/1214, 15-16=-21/21, 14-15=-1988/233, 11-14=-427/143, 12-14=-578/35
WEBS 5-21=-844/157, 9-17=-518/77, 4-22=-2017/202, 4-21=-162/2498, 3-23=-185/218, 3-22=-926/577, 6-20=0/219, 5-20=-45/116, 6-19=-479/109, 7-19=-82/1159, 8-19=-768/156, 8-18=0/497, 9-18=-393/56, 10-17=-26/991, 10-16=-273/50, 14-16=-25/1294, 10-14=-2170/27

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=56ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -2-0-0 to 3-7-3, Zone1 3-7-3 to 30-3-12, Zone2 30-3-12 to 38-2-13, Zone1 38-2-13 to 58-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 2, 127 lb uplift at joint 22, 37 lb uplift at joint 15 and 108 lb uplift at joint 12.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

December 3,2024

Job	Truss	Truss Type	Qty	Ply	Streeter
1124-060	A03	Roof Special	3	1	T35700074
					Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

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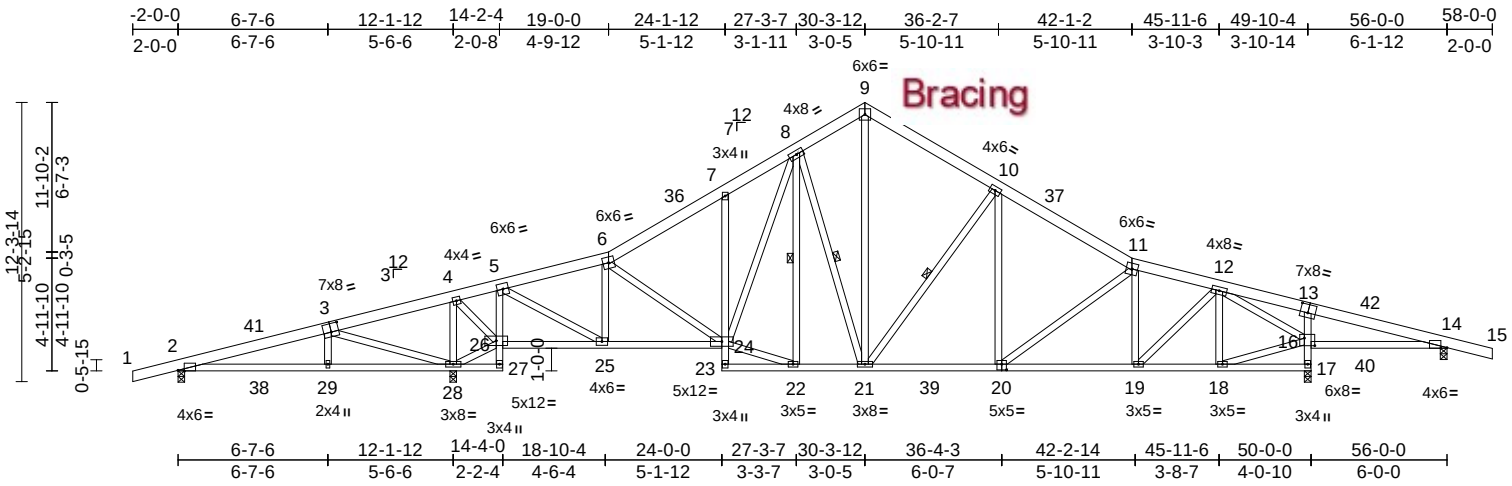

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Plate Offsets (X, Y): [2:0-3-4,Edge], [3:0-4-0,0-4-8], [13:0-4-0,0-4-8], [14:0-3-4,Edge], [16:0-5-8,0-4-0], [20:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.37	Vert(LL)	0.06	29-32	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.59	Vert(CT)	-0.23	20-21	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.71	Horz(CT)	0.06	17	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 427 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 10-21, 8-21, 8-22

REACTIONS (size) 2=0-3-8, 14=0-3-8, 17=0-3-8, 28=0-3-8
Max Horiz 2=190 (LC 11)
Max Uplift 2=-136 (LC 12), 14=-108 (LC 12), 17=-37 (LC 12), 28=-124 (LC 12)
Max Grav 2=314 (LC 23), 14=240 (LC 24), 17=1993 (LC 18), 28=2599 (LC 2)

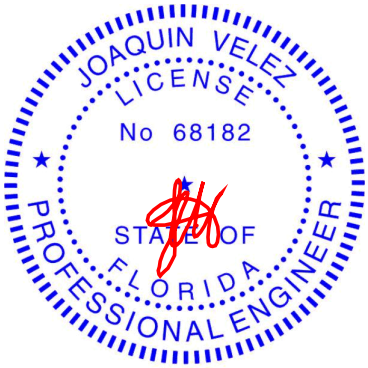
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 6-7=-1800/121, 7-8=-1765/199, 8-9=-1352/219, 9-10=-1399/212, 10-11=-1896/165, 1-2=0/29, 2-4=-210/1534, 4-5=-194/488, 5-6=-1486/5, 11-12=-1937/125, 12-14=-24/603, 14-15=0/29

BOT CHORD 2-29=-535/0, 28-29=-542/0, 27-28=-61/18, 26-27=-25/45, 5-26=-1630/166, 25-26=-426/272, 24-25=0/1483, 23-24=0/68, 7-24=-283/149, 22-23=-10/50, 21-22=0/1272, 19-21=-31/1862, 18-19=-11/1185, 17-18=-21/21, 16-17=-1921/231, 13-16=-426/143, 14-16=-538/34

WEBS 26-28=-1505/258, 5-25=-169/2050, 6-25=-824/147, 6-24=-53/143, 9-21=-120/1052, 10-21=-778/155, 11-19=-482/76, 4-28=-1457/39, 4-26=0/1405, 3-29=-174/243, 3-28=-1055/580, 10-20=0/511, 11-20=-400/57, 8-21=-362/117, 8-24=-53/807, 8-22=-314/11, 22-24=0/1282, 12-19=-25/925, 12-18=-266/51, 16-18=-28/1259, 12-16=-2080/23

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=56ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -2-0-0 to 3-7-3, Zone1 3-7-3 to 30-3-12, Zone2 30-3-12 to 38-2-13, Zone1 38-2-13 to 58-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 2, 37 lb uplift at joint 17, 108 lb uplift at joint 14 and 124 lb uplift at joint 28.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

December 3,2024

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Streeter
1124-060	A04	Roof Special Supported Gable	1	1	T35700075
					Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

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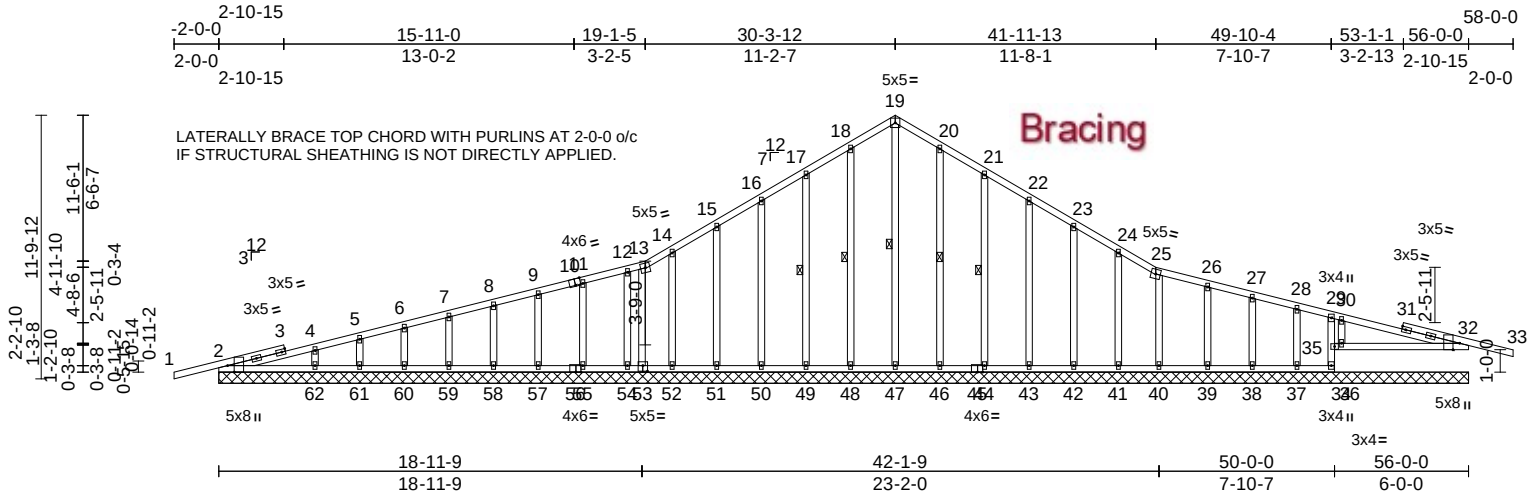

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Plate Offsets (X, Y): [2:0-3-8,Edge], [10:0-2-14,Edge], [32:0-3-8,Edge], [45:0-2-12,Edge], [53:0-2-8,0-3-0], [56:0-2-12,Edge]

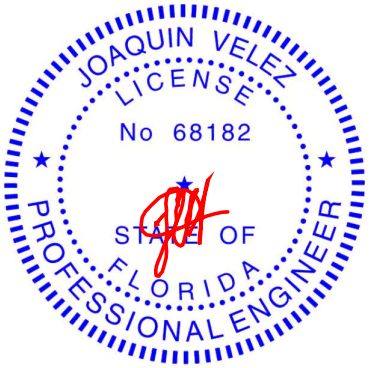
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.25	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.21	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.01	32	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 375 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 19-47, 18-48, 17-49, 20-46, 21-44

REACTIONS (size) 2=56-0-0, 32=56-0-0, 34=56-0-0, 35=56-0-0, 36=56-0-0, 37=56-0-0, 38=56-0-0, 39=56-0-0, 40=56-0-0, 41=56-0-0, 42=56-0-0, 43=56-0-0, 44=56-0-0, 46=56-0-0, 47=56-0-0, 48=56-0-0, 49=56-0-0, 50=56-0-0, 51=56-0-0, 52=56-0-0, 53=56-0-0, 54=56-0-0, 55=56-0-0, 57=56-0-0, 58=56-0-0, 59=56-0-0, 60=56-0-0, 61=56-0-0, 62=56-0-0, 63=56-0-0
Max Horiz 2=169 (LC 11), 63=169 (LC 11)
Max Uplift 2=-48 (LC 12), 32=-50 (LC 12), 35=-406 (LC 1), 37=-23 (LC 12), 39=-7 (LC 12), 40=-14 (LC 12), 41=-17 (LC 12), 42=-16 (LC 12), 43=-15 (LC 12), 44=-21 (LC 12), 46=-3 (LC 12), 48=-3 (LC 12), 49=-21 (LC 12), 50=-15 (LC 12), 51=-17 (LC 12), 52=-14 (LC 12), 53=-14 (LC 12), 54=-3 (LC 12), 55=-3 (LC 12), 57=-3 (LC 12), 58=-3 (LC 12), 59=-3 (LC 12), 61=-13 (LC 12), 63=-48 (LC 12)
Max Grav 2=304 (LC 1), 32=330 (LC 1), 34=740 (LC 1), 35=-119 (LC 9), 36=26 (LC 12), 37=137 (LC 24), 38=160 (LC 1), 39=180 (LC 24), 40=160 (LC 1), 41=149 (LC 18), 42=165 (LC 18), 43=161 (LC 18), 44=163 (LC 18), 46=166 (LC 24), 47=189 (LC 12), 48=168 (LC 17), 49=161 (LC 17), 50=161 (LC 17), 51=167 (LC 17), 52=137 (LC 1), 53=63 (LC 18), 54=122 (LC 1), 55=166 (LC 23), 57=158 (LC 1), 58=161 (LC 23), 59=157 (LC 1), 60=171 (LC 23), 61=117 (LC 1), 62=278 (LC 1), 63=304 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/29, 2-4=-167/130, 4-5=-135/119, 5-6=-128/121, 6-7=-127/118, 7-8=-124/116, 8-9=-122/115, 9-11=-119/113, 11-12=-116/110, 12-13=-113/107, 25-26=-39/48, 26-27=-38/33, 27-28=-40/35, 28-29=-41/34, 29-30=-78/10, 30-32=-156/71, 32-33=0/29, 13-14=-122/120, 14-15=-121/165, 15-16=-110/234, 16-17=-108/300, 17-18=-132/372, 18-19=-150/427, 19-20=-150/427, 20-21=-132/372, 21-22=-108/300, 22-23=-86/234, 23-24=-64/165, 24-25=-47/101
BOT CHORD 2-62=-73/152, 61-62=-36/132, 60-61=-36/132, 59-60=-36/132, 58-59=-36/132, 57-58=-36/132, 55-57=-36/132, 54-55=-36/132, 52-54=-36/132, 51-52=-35/131, 50-51=-35/131, 49-50=-35/131, 48-49=-35/131, 47-48=-35/131, 46-47=-35/131, 44-46=-35/131, 43-44=-35/131, 42-43=-35/131, 41-42=-35/131, 40-41=-35/131, 39-40=-38/135, 38-39=-38/135, 37-38=-38/135, 36-37=-38/135, 35-36=0/0, 29-35=-74/197, 34-35=-47/149, 32-34=-56/171



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

December 3,2024

Continued on page 2

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WEBS 19-47=-323/81, 18-48=-128/84, 17-49=-121/121, 16-50=-121/108, 15-51=-126/113, 14-52=-101/97, 12-54=-90/67, 11-55=-125/85, 9-57=-119/82, 8-58=-121/83, 7-59=-119/82, 6-60=-126/84, 5-61=-96/79, 4-62=-194/105, 20-46=-126/84, 21-44=-123/121, 22-43=-122/108, 23-42=-124/111, 24-41=-112/105, 26-39=-138/96, 27-38=-120/81, 28-37=-98/71, 30-34=-446/223, 13-53=-54/52, 25-40=-119/106

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=56ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) All bearings are assumed to be SP No.2 .
11) Bearing at joint(s) 35 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 2, 406 lb uplift at joint 35, 50 lb uplift at joint 32, 3 lb uplift at joint 48, 21 lb uplift at joint 49, 15 lb uplift at joint 50, 17 lb uplift at joint 51, 14 lb uplift at joint 52, 3 lb uplift at joint 54, 3 lb uplift at joint 55, 3 lb uplift at joint 57, 3 lb uplift at joint 58, 3 lb uplift at joint 59, 13 lb uplift at joint 61, 3 lb uplift at joint 46, 21 lb uplift at joint 44, 15 lb uplift at joint 43, 16 lb uplift at joint 42, 17 lb uplift at joint 41, 7 lb uplift at joint 39, 23 lb uplift at joint 37, 14 lb uplift at joint 53, 14 lb uplift at joint 40 and 48 lb uplift at joint 2.
13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 32, 34.
14) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Streeter
1124-060	B01	Common Supported Gable	1	1	T35700076
					Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

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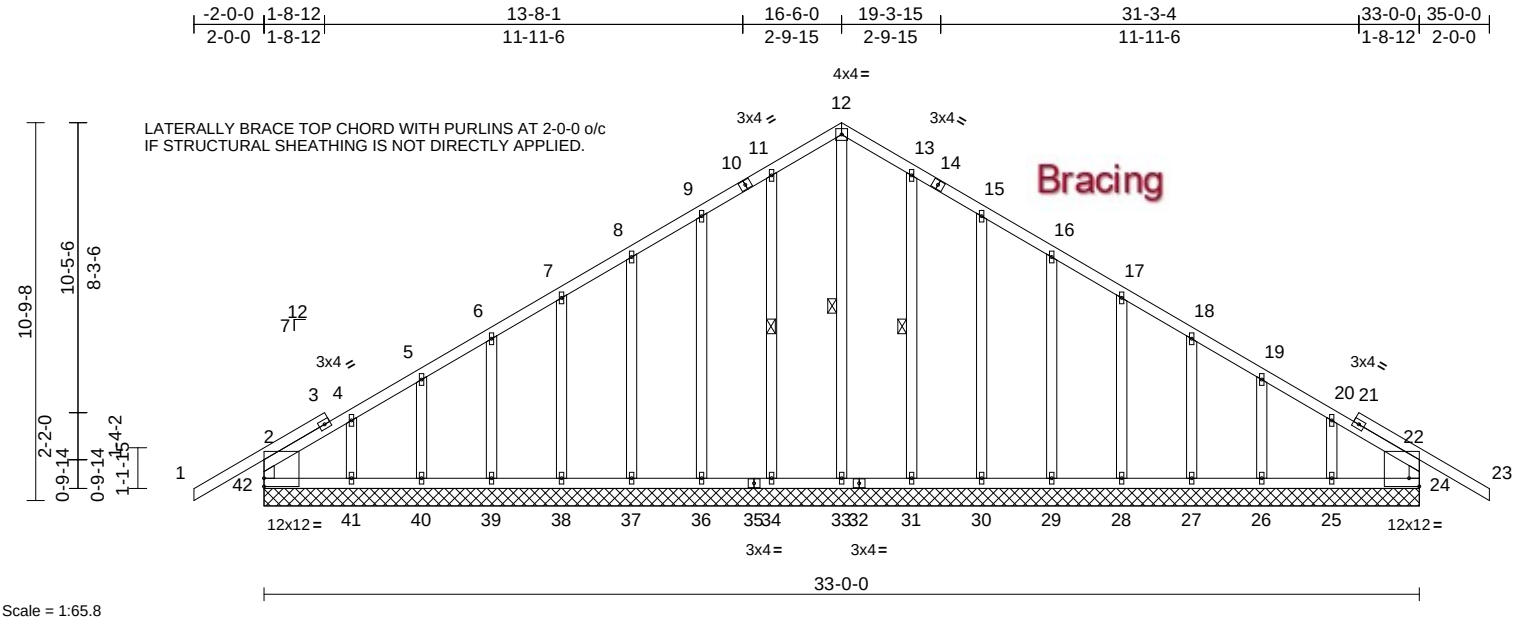


Scale = 1:65.8

Plate Offsets (X, Y): [24:Edge,0-2-13], [42:Edge,0-2-13]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.05	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.01	24	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 244 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.2

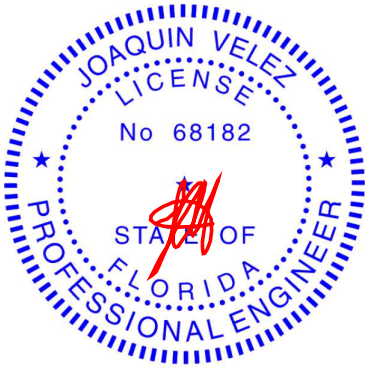
BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 12-33, 11-34, 13-31

REACTIONS (size) 24=33-0-0, 25=33-0-0, 26=33-0-0, 27=33-0-0, 28=33-0-0, 29=33-0-0, 30=33-0-0, 31=33-0-0, 33=33-0-0, 34=33-0-0, 36=33-0-0, 37=33-0-0, 38=33-0-0, 39=33-0-0, 40=33-0-0, 41=33-0-0, 42=33-0-0
Max Horiz 42=206 (LC 11)
Max Uplift 24=-43 (LC 12), 25=-21 (LC 8), 26=-16 (LC 12), 27=-16 (LC 12), 28=-16 (LC 12), 29=-15 (LC 12), 30=-21 (LC 12), 31=-4 (LC 12), 34=-4 (LC 12), 36=-21 (LC 12), 37=-15 (LC 12), 38=-16 (LC 12), 39=-16 (LC 12), 40=-16 (LC 12), 41=-29 (LC 9), 42=-43 (LC 12)
Max Grav 24=271 (LC 24), 25=145 (LC 18), 26=167 (LC 24), 27=159 (LC 18), 28=160 (LC 24), 29=160 (LC 1), 30=159 (LC 24), 31=167 (LC 24), 33=183 (LC 12), 34=167 (LC 23), 36=159 (LC 23), 37=160 (LC 1), 38=160 (LC 23), 39=160 (LC 17), 40=167 (LC 23), 41=158 (LC 17), 42=271 (LC 23)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-42=-246/87, 1-2=0/58, 2-4=-139/129, 4-5=-113/113, 5-6=-106/101, 6-7=-94/90, 7-8=-83/127, 8-9=-99/167, 9-11=-124/209, 11-12=-144/243, 12-13=-144/243, 13-15=-124/209, 15-16=-99/167, 16-17=-76/127, 17-18=-53/87, 18-19=-51/58, 19-20=-59/70, 20-22=-85/71, 22-23=0/58, 22-24=-246/83
BOT CHORD 41-42=-81/111, 40-41=-81/111, 39-40=-81/111, 38-39=-81/111, 37-38=-81/111, 36-37=-81/111, 34-36=-81/111, 33-34=-81/111, 31-33=-81/111, 30-31=-81/111, 29-30=-81/111, 28-29=-81/111, 27-28=-81/111, 26-27=-81/111, 25-26=-81/111, 24-25=-81/111
WEBS 12-33=-190/64, 11-34=-127/42, 9-36=-119/62, 8-37=-120/55, 7-38=-120/56, 6-39=-118/56, 5-40=-128/59, 4-41=-98/65, 13-31=-127/42, 15-30=-119/62, 16-29=-120/55, 17-28=-120/56, 18-27=-118/56, 19-26=-128/59, 20-25=-95/64

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=33ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
8) Gable studs spaced at 2-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
11) All bearings are assumed to be SP No.2 .



Joaquin Velez PE No.68182
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Date:

December 3,2024

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12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 42, 43 lb uplift at joint 24, 4 lb uplift at joint 34, 21 lb uplift at joint 36, 15 lb uplift at joint 37, 16 lb uplift at joint 38, 16 lb uplift at joint 39, 16 lb uplift at joint 40, 29 lb uplift at joint 41, 4 lb uplift at joint 31, 21 lb uplift at joint 30, 15 lb uplift at joint 29, 16 lb uplift at joint 28, 16 lb uplift at joint 27, 16 lb uplift at joint 26 and 21 lb uplift at joint 25.

13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Streeter
1124-060	B02	Common	8	1	T35700077
					Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

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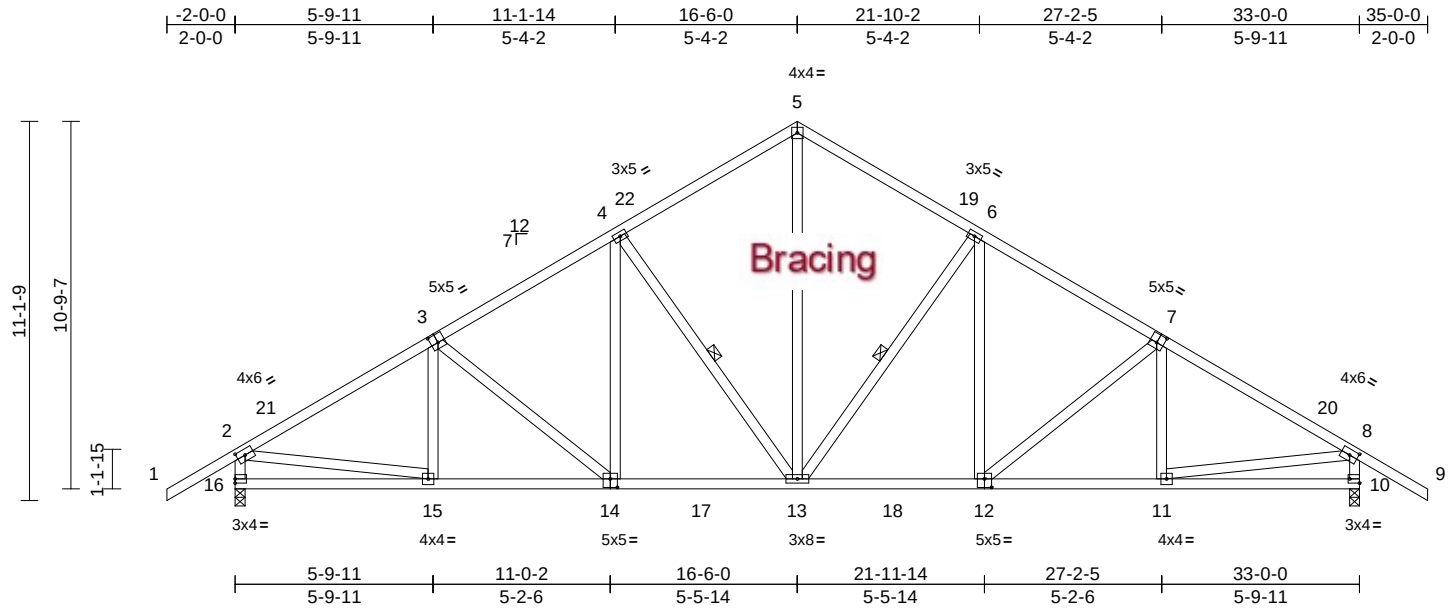


Scale = 1:67.6

Plate Offsets (X, Y): [2:0-2-14,0-2-0], [3:0-2-8,0-3-0], [7:0-2-8,0-3-0], [8:0-2-14,0-2-0], [10:Edge,0-1-8], [12:0-2-8,0-3-0], [14:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.34	Vert(LL)	-0.10	13-14	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.49	Vert(CT)	-0.19	13-14	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.33	Horz(CT)	0.06	10	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 225 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

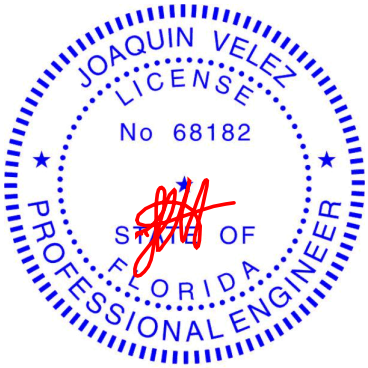
BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 4-13, 6-13

REACTIONS (size) 10=0-3-8, 16=0-3-8
Max Horiz 16=217 (LC 11)
Max Uplift 10=-51 (LC 12), 16=-51 (LC 12)
Max Grav 10=1584 (LC 18), 16=1584 (LC 17)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 5-6=-1408/156, 6-8=-1972/112, 8-9=0/65, 1-2=0/65, 2-4=-1972/112, 4-5=-1408/156, 2-16=-1481/134, 8-10=-1481/134
BOT CHORD 15-16=-118/330, 13-15=0/1767, 11-13=0/1623, 10-11=0/180
WEBS 2-15=-3/1461, 8-11=-4/1461, 3-15=-114/69, 3-14=-236/49, 4-14=0/368, 4-13=-600/88, 5-13=-58/1079, 6-13=-601/88, 6-12=0/368, 7-12=-236/49, 7-11=-114/69

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=33ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-3-10, Zone1 1-3-10 to 16-6-0, Zone2 16-6-0 to 21-2-0, Zone1 21-2-0 to 35-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 16 and 51 lb uplift at joint 10.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Streeter
1124-060	B03	Common	3	1	T35700078
					Job Reference (optional)

Mayo Truss Company, Inc., Mayo, FL - 32066,

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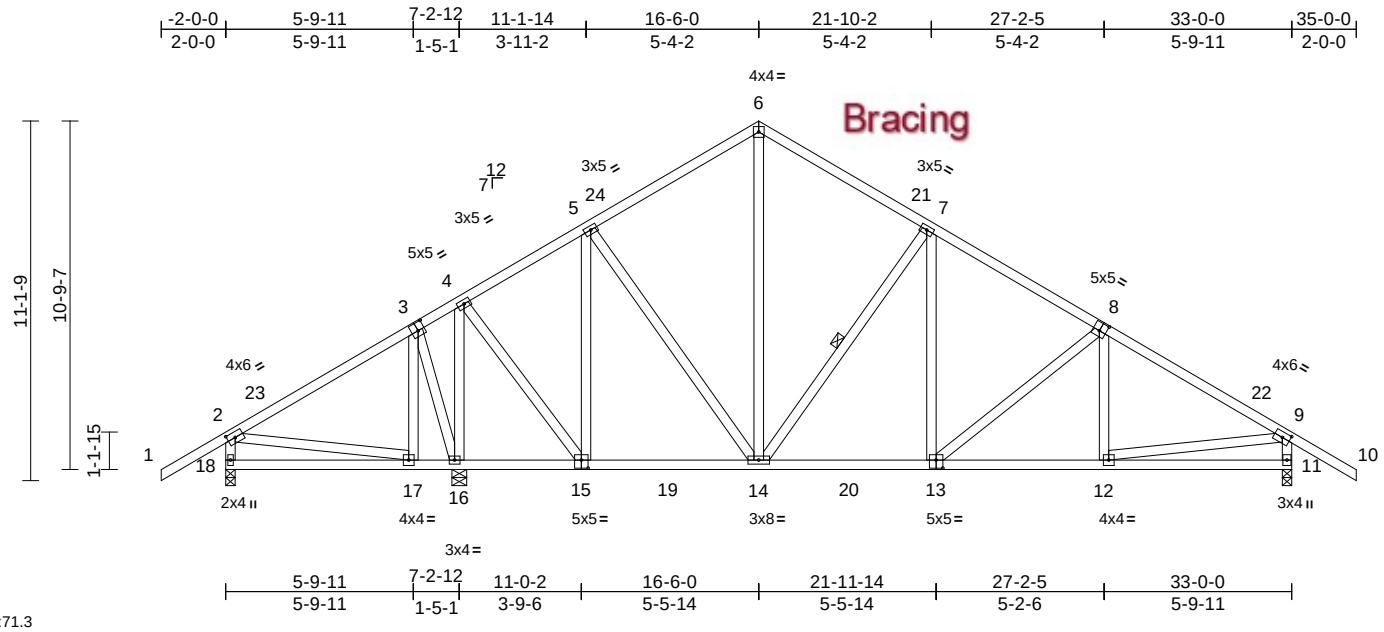


Scale = 1:71.3

Plate Offsets (X, Y): [2:0-2-14,0-2-0], [3:0-2-8,0-3-0], [8:0-2-8,0-3-0], [9:0-2-14,0-2-0], [13:0-2-8,0-3-0], [15:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.32	Vert(LL)	-0.06	13-14	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.39	Vert(CT)	-0.10	13-14	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.60	Horz(CT)	0.02	11	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 238 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

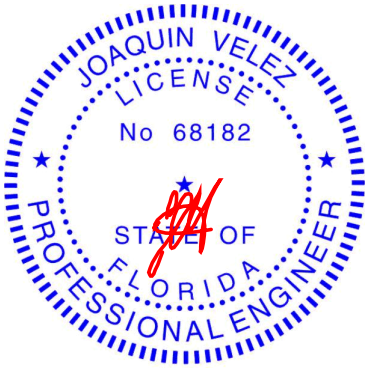
BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 7-14

REACTIONS (size) 11=0-3-8, 16=0-5-8, 18=0-3-8
Max Horiz 18=217 (LC 11)
Max Uplift 11=-52 (LC 12), 18=-55 (LC 12)
Max Grav 11=1239 (LC 18), 16=1693 (LC 17), 18=342 (LC 23)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-18=-286/112, 9-11=-1138/126, 6-7=-766/141, 7-9=-1439/98, 9-10=0/65, 1-2=0/65, 2-4=-76/262, 4-5=-519/92, 5-6=-783/144
BOT CHORD 17-18=-150/263, 16-17=-215/108, 14-16=-318/493, 12-14=0/1147, 11-12=0/158
WEBS 2-17=-257/84, 9-12=0/1005, 3-17=0/222, 5-15=-638/51, 5-14=0/360, 6-14=-45/449, 7-14=-640/88, 7-13=0/413, 8-13=-308/51, 8-12=-44/114, 4-15=0/1035, 4-16=-1228/7, 3-16=-439/50

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=33ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-3-10, Zone1 1-3-10 to 16-6-0, Zone2 16-6-0 to 21-2-0, Zone1 21-2-0 to 35-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 18 and 52 lb uplift at joint 11.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182
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16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

December 3,2024

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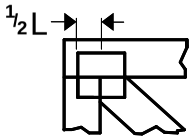
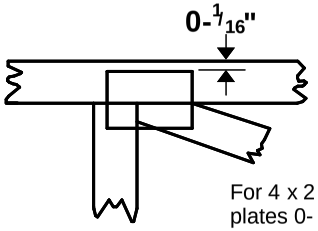
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

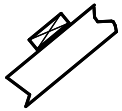
* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

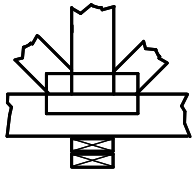
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

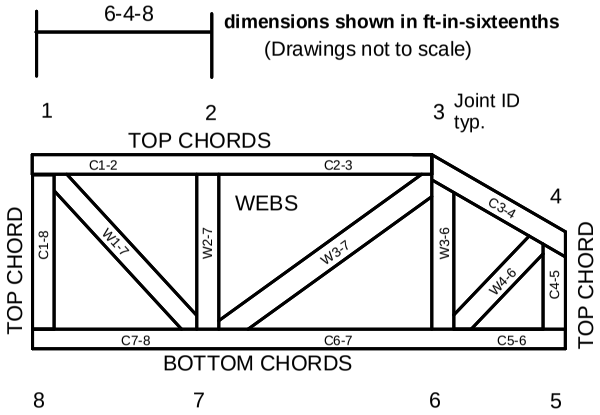


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek®

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.