Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	T17063434
1767895	CJ01	Jack-Open	4	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:15:51 2019 Page 1
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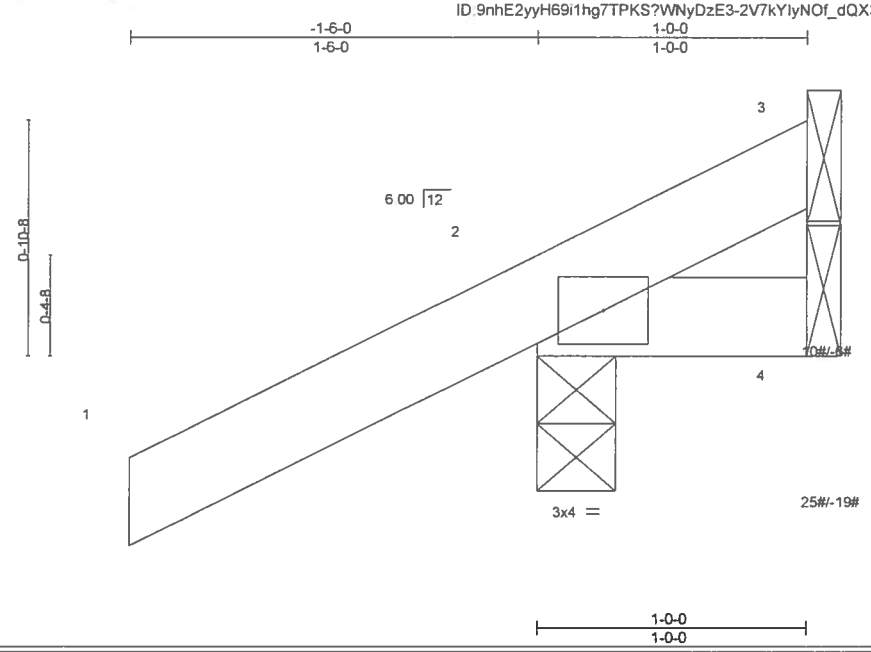


LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	0.00	7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.03	Vert(CT)	0.00	7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MP							Weight: 6 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

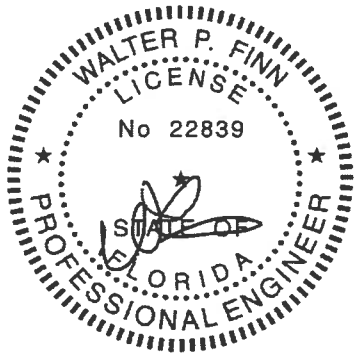
BRACING-
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=-6/Mechanical, 2=179/0-3-8, 4=-19/Mechanical
Max Horz 2=55(LC 12)
Max Uplift 3=-6(LC 1), 2=-107(LC 12), 4=-19(LC 1)
Max Grav 3=10(LC 16), 2=179(LC 1), 4=25(LC 16)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-15
BOT CHORD 2-4=0

JOINT STRESS INDEX
2 = 0.48

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint 3, 107 lb uplift at joint 2 and 19 lb uplift at joint 4.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	T17063435
1767895	CJ03	Jack-Open	2	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8 240 s Dec 6 2018 MiTek Industries, Inc Wed May 15 06 15 52 2019 Page 1
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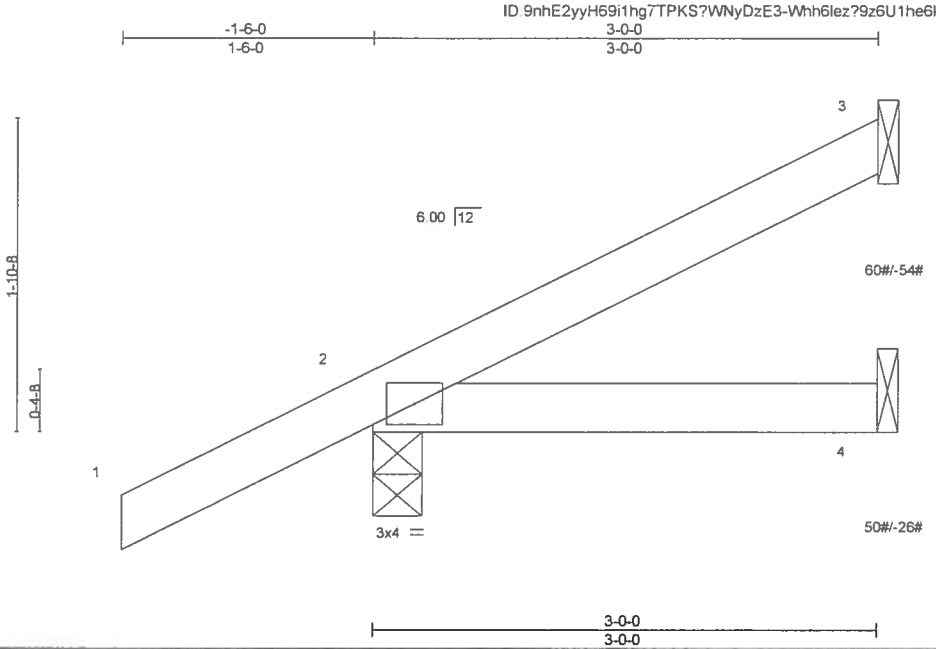

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	0.01	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.10	Vert(CT)	-0.01	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MP							Weight: 12 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=60/Mechanical, 2=210/0-3-8, 4=29/Mechanical
Max Horz 2=103(LC 12)
Max Uplift 3=-54(LC 12), 2=-97(LC 12), 4=-26(LC 9)
Max Grav 3=60(LC 1), 2=210(LC 1), 4=50(LC 3)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=21
BOT CHORD 2-4=0

JOINT STRESS INDEX
2 = 0.51

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 3, 97 lb uplift at joint 2 and 26 lb uplift at joint 4.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.

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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	T17063436
1767895	CJ03A	Jack-Open	2	1	Job Reference (optional)	

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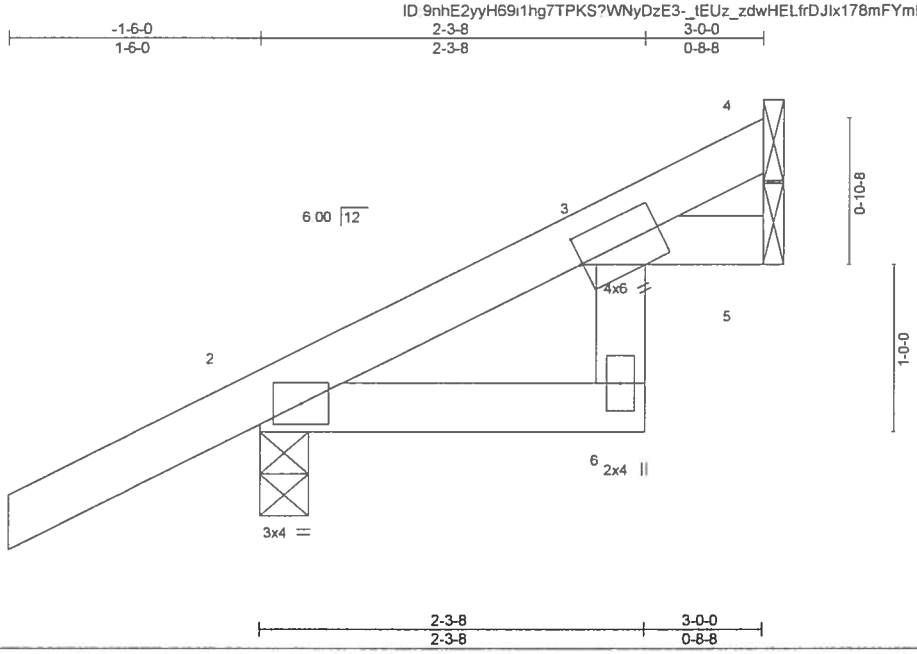


Plate Offsets (X,Y)-- [3:0-0-3,0-1-15]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.08	Vert(CT)	-0.01	6	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MR							Weight: 14 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
3-6: 2x4 SP No.3

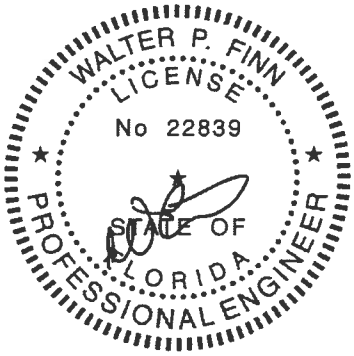
BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 4=48/Mechanical, 2=210/0-3-8, 5=41/Mechanical
Max Horz 2=103(LC 12)
Max Uplift 4=-35(LC 12), 2=-97(LC 12), 5=-19(LC 12)
Max Grav 4=48(LC 1), 2=210(LC 1), 5=43(LC 3)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-48, 3-4=19
BOT CHORD 2-6=9, 3-6=31, 3-5=-6

JOINT STRESS INDEX
2 = 0.37, 3 = 0.12 and 6 = 0.29

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 4, 97 lb uplift at joint 2 and 19 lb uplift at joint 5.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	T17063437
1767895	CJ05	Jack-Open	2	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:15:54 2019 Page 1
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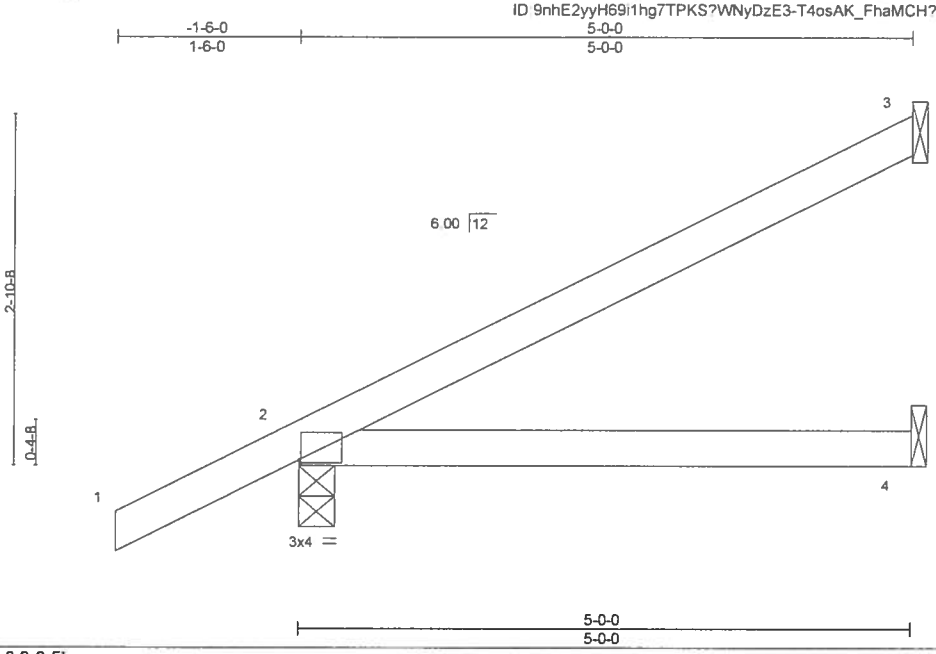


Plate Offsets (X,Y)— [2:0-0-3,0-0-5]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.41	Vert(LL)	0.09	4-7	>673	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.37	Vert(CT)	0.08	4-7	>770	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MP							Weight: 18 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

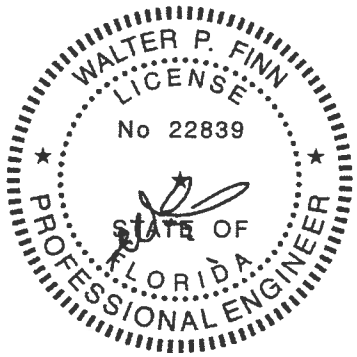
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=113/Mechanical, 2=276/0-3-8, 4=58/Mechanical
Max Horz 2=151(LC 12)
Max Uplift 3=-102(LC 12), 2=-112(LC 12), 4=-46(LC 9)
Max Grav 3=113(LC 1), 2=276(LC 1), 4=88(LC 3)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=40
BOT CHORD 2-4=0

JOINT STRESS INDEX
2 = 0.48

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 3, 112 lb uplift at joint 2 and 46 lb uplift at joint 4.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	
1767895	CJ05A	Jack-Open	2	1	Job Reference (optional)	T17063438

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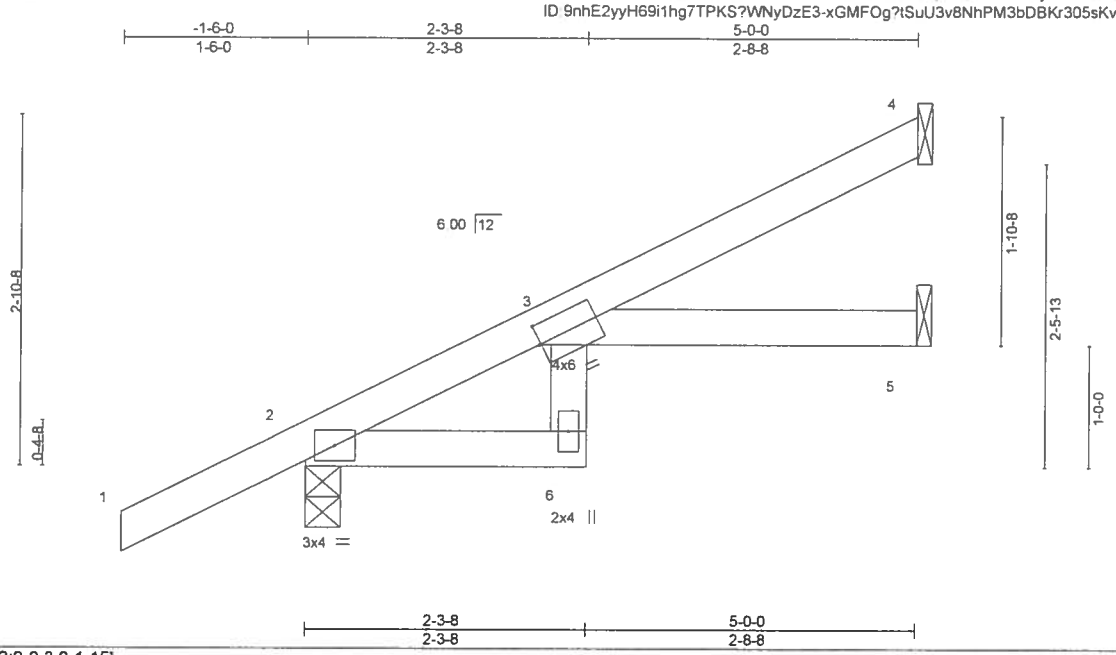


Plate Offsets (X,Y)— [3:0-0-3,0-1-15]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.38	Vert(LL)	0.06	6	>984	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.27	Vert(CT)	-0.07	6	>849	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.04	5	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MR							Weight: 20 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
3-6: 2x4 SP No.3

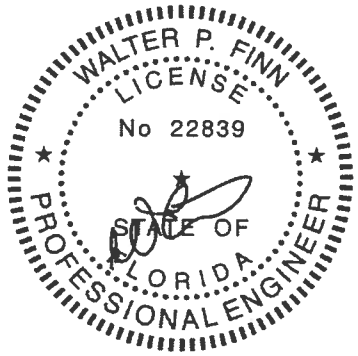
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 4=101/Mechanical, 2=276/0-3-8, 5=70/Mechanical
Max Horz 2=151(LC 12)
Max Uplift 4=-82(LC 12), 2=-112(LC 12), 5=-24(LC 12)
Max Grav 4=101(LC 1), 2=276(LC 1), 5=81(LC 3)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-113, 3-4=38
BOT CHORD 2-6=26, 3-6=75, 3-5=-3

JOINT STRESS INDEX
2 = 0.50, 3 = 0.63 and 6 = 0.57

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 4, 112 lb uplift at joint 2 and 24 lb uplift at joint 5.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	EJ01	Jack-Partial	18	1	Job Reference (optional)	T17063439

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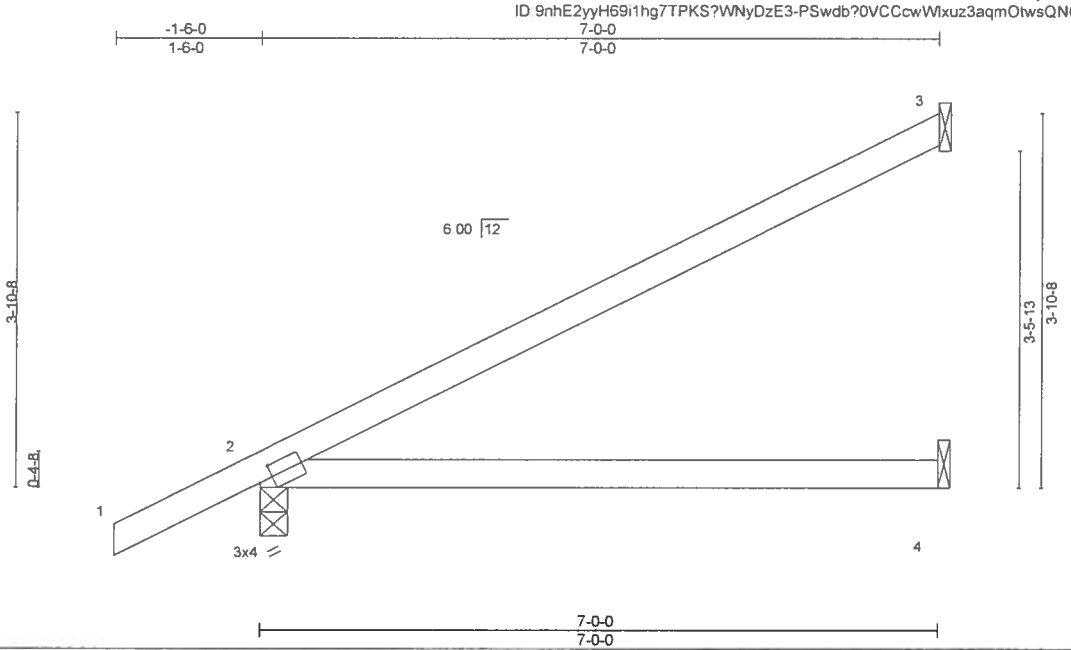

Plate Offsets (X,Y)— [2:0-1-13,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.69	Vert(LL)	0.14	4-7	>588	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.22	4-7	>385	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.01	2	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 25 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=164/Mechanical, 2=346/0-3-8, 4=84/Mechanical
Max Horz 2=138(LC 12)
Max Uplift 3=-95(LC 12), 2=-69(LC 12)
Max Grav 3=164(LC 1), 2=346(LC 1), 4=126(LC 3)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=58
BOT CHORD 2-4=0

JOINT STRESS INDEX
2 = 0.64

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3 and 69 lb uplift at joint 2.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.

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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	
1767895	EJ02	Jack-Partial	3	1	Job Reference (optional)	T17063440

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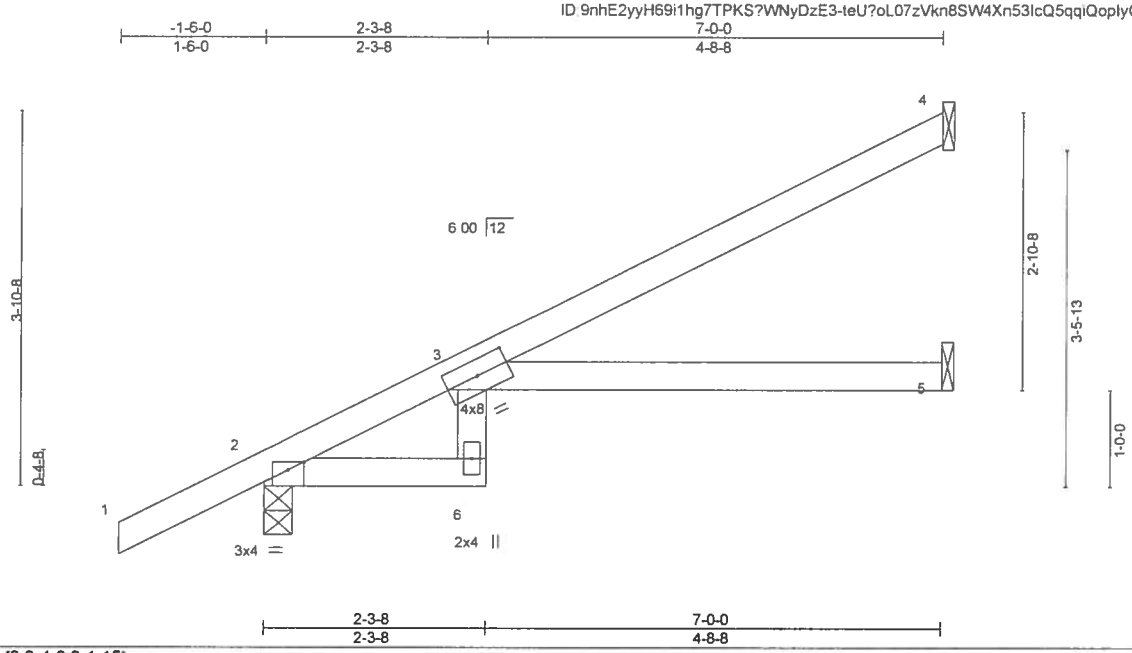

Plate Offsets (X,Y)— [3:0-4-0,0-1-15]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.68	Vert(LL)	0.21	3-5	>392	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.58	Vert(CT)	-0.26	3-5	>315	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.13	5	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MR							Weight: 26 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
3-6: 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 4=151/Mechanical, 2=347/0-3-8, 5=97/Mechanical
Max Horz 2=138(LC 12)
Max Uplift 4=-82(LC 12), 2=-69(LC 12), 5=-12(LC 12)
Max Grav 4=151(LC 1), 2=347(LC 1), 5=118(LC 3)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-159, 3-4=56
BOT CHORD 2-6=26, 3-6=107, 3-5=-2

JOINT STRESS INDEX
2 = 0.40, 3 = 0.72 and 6 = 0.48

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 4, 69 lb uplift at joint 2 and 12 lb uplift at joint 5.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.

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Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST - MICHAUD RES	
1767895	HJ10	Diagonal Hip Girder	1	1	Job Reference (optional)	T17063441

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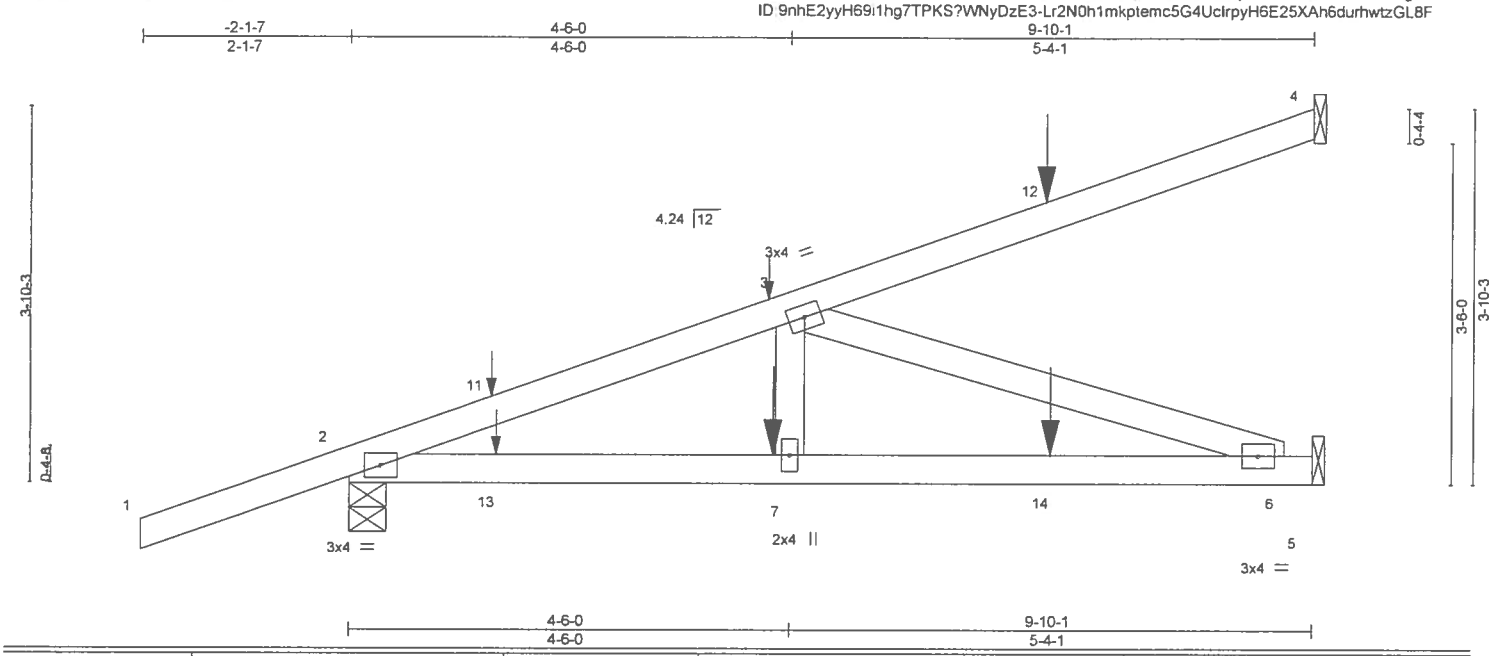

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.58	Vert(LL) 0.11 6-7 >999 240	MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.62	Vert(CT) -0.12 6-7 >992 180	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.44	Horz(CT) -0.01 5 n/a n/a	
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MS		Weight: 43 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-9-3 oc bracing.

REACTIONS. (lb/size) 4=149/Mechanical, 2=527/0-4-9, 5=299/Mechanical
Max Horz 2=217(LC 22)
Max Uplift 4=-138(LC 4), 2=-460(LC 4), 5=-266(LC 4)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=38, 2-11=-799, 3-11=-736, 3-12=-69, 4-12=40
BOT CHORD 2-13=729, 7-13=729, 7-14=729, 6-14=729, 5-6=0
WEBS 3-7=159, 3-6=-768

JOINT STRESS INDEX
2 = 0.48, 3 = 0.35, 6 = 0.31 and 7 = 0.16

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 4, 460 lb uplift at joint 2 and 266 lb uplift at joint 5.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 84 lb down and 76 lb up at 1-6-1, 84 lb down and 76 lb up at 1-6-1, 28 lb down and 45 lb up at 4-4-0, 28 lb down and 45 lb up at 4-4-0, and 51 lb down and 101 lb up at 7-1-15, and 51 lb down and 101 lb up at 7-1-15 on top chord, and 58 lb down and 43 lb up at 1-6-1, 58 lb down and 43 lb up at 1-6-1, 20 lb down and 34 lb up at 4-4-0, 20 lb down and 34 lb up at 4-4-0, and 41 lb down and 61 lb up at 7-1-15, and 41 lb down and 61 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 5-8=-20
Concentrated Loads (lb)
Vert: 7=-6(F=-3, B=-3) 12=-73(F=-36, B=-36) 14=-59(F=-29, B=-29)

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May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	HJ10A	Diagonal Hip Girder	1	1	Job Reference (optional)	T17063442

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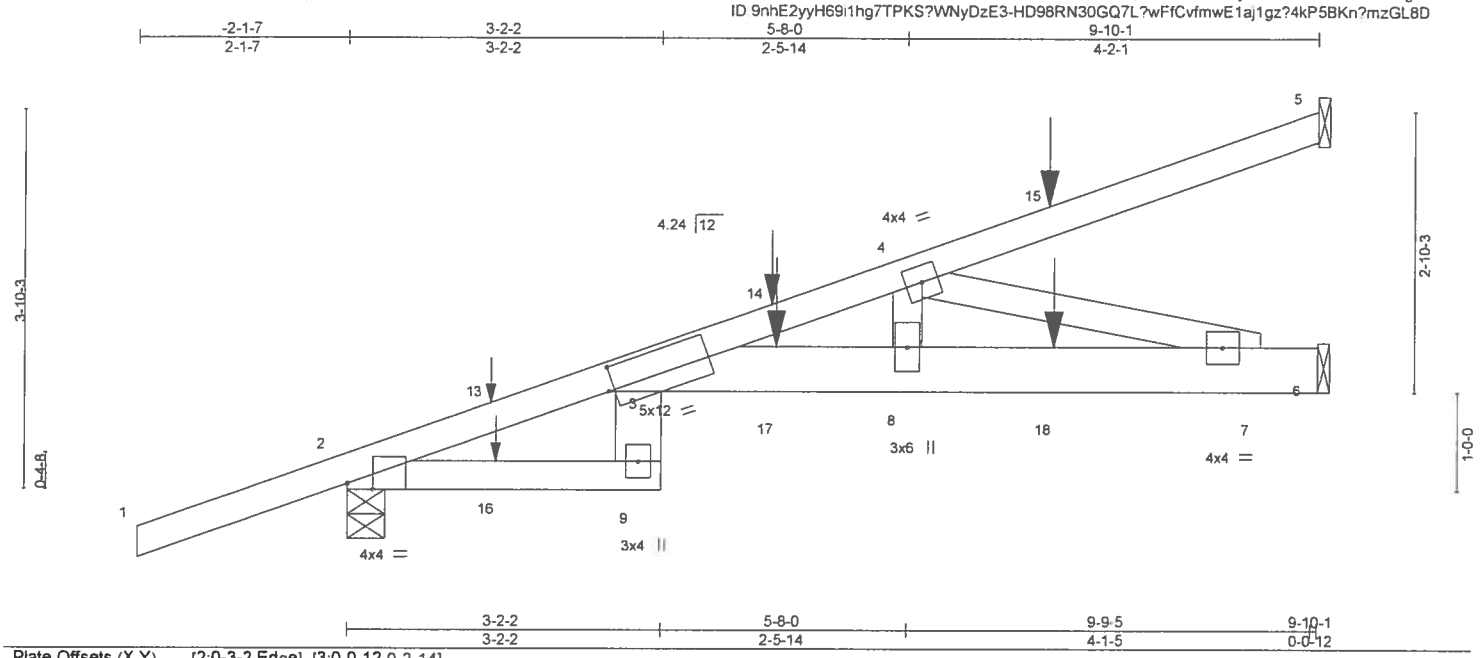


3-2-2	5-8-0	9-9-5	9-10-1		
3-2-2	2-5-14	4-1-5	0-0-12		

Plate Offsets (X,Y)— [2:0-3-2,Edge], [3:0-0-12,0-2-14]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.83	Vert(LL)	0.34	3	>340	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.85	Vert(CT)	-0.40	3	>295	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.40	Horz(CT)	0.16	6	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 48 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP M 31
BOT CHORD 2x6 SP No.2 *Except*
2-9: 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-0 oc bracing.

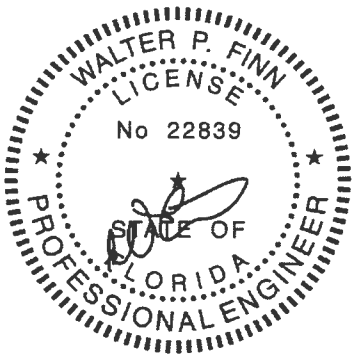
REACTIONS. (lb/size) 5=106/Mechanical, 2=539/0-4-9, 6=354/Mechanical
Max Horz 2=217(LC 4)
Max Uplift 5=-92(LC 4), 2=-378(LC 4), 6=-216(LC 8)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=38, 2-13=-298, 3-13=-256, 3-14=-1293, 4-14=-1273, 4-15=-55, 5-15=28
BOT CHORD 2-16=132, 9-16=132, 3-9=214, 3-17=1227, 8-17=1224, 8-18=1240, 7-18=1240, 6-7=0
WEBS 4-8=419, 4-7=-1286

JOINT STRESS INDEX
2 = 0.90, 3 = 0.58, 4 = 0.51, 7 = 0.54, 8 = 0.18 and 9 = 0.57

NOTES-
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 5, 378 lb uplift at joint 2 and 216 lb uplift at joint 6.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 84 lb down and 76 lb up at 1-6-1, 84 lb down and 76 lb up at 1-6-1, 26 lb down and 24 lb up at 4-4-0, 26 lb down and 24 lb up at 4-4-0, and 49 lb down and 81 lb up at 7-1-15, and 49 lb down and 81 lb up at 7-1-15 on top chord, and 25 lb down and 43 lb up at 1-6-1, 25 lb down and 43 lb up at 1-6-1, 23 lb down and 28 lb up at 4-4-0, 23 lb down and 28 lb up at 4-4-0, and 42 lb down and 40 lb up at 7-1-15, and 42 lb down and 40 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 9-10=-20, 3-6=-20



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Continued on page 2

MiTek
6904 Parke East Blvd
Tampa, FL 36610

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 14=-2(F=-1, B=-1) 15=-48(F=-24, B=-24) 17=-33(F=-16, B=-16) 18=-83(F=-42, B=-42)

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	
1767895	T01	Common	8	1	Job Reference (optional)	T17063443

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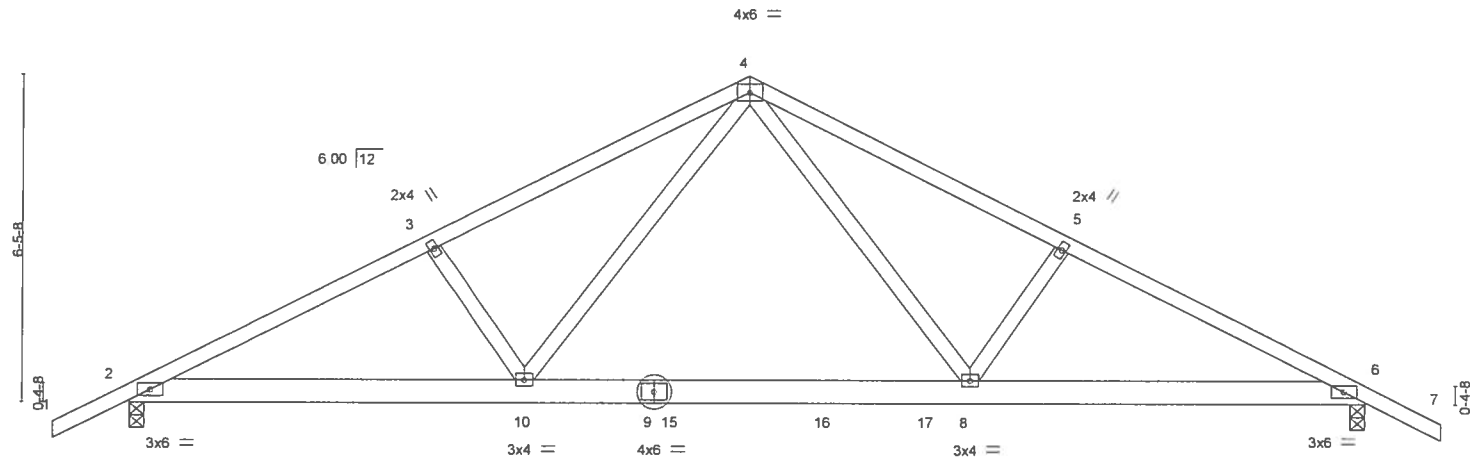
-1-6-0	5-11-14	12-2-0	18-4-2	24-4-0	25-10-0
1-6-0	5-11-14	6-2-2	6-2-2	5-11-14	1-6-0



0-3-8(0-1-8)	7-9-7	16-6-9	24-4-0	0-3-8(0-1-8)
1227#/-582#	7-9-7	8-9-2	7-9-7	1211#/-494#

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.64	Vert(LL) 0.19 8-10 >999 240	MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.76	Vert(CT) -0.32 8-10 >910 180	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.53	Horz(CT) 0.04 6 n/a n/a	
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MS		Weight: 133 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-1 oc bracing.

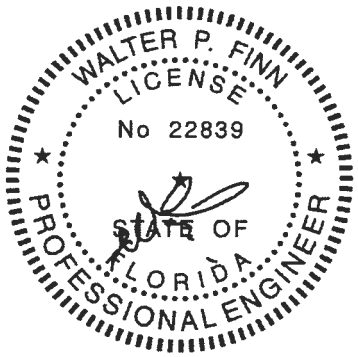
REACTIONS. (lb/size) 2=1227/0-3-8, 6=1211/0-3-8
Max Horz 2=144(LC 16)
Max Uplift 2=-502(LC 12), 6=-494(LC 13)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-2224, 3-4=-2069, 4-5=-2031, 5-6=-2186, 6-7=36
BOT CHORD 2-10=1942, 9-10=1242, 9-15=1242, 15-16=1242, 16-17=1242, 8-17=1242, 6-8=1908
WEBS 4-8=860, 5-8=-301, 4-10=918, 3-10=-301

JOINT STRESS INDEX
2 = 0.56, 3 = 0.27, 4 = 0.57, 5 = 0.27, 6 = 0.56, 8 = 0.61, 9 = 0.82 and 10 = 0.61

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 2 and 494 lb uplift at joint 6.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-20, 10-17=-80(F=-60), 6-17=-20



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Date:

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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T01G	Common Supported Gable	1	1	Job Reference (optional)	T17063444

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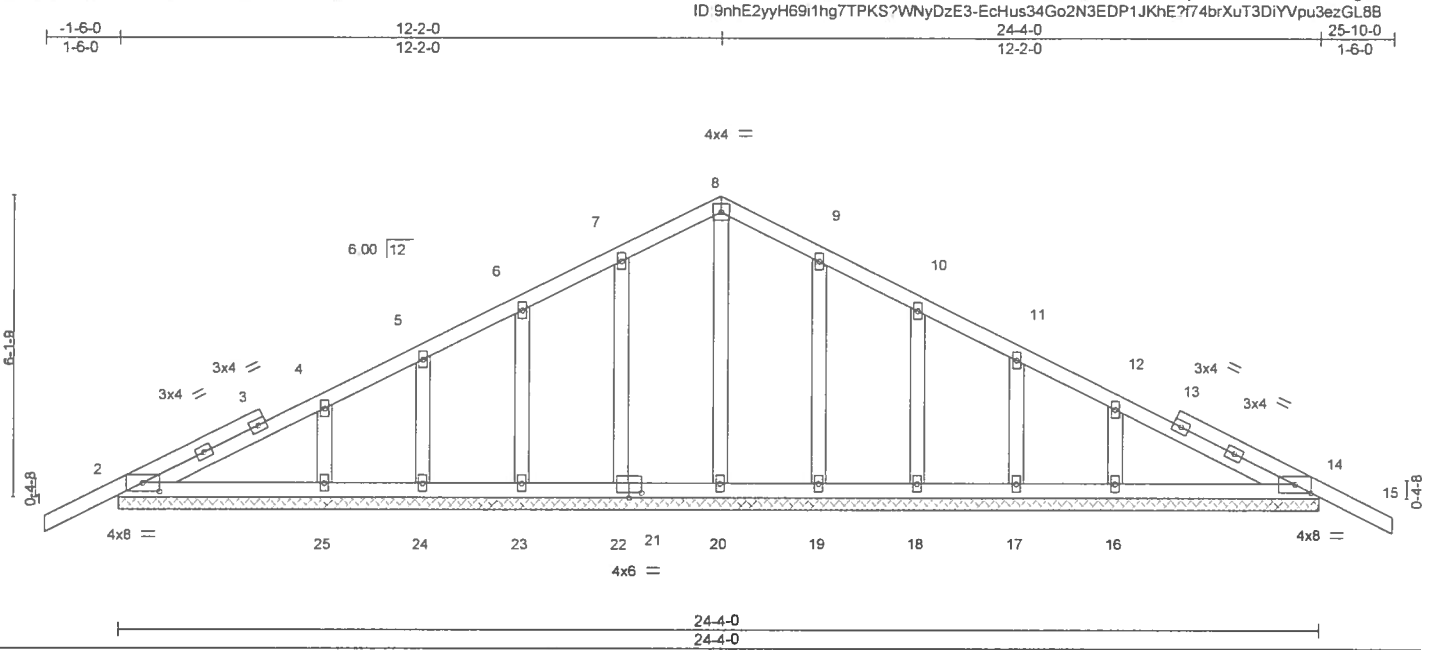


Plate Offsets (X,Y)-- [2:0-4-0,0-2-1], [14:0-4-0,0-2-1], [21:0-0-0,0-1-12], [21:0-3-0,0-1-4], [22:0-1-12,0-0-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	-0.00	15	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.11	Vert(CT)	-0.00	15	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.01	14	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S							Weight: 135 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

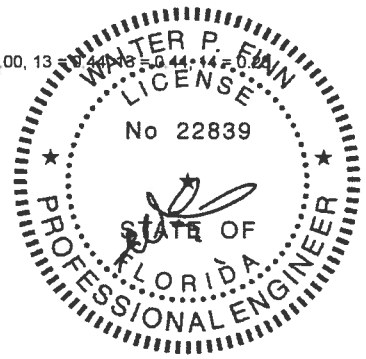
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 24-4-0.
(lb) - Max Horz 2=-137(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 22, 23, 24, 19, 18, 17 except 25=-138(LC 12), 16=-143(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 14, 20, 22, 23, 24, 19, 18, 17 except 25=262(LC 23), 16=262(LC 24)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=34, 2-3=-42, 3-4=51, 4-5=-30, 5-6=27, 6-7=26, 7-8=23, 8-9=-25, 9-10=-23, 10-11=-21, 11-12=-30, 12-13=51, 13-14=-42, 14-15=34
BOT CHORD 2-25=-1, 24-25=-1, 23-24=-1, 22-23=-1, 21-22=-1, 20-21=-1, 19-20=-1, 18-19=-1, 17-18=-1, 16-17=-1, 14-16=-1
WEBS 8-20=-99, 7-22=-113, 6-23=-111, 5-24=-88, 4-25=-181, 9-19=-113, 10-18=-111, 11-17=-88, 12-16=-181

JOINT STRESS INDEX
2 = 0.28, 3 = 0.00, 3 = 0.44, 3 = 0.44, 4 = 0.27, 5 = 0.27, 6 = 0.27, 7 = 0.27, 8 = 0.22, 9 = 0.27, 10 = 0.27, 11 = 0.27, 12 = 0.27, 13 = 0.00, 13 = 0.44, 14 = 0.44, 14 = 0.28, 16 = 0.27, 17 = 0.27, 18 = 0.27, 19 = 0.27, 20 = 0.27, 21 = 0.17, 21 = 0.24, 22 = 0.24, 23 = 0.27, 24 = 0.27 and 25 = 0.27

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 22, 23, 24, 19, 18, 17 except (jt=lb) 25=138, 16=143.
11) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T02	Common	5	1	Job Reference (optional)	T17063445

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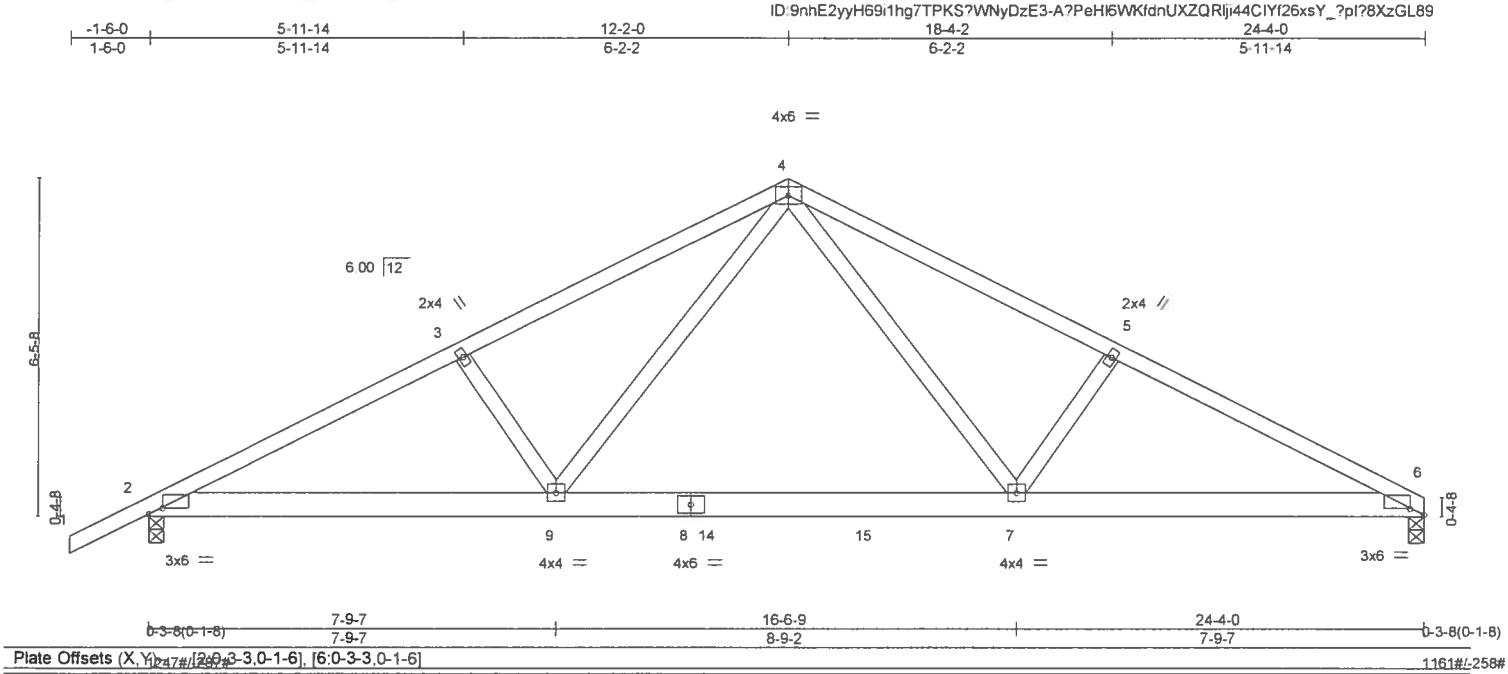


LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.65	Vert(LL)	0.19	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.77	Vert(CT)	-0.33	7-9	>895	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.54	Horz(CT)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 131 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-3-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-2 oc bracing.

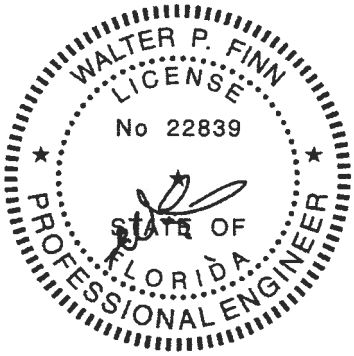
REACTIONS. (lb/size) 6=1161/0-3-8, 2=1247/0-3-8
Max Horz 2=96(LC 12)
Max Uplift 6=-258(LC 13), 2=-287(LC 12)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-2268, 3-4=-2113, 4-5=-2124, 5-6=-2279
BOT CHORD 2-9=1981, 8-9=1281, 8-14=1281, 14-15=1281, 7-15=1281, 6-7=1993
WEBS 4-7=936, 5-7=-304, 4-9=919, 3-9=-300

JOINT STRESS INDEX
2 = 0.60, 3 = 0.27, 4 = 0.58, 5 = 0.27, 6 = 0.60, 7 = 0.44, 8 = 0.83 and 9 = 0.44

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=258, 2=287.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-20, 7-9=-80(F=-60), 6-7=-20



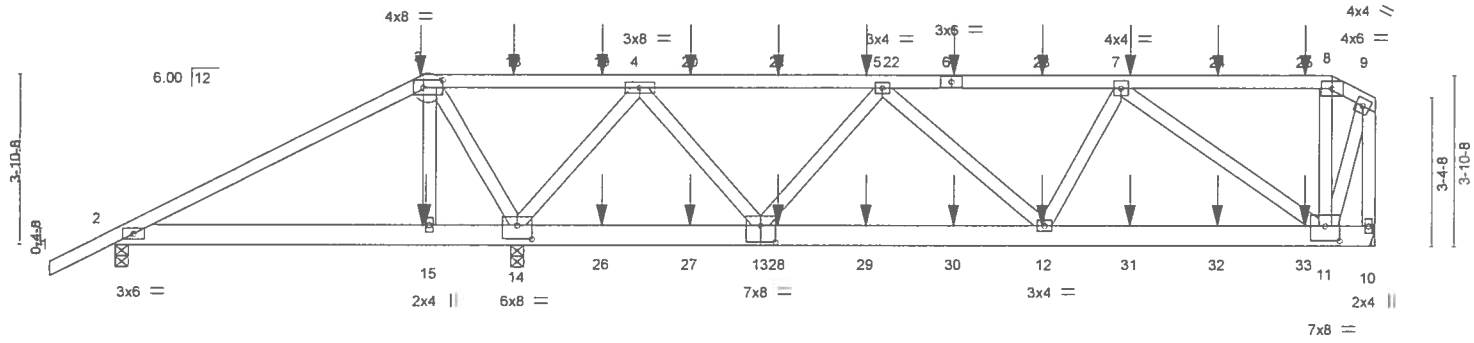
Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	T17063446
1767895	T03	Hip Girder	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc Wed May 15 06:16:06 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-6NXPhQ7nsGtVjripYAmA9VHbQSqgPg3HT7n6CQzGL87

-1-6-0	7-0-0	11-10-14	17-5-1	22-10-2	27-8-0	28-8-0
1-6-0	7-0-0	4-10-14	5-6-4	5-5-1	4-9-14	1-0-0



7-0-0	9-1-12	14-8-0	21-1-6	27-8-0	28-8-0
7-0-0	2-1-12	5-6-4	6-5-6	6-6-10	1-0-0

Plate Offsets (X,Y)– [3:0-5-4,0-2-0], [11:0-4-0,0-4-0], [13:0-4-0,0-4-8], [14:0-4-0,0-3-12]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.88	Vert(LL)	-0.05	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.44	Vert(CT)	-0.10	12-13	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.93	Horz(CT)	0.02	10	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 176 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

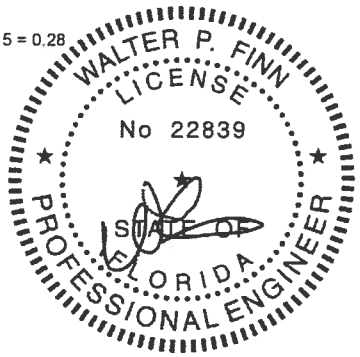
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-2-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 2=105/0-3-8, 14=2888/0-3-8, 10=1318/Mechanical
Max Horz 2=130(LC 8)
Max Uplift 2=-197(LC 24), 14=-1110(LC 5), 10=-368(LC 4)
Max Grav 2=201(LC 18), 14=2888(LC 1), 10=1324(LC 20)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=546, 3-18=1088, 18-19=1088, 4-19=1088, 4-20=-961, 20-21=-961, 21-22=-961, 5-22=-961, 5-6=-1643, 6-23=-1643, 7-23=-1643, 7-24=-403, 24-25=-403, 8-25=-403, 8-9=-434, 9-10=-1354
BOT CHORD 2-15=-421, 14-15=-403, 14-26=172, 26-27=172, 13-27=172, 13-28=1522, 28-29=1522, 29-30=1522, 12-30=1522, 12-31=1522, 31-32=1522, 32-33=1522, 11-33=1522, 10-11=17
WEBS 3-15=451, 3-14=-1311, 4-14=-2034, 4-13=1274, 5-13=-906, 5-12=166, 7-12=273, 7-11=-1397, 8-11=-130, 9-11=1257

JOINT STRESS INDEX
2 = 0.41, 3 = 0.97, 4 = 0.75, 5 = 0.41, 6 = 0.30, 7 = 0.53, 8 = 0.48, 9 = 0.60, 10 = 0.67, 11 = 0.47, 12 = 0.37, 13 = 0.33, 14 = 0.43 and 15 = 0.28

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=197, 14=1110, 10=368.
9) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Date:
May 15,2019

Continued on page 2

6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	T17063446
1767895	T03	Hip Girder	1	1	Job Reference (optional)	

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 128 lb down and 102 lb up at 7-0-0, 110 lb down and 102 lb up at 9-0-12, 110 lb down and 102 lb up at 11-0-12, 110 lb down and 102 lb up at 13-0-12, 110 lb down and 102 lb up at 15-0-12, 110 lb down and 102 lb up at 17-0-12, 110 lb down and 102 lb up at 19-0-12, 110 lb down and 102 lb up at 21-0-12, 110 lb down and 102 lb up at 23-0-12, and 110 lb down and 102 lb up at 25-0-12, and 117 lb down and 102 lb up at 27-0-12 on top chord, and 335 lb down and 300 lb up at 7-0-0, 86 lb down at 11-0-12, 86 lb down at 13-0-12, 86 lb down at 15-0-12, 86 lb down at 17-0-12, 86 lb down at 19-0-12, 86 lb down at 21-0-12, 86 lb down at 23-0-12, and 86 lb down at 25-0-12, and 86 lb down at 27-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

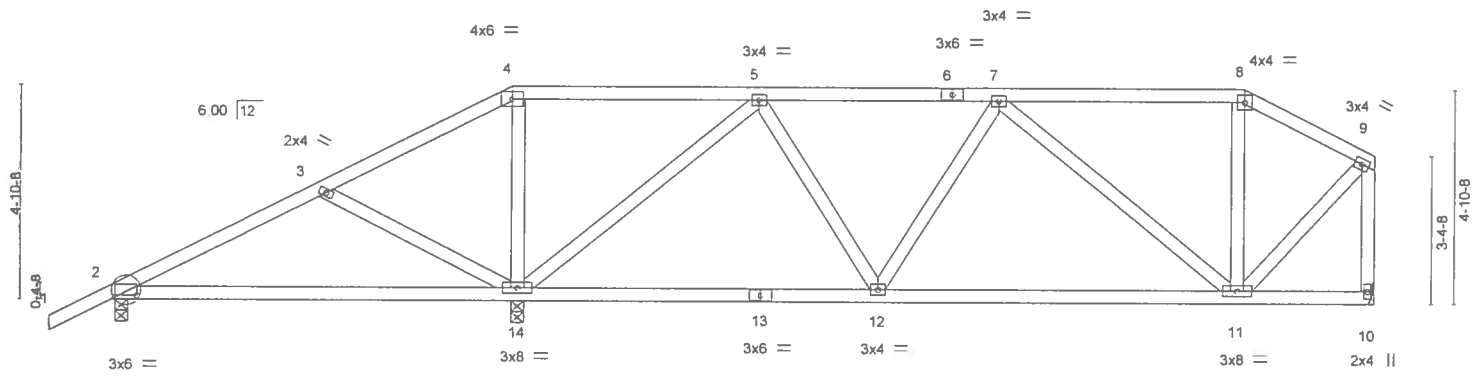
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-8=-54, 8-9=-54, 2-10=-20
   Concentrated Loads (lb)
   Vert: 3=-110(F) 6=-110(F) 15=-335(F) 12=-64(F) 7=-110(F) 18=-110(F) 19=-110(F) 20=-110(F) 21=-110(F) 22=-110(F) 23=-110(F) 24=-110(F) 25=-110(F) 26=-64(F) 27=-64(F) 28=-64(F) 29=-64(F) 30=-64(F) 31=-64(F) 32=-64(F) 33=-64(F)

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T04	Hip	1	1	Job Reference (optional)	T17063447

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc Wed May 15 06:16:07 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-aa5nvm8Pda?ML_H?6tHPijqsjs6U88tRinXfkszGL86

-1-6-0	4-9-8	9-0-0	14-7-5	20-0-11	25-8-0	28-8-0
1-6-0	4-9-8	4-2-8	5-7-5	5-5-7	5-7-5	3-0-0



9-0-0	9-1-12	17-4-0	25-8-0	28-8-0
9-0-0	0-1-12	8-2-4	8-4-0	3-0-0

Plate Offsets (X,Y)— [2:0-6-0,0-0-7]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.46	Vert(LL)	0.30	14-17	>371	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.60	Vert(CT)	0.26	14-17	>426	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.89	Horz(CT)	0.01	10	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 156 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

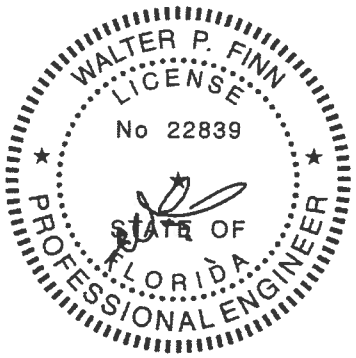
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 2=261/0-3-8, 14=1292/0-3-8, 10=639/Mechanical
Max Horz 2=142(LC 12)
Max Uplift 2=-108(LC 9), 14=-412(LC 9), 10=-147(LC 8)
Max Grav 2=264(LC 23), 14=1292(LC 1), 10=655(LC 24)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=63, 3-4=356, 4-5=289, 5-6=-561, 6-7=-561, 7-8=-373, 8-9=-447, 9-10=-638
BOT CHORD 2-14=8, 13-14=403, 12-13=403, 11-12=624, 10-11=3
WEBS 3-14=-322, 4-14=-378, 5-14=-903, 5-12=310, 7-12=-122, 7-11=-327, 8-11=-41, 9-11=538

JOINT STRESS INDEX
2 = 0.97, 3 = 0.27, 4 = 0.41, 5 = 0.45, 6 = 0.12, 7 = 0.37, 8 = 0.49, 9 = 0.46, 10 = 0.27, 11 = 0.45, 12 = 0.37, 13 = 0.26 and 14 = 0.54

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=108, 14=412, 10=147.
9) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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May 15,2019

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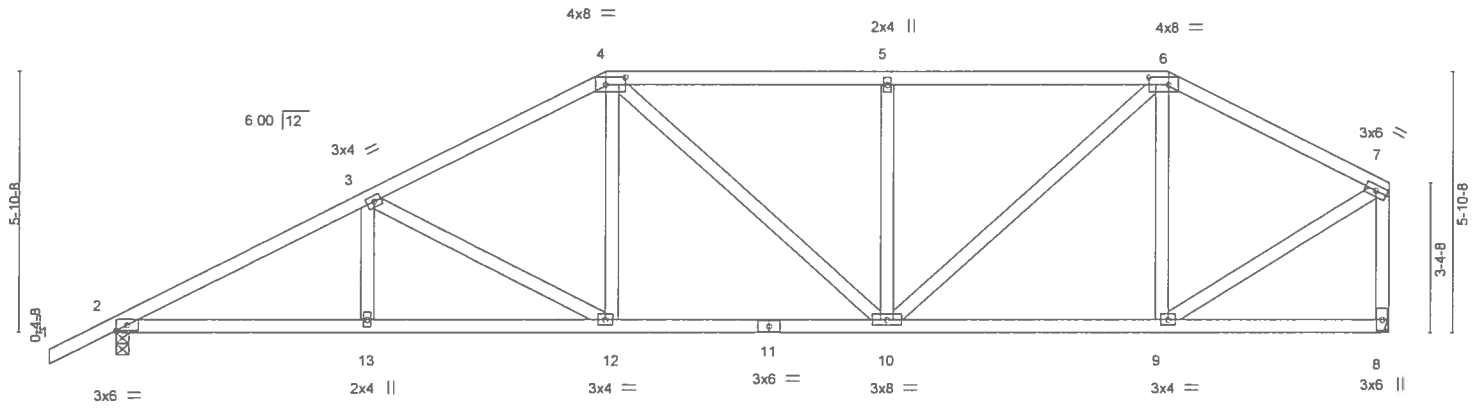
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T05	Hip	1	1	Job Reference (optional)	T17063448

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:08 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-2me96691Ou7Dy8sBgboeFwN1gGUXthPawRGDHIzGL85

-1-6-0	5-7-15	11-0-0	17-4-0	23-8-0	28-8-0
1-6-0	5-7-15	5-4-1	6-4-0	6-4-0	5-0-0



5-7-15	11-0-0	17-4-0	23-8-0	28-8-0
5-7-15	5-4-1	6-4-0	6-4-0	5-0-0

Plate Offsets (X,Y)— [4:0-5-4,0-2-0], [6:0-5-4,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	-0.07	12	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.48	Vert(CT)	-0.15	10-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.49	Horz(CT)	0.05	8	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 163 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

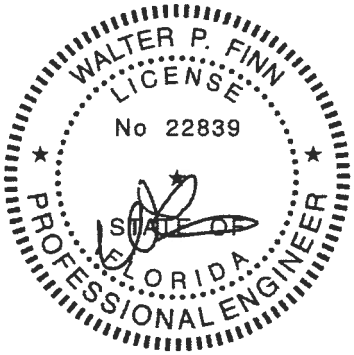
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-8 oc bracing.

REACTIONS. (lb/size) 2=1138/0-3-8, 8=1053/Mechanical
Max Horz 2=155(LC 12)
Max Uplift 2=-229(LC 12), 8=-183(LC 8)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1951, 3-4=-1512, 4-5=-1331, 5-6=-1331, 6-7=-947, 7-8=-1010
BOT CHORD 2-13=1694, 12-13=1694, 11-12=1297, 10-11=1297, 9-10=787, 8-9=22
WEBS 3-13=108, 3-12=-458, 4-12=334, 4-10=45, 5-10=-385, 6-10=726, 6-9=-368, 7-9=915

JOINT STRESS INDEX
2 = 0.49, 3 = 0.46, 4 = 0.44, 5 = 0.27, 6 = 0.49, 7 = 0.53, 8 = 0.23, 9 = 0.57, 10 = 0.58, 11 = 0.48, 12 = 0.37 and 13 = 0.27

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=229, 8=183.
9) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST - MICHAUD RES	T17063449
1767895	T06	Hip	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:09 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-WyCXKSAf9BF4aIROEIJtn8v9zgnpc7Wj950mpkzGL84

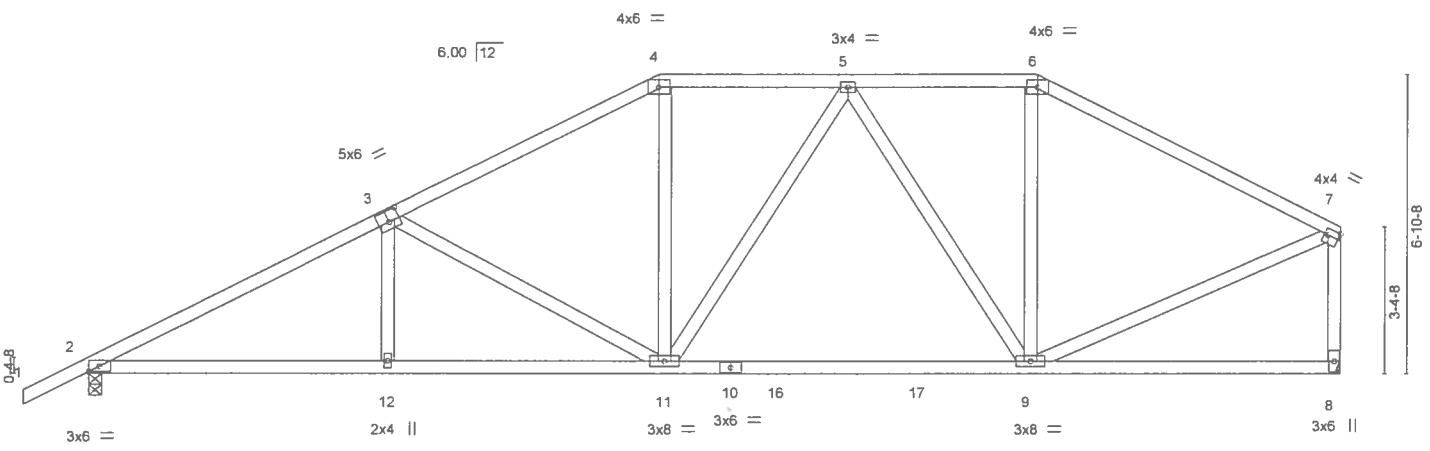


Plate Offsets (X,Y)— [3:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.67	Vert(LL)	-0.17	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.67	Vert(CT)	-0.30	9-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.56	Horz(CT)	0.05	8	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 163 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

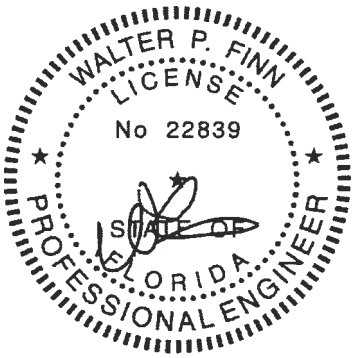
BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-9-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-2 oc bracing.

REACTIONS. (lb/size) 2=1138/0-3-8, 8=1053/Mechanical
Max Horz 2=167(LC 12)
Max Uplift 2=-240(LC 12), 8=-171(LC 13)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1904, 3-4=-1369, 4-5=-1154, 5-6=-876, 6-7=-1060, 7-8=-994
BOT CHORD 2-12=1643, 11-12=1645, 10-11=1087, 10-16=1087, 16-17=1087, 9-17=1087, 8-9=47
WEBS 3-12=136, 3-11=-567, 4-11=315, 5-11=125, 5-9=-392, 6-9=127, 7-9=905

JOINT STRESS INDEX
2 = 0.48, 3 = 0.59, 4 = 0.37, 5 = 0.37, 6 = 0.59, 7 = 0.56, 8 = 0.46, 9 = 0.66, 10 = 0.40, 11 = 0.46 and 12 = 0.27

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=240, 8=171.
9) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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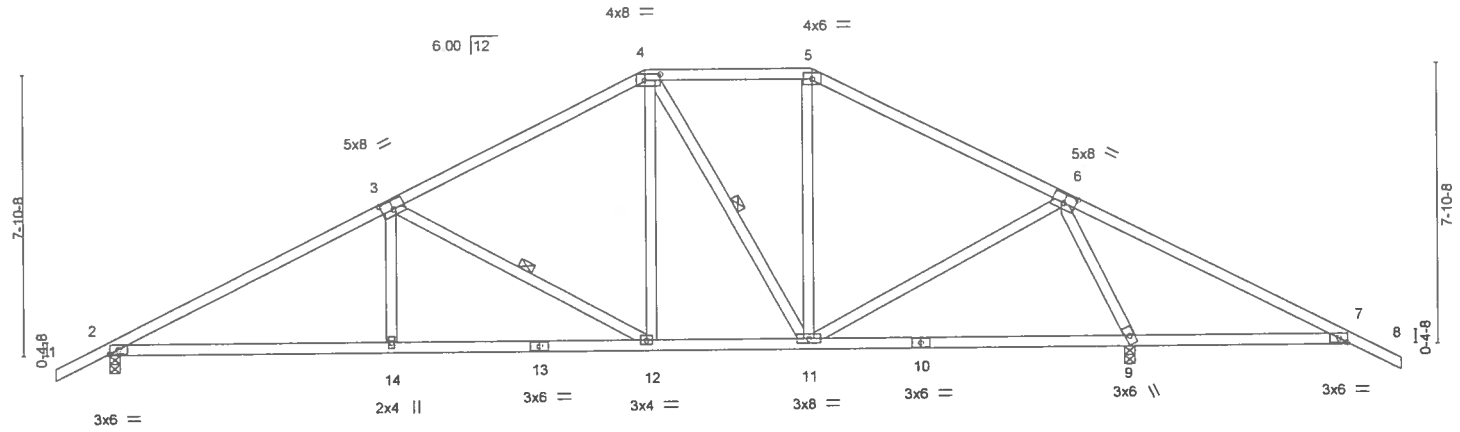
May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	T17063450
1767895	T07	Hip	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:10 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-?9mwXoBHvVNxCS0an?q6KLSJJ37oLYHtOIlJLBzGL83

-1-6-0	7-10-12	15-0-0	19-8-0	26-7-7	34-8-0	36-2-0
1-6-0	7-10-12	7-1-4	4-8-0	6-11-7	8-0-9	1-6-0



7-10-12	15-0-0	19-8-0	28-6-4	28-8-0	34-8-0
7-10-12	7-1-4	4-8-0	8-10-4	0-1-12	6-0-0

Plate Offsets (X,Y)-- [3:0-4-0,0-3-0], [4:0-5-4,0-2-0], [6:0-4-0,0-3-0], [7:0-2-15,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.76	Vert(LL)	-0.14	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.69	Vert(CT)	-0.28	9-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.04	9	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 183 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-8-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-12, 4-11

REACTIONS. (lb/size) 2=1074/0-3-8, 9=1653/0-3-8
Max Horz 2=112(LC 11)
Max Uplift 2=-247(LC 12), 9=-346(LC 13)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1721, 3-4=-1071, 4-5=-710, 5-6=-893, 6-7=733, 7-8=36
BOT CHORD 2-14=1470, 13-14=1470, 12-13=1470, 11-12=876, 10-11=91, 9-10=91, 7-9=-565
WEBS 3-14=185, 3-12=-688, 4-12=427, 4-11=-326, 5-11=71, 6-11=707, 6-9=-1576

JOINT STRESS INDEX
2 = 0.43, 3 = 0.60, 4 = 0.54, 5 = 0.50, 6 = 0.74, 7 = 0.43, 9 = 0.40, 10 = 0.63, 11 = 0.52, 12 = 0.38, 13 = 0.69 and 14 = 0.27

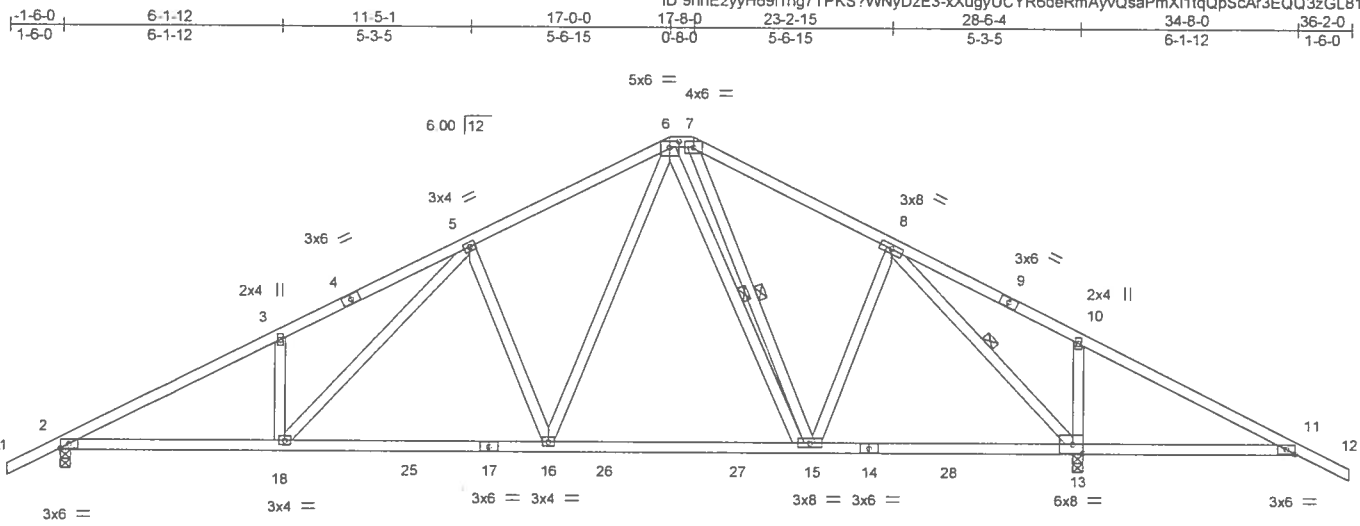
NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=247, 9=346.
8) This truss design conforms with Florida Building Code 2001, based on parameters indicated.

Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	T17063451
1767895	T08	Hip	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:12 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-xXugyUCYR6deRmAyvQsaPmXi1tqQpScAr3EQQ3zGL81



6-1-12	13-7-14	21-0-2	28-6-4	28-8-0	34-8-0
6-1-12	7-6-2	7-4-4	7-6-2	0-1-12	6-0-0

Plate Offsets (X,Y)— [6:0-3-0,0-2-0], [11:0-2-15,Edge], [13:0-3-8,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.49	Vert(LL)	-0.11	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.61	Vert(CT)	-0.21	16-18	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.67	Horz(CT)	0.04	13	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 203 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

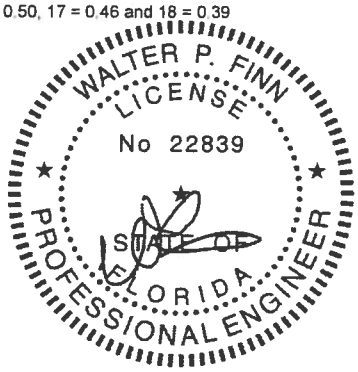
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-15, 7-15, 8-13

REACTIONS. (lb/size) 2=1070/0-3-8, 13=1658/0-3-8
Max Horz 2=-125(LC 10)
Max Uplift 2=-254(LC 12), 13=-360(LC 13)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1782, 3-4=-1779, 4-5=-1664, 5-6=-1138, 6-7=-586, 7-8=-777, 8-9=644, 9-10=529, 10-11=666, 11-12=36
BOT CHORD 2-18=1536, 18-25=1130, 17-25=1130, 16-17=1130, 16-26=707, 26-27=707, 15-27=707, 14-15=525, 14-28=525, 13-28=525, 11-13=-524
WEBS 3-18=-297, 5-18=601, 5-16=-501, 6-16=668, 6-15=-282, 7-15=122, 8-15=300, 8-13=-1555, 10-13=-334

JOINT STRESS INDEX
2 = 0.45, 3 = 0.27, 4 = 0.37, 5 = 0.46, 6 = 0.30, 7 = 0.38, 8 = 0.59, 9 = 0.21, 10 = 0.27, 11 = 0.35, 13 = 0.24, 14 = 0.23, 15 = 0.55, 16 = 0.50, 17 = 0.46 and 18 = 0.39

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=254, 13=360.
8) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST - MICHAUD RES	
1767895	T09	Common	4	1	Job Reference (optional)	T17063452

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc Wed May 15 06:16:13 2019 Page 1
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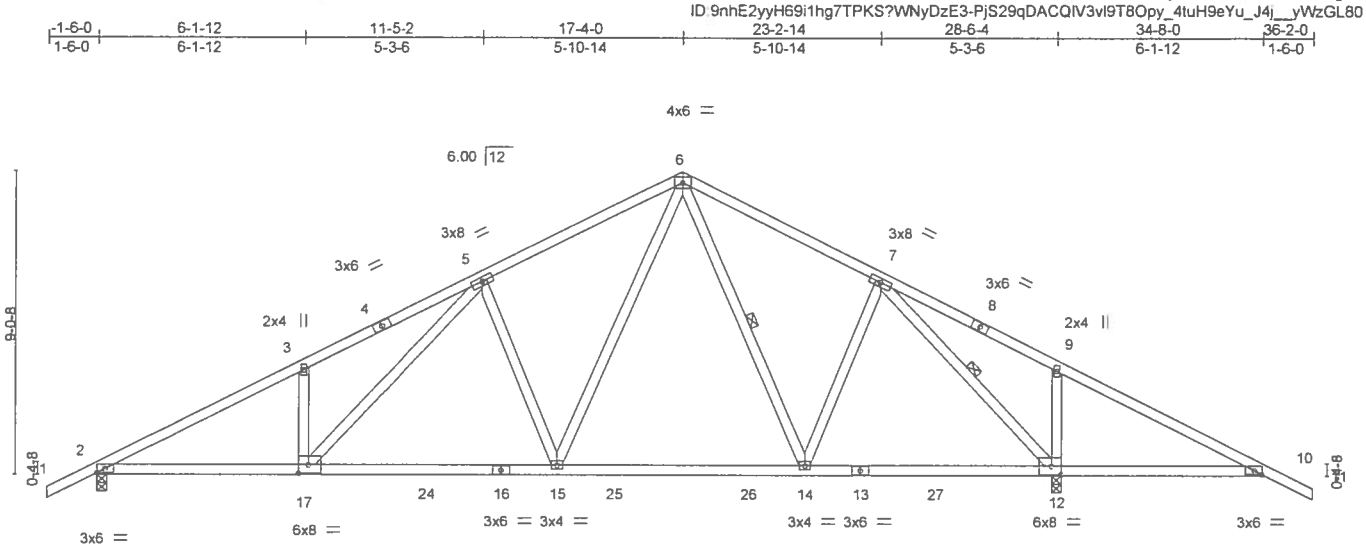


6-1-12	13-7-14	21-0-2	28-6-4	28-8-0	34-8-0
6-1-12	7-6-2	7-4-4	7-6-2	0-1-12	6-0-0

Plate Offsets (X,Y)— [10:0-2-15,Edge], [12:0-3-8,0-3-0], [17:0-3-8,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.48	Vert(LL)	-0.12	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.61	Vert(CT)	-0.21	15-17	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(CT)	0.04	12	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 190 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

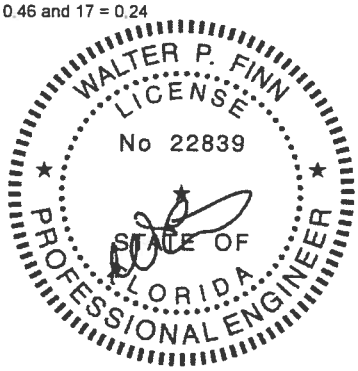
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 7-12

REACTIONS. (lb/size) 2=1070/0-3-8, 12=1658/0-3-8
Max Horz 2=-127(LC 10)
Max Uplift 2=-255(LC 12), 12=-362(LC 13)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1782, 3-4=-1778, 4-5=-1663, 5-6=-1138, 6-7=-782, 7-8=646, 8-9=532, 9-10=666, 10-11=36
BOT CHORD 2-17=1536, 17-24=1131, 16-24=1131, 15-16=1131, 15-25=688, 25-26=688, 14-26=688, 13-14=528, 13-27=528, 12-27=528, 10-12=-524
WEBS 6-14=-143, 7-14=291, 7-12=-1560, 9-12=-330, 6-15=675, 5-15=-502, 5-17=600, 3-17=-296

JOINT STRESS INDEX
2 = 0.45, 3 = 0.27, 4 = 0.37, 5 = 0.59, 6 = 0.48, 7 = 0.59, 8 = 0.37, 9 = 0.27, 10 = 0.45, 12 = 0.24, 13 = 0.46, 14 = 0.51, 15 = 0.51, 16 = 0.46 and 17 = 0.24

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=255, 12=362.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



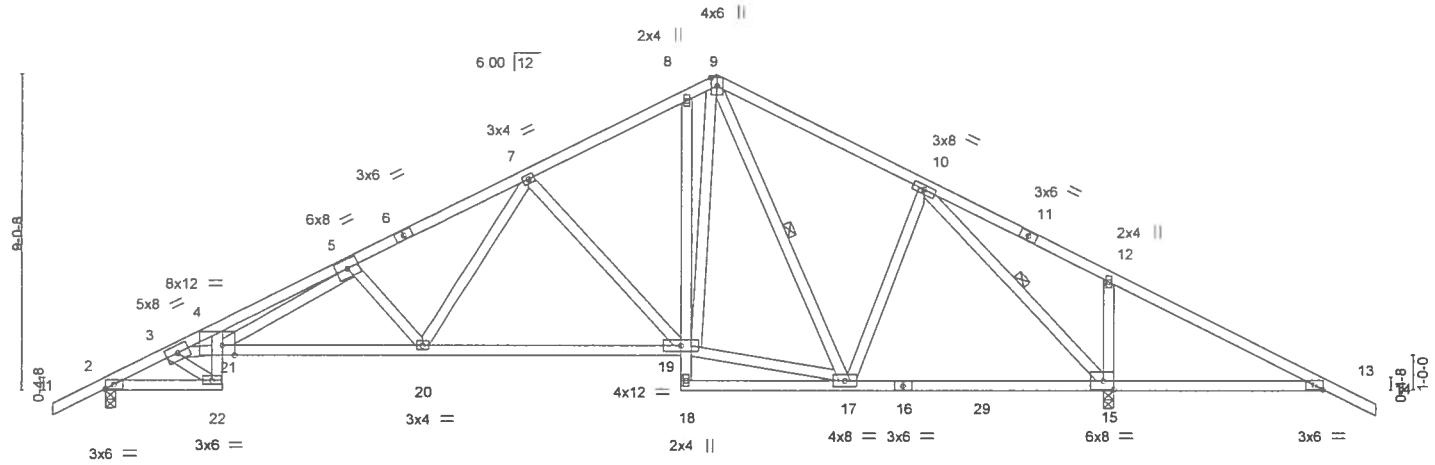
Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST - MICHAUD RES	
1767895	T10	Roof Special	2	1	Job Reference (optional)	T17063453

Builders FirstSource, Jacksonville, FL - 32244, 8 240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:15 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-L6ZoaVEQk10DIDuXaZQH1P9984mY0mjcX1T41OzGL8_

-1-6-0	1-10-11	3-3-8	7-0-0	11-11-13	16-4-0	17-4-0	23-2-14	28-6-4	34-8-0	36-2-0
1-6-0	1-10-11	1-4-13	3-8-8	4-11-13	4-4-3	1-0-0	5-10-14	5-3-6	6-1-12	1-6-0



3-3-8	9-0-0	16-4-0	21-0-2	28-6-4	28-8-0	34-8-0
3-3-8	5-8-8	7-4-0	4-8-2	7-6-2	0-1-12	6-0-0

Plate Offsets (X,Y)-- [4:0-1-12,0-0-14], [4:0-4-6,0-3-7], [13:0-2-15,Edge], [15:0-3-8,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.75	Vert(LL)	-0.22	20-21	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.91	Vert(CT)	-0.43	20-21	>800	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.19	15	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 216 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
8-18: 2x4 SP No.3
WEBS 2x4 SP No.3 *Except*
3-21: 2x4 SP No.2

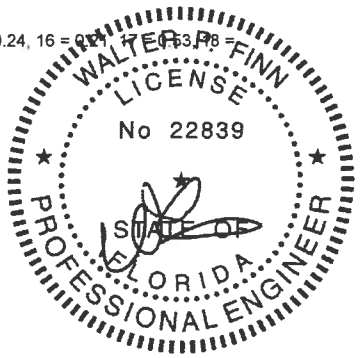
BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 9-17, 10-15

REACTIONS. (lb/size) 2=1070/0-3-8, 15=1658/0-3-8
Max Horz 2=127(LC 11)
Max Uplift 2=-255(LC 12), 15=-362(LC 13)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1850, 3-4=-3734, 4-5=-4153, 5-6=-1923, 6-7=-1857, 7-8=-1019, 8-9=-918, 9-10=-776, 10-11=645, 11-12=531, 12-13=665, 13-14=36
BOT CHORD 2-22=1595, 21-22=841, 4-21=-88, 20-21=2035, 19-20=1290, 18-19=38, 8-19=-37, 17-18=21, 16-17=527, 16-29=527, 15-29=527, 13-15=-523
WEBS 3-22=-1425, 3-21=2870, 5-21=2022, 5-20=-581, 7-20=729, 7-19=-652, 17-19=758, 9-19=791, 9-17=-341, 10-17=289, 10-15=-1556, 12-15=-329

JOINT STRESS INDEX
2 = 0.47, 3 = 0.67, 4 = 0.13, 4 = 0.66, 5 = 0.49, 6 = 0.35, 7 = 0.56, 8 = 0.28, 9 = 0.35, 10 = 0.59, 11 = 0.21, 12 = 0.27, 13 = 0.35, 15 = 0.24, 16 = 0.21, 17 = 0.63, 18 = 0.27, 19 = 0.49, 20 = 0.49, 21 = 0.66 and 22 = 0.64

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=255, 15=362.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T11	HALF HIP GIRDER	1	2	Job Reference (optional)	T17063454

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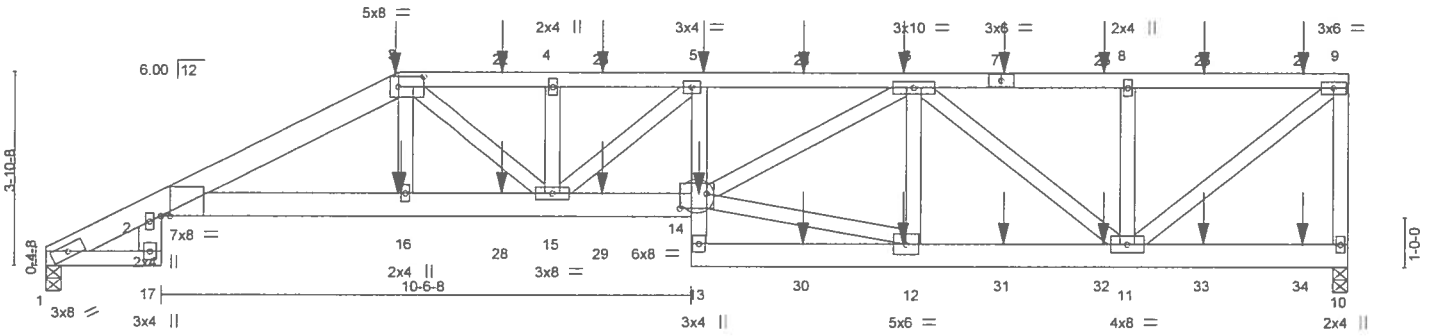
2-3-8	7-0-0	10-0-12	12-10-0	17-3-1	21-6-7	25-11-8
2-3-8	4-8-8	3-0-12	2-9-4	4-5-1	4-3-5	4-5-1



2-3-8	7-0-0	10-0-12	12-10-0	17-3-1	21-6-7	25-11-8
2-3-8	4-8-8	3-0-12	2-9-4	4-5-1	4-3-5	4-5-1

Plate Offsets (X,Y)-- [2:0-2-3,0-0-0], [3:0-6-0,0-2-8], [14:0-6-4,0-3-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.64	Vert(LL)	0.20	2-16	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.74	Vert(CT)	-0.36	14-15	>860	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.59	Horz(CT)	0.20	10	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 340 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2 *Except*
1-3: 2x6 SP M 26
BOT CHORD 2x6 SP No.2 *Except*
1-17: 2x4 SP No.2, 2-14: 2x6 SP M 26, 5-13: 2x4 SP No.3
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-1-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

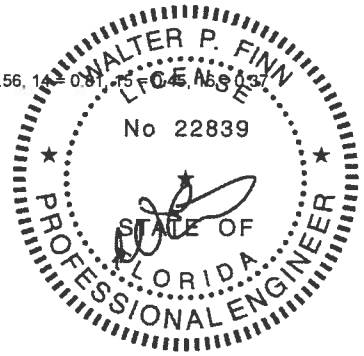
REACTIONS. (lb/size) 1=1847/0-3-8, 10=2126/0-3-8
Max Horz 1=120(LC 8)
Max Uplift 1=-607(LC 8), 10=-667(LC 5)

FORCES. (lb) - First Load Case Only
TOP CHORD 2-20=-1332, 2-3=-5095, 3-22=-5128, 4-22=-5128, 4-23=-5128, 5-23=-5128, 5-24=-5567, 6-24=-5567, 6-7=-2196, 7-25=-2196, 8-25=-2196, 8-26=-2196, 26-27=-2196, 9-27=-2196, 9-10=-1995
BOT CHORD 1-17=295, 2-17=665, 2-16=4614, 16-28=4690, 15-28=4690, 15-29=5622, 14-29=5627, 13-14=108, 5-14=35, 13-30=485, 12-30=485, 12-31=3497, 31-32=3497, 11-32=3497, 11-33=35, 33-34=35, 10-34=35
WEBS 3-16=1303, 3-15=566, 4-15=-228, 5-15=-651, 12-14=3111, 6-14=2373, 6-12=-586, 6-11=-1680, 8-11=-492, 9-11=2793

JOINT STRESS INDEX
1 = 0.54, 2 = 0.79, 2 = 0.27, 3 = 0.41, 4 = 0.27, 5 = 0.37, 6 = 0.67, 7 = 0.16, 8 = 0.27, 9 = 0.65, 10 = 0.38, 11 = 0.69, 12 = 0.43, 13 = 0.56, 14 = 0.81, 15 = 0.45, 16 = 0.37 and 17 = 0.63

NOTES-
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=607, 10=667.

Continued on page 2



Walter P. Finn PE No.22839
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6904 Parke East Blvd. Tampa FL 33610
Date: May 15,2019

NOTES-

10) This truss design conforms with Florida Building Code 2001, based on parameters indicated.

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 116 lb down and 88 lb up at 7-0-0, 97 lb down and 88 lb up at 9-0-12, 97 lb down and 88 lb up at 11-0-12, 110 lb down and 102 lb up at 13-0-12, 110 lb down and 102 lb up at 15-0-12, 110 lb down and 102 lb up at 17-0-12, 110 lb down and 102 lb up at 19-0-12, 110 lb down and 102 lb up at 21-0-12, and 110 lb down and 102 lb up at 23-0-12, and 116 lb down and 101 lb up at 25-0-12 on top chord, and 402 lb down and 277 lb up at 7-0-0, 78 lb down and 32 lb up at 9-0-12, 78 lb down and 32 lb up at 11-0-12, 86 lb down at 12-11-12, 86 lb down at 15-0-12, 86 lb down at 17-0-12, 86 lb down at 19-0-12, 86 lb down at 21-0-12, and 86 lb down at 23-0-12, and 91 lb down at 25-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 2-20=-54, 2-3=-54, 3-9=-54, 1-17=-20, 2-14=-20, 10-13=-20

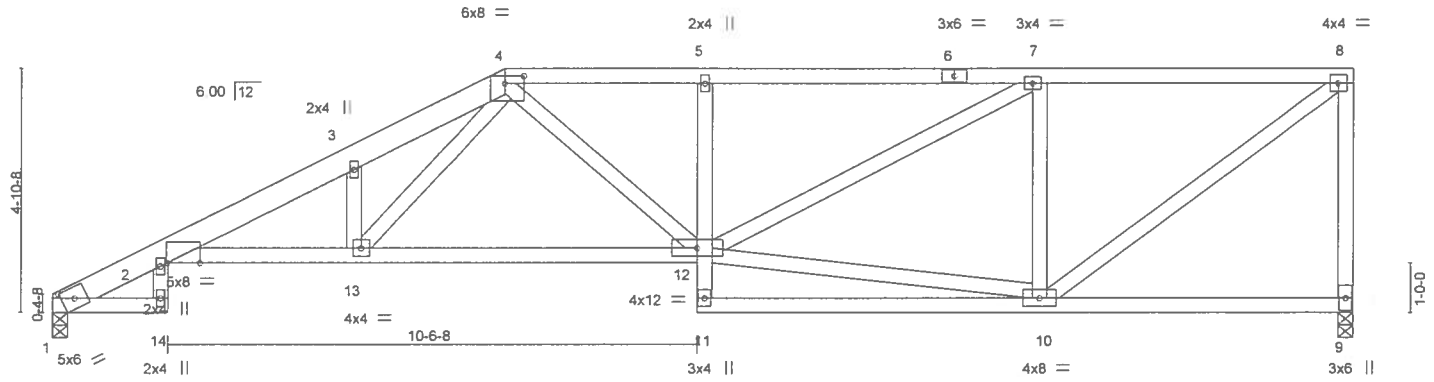
Concentrated Loads (lb)

Vert: 3=-97(B) 7=-110(B) 5=-110(B) 16=-402(B) 12=-64(B) 14=-64(B) 6=-110(B) 22=-97(B) 23=-97(B) 24=-110(B) 25=-110(B) 26=-110(B) 27=-116(B) 28=-77(B) 29=-77(B) 30=-64(B) 31=-64(B) 32=-64(B) 33=-64(B) 34=-67(B)

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	T17063455
1767895	T12	Half Hip	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:20 2019 Page 1
ID:9nhE2yyH69i1hg7TPKS?WNyDzE3-i4NhdDIZZaeWO_nVN60SkSt4f5ZDh30LhJArhczGL7v

2-3-8	6-0-0	9-0-0	12-10-0	19-8-0	25-11-8
2-3-8	3-8-8	3-0-0	3-10-0	6-10-0	6-3-8



2-3-8	6-0-0	12-10-0	19-8-0	25-11-8
2-3-8	3-8-8	6-10-0	6-10-0	6-3-8

Plate Offsets (X,Y)-- [1:0-3-0,0-2-9], [2:0-7-12,0-0-0], [4:0-4-8,0-1-12]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.51	Vert(LL)	0.20	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.55	Vert(CT)	-0.35	2-13	>885	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.19	9	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 155 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2 *Except*
1-4: 2x6 SP M 26
BOT CHORD 2x4 SP No.2 *Except*
2-14,5-11: 2x4 SP No.3, 2-12: 2x4 SP M 31
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-1-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-4-7 oc bracing: 2-13
8-4-1 oc bracing: 12-13.

REACTIONS. (lb/size) 1=954/0-3-8, 9=950/0-3-8
Max Horz 1=154(LC 12)
Max Uplift 1=-169(LC 9), 9=-245(LC 9)

FORCES. (lb) - First Load Case Only
TOP CHORD 2-17=-533, 2-3=-2509, 3-4=-2745, 4-5=-1739, 5-6=-1728, 6-7=-1728, 7-8=-1016, 8-9=-898
BOT CHORD 1-14=43, 2-14=234, 2-13=2357, 12-13=1532, 11-12=64, 5-12=-303, 10-11=100, 9-10=17
WEBS 3-13=-731, 4-13=1284, 4-12=279, 10-12=946, 7-12=785, 7-10=-757, 8-10=1254

JOINT STRESS INDEX
1 = 0.41, 2 = 0.58, 2 = 0.27, 3 = 0.28, 4 = 0.60, 5 = 0.27, 6 = 0.35, 7 = 0.48, 8 = 0.56, 9 = 0.29, 10 = 0.62, 11 = 0.59, 12 = 0.57, 13 = 0.59 and 14 = 0.80

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=169, 9=245.
8) This truss design conforms with Florida Building Code 2001, based on parameters indicated.

Walter P. Finn PE No.22839
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Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T13	Half Hip	1	1	Job Reference (optional)	T17063456

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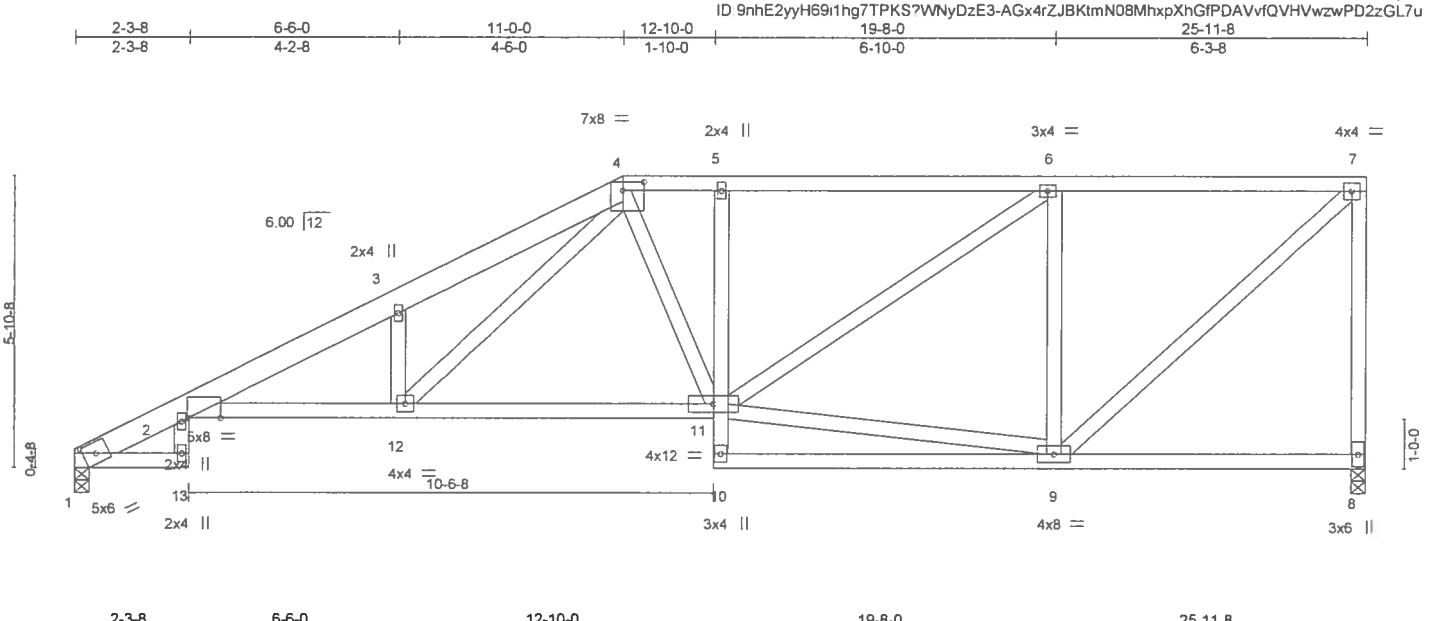


Plate Offsets (X,Y)-- [1:0-3-0,0-2-9], [2:0-7-12,0-0-0], [4:0-5-0,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.59	Vert(LL)	0.22	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.54	Vert(CT)	-0.38	2-12	>821	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.20	8	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 166 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP M 26 *Except*
4-7: 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
2-13,5-10: 2x4 SP No.3, 2-11: 2x4 SP M 31
WEBS 2x4 SP No.3

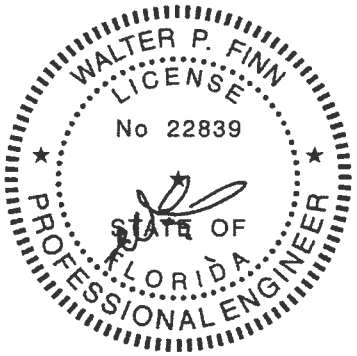
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-8-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-4-3 oc bracing: 2-12
8-11-7 oc bracing: 11-12.

REACTIONS. (lb/size) 1=954/0-3-8, 8=950/0-3-8
Max Horz 1=188(LC 12)
Max Uplift 1=-175(LC 12), 8=-243(LC 9)

FORCES. (lb) - First Load Case Only
TOP CHORD 2-16=-533, 2-3=-2379, 3-4=-2596, 4-5=-1358, 5-6=-1356, 6-7=-837, 7-8=-899
BOT CHORD 1-13=43, 2-13=234, 2-12=2224, 11-12=1288, 10-11=63, 5-11=-284, 9-10=74, 8-9=13
WEBS 3-12=-716, 4-12=1335, 4-11=177, 9-11=785, 6-11=612, 6-9=-733, 7-9=1122

JOINT STRESS INDEX
1 = 0.41, 2 = 0.56, 2 = 0.27, 3 = 0.28, 4 = 0.51, 5 = 0.27, 6 = 0.38, 7 = 0.51, 8 = 0.26, 9 = 0.56, 10 = 0.57, 11 = 0.47, 12 = 0.61 and 13 = 0.80

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=175, 8=243.
8) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T14	Hip	1	1	Job Reference (optional)	T17063457

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:22 2019 Page 1
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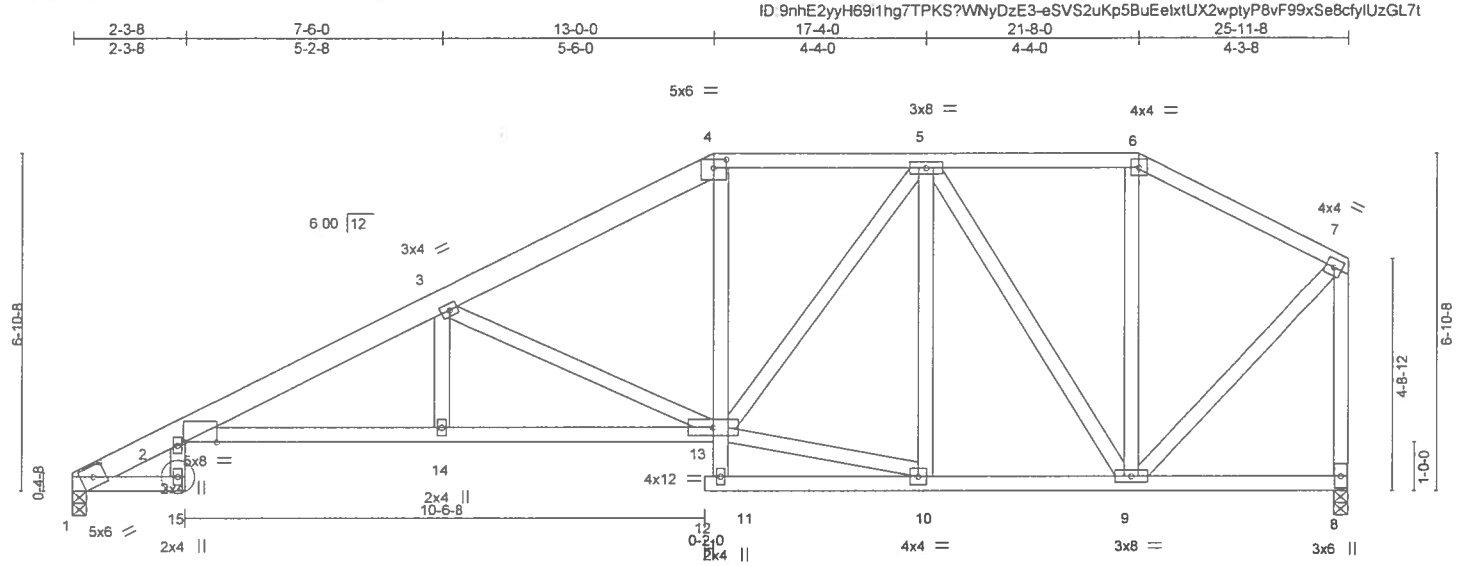


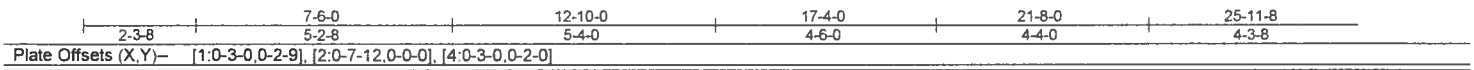


Plate Offsets (X,Y)— [1:0-3-0,0-2-9], [2:0-7-12,0-0-0], [4:0-3-0,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.51	Vert(LL)	0.24	2-14	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.52	Vert(CT)	-0.43	2-14	>720	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.22	8	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 176 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2 *Except*
1-4: 2x6 SP M 26
BOT CHORD 2x4 SP No.2 *Except*
2-15,4-11: 2x4 SP No.3, 2-13: 2x4 SP M 31
WEBS 2x4 SP No.3

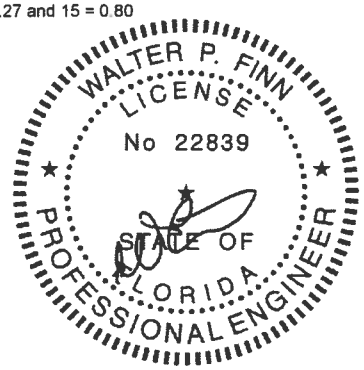
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-8 oc bracing. Except: 10-0-0 oc bracing: 11-13

REACTIONS. (lb/size) 1=957/0-3-8, 8=953/0-3-8
Max Horz 1=176(LC 12)
Max Uplift 1=-189(LC 12), 8=-148(LC 12)

FORCES. (lb) - First Load Case Only
TOP CHORD 2-18=-535, 2-3=-2205, 3-4=-1337, 4-5=-1110, 5-6=-528, 6-7=-638, 7-8=-913
BOT CHORD 1-15=44, 2-15=235, 2-14=2042, 13-14=2042, 11-13=51, 4-13=367, 11-12=0, 10-11=62, 9-10=842, 8-9=10
WEBS 3-14=226, 3-13=-1043, 10-13=802, 5-13=444, 5-9=-585, 6-9=43, 7-9=748, 5-10=-108

JOINT STRESS INDEX
1 = 0.41, 2 = 0.53, 2 = 0.27, 3 = 0.46, 4 = 0.26, 5 = 0.46, 6 = 0.43, 7 = 0.49, 8 = 0.19, 9 = 0.58, 10 = 0.37, 11 = 0.59, 13 = 0.53, 14 = 0.27 and 15 = 0.80

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=189, 8=148.
8) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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MiTek USA, Inc. FL Cert 6634
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Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST - MICHAUD RES	T17063458
1767895	T15	Hip	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:23 2019 Page 1
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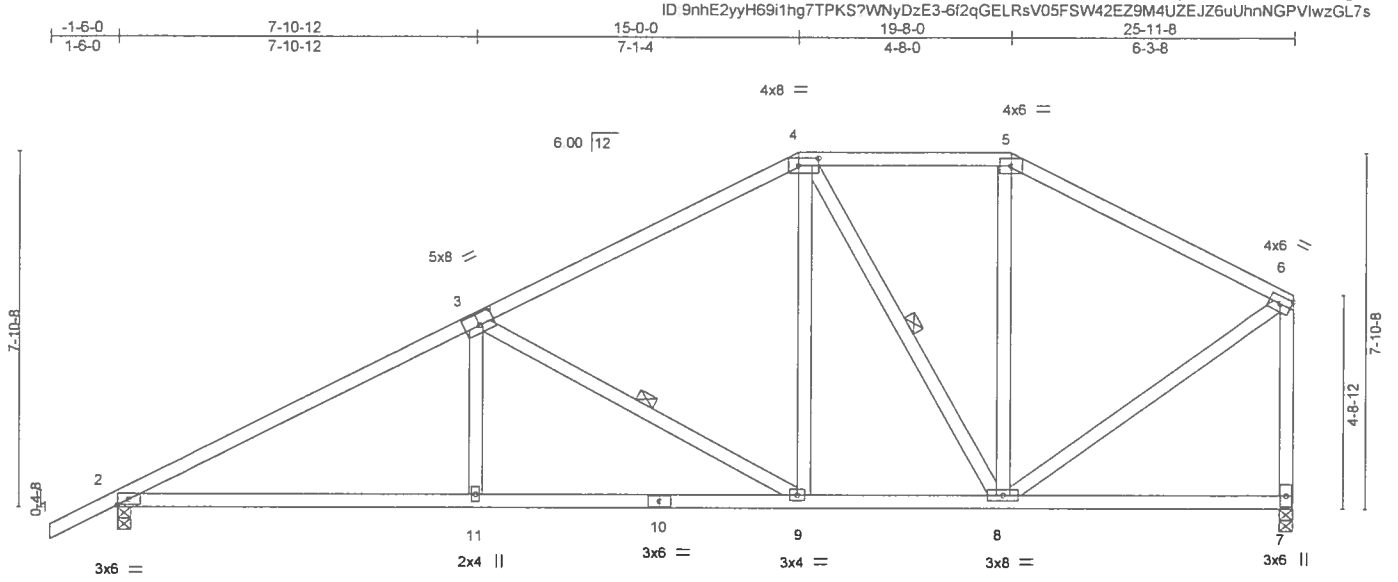


Plate Offsets (X,Y)— [3:0-4-0,0-3-0], [4:0-5-4,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.62	Vert(LL)	0.10	11-14	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.67	Vert(CT)	-0.20	11-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.39	Horz(CT)	0.04	7	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 153 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

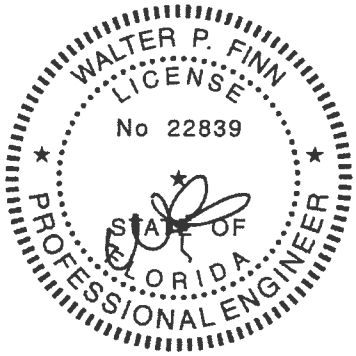
BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-9-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-5 oc bracing.
WEBS 1 Row at midpt 3-9, 4-8

REACTIONS. (lb/size) 2=1038/0-3-8, 7=953/0-3-8
Max Horz 2=209(LC 12)
Max Uplift 2=-230(LC 12), 7=-166(LC 12)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1643, 3-4=-993, 4-5=-615, 5-6=-761, 6-7=-893
BOT CHORD 2-11=1400, 10-11=1400, 9-10=1400, 8-9=807, 7-8=24
WEBS 3-11=184, 3-9=-688, 4-9=436, 4-8=-373, 5-8=12, 6-8=726

JOINT STRESS INDEX
2 = 0.41, 3 = 0.60, 4 = 0.55, 5 = 0.57, 6 = 0.52, 7 = 0.41, 8 = 0.54, 9 = 0.38, 10 = 0.62 and 11 = 0.27

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=230, 7=166.
8) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
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Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	T17063459
1767895	T16	Hip	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244, 8 240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:25 2019 Page 1
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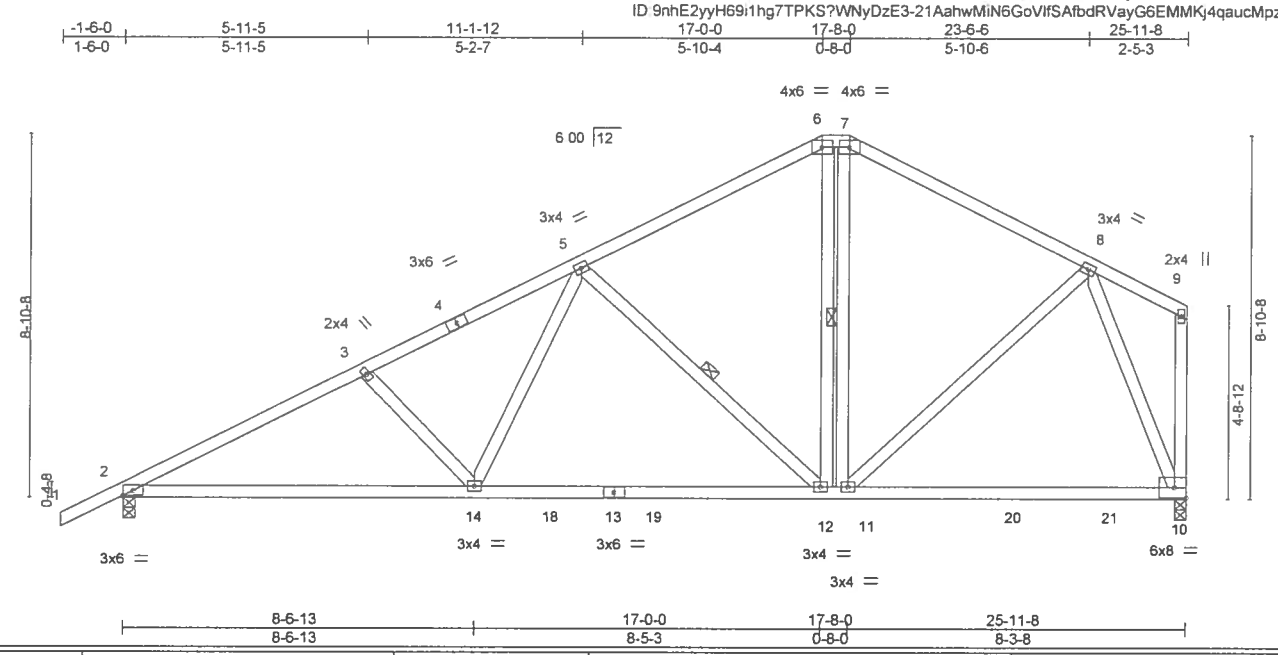


LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.39	Vert(LL) -0.15 12-14 >999 240	MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.75	Vert(CT) -0.27 12-14 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.67	Horz(CT) 0.04 10 n/a n/a	
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MS		Weight: 162 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

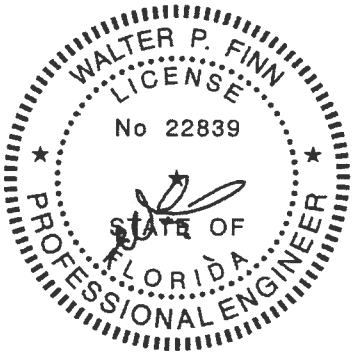
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-1-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-0 oc bracing.
WEBS 1 Row at midpt 5-12, 7-11

REACTIONS. (lb/size) 2=1038/0-3-8, 10=953/0-3-8
Max Horz 2=221(LC 12)
Max Uplift 2=-235(LC 12), 10=-185(LC 12)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1688, 3-4=-1482, 4-5=-1366, 5-6=-807, 6-7=-655, 7-8=-805, 8-9=43, 9-10=-9
BOT CHORD 2-14=1459, 14-18=1089, 13-18=1089, 13-19=1089, 12-19=1089, 11-12=655, 11-20=364, 20-21=364, 10-21=364
WEBS 3-14=-281, 5-14=436, 5-12=-600, 6-12=207, 7-11=119, 8-11=395, 8-10=-947

JOINT STRESS INDEX
2 = 0.48, 3 = 0.27, 4 = 0.29, 5 = 0.46, 6 = 0.36, 7 = 0.50, 8 = 0.57, 9 = 0.27, 10 = 0.26, 11 = 0.37, 12 = 0.37, 13 = 0.82 and 14 = 0.38

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=235, 10=185.
8) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES.	
1767895	T17	Common	2	1	Job Reference (optional)	T17063460

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:26 2019 Page 1
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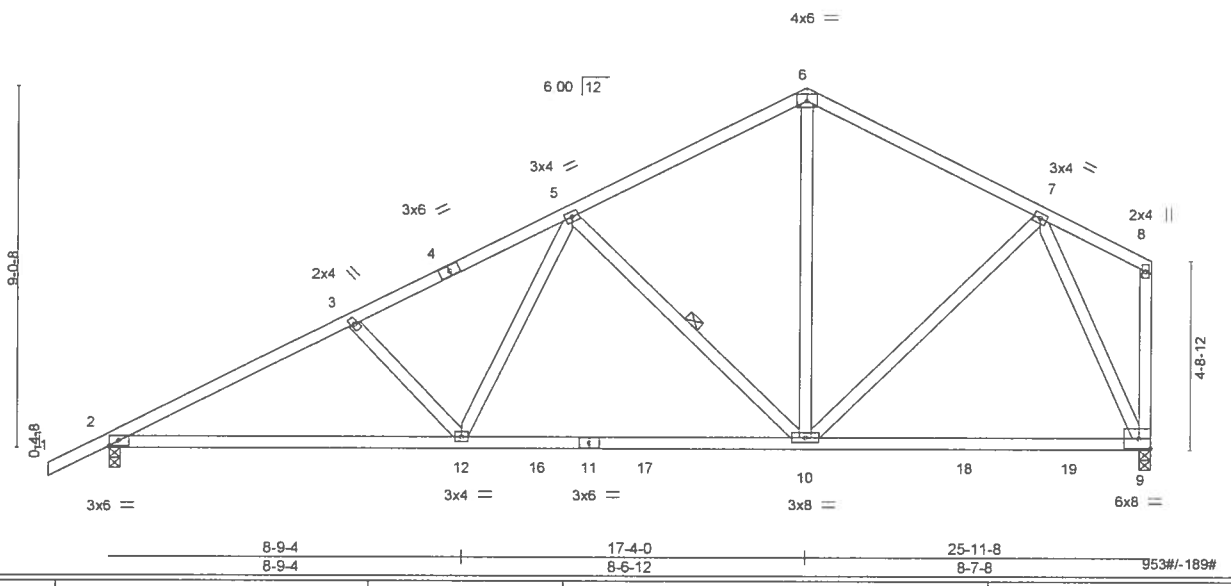


LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.39	Vert(LL)	-0.13	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.74	Vert(CT)	-0.27	9-10	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.73	Horz(CT)	0.04	9	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 151 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

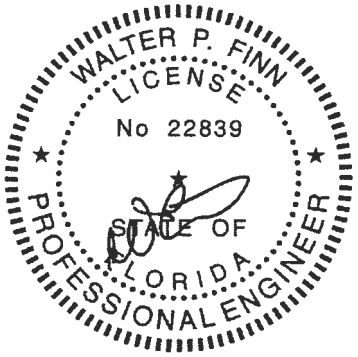
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-1-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-0 oc bracing.
WEBS 1 Row at midpt 5-10

REACTIONS. (lb/size) 2=1038/0-3-8, 9=953/0-3-8
Max Horz 2=223(LC 12)
Max Uplift 2=-236(LC 12), 9=-189(LC 12)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1664, 3-4=-1464, 4-5=-1345, 5-6=-798, 6-7=-796, 7-8=40, 8-9=-30
BOT CHORD 2-12=1449, 12-16=1063, 11-16=1063, 11-17=1063, 10-17=1063, 10-18=398, 18-19=398, 9-19=398
WEBS 3-12=-297, 5-12=441, 5-10=-588, 6-10=349, 7-10=351, 7-9=-932

JOINT STRESS INDEX
2 = 0.53, 3 = 0.27, 4 = 0.34, 5 = 0.46, 6 = 0.32, 7 = 0.58, 8 = 0.27, 9 = 0.29, 10 = 0.45, 11 = 0.71 and 12 = 0.38

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=236, 9=189.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T18	Roof Special	2	1	Job Reference (optional)	T17063461

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:27 2019 Page 1
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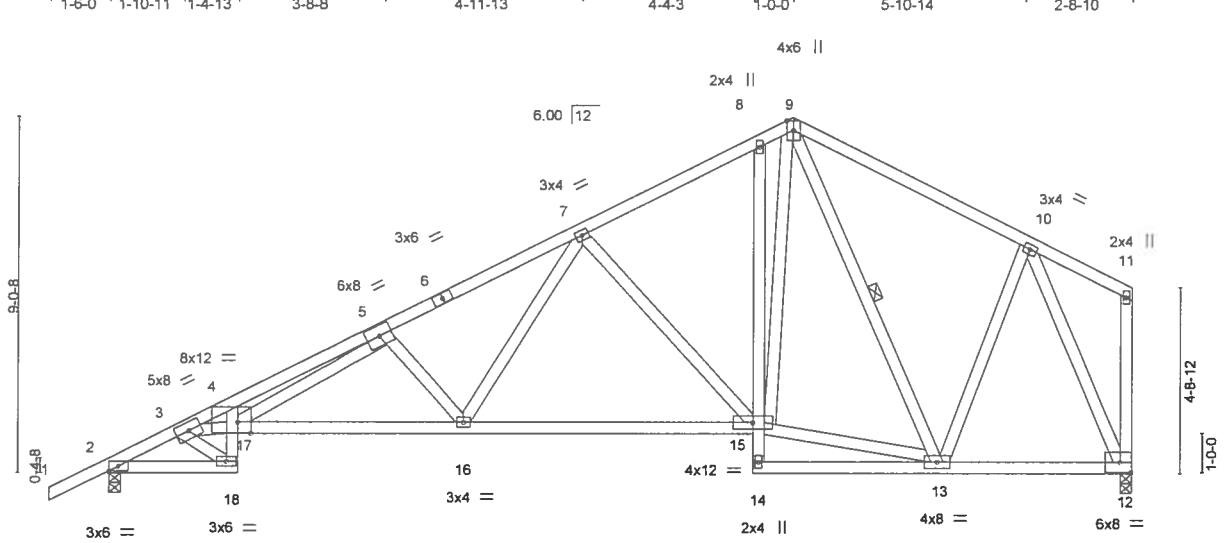


Plate Offsets (X,Y)— [4:0-1-12,0-0-14], [4:0-4-1,0-3-7]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.72	Vert(LL)	0.22	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.87	Vert(CT)	-0.40	16-17	>775	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.75	Horz(CT)	0.18	12	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 186 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
8-14: 2x4 SP No.3
WEBS 2x4 SP No.3 *Except*
3-17: 2x4 SP No.2

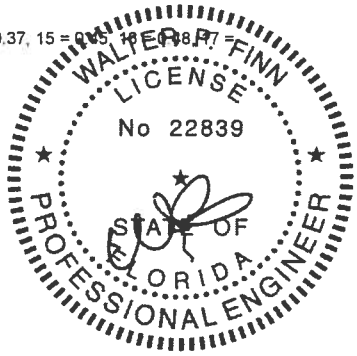
BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-3 oc bracing.
WEBS 1 Row at midpt 9-13

REACTIONS. (lb/size) 2=1038/0-3-8, 12=953/0-3-8
Max Horz 2=223(LC 12)
Max Uplift 2=-236(LC 12), 12=-189(LC 12)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-1785, 3-4=-3598, 4-5=-4002, 5-6=-1834, 6-7=-1768, 7-8=-940, 8-9=-839, 9-10=-667, 10-11=52, 11-12=-14
BOT CHORD 2-18=1538, 17-18=813, 4-17=-89, 16-17=1949, 15-16=1216, 14-15=41, 8-15=-38, 13-14=22, 12-13=397
WEBS 3-18=-1375, 3-17=2765, 5-17=1964, 5-16=-571, 7-16=718, 7-15=-646, 13-15=687, 9-15=775, 9-13=-417, 10-13=367, 10-12=-981

JOINT STRESS INDEX
2 = 0.45, 3 = 0.65, 4 = 0.12, 4 = 0.64, 5 = 0.48, 6 = 0.38, 7 = 0.55, 8 = 0.27, 9 = 0.36, 10 = 0.51, 11 = 0.27, 12 = 0.15, 13 = 0.48, 14 = 0.37, 15 = 0.45, 16 = 0.38, 17 = 0.64 and 18 = 0.61

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=236, 12=189.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST - MICHAUD RES	
1767895	T19	Common	4	1	Job Reference (optional)	T17063462

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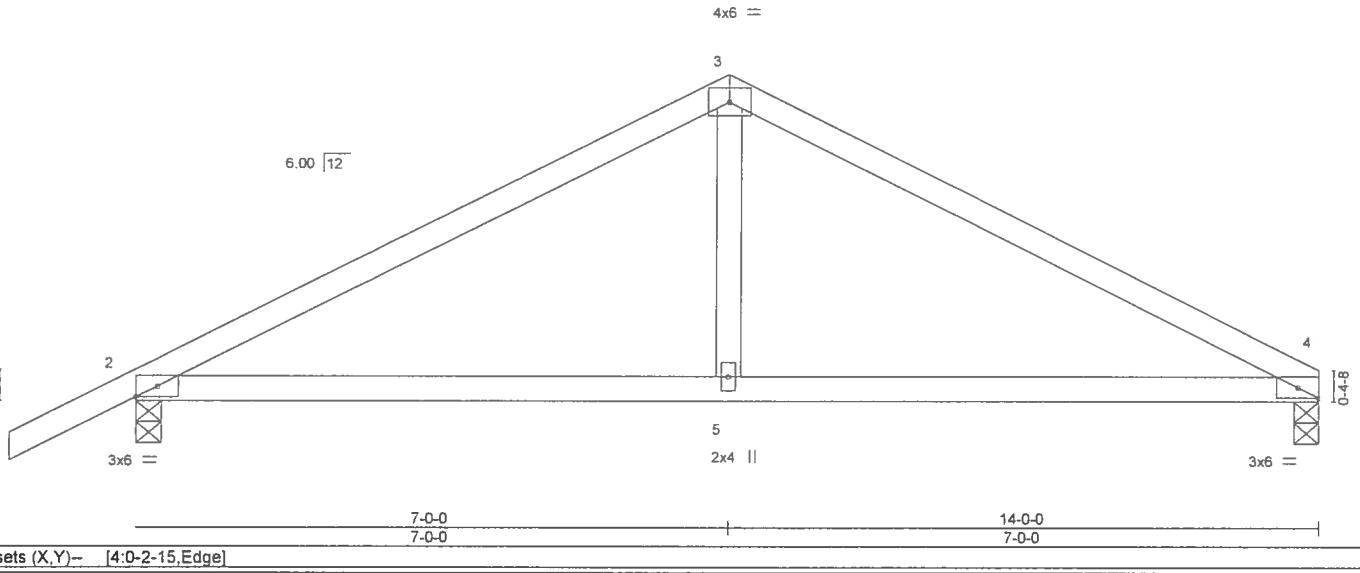


Plate Offsets (X,Y)-- [4:0-2-15,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	0.11	5-8	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.15	5-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.01	4	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 52 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

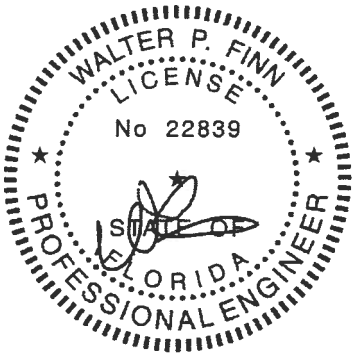
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-5-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=514/0-3-8, 2=603/0-3-8
Max Horz 2=105(LC 16)
Max Uplift 4=-192(LC 13), 2=-247(LC 12)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-732, 3-4=-730
BOT CHORD 2-5=587, 4-5=587
WEBS 3-5=218

JOINT STRESS INDEX
2 = 0.47, 3 = 0.50, 4 = 0.45 and 5 = 0.18

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=192, 2=247.
7) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Date:

May 15,2019

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T19G	Common Supported Gable	1	1	Job Reference (optional)	T17063463

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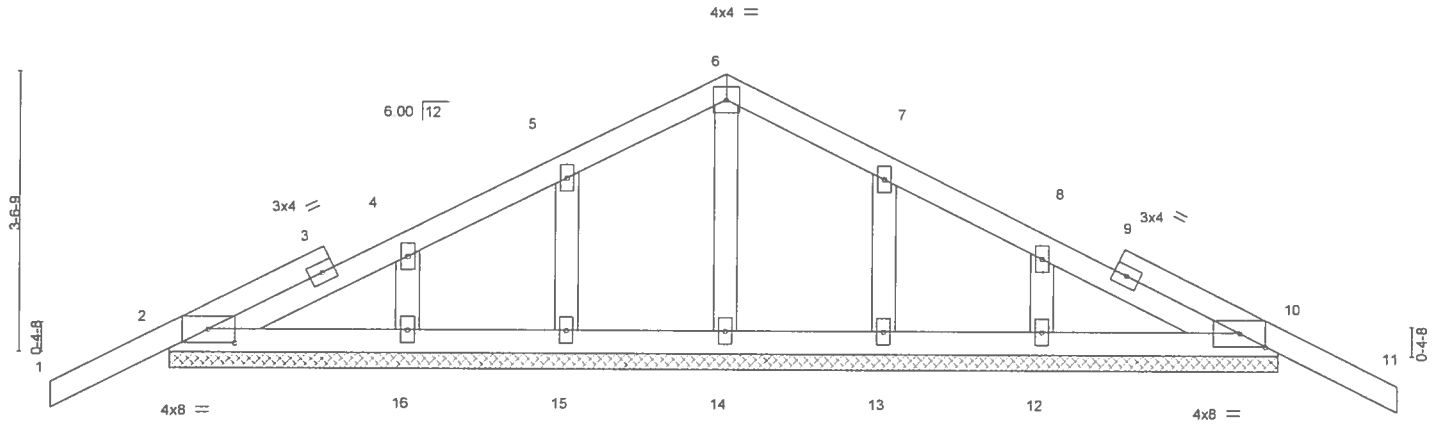
-1-6-0 | 7-0-0 | 14-0-0 | 15-6-0
1-6-0 | 7-0-0 | 7-0-0 | 1-6-0



14-0-0
14-0-0

Plate Offsets (X,Y)-- [2:0-4-0,0-2-1], [10:0-4-0,0-2-1]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	-0.00	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.06	Vert(CT)	-0.01	11	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	10	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S							Weight: 68 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

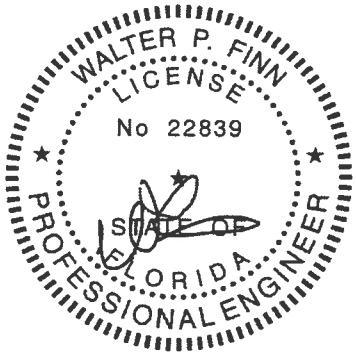
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-0-0.
(lb) - Max Horz 2=-82(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 2, 15, 16, 13 except 10=-104(LC 13), 12=-100(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 16, 13, 12

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=34, 2-3=-37, 3-4=27, 4-5=-30, 5-6=17, 6-7=-31, 7-8=-30, 8-9=27, 9-10=-37, 10-11=34
BOT CHORD 2-16=4, 15-16=4, 14-15=4, 13-14=4, 12-13=4, 10-12=4
WEBS 6-14=-95, 5-15=-111, 4-16=-127, 7-13=-111, 8-12=-127

JOINT STRESS INDEX
2 = 0.27; 3 = 0.00, 3 = 0.35, 4 = 0.07, 5 = 0.06, 6 = 0.10, 7 = 0.06, 8 = 0.07, 9 = 0.00, 9 = 0.35, 10 = 0.27, 12 = 0.08, 13 = 0.07, 14 = 0.03, 15 = 0.07 and 16 = 0.08

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 15, 16, 13 except (jt=lb) 10=104, 12=100.
11) This truss design conforms with Florida Building Code 2001, based on parameters indicated.



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Date:

May 15,2019

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	IC CONST. - MICHAUD RES	
1767895	T20	Common Girder	1	2	Job Reference (optional)	T17063464

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2018 MiTek Industries, Inc. Wed May 15 06:16:31 2019 Page 1
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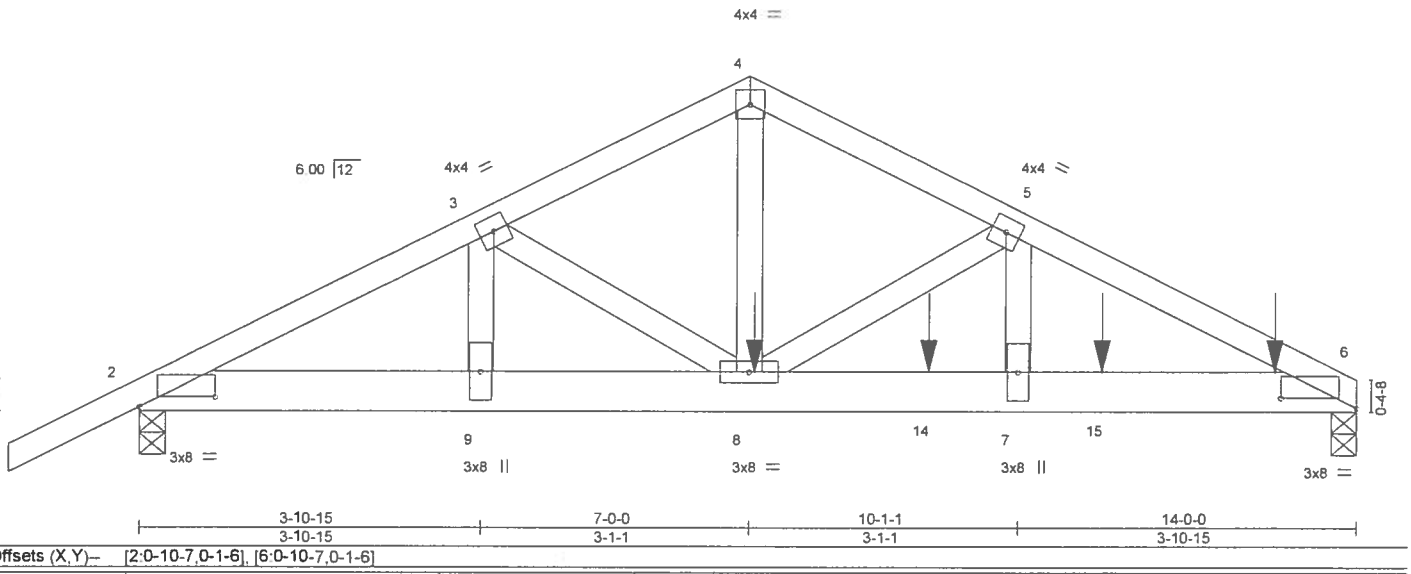


Plate Offsets (X,Y)-- [2:0-10-7,0-1-6], [6:0-10-7,0-1-6]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.34	Vert(LL)	-0.05	7-11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.77	Vert(CT)	-0.10	7-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.51	Horz(CT)	0.03	6	n/a	n/a		
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS							Weight: 156 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-2-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

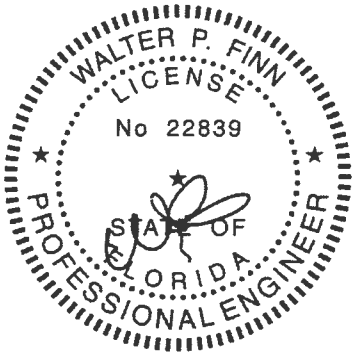
REACTIONS. (lb/size) 6=3366/0-3-8, 2=1757/0-3-8
Max Horz 2=64(LC 8)
Max Uplift 6=-710(LC 9), 2=-430(LC 8)

FORCES. (lb) - First Load Case Only
TOP CHORD 1-2=36, 2-3=-3376, 3-4=-3210, 4-5=-3222, 5-6=-5104
BOT CHORD 2-9=2991, 8-9=2991, 8-14=4565, 7-14=4565, 7-15=4565, 6-15=4565
WEBS 4-8=2671, 5-8=-2026, 5-7=1693, 3-8=-169, 3-9=8

JOINT STRESS INDEX
2 = 0.92, 3 = 0.54, 4 = 0.50, 5 = 0.54, 6 = 0.92, 7 = 0.31, 8 = 0.66 and 9 = 0.31

NOTES-
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=710, 2=430.
9) This truss design conforms with Florida Building Code 2001, based on parameters indicated.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1304 lb down and 388 lb up at 7-0-12, 635 lb down and 167 lb up at 9-0-12, and 1033 lb down and 203 lb up at 11-0-12, and 1034 lb down and 190 lb up at 13-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard



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May 15,2019

Continued on page 2

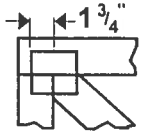
MiTek
6904 Parke East Blvd.
Tampa, FL 36610

LOAD CASE(S) Standard

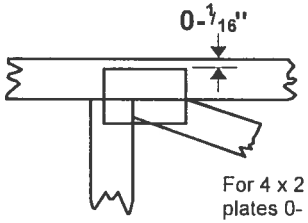
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-6=-20
   Concentrated Loads (lb)
   Vert: 8=-1304(B) 11=-1034(B) 14=-635(B) 15=-1033(B)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

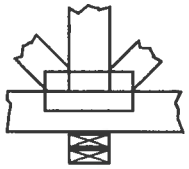
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

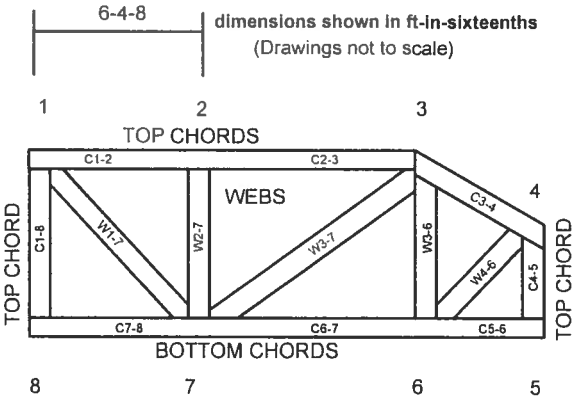


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek

MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

AUGUST 1, 2016	T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY	MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1

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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

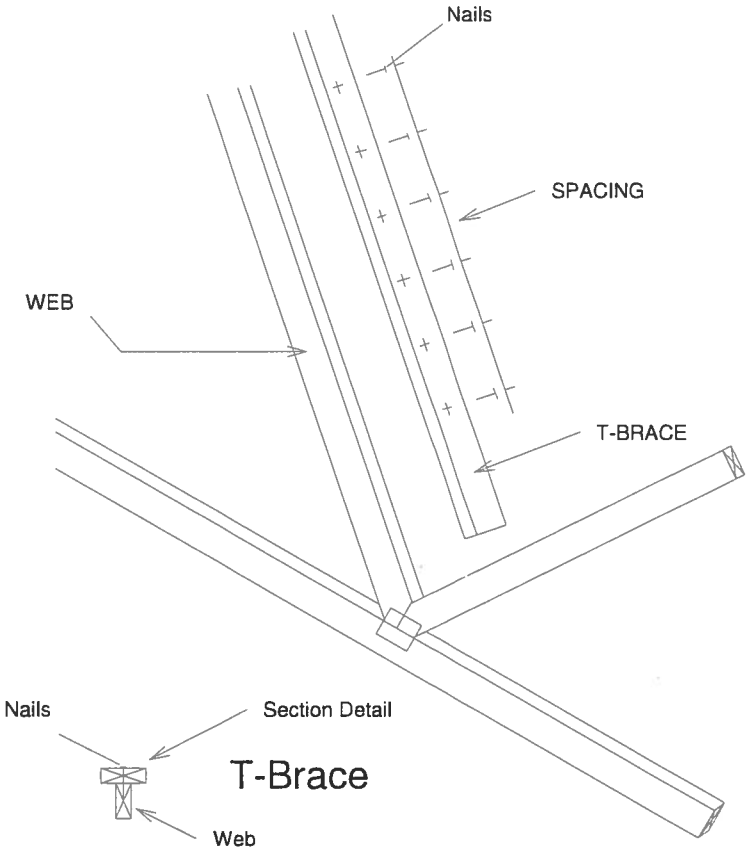
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

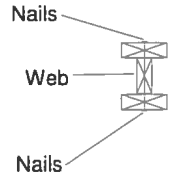
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

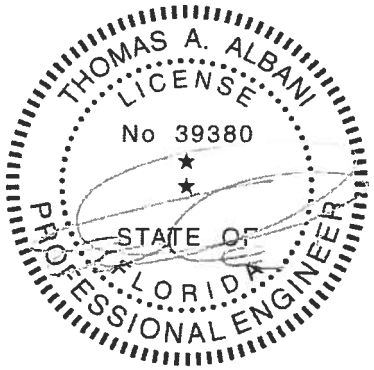
	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.





I-Brace



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February 12, 2018

AUGUST 1, 2016	SCAB-BRACE DETAIL	MII-SCAB-BRACE

MiTek USA, Inc.

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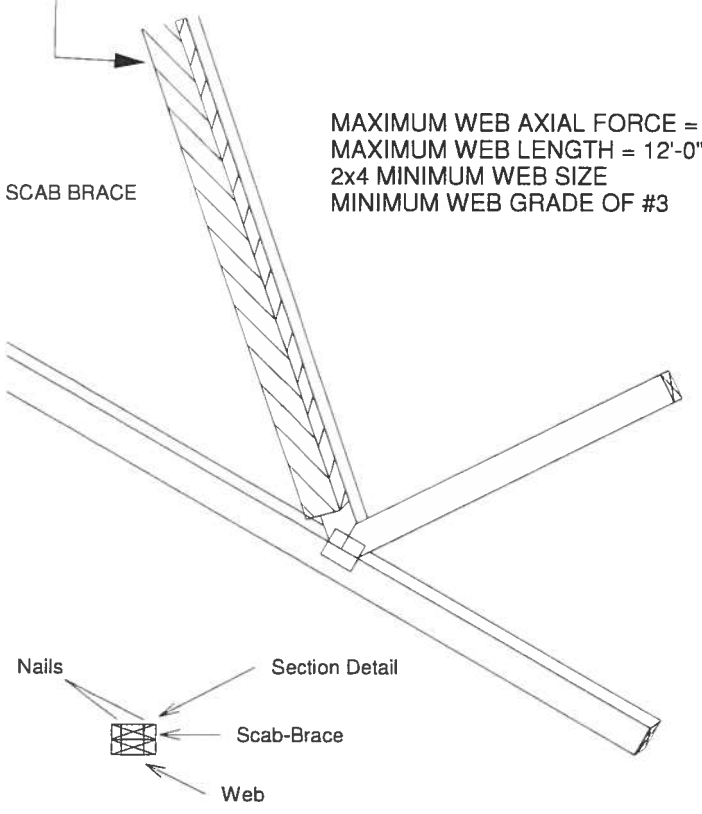
Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.
Scab must cover full length of web +/- 6".

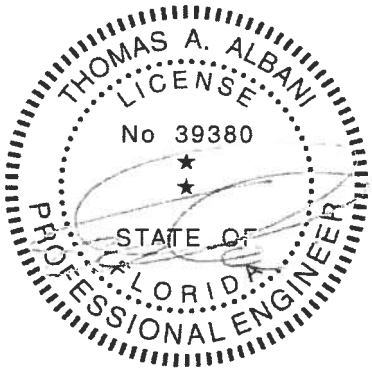
*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH
2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
SCAB MUST BE THE SAME GRADE, SIZE AND
SPECIES (OR BETTER) AS THE WEB.



MAXIMUM WEB AXIAL FORCE = 2500 lbs
MAXIMUM WEB LENGTH = 12'-0"
2x4 MINIMUM WEB SIZE
MINIMUM WEB GRADE OF #3

Scab-Brace must be same species grade (or better) as web member.



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AUGUST 1, 2016	STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)	MII-REP05

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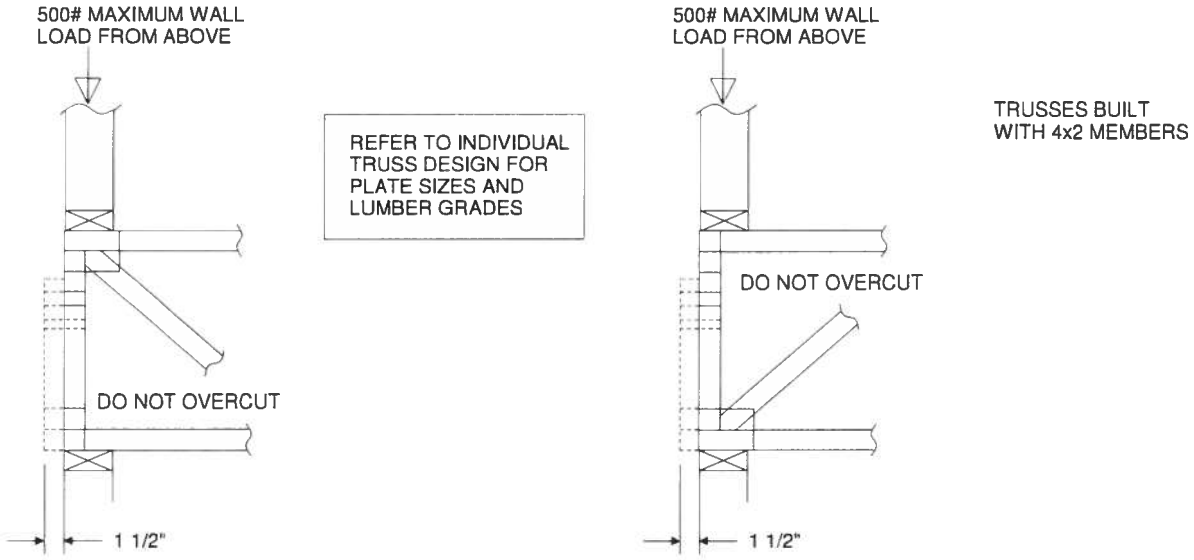
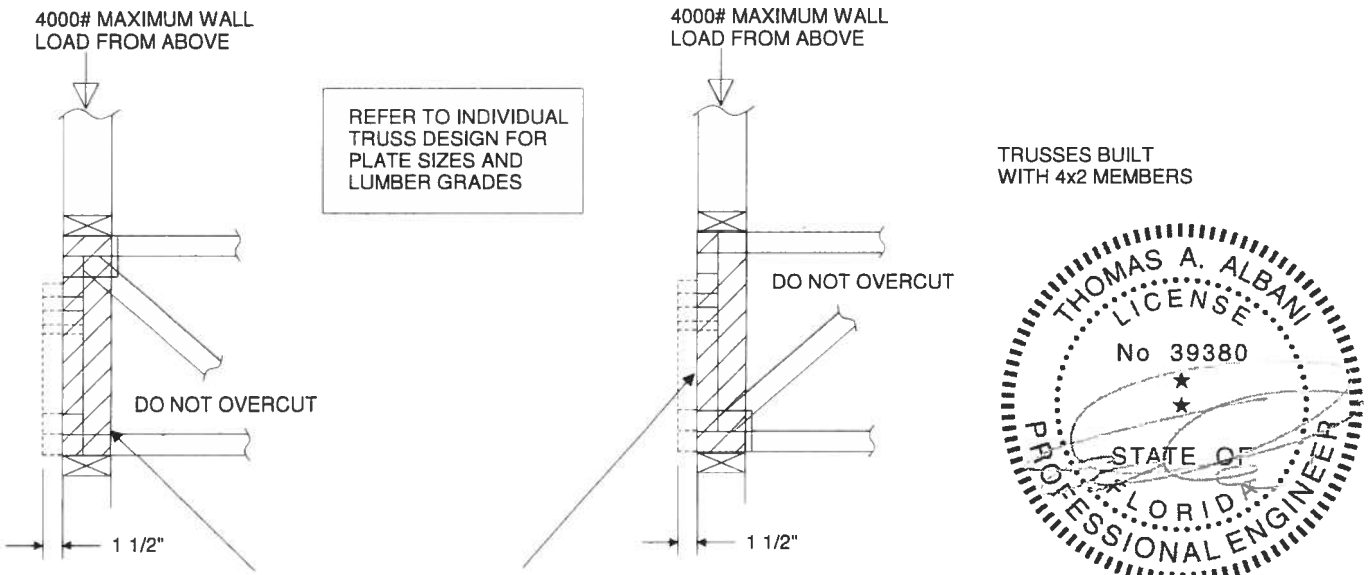
1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ ORIENTATION ONLY.
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





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AUGUST 1, 2016	Standard Gable End Detail	MII-GE130-D-SP

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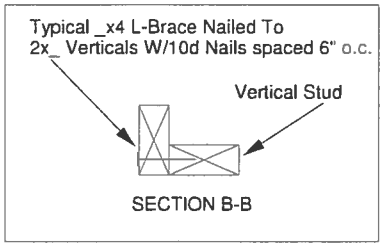
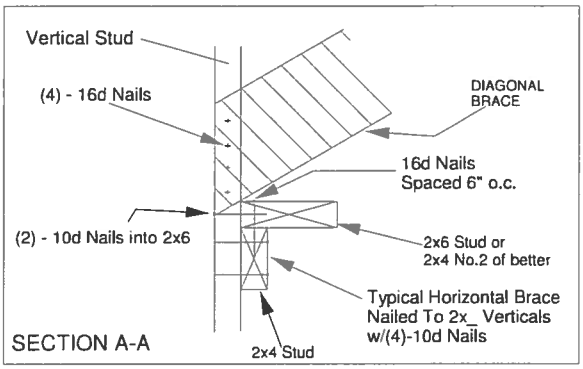
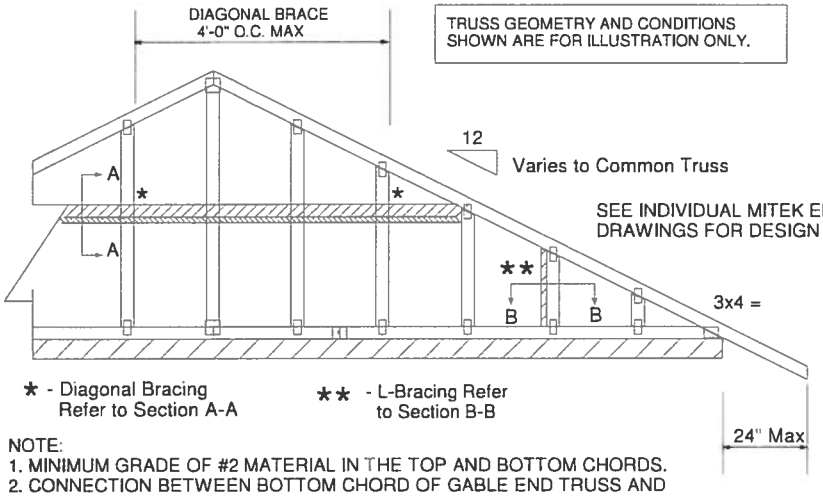


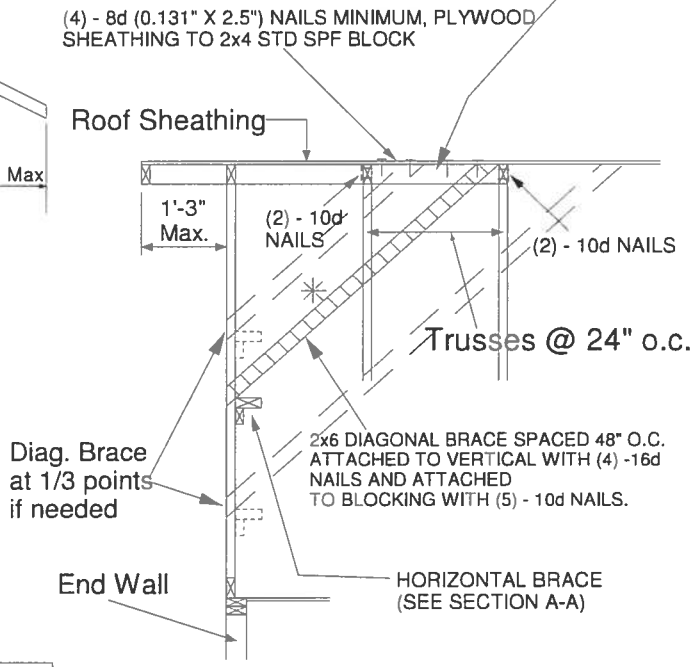




* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.



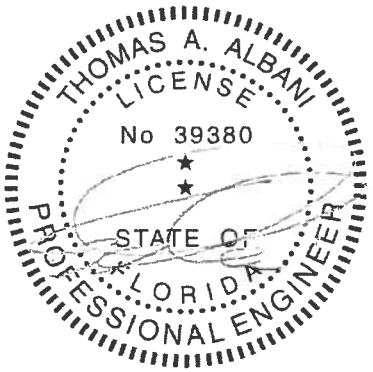
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE D
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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February 12, 2018

AUGUST 1, 2016	Standard Gable End Detail	MII-GE130-SP

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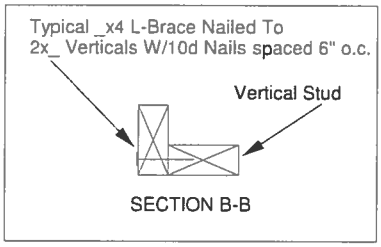
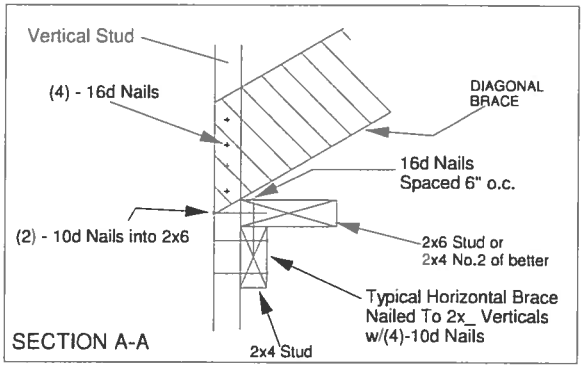
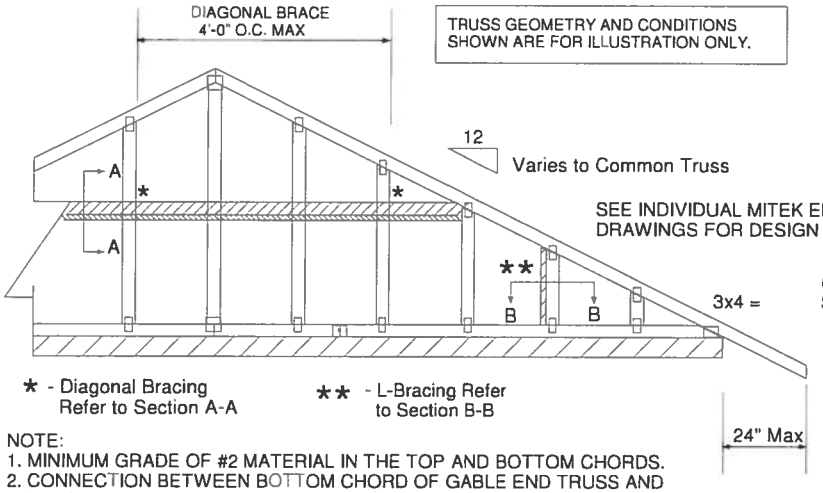


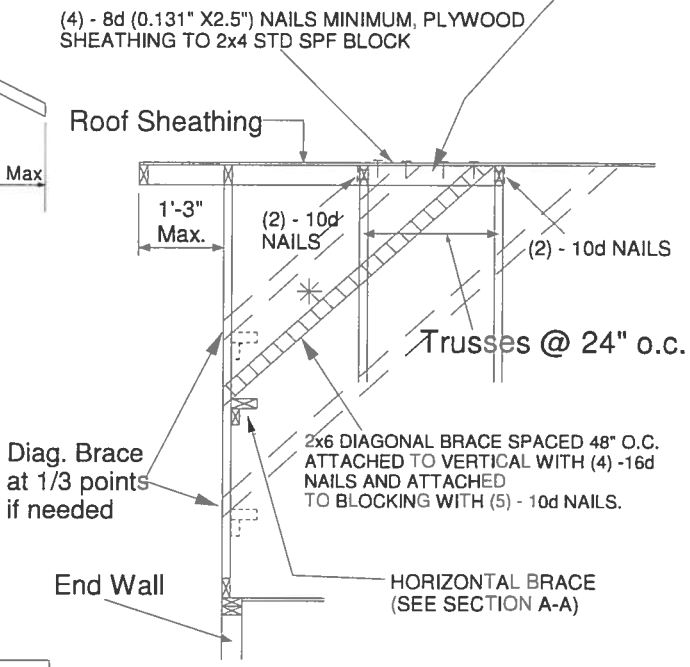




* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.



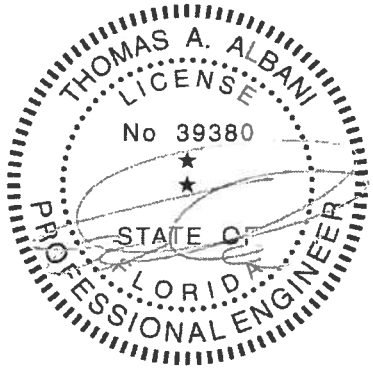
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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February 12, 2018

JANUARY 6, 2017	Standard Gable End Detail	MII-GE140-001

MiTek USA, Inc. Page 1 of 2

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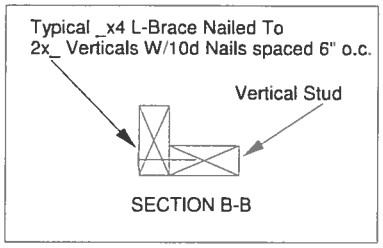
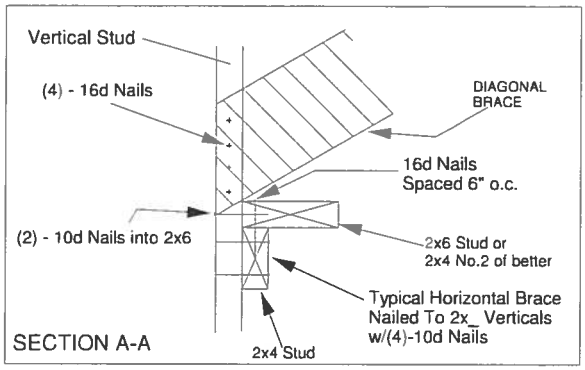
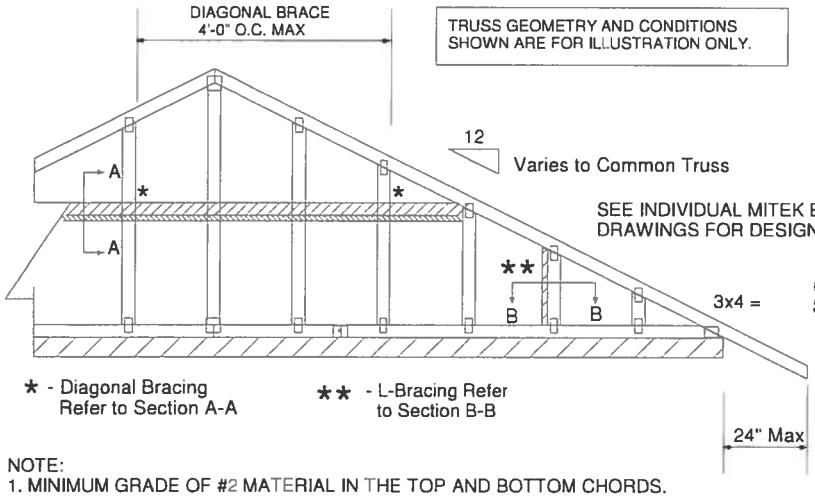


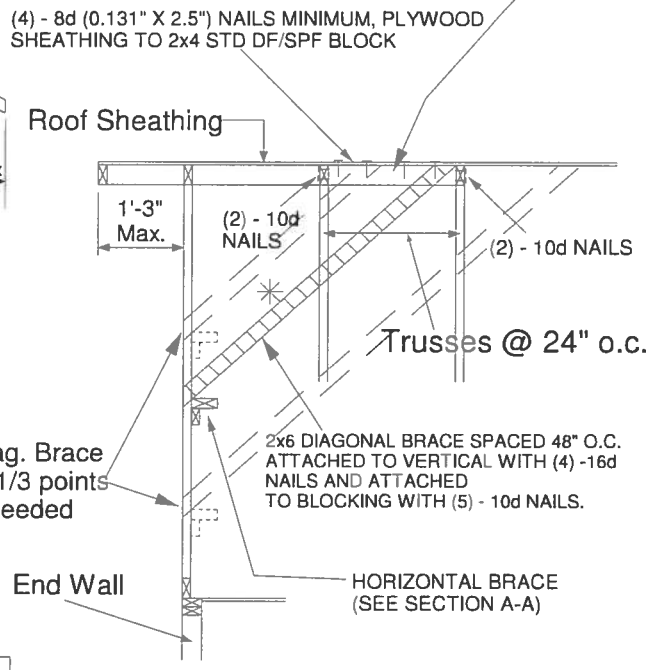




★ - Diagonal Bracing Refer to Section A-A

★★ - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.



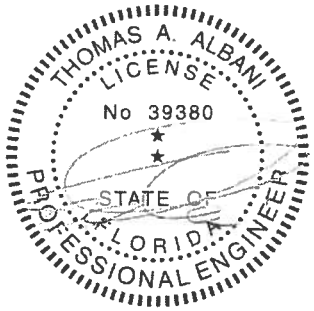
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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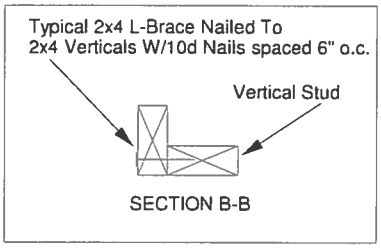
Date:

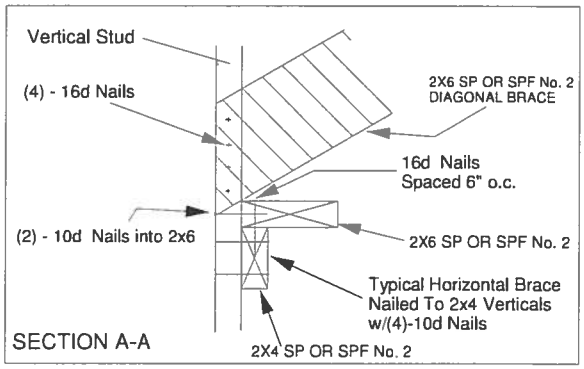
January 19, 2018

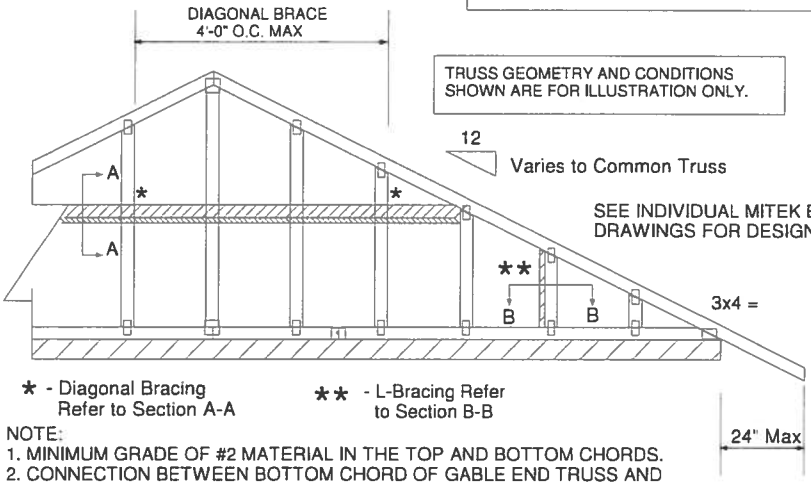
AUGUST 1, 2016	Standard Gable End Detail	MII-GE170-D-SP

MiTek USA, Inc.

MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

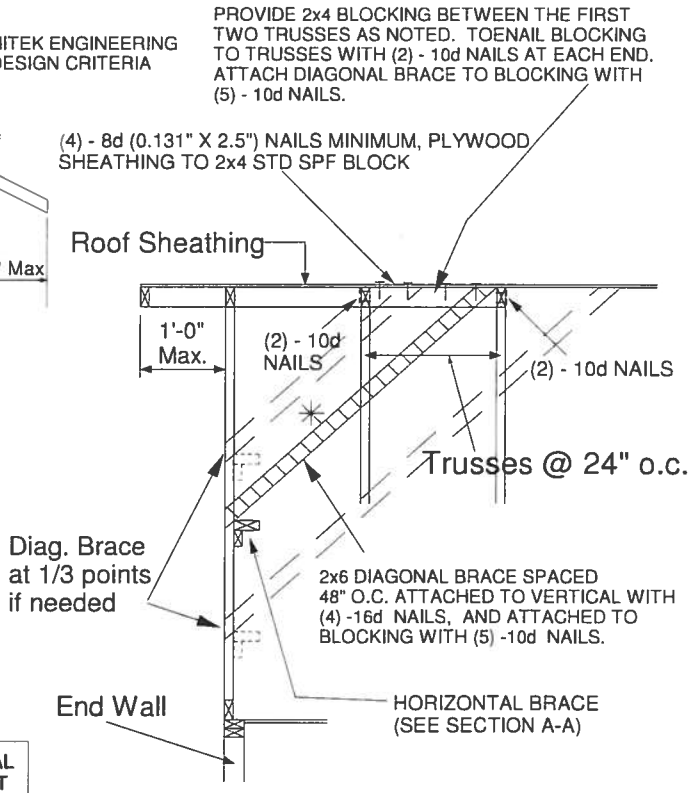






* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B



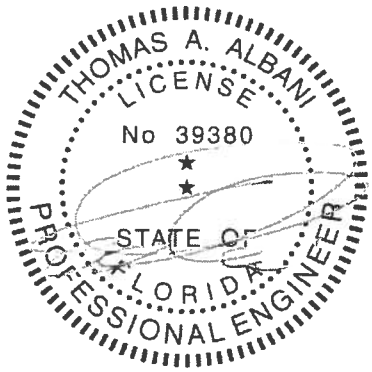
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 170 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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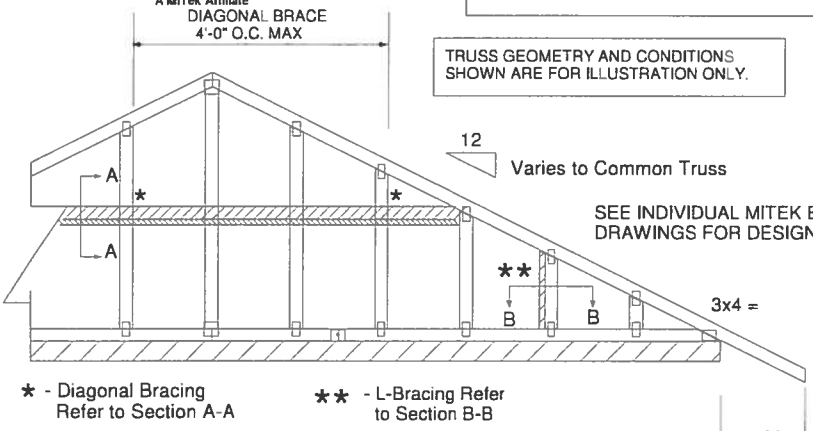
Date:
February 12, 2018

AUGUST 1, 2016	Standard Gable End Detail	MII-GE180-D-SP

MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

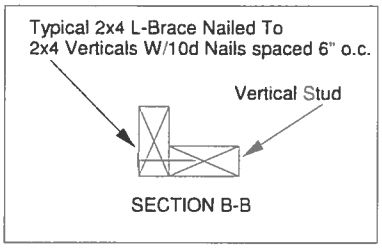


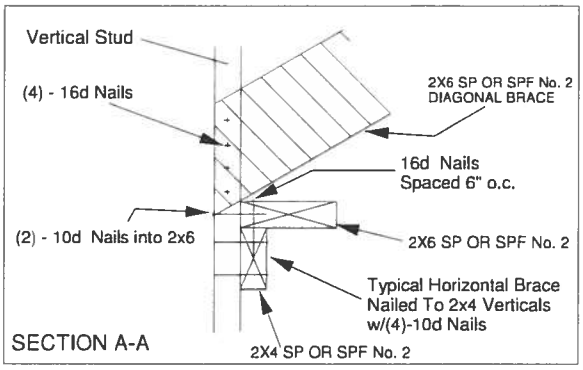
TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

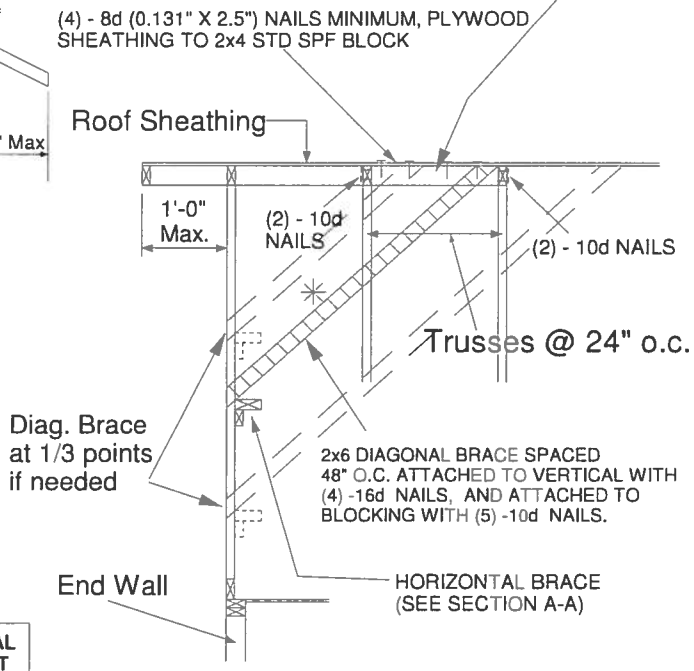
* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B





PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.



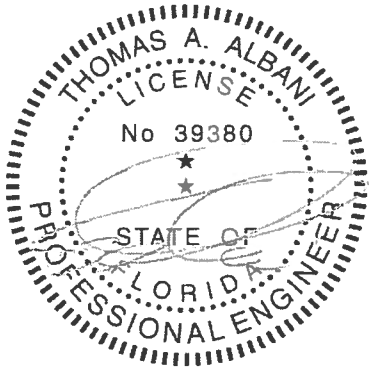
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 180 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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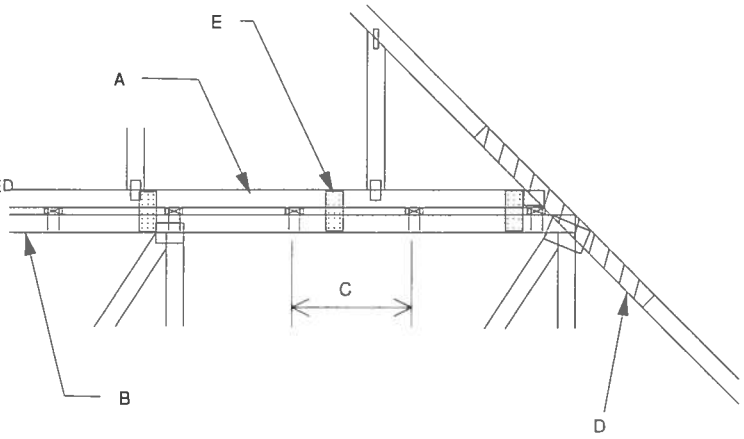
Date:
February 12, 2018

AUGUST 1, 2016	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	MII-PIGGY-7-10

MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE : 1.60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

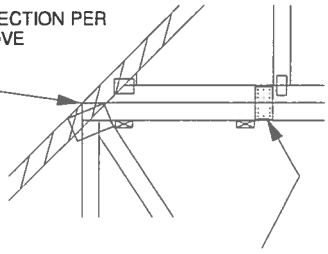
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
  1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
  2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



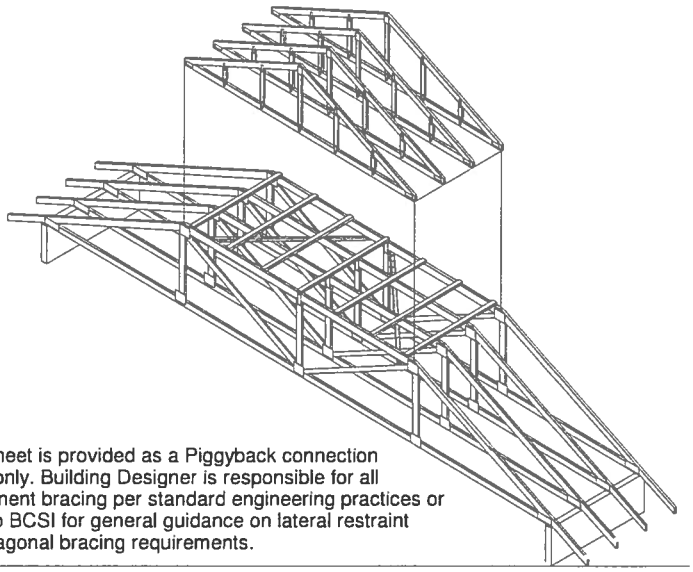
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

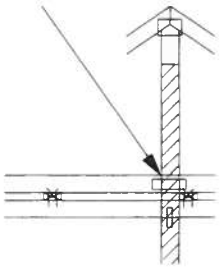


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



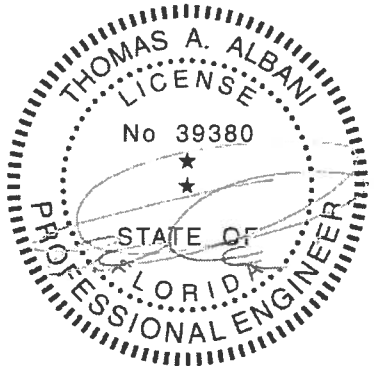
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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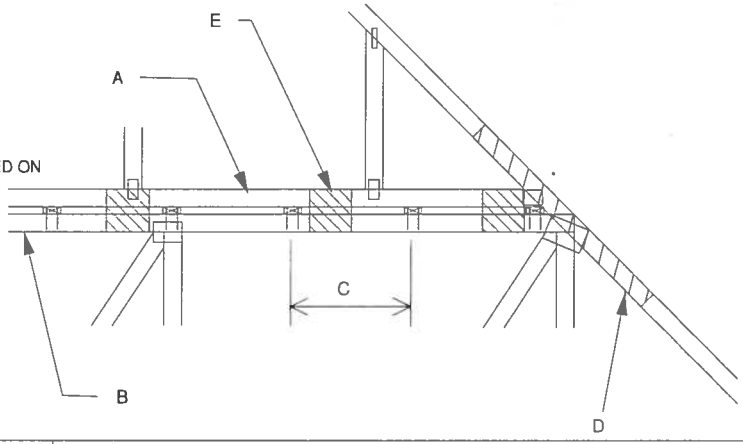
Date:
February 12, 2018

AUGUST 1, 2016	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	MII-PIGGY-ALT 7-10

MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

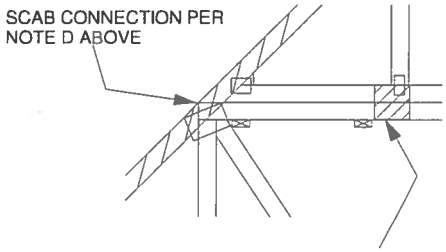
MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE : 1.60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0(0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
  1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
  2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

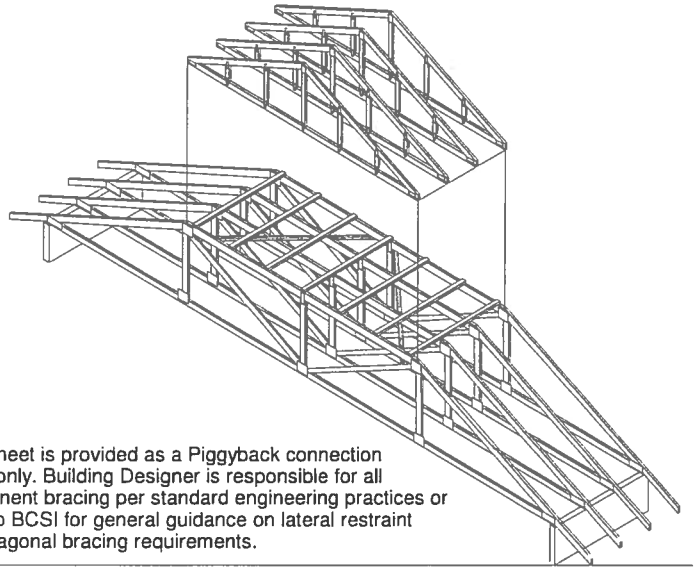


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

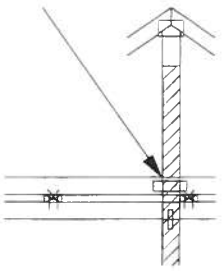


7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



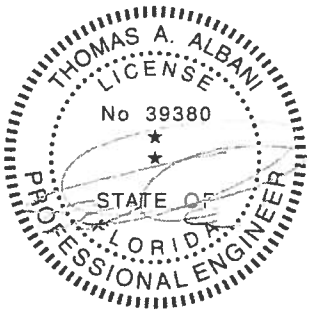
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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Date:
January 19, 2018

AUGUST 1, 2016	STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES	MII-REP01A1

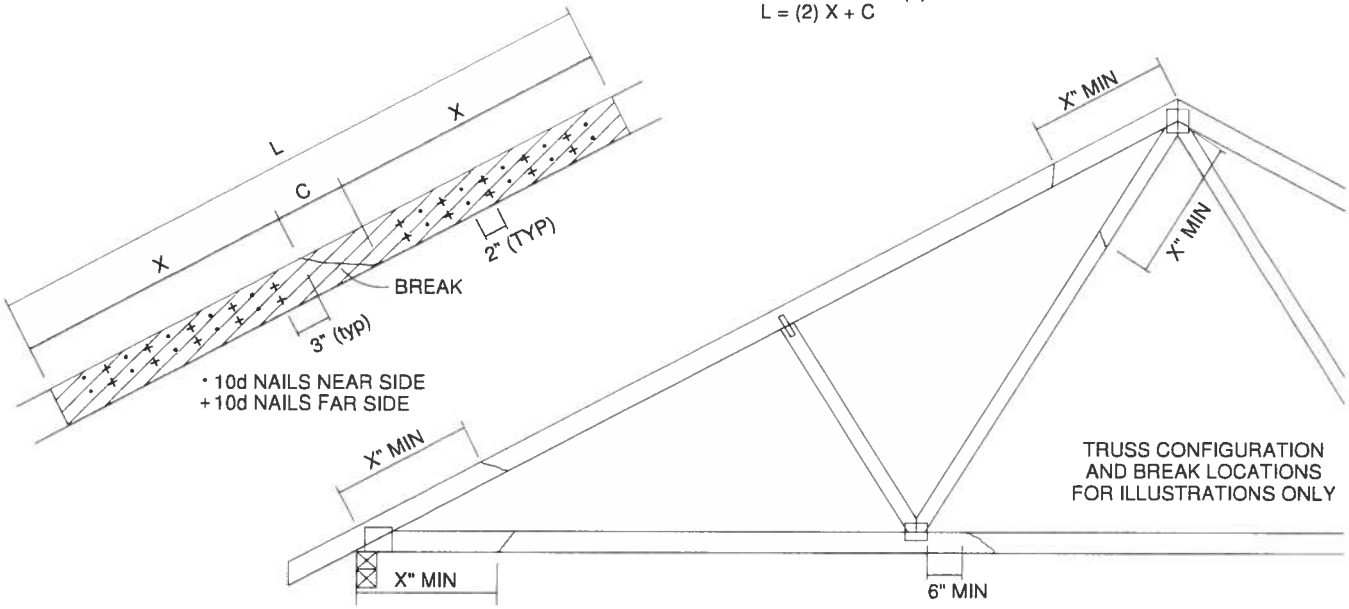
MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION							
			SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

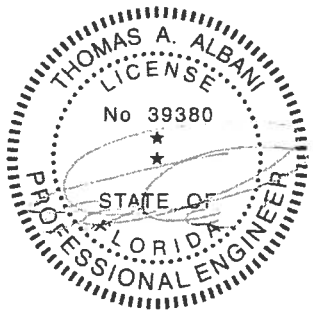
THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS)
THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:
L = (2) X + C



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:
1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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AUGUST 1, 2016	LATERAL TOE-NAIL DETAIL	MII-TOENAIL_SP

MiTek USA, Inc.
ENGINEERED BY TRENCO
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NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

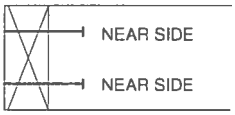
	DIAM.	SP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

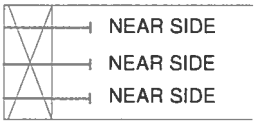
EXAMPLE:
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

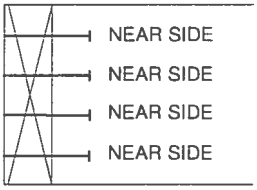
SIDE VIEW
(2x3)
2 NAILS

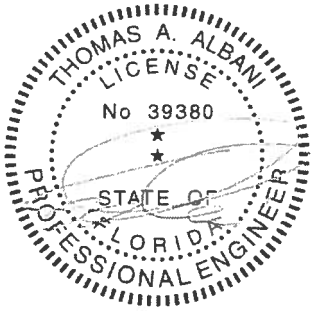


SIDE VIEW
(2x4)
3 NAILS



SIDE VIEW
(2x6)
4 NAILS





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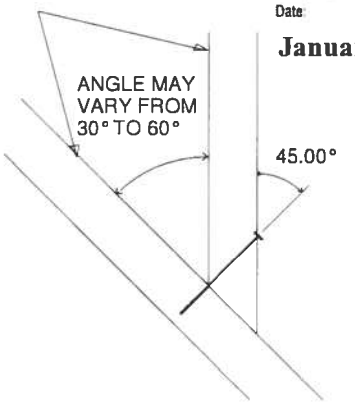
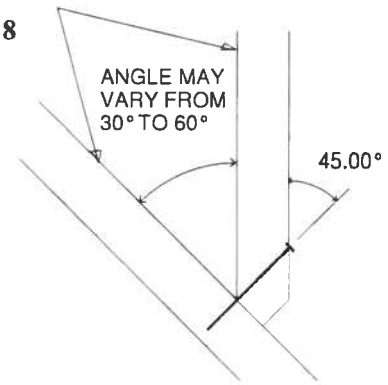
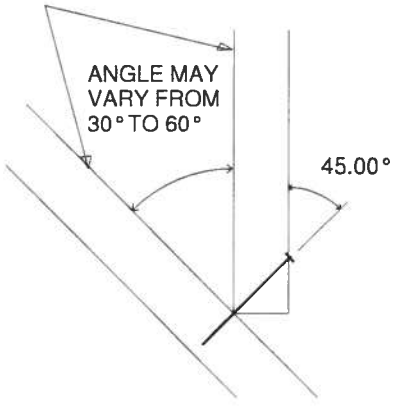




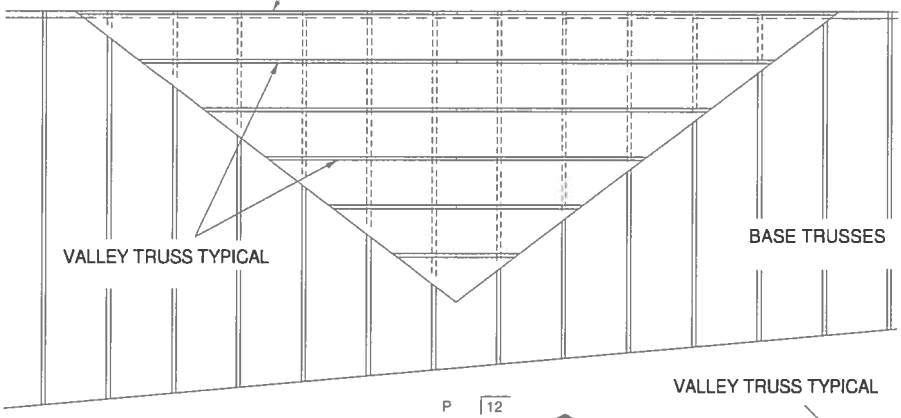


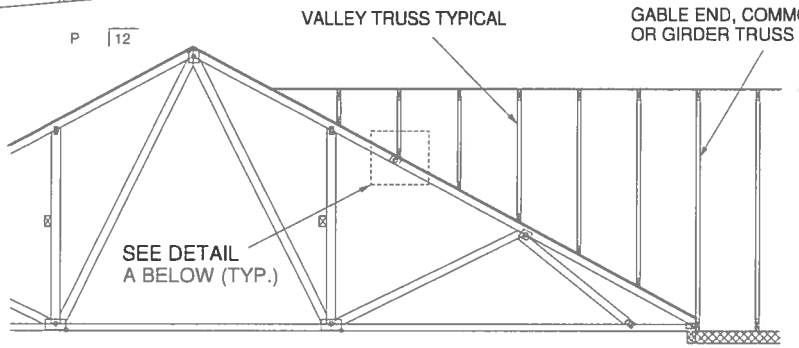
AUGUST 1, 2016	TRUSSED VALLEY SET DETAIL	MII-VALLEY HIGH WIND1

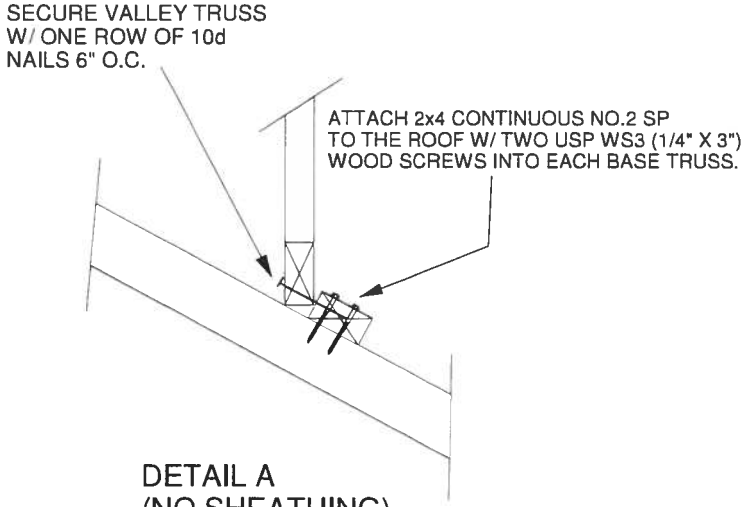
MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

GENERAL SPECIFICATIONS

1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
   DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
   SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
   INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
   EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

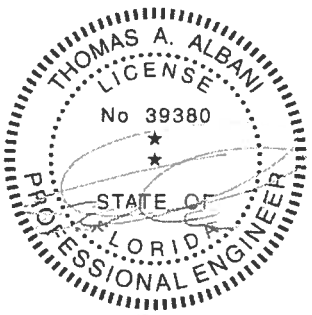






DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



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Date:

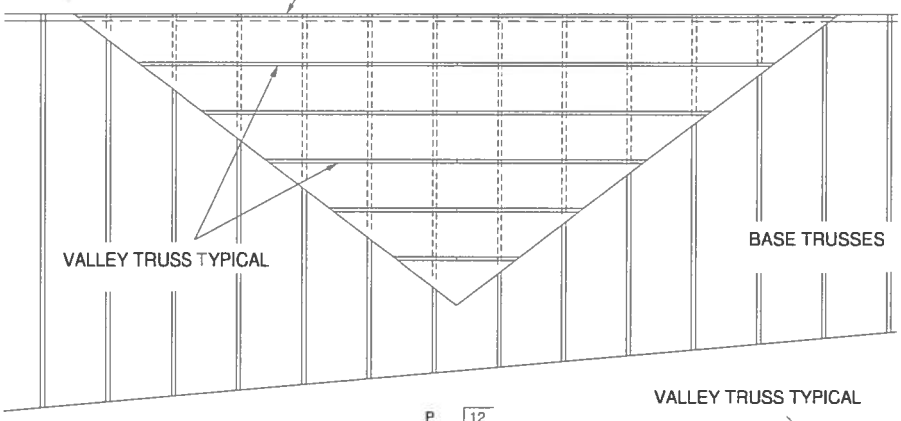
January 19, 2018

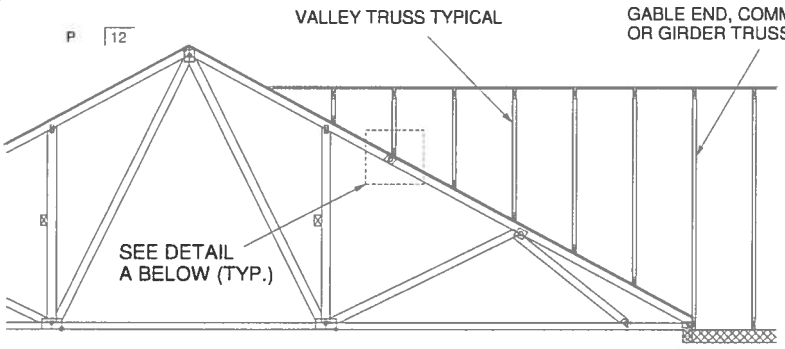
AUGUST 1, 2016	TRUSSED VALLEY SET DETAIL	MII-VALLEY HIGH WIND2

MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

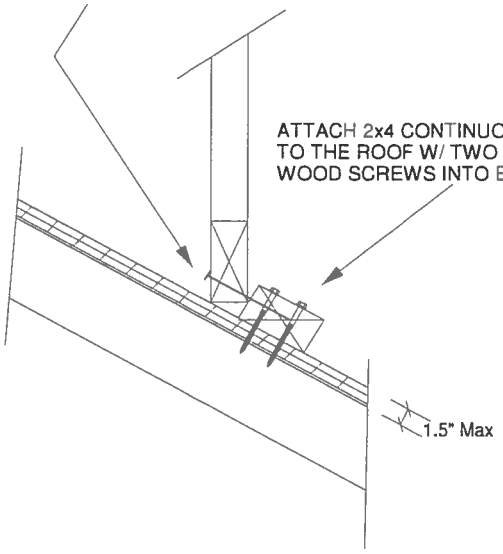
GENERAL SPECIFICATIONS

1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
6. NAILING DONE PER NDS-01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

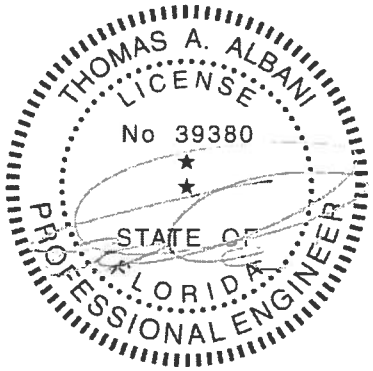




SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



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February 12, 2018

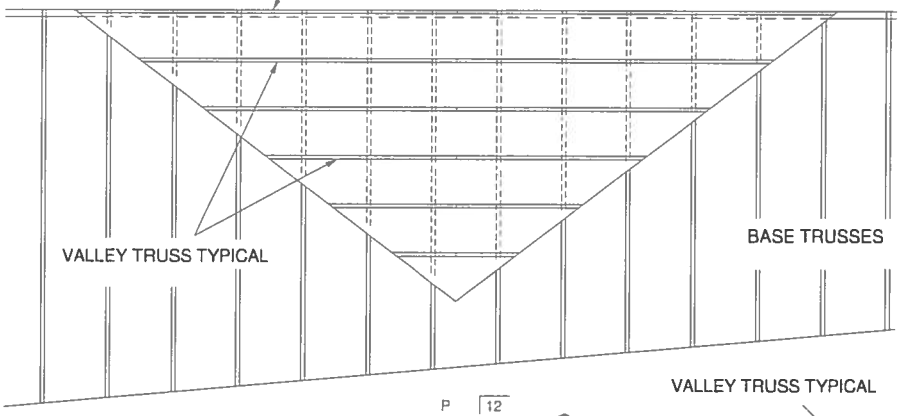
AUGUST 1, 2016	TRUSSED VALLEY SET DETAIL	MII-VALLEY SP

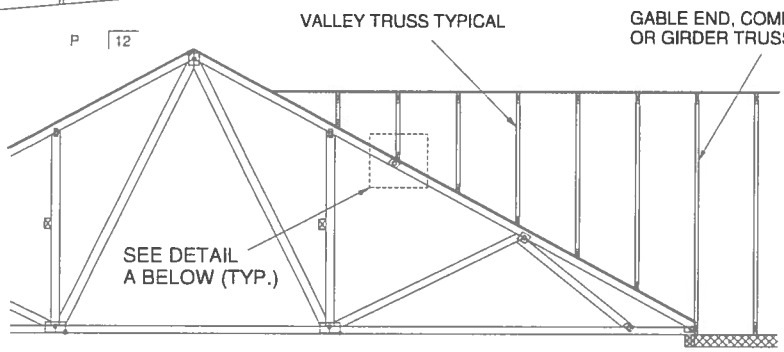
MiTek USA, Inc.

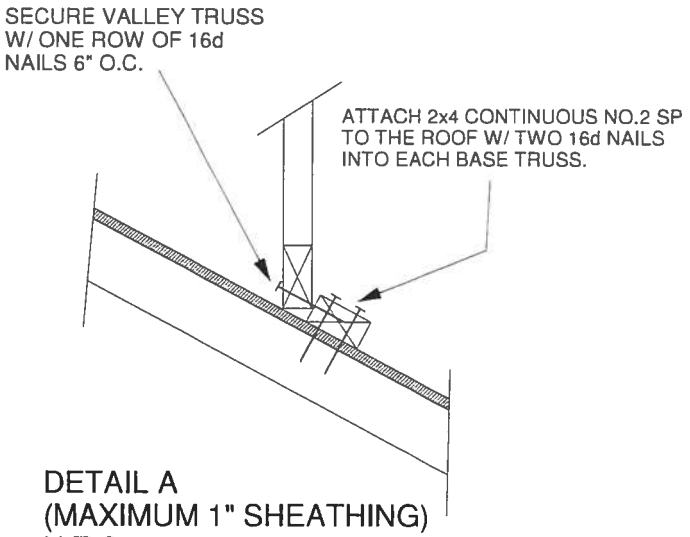
ENGINEERED BY TRENCO
A MiTek Affiliate

GENERAL SPECIFICATIONS

1. NAIL SIZE 16d (0.131" X 3.5")
2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
5. NAILING DONE PER NDS - 01
6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
7. ALL LUMBER SPECIES TO BE SP.

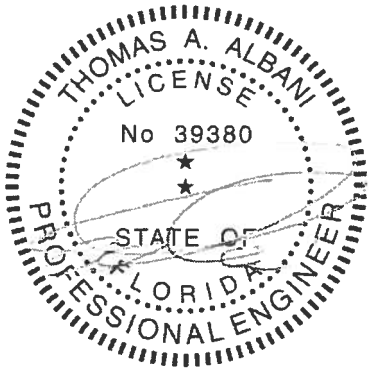






DETAIL A
(MAXIMUM 1" SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH
WIND DESIGN PER ASCE 7-10 150 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12
CATEGORY II BUILDING
EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES



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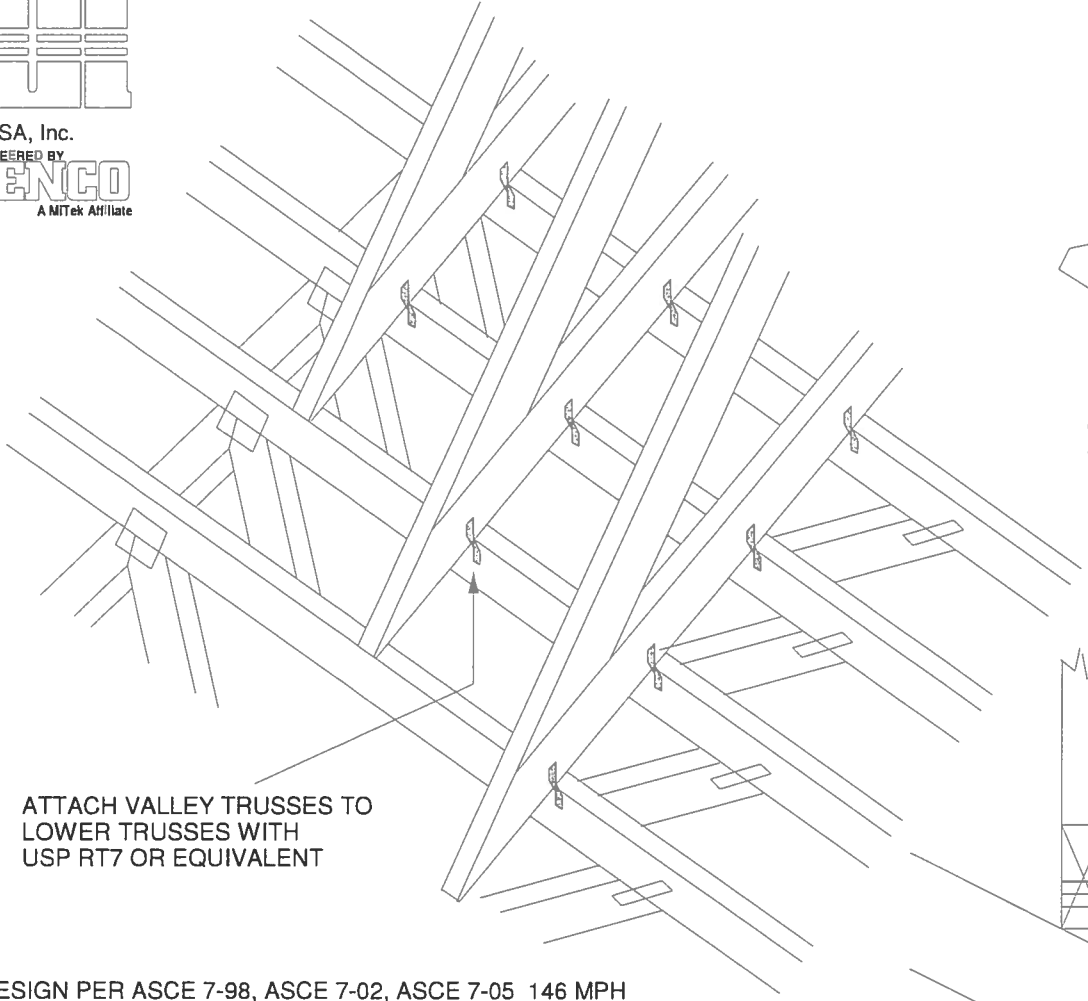
February 12, 2018

AUGUST 1, 2016	TRUSSED VALLEY SET DETAIL (HIGH WIND VELOCITY)	MII-VALLEY

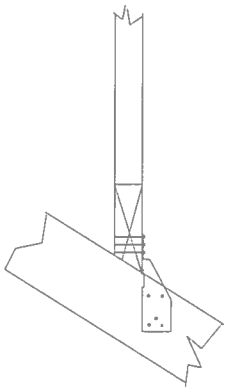
MiTek USA, Inc.

ENGINEERED BY TRENCO
A MiTek Affiliate

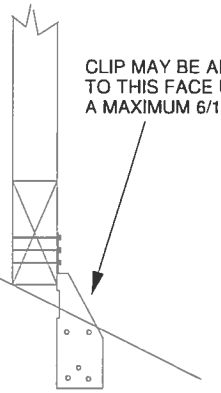
NOTE: VALLEY STUD SPACING NOT TO EXCEED 48" O.C. SPACING



ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT



FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE



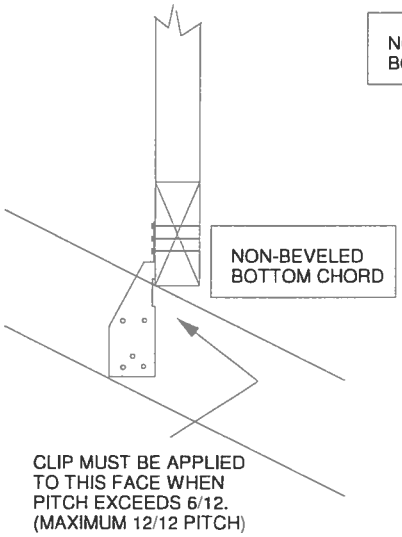
CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH

NON-BEVELED BOTTOM CHORD

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
WIND DURATION OF LOAD INCREASE : 1.6
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)

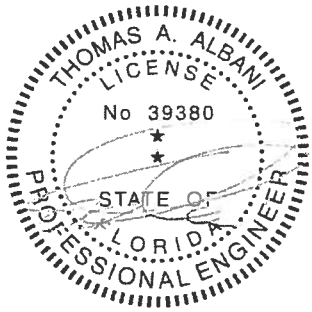
SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF 2'-10" ON AFFECTED TOP CHORDS.

NOTES:
- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.



NON-BEVELED BOTTOM CHORD

CLIP MUST BE APPLIED TO THIS FACE WHEN PITCH EXCEEDS 6/12. (MAXIMUM 12/12 PITCH)



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January 19, 2018

AUGUST 1, 2016	Standard Gable End Detail	MII-GE146-001

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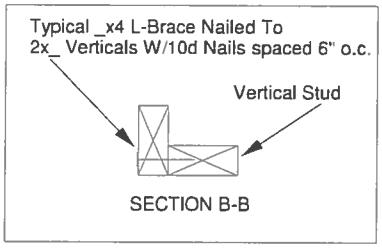
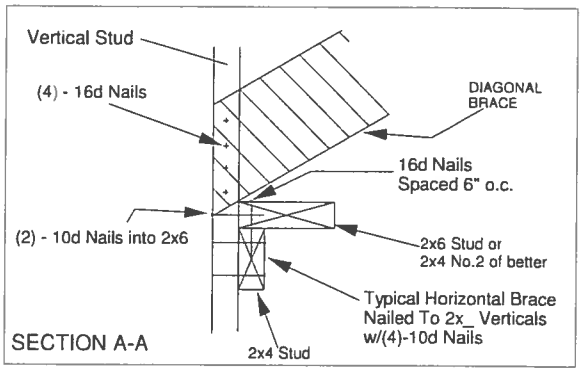
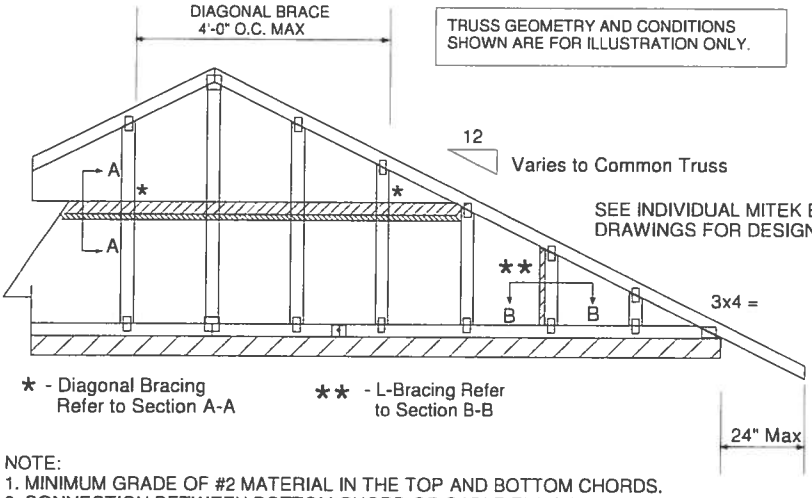




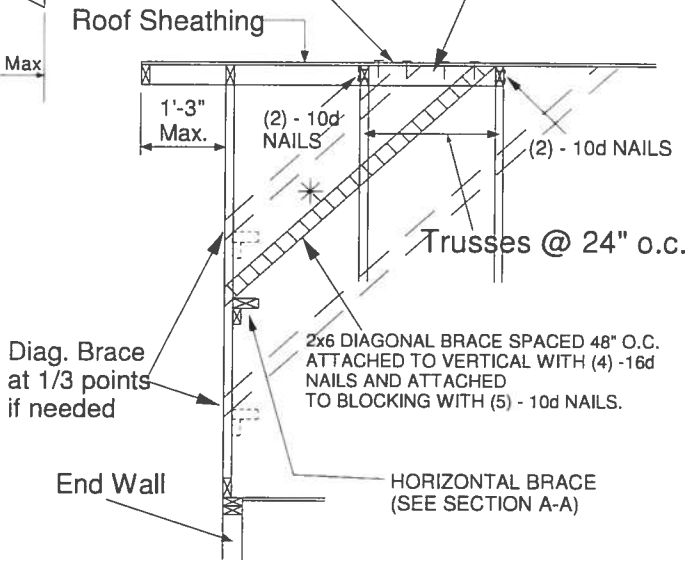


TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK



* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

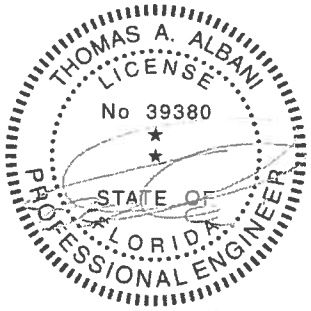
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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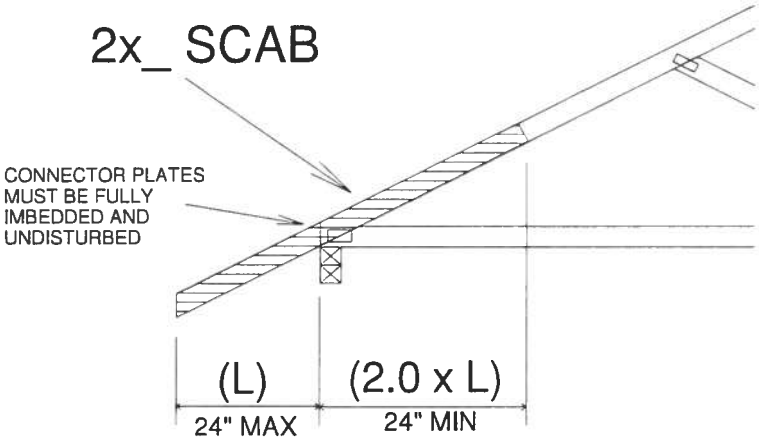
January 19, 2018

OCTOBER 5, 2016	REPLACE BROKEN OVERHANG	MII-REP13B

MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

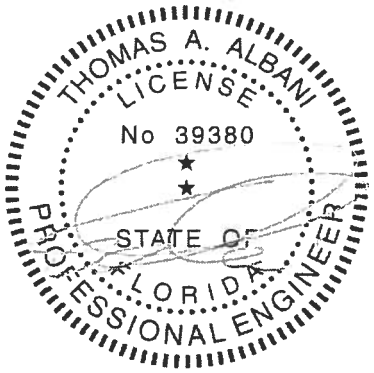
TRUSS CRITERIA:
LOADING: 40-10-0-10
DURATION FACTOR: 1.15
SPACING: 24" O.C.
TOP CHORD: 2x4 OR 2x6
PITCH: 4/12 - 12/12
HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL
END BEARING CONDITION

NOTES:
1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT
This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.
Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



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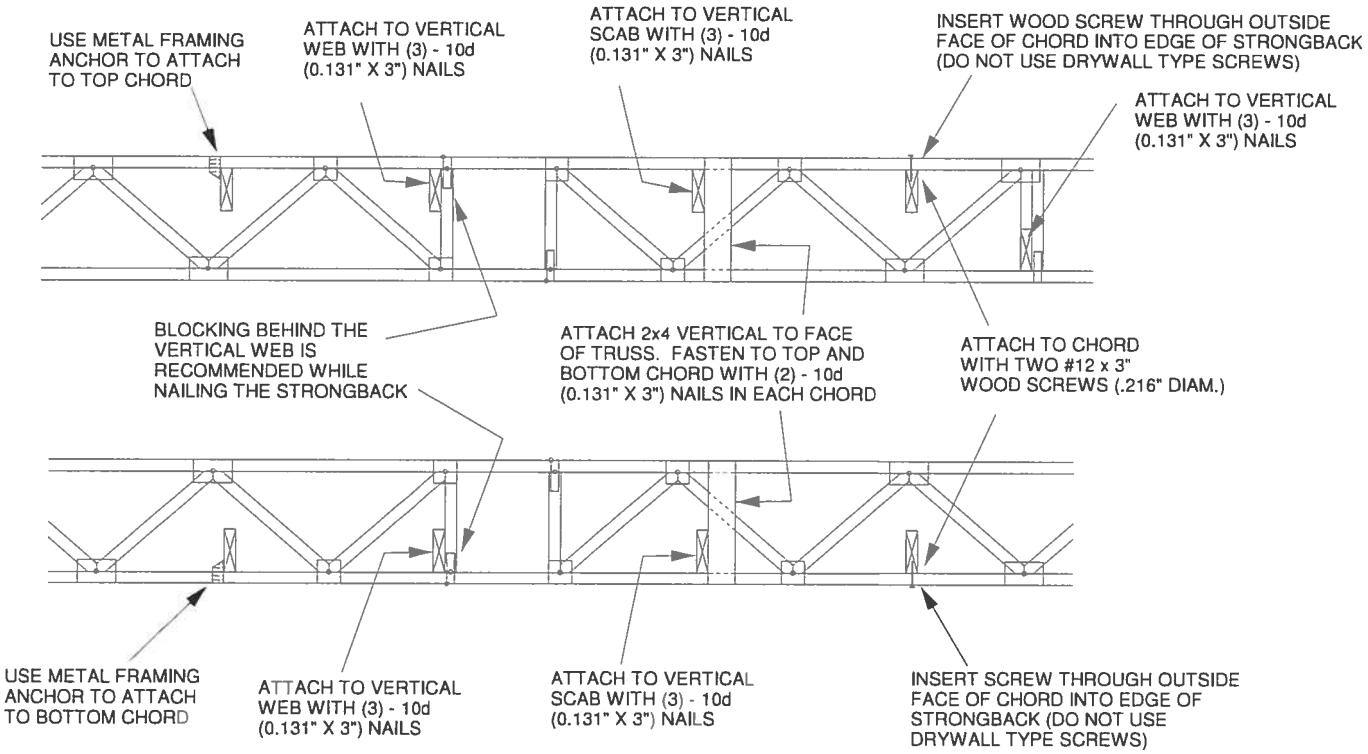
AUGUST 1, 2016	LATERAL BRACING RECOMMENDATIONS	MII-STRGBCK

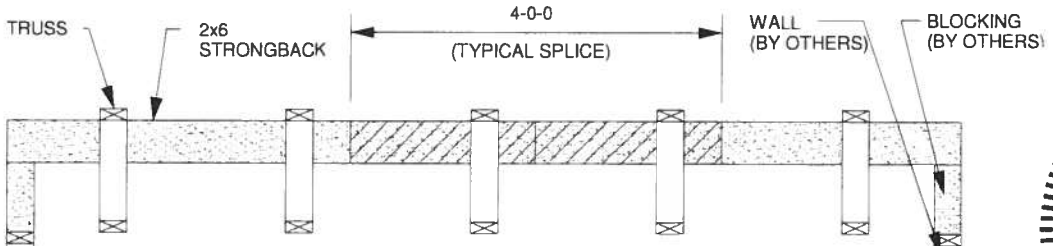
MiTek USA, Inc.
ENGINEERED BY TRENCO
A MiTek Affiliate

TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

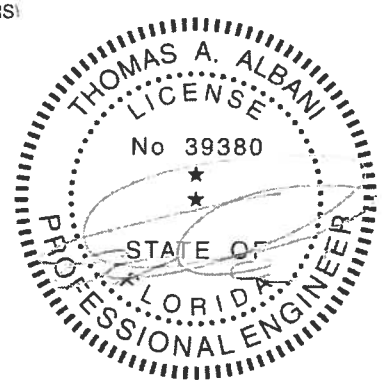
NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d (0.131" X 3") NAILS EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)



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