| DATE <u>07/0</u> | 7/2008 | Colum | PERMIT 000027142 | | | |
|---|---|---|--|--|--|---|
| | | | se Prominently Posted | on Premises During Cor | | 00002/142 |
| APPLICANT | RICHARD | | | PHONE | 386.623.4629 | FL 32025 |
| ADDRESS | 650 CTANIL EX | S.MAIN BLVD. | A.D. | LAKE CITY PHONE | 386.752.2574 | FL 32025 |
| OWNER | | & CLARA KNEPPA SW LOCKHEED L | | LAKE CITY | 380.732.2374 | FL 32025 |
| ADDRESS CONTRACTO | 483 D IAN | IES H. JOHNSTON | ANE | PHONE | 386.365.5999 | <u> </u> |
| LOCATION O | | | C-341 TL TO LOCKHE | ED LN,TL GO TO THE | | |
| LOCATION O | FIROTER | -0.000 <u>-0.000 -0.000 -0.000</u> | ON THE L. | ED EN,TE GO TO THE | TENT ELLE | |
| TYPE DEVEL | OPMENT | SFD/UTILITY | ES | TIMATED COST OF CO | NSTRUCTION | 166350.00 |
| HEATED FLO | OR AREA | 2226.00 | TOTAL ARE | EA 3327.00 | HEIGHT 21 | .30 STORIES 1 |
| FOUNDATIO | CONC | WAL | LS FRAMED R | ROOF PITCH 6'12 | FLO | OOR CONC |
| LAND USE & | ZONING | RSF-2 | | MAX | . HEIGHT 3 | 5 |
| Minimum Set | Back Requir | ments: STREET- | -FRONT 25.00 | REAR | 15.00 | SIDE 10.00 |
| NO. EX.D.U. | 0 | FLOOD ZONE | <u>X</u> | DEVELOPMENT PERM | MIT NO. | |
| PARCEL ID | 12-4S-16- | 02935-106 | SUBDIVISIO | N CANNON CREEK | ESTATES | |
| LOT 6 | BLOCK | PHASE | UNIT _ | | AL ACRES 6.3 | 37 |
| | | | CRC1328128 | 16/5 | 11 | |
| Culvert Permit | No. | Culvert Waiver | Contractor's License Nun | nber | Applicant/Owner/ | Contractor |
| EXISTING | | 08-0358-N | BLK | <u>v</u> | VR | <u>N</u> |
| Driveway Con | nection | Septic Tank Number | LU & Zonir | ng checked by App | proved for Issuance | e New Resident |
| COMMENTS: | NOC ON | FILE. 1 FOOT ABOV | /E ROAD. | | | |
| | | | | | | |
| | | | | | Check # or Ca | ash 1982 |
| | | FOR BI | JILDING & ZONIN | IG DEPARTMENT | ONLY | |
| Temporary Pov | ver | | | | | (footer/Slab) |
| | 1900 | | Foundation | | _ Monolithic _ | (footer/Slab) |
| | | date/app. by | Foundation | date/app. by | | date/app. by |
| Under slab rou | | ing | Slab | date/app. by | | date/app. by |
| | gh-in plumb | ingdate/ap | Slab _ | date/app. by | Sheathing/I | date/app. by |
| Under slab rou | gh-in plumb | ingdate/ap | Slab _ | date/app. by | Sheathing/I | date/app. by |
| | gh-in plumb date/ap | date/ap | Slab _ | date/app. by date/app. by pove slab and below wood | Sheathing/l | date/app. by Nailing date/app. by date/app. by |
| Framing | gh-in plumb date/ap h-in | date/ap | pp. by Rough-in plumbing at | date/app. by date/app. by pove slab and below wood | Sheathing/I | date/app. by Nailing date/app. by date/app. by |
| Framing | gh-in plumb date/ap h-in er | p. by date/app. by | Slab _ pp. by Rough-in plumbing ab Heat & Air Duct C.O. Final | date/app. by date/app. by pove slab and below wood date/app. by | Sheathing/l | date/app. by Nailing date/app. by date/app. by date/app. by |
| Framing Electrical roug Permanent pow | date/aph-in date/aph-in date/ap | date/ap | pp. by Rough-in plumbing at Heat & Air Duct C.O. Final | date/app. by date/app. by bove slab and below wood date/app. by date/app. by | Sheathing/I | date/app. by Nailing date/app. by date/app. by |
| Framing Electrical roug Permanent pow M/H tie downs, | date/aph-in date/aph-in date/ap | p. by date/app. by te/app. by | pp. by Rough-in plumbing at Heat & Air Duct C.O. Final | date/app. by date/app. by oove slab and below wood date/app. by date/app. by b. by | Sheathing/I I floor Peri. beam (Linte Culvert Pool | date/app. by Nailing date/app. by date/app. by date/app. by |
| Framing Electrical roug Permanent pow M/H tie downs, Reconnection | date/aph-in date/aph-in da | p. by date/app. by te/app. by ectricity and plumbing | pp. by Rough-in plumbing at Heat & Air Duct C.O. Final g date/app Pump pole date/ | date/app. by date/app. by bove slab and below wood date/app. by date/app. by | Sheathing/ld floor Peri. beam (Lintel Culvert Pool date/app. by | date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by |
| Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole | date/aph-in date/aph-in da | p. by date/app. by te/app. by ectricity and plumbing | pp. by Rough-in plumbing at Heat & Air Duct C.O. Final g date/app Pump pole date/ | date/app. by date/app. by oove slab and below wood date/app. by date/app. by Distribution of the control o | Sheathing/I I floor Peri. beam (Linte Culvert Pool | date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by |
| Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole da | date/ap h-in date/ap blocking, el te/app. by | p. by date/app. by te/app. by ectricity and plumbing date/app. by Tra | pp. by Rough-in plumbing at the second secon | date/app. by date/app. by bove slab and below wood date/app. by date/app. by Utility Po /app. by ate/app. by | Sheathing/I d floor Peri. beam (Lintel Culvert Pool date/app. by Re-roof | date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by |
| Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole da BUILDING PE | date/aph-in plumb date/aph-in er da blocking, el te/app. by | p. by date/app. by te/app. by ectricity and plumbing date/app. by Tra | Pop. by Rough-in plumbing at the second sec | date/app. by date/app. by bove slab and below wood date/app. by date/app. by Utility Polyapp. by ate/app. by E \$16.64 | Sheathing/I d floor Peri. beam (Lintel Culvert Pool date/app. by Re-roof SURCHARGE | date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by FEE \$ 16.64 |
| Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole da BUILDING PE MISC. FEES \$ | date/aph-in plumb date/aph-in er da blocking, el te/app. by RMIT FEE | p. by date/app. by te/app. by ectricity and plumbing date/app. by | Ppp. by Rough-in plumbing at the second sec | date/app. by date/app. by date/app. by date/app. by date/app. by Utility Po /app. by ate/app. by E \$ | Sheathing/I I floor Peri. beam (Linter Culvert Pool date/app. by Re-roof SURCHARGE WASTI | date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by FEE \$ 16.64 |
| Framing Electrical roug Permanent pow M/H tie downs, Reconnection M/H Pole da BUILDING PE | date/aph-in plumb date/aph-in er da blocking, el te/app. by RMIT FEE | p. by date/app. by te/app. by ectricity and plumbing date/app. by | Ppp. by Rough-in plumbing at the second sec | date/app. by date/app. by bove slab and below wood date/app. by date/app. by Utility Polyapp. by ate/app. by E \$16.64 | Sheathing/I I floor Peri. beam (Linter Culvert Pool date/app. by Re-roof SURCHARGE WASTI | date/app. by Nailing date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by FEE \$ 16.64 |

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.



OCCUPA

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in

accordance with the Columbia County Building Code.

Parcel Number 12-4S-16-02935-106

Building permit No. 000027142

Permit Holder JAMES H. JOHNSTON

Use Classification SFD/UTILITY

Fire: 48.88

Waste:

67.00

Owner of Building STANLEY & CLARA KNEPPAR

483 SW LOCKHEED LANE, LAKE CITY, FI

Location:

Date: 06/15/2009

Total: 115.88

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

Columbia County Building Permit Application

| For Office Use Only Application # 0505-12 Date Received 923 By W Permit # 27/42 |
|--|
| Zoning Official BLK Date 27.06.08 Flood Zone X FEMA Map # VIA Zoning RSF-2 |
| Land Use RES. Land Detelevation N/A MFE River N/A Plans Examiner Date 6/22/08 |
| Comments |
| NOC □ÆH □-Deed or PA □-Site Plan □-State Road Info □ Parent Parcel # |
| □ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor |
| □ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter |
| Septic Permit No. 08-0358 N RICHARD KEEN DE 625-4679 Name Authorized Person Signing Permit James Johnston Phone 365-5999 |
| Name Authorized Person Signing Permit James Johnston Phone 365-5999 |
| Address 650 5 Main Blvd. LAKE City FL 32025 |
| Owners Name Stanley + Clara Kneppar Phone 752-2574 |
| 911 Address 483 SW Lockheed Lane LAKe City FL 32025 |
| Contractors Name James Johnston Phone 365-5999 |
| Address 650 5 Main Blue LAKe City FL 32025 |
| Fee Simple Owner Name & Address Stanley + Clara Kneppar 8109 Hibiscu |
| Bonding Co. Name & Address |
| Architect/Engineer Name & Address William Freeman 128 SW Nassau St. L.C. |
| Mortgage Lenders Name & Address |
| Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy |
| Property ID Number 12-48-16-02935-106 Estimated Cost of Construction 207,000 == |
| Subdivision Name Cannon Creek Estates Plot 6 Block Unit Phase |
| Driving Directions 90 W, Turn Left on Sisters Welcome, go |
| down to Sw Lockheed lane turn left, go to end |
| Number of Existing Dwellings on Property O |
| Construction of SFD Total Acreage 6.37 Lot Size |
| Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u> Total Building Height 21 3 |
| Actual Distance of Structure from Property Lines - Front 349 Side 170 Side 102 Rear 375 |
| Number of Stories Heated Floor Area 2226 Total Floor Area 3327 Roof Pitch 4/12 |
| Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. |

Page 1 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

: JW: cilled RK Quay ON 6. 27.08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT I YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or othe services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit. March 27, 2010 DD 513324 CONTRACTORS AFFIDAVIT: By my signature I understand and agree that have intermed and provided this written statement to the owner of all the above written responsibilities in County for obtaining this Building Permit. Contractor's License Number CRC 1328 128 Contractor's Signature (Permitee) **Columbia County** Competency Card Number Affirmed under penalty of perjury to by the Contractor and subscribed before me this 23 day of June 20 08. Personally known____or Produced Identification_ SEAL: State of Florida Notary Signature (For the Contractor) LAURIE HODSON MY COMMISSION # DD 333503

Page 2 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

EXPIRES: June 28, 2008 Bonded Thru Notary Public Underwriters

| NOTICE OF COMMENCEMENT |
|--|
| Tax Parcel Identification Number 12-45-16-02935-106 |
| THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT. |
| 1. Description of property (legal description): Lot 6 Cannon Creek Estates a) Street (Job) Address: SW Lock heed Unive 2. General description of improvements: Boild SFD |
| 3. Owner Information a) Name and address: Stanley + Clara Kreppar 8109 Hibiscus Lane Taramar FL b) Name and address of fee simple titleholder (if other than owner) c) Interest in property 100% |
| 4. Contractor Information a) Name and address: James H. Johnston 650 500 Main Blyd. Lake City FL b) Telephone No.: 386-755-2826 Fax No. (Opt.) 32025 5. Surety Information |
| a) Name and address: b) Amount of Bond: c) Telephone No.: Fax No (Ont) |
| 6. Lender a) Name and address: b) Phone No. Inst 200812011831 Date 6/23/2008 Time 11 31 AM CDC P. DeWitt Cason Columbia County Page 1 of 1 B:1152 P:2786 |
| 7. Identity of person within the State of Florida designated by owner upon whom notices of outer documents may be served. |
| a) Name and address: b) Telephone No.: Fax No. (Opt.) |
| 8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(I)(b). Florida Statutes: a) Name and address: |
| b) Telephone No Fax No. (Opt.) |
| 9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): |
| WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713, 13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT. STATE OF FLORIDA COUNTY OF COLUMBIA The foregoing instrument was acknowledged before me, a Florida Notary, this day of the control of t |
| as(type of authority, e.g. officer, trustee, attorney |
| fact) for |
| Personally Known OR Produced Identification Type Notary Signature Notary Stamp or Seal: AND 11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury. I declare that flavoread the foregoing and that the facts stated in it are true to the best of my knowledge and belief. Signature of Natural Person Signing (in line #10 above.) |

| This document prepared by (and after recording Return to): Name: Michael Kneppar Firm/Company: Address: Lot 6 Cannon Creek Estates Address 2: 417 SW Lockheed Lane City, State, Zip: Lake City, Florida, 32025 Phone: 386-965-6453 |) |
|---|--|
| Donasta Associate Donasta Valuation in the Control of the Control | DC,P.DeWitt Cason,Columbia County Page 1 of 2 |
| Property Appraiser Parcel Identification Number= 12-4 | |
| QUITCLAIM DEED (Individ | |
| THIS QUITCLAIM DEED, Made the 11 Day of 1 | , 20 <u>47</u> , by |
| Michael L. Kneppar an Individual, hereinafter referred to as | "Grantor", to |
| Stanley and Clara Kneppar Joint, hereinafter called "Grantee | 3." |
| Witnesseth, that the Grantor, for and in consideration good and valuable consideration, cash in hand paid, the receis acknowledged, does hereby remise, release, and quitclaim us together with all improvements located thereon, lying in the Lot 6 Cannon Creek Estates S/D, Lake City, Florida, 32025 | ipt and sufficiency of which is herby |
| Describe Property of State "SEE DESCRIPTION A | TTACHED" |
| LOT 6 CANNON CREEK ESTATES S/D ORB 710-502, 74 | 5-1237, 826-1323, 904-2415. WD 1090- 2429. |
| Prior instrument reference: Book, Page, the Clerk of the Court for Columbia County, Florida. | Document No, of the Public Records of |
| Said Address: Lot 6 Cannon Creek Estates, Lake City, Florid | a 32025 |
| LESS AND EXCEPT all oil, gas and minerals, on a by Grantor, if any, which are reserved by Grantor. | nd under the above described property owned |
| SUBJECT to restrictions, reservations, easements, a not serve to reimpose the same. | nd covenants of record, reference hereto will |
| TO HAVE AND TO HOLD the same together with belonging or in anywise appertainting, and all the estate, right whatsoever of the said Grantor(s), either in law or equity, to to Grantee(s) forever. | t, title interest, lien equity and claim |
| Taxes for the yearshall beprorated between y Grantor and Grantee, or paid by Grantee, or <u>X</u> paid | en Grantor and Grantee as of the date selected d by Grantor. |
| The property herein conveyed X is not part of the conveyed | homestead of Grantor, or is part of the |

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED:

5/9/2008

DATE ISSUED:

5/12/2008

ENHANCED 9-1-1 ADDRESS:

483

SW LOCKHEED

LN

LAKE CITY

FL 32025

PROPERTY APPRAISER PARCEL NUMBER:

12-45-16-02935-106

Remarks:

LOT 6 CANNON CREEK ESTATES S/D

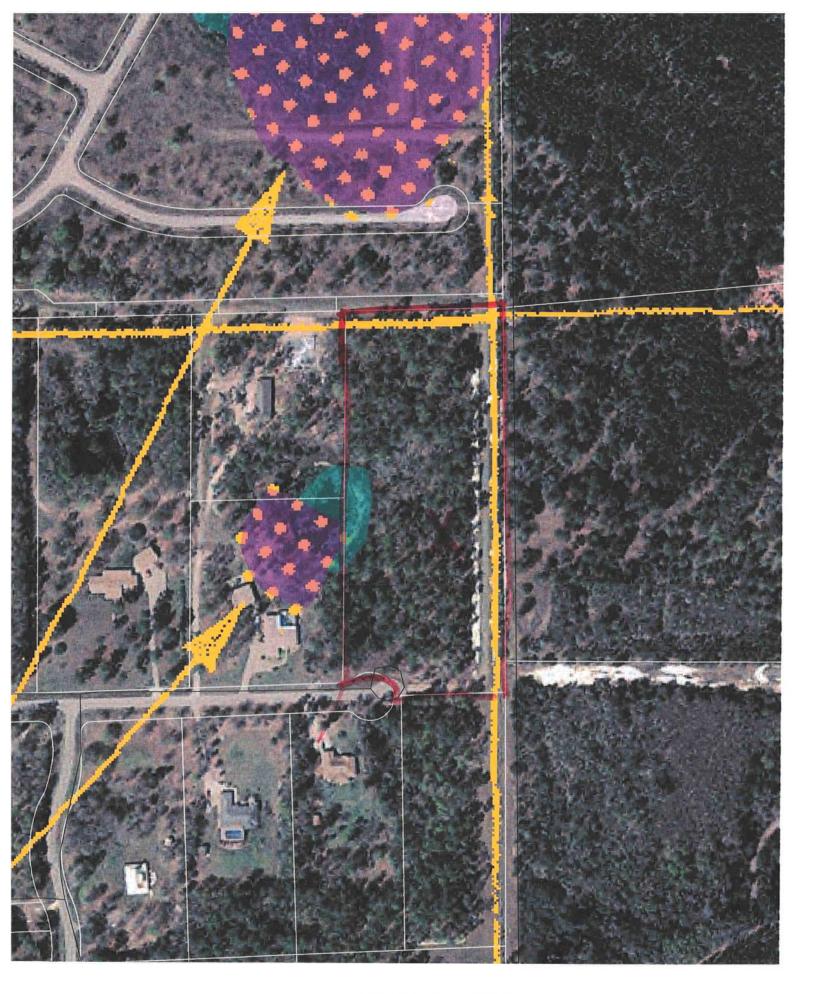
Address Issued By County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

A & B Well Drilling, Inc. 5673 NW Lake Jeffery Road Lake City, FL, 32055 386-758-3409

6/18/2008

| To: Suwannee County B | 17.0 | | | |
|---|-----------|--------------|---------------------|-------------------|
| Description of well to be Located at Address: | installed | for Customer | : Knepper | |
| Located at Address: | SW | Lockhead | lane Lak | elity |
| 1 hp 20 gpm- 1 ¼" drop back flow prevention. W | | | nt captive tank wit | th cycle stop and |
| Willia Bir | · · | | | |
| William Bias | | 94 | | |



0806-42

Project Name:

with the Florida Energy Code.

OWNER/AGENT: (

DATE:

Address:

City, State:

Kneppar Residence

Lake City, FL

Lake City Could Bio

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Builder:

Permitting Office:

Permit Number:

| Owner: Climate Zone: North | Jurisdiction Number: 22/000 |
|--|---|
| 1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor: | 12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) |
| Glass/Floor Area: 0.08 Total as-built po | DVC.C. |
| I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: | Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 |

Florida Statutes.

DATE:

BUILDING OFFICIAL: ___

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

| | BASE | | | AS-BUILT | | | | | | | | |
|---------------------------------------|---------------|---------------|-----------------|-------------------------------|-------|---------------|--------|-----------|------|----------|------|----------|
| GLASS TYPES .18 X Condition Floor Are | | SPM = | Points | Type/SC | | erhang Len | Hgt | Area X | SPI | мх | SOF | = Points |
| .18 2184. | 8 | 18.59 | 7311.0 | 1.Double, Clear | N | 110. 1 | 0.0 | 24.0 | 19.2 | 20 | 0.59 | 273.0 |
| | | | | 2.Double, Clear | N | | 6.0 | 16.0 | 19.2 | 20 | 0.98 | 299.0 |
| | | | | 3.Double, Clear | S | | 0.0 | 54.0 | 35.8 | 87 | 0.61 | 1187.0 |
| | | | | 4.Double, Clear | S | | 6.0 | 54.0 | 35.8 | 530 | 0.94 | 1828.0 |
| | | | | 5.Double, Clear | Е | 1.0 | 6.0 | 36.0 | 42.0 | 06 | 0.97 | 1468.0 |
| | | | | As-Built Total: | | | | 184.0 | | | | 5055.0 |
| WALL TYPES | Area X | BSPM | = Points | Туре | | R-V | /alue | Area | Х | SPM | = | Points |
| Adjacent Exterior | 0.0 1929.5 | 0.00 1.70 | 0.0 3280.2 | 1. Face Brick, Wood, Exterior | r | 1 | 3.0 | 1929.5 | | 0.35 | | 675.3 |
| Base Total: | 1929.5 | | 3280.2 | As-Built Total: | | | | 1929.5 | | | | 675.3 |
| DOOR TYPES | Area X | BSPM | = Points | Туре | | | | Area | Х | SPM | = | Points |
| Adjacent | 0.0 | 0.00 | 0.0 | 1.Exterior Insulated | | | | 33.0 | - | 4.10 | - | 135.3 |
| Exterior | 71.3 | 6.10 | 434.8 | 2.Exterior Insulated | | | | 19.8 | | 4.10 | | 81.2 |
| | | | | 3.Exterior Insulated | | | | 18.5 | | 4.10 | | 75.8 |
| Base Total: | 71.3 | | 434.8 | As-Built Total: | | | | 71.3 | | 25.000 m | | 292.2 |
| CEILING TYPES | Area X | BSPM | = Points | Туре | | R-Value | e Ai | rea X S | PM | X SCI | M = | Points |
| Under Attic | 2184.8 | 1.73 | 3779.7 | 1. Under Attic | | 30 | 0.0 2 | 2184.8 1. | 73 X | 1.00 | | 3779.7 |
| Base Total: | 2184.8 | | 3779.7 | As-Built Total: | | | 2 | 2184.8 | | | | 3779.7 |
| FLOOR TYPES | Area X | BSPM | = Points | Туре | | R-V | alue | Area | Х | SPM | = | Points |
| Slab 30 Raised | 0.0(p) 0.0 | -37.0 0.00 | -11100.0 0.0 | 1. Slab-On-Grade Edge Insula | ation | (| 0.0 30 | 00.0(p | -4 | 41.20 | | -12360.0 |
| Base Total: | | | -11100.0 | As-Built Total: | | | | 300.0 | | | | -12360.0 |
| INFILTRATION | Area X | BSPM | = Points | | | | | Area | X | SPM | = | Points |
| | 2184.8 | 10.21 | 22306.8 | | | | | 2184.8 | | 10.21 | | 22306.8 |

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

| | BASE | | AS-BUILT | | | | | | |
|------------------------|------------------------|---------------------|---|--|--|--|--|--|--|
| Summer Ba | se Points: | 26012.5 | Summer As-Built Points: 19749.1 | | | | | | |
| Total Summer Points | X System Multiplier | = Cooling Points | Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU) | | | | | | |
| 26012.5 | 0.3250 | 8454.1 | (sys 1: Central Unit 32000btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Int(AH),R6.0(INS) 19749 | | | | | | |

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

| BASE | AS-BUILT | | | | | | | |
|--|--|--|--|--|--|--|--|--|
| GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area | Overhang Type/SC Ornt Len Hgt Area X WPM X WOF = Points | | | | | | | |
| .18 2184.8 20.17 7932.0 | 1.Double, Clear N 110. 10.0 24.0 24.58 1.03 605.0 2.Double, Clear N 1.0 6.0 16.0 24.58 1.00 393.0 3.Double, Clear S 6.0 10.0 54.0 13.30 1.90 1361.0 4.Double, Clear S 1.0 6.0 54.0 13.30 1.02 734.0 5.Double, Clear E 1.0 6.0 36.0 18.79 1.02 687.0 As-Built Total: 184.0 3780.0 | | | | | | | |
| WALL TYPES Area X BWPM = Points | Type R-Value Area X WPM = Points | | | | | | | |
| Adjacent 0.0 0.00 0.0 Exterior 1929.5 3.70 7139.1 | 1. Face Brick, Wood, Exterior 13.0 1929.5 3.17 6126.2 | | | | | | | |
| Base Total: 1929.5 7139.1 | As-Built Total: 1929.5 6126.2 | | | | | | | |
| DOOR TYPES Area X BWPM = Points | Type Area X WPM = Points | | | | | | | |
| Adjacent 0.0 0.00 0.0 Exterior 71.3 12.30 876.7 Base Total: 71.3 876.7 | 1.Exterior Insulated 33.0 8.40 277.2 2.Exterior Insulated 19.8 8.40 166.3 3.Exterior Insulated 18.5 8.40 155.2 As-Built Total: 71.3 598.8 | | | | | | | |
| CEILING TYPES Area X BWPM = Points | Type R-Value Area X WPM X WCM = Points | | | | | | | |
| Under Attic 2184.8 2.05 4478.8 | 1. Under Attic 30.0 2184.8 2.05 X 1.00 4478.8 | | | | | | | |
| Base Total: 2184.8 4478.8 | As-Built Total: 2184.8 4478.8 | | | | | | | |
| FLOOR TYPES Area X BWPM = Points | Type R-Value Area X WPM = Points | | | | | | | |
| Slab 300.0(p) 8.9 2670.0 Raised 0.0 0.00 0.0 | 1. Slab-On-Grade Edge Insulation 0.0 300.0(p 18.80 5640.0 | | | | | | | |
| Base Total: 2670.0 | As-Built Total: 300.0 5640.0 | | | | | | | |
| INFILTRATION Area X BWPM = Points | Area X WPM = Points | | | | | | | |
| 2184.8 -0.59 -1289.0 | 2184.8 -0.59 -1289.0 | | | | | | | |

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

| | BASE | | AS-BUILT | | | | | | | |
|--------------------------|------------------------|-------------------|--|--|--|--|--|--|--|--|
| Winter Base | Points: | 21807.7 | Winter As-Built Points: 19334.7 | | | | | | | |
| Total Winter X Points | System = Multiplier | Heating Points | Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU) | | | | | | | |
| 21807.7 | 0.5540 | 12081.5 | (sys 1: Electric Heat Pump 32000 btuh ,EFF(8.5) Ducts:Con(S),Con(R),Int(AH),R6.0 19334.7 1.000 (1.000 x 1.169 x 0.93) 0.401 0.950 8011.1 19334.7 1.00 1.087 0.401 0.950 8011.1 | | | | | | | |

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FL, PERMIT #:

| | BASE | AS-BUILT | | | | | | | | | | |
|------------------------------------|------|------------|---|--------|----------------|-------|--------------------|---|-----------------|------------|------------|---------|
| WATER HEA Number of Bedrooms | X | Multiplier | = | Total | Tank Volume | EF | Number of Bedrooms | х | Tank X Ratio | Multiplier | X Credit = | = Total |
| 3 | | 2635.00 | | 7905.0 | 50.0 | 0.94 | 3 | | 1.00 | 2578.94 | 1.00 | 7736.8 |
| | | | | | As-Built To | otal: | | | | | | 7736.8 |

| | CODE COMPLIANCE STATUS | | | | | | | | | | | | |
|-------------------|------------------------|-------------------|---|---------------------|---|-----------------|-------------------|----------|-------------------|---|---------------------|---|-----------------|
| | BASE | | | | | | | AS-BUILT | | | | | |
| Cooling Points | + | Heating Points | + | Hot Water Points | = | Total Points | Cooling Points | + | Heating Points | + | Hot Water Points | = | Total Points |
| 8454 | | 12081 | | 7905 | | 28441 | 4837 | | 8011 | | 7737 | | 20585 |

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

| ADDRESS I I SI SI | |
|---------------------------|----------------|
| ADDRESS: , Lake City, FL, | PERMIT #: |
| 18 M.S. M | i Citati i ii. |

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

| COMPONENTS | SECTION | REQUIREMENTS FOR EACH PRACTICE | CHECK |
|-------------------------------|-----------------|--|----------|
| Exterior Windows & Doors | 606.1.ABC.1.1 | Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area. | - GINEON |
| Exterior & Adjacent Walls | 606.1.ABC.1.2.1 | Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate. | |
| Floors | 606.1.ABC.1.2.2 | Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams. | |
| Ceilings | 606.1.ABC.1.2.3 | Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams. | |
| Recessed Lighting Fixtures | 606.1.ABC.1.2.4 | Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested. | |
| Multi-story Houses | 606.1.ABC.1.2.5 | Air barrier on perimeter of floor cavity between floors. | |
| Additional Infiltration reqts | 606.1.ABC.1.3 | Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air. | |

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

| COMPONENTS | SECTION | REQUIREMENTS | CHECK |
|--------------------------|--------------|--|-------|
| Water Heaters | 612.1 | Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required. | |
| Swimming Pools & Spas | 612.1 | Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%. | |
| Shower heads | 612.1 | Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG. | |
| Air Distribution Systems | 610.1 | All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation. | |
| HVAC Controls | 607.1 | Separate readily accessible manual or automatic thermostat for each system. | |
| Insulation | 604.1, 602.1 | Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11. | |

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 89.6

The higher the score, the more efficient the home.

, , Lake City, FL,

| 1. | New construction or existing | New | | 12 | Cooling systems | | |
|-------|--|-----------------------------------|---------------|-------|--|-------------------|----|
| 2. | Single family or multi-family | Single family | - | | Central Unit | C 22 0 LD+ 4 | 2 |
| | Number of units, if multi-family | omgie ranniy | _ | a. | Central Onit | Cap: 32.0 kBtu/hr | |
| 4. | Number of Bedrooms | 3 | _ | h | N/A | SEER: 13.00 | _ |
| 5. | Is this a worst case? | No |) <u>U.S.</u> | . 0. | IVA | | _ |
| 6. | Conditioned floor area (ft²) | 2184.8 ft² | | | N/A | | - |
| 7. | Glass type 1 and area: (Label reqd. | by 13-104 4 5 if not default) | | C. | IVA | | _ |
| | U-factor: | Description Area | | 12 | Hasting work | 76.7 | _ |
| | (or Single or Double DEFAULT) | 72 (DLL D.C. IV. 194 9 92 | | | Heating systems | | |
| b | SHGC: | 7a. (Dole Default) 184.0 ft | _ | a, | Electric Heat Pump | Cap: 32.0 kBtu/hr | _ |
| | (or Clear or Tint DEFAULT) | 7b. (Clear) 184 0 ft ² | | h | NI/A | HSPF: 8.50 | _ |
| 8 | Floor types | 7b. (Clear) 184.0 ft ² | _ | D. | N/A | | _ |
| | Slab-On-Grade Edge Insulation | R=0.0, 300.0(p) ft | | | NI/A | | _ |
| | N/A | K-0.0, 300.0(p) It | | C. | N/A | | _ |
| | N/A | | - | 14 | TT | | - |
| | Wall types | | - | | Hot water systems | 7020 N23000 N24 | |
| | Face Brick, Wood, Exterior | B-12 0 1020 5 63 | | a. | Electric Resistance | Cap: 50.0 gallons | |
| | N/A | R=13.0, 1929.5 ft ² | | 10.0 | 37/4 | EF: 0.94 | - |
| | N/A | | _ | b. | N/A | | _ |
| | N/A | | - | | | | _ |
| | N/A | | - | C. | Conservation credits | | _ |
| | Ceiling types | | _ | | (HR-Heat recovery, Solar | | |
| | Under Attic | D 200 2104022 | | | DHP-Dedicated heat pump) | | |
| | N/A | R=30.0, 2184.8 ft ² | - | 15. | HVAC credits | PT, CF, | _ |
| | N/A | | _ | | (CF-Ceiling fan, CV-Cross ventilation, | | |
| | Ducts | | _ | | HF-Whole house fan, | | |
| | | C D (0.0000 | | | PT-Programmable Thermostat, | | |
| | Sup: Con. Ret: Con. AH: Interior | Sup. R=6.0, 80.0 ft | - | | MZ-C-Multizone cooling, | | |
| D. | N/A | | _ | | MZ-H-Multizone heating) | | |
| | | | | | | | |
| I cer | tify that this home has complie | ed with the Florida Energy | Effici | encv | Code For Building | | |
| Cons | struction through the above end | ergy saving features which | will k | e inc | talled (or exceeded) | OF THE STATE | |
| in th | is home before final inspection | 1 Otherwise a new EDI T | Dienlar | Com | d will be completed | 3 O | A |
| hase | d on installed Code compliant | features | Jispiay | Car | d will be completed | 13/12/1 | 13 |
| | 1. The second se | | | | | 3 | Z |
| Bull | der Signature: | | Date: | | | 15 | DA |
| Addı | ress of New Home: | | City/F | L Zi | p: | | |
| | | | | | | OD WE TRANSPORT | |
| IVO | 1 L. The nome's estimated ener | rgy perjormance score is | only av | vaila | ble through the FLA/RES compute | er program. | |

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTMdesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5)

BUILDING INPUT SUMMARY REPORT

| | | Title | И Б | | | | | | INVESTIGATION OF THE PARTY. | | | | | |
|----------|-----|------------------------------|--|----------------------|------------------|-------------------------------------|---------------|---|-----------------------------|---------------------------|----------------------|-------------------------|--------------------|------------------|
| ١. | | Title: | Kneppar Re | esidence | | Family | | | Single | | Address Ty | | Address | 3 |
| PROJECT | | Owner: # of Units: | (blank) | | | New/E | | : | New | | Lot #: | N/A | | |
| | | C-7 | 1 | | | Bedro | | 6 | 3 | | Subdivisio | 53.53 | | |
| Ιď | | Builder Name: | | | | | | | 2184.8 | | Platbook: | N/A | | |
| Š. | 1 | Climate: | North | | | Total S | | | 1 | | Street: | (blank) | | |
| ۱" | | | Lake City | | | Worst | | | No | | County: | Columi | bia | |
| | | Jurisdiction #: | (blank) | N. | | Rotate | Angle | : | (blank) | | City, St, Zip | : Lake C | ity, FL, | |
| S | # | Floor Type | | R-Val | Area/Per | imeter Ur | nits | S | # | Door Type | Orienta | tion Ar | ea | Units |
| 16 | 1 | Slab-On-Grade Ed | dge Insulation | 0.0 | 300.0(p) f | ft 1 | | DOORS | 1 | Insulated | Exterior | | 3.0 ft² | 1 |
| ١ŏ | | | | | | | | 18 | 2 | Insulated Insulated | Exterior Exterior | | 1.8 ft² 1.5 ft² | 1 |
| FLOORS | | | | | | | | lβ | | | Exterior | ,,, | | |
| | # | Ceiling Type | | D.Val. Av | | Dana A | 11-9- | - | | | | | 15 228 | |
| CEILINGS | 1 | Under Attic | | 30.0 21 | 200 | Base Area 2184.8 ft ² | - minore well | O | | stem Type | | | iency | Capacity |
| ١× | | Onder Attic | | 30.0 21 | 04.01[- 2 | 2104.0 11- | 1 | Z | 1 Centr | al Unit | | SEE | R: 13.00 | 32.0 kBtu/hr |
| | | | | | | | | ᅵᅥ | | | | | | |
| I III | | | | | | | | OOLING | | | | | | |
| ٥ | Cre | edit Multipliers: No | one | | | | | ŭ | Credit I | Multipliers: | Ceil Fn, PT | | | |
| at- | # | Wall Type | | Location | R-Val | Area | Units | | # Svs | stem Type | | Effic | iency | Capacity |
| ဖြ | 1 | Face Brick - Wood | | Exterior | 13.0 | 1929.5 ft² | 1 | HEATING | _ | ic Heat Pump |) | | F: 8.50 | 32.0 kBtu/hr |
| WALLS | | | | | | | | ΙÉ | | | | 1101 | . 0.00 | OZ.O KDIU/III |
| ¥ | | | | | | | | 4 | | | | | | |
| 3 | | | | | | | | | | | | | | |
| | | | | | | | | | Credit M | Multipliers: | PT | | | |
| | # | Panes Tint | Ornt | Area Ol | H Length | OH Hgh | t Unite | | # Suj | oply Return | n Air Hai | ndler Suppl on R-Val | у | Supply Length |
| | 1 | Double Clear | N | 24.0 ft² | | 10.0 f | t 1 | က | 1 Cor | | Interior | | | 80.0 ft |
| | 2 | Double Clear Double Clear | | 16.0 ft² | 1.0 ft | 6.0 f | | DUCTS | | | | | | 00.0 10 |
| | 4 | Double Clear | S | 18.0 ft² 18.0 ft² | 6.0 ft 1.0 ft | 10.0 ft 6.0 ft | | | | | | | | |
| | 5 | Double Clear | E | 18.0 ft ² | 1.0 ft | 6.0 f | | | Candia | Multipliers: 1 | · · | | | |
| | | | | | | | | _ | | 1.1103-14-07/1103-144-0-1 | | | | |
| | | | | | | | | 22 | | tem Type | | CHANGE CO. | vation Ty | |
| | | | | | | | | ▮≝ | 1 Ele | ctric Resistan | ce 0.94 5 | 0.0 None | | 0.00 |
| | | | | | | | | WATER | | | | | | |
| | | | | | | | | 5 | | | | | | |
| S | | | | | | | | | # U | se Default? | Annual O | perating Cost | Electr | ic Rate |
| | | | | | | | | œ | 1 Y | es | N/A | | N/A | |
| 8 | | | | | | | | <u> </u> | | | | | 0.00000 | |
| Z | | | | | | | | REFR. | | | | | | |
| WINDOW | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | Rater Name: | | CodeOnlyPr | ro Cl | ass #: | | | | 3 | | Daal Of | a. ^ | |
| | | Rater Certificatio | | CodeOnlyPi | | ass #: ict Leaka | ne Tue | ۵. | | 0.00 | | Pool Size | | 10 5- |
| SC | | Area Under Fluor | or of the same of |).0 0.0 | | | 51 | | | N/A | | Pump Si | | Co Co military |
| MISC | | Area Under Incar | New York of the Party of the Pa | | | sible Duc | | | | N/A | | Dryer Ty | | |
| _ | | NOTE: Not all Ra | | | | ak Free D | | | | | | Stove Ty | | ectric |
| | - | | ang inio si | OWII - | | RV/ERV S | | | | No | | Avg Ceil | Hgt: | |
| | | | | | - nore | | 01/1/00 | | 1000 | III.A E | | | | |

EnergyGauge® (Version: FLRCPB v4.5)

Residential System Sizing Calculation

Summary

Project Title: Kneppar Residence

Code Only Professional Version Climate: North

Lake City, FL

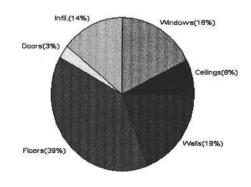
6/17/2008

| | | | | 0/1//200 | _ | | | |
|--|--------------|---------------|------------------------------------|-----------|-------|--|--|--|
| Location for weather data: Gaine | sville - Def | aults: Latitu | ude(29) Altitude(152 ft.) Temp Ran | ge(M) | | | | |
| Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.) | | | | | | | | |
| Winter design temperature | 33 | F | Summer design temperature | 92 | F | | | |
| Winter setpoint | 70 | F | Summer setpoint | 75 | F | | | |
| Winter temperature difference | 37 | F | Summer temperature difference | 17 | F | | | |
| Total heating load calculation | 33575 | Btuh | Total cooling load calculation | 17765 | Btuh | | | |
| Submitted heating capacity | % of calc | Btuh | Submitted cooling capacity | % of calc | Btuh | | | |
| Total (Electric Heat Pump) | 95.3 | 32000 | Sensible (SHR = 0.75) | 157.5 | 24000 | | | |
| Heat Pump + Auxiliary(0.0kW) | 95.3 | 32000 | Latent | 316.3 | 8000 | | | |
| | | | Total (Electric Heat Pump) | 180.1 | 32000 | | | |

WINTER CALCULATIONS

Winter Heating Load (for 2185 sqft)

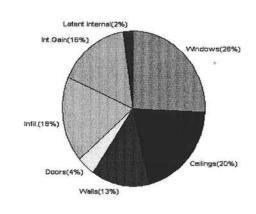
| Load component | | | Load | |
|-----------------|------|------|-------|------|
| Window total | 184 | sqft | 5923 | Btuh |
| Wall total | 1930 | sqft | 6337 | Btuh |
| Door total | 71 | sqft | 923 | Btuh |
| Ceiling total | 2185 | sqft | 2574 | Btuh |
| Floor total | 300 | sqft | 13098 | Btuh |
| Infiltration | 117 | cfm | 4720 | Btuh |
| Duct loss | | | 0 | Btuh |
| Subtotal | | | 33575 | Btuh |
| Ventilation | 0 | cfm | 0 | Btuh |
| TOTAL HEAT LOSS | | | 33575 | Btuh |



SUMMER CALCULATIONS

Summer Cooling Load (for 2185 sqft)

| Load component | | | Load | |
|---------------------------|------|------|-------|------|
| Window total | 184 | sqft | 4578 | Btuh |
| Wall total | 1930 | sqft | 2398 | Btuh |
| Door total | 71 | sqft | 699 | Btuh |
| Ceiling total | 2185 | sqft | 3618 | Btuh |
| Floor total | | | 0 | Btuh |
| Infiltration | 58 | cfm | 1084 | Btuh |
| Internal gain | | | 2860 | Btuh |
| Duct gain | | | 0 | Btuh |
| Sens. Ventilation | 0 | cfm | 0 | Btuh |
| Total sensible gain | | | 15236 | Btuh |
| Latent gain(ducts) | | | 0 | Btuh |
| Latent gain(infiltration) | | | 2129 | Btuh |
| Latent gain(ventilation) | | | 0 | Btuh |
| Latent gain(internal/occ | 400 | Btuh | | |
| Total latent gain | | | 2529 | Btuh |
| TOTAL HEAT GAIN | | | 17765 | Btuh |



Version 8
For Florida residences only

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Project Title: Kneppar Residence

Professional Version

Lake City, FL

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

6/17/2008

Component Loads for Whole House

| Window | Panes/SHGC/Frame/U | Orientation | Area(sqft) X | HTM= | Load |
|--------------|-----------------------------|-------------|---------------------|--------------|------------|
| 1 | 2, Clear, Metal, 0.87 | N | 24.0 | 32.2 | 773 Btuh |
| 2 | 2, Clear, Metal, 0.87 | N | 16.0 | 32.2 | 515 Btuh |
| 3 | 2, Clear, Metal, 0.87 | S | 54.0 | 32.2 | 1738 Btuh |
| 4 | 2, Clear, Metal, 0.87 | S | 54.0 | 32.2 | 1738 Btuh |
| 5 | 2, Clear, Metal, 0.87 | E | 36.0 | 32.2 | 1159 Btuh |
| | Window Total | | 184(sqft) | | 5923 Btuh |
| Walls | Туре | R-Value | Area X | HTM= | Load |
| 1 | Face Brick - Wood - Ext(0.0 | 9) 13.0 | 1930 | 3.3 | 6337 Btuh |
| | Wall Total | | 1930 | | 6337 Btuh |
| Doors | Туре | | Area X | HTM= | Load |
| 1 | Insulated - Exterior | | 18 | 12.9 | 239 Btuh |
| 2 | Insulated - Exterior | | 20 | 12.9 | 256 Btuh |
| 3 | Insulated - Exterior | | 33 | 12.9 | 427 Btuh |
| | Door Total | | 71 | | 923Btuh |
| Ceilings | Type/Color/Surface | R-Value | Area X | HTM= | Load |
| 1 | Vented Attic/D/Shin | 30.0 | 2185 | 1.2 | 2574 Btuh |
| | Ceiling Total | | 2185 | | 2574Btuh |
| Floors | Туре | R-Value | Size X | HTM= | Load |
| 1 | Slab On Grade | 0 | 300.0 ft(p) | 43.7 | 13098 Btuh |
| | Floor Total | | 300 | | 13098 Btuh |
| | | | Envelope Su | ıbtotal: | 28855 Btuh |
| Infiltration | Туре | ACH X Volu | ume(cuft) walls(sqf | t) CFM= | |
| | Natural | 0.32 | 21848 1930 | 116.5 | 4720 Btuh |
| Ductload | | | (D | LM of 0.000) | 0 Btuh |
| All Zones | | Sens | sible Subtotal Al | l Zones | 33575 Btuh |

| ALS |
|------|
| ALU: |
| |

| Subtotal Sensible Ventilation Sensible Total Btuh Loss | 33575 Btuh 0 Btuh |
|--|----------------------|
| Total Bluff Loss | 33575 Btuh |

Manual J Winter Calculations

Residential Load - Component Details (continued) Project Title:

Lake City, FL

Kneppar Residence

Code Only Professional Version Climate: North

6/17/2008

| EQUIPMENT | |
|-----------|--|
| | |

1. Electric Heat Pump

#

32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Project Title: Kneppar Residence

Professional Version

Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

6/17/2008

Component Loads for Zone #1: Main

| Window | Panes/SHGC/Frame/U | Orientation | Area(sqft) X | HTM= | Load |
|--------------|---------------------------|------------------|---------------------|--------------|------------|
| 1 | 2, Clear, Metal, 0.87 | N | 24.0 | 32.2 | 773 Btuh |
| 2 | 2, Clear, Metal, 0.87 | N | 16.0 | 32.2 | 515 Btuh |
| 3 | 2, Clear, Metal, 0.87 | S | 54.0 | 32.2 | 1738 Btuh |
| 4 | 2, Clear, Metal, 0.87 | S | 54.0 | 32.2 | 1738 Btuh |
| 5 | 2, Clear, Metal, 0.87 | E | 36.0 | 32.2 | 1159 Btuh |
| | Window Total | | 184(sqft) | | 5923 Btuh |
| Walls | Туре | R-Value | Area X | HTM= | Load |
| 1 | Face Brick - Wood - Ext(0 | .09) 13.0 | 1930 | 3.3 | 6337 Btuh |
| | Wall Total | | 1930 | 5500-96 | 6337 Btuh |
| Doors | Туре | | Area X | HTM= | Load |
| 1 | Insulated - Exterior | | 18 | 12.9 | 239 Btuh |
| 2 | Insulated - Exterior | | 20 | 12.9 | 256 Btuh |
| 3 | Insulated - Exterior | | 33 | 12.9 | 427 Btuh |
| | Door Total | | 71 | | 923Btuh |
| Ceilings | Type/Color/Surface | R-Value | Area X | HTM= | Load |
| 1 | Vented Attic/D/Shin | 30.0 | 2185 | 1.2 | 2574 Btuh |
| | Ceiling Total | | 2185 | | 2574Btuh |
| Floors | Туре | R-Value | Size X | HTM= | Load |
| 1 | Slab On Grade | 0 | 300.0 ft(p) | 43.7 | 13098 Btuh |
| | Floor Total | | 300 | | 13098 Btuh |
| | | Z | one Envelope Su | ubtotal: | 28855 Btuh |
| Infiltration | Туре | ACH X Volu | ume(cuft) walls(sqf | t) CFM= | |
| | Natural | 0.32 | 21848 1930 | 116.5 | 4720 Btuh |
| Ductload | Average sealed, Supply(Re | 6.0-Cond.), Retu | urn(R6.0-Cond)D | LM of 0.000) | 0 Btuh |
| Zone #1 | | Sens | sible Zone Subte | otal | 33575 Btuh |

| | | A STATE OF THE PARTY OF THE PAR | CONTRACTOR OF THE PARTY OF THE | |
|----------|-------|--|---|--|
| MA/LI/NI | E HOL | ICE TA | TAIC | |
| | | | | |

| | Subtotal Sensible Ventilation Sensible Total Btuh Loss | 33575 Btuh 0 Btuh 33575 Btuh |
|--|--|------------------------------------|
|--|--|------------------------------------|

Manual J Winter Calculations

Residential Load - Component Details (continued) Project Title:

Lake City, FL

Kneppar Residence

Code Only Professional Version Climate: North

6/17/2008

| EQUIPMENT | |
|-----------|-----|
| | 100 |

1. Electric Heat Pump

#

32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier) Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Project Title: Kneppar Residence Code Only Professional Version Climate: North

Lake City, FL

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

6/17/2008

Component Loads for Whole House

| | Type* | | Over | hang | Wine | dow Area | a(sqft) | H | MTI | Load | |
|--------------|--------------------------|------|---------|--------|---------|-------------|------------|----------|-----------|-------|------|
| Window | Pn/SHGC/U/InSh/ExSh/IS | Ornt | Len | Hgt | Gross | Shaded | Unshaded | Shaded | Unshaded | | |
| 1 | 2, Clear, 0.87, B-D, N,F | N | 110ft | 10ft. | 24.0 | 0.0 | 24.0 | 19 | 19 | 448 | Btuh |
| 2 | 2, Clear, 0.87, B-D, N,F | N | 1ft. | 6ft. | 16.0 | 0.0 | 16.0 | 19 | 19 | 299 | Btuh |
| 3 | 2, Clear, 0.87, B-D, N,F | S | 6ft. | 10ft. | 54.0 | 54.0 | 0.0 | 19 | 23 | 1009 | Btuh |
| 4 | 2, Clear, 0.87, B-D, N,F | S | 1ft. | 6ft. | 54.0 | 54.0 | 0.0 | 19 | 23 | 1009 | Btuh |
| 5 | 2, Clear, 0.87, B-D, N,F | E | 1ft. | 6ft. | 36.0 | 5.0 | 31.0 | 19 | 55 | 1813 | Btuh |
| | Window Total | | | | 184 (| sqft) | | | | 4578 | Btuh |
| Walls | Type | | R-Va | alue/L | I-Value | Area | (sqft) | | HTM | Load | |
| 1 | Face Brick - Wood - Ext | | | 13.0/ | 0.09 | 192 | 9.5 | | 1.2 | 2398 | Btuh |
| | Wall Total | | | | | 193 | 0 (sqft) | | | 2398 | Btuh |
| Doors | Туре | | | | | Area | (sqft) | | HTM | Load | |
| 1 | Insulated - Exterior | | | | | 18 | 5 8 8 | | 9.8 | 181 | Btuh |
| 2 | Insulated - Exterior | | | | | 19 | | | 9.8 | 194 | Btuh |
| 3 | Insulated - Exterior | | | | | 33 | .0 | | 9.8 | 323 | |
| | Door Total | | | | | 7 | 1 (sqft) | | | 699 | Btuh |
| Ceilings | Type/Color/Surface | | R-Value | | Area | | | HTM | Load | | |
| 1 | Vented Attic/DarkShingle | | 30.0 | | | 218 | | | 1.7 | | Btuh |
| | Ceiling Total | | 33.0 | | | 2185 (sqft) | | | | 3618 | |
| Floors | Туре | | R-Value | | | Siz | | | HTM | Load | |
| 1 | Slab On Grade | | | 0.0 | | 30 | 00 (ft(p)) | | 0.0 | 0 | Btuh |
| | Floor Total | | | | | | 0 (sqft) | | | | Btuh |
| | | | | | | | 0 (04.1) | | | | Dian |
| | | | | | | Er | velope : | Subtota | : | 11292 | Btuh |
| Infiltration | Type | | Α | СН | Volum | e(cuft) v | vall area | (saft) | CFM= | Load | |
| | SensibleNatural | | | 0.16 | | 21848 | 1930 | (-1) | 116.5 | 1084 | Btuh |
| Internal | | (| Occup | ants | | Btuh/oc | cupant | 1 | Appliance | Load | |
| gain | | | • | 2 | | X 23 | | | 2400 | 2860 | Btuh |
| | | | | | | Se | ensible E | nvelope | Load: | 15236 | Btuh |
| Duct load | | | | | | | (DGI | M of 0.0 | 00) | 0 | Btuh |
| | | | | | | Ser | sible Lo | oad All | Zones | 15236 | Btuh |

Manual J Summer Calculations

Residential Load - Component Details (continued) Project Title:

Kneppar Residence Lake City, FL

Code Only Professional Version Climate: North

6/17/2008

WHOLE HOUSE TOTALS

| | Sensible Envelope Load All Zones | 15236 | Btuł |
|---------------------------|---|-------|------|
| | Sensible Duct Load | 0 | Btuh |
| | Total Sensible Zone Loads | 15236 | Btul |
| | Sensible ventilation | 0 | Btuh |
| | Blower | 0 | Btuh |
| Whole House | Total sensible gain | 15236 | Btul |
| Totals for Cooling | Latent infiltration gain (for 54 gr. humidity difference) | 2129 | Btuh |
| | Latent ventilation gain | 0 | Btuh |
| | Latent duct gain | 0 | Btuh |
| | Latent occupant gain (2 people @ 200 Btuh per person) | 400 | Btuh |
| | Latent other gain | 0 | Btuh |
| | Latent total gain | 2529 | Btul |
| | TOTAL GAIN | 17765 | Btul |

| EQUIPMENT | | |
|-----------------|---|------------|
| 1. Central Unit | # | 32000 Btuh |

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Project Title: Kneppar Residence

Code Only
Professional Version

Climate: North

Lake City, FL

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

6/17/2008

Component Loads for Zone #1: Main

| | Type* | | Over | hang | Wind | dow Area | a(sqft) | H | ITM | Load | |
|---|--------------------------|---------|---------|--------|------------|----------|------------|----------|-----------|---|------|
| Window | Pn/SHGC/U/InSh/ExSh/IS | Ornt | Len | Hgt | Gross | Shaded | Unshaded | Shaded | Unshaded | | |
| 1 | 2, Clear, 0.87, B-D, N,F | N | 110ft | 10ft. | 24.0 | 0.0 | 24.0 | 19 | 19 | 448 | Btuh |
| 2 | 2, Clear, 0.87, B-D, N,F | N | 1ft. | 6ft. | 16.0 | 0.0 | 16.0 | 19 | 19 | 299 | Btuh |
| 3 | 2, Clear, 0.87, B-D, N,F | S | 6ft. | 10ft. | 54.0 | 54.0 | 0.0 | 19 | 23 | 1009 | |
| 4 | 2, Clear, 0.87, B-D, N,F | S | 1ft. | 6ft. | 54.0 | 54.0 | 0.0 | 19 | 23 | 1009 | Btuh |
| 5 | 2, Clear, 0.87, B-D, N,F | E | 1ft. | 6ft. | 36.0 | 5.0 | 31.0 | 19 | 55 | 1.5 | Btuh |
| | Window Total | | | | 184 (| sqft) | | | | 4578 | Btuh |
| Walls | Туре | | R-Va | alue/U | I-Value | Area | (sqft) | | HTM | Load | |
| 1 | Face Brick - Wood - Ext | | | 13.0/ | 0.09 | 192 | 29.5 | | 1.2 | 2398 | Btuh |
| | Wall Total | | | | | 193 | 0 (sqft) | | | 2398 | Btuh |
| Doors | Туре | | | | | Area | (sqft) | | HTM | Load | |
| 1 | Insulated - Exterior | | | | | 18 | 3.5 | | 9.8 | 181 | Btuh |
| 2 | Insulated - Exterior | | | | | 19 | 9.8 | | 9.8 | 194 | Btuh |
| 3 | Insulated - Exterior | | | | | 33 | 3.0 | | 9.8 | | Btuh |
| | Door Total | | | | | 7 | 1 (sqft) | | | 699 | Btuh |
| Ceilings | Type/Color/Surface | | R-Value | | Area(sqft) | | | HTM | Load | | |
| 1 | Vented Attic/DarkShingle | | 30.0 | | 2184.8 | | | 1.7 | 3618 | Btuh | |
| | Ceiling Total | | 2185 (| | 5 (sqft) | | 5-24 | 3618 | Btuh | | |
| Floors | Туре | | R-Va | alue | | | ze | | HTM | Load | |
| 1 | Slab On Grade | | | 0.0 | | 30 | 00 (ft(p)) | | 0.0 | 0 | Btuh |
| | Floor Total | | | | | 300 | .0 (sqft) | | | 0 | Btuh |
| | | | | | | Z | one Enve | elope Si | ubtotal: | 11292 | Btuh |
| Infiltration | Type | | Δ | CH | Volum | e(cuft) | wall area | (eaft) | CFM= | Load | |
| iiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiii | SensibleNatural | | , | 0.16 | VOIGIT | 21848 | 1930 | (Sqrt) | 58.3 | (Table 1) (1) (1) (1) (1) (1) (1) (1) (1) (1) | Btuh |
| Internal | | | Occup | | | | cupant | | Appliance | Load | |
| gain | | | | 2 | | X 23 | | | 2400 | 2860 | Btuh |
| | | | | | | S | ensible E | Envelop | e Load: | 15236 | Btuh |
| Duct load | Average sealed, Supply | /(R6.0- | Cond. |), Ret | urn(R6 | .0-Cond | 1) | (DGM | of 0.000) | 0 | Btuh |
| | | | | | | | Sensib | ole Zone | Load | 15236 | Btuh |

Manual J Summer Calculations

Residential Load - Component Details (continued)

Lake City, FL

Project Title: Kneppar Residence

Code Only Professional Version Climate: North

6/17/2008

WHOLE HOUSE TOTALS

| | Sensible Envelope Load All Zones | 15236 | Btuh |
|---------------------------|---|-------|------|
| | Sensible Duct Load | 0 | Btuh |
| | Total Sensible Zone Loads | 15236 | Btuh |
| | Sensible ventilation | 0 | Btuh |
| | Blower | 0 | Btuh |
| Whole House | Total sensible gain | 15236 | Btuh |
| Totals for Cooling | Latent infiltration gain (for 54 gr. humidity difference) | 2129 | Btuh |
| | Latent ventilation gain | 0 | Btuh |
| 8 | Latent duct gain | 0 | Btuh |
| | Latent occupant gain (2 people @ 200 Btuh per person) | 400 | Btuh |
| | Latent other gain | 0 | Btuh |
| | Latent total gain | 2529 | Btuh |
| | TOTAL GAIN | 17765 | Btuh |

| EQUIPMENT | | |
|-----------------|---|------------|
| 1. Central Unit | # | 32000 Btuh |

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

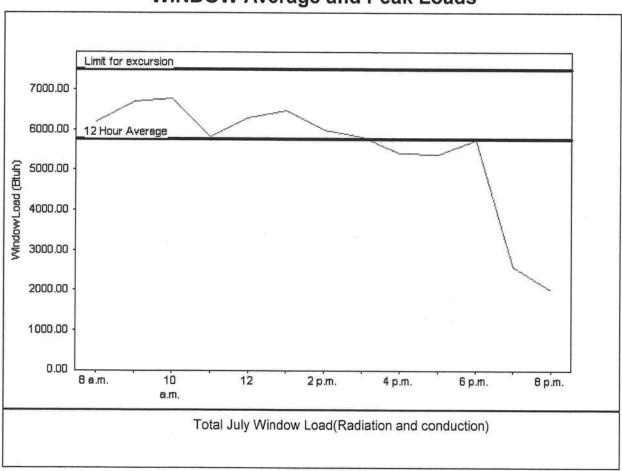
Project Title: Kneppar Residence Code Only Professional Version Climate: North

6/17/2008

Lake City, FL

| Weather data for: Gainesville - Defaults | | | | | | |
|--|----|-------|------------------------------|-----------|--|--|
| Summer design temperature | 92 | F | Average window load for July | 5769 Btuh | | |
| Summer setpoint | 75 | F | Peak window load for July | 6784 Btuh | | |
| Summer temperature difference | 17 | F | Excusion limit(130% of Ave.) | 7500 Btuh | | |
| Latitude | 29 | North | Window excursion (July) | None | | |

WINDOW Average and Peak Loads



The midsummer window load for this house does not exceed the window load excursion limit. This house has adequate midsummer window diversity.

EnergyGauge® System Sizing for Florida residences only PREPARED BY: A Company of the Company of

DATE: 0-17-0

EnergyGauge® FLRCPB v4.5



Energy Code + manual J

Summary Energy Code Results

Residential Whole Building Performance Method A

Lake City, FL

Project Title: Kneppar Residence

Code Only Professional Version Climate: North

6/17/2008

| Building Loads | | | | | | |
|----------------|--------------|------------|--------------|--|--|--|
| В | ase | As-Built | | | | |
| Summer: | 26012 points | Summer: | 19749 points | | | |
| Winter: | 21808 points | Winter: | 19335 points | | | |
| Hot Water: | 7273 points | Hot Water: | 7273 points | | | |
| Total: | 55093 points | Total: | 46356 points | | | |

| Energy Use | | | | | | | |
|------------|--------------|------------|--------------|--|--|--|--|
| | Base | As-Built | | | | | |
| Cooling: | 8454 points | Cooling: | 4837 points | | | | |
| Heating: | 12081 points | Heating: | 8011 points | | | | |
| Hot Water: | 7905 points | Hot Water: | 7737 points | | | | |
| Total: | 28441 points | Total: | 20585 points | | | | |

PASS e-Ratio: 0.72

EnergyGauge®(Version: FLRCPB v4.5)



PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information

about statewide product approval can be obtained at www.floridabuilding.org

| Category/Subcategory | Manufacturer | Product Description | Approval Number(s) |
|-------------------------------------|---------------|--|--------------------|
| A. EXTERIOR DOORS | Therma Tru | Door w/ Sidelite Inswing Outswi | n FL 7186.2 |
| Swinging | | The state of the s | 5 |
| 2. Sliding | | | |
| 3. Sectional | Clopary | Double Car Garage | FL 5684.2 |
| 4. Roll up | 1 1 | 0 | 10000 |
| 5. Automatic | | | |
| 6. Other | 1 | | |
| B. WINDOWS | 1 | | |
| Single hung | Kinko, Ltd | Aluminum Single Nung | FL 123.3 |
| Horizontal Slider | | 3 1143 | |
| Casement | | | |
| 4. Double Hung | | | |
| 5. Fixed | | | |
| 6. Awning | | | |
| 7. Pass -through | | | |
| Projected | | | |
| 9. Mullion | | | |
| 10. Wind Breaker | | | |
| 11 Dual Action | | | |
| 12. Other | | | |
| C. PANEL WALL | | | |
| 1. Siding | | | |
| 2. Soffits | | | |
| 3. EIFS | | | |
| 4. Storefronts | | | |
| 5. Curtain walls | | | |
| 6. Wall louver | | | |
| 7. Glass block | | | |
| 8. Membrane | | | |
| 9. Greenhouse | | | |
| 10. Other | | | |
| D. ROOFING PRODUCTS | | | |
| Asphalt Shingles | GAF MATERIALS | FIBERGLASS REINFORCED | PL 183.1 |
| 2. Underlayments | CHI HAILKING | 1 THEREGERS REGINFORCED | PL 105.1 |
| Roofing Fasteners | | | |
| Non-structural Metal Rf | | | |
| 5. Built-Up Roofing | | STY BUILDA | |
| 6. Modified Bitumen | | (8) Barri | |
| 7. Single Ply Roofing Sys | | Received for | 189 |
| Roofing Tiles | | \$ F1/ P | 730) |
| Roofing Insulation | | B FILE COP | Υ |
| 10. Waterproofing | | (O) Code | 7 |
| 11. Wood shingles /shakes | | Compliance | |
| Trood offingles foliakes | 1 | 1/0 | / |

France h

| Catego y/Subcategory (cont.) | Manufacturer | Product Description | | Approval Number(s) |
|---|---|--|--|--|
| 13. Liquid Applied Roof Sys | | | | |
| 14. Cements-Adhesives – | | | | |
| Coatings | | | | |
| 15. Roof Tile Adhesive | | | | |
| Spray Applied Polyurethane Roof | | | | |
| 17. Other | | 1 | | |
| E. SHUTTERS | | | | |
| 1. Accordion | | | | |
| 2. Bahama | | | | |
| Storm Panels | | | | |
| 4. Colonial | | | | |
| 5. Roll-up | | | | |
| 6. Equipment | | | | |
| 7. Others | | - | | |
| F. SKYLIGHTS | | + | | |
| 1. Skylight | | | | |
| 2. Other | | | | |
| G. STRUCTURAL | | | | |
| COMPONENTS | | | | |
| Wood connector/anchor | | | | |
| Truss plates | | | | |
| Truss plates Busineered lumber | | | | |
| Railing | | | | |
| 5. Coolers-freezers | | | | |
| Concrete Admixtures | | | | |
| 7. Material | | | | |
| 8. Insulation Forms | | | | |
| 9. Plastics | | | | |
| 10. Deck-Roof | | | | |
| 11. Wall | | - | | |
| 12. Sheds | | | | |
| 13. Other | | | | |
| | | | | |
| H. NEW EXTERIOR | | | | |
| 1. ENVELOPE PRODUCTS | | | | |
| 2. | + | | | • |
| The products listed below ditime of inspection of these plobsite; 1) copy of the produand certified to comply with, I understand these products | oroducts, the fo ct approval, 2) 3) copy of the | llowing information must I the performance characte applicable manufacturers | be available to the ristics which the properties in the properties in the properties of the properties | e inspector on the product was tested rements. |
| Tunderstand these products | | e removed ii approvar car | | ated during inspection |
| | | × × × × × × × × × × × × × × × × × × × | | |
| Contractor or Contractor's Authorize | Print Nan | ne | Date | |
| Location | Permit # | Permit # (FOR STAFF USE ONLY) | | |

COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006 Supplements and One (1) and Two (2) Family Dwellings

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS:

- Two (2) complete sets of plans containing the following:
- All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

Site Plan information including:

- Dimensions of lot or parcel of land
- Dimensions of all building set backs
- Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required:

- Plans or specifications must meet state compliance with FRC Chapter 3
- The following information must be shown as per section FRC
- Basic wind speed (3-second gust), miles per hour
- Wind importance factor and nature of occupancy
- Wind exposure if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.

Elevations Drawing including:

- All side views of the structure
- Roof pitch
- Overhang dimensions and detail with attic ventilation
- o Location, size and height above roof of chimneys
- o Location and size of skylights with Florida Product Approval
- Number of stories
- e) Building height from the established grade to the roofs highest peak

Floor Plan including:

- Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- 6/ All exterior and interior shear walls indicated
- Shear wall opening shown (Windows, Doors and Garage doors
- Emergency escape and rescue opening in each bedroom (net clear opening shown)
- Safety glazing of glass where needed
- Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- Plans must show and identify accessibility of bathroom (see FRC 322)

All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

Foundation Plans Per FRC 403:

- a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling.
 - d) Assumed load-bearing valve of soil (psf)
- e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

CONCRETE SLAB ON GRADE Per FRC R506

- Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports

PROTECTION AGAINST TERMITES Per FRC 320:

Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606

- Show all materials making up walls, wall height, and Block size, mortar type
- Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

- Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers
- o Girder type, size and spacing to load bearing walls, stem wall and/or priers
- Attachment of joist to girder
- Wind load requirements where applicable
- Show required under-floor crawl space
- Show required amount of ventilation opening for under-floor spaces
- Show required covering of ventilation opening.
- Show the required access opening to access to under-floor spaces
- Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- Show Draft stopping, Fire caulking and Fire blocking
- Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- Provide live and dead load rating of floor framing systems (psf).

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

ROOF SYSTEMS:

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- o Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

ROOF SHEATHING FRC Table R602,3(2) FRC 803

Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

Include all materials which will make up the roof assembles covering; with Florida Product Approval numbers for each component of the roof assembles covering.

ECB Chapter 13 Florida Energy Efficiency Code for Building Construction

- Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- Ceiling fans
- Smoke detectors
- Service panel, sub-panel, location(s) and total ampere ratings

- On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- Appliances and HVAC equipment and disconnects
- Arc Fault Circuits (AFCI) in bedrooms
- Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) <u>Notice</u>
 <u>Of Commencement is required to be filed with the building department Before Any</u>
 Inspections Will Be Done.

Private Potable Water

- Size of pump motor
- d / Size of pressure tank
- Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- <u>City Approval:</u> If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- o Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED. A development permit will also be required. The permit cost is \$50.00.
- Oriveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.



Project Information for:

L276566

Lot:

6

Subdivision:

CANNON CREEK

County:

COLUMBIA

Truss Count:

47

Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002
Truss Design Load Information:

Gravity:

Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

James H. Johnston, III Florida Registered Residential Contractor License No. RR0066976

Address: RT. #15 Box 3693 Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes

22

23

24

25

26

27

28

J1968761

J1968762

J1968763

J1968764

J1968765

J1968766

J1968767

T08

T09

T10

T11

T12

T13

T14

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

| No. | Drwg. # | Truss ID | Date | No. | Drwg. # | Truss ID | Date |
|-----|----------|----------|---------|-----|---|----------|---------|
| 1 | J1968740 | CJ1 | 5/29/08 | 29 | J1968768 | T16 | 5/29/08 |
| 2 | J1968741 | CJ1A | 5/29/08 | 30 | J1968769 | T17 | 5/29/08 |
| 3 | J1968742 | CJ1B | 5/29/08 | 31 | J1968770 | T18 | 5/29/08 |
| 4 | J1968743 | CJ3 | 5/29/08 | 32 | J1968771 | T19 | 5/29/08 |
| 5 | J1968744 | CJ3A | 5/29/08 | 33 | J1968772 | T20 | 5/29/08 |
| 6 | J1968745 | CJ5 | 5/29/08 | 34 | J1968773 | T21 | 5/29/08 |
| 7 | J1968746 | CJ5A | 5/29/08 | 35 | J1968774 | T22 | 5/29/08 |
| 8 | J1968747 | EJ6 | 5/29/08 | 36 | J1968775 | T23 | 5/29/08 |
| 9 | J1968748 | EJ7 | 5/29/08 | 37 | J1968776 | T24 | 5/29/08 |
| 10 | J1968749 | EJ7A | 5/29/08 | 38 | J1968777 | T25 | 5/29/08 |
| 11 | J1968750 | FG1 | 5/29/08 | 39 | J1968778 | T26 | 5/29/08 |
| 12 | J1968751 | HJ9 | 5/29/08 | 40 | J1968779 | T27 | 5/29/08 |
| 13 | J1968752 | HJ9A | 5/29/08 | 41 | J1968780 | T28 | 5/29/08 |
| 14 | J1968753 | HJ9B | 5/29/08 | 42 | J1968781 | T29 | 5/29/08 |
| 15 | J1968754 | T01 | 5/29/08 | 43 | J1968782 | T30 | 5/29/08 |
| 16 | J1968755 | T02 | 5/29/08 | 44 | J1968783 | T31 | 5/29/08 |
| 17 | J1968756 | T03 | 5/29/08 | 45 | J1968784 | T32 | 5/29/08 |
| 18 | J1968757 | T04 | 5/29/08 | 46 | J1968785 | T33 | 5/29/08 |
| 19 | J1968758 | T05 | 5/29/08 | 47 | J1968786 | T33G | 5/29/08 |
| 20 | J1968759 | T06 | 5/29/08 | | *************************************** | | • |
| 21 | J1968760 | T07 | 5/29/08 | | | | |

5/29/08

5/29/08

5/29/08

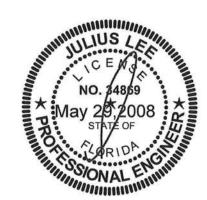
5/29/08

5/29/08

5/29/08

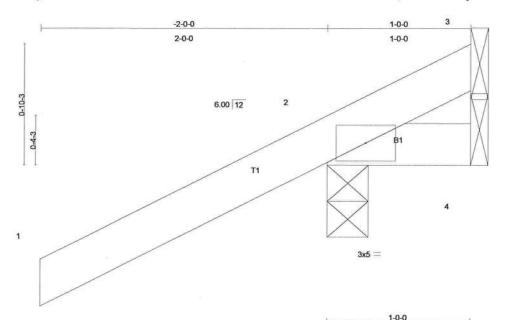
5/29/08





| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|---------------------------------------|-----|-----|-----------------------------|----------|
| L276566 | CJ1 | JACK | 10 | 1 | | J1968740 |
| | | 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:06 2008 Page 1



1-0-0 2-0-0 LOADING (psf) **SPACING** CSI DEFL (loc) I/defl ∐d **PLATES** GRIP in 20.0 TCLL 1.25 0.28 Plates Increase TC Vert(LL) -0.002 >999 360 **MT20** 244/190 TCDL 1.25 BC 0.01 2 7.0 Lumber Increase Vert(TL) -0.00>999 240 * Rep Stress Incr **BCLL** 10.0 YES **WB** 0.00 Horz(TL) 0.00 3 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-274(load case 6), 3=-90(load case 1)

Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.17

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3. Continued on page 2

Truse Design Engineer Florida PE No. 24899 1 108 Crestal Bay Blvd Boynton Beach, FL 33435

May 29,2008

Scale: 1.5"=1"

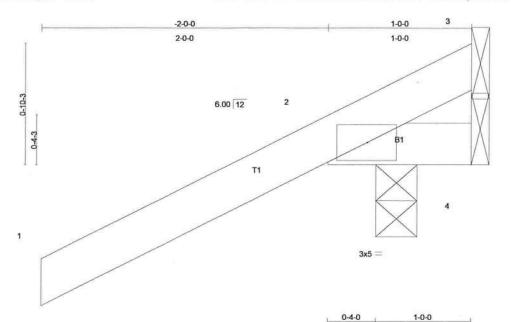
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation will be Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 121002000000000000000000000000000000000 |
|---------|-------|------------|-----|-----|-----------------------------|---|
| L276566 | CJ1A | JACK | 4 | 1 | | J1968741 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:07 2008 Page 1



2-0-0 LOADING (psf) SPACING CSI DEFL in (loc) I/defl L/d PLATES GRIP 20.0 TCLL Plates Increase 1.25 TC 0.28 Vert(LL) -0.002 >999 360 MT20 244/190 BC 2 TCDL 7.0 Lumber Increase 1.25 0.01 Vert(TL) -0.00>999 240 **BCLL** 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) 0.00 3 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 4=5/Mechanical, 2=256/0-3-8, 3=-90/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-274(load case 6), 3=-90(load case 1)

Max Grav 4=14(load case 2), 2=256(load case 1), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.17

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3. Continued on page 2

Julius Lee Truse Cesign Engineer Florida FE No. 3-1898 1-109 Crestal Bay Sive Boynton Besch, Ft. 20-405

May 29,2008

Scale: 1.5"=1"

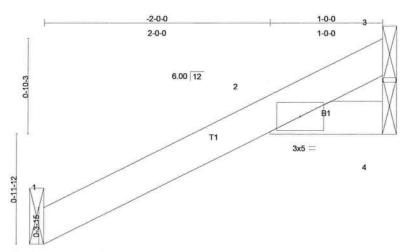
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000740 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | CJ1B | JACK | 4 | 1 | | J1968742 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:08 2008 Page 1



Structural sub-fascia beam required. Sub-fascia beam and its connections to be designed and furnished by others.

| | | 1 | | | -2-0-0 | | 1 | 1-0-0 | - | | | |
|-------------|----------|-------------------|--------|------|--------|--|-------|-------|--------|-----|--------------|---------|
| | | T | | 1 | 2-0-0 | | | 1-0-0 | | | 1 | |
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.13 | Vert(LL) | 0.02 | 1-2 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.01 | Vert(TL) | -0.02 | 1-2 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.01 | 4 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TI | PI2002 | (Mat | rix) | 00000000000000000000000000000000000000 | | | | | Weight: 7 lb | |
| LUMBE | R | | | | | BRACING | | | | | | |

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=95/Mechanical, 4=5/Mechanical, 3=89/Mechanical

Max Horz 1=67(load case 6)

Max Uplift 1=-33(load case 6), 4=-9(load case 4), 3=-74(load case 6)

Max Grav 1=95(load case 1), 4=15(load case 2), 3=89(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-62/8, 2-3=-65/40

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.01

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1, 9 lb uplift at joint 4 and 74 lb uplift at joint 3. Continued on page 2

May 29,2008

Scale = 1:10.2

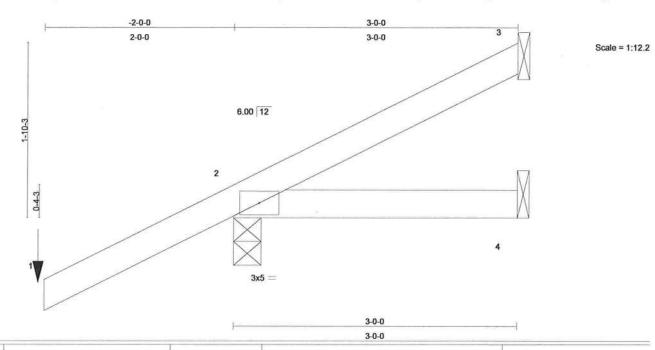
▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-97 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | DURATION / ST. ALBANS WOOD / ROOF |
|---------|-------|------------|-----|-----|--|
| | 0.10 | 14.016 | 1 | | J1968743 |
| L276566 | CJ3 | JACK | 14 | 1 | 100 May 100 Ma |
| | | | | | Job Reference (optional) |

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 15:11:06 2008 Page 1



| LOADING TCLL TCDL BCLL | G (psf) 20.0 7.0 10.0 | * | SPACING Plates Increase Lumber Increase Rep Stress Incr | 2-0-0 1.25 1.25 NO | CSI TC BC WB | 0.70 0.06 0.00 | Vert(LL) Vert(TL) Horz(TL) | in -0.00 -0.01 -0.00 | (loc) 2-4 2-4 | l/defl >999 >999 n/a | L/d 360 240 n/a | PLATES MT20 | GRIP 244/190 |
|---------------------------------|--------------------------------|---|---|---|-----------------------|----------------------|----------------------------|-------------------------------|---------------------|-------------------------------|--------------------------|----------------|---------------------|
| BCDL | 5.0 | | Code FBC2004/TI | 1000 1000 B B B B B B B B B B B B B B B | (Mati | | () | 0.00 | | | | Weight: 13 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-0-0

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=-42/Mechanical, 2=418/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-42(load case 2), 2=-286(load case 6)

Max Grav 3=22(load case 6), 2=418(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2

1-2=-33/86, 2-3=-92/10

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.23

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 3 and 286 lb uplift at joint 2.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 6) 95# concentrated load from a structural sub-fascia beam. Sub-fascia beam and its connections to be designed and furnished by others.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

May 29,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation and bracing label from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job Truss Truss Type Qty Ply RICHARD KEEN - KNEPPAR RES. J1968744 L276566 CJ3A **JACK** Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:09 2008 Page 1 -2-0-0 3 2-0-0 3-0-0 Scale = 1:12.5 6.00 12 T1 0-4-3 3x5 0-4-0 3-0-0 0-4-0 2-8-0 LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 0.29 Vert(LL) Plates Increase 1.25 TC 0.01 2-4 >999 360 **MT20** 244/190 TCDL 7.0 1.25 BC Lumber Increase 0.08 Vert(TL) -0.012-4 >999 240 * Rep Stress Incr BCLL 10.0 YES WB 0.00 Horz(TL) -0.003 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 13 lb LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)

Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.15

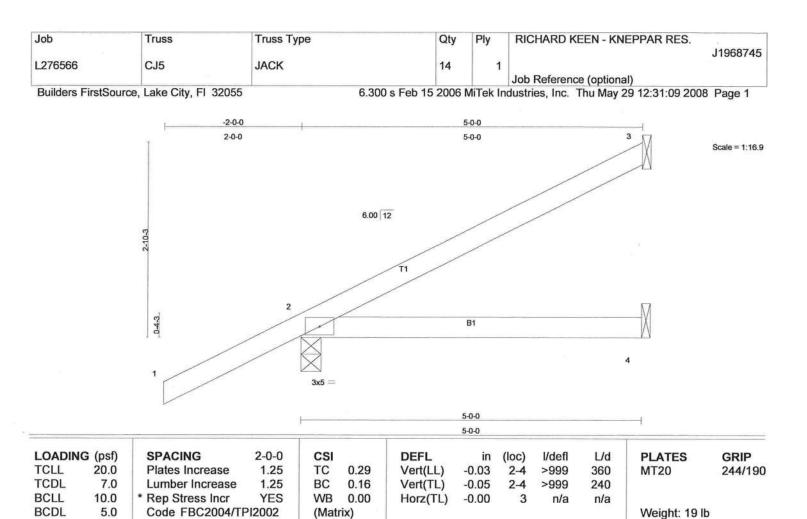
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4. Continued on page 2

May 29,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connec Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erecti and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-199(load case 6)

Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-88/36

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.17

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2. Continued on page 2

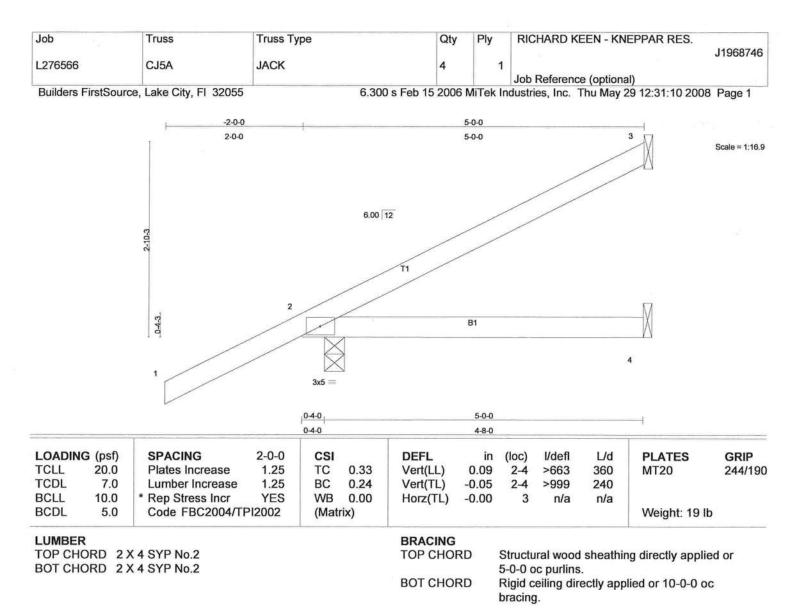
Truss Ossian Engineer Florida FE No. 24899 1 100 Crisstal Bay Blvd Boynton Beach, FL 93435

May 29,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





REACTIONS

(lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4)

Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-88/36

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.17

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

Truss Design Engineer Florida PE No. 34888 1 100 Chastal Bay Blvd Boynton Beach, FL 33435

May 29,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP/1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job Truss Type Qty Ply RICHARD KEEN - KNEPPAR RES.

L276566 EJ6 MONO TRUSS 3 1

Builders FirstSource, Lake City, FI 32055 G.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:11 2008 Page 1

2-0-0
2-0-0
6-0-0
2-0-0
2-0-0
6-0-0
2-0-0
2-0-0
6-0-0
2-0-1
2-0-0
6-0-0
2-0-1
3-0-1-12
8-1
6-0-0
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1-12
0-1

| LOADIN | G (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|--------|---------|-------------------|--------|------|------|----------|-------|-------|--------|-----|----------------|---------|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.36 | Vert(LL) | 0.16 | 2-6 | >426 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.55 | Vert(TL) | -0.09 | 2-6 | >764 | 240 | 9541.A4AC 1355 | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.04 | Horz(TL) | 0.00 | | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | PI2002 | (Mat | rix) | | | | | | Weight: 26 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=316/0-3-8, 6=163/Mechanical

Max Horz 2=201(load case 6)

Max Uplift 2=-269(load case 6), 6=-175(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-104/45, 3-4=-2/0

BOT CHORD WEBS 2-6=0/0, 5-6=0/0 3-6=-132/177

JOINT STRESS INDEX

2 = 0.18, 3 = 0.09 and 6 = 0.10

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 2 and 175 lb uplift at joint 6. Continued on page 2

Truss Design Engineer Florida FE No. 24898 1100 Chastal Bay Blvd Bovnton Beach, Et. 22427

May 29,2008

Scale = 1:21.1

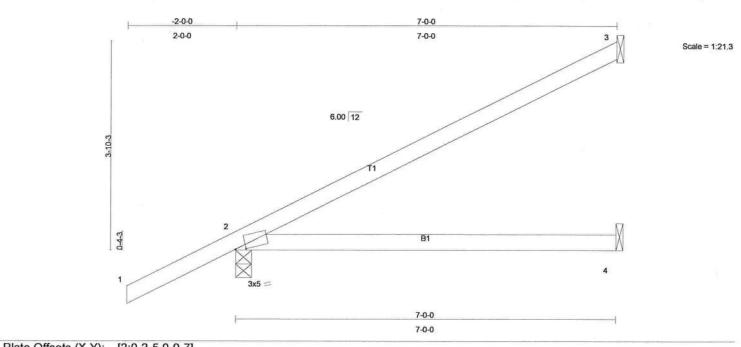
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation will be Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|-------|
| L276566 | EJ7 | MONO TRUSS | 26 | 1 | J19 | 68748 |
| LETOGGG | Lor | More mose | 20 | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:11 2008 Page 1



| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
|--------|----------|-------------------|--------|------|------|----------|-------|-------|--------|-----|---------------|---------|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.48 | Vert(LL) | -0.08 | 2-4 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.28 | Vert(TL) | -0.16 | 2-4 | >501 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | | | | | | Weight: 26 lb | |

| L | U | M | В | E | ł | ₹ |
|---|-----|------|-----|---|---|---|
| _ | 100 | 7000 | 100 | | | |

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-84(load case 6), 2=-139(load case 6)

Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-119/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.81

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb Collisions and 139 lb uplift at joint 2.

Truss Ossian Engineer Florida FE No. 24889 1100 Cassial Bay Blvd Boynton Beach, FL 35435

May 29,2008

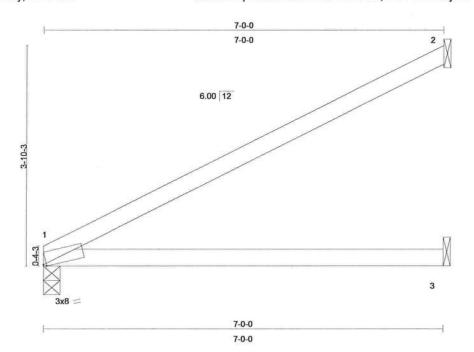
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | DURATION / ST. ALBANS WOOD / ROOF |
|-----------|---------------|------------|-----|---------|-----------------------------------|
| 100000000 | 0.571.590.964 | | | 1000100 | J1968749 |
| L276566 | EJ7A | MONO TRUSS | 4 | 1 | 2 |
| | | 35.00 | | | Job Reference (optional) |

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 14:50:02 2008 Page 1



| Plate Off | sets (X,Y) |): [1:0-0-10,Edge] | | | | | | | | | | |
|-----------|------------|--------------------|--------|-------|------|----------|-------|-------|--------|-----|--------------|---------|
| LOADING | G (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.46 | Vert(LL) | 0.15 | 1-3 | >540 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.36 | Vert(TL) | -0.18 | 1-3 | >459 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 2 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mati | rix) | | | | | | Weight: 22 I | b |

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=217/0-3-8, 2=163/Mechanical, 3=54/Mechanical

Max Horz 1=113(load case 6)

Max Uplift 1=-31(load case 6), 2=-93(load case 6)

Max Grav 1=217(load case 1), 2=163(load case 1), 3=96(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-126/58

1-3=0/0 **BOT CHORD**

JOINT STRESS INDEX

1 = 0.86

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

Design Engineer

May 29,2008

Scale = 1:20.2

Continued on page 2

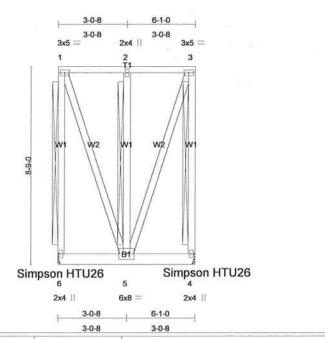
🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000750 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | FG1 | SPECIAL | 1 | 1 | 7. | J1968750 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:12 2008 Page 1



| LOADIN | G (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L∕d | PLATES | GRIP |
|--------|---------|-------------------|--------|------|------|----------|-------|-------|--------|-----|---------------|---------|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.35 | Vert(LL) | -0.01 | 5 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.15 | Vert(TL) | -0.02 | 5 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | NO | WB | 0.20 | Horz(TL) | -0.00 | 4 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TI | PI2002 | (Mat | rix) | | | | | | Weight: 84 lb | |

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD**

WEBS

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

T-Brace:

2 X 4 SYP No.3 - 1-6,

Scale = 1:51.1

3-4, 2-5

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Structural wood sheathing directly applied or

Brace must cover 90% of web length.

REACTIONS (lb/size) 6=896/Mechanical, 4=896/Mechanical Max Uplift 6=-248(load case 3), 4=-248(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-6=-575/168, 1-2=-176/49, 2-3=-176/49, 3-4=-575/168

BOT CHORD 5-6=-0/0, 4-5=-0/0

WEBS 1-5=-148/539, 2-5=-182/82, 3-5=-148/539

JOINT STRESS INDEX

1 = 0.49, 2 = 0.06, 3 = 0.49, 4 = 0.20, 5 = 0.24 and 6 = 0.20

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.

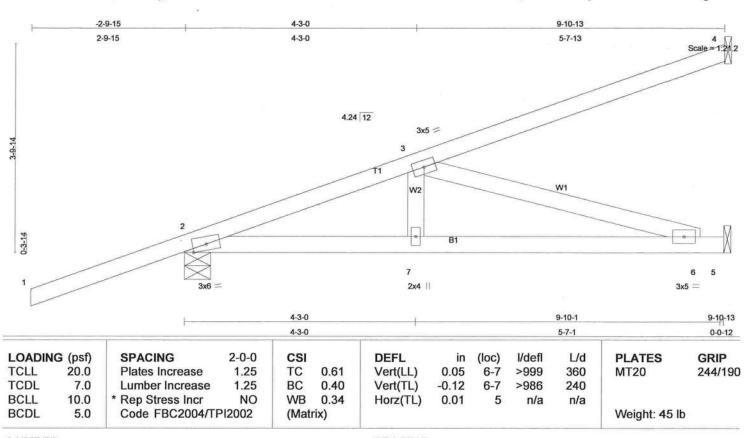
Continued on page 2

May 29,2008



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | J1968751 |
|--|-------|------------|-----|-----|-----------------------------|-----------|
| L276566 | НЈ9 | MONO TRUSS | 5 | 1 | | J 1900/51 |
| The state of the s | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:13 2008 Page 1



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical

Max Horz 2=269(load case 3)

Max Uplift 4=-232(load case 3), 2=-281(load case 3), 5=-62(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/50, 2-3=-647/120, 3-4=-105/65

BOT CHORD

2-7=-308/599, 6-7=-308/599, 5-6=0/0

WEBS

3-7=0/190, 3-6=-624/321

JOINT STRESS INDEX

2 = 0.77, 3 = 0.18, 6 = 0.21 and 7 = 0.13

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back

Continued on page 2

Truss Cesian Engineer Florida PE No. 24868 1100 Cesstal Bay Blvd Boynton Besch, FL 3343

May 29,2008

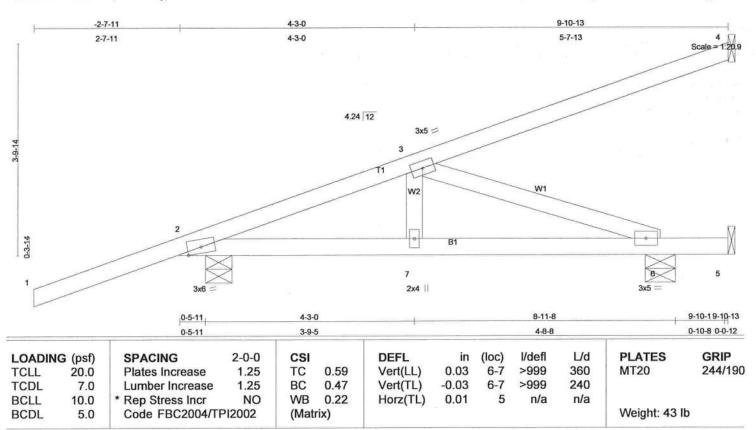
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|----------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | НЈ9А | MONO TRUSS | 2 | 1 | 4 | J1968752 |
| LL. 0000 | 1.00. | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:13 2008 Page 1



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-8-2 oc

bracing.

(lb/size) 4=265/Mechanical, 2=407/0-5-11, 5=-40/Mechanical, 6=300/0-6-7 REACTIONS

Max Horz 2=265(load case 3)

Max Uplift 4=-230(load case 3), 2=-357(load case 3), 5=-40(load case 1), 6=-241(load

Max Grav 4=265(load case 1), 2=407(load case 1), 5=52(load case 3), 6=300(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-531/270, 3-4=-105/64

BOT CHORD

2-7=-452/494, 6-7=-452/494, 5-6=0/0

WEBS

3-7=-56/116, 3-6=-522/478

JOINT STRESS INDEX

2 = 0.76, 3 = 0.22, 6 = 0.17 and 7 = 0.08

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

diss Les ries Cesian Endine londs Mil. No. 34 Mil 198 Cessial May Bly

May 29,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. This design is based only upon the parameters anown for an individual building component that is installed and in deader vertically and rationated with with expliciability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Qty Ply DURATION / ST. ALBANS WOOD / ROOF Job Truss Truss Type J1968753 L276566 HJ9B MONO TRUSS 2 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 15:12:51 2008 Page 1 -2-8-7 4-3-0 9-10-13 4-3-0 5 2-8-7 Scale = 1:20.6 4.24 12 3x5 = 3-9-14 3x5 = 3x5 5x6 = 8 2x4 || 3x5 = 4-3-0 9-10-1 9-10-13 5-7-1 2-1-7 0-1-12 1-11-13 0-0-12 Plate Offsets (X,Y): [2:0-8-6,0-1-8] **SPACING** 2-0-0 CSI L/d **PLATES** GRIP LOADING (psf) DEFL in (loc) I/defl TCLL 20.0 Plates Increase 1.25 TC 0.56 Vert(LL) 0.07 7-8 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.31 Vert(TL) -0.107 - 8>999 240 BCLL 10.0 Rep Stress Incr NO **WB** 0.24Horz(TL) 0.01 6 n/a n/a Code FBC2004/TPI2002 (Matrix) Weight: 72 lb BCDL 5.0 LUMBER BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

TOP CHORD 2 X 4 SYP No.1D

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

LBR SCAB

1-5 2 X 4 SYP No.1D one side

SLIDER

Left 2 X 8 SYP No.1D 2-6-12

REACTIONS

(lb/size) 5=271/Mechanical, 2=685/0-3-8, 6=165/Mechanical

Max Horz 2=265(load case 3)

Max Uplift 5=-235(load case 3), 2=-522(load case 3), 6=-113(load case 3) Max Grav 5=271(load case 1), 2=685(load case 1), 6=169(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-52/107, 2-3=-513/157, 3-4=-487/160, 4-5=-106/66

BOT CHORD

2-8=-297/422, 7-8=-297/422, 6-7=0/0

WEBS

4-8=-52/194, 4-7=-440/310

JOINT STRESS INDEX

Continued on DQL = 260.

1 = 0.00, 1 = 0.00, 2 = 0.40, 2 = 0.00, 2 = 0.23, 2 = 0.13, 3 = 0.00, 3 = 0.00, 4 = 0.16, 4 = 0.00, 5 = 0.00, 5 = 0.00, 7 = 0.15 and 8 = 0.00, 4 = 0.16, 40.14

NOTES

1) Attached 13-5-11 scab 1 to 5, front face(s) 2 X 4 SYP No.1D with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 1-11-15 from end at joint 1, nail 1 row(s) at 2 o.c. for 2-0-0; starting at 5-2-8 from end at joint 1, nail 1 row(s) at 7 o.c. for 3-2-0.

22425

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed; porch left exposed; Lumber DOL=1.60

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

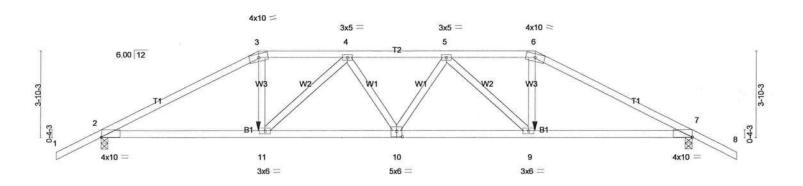
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | J1968754 |
|---------|-------|------------|-----|-----|-----------------------------|-----------|
| L276566 | T01 | HIP | 1 | 1 | | 3 1300734 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:15 2008 Page 1





| 1 | 7-0-0 | 13-2-0 | 19-4-0 | 26-4-0 | |
|---|-------|--------|--------|--------|------|
| | 7-0-0 | 6-2-0 | 6-2-0 | 7-0-0 | - (- |

| Plate Of | ffsets (X,Y | '): [2:Edge,0-0-2], [7: | Edge,0-0- | 2], [10:0 | 0-3-0,0-3 | -0] | | | | | | |
|----------|-------------|-------------------------|-----------|-----------|-----------|----------|-------|-------|--------|-----|----------------|---------|
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.50 | Vert(LL) | -0.20 | 10 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.73 | Vert(TL) | -0.39 | 10-11 | >805 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | NO | WB | 0.44 | Horz(TL) | 0.14 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TI | PI2002 | (Mat | rix) | | | | | | Weight: 124 lb | |

| LUMBER | |
|------------------|----------------|
| TOP CHORD | 2 X 4 SYP No.2 |
| BOT CHORD | 2 X 4 SYP No.2 |
| WEBS | 2 X 4 SYP No.3 |

SYP No.2 SYP No.2 SYP No.3 BRACING
TOP CHORD
Structural wood sheathing directly applied or 3-1-0 oc purlins.
BOT CHORD
Rigid ceiling directly applied or 5-11-13 oc

Rigid ceiling directly applied or 5-11-13 oc bracing.

REACTIONS (lb/size) 2=1823/0-3-8, 7=1823/0-3-8

Max Horz 2=-77(load case 6)

Max Uplift 2=-591(load case 5), 7=-591(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3352/1054, 3-4=-2948/980, 4-5=-3570/1158, 5-6=-2948/980,

6-7=-3352/1054, 7-8=0/47

BOT CHORD 2-11=-902/2904, 10-11=-1126/3529, 9-10=-1110/3529, 7-9=-869/2904

WEBS 3-11=-344/1123, 4-11=-898/367, 4-10=0/143, 5-10=0/143, 5-9=-898/367,

6-9=-344/1123

JOINT STRESS INDEX

2 = 0.76, 3 = 0.64, 4 = 0.41, 5 = 0.41, 6 = 0.64, 7 = 0.76, 9 = 0.71, 10 = 0.82 and 11 = 0.71

NOTES

1) Unbalanced roof live loads have been considered for this design.

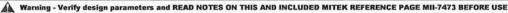
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp # B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

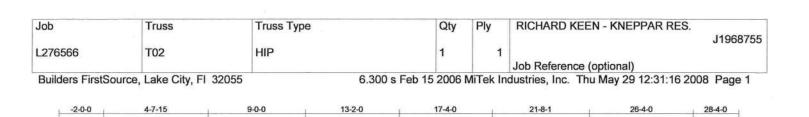
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Collygue ads page 2

londa PE No. 34868 109 Cassial Bay Sivil loynton Besch, FL 33435

May 29,2008

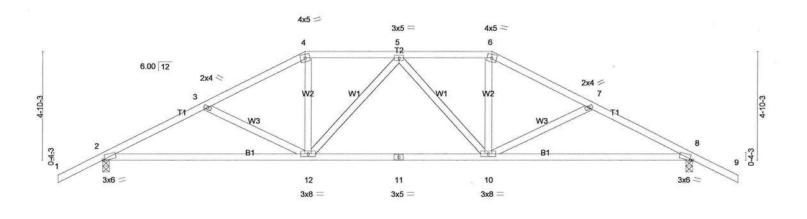






4-2-0

4-2-0



| Plate Of | fsets (X,Y | (): [2:0-1-5,0-0-7], [8: | 0-1-5,0-0- | 7] | | | | | | | | |
|----------|------------|--------------------------|------------|------|------|---|-------|-------|--------|-----|----------------|---------|
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.30 | Vert(LL) | -0.14 | 2-12 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.40 | Vert(TL) | -0.26 | 2-12 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.15 | Horz(TL) | 0.05 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | 20.1350.750.750.00.00.00.00.00.00.00.00.00.00.00.00.0 | | | | | Weight: 133 lb | |

17-4-0

8-4-0

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WFBS

2-0-0

4-7-15

4-4-1

2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

26-4-0

9-0-0

5-1-2 oc purlins.

4-4-1

4-7-15

2-0-0 Scale = 1:51.4

BOT CHORD Rigid ceiling directly applied or 8-5-9 oc

bracing.

REACTIONS (lb/size) 2=949/0-3-8, 8=949/0-3-8

Max Horz 2=-89(load case 7)

9-0-0

9-0-0

Max Uplift 2=-253(load case 6), 8=-253(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1497/788, 3-4=-1251/678, 4-5=-1081/662, 5-6=-1081/662,

6-7=-1251/678, 7-8=-1497/788, 8-9=0/47

BOT CHORD 2-12=-541/1277, 11-12=-411/1172, 10-11=-411/1172, 8-10=-541/1277

WEBS 3-12=-234/213, 4-12=-103/329, 5-12=-229/104, 5-10=-229/103, 6-10=-103/329,

7-10=-234/213

JOINT STRESS INDEX

2 = 0.81, 3 = 0.33, 4 = 0.64, 5 = 0.39, 6 = 0.64, 7 = 0.33, 8 = 0.81, 10 = 0.56, 11 = 0.43 and 12 = 0.56

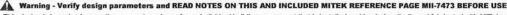
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp + 110mph B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

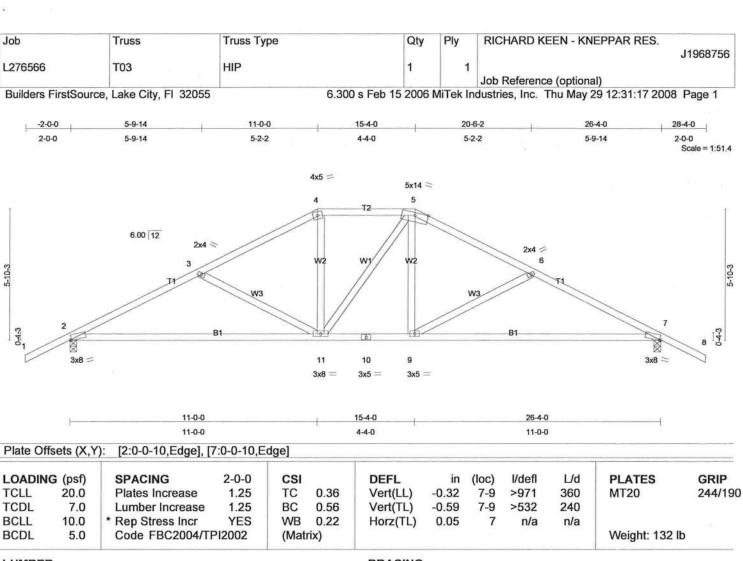
Continued on page 2

May 29,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

VVEBS 2X4SYPN

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

5-0-5 oc purlins.

5-0-5 oc pariiris.

Rigid ceiling directly applied or 8-6-0 oc

bracing.

REACTIONS (lb/size) 2=949/0-3-8, 7=949/0-3-8

Max Horz 2=-101(load case 7)

Max Uplift 2=-266(load case 6), 7=-266(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1460/799, 3-4=-1136/646, 4-5=-961/638, 5-6=-1136/646,

6-7=-1460/799, 7-8=0/47

BOT CHORD 2-11=-540/1241, 10-11=-278/960, 9-10=-278/960, 7-9=-540/1241

WEBS 3-11=-324/298, 4-11=-90/289, 5-11=-127/128, 5-9=-90/290, 6-9=-324/298

JOINT STRESS INDEX

2 = 0.91, 3 = 0.33, 4 = 0.68, 5 = 0.54, 6 = 0.33, 7 = 0.91, 9 = 0.39, 10 = 0.79 and 11 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colly have the page 2

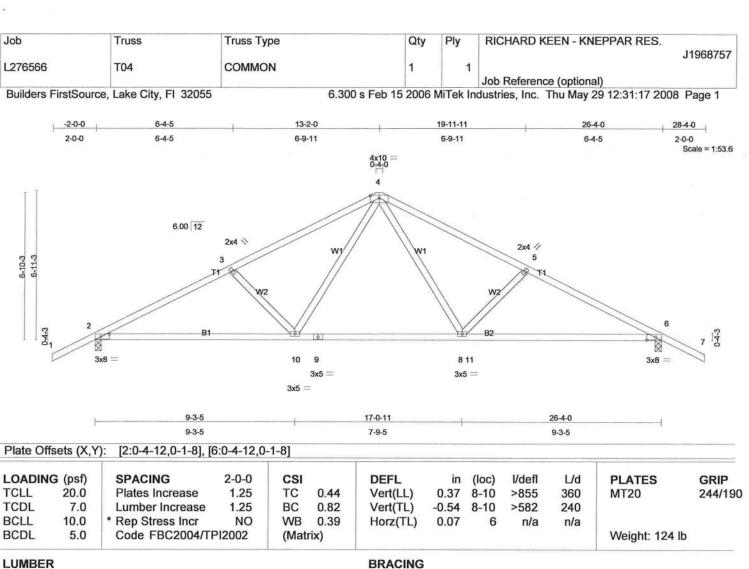
Julius Lee Truss Ossian Engineer Flonds PE No. 34899 1109 Ceastal Bay Blvd. Boynton Besch, Ft. 33435

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation and bracing the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or

4-3-5 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-8-13 oc

bracing.

REACTIONS (lb/size) 2=1189/0-3-8, 6=1195/0-3-8

Max Horz 2=-114(load case 7)

Max Uplift 2=-343(load case 6), 6=-344(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2019/1120, 3-4=-1783/1060, 4-5=-1796/1067, 5-6=-2032/1127,

BOT CHORD 2-10=-819/1726, 9-10=-434/1196, 8-9=-434/1196, 8-11=-826/1738, 6-11=-826/1738

WEBS 3-10=-317/297, 4-10=-356/667, 4-8=-369/690, 5-8=-317/297

JOINT STRESS INDEX

2 = 0.75, 3 = 0.33, 4 = 0.43, 5 = 0.33, 6 = 0.75, 8 = 0.53, 9 = 0.45 and 10 = 0.53

NOTES

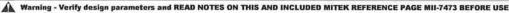
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

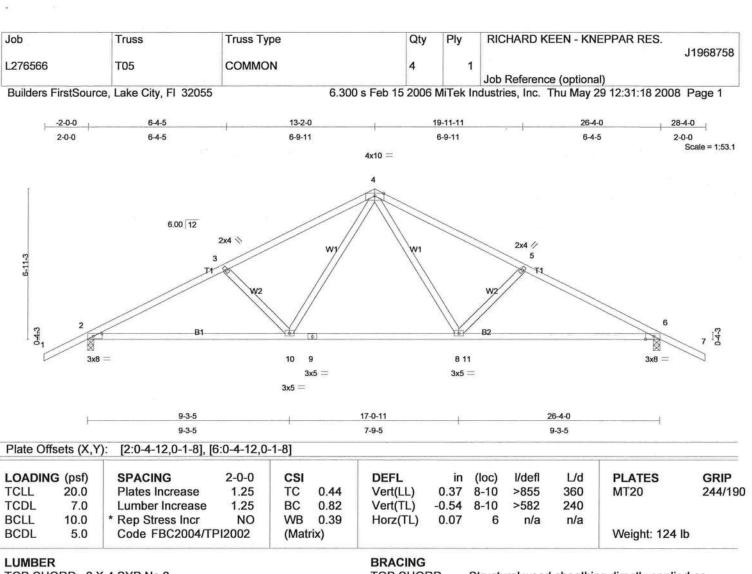
Charle regions age assumed to be SYP No.2 crushing capacity of 565.00 psi

May 29,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 **WEBS**

TOP CHORD Structural wood sheathing directly applied or

4-3-5 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-8-13 oc

bracing.

REACTIONS (lb/size) 2=1189/0-3-8, 6=1195/0-3-8

Max Horz 2=-114(load case 7)

Max Uplift 2=-343(load case 6), 6=-344(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2020/1120, 3-4=-1784/1060, 4-5=-1797/1068, 5-6=-2033/1127,

BOT CHORD 2-10=-820/1728, 9-10=-434/1196, 8-9=-434/1196, 8-11=-826/1739, 6-11=-826/1739

WEBS 3-10=-318/297, 4-10=-357/667, 4-8=-370/690, 5-8=-318/297

JOINT STRESS INDEX

2 = 0.74, 3 = 0.33, 4 = 0.43, 5 = 0.33, 6 = 0.74, 8 = 0.53, 9 = 0.45 and 10 = 0.53

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall hearing sage assumed to be SYP No.2 crushing capacity of 565.00 psi

00405

May 29,2008

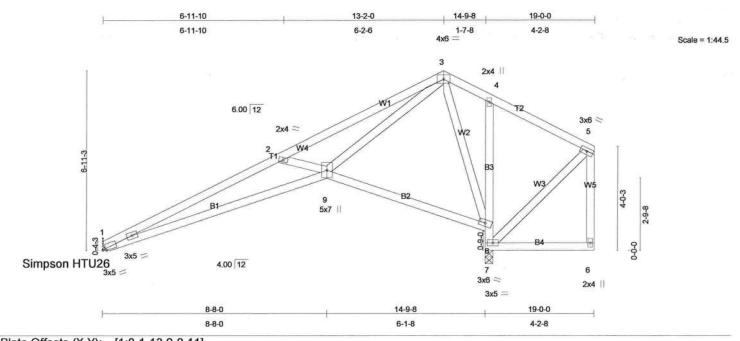
Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|--------|
| | | 522 = | 189 | 188 | J1: | 968759 |
| L276566 | T06 | SPECIAL | 4 | 1 | | |
| | | | | | Job Reference (optional) | |

06 Milek Industries, Inc. Thu May 29



| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
|-------------|------------------------------|-------------------|-------|------|------|----------|-------|-------|--------|----------------|--------|--------|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.34 | Vert(LL) | 0.19 | 1-9 | >962 | 360 | MT20 | 244/19 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.46 | Vert(TL) | -0.33 | 1-9 | >544 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.44 | Horz(TL) | 0.16 | 7 | n/a | n/a | | |
| BCDL | CDL 5.0 Code FBC2004/TPI2002 | | (Mat | rix) | | | | | | Weight: 103 lb | | |

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

B3 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

4-5-9 oc purlins, except end verticals.

Rigid ceiling directly applied or 5-10-5 oc

bracing.

REACTIONS

(lb/size) 1=443/Mechanical, 7=760/0-3-8

Max Horz 1=157(load case 6)

Max Uplift 1=-99(load case 6), 7=-166(load case 6) Max Grav 1=459(load case 10), 7=760(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1690/1027, 2-3=-1201/716, 3-4=0/139, 4-5=-20/134, 5-6=0/58

BOT CHORD 1-9=-1076/1529, 8-9=-45/140, 7-8=-657/408, 4-8=-167/93, 6-7=-13/16

WEBS 2-9=-459/471, 3-9=-697/1152, 3-8=-580/354, 5-7=-128/101

JOINT STRESS INDEX

1 = 0.70, 1 = 0.19, 2 = 0.33, 3 = 0.85, 4 = 0.36, 5 = 0.59, 6 = 0.33, 7 = 0.39, 8 = 0.67 and 9 = 0.83

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colivinue of page 2

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job Truss Truss Type Qty Ply DURATION / ST. ALBANS WOOD / ROOF J1968760 L276566 T07 SPECIAL 1 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 15:02:41 2008 Page 1 13-2-0 3-7-12 7-0-0 8-8-0 17-0-8 19-4-0 23-4-0 27-4-0 30-6-12 34-4-0 36-4-0 3-7-12 344 1-8-0 4-6-0 3-10-8 2-3-8 4-0-0 2-0-0 4-0-0 3-2-12 3-9-4 Scale = 1:65.9 4x5 = 6.00 12 5 4x10 = 3x5 < 4x12 = 4x6 6 2x4 || 3x6 = 2x4 || 5x6 = 7 22 2 3 8 3x5 < 10 3-10-3 3-10-3 19 7x8 = 18 5x14 = Simpson HTU26 5x8 = 17 16 23 15 24 14 13 21 4.00 12 2x4 || 5x14 2x4 || 4x10 = 2x4 || 4x5

| 3 | 3-7-12 | 8-8-0 | 13-2-0 | 17-0-8 | 19-4-0 | 23-4-0 | 28-2-4 | 30-6-12 | 34-4-0 |
|---|--------|-------|--------|--------|--------|--------|--------|---------|--------|
| 3 | 3-7-12 | 5-0-4 | 4-6-0 | 3-10-8 | 2-3-8 | 4-0-0 | 4-10-4 | 2-4-8 | 3-9-4 |

| Plate Of | fsets (X,Y) | [9:0-3-0,0-2-0] | | | | | | | | | | |
|----------|-------------|-------------------|--------|------|------|----------|-------|-------|--------|-----|----------------|---------|
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.36 | Vert(LL) | -0.32 | 18-19 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.91 | Vert(TL) | -0.61 | 18-19 | >550 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | NO | WB | 0.93 | Horz(TL) | 0.36 | 14 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/7 | PI2002 | (Mat | rix) | | | | | | Weight: 206 lb | |

| LUMBER | |
|------------------|----------------|
| TOP CHORD | 2 X 4 SYP No.2 |
| BOT CHORD | 2 X 4 SYP No.2 |
| WEBS | 2 X 4 SYP No.3 |

BRACING TOP CHORD

Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.

BOT CHORD WEBS Rigid ceiling directly applied or 6-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 4-18

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum and distance

Brace must cover 90% of web length.

REACTIONS (lb/size) 21=923/Mechanical, 14=2242/0-3-8

Max Horz 21=-199(load case 6)

Max Uplift 21=-219(load case 5), 14=-811(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-21=-914/211, 1-2=-1220/258, 2-3=-1220/257, 3-4=-3837/759, 4-5=-1454/324,

5-6=-1461/338, 6-7=-1582/360, 7-22=-1120/287, 8-22=-1120/288, 8-9=-1120/288,

9-10=-476/717, 10-11=-401/579, 11-12=0/47

BOT CHORD 20-21=-5/212, 19-20=-714/3738, 18-19=-661/3624, 17-18=-236/1472, 16-17=-299/1692,

16-23=-298/1693, 15-23=-298/1693, 15-24=-161/389, 14-24=-161/389, 13-14=-477/433,

11-13=-477/433

1-20=-309/1472, 2-20=-263/103, 3-20=-2870/575, 3-19=-130/81, 4-19=-414/2360,

4-18=-2625/548, 5-18=-197/982, 6-18=-239/121, 6-17=-62/99, 7-17=-566/148, 7-16=0/95,

7-15=-766/189, 8-15=-506/225, 9-15=-423/1675, 9-14=-2057/725, 10-14=-156/154,

10-13=-63/62

Julius Lee Truss Cesign Engineer Florida PE No. 34868 1100 Chastal Bay Blvd Bovnjon Beach, Ft. 33426

JOINT STRESS INDEX

WEBS

1 = 0.87, 2 = 0.34, 3 = 0.89, 4 = 0.97, 5 = 0.52, 6 = 0.48, 7 = 0.48, 8 = 0.34, 9 = 0.75, 10 = 0.48, 11 = 0.35, 13 = 0.34, 14 = 0.62, 15 May 29,2008 Continued on page 2 17 = 0.45, 18 = 0.74, 19 = 0.94, 20 = 0.83 and 21 = 0.35

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation will be from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000704 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T08 | SPECIAL | 1 | 1 | | J1968761 |
| | | | | | Job Reference (optional) | |

2x4 ||

Simpson HTU26

15

3x6 =

4-10-3 4-10-3 4-10-3 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:22 2008 Page 1



4x5 = 6.00 12 4x5 = 3x8 = 5x8 = 5 6 2x4 //

| | | 8-8-0 | 13-2-0 | | 17-0-8 | 25- | 4-0 | | 28-2-4 | 34-0 | 0-0 34-4-0 | |
|-------------|-------------------------------|-------------------|--------|------|--------|----------|----------|-------|--------|------|-------------|---------|
| | 8-8-0 4-6-0 | | | | 3-10-8 | 8-3-8 | | | 2-10-4 | 5-9- | -12 0-4-0 | |
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in (| (loc) | I/defl | L∕d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.66 | Vert(LL) | -0.17 13 | 3-14 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.53 | Vert(TL) | -0.34 14 | 4-15 | >988 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.94 | Horz(TL) | 0.19 | 10 | n/a | n/a | | |
| BCDL | BCDL 5.0 Code FBC2004/TPI2002 | | 212002 | (Mat | rix) | 11100000 | | | | | Weight: 194 | lb |

12

5x6 =

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD

BOT CHORD

3-11-8 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc

Structural wood sheathing directly applied or

bracing.

B3

11

3x5 =

10

4x5 =

WEBS

T-Brace:

2 X 4 SYP No.3 -

5-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 15=763/Mechanical, 8=-270/0-3-8, 10=1802/0-3-8

5x8 =

4.00 12

13

3x8 =

Max Horz 15=-219(load case 7)

Max Uplift 15=-181(load case 6), 8=-318(load case 10), 10=-369(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-15=-105/71, 1-2=-43/2, 2-3=-2399/955, 3-4=-1010/585, 4-5=-1029/597,

5-6=-6/221, 6-7=-411/1342, 7-8=-408/1218, 8-9=0/47

BOT CHORD 14-15=-109/910, 13-14=-689/2438, 12-13=-305/896, 11-12=-292/860,

10-11=-235/211, 8-10=-1031/493

2-15=-1218/570, 2-14=-612/1669, 3-14=0/481, 3-13=-1832/631, 4-13=-360/664,

5-13=-138/162, 5-12=-216/137, 5-11=-1175/573, 6-11=-204/638, 6-10=-1851/762,

7-10=-195/206

JOINT STRESS INDEX

WEBS

1 = 0.57, 2 = 0.75, 3 = 0.69, 4 = 0.40, 5 = 0.86, 6 = 0.69, 7 = 0.33, 8 = 0.70, 10 = 0.38, 11 = 0.46, 12 = 0.55, 13 = 0.95, 14 = 0.40, 12 = 0.40, 12 = 0.40, 13 = 0.40, 14 = 0

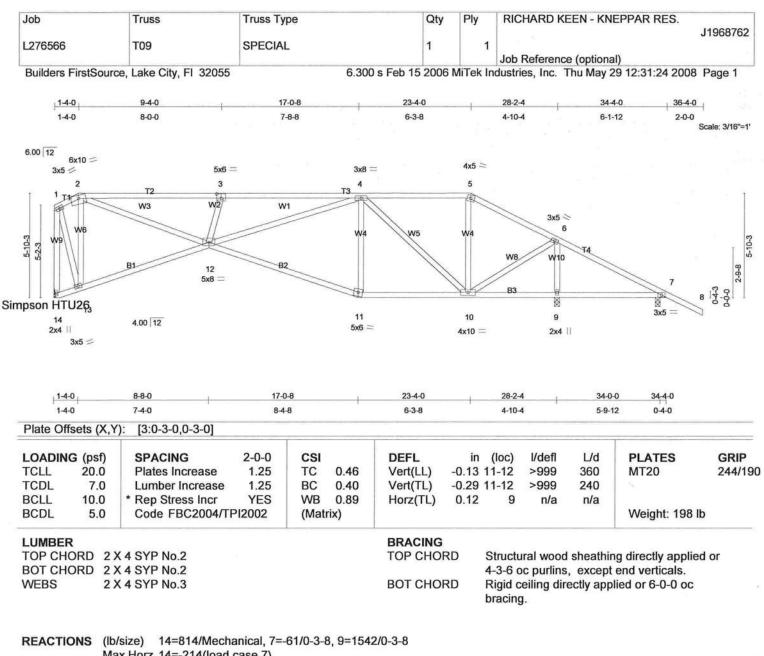
0.88 and 15 = 0.64 Continued on page 2

May 29,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





Max Horz 14=-214(load case 7)

Max Uplift 14=-214(load case 5), 7=-264(load case 10), 9=-397(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-212/84, 2-3=-1735/683, 3-4=-1840/760, 4-5=-336/348, 5-6=-441/337,

6-7=-238/845, 7-8=0/47, 1-14=-820/333

BOT CHORD 13-14=0/353, 12-13=0/255, 11-12=-244/932, 10-11=-227/878, 9-10=-686/356,

7-9=-686/356

WEBS 2-13=-871/459, 2-12=-587/1605, 3-12=-448/326, 4-12=-214/1017, 4-11=-205/113,

4-10=-767/327, 5-10=-97/51, 6-10=-406/1202, 6-9=-1463/713, 1-13=-400/886

JOINT STRESS INDEX

1 = 0.73, 2 = 0.81, 3 = 0.66, 4 = 0.91, 5 = 0.60, 6 = 0.76, 7 = 0.59, 9 = 0.52, 10 = 0.55, 11 = 0.76, 12 = 0.91, 13 = 0.79 and 14 = 0.36

NOTES

1) Unbalanced roof live loads have been considered for this design.

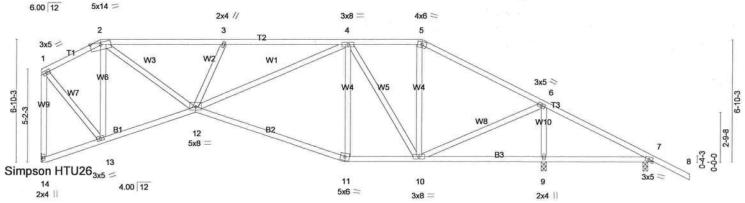
Julius Lee Truss Design Engineer Florida PE No. 24868 1 100 Cassial Bay Blyd Boynton Besch, Ft. 33435

May 29,2008

Continued on page 2



| 276566 T | 40 | 17 m | | | | | | | 300700 |
|--------------------------|--------------------|---------|---------------|--------|---|------------------|----------------|--------------|-----------|
| | 10 | SPECIAL | | 1 | 1 | Job Reference | (optional) | J19 | 968763 |
| Builders FirstSource, La | ake City, FI 32055 | 17-0-8 | 6.300 s Feb 1 | 5 2006 | | dustries, Inc. T | hu May 29 12:3 | 1:25 2008 Pa | ige 1 |
| 3-4-0 | 6-10-9 | 6-9-15 | 4-3-8 | | | 10-4 | 6-1-12 | 2-0-0 | e: 3/16"= |



| 1 | 3-4-0 | 8-8-0 | 1/-0 |)-8 | | 21-4-0 | | 28-2-4 | | 34-0 | -0 34-4-0 | |
|--------|----------|-------------------|--------|------|------|-------------------------|-------|--------|--------|-------|----------------|---------|
| | 3-4-0 | 5-4-0 | 8-4 | -8 | | 4-3-8 | | 6-10-4 | | 5-9-1 | 2 0-4-0 | |
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.44 | Vert(LL) | -0.13 | 11-12 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.46 | Vert(TL) | -0.26 | 11-12 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.51 | Horz(TL) | 0.08 | 9 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | 283-2000 2000 200 200 A | | | | | Weight: 205 lb | |

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or 4-10-4 oc purlins, except end verticals.

BOT CHORD

BRACING

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 14=859/Mechanical, 7=148/0-3-8, 9=1287/0-3-8

Max Horz 14=-226(load case 7)

Max Uplift 14=-197(load case 5), 7=-239(load case 7), 9=-315(load case 4)

Max Grav 14=859(load case 1), 7=153(load case 11), 9=1287(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-553/259, 2-3=-1334/562, 3-4=-1504/686, 4-5=-618/494, 5-6=-779/474,

6-7=-63/372, 7-8=0/47, 1-14=-844/383

BOT CHORD 13-14=-10/349, 12-13=-48/503, 11-12=-187/898, 10-11=-173/845, 9-10=-272/186,

7-9=-272/186

WEBS 2-13=-677/313, 2-12=-354/1059, 3-12=-435/313, 4-12=-165/730, 4-11=-205/87,

4-10=-504/173, 5-10=-24/136, 6-10=-294/957, 1-13=-302/741, 6-9=-1206/645

JOINT STRESS INDEX

1 = 0.58, 2 = 0.76, 3 = 0.33, 4 = 0.66, 5 = 0.71, 6 = 0.56, 7 = 0.59, 9 = 0.43, 10 = 0.87, 11 = 0.68, 12 = 0.86, 13 = 0.57 and 14 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Les Truss Design Engineer Floride FE No. 34866 1 109 Cassel Bay Elvi Boynton Besch, FL 30435

May 29,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

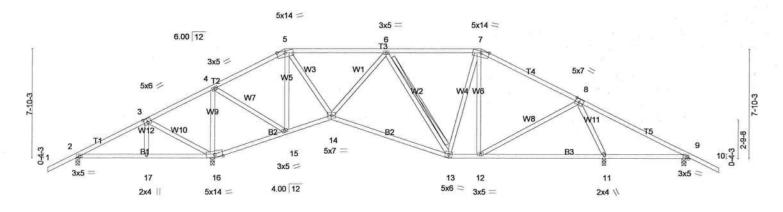
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or ItIle-91 Handling Installing and Bracing Recommendation will be from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000704 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T11 | SPECIAL | 2 | 1 | | J1968764 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:26 2008 Page 1





| 1 | 5-1-0 | 9-9-12 | 9-11-8 | 15-0-0 | 18-4-0 | 26-8-8 | 29-0-0 | 37-10-4 | 43-8-0 | 44-0-0 |
|---|-------|--------|--------|--------|--------|--------|--------|---------|--------|--------|
| - | 5-1-0 | 4-8-12 | 0-1-12 | 5-0-8 | 3-4-0 | 8-4-8 | 2-3-8 | 8-10-4 | 5-9-12 | 0-4-0 |

| Plate Offsets (X,Y): | [3:0-3-0.0-3-0] | [8:0-3-8.0-3-0] | [9:0-1-12 Edge] |
|----------------------|-----------------|-----------------|-----------------|
| | | | |

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
|---------------|------|-------------------|--------|------|------|----------|-------|-------|--------|-----|----------------|---------|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.42 | Vert(LL) | 0.10 | 9-11 | >724 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.54 | Vert(TL) | -0.22 | 13-14 | >999 | 240 | 17/50CRI | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.50 | Horz(TL) | 0.04 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | | | | | | Weight: 249 lb | |

| LUMBER | |
|------------------|----------------|
| TOP CHORD | 2 X 4 SYP No.2 |
| BOT CHORD | 2 X 4 SYP No.2 |
| WEBS | 2 X 4 SYP No.3 |

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

6-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=187/0-3-8, 16=1515/0-3-8, 9=203/0-3-8, 11=1124/0-3-8

Max Horz 2=125(load case 6)

Max Uplift 2=-240(load case 6), 16=-443(load case 5), 9=-241(load case 7),

11=-263(load case 4)

Max Grav 2=207(load case 10), 16=1515(load case 1), 9=209(load case 11), 11=1124(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=0/323, 3-4=-185/581, 4-5=-402/262, 5-6=-668/390, 6-7=-603/523,

7-8=-742/503, 8-9=-68/356, 9-10=0/47

BOT CHORD 2-17=-271/62, 16-17=-271/63, 15-16=-540/476, 14-15=0/316, 13-14=-111/818,

12-13=-56/580, 11-12=-57/268, 9-11=-227/224

3-17=-177/149, 3-16=-344/479, 4-16=-1127/564, 4-15=-323/948, 5-15=-727/288,

5-14=-176/661, 6-14=-165/269, 6-13=-369/118, 7-13=-49/151, 7-12=-140/71,

Continued on page 12=-47/392, 8-11=-1123/647

WEBS

Julius Lee Trues Cesian Engineer Florida FE No. 24866 1 109 Cessial Bay Blvd Boynton Besch, FL 33435

May 29,2008



| Job _276566 | Truss | Truss Typ | е | Qty 2 | Ply 1 | RICHARD | KEEN - KNE | PPAR RES. | J1968765 |
|----------------------------|-------------------------|-----------------|-------------|--|---------------|---------------|-----------------|--------------------|-------------------------|
| 270000 | 1.12 | OI LOIAL | | - | | Job Referen | ce (optional) |) | |
| Builders FirstSour | ce, Lake City, FI 32055 | | 6.3 | 00 s Feb 15 2006 | MiTek In | | | | 008 Page 1 |
| 200 | | na suntantant | 10000000000 | 112270121 | 0000 | 2020 | 2000000 | Self-Chich | |
| 2-0-0 5-1-1 2-0-0 5-1-1 | | 17-0-0 7-0-8 | 5-7-1 | 27-0-0 4-5-0 | 6-0 | 1 | 7-10-4 -9-13 | 44-0-0 | 46-0-0 |
| 2-0-0 5-1- | J 4-10-8 | 7-0-8 | 5-7-1 | 4-5-0 | 6-0 | H5 4 | -9-13 | 6-1-12 | 2-0-0 Scale = 1:82.7 |
| | | | 5x6 = | | | | | | |
| | | | | | ix6 = | | | | |
| ī | 6.00 12 | | 5 | T3 ₍₆₎ | 7 | | | | |
| | (8) | | 117 | | | | | | |
| | 2x4 | | W3 W1 | | | 3x5 > T4 8 | | | |
| | 5x6 = 4 12 | W5// | \\ // | W2 V | V4 | M | 5x6 < | | 2 |
| | | | \\ // | | Teson | | 9 | | 0 |
| | 3 W9 | | | | WE | 3/ W7 | M | | 0 |
| | W12 W10 | // B2/ | | B2 \\ | | | W8 W11 | | _ |
| n 2 II | | | 15 | The state of the s | | | | 75 | 10 8 |
| §1 2 | BJ | | 6x8 = | 1 | 4 | 6 | B3 g | | 1114 |
| 3x5 = | 17 16 | | | | 14 | 13 | 12 | 3 | x5 = |
| | 2x4 5x14 = | 4.00 1 | 2 | | 14 = | 3x5 = | 2x4 | | |
| | | | | | | | | | |
| | | | | | | | | | |
| 5-1-0 | | 18-4-0 8-4-8 | | 26-8-8 8-4-8 | 33-0- 6-4- | | 9-13 | Description of the | 0-0 |
| | | | | 8-4-8 | 6-4- | 0 4- | 9-13 | 5-9-12 0 | 4-0 |
| Plate Offsets (X,Y | (): [3:0-3-0,0-3-0], [9 | :0-3-0,0-3-0 | 1 | | | | | | |
| OADING (psf) | SPACING | 2-0-0 | CSI | DEFL | in (| (loc) I/defl | L/d | PLATES | GRIP |
| CLL 20.0 | Plates Increase | 1.25 | TC 0.42 | Vert(LL) | -0.10 14 | | 360 | MT20 | 244/19 |
| CDL 7.0 | Lumber Increase | 1.25 | BC 0.45 | Vert(TL) | -0.19 14 | | 240 | | |
| 3CLL 10.0 | * Rep Stress Incr | YES | WB 0.93 | Horz(TL) | 0.06 | 12 n/a | n/a | | |
| JULL 10.0 | Code FBC2004/TI | | | | | | | Weight: 2 | |

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -5-16, 6-14, 7-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=290/0-3-8, 16=1363/0-3-8, 10=187/0-3-8, 12=1190/0-3-8

Max Horz 2=-136(load case 7)

Max Uplift 2=-229(load case 6), 16=-441(load case 6), 10=-243(load case 7),

12=-269(load case 7)

Max Grav 2=305(load case 10), 16=1363(load case 1), 10=200(load case 11), .12=1190(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-116/185, 3-4=-151/335, 4-5=-49/354, 5-6=-711/424, 6-7=-597/543,

7-8=-739/532, 8-9=-619/442, 9-10=-18/318, 10-11=0/47

BOT CHORD 2-17=-97/45, 16-17=-96/46, 15-16=-22/648, 14-15=-53/760, 13-14=-87/501,

12-13=-187/148, 10-12=-215/163

3-17=-184/127, 3-16=-290/447, 4-16=-346/355, 5-16=-1307/440, 5-15=-64/545,

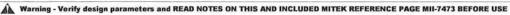
6-15=-75/188, 6-14=-330/88, 8-14=-38/196, 8-13=-419/214, 9-13=-283/826,

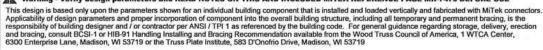
Continued on page 12=-1115/586, 7-14=-2/123

WEBS

lisia Lee Lee Cesign Engineer Inde PE No. 24888 Od Chastal Bay Alvel Synton Besch. FL 00435

May 29,2008







| Job L276566 | | Truss Truss Type T13 SPECIAL | | | | Qty 2 | Ply | RICHARD KEEN - KNEPPAR RES. J1968766 | | | | | |
|----------------|---------|------------------------------|----------|-------|-------|----------|------------|---|----------|--------------------|--------------------------|----------|-------------|
| | _ | | | | OI LO | IAL . | | | 2 | Job Reference (d | of February and American | | |
| Builders Firs | tSource | , Lake | City, FI | 32055 | | | 6.300 s Fe | b 15 2006 | MiTek In | dustries, Inc. Thu | May 29 | 12:31:28 | 2008 Page 1 |
| -2-0-0 | 5-1-0 | | 9-11-8 | 14 | -4-0 | 18-8-8 | 25-3-8 | 26-8-8 | 32-3-6 | 37-10-4 | 38-0-0 | 44-0-0 | 46-0-0 |
| | 5-1-0 | 1.75 | 4-10-8 | 4 | 4-8 | 4-4-8 | 6-7-0 | 1-5-0 | 5-6-14 | 5-6-14 | 0-1-12 | 6-0-0 | 2-0-0 |

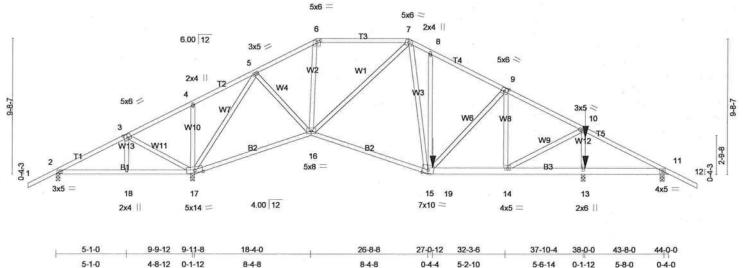


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [6:0-3-0,0-2-0], [9:0-3-0,0-3-0] 2-0-0 LOADING (psf) SPACING CSI DEFL L/d **PLATES** GRIP in (loc) I/defl TCLL 20.0 1.25 TC 0.50 Vert(LL) Plates Increase 0.06 14-15 >999 360 MT20 244/190 TCDL 7.0 1.25 BC Vert(TL) Lumber Increase 0.25 -0.09 16-17 >999 240 * Rep Stress Incr BCLL 10.0 WB NO 0.65 0.03 Horz(TL) 13 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 Weight: 561 lb (Matrix)

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

B3 2 X 6 SYP No.1D

WEBS

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 2=90/0-3-8, 17=2292/0-3-8, 13=3788/0-3-8, 11=103/0-3-8

Max Horz 2=-148(load case 6)

Max Uplift 2=-137(load case 10), 17=-1035(load case 5), 13=-2083(load case 6),

11=-140(load case 6)

Max Grav 2=117(load case 9), 17=2292(load case 1), 13=3788(load case 1),

11=119(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-190/501, 3-4=-367/777, 4-5=-295/763, 5-6=-1525/642,

6-7=-1308/592, 7-8=-2069/1229, 8-9=-2313/1267, 9-10=-2474/1363,

10-11=-153/400, 11-12=0/51

BOT CHORD 2-18=-429/242, 17-18=-429/242, 16-17=-182/645, 15-16=-734/1744,

15-19=-916/1953, 14-19=-923/1964, 13-14=-267/189, 11-13=-267/189

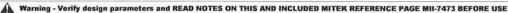
3-18=-116/124, 3-17=-325/277, 4-17=-274/155, 5-17=-2338/922, 5-16=-419/1080, 6-16=-211/375, 7-16=-548/504, 7-15=-922/1113, 8-15=-565/405, 9-15=-244/259,

9-14=-1102/590, 10-14=-1240/2528, 10-13=-3336/1792

JOINT STRESS INDEX

2 = 0.27, 3 = 0.28, 4 = 0.33, 5 = 0.46, 6 = 0.49, 7 = 0.32, 8 = 0.33, 9 = 0.66, 10 = 0.76, 11 = 0.26, 13 = 0.42, 14 = 0.58, 15 = 0.42Continued 16h = 25217 = 0.49 and 18 = 0.33

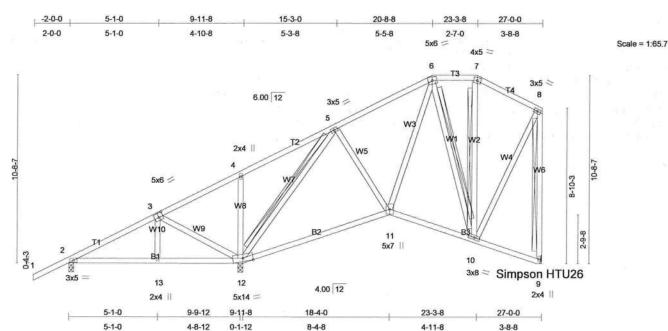
May 29,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| | 1400 |
|---------|--------------------------|
| | J196 |
| SPECIAL | 3 1 |
| | Job Reference (optional) |
| it | SPECIAL 6 |



| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
|--------|----------|-------------------|-------|------|------|----------|-------|-------|--------|-----|----------------|---------|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.29 | Vert(LL) | -0.10 | 11-12 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.31 | Vert(TL) | -0.17 | 11-12 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.36 | Horz(TL) | 0.03 | 9 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 2002 | (Mat | rix) | | | | | | Weight: 192 lb | |

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or

biacin

2 X 4 SYP No.3 -

WEBS

T-Brace:

5-12, 6-10, 7-10, 8-9 narrow edge of web

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=329/0-3-8, 12=1012/0-3-8, 9=484/Mechanical

Max Horz 2=346(load case 6)

Max Uplift 2=-179(load case 6), 12=-455(load case 6), 9=-99(load case 6) Max Grav 2=333(load case 10), 12=1012(load case 1), 9=484(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-182/51, 3-4=-399/256, 4-5=-257/264, 5-6=-376/215, 6-7=-152/174,

7-8=-212/149, 8-9=-464/297

2-13=-365/106, 12-13=-364/106, 11-12=-198/262, 10-11=-140/242, 9-10=-13/22

3-13=-228/121, 3-12=-315/530, 4-12=-272/278, 5-12=-719/494, 5-11=0/142,

6-11=-92/202, 6-10=-297/187, 7-10=-127/52, 8-10=-179/331

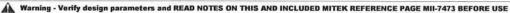
JOINT STRESS INDEX

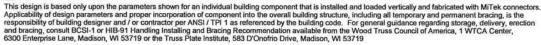
BOT CHORD

WEBS

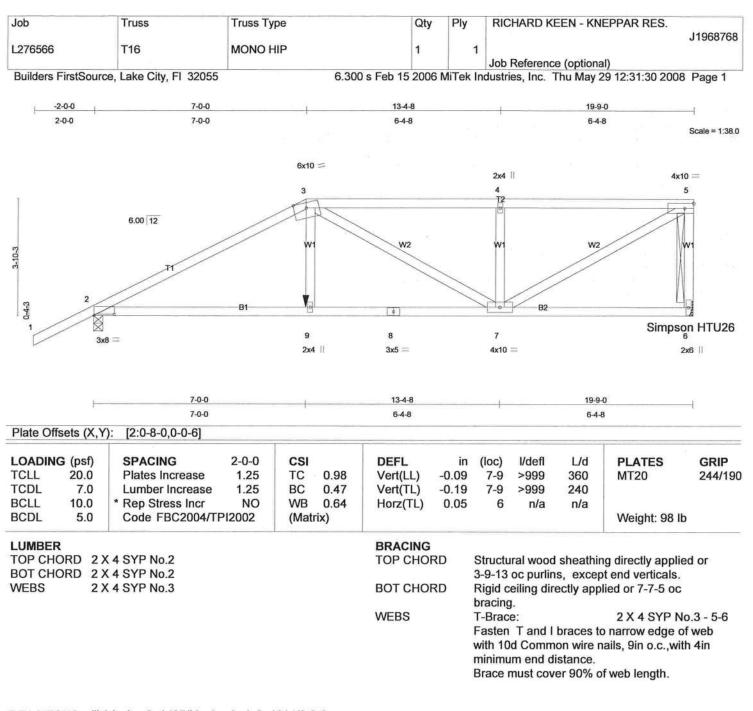
2 = 0.62, 3 = 0.36, 4 = 0.33, 5 = 0.41, 6 = 0.32, 7 = 0.29, 8 = 0.46, 9 = 0.33, 10 = 0.65, 11 = 0.70, 12 = 0.43 and 13 = 0.33. Continued on page 2

May 29,2008









REACTIONS (lb/size) 6=1405/Mechanical, 2=1314/0-3-8

Max Horz 2=163(load case 5)

Max Uplift 6=-485(load case 4), 2=-440(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2273/718, 3-4=-1835/636, 4-5=-1835/636, 5-6=-1327/515

BOT CHORD 2-9=-673/1953, 8-9=-677/1974, 7-8=-677/1974, 6-7=-34/102

WEBS 3-9=-106/523, 3-7=-160/100, 4-7=-788/430, 5-7=-693/1995

JOINT STRESS INDEX

2 = 0.70, 3 = 0.92, 4 = 0.33, 5 = 0.69, 6 = 0.70, 7 = 0.90, 8 = 0.75 and 9 = 0.37

NOTES

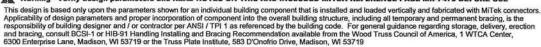
 Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

& hinvide adeauste drainage to prevent water ponding.

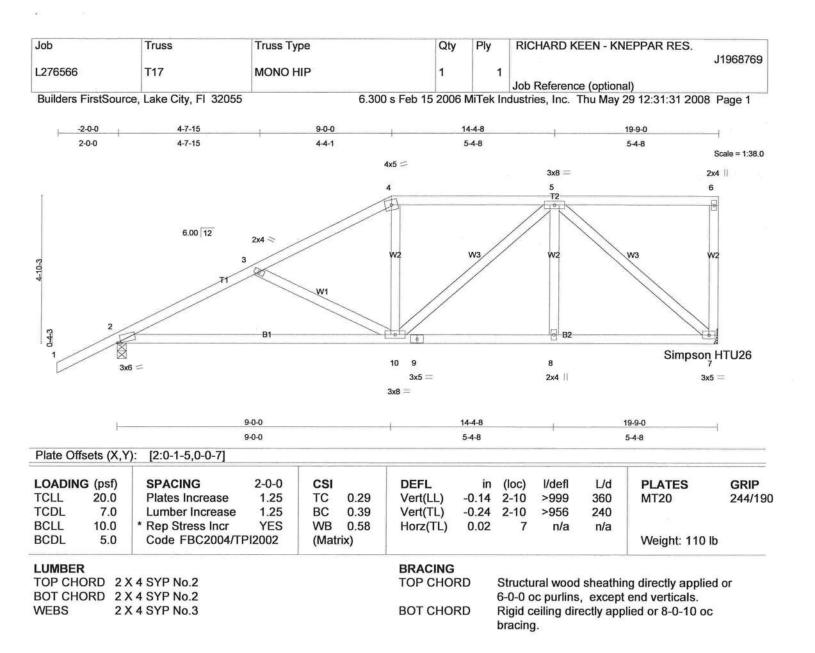
Truss Cesian Engineer Florida FE No. 34869 1103 Casstal Bay Blvd Boynton Beach, FL 33435

May 29,2008









REACTIONS (lb/size) 7=616/Mechanical, 2=745/0-3-8

Max Horz 2=195(load case 6)

Max Uplift 7=-165(load case 5), 2=-218(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1059/504, 3-4=-808/391, 4-5=-679/402, 5-6=-23/11, 6-7=-128/92

BOT CHORD 2-10=-599/889, 9-10=-314/548, 8-9=-314/548, 7-8=-314/548

WEBS 3-10=-237/217, 4-10=0/183, 5-10=-117/174, 5-8=0/131, 5-7=-696/402

JOINT STRESS INDEX

2 = 0.86, 3 = 0.33, 4 = 0.67, 5 = 0.56, 6 = 0.68, 7 = 0.39, 8 = 0.33, 9 = 0.48 and 10 = 0.56

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

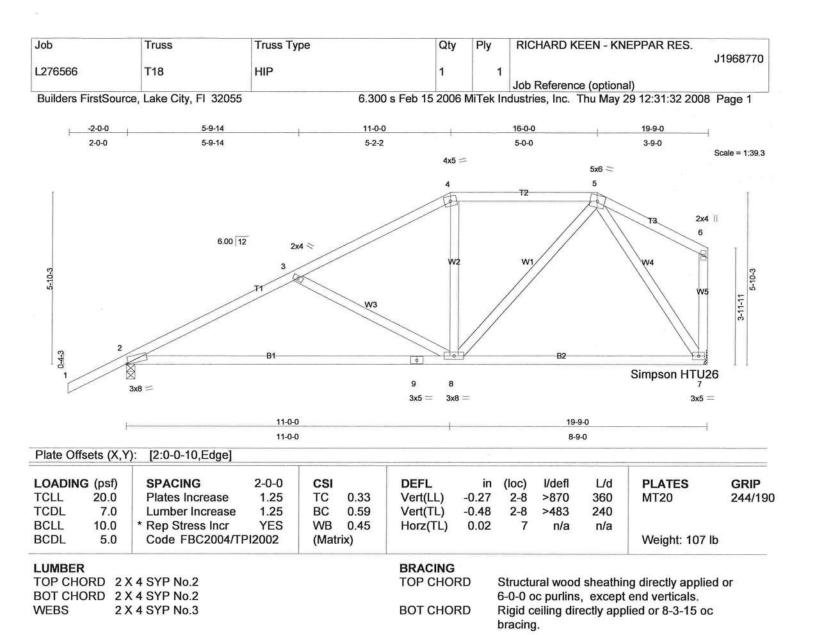
Flonda FE No. 34898 1 100 Chastal Bay Blvd Boynton Beach, FL 99425

May 29,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-97 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





REACTIONS (lb/size) 2=745/0-3-8, 7=616/Mechanical

Max Horz 2=190(load case 6)

Max Uplift 2=-230(load case 6), 7=-103(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-10

1-2=0/47, 2-3=-1021/524, 3-4=-695/370, 4-5=-562/389, 5-6=-59/60, 6-7=-92/92

BOT CHORD

2-9=-564/850, 8-9=-564/850, 7-8=-185/337

WEBS

3-8=-325/300, 4-8=-19/150, 5-8=-169/369, 5-7=-583/330

JOINT STRESS INDEX

2 = 0.82, 3 = 0.33, 4 = 0.75, 5 = 0.35, 6 = 0.35, 7 = 0.55, 8 = 0.56 and 9 = 0.50

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All hearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

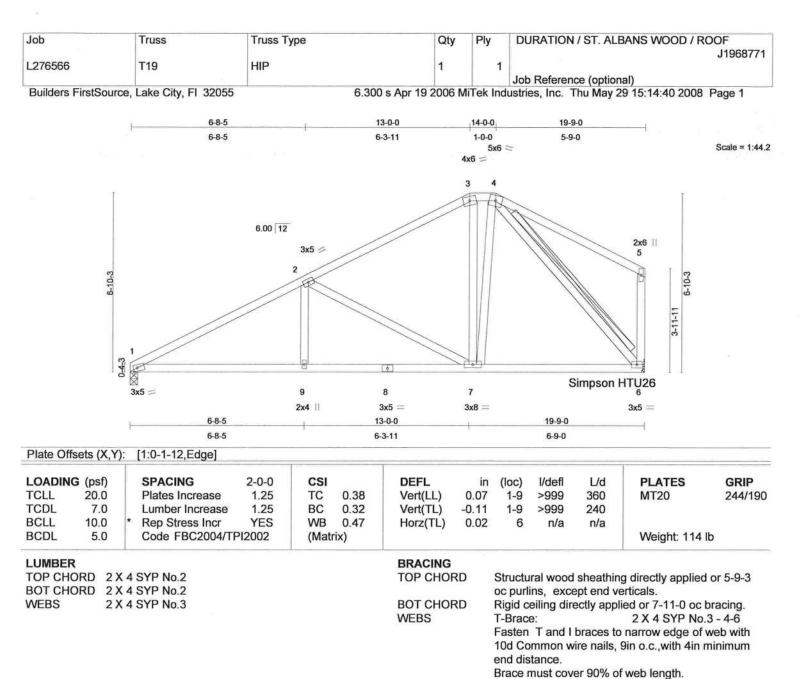
Truss Design Engineer Florida FE No. 24888 1100 Chastal Bay Blyd Boynton Beach, FL 33435

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





REACTIONS (lb/size) 1=623/0-3-8, 6=623/Mechanical

Max Horz 1=153(load case 6)

Max Uplift 1=-139(load case 6), 6=-128(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-1062/549, 2-3=-566/349, 3-4=-433/377, 4-5=-110/118, 5-6=-166/161

BOT CHORD 1-9=-587/880, 8-9=-587/880, 7-8=-587/880, 6-7=-218/400 **WEBS** 2-9=0/210, 2-7=-505/398, 3-7=-99/157, 4-7=-125/397, 4-6=-553/293

JOINT STRESS INDEX

1 = 0.76, 2 = 0.48, 3 = 0.65, 4 = 0.46, 5 = 0.58, 6 = 0.48, 7 = 0.58, 8 = 0.32 and 9 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

Lessian Engineer a PE No. 34888 Coastal Bay filve on Beach, FL 35

May 29,2008

Continued on page 2

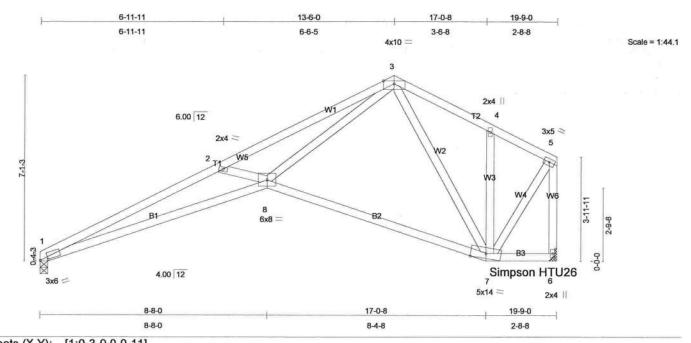
🛦 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000770 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T20 | SPECIAL | 3 | 1 | | J1968772 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:33 2008 Page 1



| Plate Offsets (A, f |). | [1:0-3-9,0-0-11] |
|---------------------|----|------------------|
| | | |

| LOADIN | G (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
|--------|---------|-------------------|--------|------|------|---|-------|-------|--------|-----|----------------|---------|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.41 | Vert(LL) | 0.31 | 1-8 | >758 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.61 | Vert(TL) | -0.44 | 1-8 | >527 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.78 | Horz(TL) | 0.28 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | 0.000 | | | | | Weight: 105 lb | |

| L | U | M | В | E | R | |
|---|---|---|---|---|---|--|
| | | | | | | |

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-6-1 oc purlins, except end verticals. Rigid ceiling directly applied or 4-8-14 oc

bracing.

REACTIONS (lb/size) 1=623/0-3-8, 6=623/Mechanical

Max Horz 1=157(load case 6)

Max Uplift 1=-139(load case 6), 6=-133(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-2631/1644, 2-3=-2170/1349, 3-4=-342/281, 4-5=-345/210, 5-6=-633/367

BOT CHORD

1-8=-1648/2404, 7-8=-299/528, 6-7=-8/6

WEBS

2-8=-431/452, 3-8=-1098/1786, 3-7=-436/262, 4-7=-175/127, 5-7=-265/507

JOINT STRESS INDEX

1 = 0.78, 2 = 0.33, 3 = 0.61, 4 = 0.33, 5 = 0.46, 6 = 0.33, 7 = 0.28 and 8 = 0.75

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

May 29,2008

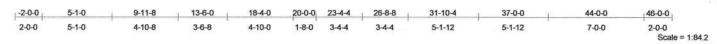
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

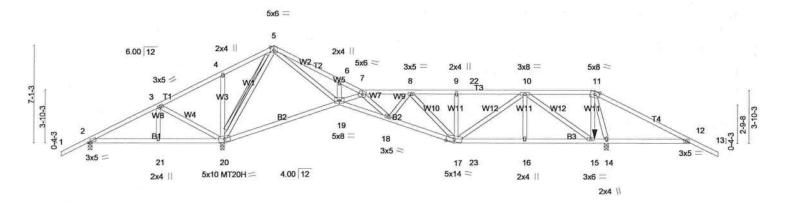
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|--------------------------------|-----|-----|-----------------------------|----------|
| L276566 | T21 | SPECIAL | 1 | 1 | | J1968773 |
| = | | Design Control Control Control | 100 | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:35 2008 Page 1





| Plate Of | fsets (X,Y | '): [2:0-2-0,0-0-7] | | | | 400 | ¥ | 1-12 | 3-1-12 | 1-0-0 | | |
|----------|------------|---------------------|--------|------|------|----------|-------|-------|--------|-------|----------------|---------|
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.54 | Vert(LL) | -0.12 | 19-20 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.40 | Vert(TL) | -0.25 | 19-20 | >999 | 240 | MT20H | 187/143 |
| BCLL | 10.0 | * Rep Stress Incr | NO | WB | 0.89 | Horz(TL) | 0.07 | 14 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | | | | | | Weight: 237 II |) |

22-0-0

3-8-0

| LUMBER | |
|------------------|----------------|
| TOP CHORD | 2 X 4 SYP No.2 |
| BOT CHORD | 2 X 4 SYP No.2 |
| WEBS | 2 X 4 SYP No.3 |

WEBS

5-1-0

5-1-0

9-9-12

4-8-12

9-11-8

0-1-12

18-4-0

8-4-8

BRACING TOP CHORD

4.8.8

OP CHORD Structural wood sheathing directly applied or

37-0-0

5-1-12

38-0-0

1-0-0

44-0-0

6.0.0

5-5-10 oc purlins.

31-10-4

5-1-12

BOT CHORD Rigid ceiling directly applied or 5-1-4 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

5-20

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=-427/0-3-8, 20=2433/0-3-8, 14=2111/0-3-8

Max Horz 2=-116(load case 6)

Max Uplift 2=-683(load case 10), 20=-587(load case 6), 14=-957(load case 6)

Max Grav 20=2433(load case 1), 14=2113(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-193/1667, 3-4=-368/1906, 4-5=-310/1894, 5-6=-262/64, 6-7=-207/0,

7-8=-1182/332, 8-9=-1182/381, 9-22=-1182/381, 10-22=-1182/381, 10-11=-176/118

, 11-12=-468/722, 12-13=0/47

BOT CHORD 2-21=-1461/283, 20-21=-1461/283, 19-20=-781/351, 18-19=-156/1071,

17-18=-332/1316, 17-23=-312/1089, 16-23=-312/1089, 15-16=-312/1089,

14-15=-77/318, 12-14=-562/508

3-21=-117/123, 3-20=-370/283, 4-20=-242/137, 5-20=-1944/342, 5-19=-188/1175,

6-19=-213/126, 7-19=-910/311, 7-18=-152/259, 8-18=-109/90, 8-17=-95/74,

Continued on page 27=-332/160, 10-17=-102/116, 10-16=0/219, 10-15=-1373/531, 11-15=-304/1139,

Trues Ossign Engineer Flonds FE No. 34888 1109 Ceastal Bay Blvd. Boynton Besch, FL 33435

May 29,2008

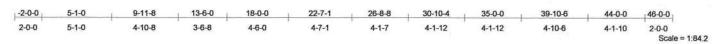
Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

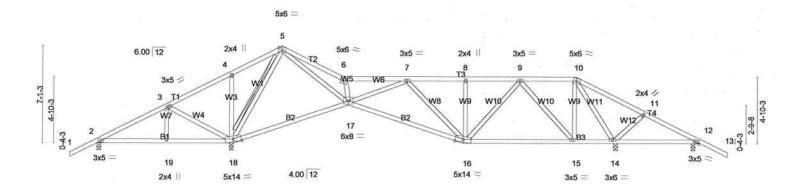
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|--|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T22 | SPECIAL | 1 | 1 | | J1968774 |
| Section Control of the Control of th | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:36 2008 Page 1





| | 5-1-0 9-9-12 9-11-8 18-4 | | | 26-8-8 | | | 35-0-0 | | | 37-10-4 | 43-8-0 | 44-0-0 | |
|---------------------|--------------------------|----------------------|-------|----------|------|----------|--------|-------|--------|---------|----------------|--------------|---------|
| 5-1-0 4-8-12 0-1-12 | | | 8-4-8 | 8-4-8 | | 8-4-8 | | 8-3-8 | | 2-10-4 | 5-9-12 | 5-9-12 0-4-0 | |
| LOADIN | G (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L/d | PLATE | S | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.38 | Vert(LL) | -0.10 | 17-18 | >999 | 360 | MT20 | | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.32 | Vert(TL) | -0.20 | 16-17 | >999 | 240 | | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.63 | Horz(TL) | 0.05 | 14 | n/a | n/a | | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 241 lb | | |

| LUMBER | |
|------------------|----------------|
| TOP CHORD | 2 X 4 SYP No.2 |
| BOT CHORD | 2 X 4 SYP No.2 |
| WEBS | 2 X 4 SYP No.3 |
| | |

LUMBED

WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing

Except:

10-0-0 oc bracing: 16-17,15-16.

WEBS

T-Brace:

2 X 4 SYP No.3 -

5-18

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=-164/0-3-8, 18=1922/0-3-8, 12=-14/0-3-8, 14=1284/0-3-8

Max Horz 2=-116(load case 7)

Max Uplift 2=-419(load case 11), 18=-491(load case 7), 12=-165(load case 7),

14=-403(load case 7)

Max Grav 2=175(load case 10), 18=1922(load case 1), 14=1291(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-180/1101, 3-4=-524/1335, 4-5=-404/1322, 5-6=-611/318,

6-7=-401/159, 7-8=-545/438, 8-9=-545/438, 9-10=0/136, 10-11=-247/826,

11-12=-171/616, 12-13=0/47

BOT CHORD 2-19=-960/296, 18-19=-960/296, 17-18=-475/488, 16-17=-181/717, 15-16=-79/349,

14-15=-46/179, 12-14=-498/271

3-19=-183/127, 3-18=-355/498, 4-18=-240/229, 5-18=-1482/584, 5-17=-473/1204,

6-17=-506/332, 7-17=-305/342, 7-16=-182/53, 8-16=-222/162, 9-16=-134/298,

9-15=-575/323, 10-15=-168/479, 10-14=-1254/549, 11-14=-251/251 Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCS-I1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

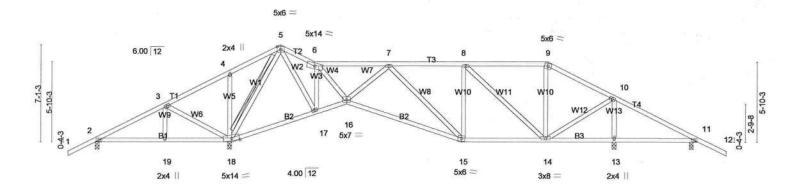


May 29,2008

| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T23 | SPECIAL | 1 | 1 | 51 P | J1968775 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:37 2008 Page 1





| | 5- | 1-0 9-9-12 9-11-8 | 16-0-0 | 18-4-0 | | 26-8-8 | 1 | 33-0-0 | 37- | 10-4 | 43-8-0 | 44-0-0 | |
|--------|---------|-------------------|--------|---------|------|--|-------|--------|--------|------|--------|-----------|---------|
| | 5- | 1-0 4-8-12 0-1-12 | 6-0-8 | 2-4-0 | | 8-4-8 | | 6-3-8 | 4-1 | 10-4 | 5-9-12 | 0-4-0 | |
| LOADIN | G (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATI | ES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.34 | Vert(LL) | -0.11 | 15-16 | >999 | 360 | MT20 | | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.41 | Vert(TL) | -0.23 | 15-16 | >999 | 240 | | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.50 | Horz(TL) | 0.05 | 13 | n/a | n/a | | | |
| BCDL | 5.0 | Code FBC2004/T | PI2002 | (Matrix | x) | ************************************** | | | | | Weigh | t: 247 lb | |

| LUMBER | |
|------------------|----------------|
| TOP CHORD | 2 X 4 SYP No.2 |
| BOT CHORD | 2 X 4 SYP No.2 |
| WEBS | 2 X 4 SYP No.3 |

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -

5-18

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=-15/0-3-8, 18=1757/0-3-8, 11=111/0-3-8, 13=1177/0-3-8

Max Horz 2=-116(load case 7)

Max Uplift 2=-266(load case 11), 18=-473(load case 5), 11=-201(load case 7),

13=-363(load case 7)

Max Grav 2=229(load case 10), 18=1757(load case 1), 11=111(load case 1), 13=1181(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-71/774, 3-4=-380/1014, 4-5=-252/992, 5-6=-337/266, 6-7=-684/316,

7-8=-671/517, 8-9=-354/361, 9-10=-459/350, 10-11=-109/479, 11-12=0/47

BOT CHORD 2-19=-670/177, 18-19=-670/177, 17-18=-220/385, 16-17=0/243, 15-16=-190/886,

14-15=-159/672, 13-14=-359/243, 11-13=-359/243

3-19=-178/140, 3-18=-347/485, 4-18=-248/247, 5-18=-1378/514, 5-17=-399/919,

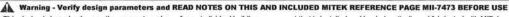
6-17=-911/438, 6-16=-211/712, 7-16=-207/293, 7-15=-228/43, 8-15=-33/123,

8-14=-434/215, 9-14=-112/61, 10-14=-288/842, 10-13=-1106/592

Julius Les Truss Design Engineer Flonds FE No. 24899 1100 Chastel Bay Slvri Boynton Besch, FL 22425

Continued on page 2

WEBS

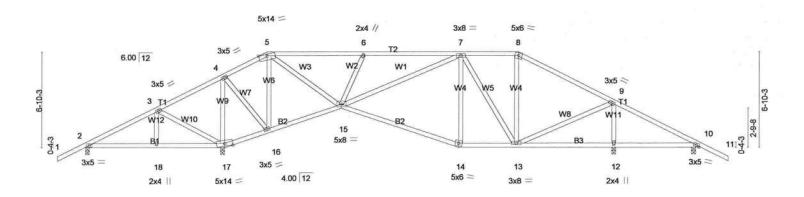




| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000770 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T24 | SPECIAL | 1 | 1 | 22 | J1968776 |
| | | | | CI: | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:38 2008 Page 1





| | 5-1-0 | 9-9-12 9-11-8 | - 1 | 18-4-0 | | 26-8-8 | 31-0-0 | 0 | 37-10-4 | | 43-8-0 | 14 ₋ 0-0 |
|--------|---------|-------------------|--------|--------|------|----------|--------|-------|---------|-----|---------|---------------------|
| 5-1-0 | | 4-8-12 0-1-12 3-0 |)-8 | 5-4-0 | | 8-4-8 | 4-3-8 | | 6-10-4 | | 5-9-12 | 0-4-0 |
| LOADIN | G (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L∕d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.33 | Vert(LL) | -0.12 | 14-15 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.46 | Vert(TL) | -0.23 | 14-15 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | YES | WB | 0.53 | Horz(TL) | 0.04 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | | | | | | Weight: | 250 lb |

| LUMBER | BRACING | |
|--------------------------|--|------------|
| TOP CHORD 2 X 4 SYP No.2 | TOP CHORD Structural wood sheathing directly a | pplied or |
| BOT CHORD 2 X 4 SYP No.2 | 6-0-0 oc purlins. | |
| WEBS 2 X 4 SYP No.3 | BOT CHORD Rigid ceiling directly applied or 6-0-0, Except: | oc bracing |
| | 10-0-0 oc bracing: 14-15.13-14. | |

REACTIONS (lb/size) 2=89/0-3-8, 17=1642/0-3-8, 10=190/0-3-8, 12=1108/0-3-8

13-0-0

Max Horz 2=113(load case 6)

Max Uplift 2=-240(load case 6), 17=-505(load case 5), 10=-238(load case 7),

12=-292(load case 4)

Max Grav 2=105(load case 10), 17=1642(load case 1), 10=192(load case 11),

12=1112(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-41/554, 3-4=-285/800, 4-5=-27/146, 5-6=-720/357, 6-7=-893/482,

7-8=-517/460, 8-9=-667/436, 9-10=-42/282, 10-11=0/47

BOT CHORD 2-18=-475/118, 17-18=-475/118, 16-17=-750/575, 15-16=-122/350, 14-15=-123/707

, 13-14=-111/661, 12-13=-196/161, 10-12=-196/161

3-18=-172/159, 3-17=-365/490, 4-17=-1186/556, 4-16=-337/948, 5-16=-890/367,

5-15=-332/982, 6-15=-440/317, 7-15=-49/260, 7-14=-147/68, 7-13=-346/119,

8-13=-34/98, 9-13=-231/758, 9-12=-1032/588

Julius Les Truss Cesign Engineer Flonds FE No. 24889 4100 Chastal Bay Blod Boynton Besch, FL 22405

JOINT STRESS INDEX

WEBS

2 = 0.62, 3 = 0.46, 4 = 0.74, 5 = 0.72, 6 = 0.33, 7 = 0.58, 8 = 0.48, 9 = 0.46, 10 = 0.55, 12 = 0.37, 13 = 0.69, 14 = 0.65, 15 = 0.90, 16 = 0.73, 17 = 0.71 and 18 = 0.33

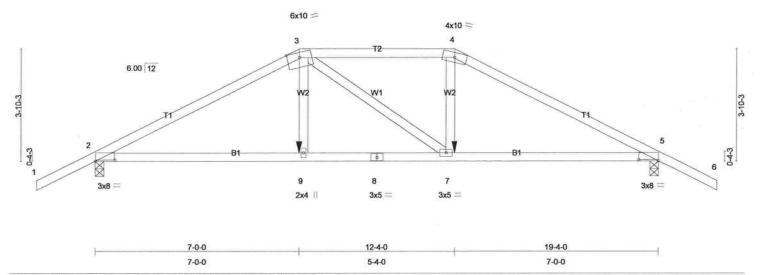
Continued on page 2





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:39 2008 Page 1





| Plate Of | fsets (X,) | (): [2:0-8-0,0-0-6], [5: | -0-8-0,0-0 | 6] | | | | | | | | |
|----------|------------|--------------------------|------------|------|------|----------|-------|-------|--------|-----|---------------|---------|
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L∕d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.44 | Vert(LL) | -0.08 | 7-9 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.47 | Vert(TL) | -0.16 | 7-9 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | NO | WB | 0.18 | Horz(TL) | 0.06 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TI | 212002 | (Mat | rix) | | | | | | Weight: 85 lb | |

| LUMBER | L | U | IV | IB | Ε | R |
|--------|---|---|----|----|---|---|
|--------|---|---|----|----|---|---|

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD

3-10-11 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-2-7 oc

Structural wood sheathing directly applied or

bracing.

REACTIONS (lb/size) 2=1335/0-3-8, 5=1335/0-3-8

Max Horz 2=-77(load case 6)

Max Uplift 2=-461(load case 5), 5=-461(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2303/690, 3-4=-1998/651, 4-5=-2304/690, 5-6=0/47

BOT CHORD 2-9=-582/1976, 8-9=-587/1997, 7-8=-587/1997, 5-7=-548/1977

WEBS 3-9=-115/510, 3-7=-123/125, 4-7=-134/566

JOINT STRESS INDEX

2 = 0.71, 3 = 0.85, 4 = 0.88, 5 = 0.71, 7 = 0.41, 8 = 0.70 and 9 = 0.36

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

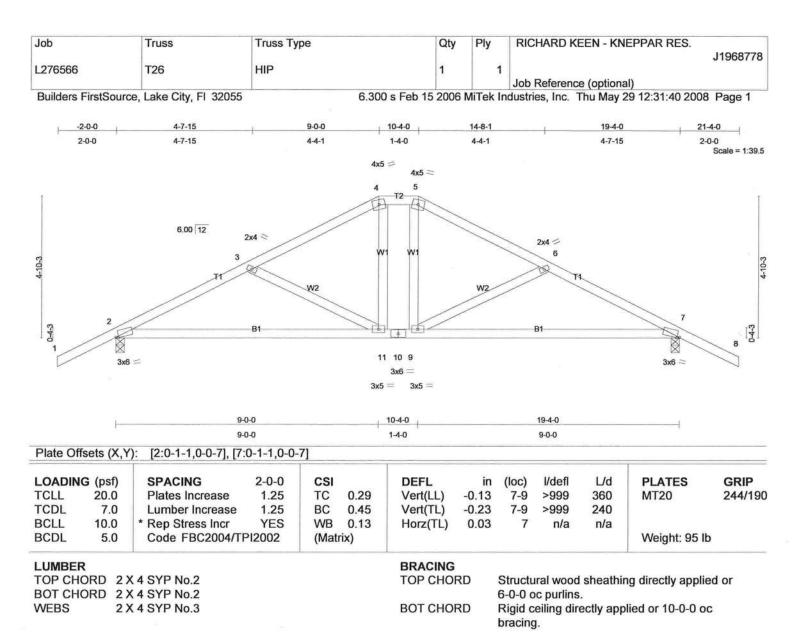
3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2





REACTIONS (lb/size) 2=725/0-3-8, 7=725/0-3-8

Max Horz 2=-89(load case 7)

Max Uplift 2=-226(load case 6), 7=-226(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1021/542, 3-4=-761/423, 4-5=-635/432, 5-6=-761/423,

6-7=-1021/542, 7-8=0/47

BOT CHORD 2-11=-324/858, 10-11=-122/635, 9-10=-122/635, 7-9=-324/858 WEBS 3-11=-276/227, 4-11=-62/203, 5-9=-62/203, 6-9=-276/227

JOINT STRESS INDEX

2 = 0.89, 3 = 0.33, 4 = 0.53, 5 = 0.53, 6 = 0.33, 7 = 0.89, 9 = 0.39, 10 = 0.68 and 11 = 0.39

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

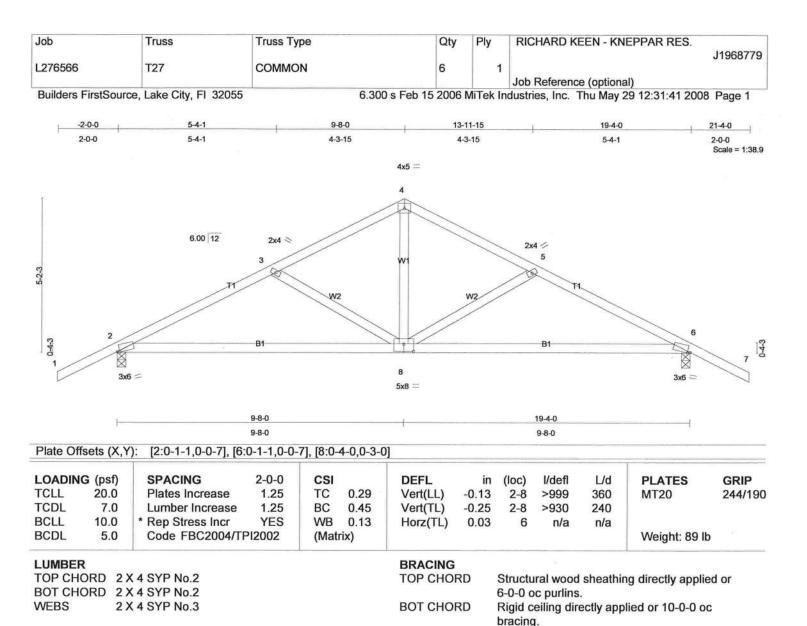
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Collynder on page 2

Julius Lee Truss Ossian Engineer Florida FE No. 34895 1 100 Coastal Bay Blvd Boynton Beach, FL 90495

May 29,2008







REACTIONS (lb/size) 2=725/0-3-8, 6=725/0-3-8

Max Horz 2=-93(load case 7)

Max Uplift 2=-229(load case 6), 6=-229(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-998/531, 3-4=-732/420, 4-5=-732/420, 5-6=-998/531, 6-7=0/47

BOT CHORD 2-8=-307/831, 6-8=-307/831

WEBS 3-8=-279/239, 4-8=-169/419, 5-8=-279/239

JOINT STRESS INDEX

2 = 0.84, 3 = 0.33, 4 = 0.55, 5 = 0.33, 6 = 0.84 and 8 = 0.68

NOTES

1) Unbalanced roof live loads have been considered for this design.

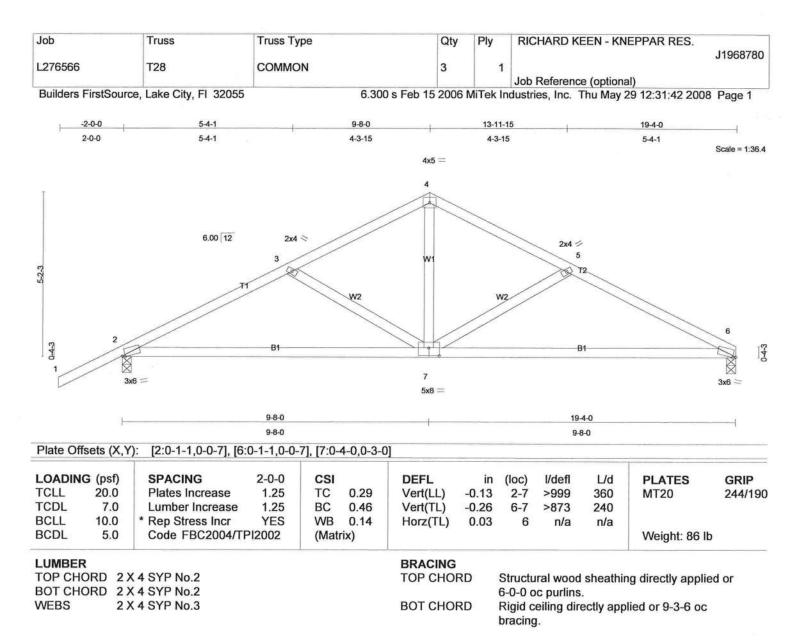
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

May 29,2008

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





REACTIONS (lb/size) 6=603/0-3-8, 2=732/0-3-8

Max Horz 2=106(load case 6)

Max Uplift 6=-132(load case 7), 2=-230(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1015/558, 3-4=-748/447, 4-5=-750/450, 5-6=-1018/585

BOT CHORD 2-7=-408/845, 6-7=-446/868

WEBS 3-7=-279/238, 4-7=-203/424, 5-7=-306/282

JOINT STRESS INDEX

2 = 0.84, 3 = 0.33, 4 = 0.51, 5 = 0.33, 6 = 0.84 and 7 = 0.69

NOTES

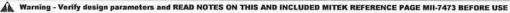
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

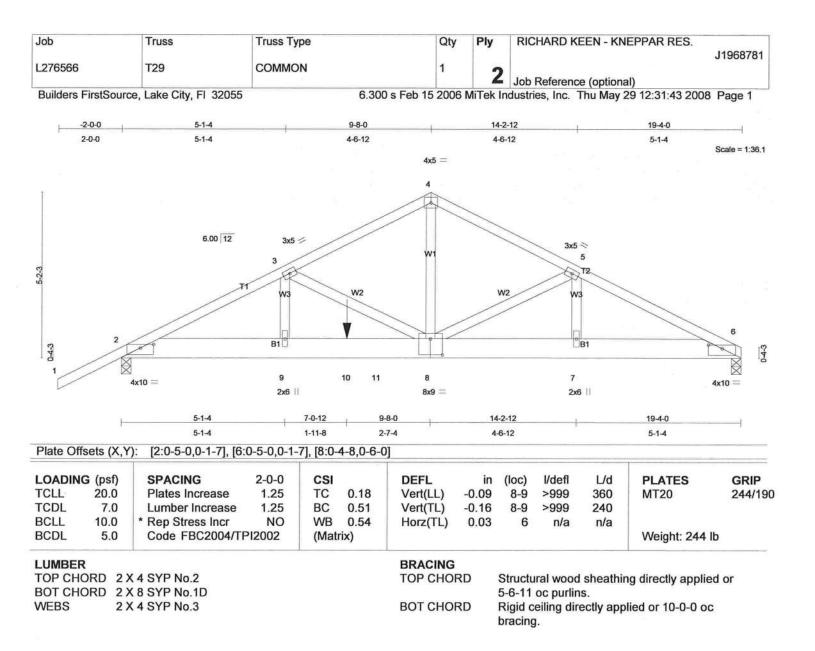
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Les Truss Design Engineer Florida FE No. 3-1869 1100 Caastal Bay Blvd Boynton Beach, FL 33-435

May 29,2008







REACTIONS (lb/size) 6=3357/0-3-8, 2=2560/0-3-8

Max Horz 2=112(load case 5)

Max Uplift 6=-894(load case 6), 2=-736(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-5189/1324, 3-4=-4129/1114, 4-5=-4130/1103, 5-6=-5737/1514

BOT CHORD 2-9=-1191/4597, 9-10=-1191/4597, 10-11=-1191/4597, 8-11=-1191/4597,

7-8=-1312/5098, 6-7=-1312/5098

WEBS 3-9=-164/743, 3-8=-1093/319, 4-8=-896/3370, 5-8=-1661/508, 5-7=-326/1213

JOINT STRESS INDEX

2 = 0.63, 3 = 0.51, 4 = 0.79, 5 = 0.51, 6 = 0.63, 7 = 0.29, 8 = 0.62 and 9 = 0.29

NOTES

 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

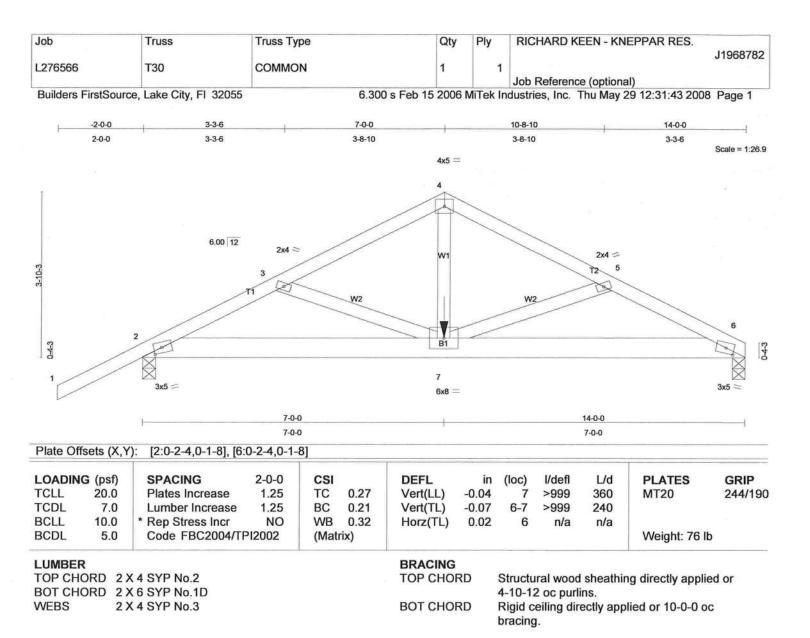
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads Connected S(F) as (B), unless otherwise indicated.

Julius Lee Truss Cession Engineer Flonds PE No. 24869 1100 Caustal Bay Sive Boynton Besch, FL 20425

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





REACTIONS (lb/size) 6=840/0-3-8, 2=974/0-3-8

Max Horz 2=94(load case 5)

Max Uplift 6=-260(load case 6), 2=-360(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-1621/498, 3-4=-1436/480, 4-5=-1437/469, 5-6=-1638/537

BOT CHORD 2-7=-449/1406, 6-7=-449/1428

WEBS 3-7=-189/100, 4-7=-338/1005, 5-7=-211/122

JOINT STRESS INDEX

2 = 0.80, 3 = 0.12, 4 = 0.47, 5 = 0.12, 6 = 0.80 and 7 = 0.24

NOTES

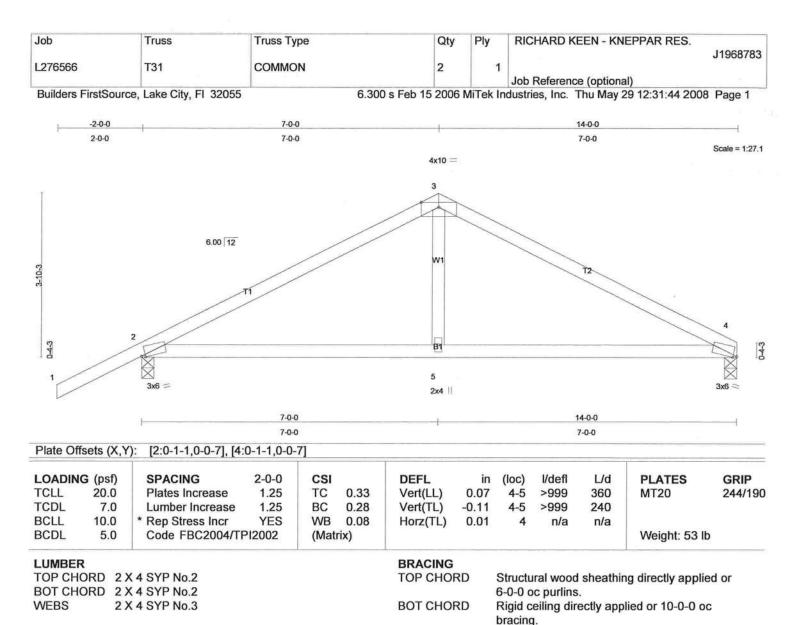
- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 Collain big and 360 lb uplift at joint 2.

Truss Cesign Engineer Florida PE No. 34868 1100 Chastal Bay Blvd Boynton Beach, FL 93435

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





REACTIONS (lb/size) 4=430/0-3-8, 2=564/0-3-8

Max Horz 2=90(load case 6)

Max Uplift 4=-94(load case 7), 2=-195(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-642/360, 3-4=-638/354

BOT CHORD 2-5=-214/501, 4-5=-214/501

WEBS 3-5=0/240

JOINT STRESS INDEX

2 = 0.86, 3 = 0.97, 4 = 0.86 and 5 = 0.17

NOTES

1) Unbalanced roof live loads have been considered for this design.

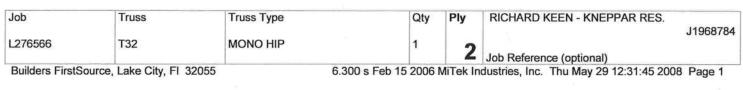
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

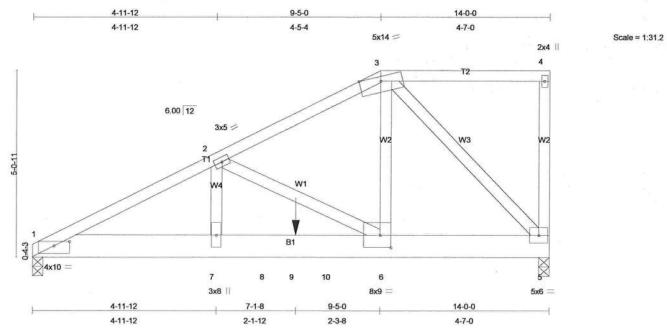
Continued on page 2

May 29,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE







| Plate Of | isets (X, Y | 7): [1:0-5-0,0-1-7], [6: | 0-3-8,0-4- | ·O] | | , | | | | | | |
|----------|-------------|--------------------------|------------|------|------|-----------------|-------|-------|--------|-----|----------------|---------|
| LOADIN | IG (psf) | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | I/defl | L∕d | PLATES | GRIP |
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.25 | Vert(LL) | -0.06 | 6-7 | >999 | 360 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.39 | Vert(TL) | -0.11 | 6-7 | >999 | 240 | | |
| BCLL | 10.0 | * Rep Stress Incr | NO | WB | 0.56 | Horz(TL) | 0.02 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TF | 212002 | (Mat | rix) | 10.89.000Mcan40 | | | | | Weight: 193 lb | |

| LUMBER | |
|------------------|-----------------|
| TOP CHORD | 2 X 4 SYP No.2 |
| BOT CHORD | 2 X 8 SYP No.1D |
| WEBS | 2 X 4 SYP No.3 |

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-11-13 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=2923/0-3-8, 5=3425/0-3-8

Max Horz 1=148(load case 5)

Max Uplift 1=-775(load case 5), 5=-937(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-5025/1316, 2-3=-2724/715, 3-4=-50/16, 4-5=-122/55 **BOT CHORD**

1-7=-1284/4459, 7-8=-1284/4459, 8-9=-1284/4459, 9-10=-1284/4459,

6-10=-1284/4459, 5-6=-697/2500

WEBS 2-7=-465/1819, 2-6=-2331/695, 3-6=-948/3495, 3-5=-3529/987

JOINT STRESS INDEX

1 = 0.55, 2 = 0.77, 3 = 0.56, 4 = 0.46, 5 = 0.35, 6 = 0.35 and 7 = 0.29

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.

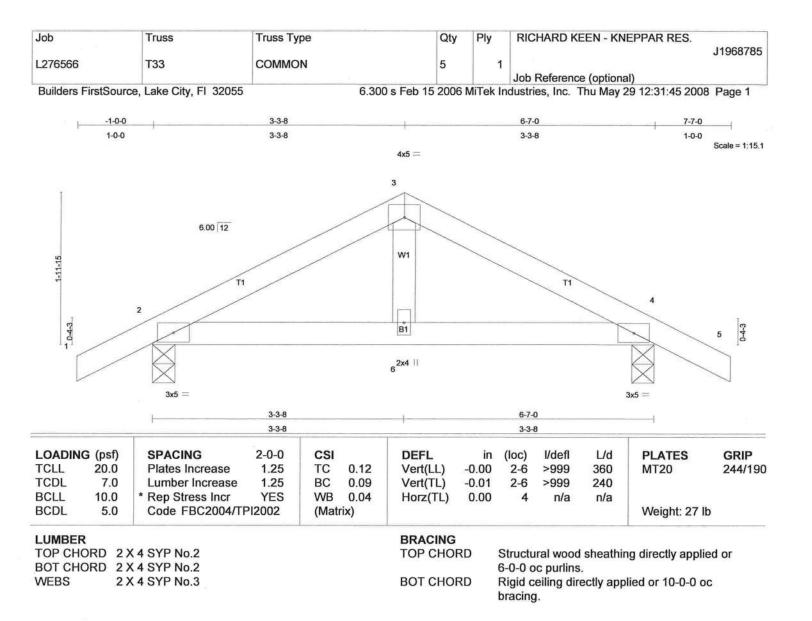
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads Contrible 35 (F) as (B), unless otherwise indicated.

May 29,2008







REACTIONS (lb/size) 2=263/0-3-8, 4=263/0-3-8

Max Horz 2=-37(load case 7)

Max Uplift 2=-96(load case 6), 4=-96(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/23, 2-3=-239/142, 3-4=-239/142, 4-5=0/23

BOT CHORD 2-6=-20/171, 4-6=-20/171

WEBS 3-6=0/115

JOINT STRESS INDEX

2 = 0.16, 3 = 0.05, 4 = 0.16 and 6 = 0.08

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 2 and 96 lb uplift at joint 4. Continued on page 2

Julius Lee Truss Cesign Engineer Florida FE No. 34868 1109 Cassial Bay Slvd Boynton Beach, FL 33435

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



| Job | Truss | Truss Typ | е | Qty | Ply | RIC | CHARD K | EEN - KN | EPPAR RES. | J1968786 |
|------------------------------------|--------------------------------------|----------------|---------------------|---------------------|-------------|------------|----------------|------------|------------------|------------------------|
| 276566 | T33G | COMMON | 1 | 1 | | 1 | | | | J 1900/00 |
| D :: F: | 1 -1 - 0't - El 20055 | | 0.000 | - F-1- 45 000 | > 8 6:T - I | | | e (optiona | | |
| Builders FirstSource | e, Lake City, FI 32055 | | 6.300 | s Feb 15 200 | o Miller | Industr | ies, Inc. | i nu May 2 | 29 12:31:46 200 | 8 Page 1 |
| -1-0-0 | 1 | 3-3-8 | | | | 6 | -7-0 | | | 7-7-0 |
| 1-0-0 | | 3-3-8 | | | | 3 | -3-8 | | | 1-0-0 Scale = 1:14. |
| | | | | 4 4x5 = | | | | | | 5545 |
| | | § 00 12 | | | | | 3x5 <> 5 | | | |
| | | 3 T2 | | W1 | | 12 | E. | | | |
| 2 | | | | | | | // | _ | 6 | |
| + | T1 | | | B1 | | | | | TI | |
| E-10 | | | | ВП | | | | | | 7 |
| | 5x7 = | | | 8 ^{2x4} | | | | 5x | 7 = | |
| | \boxtimes | | | 6 | | | | | \boxtimes | |
| | | 3-3-8 3-3-8 | | -1 | | | -7-0 -3-8 | | | |
| Plate Offsets (X,Y): | [2:0-3-8,0-2-13], [| COLORADORA | -13] | | | | - | | | |
| 1 | | | | | | | | | | |
| CLL 20.0 | SPACING Plates Increase | 2-0-0 1.25 | CSI TC 0.22 | DEFL Vert(LL) | in -0.01 | (loc) 8 | l/defl >999 | L/d 360 | PLATES MT20 | GRIP 244/19 |
| TCDL 7.0 | Lumber Increase | 1.25 | BC 0.10 | Vert(TL) | -0.01 | 8 | >999 | 240 | MILO | 2101 |
| BCLL 10.0 BCDL 5.0 | * Rep Stress Incr Code FBC2004/TI | NO | WB 0.04 (Matrix) | Horz(TL) | 0.00 | 6 | n/a | n/a | Weight: 29 | lb |
| JODE 3.0 | Code 1 BC2004/11 | 12002 | (Matrix) | | | | | | vveignt. 29 | ID |
| LUMBER TOP CHORD 2 X BOT CHORD 2 X | | | | BRACING TOP CHOI | RD | | ural wood | | ng directly appl | lied or |
| | 4 SYP No.3 | | | BOT CHO | RD | | eiling dir | | lied or 10-0-0 | ос |

REACTIONS (lb/size) 2=370/0-3-8, 6=370/0-3-8

Max Horz 2=-37(load case 7)

Max Uplift 2=-209(load case 6), 6=-209(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-5/34, 2-3=-357/224, 3-4=-315/219, 4-5=-315/219, 5-6=-357/224, 6-7=-5/34

BOT CHORD 2-8=-116/284, 6-8=-116/284

WEBS 4-8=0/111

JOINT STRESS INDEX

2 = 0.62, 3 = 0.00, 3 = 0.31, 4 = 0.07, 5 = 0.00, 5 = 0.31, 6 = 0.62 and 8 = 0.08

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Charling page assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee Truss Design Engineer Florida ME No. 34866 1106 Chastal Bay Slvd. Boynton Besch. Ft. 93435

May 29,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

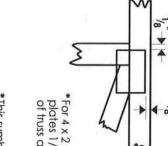


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless plates to both sides of truss and Dimensions are in inches. Apply securely seat. dimensions indicate otherwise.



*For 4 x 2 orientation, locate of truss and vertical web. plates 1/8" from outside edge



*This symbol indicates the required direction of slots in connector plates

PLATE SIZE

4 × 4

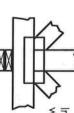
dimension is the length parallel perpendicular to slots. Second The first dimension is the width to slots.

LATERAL BRACING



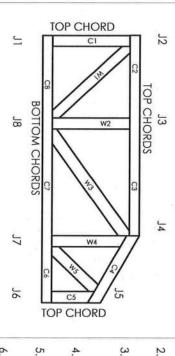
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO

BOCA

96-31, 96-67

SBCCI

9667, 9432A 3907, 4922

WISC/DILHR 960022-W, 970036-N

NER

561





General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

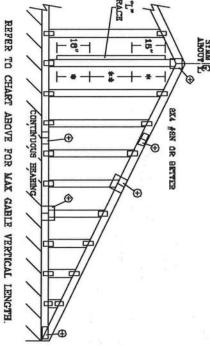
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 2 Cut members to bear tightly against each
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- 4 Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- S Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 0 Unless expressly noted, this design is not preservative treated lumber. applicable for use with fire retardant or
- 7. is the responsibility of truss fabricator. General Camber is a non-structural consideration and practice is to camber for dead load deflection
- 00 shown indicate minimum plating requirements Plate type, size and location dimensions
- 9 in all respects, equal to or better than the Lumber shall be of the species and size, and grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed unless otherwise noted
- 12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials
- Do not cut or alter truss member or plate engineer. without prior approval of a professional
- Care should be exercised in handling erection and installation of trusses

0 1993 MiTek® Holdings, Inc.

MiTek Engineering Reference Sheet: MII-7473

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I II 1.00, EXPOSURE C

| | |] | M | A | X | | C | i I | I | 3] | | 3 | • | V | E | R | T | Ί | C. | A | L | | L | E | N | [(| רי | Ή | | |
|---|--|---|--|---|---------------------------|--------------------------|---------|---------|---------|--------------------|---------------------------------|------------|-----------------|----------|----------|---------|--------|----------------|----------|--------|---------------------------------|-----------|---|----------------------|--------|--------|-----------------------------------|-------------------|----------------|-----|
| | 3 | 1 | 2 | 93 | | 0 | .(| 3. | 95 | | 1 | 6 | 51 | | 0 | .(| 3. | , | | 2 | 4 | 31 | | 0 | .(| C | | SPACING | CART | |
| | | L F | 1 | (V) | j | TIL | ij | מלק | בו בו | | L H | 1 | \(\frac{1}{2}\) | j | TIL | ij | ピスプ | בו | | D F'L | 1 | | ì | TIL | ij | מלק | מחו | SPACING SPECIES | CABIE VERTICAL | |
| | STANDARD | STUD | ‡ 3 | #22 | #1 | STANDARD | STUD | i S | £1 / #2 | STANDARD | STUD | * 3 | 器 | 41 | STANDARD | CUITS | ₽‡ | 打 / #2 | STANDARD | STUD | 43 | 記券 | 11 | STANDARD | STUD | ₽\$ | \$1 / #2 | GRADE | BRACE | |
| | 4' 0" | 4. 23, | 4, 2, | 4' 4" | | 3' 11" | | | 4' 0" | 3' 8" | 3, 8, | 3. 8. | 3' 11" | 4' 0" | 3' 7" | 3' 7" | 3' 7" | 3. 8 | 3' 0" | 3' 3' | 3. 3. | 3' 6" | 3' 6" | 2' 11" | 3' 1" | 3' 1" | si, | BRACES | 5 | |
| | 5 6 | 6' 4' | 6' 6" | 6' 11" | | 5' 4" | 6' 3" | B' 3" | 6' 11" | 4' 9" | 5' 6" | 5. 7. | 8' 4" | B' 4" | | 5' 6" | | | 3' 10" | 4 6 | 4. 6. | 5, 6, | | | 4' 6" | 4' 5" | 5' 6' | GROUP A | (1) 1X4 | |
| | 5' 6" | 6' 4" | 6' 5" | 7' 6" | 7' 8" | 5' 4" | 6' 3" | 6, 3, | 7' 2" | 4' 9" | 5' 6" | 6. 7. | 8' 10" | B' 10* | 4. B. | 6' 5" | 5' 5" | 6' 6" | | 4' 6" | | 5' 11" | | 3′ 9" | 4' 5" | 4' 5" | 6′ 8″ | GROUP H | "L" BRACE . | |
| | 7' 3" | 8' 3" | 8' 3" | | 8' 3" | 7' 1° | B' 3" | 8, 3,, | 6' 3" | 6' 3" | | 7. 4. | 7' 6" | | | 7' 2" | 7' 2" | 7' 6" | 6" 1" | | | 6, 6, | - | 6, 0, | | e, 10. | 6, 6, | GROUP A | (1) 2X4 | |
| CAS | 7' 3" | 8' 6" | | 8' 11" | | | | | 8' 6" | | 7' 3" | | B' 1* | | | 7' 2" | | | | | | 7' 0" | 7' 0° | 5' 0" | | 5' 10" | 6' 9" | GROUP B | "L" BRACE . | |
| SAMUA IE | 8, 8, | 9′ 10″ | | 8, 10, | | 9, 6,, | 9' 10" | 8, 10,, | 9, 10, | B' 5" | | 8' 11" | 8' 11" | B' 11" | 11.7 | | | 8' 11" | | 1 | | 7' 10" | | 8, 9, | 7' 10" | 7' 10" | 7' 10" | GROUP A | (2) 2X4 | |
| | 8' 9" | | 10′ 4″ | 10' 7" | 10' 7" | 9' 6" | 9' 10" | 9' 10" | | | 8, 5, | | 8, 4, | | | 8' 11" | 8' 11" | 6. 55 | - 6° 11° | 8' 0" | 8, 1. | 8, 2, | 8, 2, | 6, 9, | 7' 10" | 7' 10" | | GROUP B | "L" BRACE ** | PCS |
| | 11' 4" | 12' 11" | 12' 11" | 12. 11. | 12' 11" | 11' 1" | 18, 10. | 12' 11" | 12' 11" | 8, 8, | 11' 4" | 11. 5. | 11' 9" | 11, 8, | 8. 2. | 11. 1" | 11, 5, | 11. 9. | B' 0* | 8, 3, | 9' 4" | 10′ 3° | 10' 3" | 7' 10" | 9' 1" | 9, 1, | 10' 3" | GROUP A | (1) exe "L" | 0 |
| | 11' 4" | | 18, 3, | 13' 11" | 13' 11" | 11' 1" | 12' 10" | 12' 11" | 15' 4" | 8, 8 _{ll} | 11, | 11. 6. | | 12, 8, | 8. 4. | 11. 1. | 11. 2. | 12 1 | 8, 0, | 8, 3, | 8 4 | 11, 1, | F. | 1 | 9' 1" | 9' 1" | 10′ 7″ | | L BRACE * | |
| | 14' 0" | 14. O. | 14' 0" | 14' 0" | 14. 00 | 14' 0" | 14' 0" | 14, 00 | T4. 0" | 13' 3" | 14 0 | 14. O. | 14' 0" | 14' O* | 18. 11. | 14' 0" | 14. 0 | -1 | 10' 10" | 12' 3" | 12' 3' | | 12' 3° | 10' 7" | *B 'S1 | 12' 3" | 12' 3" | CROUP B CROUP A | (2) ZXB "L" | |
| | 14' 0" | 14. 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0° | 14' 0" | 13' 9" | 14 00 | 14. 0. | | 14' 0" | 12' 11' | 14' 0° | | 14. 0. | | | 12, 8, | - 1 | 13' 2" | | | 12' 3" | 12' 7" | скопь в | BRACE ** | |
| DITTOMORES WITH E O DYEMHANG, DR 12" PLYMOND OVERLANG. | The state of the s | CONTINUING BEADING /A DEE TO DEAT TOATH | BEAUTY IN THE PROPERTY OF THE PARTY OF THE P | LIVE LOAD DEPLICATION CRETERIA IS 1/240 | CARLE INCOS DETAIL NOTES: | CARLE TRIES DETAIL MOTES | | | *2 | | SOUTHERN PINE DOUGLAS FIR-LARCH | | AT & BIR | SEEM-PER | GROUP B: | Oppin D | | CONTRACTOR CO. | <u>-</u> | 25 | DOUGLAS FIR-LARCH SOUTHERN PORE | | TOTAL SE STATE OF THE STATE OF | RUCK-PINE-ITR HEK-FO | OUT A | | BRACING GROUP SPECIES AND GRADES: | - | | |



DIAGONAL BEACE OPTION:
VERTICAL LENGTE MAY BE
DOUBLED WHEN DIAGONAL
HEACE IS USED. CONNECT
IMACONAL BEACE TOR SEGS
AT EACH IND. MAX WEB
TOTAL LINGTH IS 14*.

SEDEL THUSS

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR
DIT-L #2 OH
BETTER DIAGONAL
BRACE; SUGLE
OR DOUBLE
CUT (AS SHOWN)
AT UPPER END

MIDPOINT OF VERTICAL WEB.

| _ |
|-------|
| THA |
| 7 |
| SSONI |
| DE. |
| I'AIL |
| NOTES |
| |

ATTACE EACH "L" BRACE WITH 104 MAIS.

FOR (1) "L" BRACE, SPACE WAITS AT 8" O.C.

FOR (2) "L" BRACES, SPACE MAIS AT 3" O.C.

FUR (2) "L" BRACES; SPACE MAIS AT 3" O.C.

IN 18" IND ZONIS AND 4" O.C. BETWIEN ZONES. "I" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH

| 2.5X4 | REFER TO COMMON TRUS |
|------------|----------------------|
| 2014 | GREATER THAN 11' B" |
| 1X4 DR BX3 | IPSS THAN 4' 0° |
| ND SPLICE | VEHTICAL LENGTH |
| RAZIE 3 | GABLE VERTICAL PLAT |

| RAYMONDIA: TRANSES REBURE EXTREME CARE IN FARRICATING, HANDLING, SADPING, DISTALING AND RAYMONDIA: THE STATE AND REPORT OF AIRCRAFT OF SATE INTERPRETATION OF THE STATE OF THE | |
|--|---|
| E CO | 0 |

| P | | | AA | S S | |
|-------------------------------|----------------------|-------|----------------------------------|---------|--------------|
| No: 34869 STATE OF FLORIDA | | | DELEGAL BETWEEN AL SOUTH - STOIL | NEERS P | S. HH. IIII. |
| MAX. | MAX. | | | | |
| MAX. SPACING 24.0" | MAX. TOT. LD. 60 PSF | | | | |
| CING | Ē | | | | |
| 24 | 60 | | | | |
| o _n | PSF | V1111 | | | |
| | 5: | -ENG | DWG M | DATE | REF |

ITEK STD GABLE 30' E HT 11/26/09 ASCE7-02-GAB13030

BOP CHORD CHORD WEBS 284 200 R BETTER

PIGGYBACK DETAIL

TYPE

SNAGE

셤

궁

30

34

86

58

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED.
TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PEGCYHACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY HE APPLIED HENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

110 MPH WIND, 30' MEAN HGT, ASCE 7-03, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FEG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF, WIND BC DL-5 PSF

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND HC DL=6 PSF

H

2 584

OR 3X6 TRULOX AT 4' OC, HOTATED VERTICALLY

U

5X6

635

5X6

0

BX6.

1.5X4

1.6X4

1.5X4

ш

4X8

5X8

8X6

5Xe

>

284

2.5X4

2.6X4

3XE

FRONT FACE (5,*) FLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. ACCEPTABLE LOCATION IS MAX C *ATTACH PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE 20' FLAT TOP CHORD MAX SPAN ы H 要 MAX SIZE OF ZX12 H C-TYP. D-SPLICE 中

AC

D

O

* PICCYBACK SPECIAL PLATE

| ATTACH THULOX EQUAL, PER FACE BE CONNECTED. INFORMATION. |
|--|
| ITACH THULOX PLATES WITH (8) 0.120" X 1.575" NAILS, OR SQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR THULOX NFORMATION. |
| |

| WEB LENGTH | REQUIRED BRACING |
|-------------|--|
| o' To 7'9" | NO BRACING |
| 7'9" TO 10' | 1x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC. |
| 10' TO 14' | 2x4 "t" brace. Same grade, species as web member, or better, and 80% length of web member. Attach with 16d nails at 4" oc. |

| | - | | |
|-------|----|--------|--------|
| | - | 0 0 | |
| 0 0 0 | b | a 0 | |
| 0 0 | ų. | 0 | (|
| | - | 0 |)) |

C

O

THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

| | | | STRUCTURAL PARELY AND BUTTON CHICKO SHALL HAVE A PROPERLY ATTACHED RIGIO CELLOIG. | CTIONS. UNLESS OTHERWISE INDICATED, TOP CHOR | WYARKIGIAW TRUSSES REGURE CYREME LARE IN FABRICATING, HWOLING, SHIPPING, INSTALLING AND BRACING REFER TO BEST I'D BEST I'D BUILLING COMPONENT SARETY INFORMATION), PUBLICHED BY TPI (TRUSS PALTE INSTITUTE SPA ITMIFTED THE STRUKEN SHIPPING IN 1. 1975) AND I'D AND TONG STRUKEN SHIPPING IN 1. 1975) AND I'D AND TONG STRUKEN SHIPPING IN 1. 1975 AND TONG STRUKEN SHIPPING SHIPPI | |
|------------------|-----------------------------|----------------|---|---|--|-------------------|
| STATE OF FLORIDA | H | | | TOTAL AND MAN AND DALL OF THE MAN AND THE | CONS. ENGINEERS P.A. | 7, HH J SIII IIII |
| SPACING 24.0" | 47 PSF AT 1.15 DUR. FAC. | 1.25 DUR. FAC. | 50 PSF AT | 1.33 DUR. FAC. | 55 PSF AT | MAX LOADING |
| | | | -ENG JL | DRWGMITEK STD PIGGY | DATE 09/12/07 | REF PIGGYBACK |

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

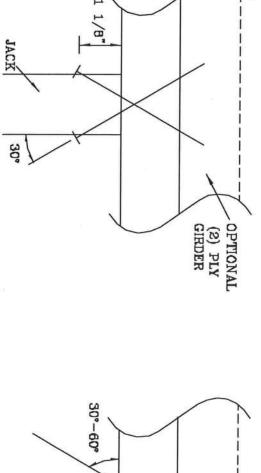
PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 END DISTANCE, SPACING: "EDGE DISTANCES, SPACINGS FOR NAILS AND SPIKES SHALL BE PREVENT SPLITTING OF THE WOOD." - EDGE DISTANCE, END DISTANCES AND SUFFICIENT TO

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

| | | _ | _ | 1 | 1. | . 17 |
|---|------|------|------|------|------------|-------------------|
| ALL VALUE | 5 | 4 | မ | ผ | STIWN-30.1 | NUMBER OF |
| S MAY BE | 493# | 394# | 296# | 187# | 1 PLY | |
| MULTIPLIE | 639# | 511# | 383# | 256# | 2 PLIES | SOUTHERN PINE |
| D BY APP | 452# | 361# | 271# | 181# | 1 PLY | DOUGLAS |
| ROPRIATE | 585# | 468# | 351# | 234# | 2 PLIES | DOUGLAS FIR-LARCH |
| DURATION | 390# | 312# | 234# | 156# | 1 PLY | |
| ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR | 507# | 406# | 304# | 203# | 2 PLIES | HEM-FIR |
| ACTOR | 384# | 307# | 230# | 154# | 1 PLY | SPRUCE |
| | 496# | 397# | 298# | 189# | 2 PLIES | PINE FIR |



THIS DRAWING REPLACES DRAWING 784040

JACK

ALTERNATIVE CONDITION

1/8"

GIRDER

OPTIONAL (2) PLY

| | | | E S | ED DR., SUITE 200, NADISON, WE 537(9) AN LM, NADISON, WI 537(9) FOR SAFETY PRACE ERVISE INDICATED, TOP CHIRD SHALL HAVE | JAK EXTREM | |
|------------------|-----------|----------|---------|---|----------------------|----------------|
| STATE OF FLORIDA | No: 34869 | | | DELICAY BEACH, FL 33444-2161 | CONS. ENGINEERS P.A. | S, HET SOITION |
| SPACING | DUR. FAC. | TOT. LD. | BC LL | BC DL | TC DL | TC LL |
| | 1.00 | PSF | PSF | PSF | PSF | PSF |
| | | | -ENG JL | DRWG | DATE | REF |
| | | | JL . | CNTONAIL1103 | 09/12/07 | TOE-NAIL |

TRULOX CONNECTION DETAIL

SHOWN (+). 11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE

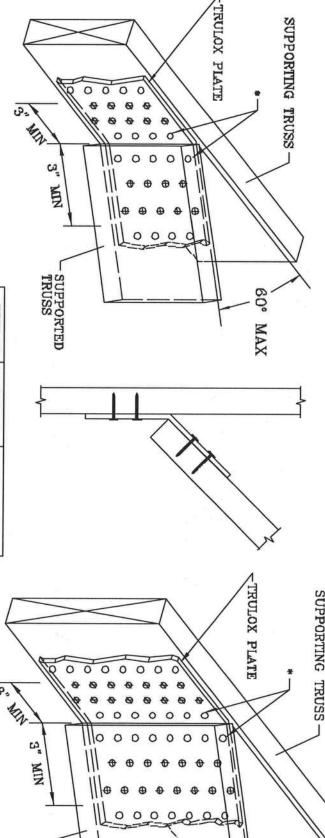
NAILS MAY BE OMITTED FROM THESE ROWS

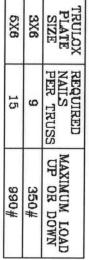
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST

EXCEED THE TRULOX PLATE WIDTH.

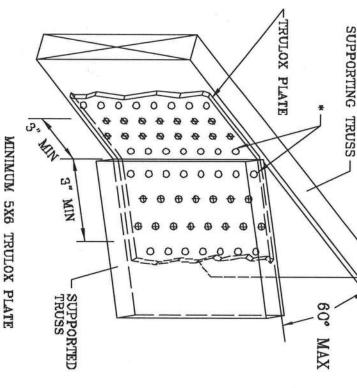
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN





MINIMUM 3X6 TRULOX PLATE

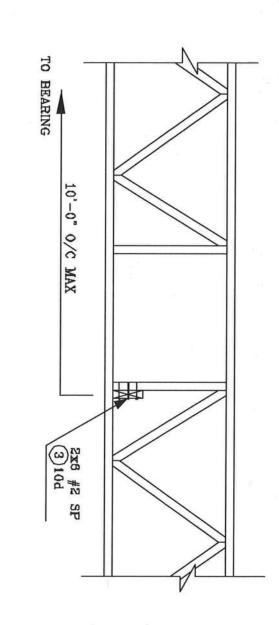


1,154,844 THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,152,217 1,152,017 1,159,154 & 1,151,524

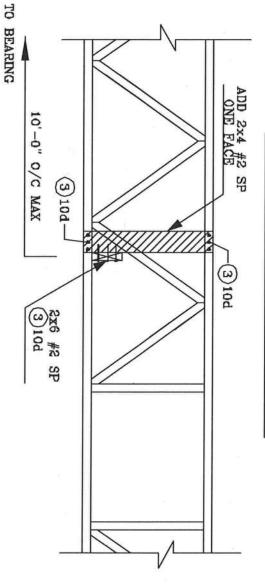
NINGAM TRUSSES REQUIRE EXTREME CARE IN FABRICATTM, HAMILING, SHOPPIN NS. REFER TID 8231 1-04 (GUILLIDING LODGO)REMT SAFETY DEFORMTION, PUBLISS ONSTITUTE, 3883 D'ONIFREID IN, SUITE BOD, MOUSIDN, V.Z. 33735 AND VITCA ON ERIDA, 6400 ENTERROSE LA, MADISCIN, VE 33735 FOR SAFETY PRACTICES FRIE FLAVOTIONS. UNLESS DIFEREYESE MOUBLAFED, TO "GORD SHALL HAVE PROFESS"

| | D CELTING | OUR TO PERFORMING | DAG, JASTALLING AND | |
|-------------------------------|-----------|------------------------------|---------------------|--------------|
| No: 34869 STATE OF FLORIDA | 1 | DELEAY BEACK, IL. 35444-SIEL | _ | JULIUS LEE'S |
| , | -ENG JL | DRWG | DATE | REF |
| |) IL | CNTRULOX1103 | 11/26/09 | TRULOX |

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S CONS. ENGINEERS P.A.
1425 SM 4th AVENUE
DELRAY BEAGE, 7L 33444-2161

No: 34869 STATE OF FLORIDA

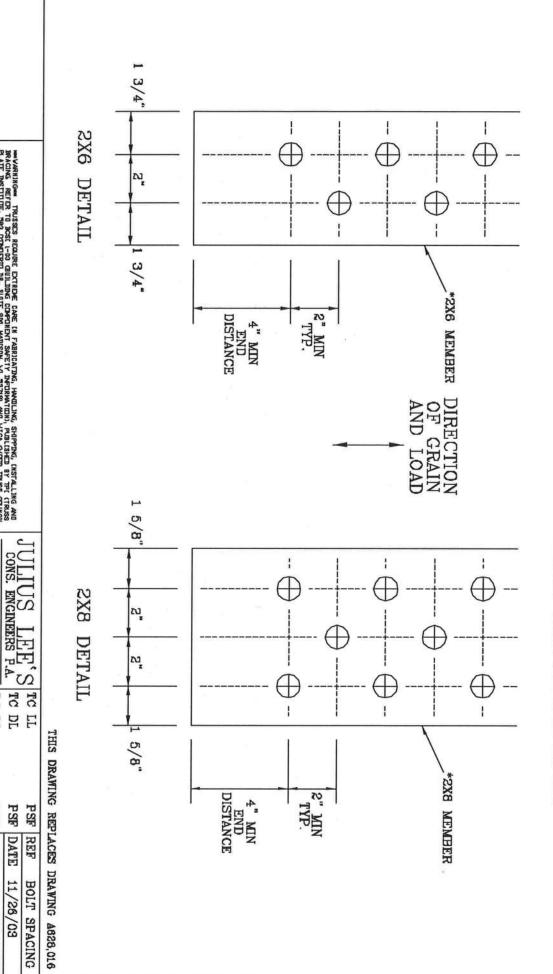
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/S2" OF 1/16" LARGER THAN BOLT DIAMETER. TO A MAXIMUM

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PAITERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



DELRAY BEACH, FL 33444-2161

TC DL BC DL

DATE DRWG -ENG

No: 34869 STATE OF FLORIDA

SPACING

DUR. FAC

TOT. BC LL

Ð.

PSF PSF PSF PSF

H

CNBOLTSP1103 11/26/03

VALLEYTRUSS DETAIL

TOP CHORD CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTEH.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: BUILDING, EXP. C. RESIDENTIAL, WIND TO DI=5 FEC 2004 110 MPH, ASCE 7-02 110 MPH WIND ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, 18d HOX (0.135" X 3.5") NAILS TOE-NAILED FOR OR (3) 16d 1 ENCLOSED PSF. FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH INSTALLATION PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS

ENGINEERS' SEALED DESIGN. PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

* ++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

12 MAX.

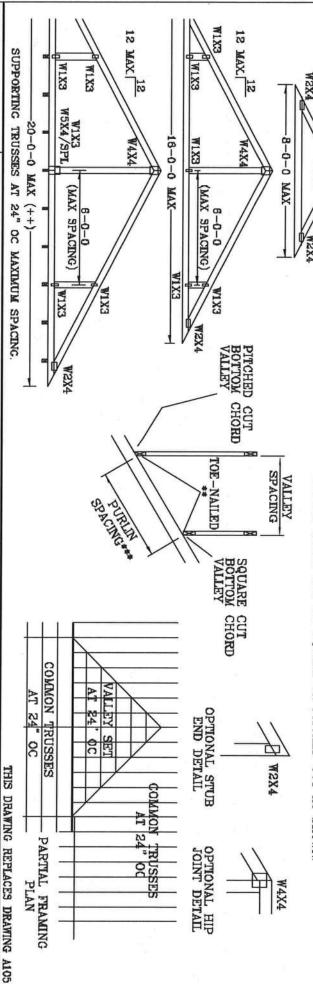
W2X4

12

MAX

NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



- PARTIAL FRAMING - PLAN

CONS. ENGINEERS P.A. DELEATE BEACH, IL 33444-2161 STATE OF FLORIDA Na: 34869 BC II BC DI SPACING C DUR.FAC. 1.25 TOT. LD PL 32 24 1.25 40 15 PSF PSF PSF PSF DATE DRWG -ENG

HH

S

TC

20

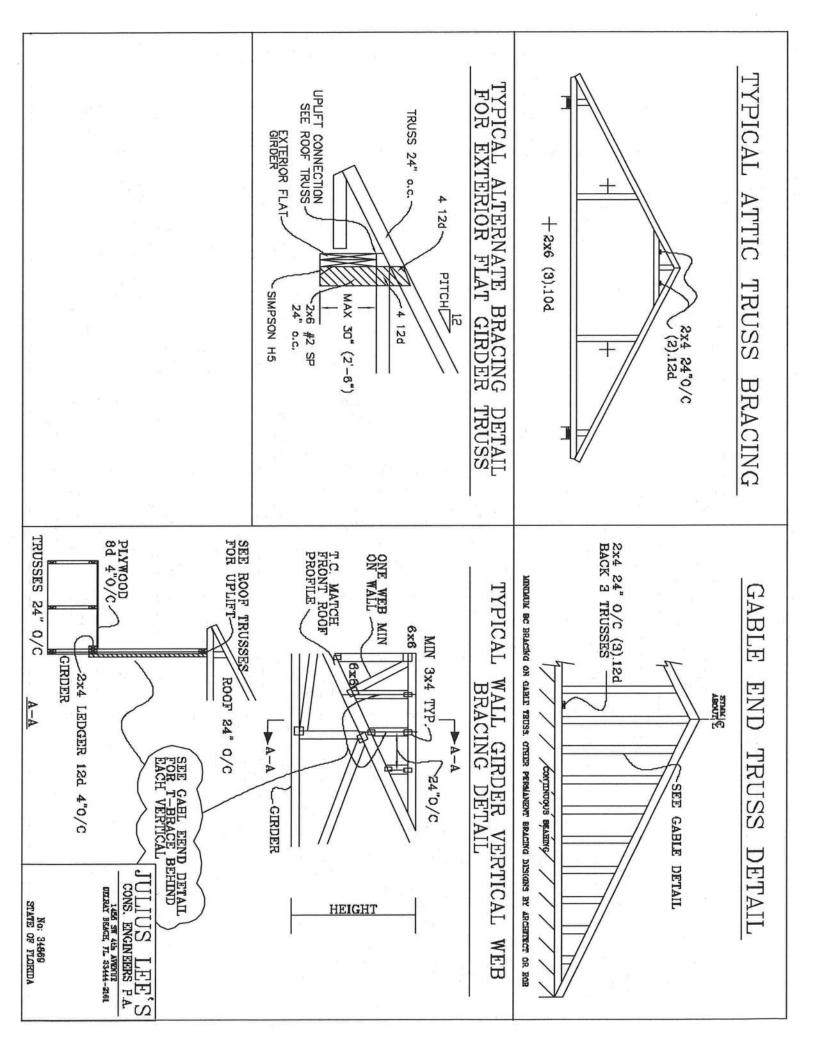
20

PSF REF

VALTRUSS1103

11/26/09 VALLEY DETAIL

IL



DIAGONAL BRACE OPTION: VENTICAL LENGTH MAY BE DOGRACE BY USED. CONNECT ERACE ES USED. CONNECT ENACONAL BRACE FOR 840 # GABLE VERTICAL LENGTH TOTAL LENGTH IS 14". AT EACH END. MAX WEB MAX SPACING SPECIES GRADE VERTICAL LENGTH SHOWN IN TABLE ABOVE. 24" O.C. O.C. O.C. 16 GABLE VERTICAL CONNECT DIAGONAL AT SPF SPF SPF DFL DFL SP SP H H H ASCE STANDARD #1 / #2 STANDARD STANDARD STANDARD STANDARD STUD STUD CLLIS COLS BHA BRACE 7-02: ***MARUNG** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, MANILLING, MODPANG, INSTALLING AND IRACING. REFER TID BESS 1-40 (BUILLING CAPPENG) SAFETY IN FROM TIDIO, MUBLISHED BY THE (TRUSS EDIAMEL INSTALLING, SHE SHOUTHER MANISON, MANISON, MASS STANDAMEN VITA (MUDICAL MANISON, MANISON #2 GABLE TRUSS BRACES 3. 4. 10. 2 4 2 8 130 GROUP A (1) 1X4 "L" BRACE * ZZ4 SP #ZN, DF-L #Z, SPF #/#Z, OR BETTER DIAGONAL BRACE; SINGLE OR DOUBLE COT (AS SERVE) AT MPH 10 UPPER END. GROUP H WIND 6' 11" ų 4' 3" œ. 10 v v o GROUP A GROUP B GROUP A (1) 2X4 "L" BRACE * SPEED, 7' 10" REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH 15 18" SYNOW E 18 (2) 2X4 "L" BRACE ** 10' 5" 10' 5" 6 Ø MEAN 10' 5" 8 EX4 #EN OR BETTER GROUP B CONLINCOR BRYBING 10' 6" 10' 2" 10 HEIGHT, 10 0 0 C (1) 2X6 °L° BRACE • (2) 2XB GROUP A ULIUS LEI 12 5" 10' 10" DELEAY BEACH, FL 33444-2161 13' 8' 12 4 4 6 10' 10' Ö 10 3 12 6 2 ä No: 34869 STATE OF FLORIDA O, 1 ENCLOSED, GROUP B 12' 6" 13' 5" 12. 8 10, 12, 2 12' 3" 13 ä F.A.S CROUP A 12 11 12 14' 0" Н r. XAX MAX. TOT. GROUP B BRACE 11 2 14 0 13 14' 0" 1.00, SPACING Ē ATTACE EACH "L" BRACE WITH 104 NAILS. # FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. # FOR (2) "L" BRACES; SFACE NAILS AT 3" O.C. ## FUR (2) "L" BRACES; SFACE NAILS AT 3" O.C. IN 18" IND ZONIN AND 6" O.C. BETWIEN ZONES. CARL END SUPPORTS LOAD FROM 4, 0, PROVIDE UPLAT CONNECTIONS FOR 136 FLF OVE CONTINUOUS BEARING (6 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. T." BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTK. DOUGLAS FIR-LARCH #3 STUD STANDARD SIPRUCE-PINE-INR #3 STUD BRACING GROUP SPECIES PLYWOOD OVERHANG. EXPOSURE CABLE TRUSS DETAIL NOTES: 60 GREATER THAN 4' D' BUT LESS THAN 11' B' GREATER THAN 11' B' PEAK, SPLICE, AND HEEL PLATES. ARMIN' 7, 0, 24.0" GABLE VERTICAL PLATE SIZES PSF 8 DRWG DATE REF GROUP GROUP MI & BIR HEM-PIR 0 DOUGLAS FIR-LARCE MITEN SID GABLE 15 E HI 11/26/09 B A: ASCE7-02-GAB13015 SOUTHERN POR AND STANDARD St #3 2.534 力性 22 STANDARD GRADES: CIVIS

| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T33G | COMMON | 1 | 1 | | J1968786 |
| LETOCOO | 1000 | COMMON | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:46 2008 Page 2

NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 2 and 209 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-79(F=-25), 4-7=-79(F=-25), 2-6=-10

Julius Les Truss Design Engineer Florida Fis No. 24899 1106 Calestel Bay Blvd Boynton Beson, Ft. 20405



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-------|-----------------------------|----------|
| L276566 | T33 | COMMON | 5 | 1 | | J1968785 |
| | | | | 0.400 | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:45 2008 Page 2

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida FE No. 34898 1 109 Chastal Bay Blvd Boynton Besch, FL 33436



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|----------|-------|------------|-----|-----|-----------------------------|----------|
| 1 276566 | T32 | MONO HIP | 1 | | | J1968784 |
| L270300 | 102 | MONOTHI | | 2 | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:45 2008 Page 2

NOTES

- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 4) Provide adequate drainage to prevent water ponding.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 775 lb uplift at joint 1 and 937 lb uplift at joint 5.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 1-8=-285(F=-275), 8-10=-10, 5-10=-431(F=-421)

Concentrated Loads (lb)

Vert: 9=-1300(F)

Julius Lee Truss Design Engineer Flonds FE No. 24856 1 108 Coasts! Bay Blvd Bovoton Besch, FL 99425





| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T31 | COMMON | 2 | 1 | | J1968783 |
| | | | | | Job Reference (optional) | |

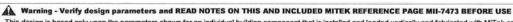
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:44 2008 Page 2

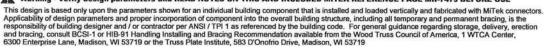
NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 4 and 195 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonds FE No. 2-1898 1 100 Crestal Bay Blvd Bovnton Beach, FL 20425







| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 11000700 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T30 | COMMON | 1 | 1 | , | J1968782 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:43 2008 Page 2

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-6=-10

Concentrated Loads (lb)

Vert: 7=-821(F)



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T29 | COMMON | 1 | _ | | J1968781 |
| 270000 | 120 | COMMON | | 2 | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:43 2008 Page 2

NOTES

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 894 lb uplift at joint 6 and 736 lb uplift at joint 2.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

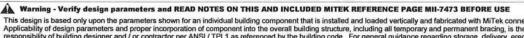
Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-294(F=-284)

Concentrated Loads (lb)

Vert: 10=-1405(F)

May 29,2008





| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T28 | COMMON | 3 | 1 | | J1968780 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:42 2008 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 6 and 230 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Ossian Engineer Florida FE No. 34898 1109 Cesstal Rey Blvd Dovnton Beson, FL 93495



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000770 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T27 | COMMON | 6 | 1 | | J1968779 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:41 2008 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 2 and 229 lb uplift at joint 6.

LOAD CASE(S) Standard



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-----------|------------|------|-----|-----------------------------|----------|
| | | 789 | 35.5 | 100 | | J1968778 |
| L276566 | T26 | HIP | 1 | 1 | | |
| | 1 1000000 | 1 | | | Job Reference (optional) | |

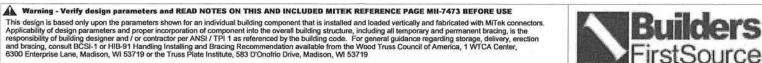
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:40 2008 Page 2

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 2 and 226 lb uplift at joint 7.

LOAD CASE(S) Standard





| I - KNEPPAR RES. |
|------------------|
| J1968777 |
| ntional) |
| e (o |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:39 2008 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2 and 461 lb uplift at joint 5.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-9=-10, 7-9=-22(F=-12), 5-7=-10

Concentrated Loads (lb)

Vert: 9=-411(F) 7=-411(F)

Julius Lee Truss Design Engineer Flonds FE No. 24899 1100 Crestal Bay Blvd Dovnton Beson, FL 23425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000770 |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T24 | SPECIAL | 1 | 1 | | J1968776 |
| 22.0000 | 1 | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:39 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2, 505 lb uplift at joint 17, 238 lb uplift at joint 10 and 292 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Truss Cesian Engineer Florida FE No. 2-1898 1-169 Coestal Bay Blvd Bownton Besch El 20425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T23 | SPECIAL | 1 | 1 | | J1968775 |
| | 1 | | | | Job Reference (optional) | 11 |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:37 2008 Page 2

JOINT STRESS INDEX

2 = 0.59, 3 = 0.46, 4 = 0.33, 5 = 0.40, 6 = 0.25, 7 = 0.39, 8 = 0.39, 9 = 0.36, 10 = 0.53, 11 = 0.56, 13 = 0.39, 14 = 0.78, 15 = 0.76, 16 = 0.86, 17 = 0.79, 18 = 0.76 and 19 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x5 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 2, 473 lb uplift at joint 18, 201 lb uplift at joint 11 and 363 lb uplift at joint 13.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda ME No. 24888 1 109 Ceastal Bay filvri Bovnton Beach, FL 23425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|----------|------------|-----|-----|-----------------------------|----------|
| L276566 | T22 | SPECIAL | 1 | 1 | | J1968774 |
| | 55275556 | | 17 | 200 | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:36 2008 Page 2

JOINT STRESS INDEX

2 = 0.59, 3 = 0.46, 4 = 0.33, 5 = 0.50, 6 = 0.42, 7 = 0.39, 8 = 0.33, 9 = 0.39, 10 = 0.44, 11 = 0.33, 12 = 0.84, 14 = 0.47, 15 = 0.39, 16 = 0.24, 17 = 0.67, 18 = 0.83 and 19 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 419 lb uplift at joint 2, 491 lb uplift at joint 18, 165 lb uplift at joint 12 and 403 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee Truss Ossign Engineer Florida HE No. 24869 1109 Caestel Bay Blvd Goynton Beson, FL 93425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T21 | SPECIAL | 1 | 1 | | J1968773 |
| 22,000 | | 0. 20. 12 | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:35 2008 Page 2

JOINT STRESS INDEX

2 = 0.75, 3 = 0.46, 4 = 0.33, 5 = 0.48, 6 = 0.33, 7 = 0.29, 8 = 0.52, 9 = 0.33, 10 = 0.64, 11 = 0.76, 12 = 0.55, 14 = 0.72, 15 = 0.72, 16 = 0.33, 17 = 0.32, 18 = 0.44, 19 = 0.83, 20 = 0.80 and 21 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 683 lb uplift at joint 2, 587 lb uplift at joint 20 and 957 lb uplift at joint 14.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-54, 5-7=-54, 7-22=-54, 11-22=-118(F=-64), 11-13=-54, 2-20=-10, 19-20=-10, 17-19=-10, 17-23=-10, 15-23=-22(F=-12), 12-15=-10

Concentrated Loads (lb)

Vert: 15=-411(F)

Julius Lee Truss Design Engineer Flonda FE No. 24899 1409 Casstal Bay Blvd Boynton Beach, FL 23425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T20 | SPECIAL | 3 | 1 | | J1968772 |
| LETOGGG | 120 | OF LOWLE | J | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:33 2008 Page 2

NOTES

- 5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 1 and 133 lb uplift at joint 6.

LOAD CASE(S) Standard

May 29,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | DURATION / ST. ALBANS WOOD / ROOF |
|---------|-------|------------|-----|-----|--|
| | 207 | | | | J1968771 |
| L276566 | T19 | HIP | 1 | 1 | The second secon |
| | | | | | Job Reference (optional) |

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 15:14:40 2008 Page 2

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 1 and 128 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee Truse Cesian Endineer Flonda FE No. 24899 1 168 Chastal Bay Blyd Boynton Beach, FL 33435



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T18 | HIP | 1 | 1 | | J1968770 |
| L270300 | 110 | | | | Job Reference (optional) | |

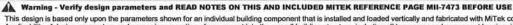
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:32 2008 Page 2

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2 and 103 lb uplift at joint 7.

LOAD CASE(S) Standard

May 29,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-------|-----|-----------------------------|----------|
| L276566 | T17 | MONO HIP | 1 | 1 | | J1968769 |
| | 1 | morro i m | 5 1.0 | | Job Reference (optional) | |

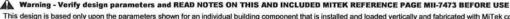
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:31 2008 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint 7 and 218 lb uplift at joint 2.

LOAD CASE(S) Standard

May 29,2008



Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|----------|-----|-----------------------------|----------|
| L276566 | T16 | MONO HIP | 1 | 1 | | J1968768 |
| | | | <u> </u> | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:30 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 485 lb uplift at joint 6 and 440 lb uplift at joint 2.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-118(F=-64), 2-9=-10, 6-9=-22(F=-12)

Concentrated Loads (lb) Vert: 9=-411(F)

> Julius Lee Truse Cesign Engineer Flonda Hs No. 24889 1189 Chestal Bay Blvd Covers Bases 1, 2014



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | Managara Carana (1900) |
|---------|-------|------------|-----|-----|-----------------------------|------------------------|
| L276566 | T14 | SPECIAL | 3 | 1 | | J1968767 |
| L270000 | 1.14 | OI LOI/L | J. | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:30 2008 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 2, 455 lb uplift at joint 12 and 99 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Flonds ME No. 24898 1169 Coestal Bay Blvd Boynton Beson, FL 93495



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T13 | SPECIAL | 2 | | | J1968766 |
| | 1 | 0. 202 | | 2 | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:29 2008 Page 2

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

5) Provide adequate drainage to prevent water ponding.

6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 2, 1035 lb uplift at joint 17, 2083 lb uplift at joint 13 and 140 lb uplift at joint 11.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-6=-54, 6-7=-54, 7-8=-54, 10-12=-54, 2-17=-10, 16-17=-10, 15-16=-10, 11-15=-10

Concentrated Loads (lb)

Vert: 10=-146(F) 13=-365(F) 19=-896(F)

Trapezoidal Loads (plf)

Vert: 8=-185(F=-131)-to-10=-258(F=-204)

Julius Lee Truss Design Engineer Florida Pis No. 24828 1400 Cossial Bay Blyd Boynton Beach, Ft. 93435



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T12 | SPECIAL | 2 | 1 | | J1968765 |
| 22,0000 | 112 | OF LOWE | _ | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:27 2008 Page 2

JOINT STRESS INDEX

2 = 0.55, 3 = 0.29, 4 = 0.33, 5 = 0.68, 6 = 0.45, 7 = 0.47, 8 = 0.46, 9 = 0.52, 10 = 0.52, 12 = 0.40, 13 = 0.54, 14 = 0.24, 15 = 0.72, 16 = 0.58 and 17 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 2, 441 lb uplift at joint 16, 243 lb uplift at joint 10 and 269 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida FE No. 34899 1109 Cassial Bay Blvd Boynton Besch, FL 25425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 9 |
|---------|--------|------------|-----|-----|-----------------------------|----------|
| L276566 | T11 | SPECIAL | 2 | 1 | | J1968764 |
| 1270000 | E-10-1 | OI LOIAL | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:26 2008 Page 2

JOINT STRESS INDEX

2 = 0.60, 3 = 0.42, 4 = 0.59, 5 = 0.90, 6 = 0.42, 7 = 0.92, 8 = 0.79, 9 = 0.75, 11 = 0.44, 12 = 0.39, 13 = 0.65, 14 = 0.89, 15 = 0.59, 16 = 0.65 and 17 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2, 443 lb uplift at joint 16, 241 lb uplift at joint 9 and 263 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Lee Truss Cesian Engineer Florida PE No. 34888 1409 Cesstal Bay Slyd Boynton Beach, Ft. 33438



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T10 | SPECIAL | 1 | 1 | | J1968763 |
| 2270000 | 110 | OI EON LE | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:25 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 197 lb uplift at joint 14, 239 lb uplift at joint 7 and 315 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truss Cesian Endineer Flonda FE No. 24899 1109 Cestal Bay Blvd Bovnton Beson, FL 33425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T09 | SPECIAL | 1 | 1 | | J1968762 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:24 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 14, 264 lb uplift at joint 7 and 397 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 24869 1109 Ceastal Bay Blvd. Boynton Beach, FL 33435



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | тов | SPECIAL | 1 | 1 | | J1968761 |
| L270300 | 100 | OI LOIAL | ' | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:22 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 15, 318 lb uplift at joint 8 and 369 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 24868 1109 Coastal Bay Blvd Boynton Besch, FL 33435



| Job | Truss | Truss Type | Qty | Ply | DURATION / ST. ALBANS WOOD / ROOF |
|---------|-------|------------|-----|-----|-----------------------------------|
| | | | | | J1968760 |
| L276566 | T07 | SPECIAL | 1 | 1 | South Cartes Sport Cartes |
| | | | | | Job Reference (optional) |

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 15:02:41 2008 Page 2

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 21 and 811 lb uplift at
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

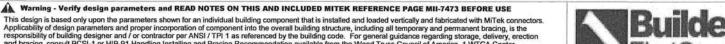
Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 7-22=-54, 9-22=-118(F=-64), 9-12=-54, 19-21=-10, 17-19=-10, 17-23=-10, 23-24=-22(F=-12), 11-24=-10

Concentrated Loads (lb)

Vert: 9=-265(F) 24=-52(F)

May 29,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | Т06 | SPECIAL | 4 | 1 | | J1968759 |
| | | Or Low L | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:19 2008 Page 2

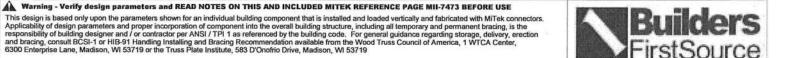
NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 1 and 166 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 24869 1100 Cassial Bay Blvd Boynton Beach, FL 20435



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 39 Y 00 1 10 10 10 10 10 10 10 10 10 10 10 1 |
|---------|-------|------------|-----|-----|-----------------------------|--|
| L276566 | T05 | COMMON | 4 | 1 | | J1968758 |
| | | Common | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:18 2008 Page 2

NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 343 lb uplift at joint 2 and 344 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 10-11=-70(F=-60), 6-11=-10

Julius Lee Truss Cesign Engineer Florida FE No. 24889 1109 Cessial Bay 61vd Boyono Basen E. 23427



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | ************************************** |
|---------|-------|------------|-----|-----|-----------------------------|--|
| L276566 | T04 | COMMON | 1 | 1 | | J1968757 |
| | | Common | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:18 2008 Page 2

NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 343 lb uplift at joint 2 and 344 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 10-11=-70(F=-60), 6-11=-10

Julius Les Truss Design Engineer Florida FE No. 24869 1100 Chastal Bay Blvd



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T03 | НІР | 1 | 1 | | J1968756 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:17 2008 Page 2

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 2 and 266 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda Ps No. 34898 1166 Chestel Bey Blvd



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | T02 | HIP | 1 | 1 | | J1968755 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:16 2008 Page 2

NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2 and 253 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda PE No. 34869 1100 Coastal Bay Sive



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | Automotive and the same |
|--|-------|------------|-----|-----|-----------------------------|-------------------------|
| L276566 | T01 | HIP | 1 | 1 | | J1968754 |
| The state of the s | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:15 2008 Page 2

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 591 lb uplift at joint 2 and 591 lb uplift at joint 7.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-11=-10, 9-11=-22(F=-12), 7-9=-10

Concentrated Loads (lb)

Vert: 11=-411(F) 9=-411(F)

Julius Lee Truse Cesian Engineer Flonds PE No. 24889 1109 Coastal Bay Blvd



| Job | Truss | Truss Type | Qty | Ply | DURATION / ST. ALBANS WOOD / ROOF |
|---------------|-------|------------|-----|-----|-----------------------------------|
| Market Market | | | | | J1968753 |
| L276566 | HJ9B | MONO TRUSS | 2 | 1 | |
| | | | | | Job Reference (optional) |

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 15:12:51 2008 Page 2

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 235 lb uplift at joint 5, 522 lb uplift at joint 2 and 113 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 7) 190# concentrated load from a structural sub-fascia beam. Sub-fascia beam and its connections to be designed and furnished by others.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Concentrated Loads (lb)

Vert: 1=-190(F)

Trapezoidal Loads (plf)

Vert: 2=-2(F=26, B=26)-to-5=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-6=-25(F=-7, B=-7)



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | НЈ9А | MONO TRUSS | 2 | 1 | | J1968752 |
| | | mene mee | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:13 2008 Page 2

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 357 lb uplift at joint 2, 40 lb uplift at joint 5 and 241 lb uplift at joint 6.

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee Truss Design Engineer Florida FE No. 24889 1189 Coastal Bay Blvd Bovoton Basco, El 23425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 14000754 |
|---------|--------|------------|-----|-----|-----------------------------|----------|
| L276566 | HJ9 | MONO TRUSS | 5 | 1 | | J1968751 |
| | (2000) | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:13 2008 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | FG1 | SPECIAL | 1 | 1 | , , | J1968750 |
| 2270000 | | OI LOWLE | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:12 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 6 and 248 lb uplift at joint 4.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
 Vert: 1-3=-54, 4-6=-255(F=-245)

> Julius Law Trium Comings Crystoner



| Job | Truss | Truss Type | Qty | Ply | DURATION / ST. ALBANS WOOD / ROOF |
|---------|------------------|--|-------|---------|-----------------------------------|
| 1000000 | N-0.000000 | | 1,000 | 0.00.00 | J1968749 |
| L276566 | EJ7A | MONO TRUSS | 4 | 1 | |
| | Distribution Co. | VVV = 25-200 (200) 20 | 5.75 | | Job Reference (optional) |

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 14:50:02 2008 Page 2

NOTES

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1 and 93 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Less Truss Cesign Engineer Florida PE No. 34888 1109 Gaestal Bay Blvd Bovnton Besch, FL 20425



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | Section and the second section and the sec |
|----------|-------|--------------|-----|-----|-----------------------------|--|
| L276566 | EJ7 | MONO TRUSS | 26 | 1 | | J1968748 |
| LL. 0000 | | ineria inces | | 1 | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:11 2008 Page 2

LOAD CASE(S) Standard

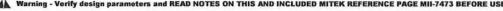
Julius Lee Truss Cesian Engineer Florida PE No. 24899 1100 Coestal Bay Blvd Boyoton Basen E. 20405



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | EJ6 | MONO TRUSS | 2 | 1 | | J1968747 |
| 1270300 | E30 | MONO TROSS | 3 | , ' | Job Reference (optional) | 3 |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:11 2008 Page 2

LOAD CASE(S) Standard







| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | 200000000000000000000000000000000000000 |
|---------|-------|------------|-----|-----|-----------------------------|---|
| L276566 | CJ5A | JACK | 4 | 1 | | J1968746 |
| 270000 | 000/1 | U/OIC | - | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:10 2008 Page 2

LOAD CASE(S) Standard



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | CJ5 | JACK | 14 | 1 | | J1968745 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:10 2008 Page 2

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Floride Ple No. 34868 1 198 Gessial Bay Blyd Boynton Besch. Ft. 92426



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | CJ3A | JACK | 4 | 1 | | J1968744 |
| | | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:09 2008 Page 2

LOAD CASE(S) Standard



| Job | Truss | Truss Type | Qty | Ply | DURATION / ST. ALBANS WOOD / ROOF |
|---------|-------|------------|-----|-----|-----------------------------------|
| | | | 222 | | J1968743 |
| L276566 | CJ3 | JACK | 14 | 1 | |
| | | | | | Job Reference (optional) |

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu May 29 15:11:06 2008 Page 2

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 2-4=-10 Concentrated Loads (lb)

Vert: 1=-95(F)

Julius Les Truss Cesian Engineer Flonda Fili No. 34869 1 100 Crestal Ray Blvd Bovnton Besch, Fl. 23435

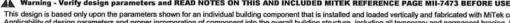


| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|--------------|------------|-----|-----|-----------------------------|----------|
| L276566 | CJ1B | JACK | 4 | 1 | | J1968742 |
| | 100000000000 | | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:08 2008 Page 2

LOAD CASE(S) Standard

May 29,2008



Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | CJ1A | JACK | 4 | 1 | | J1968741 |
| 1270000 | | U) (U) (U) | | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:07 2008 Page 2

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34888 1100 Coastal Bay Blvd Boynton Beach, FL 33435



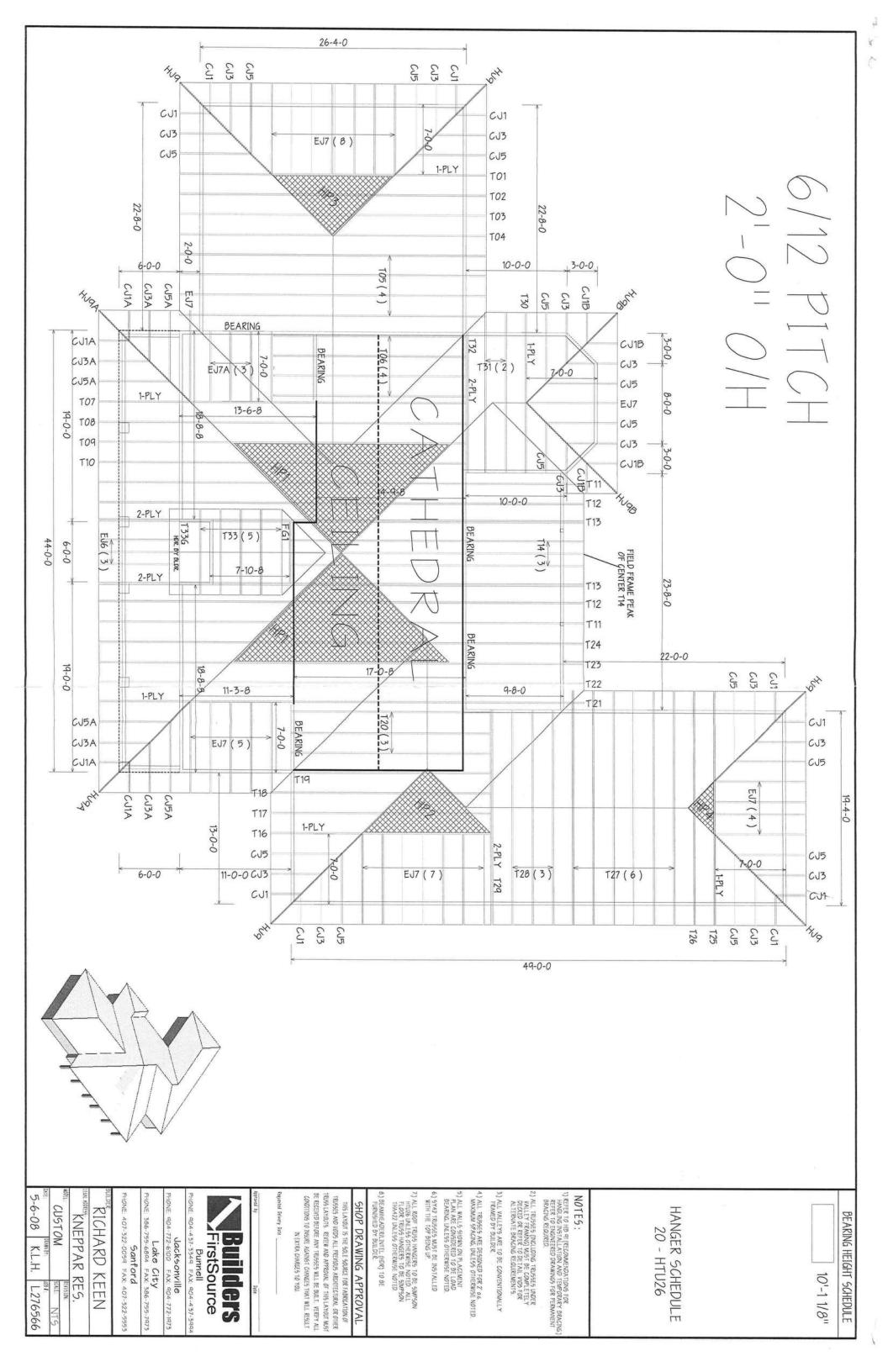
| Job | Truss | Truss Type | Qty | Ply | RICHARD KEEN - KNEPPAR RES. | |
|---------|-------|------------|-----|-----|-----------------------------|----------|
| L276566 | CJ1 | JACK | 10 | 1 | | J1968740 |
| 1270300 | 001 | SAOK . | 10 | | Job Reference (optional) | |

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu May 29 12:31:07 2008 Page 2

LOAD CASE(S) Standard

Julius Lee Truss Cesign Engineer Flonds PE No. 24868 1100 Coestal Bay Blvd





| Notice of Treatment | | | | | | |
|--|--------------------------|----------------------|--|--|--|--|
| Applicator: Florida Pest Control & Chemical Co. (www.flapest.com) Address: 536 56 Baga De City Lake City Phone 757,1903 | | | | | | |
| Site Location: Subdivision Charles Arrange Lot # Block# Permit # 27/42 Address | | | | | | |
| Product used | Active Ingredient | % Concentration | | | | |
| Premise | Imidacloprid | 0.1% | | | | |
| ☐ <u>Termidor</u> | Fipronil | 0.12% | | | | |
| Bora-Care D | isodium Octaborate Tetra | hydrate 23.0% | | | | |
| T | D. 8. 11 D. 11 | | | | | |
| Type treatment: | □ Soil □ Woo | bd | | | | |
| Area Treated | Square feet Linear f | | | | | |
| Main Book | 3327 29 | 4 240 | | | | |
| LINKING / POPCh 63 | | | | | | |
| | | | | | | |
| As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval. | | | | | | |
| If this notice is for the final exterior treatment, initial this line | | | | | | |
| 8/4/08 | # 7:52 | Not | | | | |
| Date | Time Prin | t Technician's Name | | | | |
| Remarks: | | | | | | |
| Applicator - White | Permit File - Canary | Permit Holder - Pink | | | | |

TOTAL FEES CHARGED 13,063,67 SCHOOL IMPACT FEE 00100003632900 CORRECTIONS IMPACT FEE 00100003632200 FIRE PROTECTION IMPACT FEE 10200003632220 ROAD IMPACT FEE 10100003632400 EMS IMPACT FEE FEES: 10300003632210 P27. 88 P1. 500-00 CODE CHECK NUMBER