

ELECTRICAL PLAN NOTES

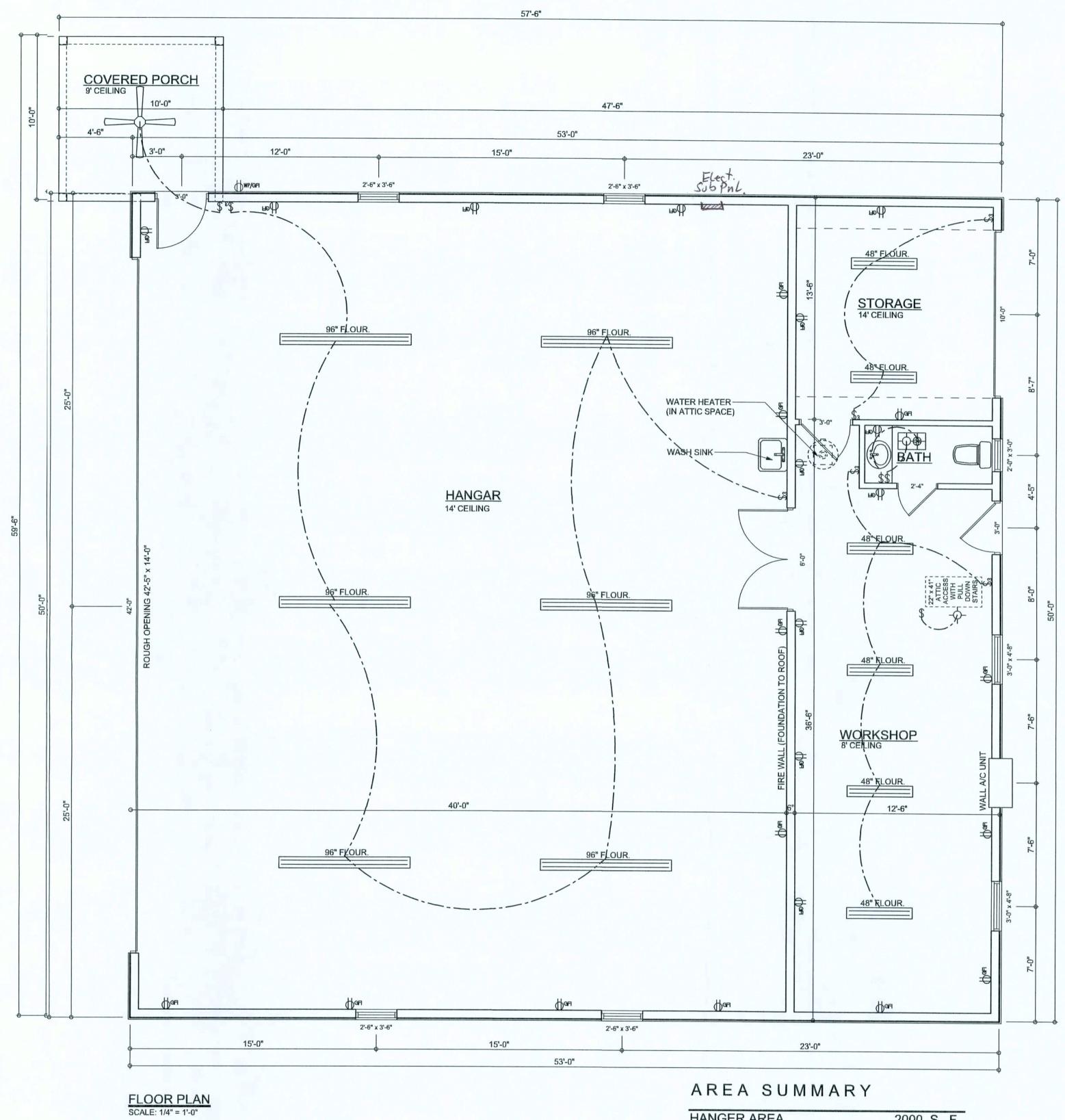
- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4

 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
 BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
 NEAR ALL BEDROOMS.
- E -5

 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
 DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
 SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING.
- E -10

 CONDUCTORS ENTER THE BUILDING.

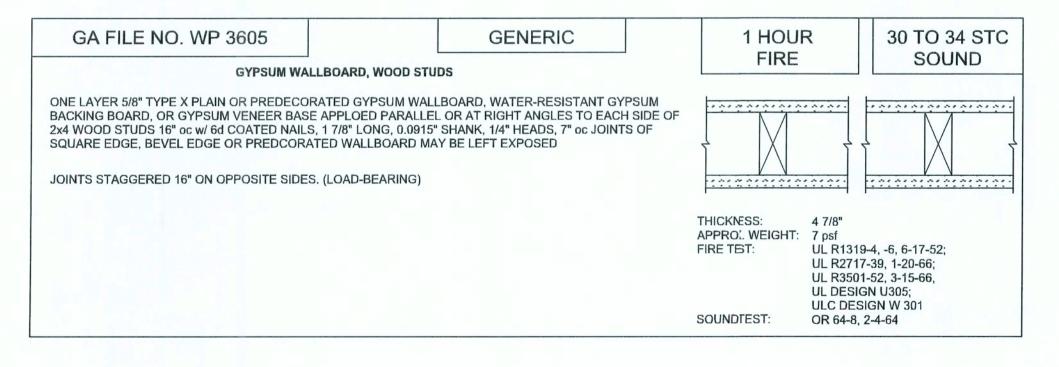
 SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL



TYPICAL DESIGN WALL SECTION

NON - STRUCTURAL DATA

SCALE: 1" = 1'- 0"



HANGER DOOR, GABLE TRUSS, AND BUILDING LATERAL BRACING NOTES:

1. GABLE END GIRDER TRUSS IS TO BE DESIGNED) AS A DRAG TRUSS TO TRANSFER 5000 Ib IN PLANE LATERAL LOAD FROM THE ROOF DIAPHRAGM TO THE WALL AT COORNERS (2500 Ib EACH END).

2. GABLE END GIRDER TRUSS IS ALSO TO BE DESINGNED TO SUPPORT REACTIONS FROM WEIGHT OF HANGAR DOOR IN OPENED AND CLOSED POSITIONS, WIND LOADS, AND ROOFF LOADS. BUILDER IS TO FURNISH DOOR ENGINEERING TO TRUSS ENGINEER. CONTRACTOR TO VERIFY ALLOWABLE TRUSS DEF=LECTION W/ DOOR MFG.

3. GABLE END TRUSS MUST BE BRACED TO RESIS¹T LATERAL LOADS FROM DOOR (SEE MFG DETAILS)

4. BUILDER MUST FURNISH SHOP DRAWINGS SPECIFYING THE DOOR, DRAG TRUSS, AND ALL CONNECTIONS FOR APPROVAL BY WINDLOAD ENGINEER PRIOR TO PERMITTING. TRUJSS ENGINEERING SPECS WERE NOT AVAILABLE FOR REVIEW BY WINDLOAD ENGINEER AT TIME THIS DOCUMENT WAS SEALED). BUILDING OFFICIAL MUST MAKE SURE DOCUMENTATION AGREES WITH 1 - 3 ABOVE.

5. BUILDER TO ATTACH HYDROSWING DOOR JAMB3 TO WALL USING ANCHORS SUPPLIED BY HYDROSWING. (VERIFY MINIMUM EDGE SPACING OF ANCHORS) (CONTRACTOR TO VERIFY OPENING SIZE WITH HYDROSWING)

HANGER AREA 2000 S.F.
WORKSHOP/STORAGE AREA 650 S.F.
PORCH AREA 98 S.F.

TOTAL AREA 2748 S.F.

Mark & Sue Wiencek Hangar

PE No.53915, POE 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

Do not proceed without clarification.

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CERTIFICATION: Ihereby certify that I have

examined this plan and that the applicable

LIMITATION: This lesign is valid for one

MARK DISOSWAY F.E. 53915

ouilding, at specified location.

portions of the plan relating to wind engineering comply with section R301.2.1, florida building code residentall 2014, to the best of my

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTMARE

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PRINTED DATE:
August 08, 2006

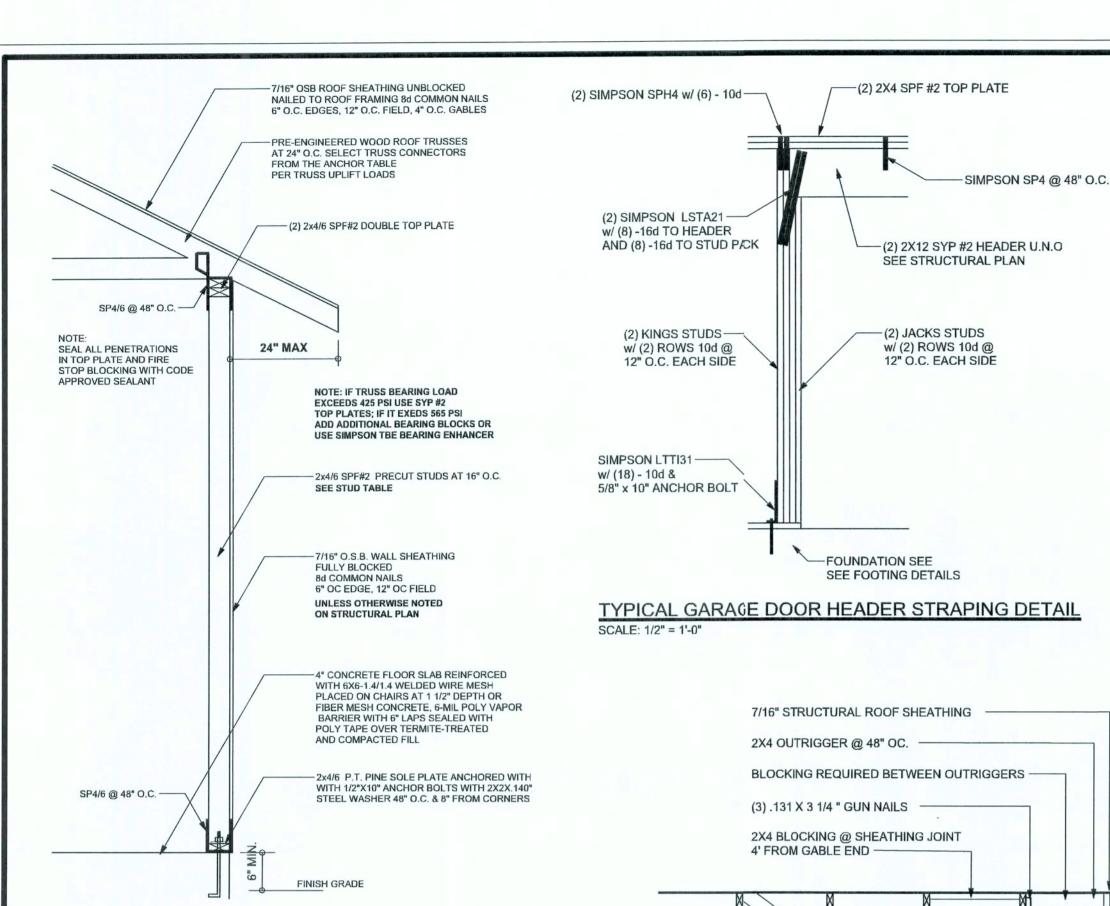
DRAWN BY: CHECKED BY: Evan Beamsley

Aug. 8, 2006

JOB NUMBER:

607209
DRAWING NUMBER **A-2**

OF 5 SHEETS



ONE STORY WALL SECTION

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTH: RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING

(1) 2x4 @ 12" OC

(1) 2x6 @ 16" OC

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

TO 13'-0" STUD HEIGHT

TO 18'-10' STUD HEIGHT

SCALE: 3/4" = 1'-0"

2X4 SCAB CONT. TOP TO

CHORD@ 8' FROM GABLE

2X4 SCAB IF VERT. WEB IS

CONT. 2X4X8' #2 SYP LATERAL

2X4 BLOCKING @ 48" OC.

BETWEEN GABLE AND FIRST

NOT PRESENT -

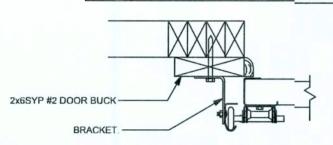
BRACE @ 48" OC.

4 - 10d NAILS OR 4 - .131"x 3.25"

TYPICAL AT ALL CONNECTIONS

2x6 SYP #2 GARAGE DOOR BUCK AT TACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LI A COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 33 1/4"

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALL ATION DETAIL

48" OC.

- FASCIA

- SHINGLE STRIP

DROP 3 1/2"

END TRUSS

- 2 - 2X4 TOP PLATE

2X4 STUDS @16" OC.

HURRICANE CLIP H-2.5 OR EQUAL

TOP CHORD OF GABLE END TRUSS

CONT. 2X4 SCAB FROM TOP TO

BOTTOM CHORD @ X-BRACING

(PROVIDE ADDITIONAL 2X4'S @

VERTICAL IF HIGHER THAN 48".

TOE NAIL TRUSS TO DOUBLE PLATE w/ 16d COM @8" OC.

BOTTOM CHORD OF GABLE

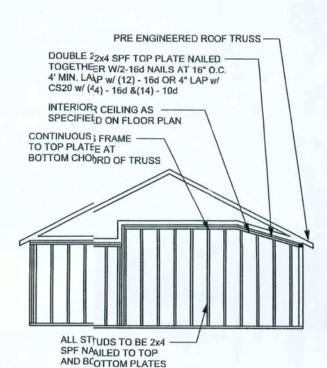
SIMPSON LSTA 24 @ 48" OC.

TO FORM AN "L" SHAPE.)

2X4 BARGE RAFTER CONT.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO

CEILIING DIAPHRAGM DETAIL

NAIL SHEATHING TO HEADER AND TOP

WITH 22-16d NAILS

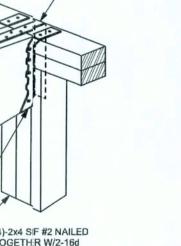
SCALE: N. T.S.

T'PICAL GABLE END (X-BRACING)

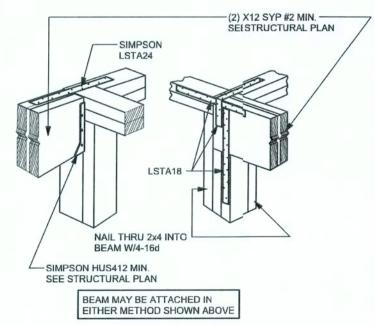
2X4 X-BRACE @ 6'-0" OC. -

ALL MEMBERS SHALL BE SYP

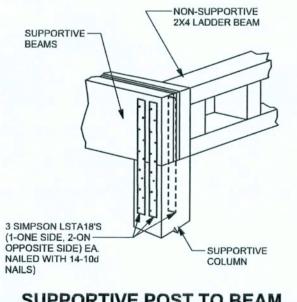
(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN SIMPSON HUS412 MIN. -SEE STRUCTURAL PLAN - (4)-2x4 SIF #2 NAILED TOGETH:R W/2-16d NAILS AT16" O.C. MIN. (SEISTRUCTURAL PLAN)

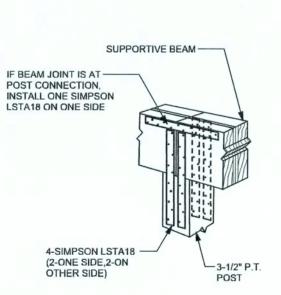


BEAM MID-WALL CONNECTION DETAIL SCALE: N.T.S.

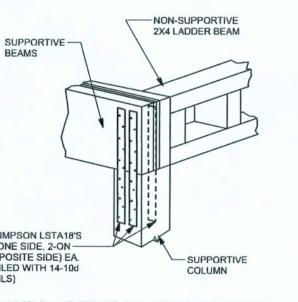


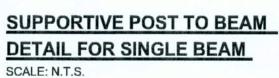
BEAM CORNER CONNECTON. DETAIL SCALE: N.T.S.

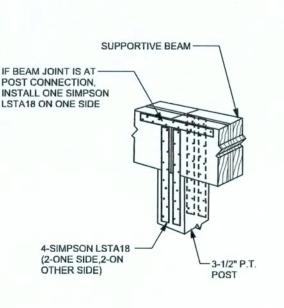


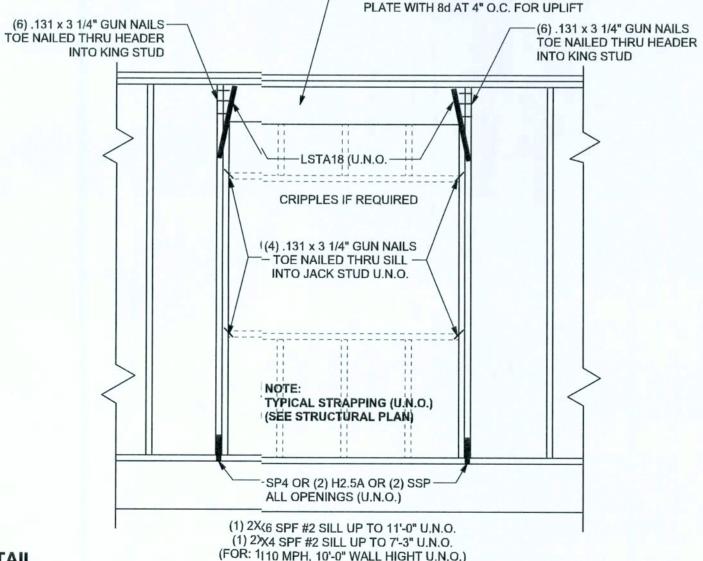


SUPPORTIVE CENTER POST TO BEAM DETAIL









TYPICAL IHEADER STRAPING DETAIL SCALE: 1/2" = 11'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET Y LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTMIC 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT, DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	R AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE LY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND SHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	ERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 IS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE	NTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL D ENGINEER IMMEDIATELY.
DESIGN, PLAC	RUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS EMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, JSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL ATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1606 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

Grout

3.3.E.7 Movement joints

CMU standard

ACI530.1-02 Section

Compressive strength

Clay brick standard

Reinforcing bars, #3 - #11

Coating for corrosion protection

IN WRITING.

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

Specific Requirements

5.5"x2.75"x11.5"

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

ASTM C 476, admixtures require approval

medium surface finish, 8"x8"x16" running

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

ANCHOR TABLE

< 455

< 360

< 455

< 415

< 600

< 950

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

< 2900

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TRUSS CONNECTOR*

H2.5

H2.5A

H14-1

H14-2

H10-1

H10-2

H16-1

H16-2

MTS24C

HTS24

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWNS*

MGT

HGT-2

HGT-3

HGT-4

STUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SP4

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS*

LTT19

LTTI31

HD2A

HTT16

PAHD42

HPAHD22

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2'

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

16 -10d

4-8d

4-8d

4-8d

5-8d

5-8d

5-10d, 1 1/2'

13-8d

8-8d, 1 1/2'

6-10d

10-10d, 1 1/2"

10-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

1 -10d

6-10d

14-10d

16-10d

18-8d

TO STUDS

18-10d, 1 1/

2-5/8" BOLTS

16-16d

16-16d

12-16d

12-16d

18 - 16d

15-8d

TO STUDS

TO FOUNDATION

1-5/8" THREADED ROD

2-5/8" THREADED ROL

12" EMBEDMENT

12" EMBEDMENT

2-5/8" THREADED ROL

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

TO STUDS

4 -10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

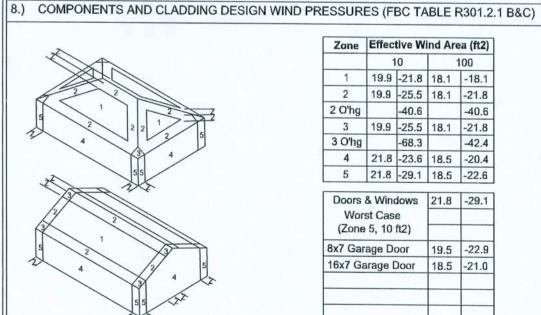
1/2" AB

1/2" AB

2-5/8" AB

UPLIFT LBS. SYP UPLIFT LBS. SPF

WIN	D LOADS PER FLORIDA BUILDING CODE 2004, SECTION R301.2.1
ME/ ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH
2.)	WIND EXPOSURE = B
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING, R301.2.1)
0.1	COMPONENTE AND



1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
	st Cas 5, 10	7000		
(Zone	5, 10	ft2)		
8x7 Gar	age D	oor	19.5	-22.9
16x7 Ga	arage [Door	18.5	-21.0

Zone Effective Wind Area (ft2)

10 100

DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)		-	
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			

REVISIONS

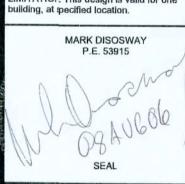
SOFTPIAN

PE No.53915, POB 868, Lake City, FL 32056, 386-'54-5419 Stated dimensions supercede scaled limensions.Refer all questions to

Mark Disosvay, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosvay, P.E. hereby expressly reserv its common aw copyrights and property right in these instrunents of service. This document is not to be reproduced, altered or copied in any form or mamer without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined the plan, and that the applicable portions of tie plan, relating to wind engineer comply withsection R301.2.1, florida building code resideitail 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.



Mart & Sue Wiencek Hangar

> ADDRESS: 443 SW Airpark Glen Lace City, Florida 32025

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FINALS DATE: Aug. 8,2006 JOB NUMBER: 607209

> **S-1** OF 5 SHEETS

DFAWING NUMBER

