DATE 07/25/2005

# Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

**PERMIT** 000023411

APPLICANT LINDA RODER PHONE 752-2281

ADDRESS 387 SW KEMP COURT LAKE CITY FL 32024

OWNER ED & DIANE WHITE PHONE

ADDRESS 7018 SW STATE ROAD 47 LAKE CITY FL 32024

CONTRACTOR ISAAC CONSTRUCTION PHONE 719-7143

LOCATION OF PROPERTY 47S, 1 MILE PAST WALTER AVE ON RIGHT

TYPE DEVELOPMENT ADDITION TO SFD ESTIMATED COST OF CONSTRUCTION .00

HEATED FLOOR AREA 2440.00 TOTAL AREA 2440.00 HEIGHT .00 STORIES 2

FOUNDATION CONC WALLS FRAMED ROOF PITCH 3/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 32

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 02-5S-16-03443-002 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES

| | | | | |
|---|---|---|---|---|
| | | CBC059323 | | |
| Culvert Permit No. | Culvert Waiver | Contractor's License Number | | Applicant/Owner/Contractor |
| EXISTING | 05-0461-N | BK | | N |
| Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident |

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 807

## FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 950.00 CERTIFICATION FEE $ 12.20 SURCHARGE FEE $ 12.20

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ _____ WASTE FEE $ _____

FLOOD ZONE DEVELOPMENT FEE $ _____ CULVERT FEE $ _____ **TOTAL FEE** 1024.40

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

**This Permit Must Be Prominently Posted on Premises During Construction**

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Edward C White
Diane Bishop White
P O Box 2111
Lake City, FL 32056
386-758-9486

July 28, 2006

John Kerce, Building Official
Columbia County Building and Zoning Department
135 NE Hernando Avenue
Suite B21
Lake City, Florida 32055

Dear Mr. Kerce;

In compliance with the directive that I received from Mr. Joe Haltiwanger issued with regard to my property at 7018 SW SR 47 on December 13, 2005, I am reporting to you on results of structural evaluations on a residential home at the above listed address. According to the professional engineers that examined this structure, your inspectors were indeed correct with regard to structural deficiencies in this project. Additionally, during an inspection yesterday, a structural engineer pointed out that completed work seriously deviated from permitted plans. To him, this constitutes a serious violation of the permit obtained for this project.

Based on this latest information, I feel that your department should conduct an additional inspection of this job site. If it is found that the permit was, in fact, violated, I would expect appropriate actions to be taken to include written documentation of the permit violations.

This has been a long and painful process for my wife and I at a time when we lost our parents to painful illness and tried to transition to our family home here in Lake City. Your immediate attention to this prolonged process would be most appreciated.

Respectfully,

Ed White

FEAGLE & FEAGLE, ATTORNEYS, P.A.
ATTORNEYS AT LAW
153 NE MADISON STREET
POST OFFICE BOX 1653
LAKE CITY, FLORIDA 32056-1653
(386) 752-7191
Fax: (386) 758-0950

Marlin M. Feagle
e-mail: leagle@bellsouth.net

Mark E. Feagle
e-mail: mefeagle@bellsouth.net

August 24, 2006

Mr. Joe Haltiwanger
Plan Examiner
Columbia County Building Department
135 NE Hernando Avenue, Suite B21
Lake City, Florida 32055

Re: Edward C. White and his wife, Diane Bishop White

Dear Joe:

This will confirm our recent office conference where we discussed the letter from Edward White dated July 28, 2006 addressed to John Kerce, a copy of which is enclosed for your reference. In his letter Mr. White describes that according to the professional engineers that examined the structure, his home, structural deficiencies were noted. Mr. White also points out that during an inspection the structural engineer pointed out that completed work deviated from permitted plans.

Mr. White will need to have his structural engineer and/or architect complete their inspection and investigation. Then, they will need to provide the County with a specific corrective action plan. Based upon this corrective action plan which may include modification of the original construction plans, the County should be in a position to review the same and issue a second permit to either the owner or a state certified licensed building contractor. The County cannot issue a second permit until this information is provided to the County.

If Mr. White believes his state licensed contractor failed to complete the work in a professional manner, his complaint and recourse should be directed to the State of Florida, Department of Business and Professional Regulations, which regulates licensed contractors.

If you need anything further in this regard, please do not hesitate to give me a call.

Very truly yours,

Marlin M. Feagle

MMF:dse

Freeman Design Group inc
Engineers • Planners

161 N.W. Madison St., Suite 102
Lake City, Florida 32055
Tel: 386-758-4209
Fax: 386-758-4290

7/28/2006

Ed White
P. O. Box 1376
Lake City, FL. 32056

Re: Structural Inspection

Dear Mr. White,

I have completed the structural inspection and have reviewed the original plans and the following evidence is evident:

The plans call for an exterior bearing wall height of 9'0". The exterior bearing walls were constructed at an 8'0" height. The ceiling height of the master bedroom is shown as a 9'0" ceiling height but was constructed as a 10'0" ceiling height. During construction the roof of the first floor interfered with the floor level of the second floor since the wall was constructed 1'0" too short and the floor level 1'0" too high. This caused a problem with floor joist layout as indicated on the plans. Since the floor joists interfered with the incorrectly placed roof system, the floor joists were altered so that they were no longer bearing on the walls as indicated on the plans. The plans did indicate structural load bearing walls to support the upper floor joist system, however, by shifting the bearing locations to new non-structural walls there was no provision for a structural foundation or structural headers. This could cause failure in the slab and wall components.

The two-story portion of the house was not constructed to the proper wall height as indicated on the plans. The original design called for a set of four (4) beams to run continuous from front to rear of the structure. The exterior walls were constructed in this manor to perform as a post and beam system. The beams are intended to span a certain distance and then bear on a post which has a structural footing directly underneath. The exterior walls on the two story portion were altered to bring the height up according to the original design. When this was done, the structural beam was removed in order to build a new knee wall above the original wall that was built too low.
This created two main problems. When the beam was removed, the post-beam structure became a continuous load-bearing wall. There is no interior structural footing to carry the load along the entire length of wall. When the knee wall was constructed to extend the

wall height per design, a hinge point was created in the wall. This could likely fail when wind pressure is applied to the wall.

A new wall was constructed on top of the original house. At this point, there is no evidence that the exterior wall is resting on anything structural. The weight of the wall and the roof trusses are only bearing on the existing roof decking and is showing signs of excessive deflection. The load-bearing wall needs to have a continuous load path from the foundation, up to the roof.

In order to solve the issues discussed above the structure would need to be dismantled. A redesign of the project should be accomplished taking into account the correct pitch of the existing roof. It should contain detail for the joining of the new and the old structure and foundation requirements to support changes in the correct plans. Because the structure could not be completed the trusses have now been exposed to the weather for nearly a year and have shown signs of twisting and buckling.

If you have any questions, you may contact me at (386) 758-4209.

Sincerely,

William Freeman, P.E.

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Dear Mr. Ed White

On December 13, 2005 per your request Mr. Randy Jones Assistant Building Official, Mr. Harry Dicks Building Inspector and I as the Plans Examiner accompanied you with a structural inspection of your resident at 7018 SW SR 47. The inspection pertains to Building permit number 23411 issued July 25, 2005 to ISAAC. This build permit was to construct an addition on to an existing single family dwelling.

You requested that we inspect the structural framing which has been completed thus far for building code compliance. The Columbia County Building Department has several concerns which are:

1. Mr. Robert Taylor Architect, of Associated Florida Architects, Inc. 802 NW 23rd Avenue Gainesville Florida, State of Florida Registration Number AR-0007526 filed a letter with the Columbia County Building Department on July 22, 2005 which stated that Associated Florida Architects, Inc. were the architects of record for the construction of a addition on to your single family dwelling. Mr. Robert Taylor in this letter also stated that certain structural issues which I as the plan examiner had would be addressed once the construction of this addition was started. Mr. Robert Taylor stated that within forty-five (45) days of commencing with construction a written structural report addressing the needed issues would be filed with the Columbia County Building Department. **No reports have been**

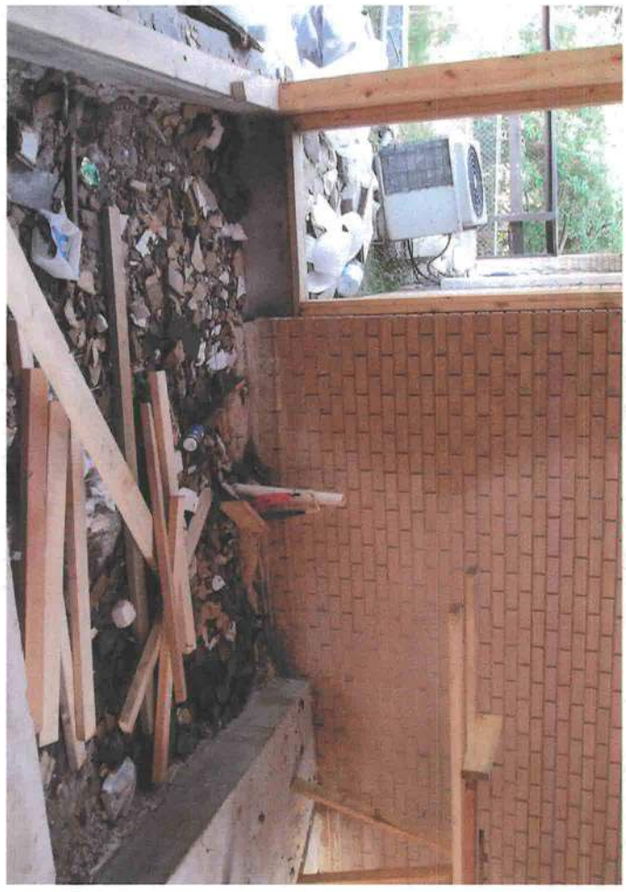
**filed with the Columbia County Building Department by Mr. Taylor or Associated Florida Architects.**

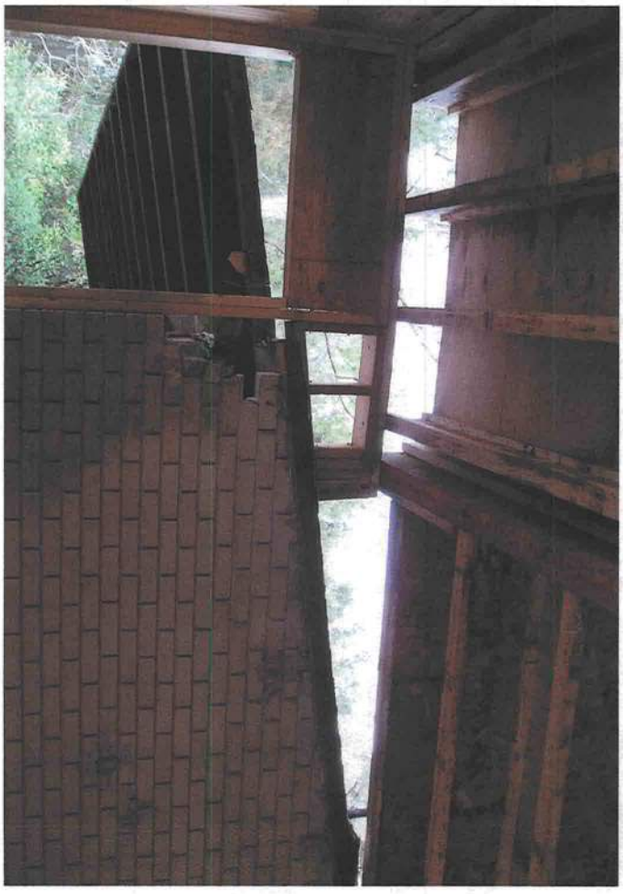
2. An example of one of the issues which was not addressed was the use of an exterior load bearing foundations and shear walls which was used to construct a second story load bearing wall without the contractor and the architect confirming that the foundation and the structural walls could safely support the additional structural load-bearing weight which has been applied to the foundation and walls for of a second story and a roof truss system.
3. The addition onto the structure of 3,044 square feet incorporated a first and second floor, using wood frame structural walls. Within the first floor several exterior and interior load bearing walls have been created to provide support for the second floor, flooring and roof truss system. Several first floor walls lack sufficient structural strength to properly support the second floor.
2. The roof trusses have been modified by the contractor, the truss designer will be required to assess the trusses system to certify that the trusses system meet the original truss system design.

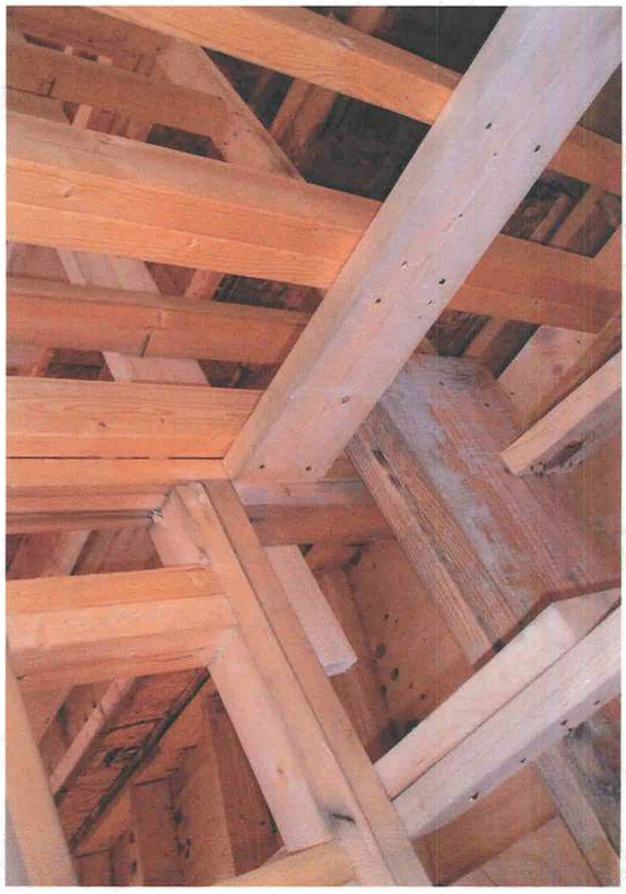
In order to continue work on this structure, a structural evaluation by a professional architect or engineer is needed to comply with the Florida Building Codes 2001. If you should have any additional question please contact me.

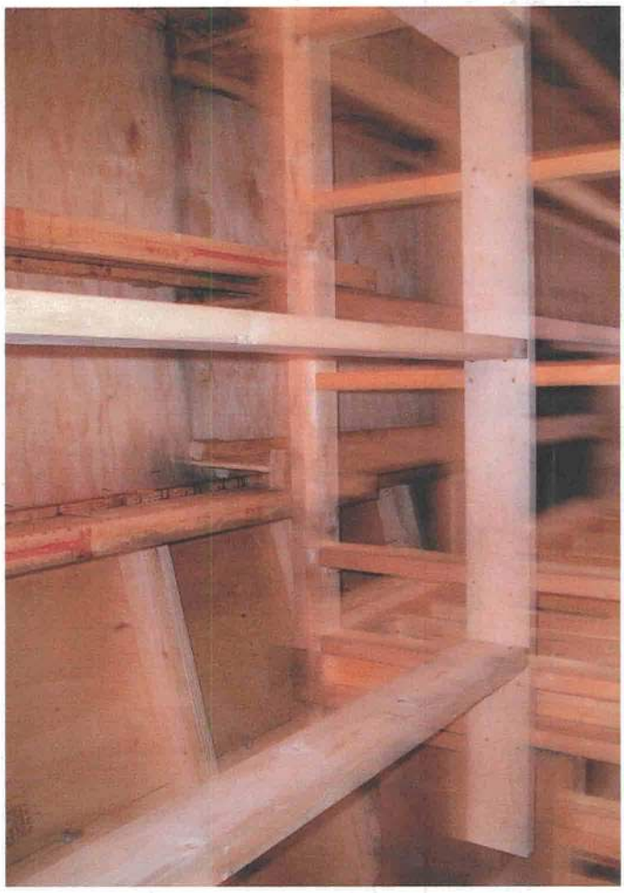
Thank you,

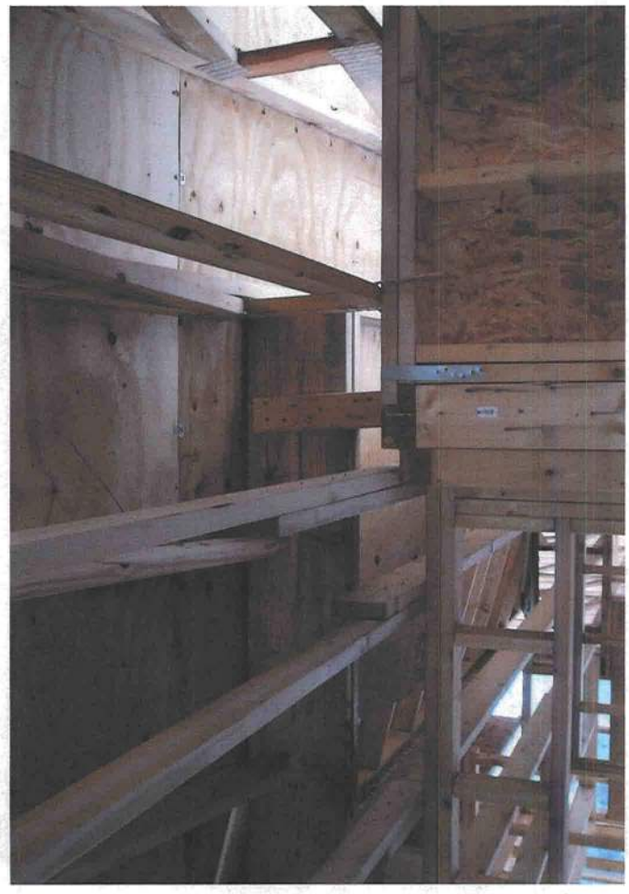
Joe Haltiwanger
Plan Examiner
Columbia County Building Department

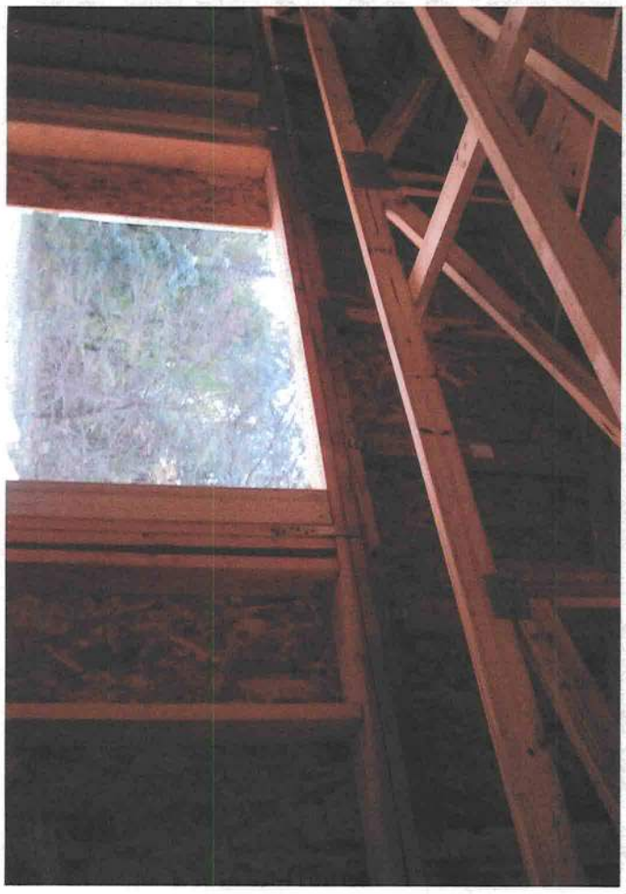


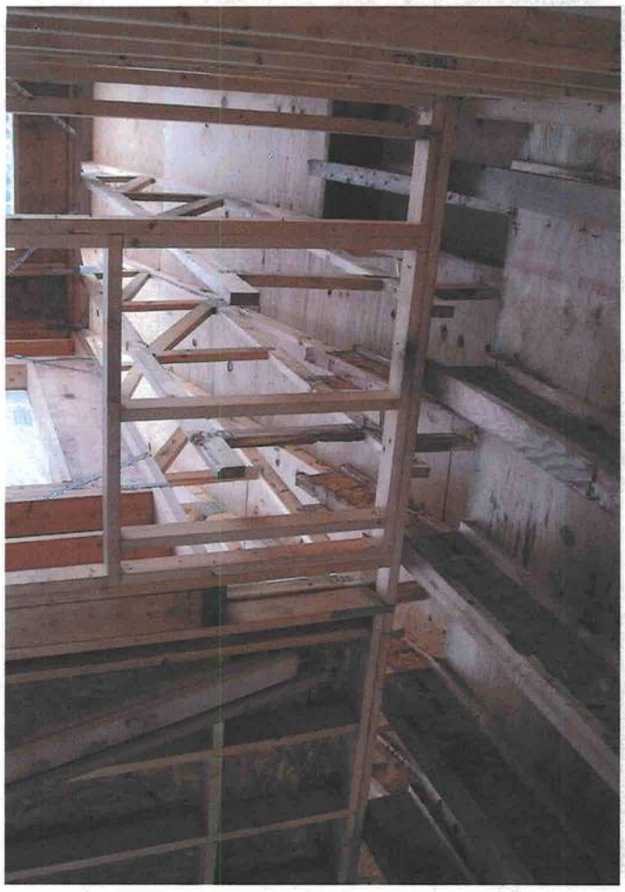


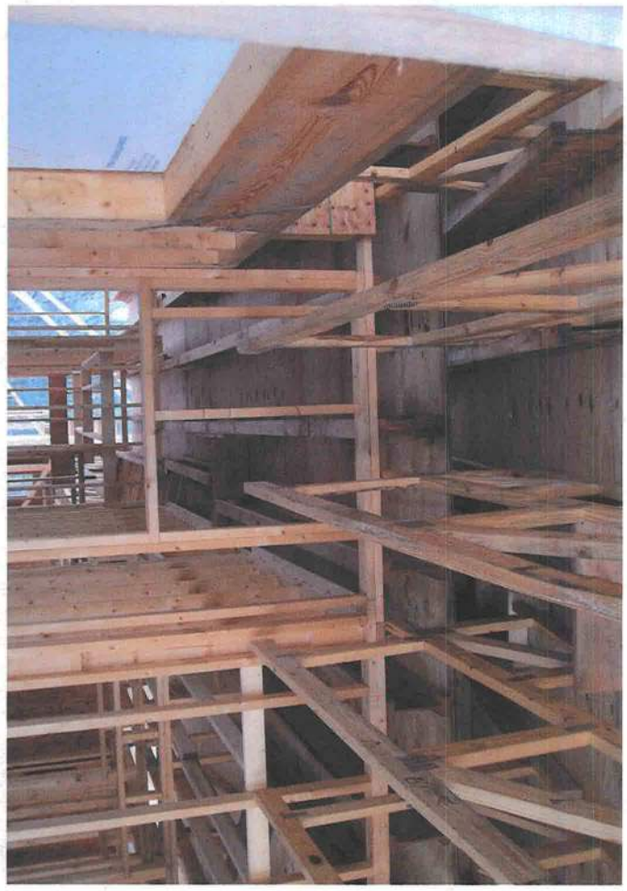


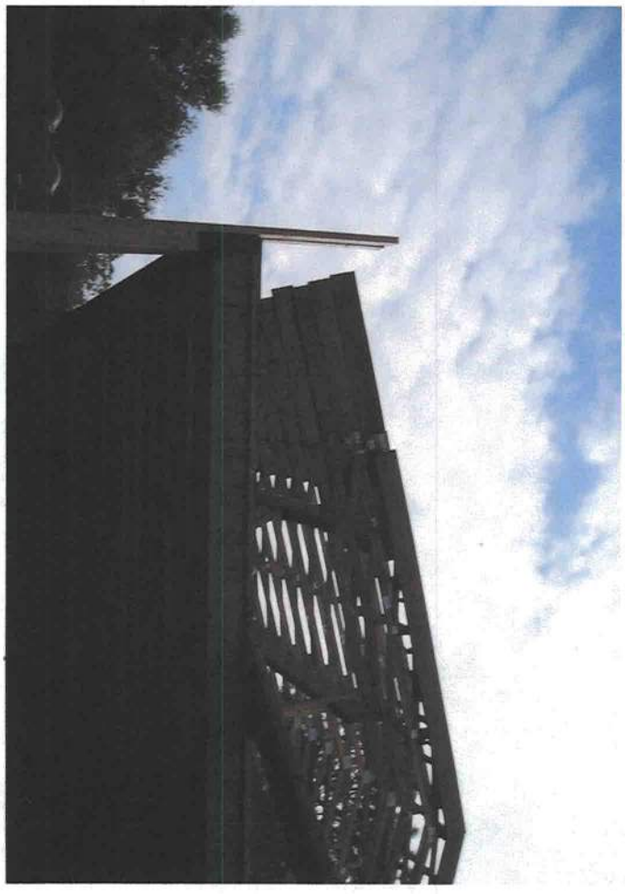


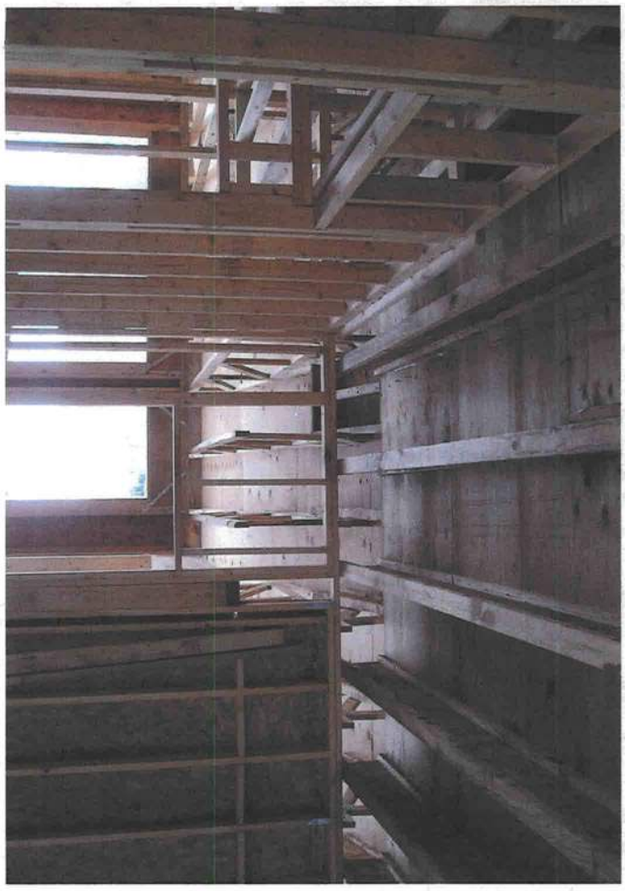


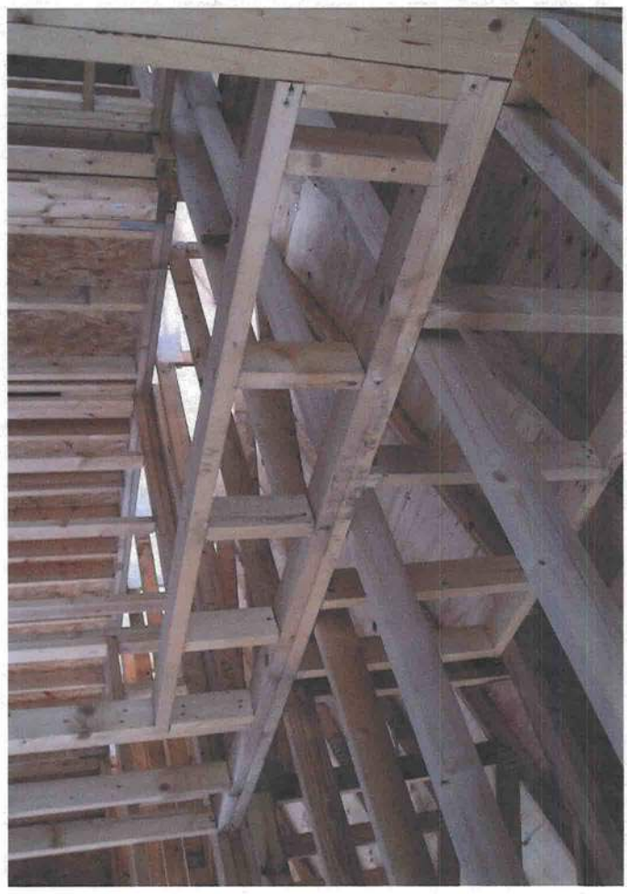


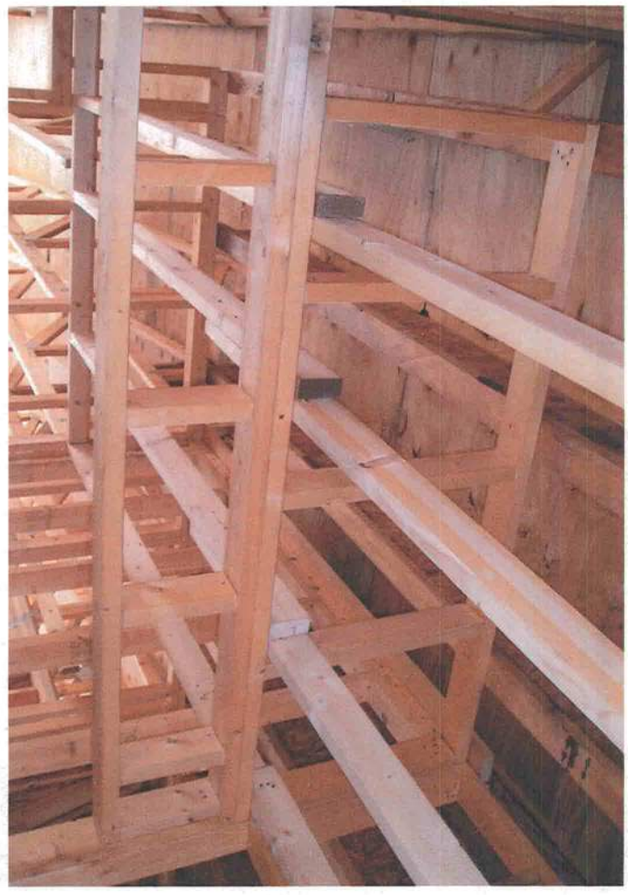


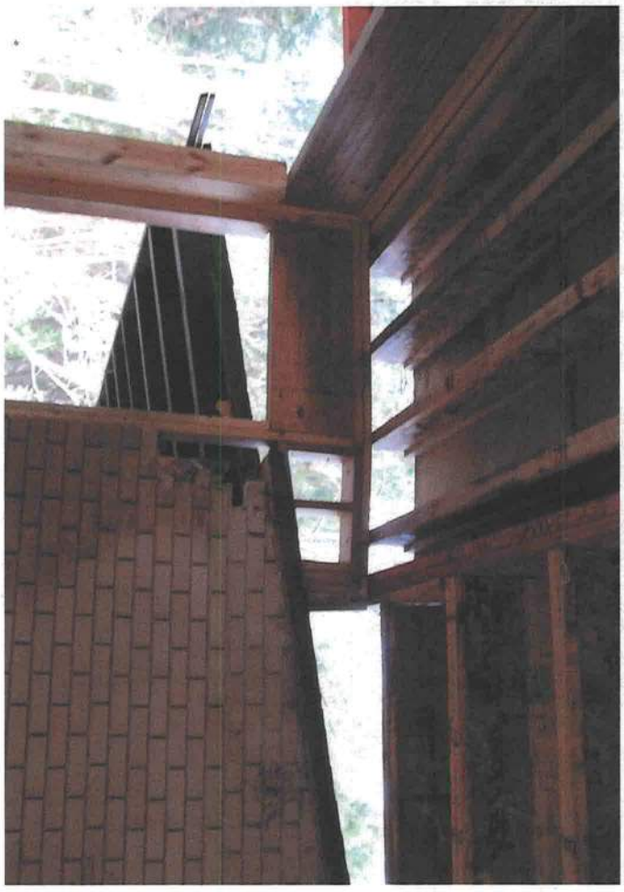


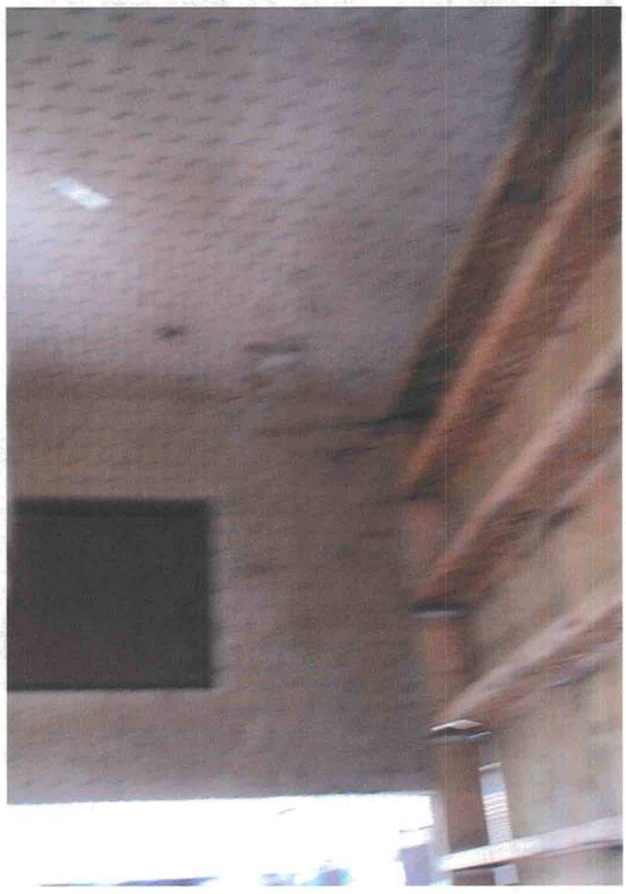


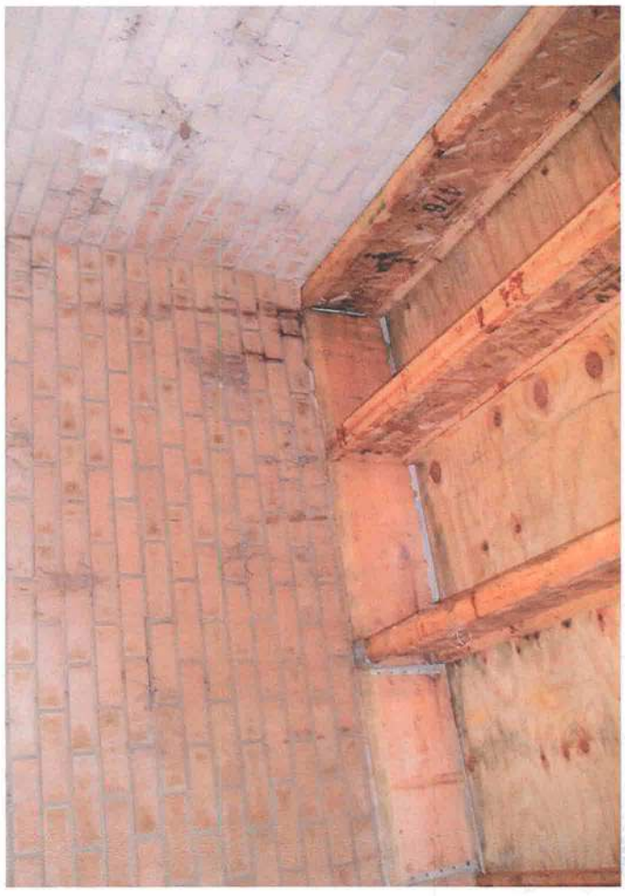


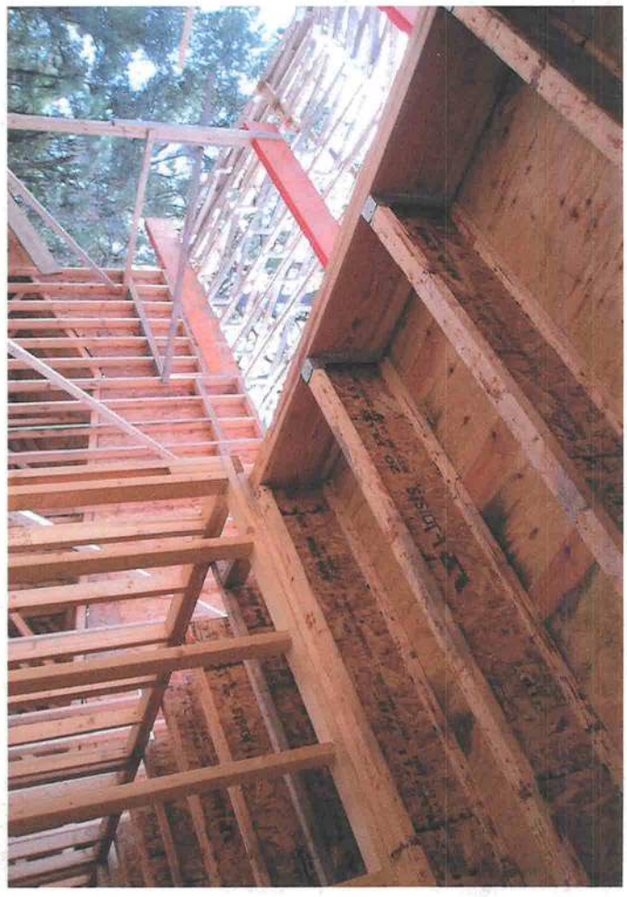












**associated florida architects, inc.**
florida registration AA-C001907
802 nw 23rd avenue • gainesville, florida 32609-3534
352 375-3005 • 352 378-2523 • 352 375-5397 fax

July 22, 2005

Columbia County Building Department
135 NE Hernando Ave.
Lake City, FL 32055
Re: Ed & Diane Bishop-white Residence

To Whom It May Concern:

We are the architects for the referenced project. We have a full service contract with the owners including contract administration.

Due to the fact that this is an addition we will be able to do the discovery work on the existing residence when construction commences.

We will write a report with findings and recommendations within forty-five (45) days of start of construction. As per 3401.8.2.3.1, 3401.8.2.3.2 and 3401.8.2.3.3 of 2001 Florida Building Code.

Robert S. Taylor, Sr. - Architect
Florida Registration AR-0007526

**associated florida architects, inc.**
florida registration AA-C001907
802 nw 23rd avenue • gainesville, florida 32609-3534
352 375-3005 • 352 378-2523 • 352 375-5397 fax

July 22, 2005

Isaac Construction
Lake City, Florida 34474

## ADDENDUM NUMBER ONE:

**Contract Drawings – Dated 09 May 05**
**Revision Date 21 July 2005**
**Architect's File Number 0317**

The following changes, revisions or deletions to the original Drawings, or previously issued Addenda shall be made by this Addendum.

This Addendum is issued to revise and clarify items addressed by a Letter from the Columbia County Building & Zoning Department dated 08 July 2005 dealing with Plan Review.

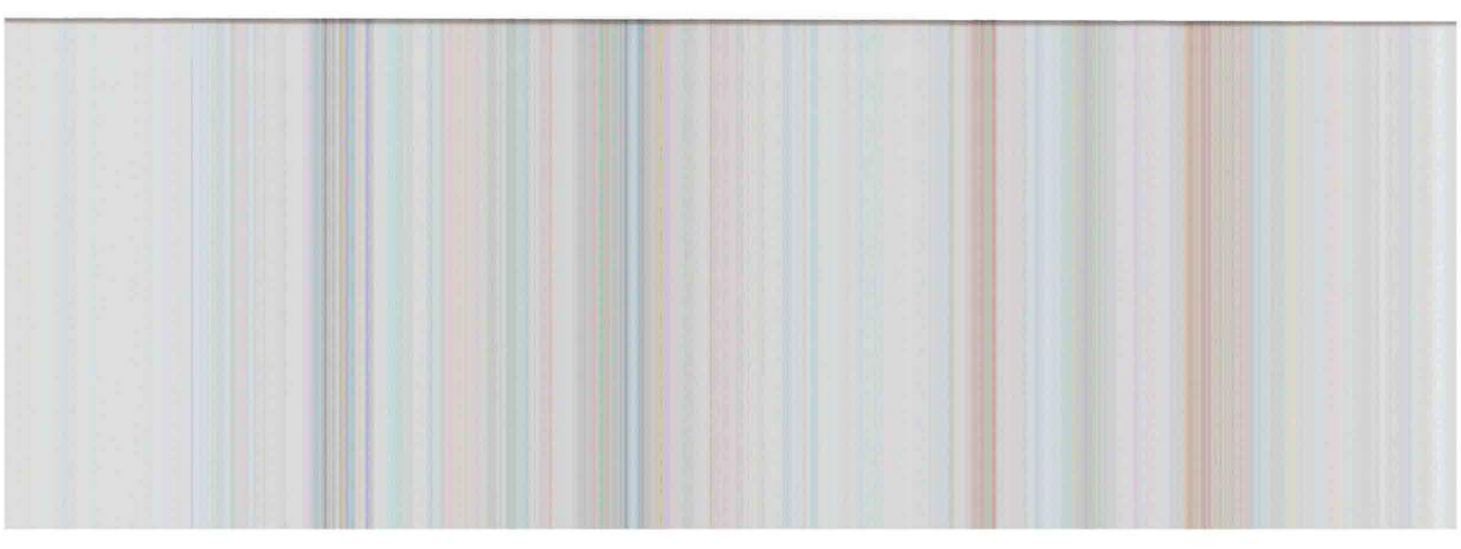
1.) REPLACE EXISTING FLOOR PLAN SHEET WITH NEW FLOOR PLAN (SHOWING DOOR SIZES)
2.) REPLACE EXISTING FRAMING PLAN SHEET WITH NEW FRAMING PLAN SHEET.
3.) REPLACE EXISTING ELEVATION SHEET WITH NEW ELEVATION SHEET.

4.) ADD SHEETS S-1 (STRUCTURAL NOTES) AND S-2 (STRUCTURAL DETAILS.

5.) PROVIDE G.F.C.I. OUTLETS IN ALL SLEEPING ROOMS AS PER 2001 FLORIDA BUILDING CODE.
6.) PROVIDE SMOKE DETECTORS AS PER 2001 FLORIDA BUILDING CODE
7.) ALL GLASS WITH 18 INCHES ABOVE THE FLOOR SHALL AND THE GARDEN TUB SHALL BE SAFETY GLAZING AS PER 2405.2.4. OF THE 2001 FLORIDA BUILDING CODE.

Robert S. Taylor, Sr. - Architect
Florida Registration AR-0007526

| Date | Inspection | Inspect. | Owner | Pass | Location | Permit |
|---|---|---|---|---|---|---|
| 08/09/05 | Temp Service | Randy | Issac Bratkovich - White | OK | 7018 SW SR 47 | 23411 |
| 08/15/05 | Rough Plumbing | Randy | Issac Bratkovich - White | OK | 7018 SW SR 47 | 23411 |
| 09/06/05 | Mono Slab | Randy | Issac Bratkovich - White | OK | 7018 SW SR 47 | 23411 |
| 09/06/05 | Set Backs | Randy | Issac Bratkovich - White | OK | 7018 SW SR 47 | 23411 |
| 12/13/05 | Complaint | HD-RJ-JH | Issac Bratkovich - White | Met With | 7018 SW SR 47 | 23411 |



# New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525
(exp. 10/31/2005)

This form is completed by the licensed Pest Control Company.

**Public reporting burden** for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#23411

**Section 1: General Information** (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109476 Company Phone No. 386-755-3611
FHA/VA Case No. (if any)

**Section 2: Builder Information**

Company Name: Isaac Co.,T Company Phone No.

**Section 3: Property Information**

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 7014 S.W State Rd 47 Fla. City, Fl

Type of Construction (More than one box may be checked) ☑ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 0 Inside 0 Type of Fill Dirt

**Section 4: Treatment Information**

Date(s) of Treatment(s) 9-6-05
Brand Name of Product(s) Used Surround
EPA Registration No. 70960-7-53883
Approximate Final Mix Solution % 0.5%
Approximate Size of Treatment Area: Sq. ft. 2440 Linear ft. 0 Linear ft. of Masonry Voids 0
Approximate Total Gallons of Solution Applied 245
Was treatment completed on exterior? ☐ Yes ☑ No
Service Agreement Available? ☐ Yes ☑ No
*Note: Some state laws require service agreements to be issued. This form does not preempt state law.*

Attachments (List)
Comments 2440 Sq Ft more Addition to South Side of home

Name of Applicator(s) [signature] Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature [signature] Date 9-6-05

**Warning:** HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Ed White

Addition

## Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0506-88 Date Received 6/8/05 By JW Permit # 23411

Application Approved by - Zoning Official BLK Date 13.07.05 Plans Examiner OKJH Date 7-22-05

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments

- NOC NEEDED -

Applicants Name Linda or Melanie Roder Phone 752-2281

Address 387 S.W. Kemp Ct. Lake City FL 32024

Owners Name Ed & Diane White Phone

911 Address 7018 S.W. State Rd 47 Lake City FL 32024

Contractors Name Isaac Construction. Inc. Phone 719-7143

Address 1005 S.W. Walter Ave.

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Associated Florida Architects / Mark Disosway

Mortgage Lenders Name & Address Columbia County Bank

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 02-5S-16-03443-002 Estimated Cost of Construction 190,000 Changed

Subdivision Name Lot Block Unit Phase

Driving Directions 47 S. 1 mile past Walter ave see Isaac Sign On R - that is the driveway. go R

Type of Construction SFD addition Number of Existing Dwellings on Property 1

Total Acreage 10 Lot Size Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 300' Side 400' Side 1000' Rear 500'

Total Building Height 32' ave 28' Revised Number of Stories 2 300 Heated Floor Area 3044 280 130 500 Roof Pitch 3-12

of addition

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor) — Contractor Signature

Contractors License Number CBC 059323

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL

Barbara C. Webster
Commission # DD329279
Expires July 2, 2008
Bonded Troy Fain - Insurance, Inc. 800-385-7019

Sworn to (or affirmed) and subscribed before me this 22nd day of April 2005.

Personally known X or Produced Identification

Notary Signature

THIS DOCUMENT WAS PREPARED WITHOUT BENEFIT OF TITLE SEARCH.

FOR RECORDER

Inst:2003008855 Date:04/29/2003 Time:09:29
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:981 P:2183

THIS INSTRUMENT WAS PREPARED BY:
Krista L. Waldron, Esquire (PES)
Fisher, Tousey, Leas & Ball, P.A.
818 North A1A, Suite 104
Ponte Vedra Beach, Florida 32082

# SPECIAL WARRANTY DEED

THIS INDENTURE, made this 28 day of APRIL, 2003, between VIRGINIA H. BISHOP, as Trustee of the Virginia H. Bishop Living Trust, whose address is P. O. Box 1298, Lake City, Florida 32056, party of the first part, and EDWARD C. WHITE, JR. and DIANE B. WHITE, husband and wife, whose address is 80 Cliffcreek Trace, Atlanta, Georgia 30350, parties of the second part.

WITNESSETH:

That the said party of the first part, in consideration of love and affection, has granted, bargained, and conveyed to the said parties of the second part, their heirs, successors and assigns forever, her remaining one-half interest in the following described lands, situate, lying and being in Columbia County, Florida, to wit:

Commence at the Southwest Corner of the Southwest ¼ of Northwest ¼ of Section 2, Township 5 South, Range 16 East and run thence North 0 degrees 49 minutes West along the West line of said Section 2, 658.91 feet for Point of Beginning; thence continue North 0 degrees 49 minutes West along the West line of said Section 2, 500 feet; thence run South 65 degrees 59 minutes 30 seconds East, 1000.05 feet to the Northerly line of State Road No. 47; thence run South 40 degrees 53 minutes West along the West line of State Road No. 47 a distance of 474.23 feet; thence run North 65 degrees 59 minutes 30 seconds West, a distance of 652.5 feet more or less to the Point of Beginning, said land lying in the Southwest ¼ of the Northwest ¼ of Section 2, Township 5 South, Range 16 East, Columbia County, Florida, containing 10 acres, more or less.

Real Estate Assessment No: 02-5S-16-03443-002

Subject to covenants, restrictions, easements, mortgages and all other encumbrances of record and taxes assessed subsequent to December 31, 2002; provided, however, this reference will not serve to reimpose any such covenants, restrictions or easements.

Inst:2003008855 Date:04/29/2003 Time:09:29
Doc Stamp-Deed : 0.70
MK DC,P.DeWitt Cason,Columbia County B:981 P:2184

And the said party of the first part does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever claiming by, through or under the said party of the first part, but not otherwise.

IN WITNESS WHEREOF, the undersigned has hereunto set her hand and seal as of the day and year first above written.

Signed and Sealed in Our Presence:

Sign: Doris A. Johnson
Print Name: Doris A. Johnson

Sign: Sheryl Litteral
Print Name: Sheryl Litteral

Virginia H. Bishop
VIRGINIA H. BISHOP, as Trustee of the Virginia H. Bishop Living Trust

STATE OF FLORIDA
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 28 day of April, 2003, by VIRGINIA H. BISHOP, as Trustee of the Virginia H. Bishop Living Trust, (notary must check one box): (X) who is personally known to me or ( ) who has produced _____ (State) driver's license No. _____ as identification.

Elaine K. Tolar
Name: ELAINE K. TOLAR
NOTARY PUBLIC, State of FLORIDA
Commission Number: _____

ELAINE K. TOLAR
MY COMMISSION # DD 08[illegible]
EXPIRES: October 2, 2005
Bonded Thru Notary Public Underwriters

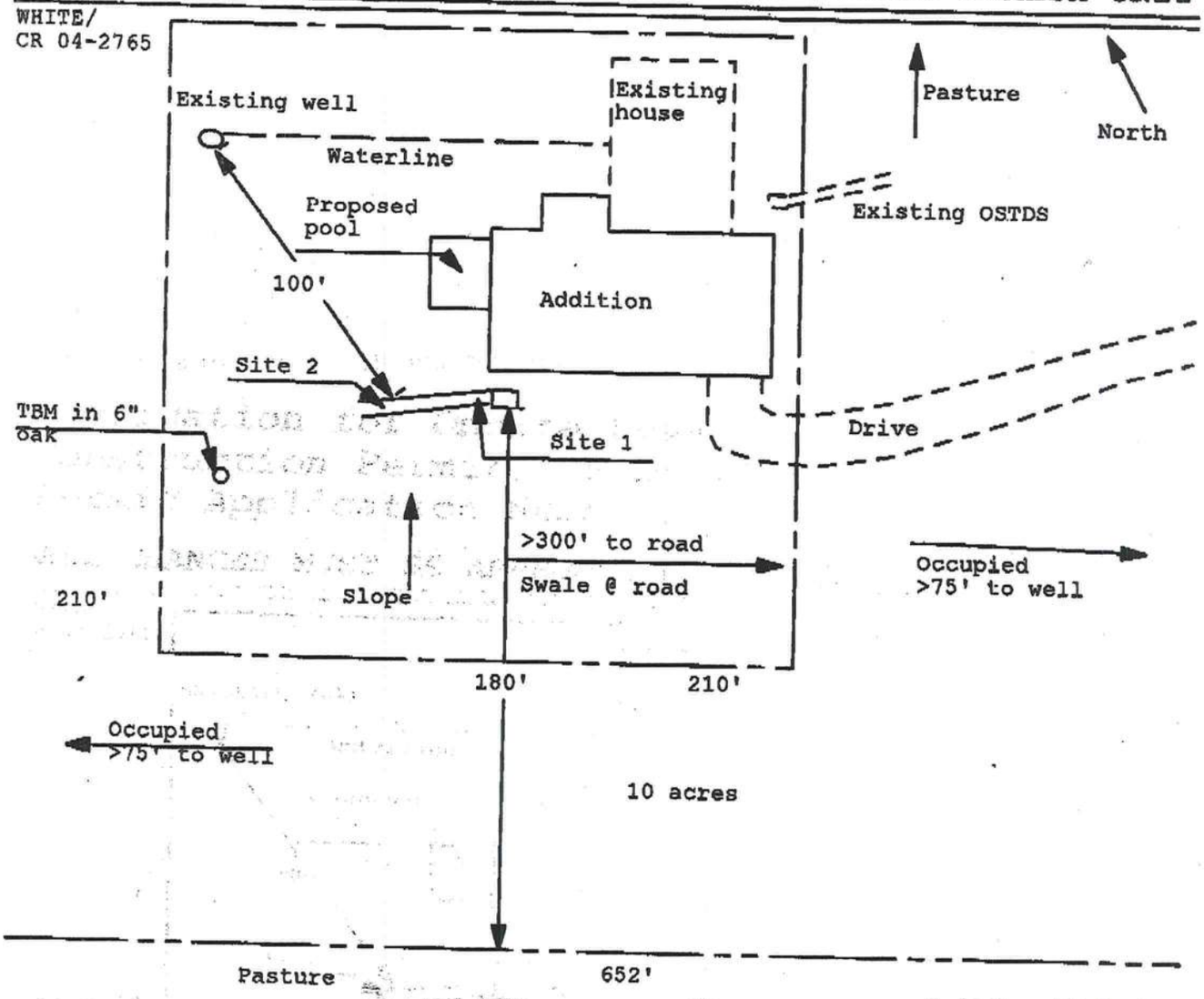
36293

# Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

**Permit Application Number:** 05-0461N

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT**

WHITE/
CR 04-2765



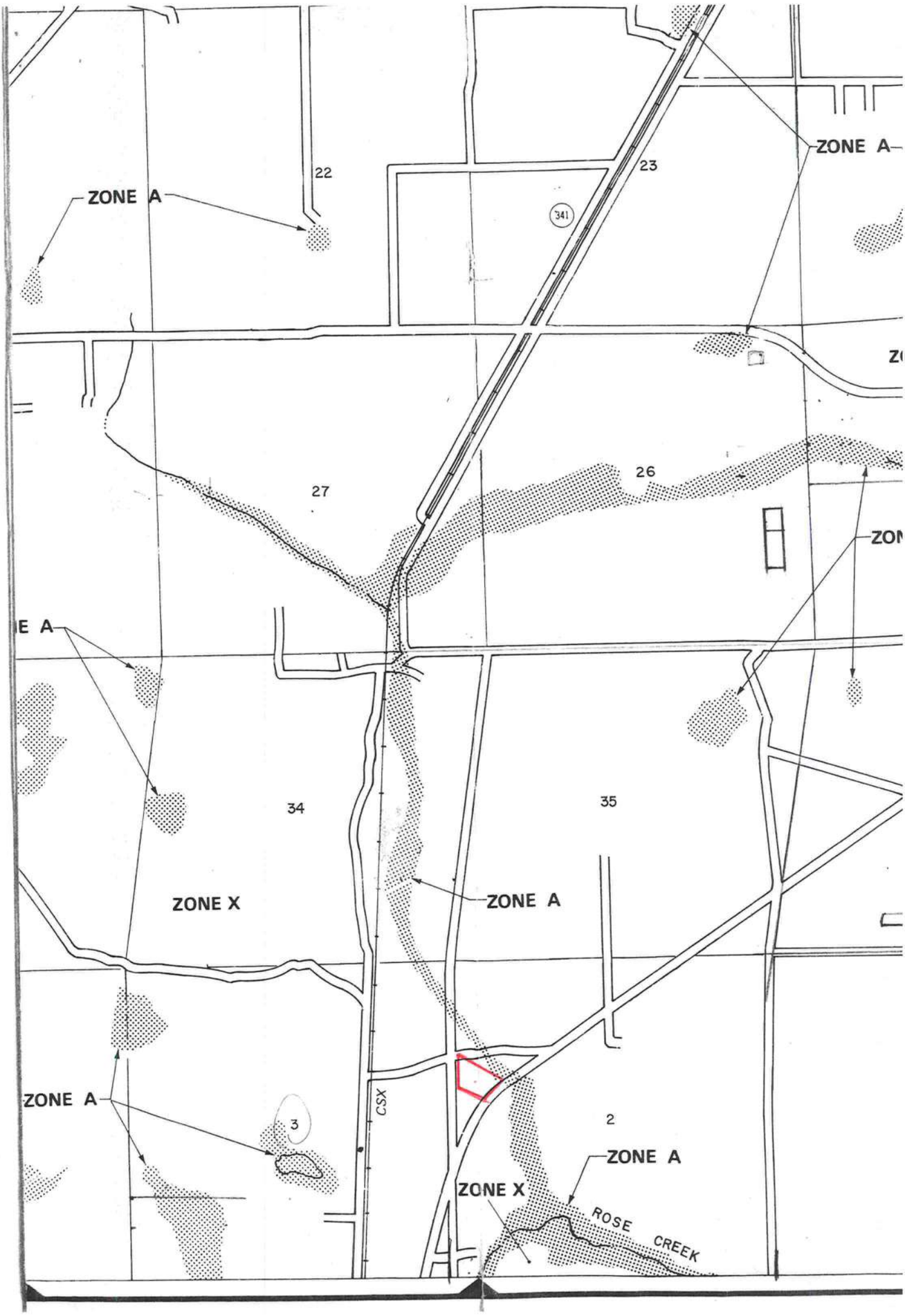

1 inch = 50 feet

Site Plan Submitted By Paul Lloyd Date 4/21/05

Plan Approved ✓ Not Approved ___ Date 6-28-05

By [signature] Columbia CPHU

Notes: _____


ZONE A
22
23
341
ZONE A
27
26
E A
34
35
ZONE X
ZONE A
ZONE A
3
CSX
2
ZONE A
ZONE X
ROSE CREEK

0506-88

758-2160

# FAX

Date: 7-12-05

To: Brian Kepner

From: Linda

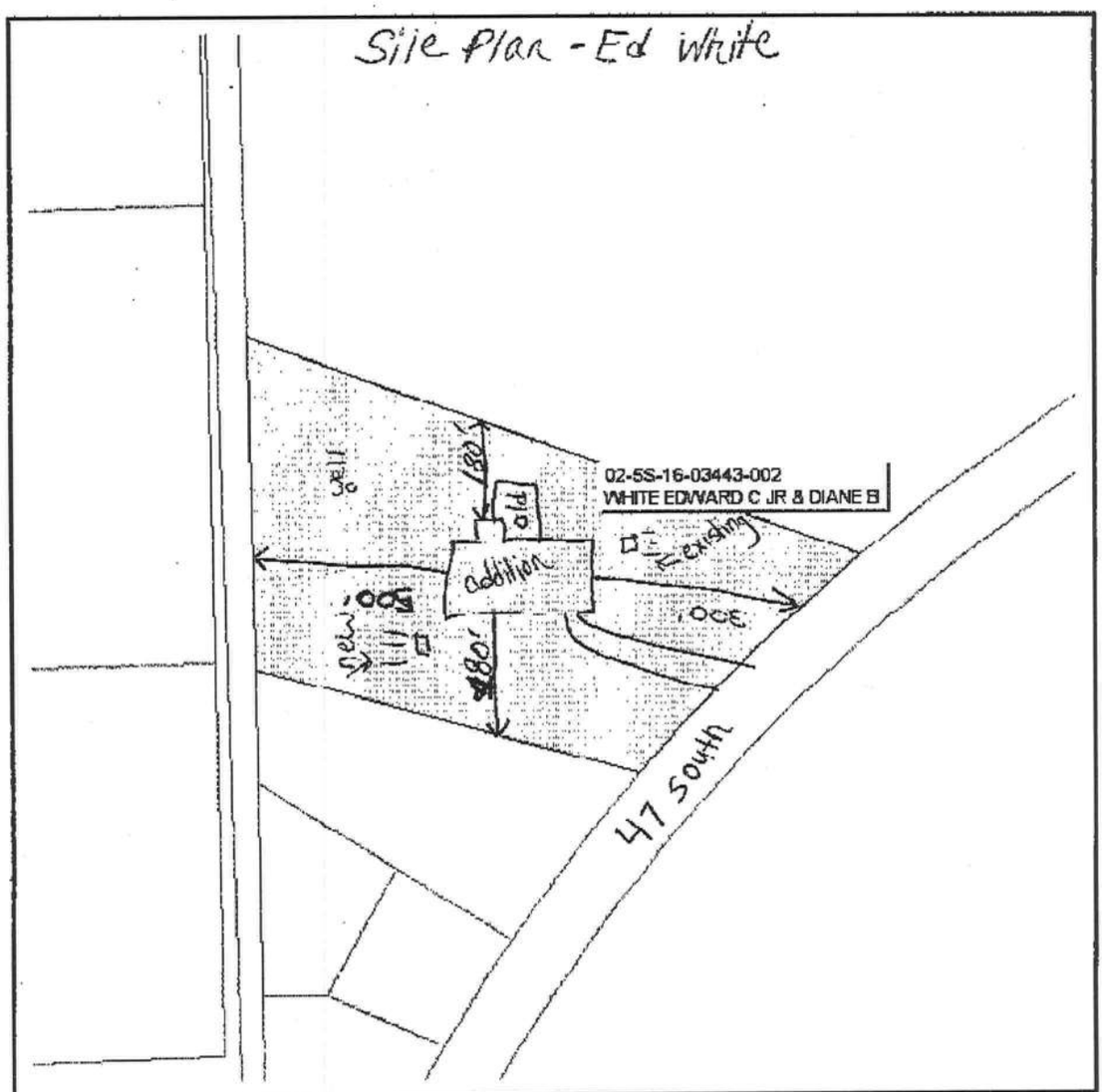
North Florida Permit Services, INC.
Ph: 386-752-2281 Fax: 386-752-2282

Pages: 2

Attn: Brian

Here is revised site plan for Edwhite.

Linda




## Columbia County Property Appraiser

J. Doyle Crews, CPA - Lake City, Florida - 386-758-1083

**PARCEL: 02 5S 16 03443 002** - IMPROVED A (005000)

COMM SW COR OF SW1/4 OF NW1/4, RUN N 658.91 FT FOR POB, CONT N 500 FT, SE 1000.05 FT TO W

| | | | | | |
|---|---|---|---|---|---|
| Name: | WHITE EDWARD C JR & DIANE B | | | LandVal | $9,600.00 |
| Site: | SR 47 | | | BldgVal | $106,797.00 |
| Mail: | P O BOX 2111 | | | ApprVal | $121,474.00 |
| | LAKE CITY, FL 32056 | | | JustVal | $160,372.00 |
| Sales | 4/28/2003 | $100.00 | I / U | Assd | $121,474.00 |
| Info | 7/26/2001 | $100.00 | I / U | Exmpt | $0.00 |
| | | | | Taxable | $121,474.00 |

This information, GIS Map Updated: 6/2/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

# HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
904 NW Main Blvd.
LAKE CITY, FLORIDA 32055

June 12, 2002

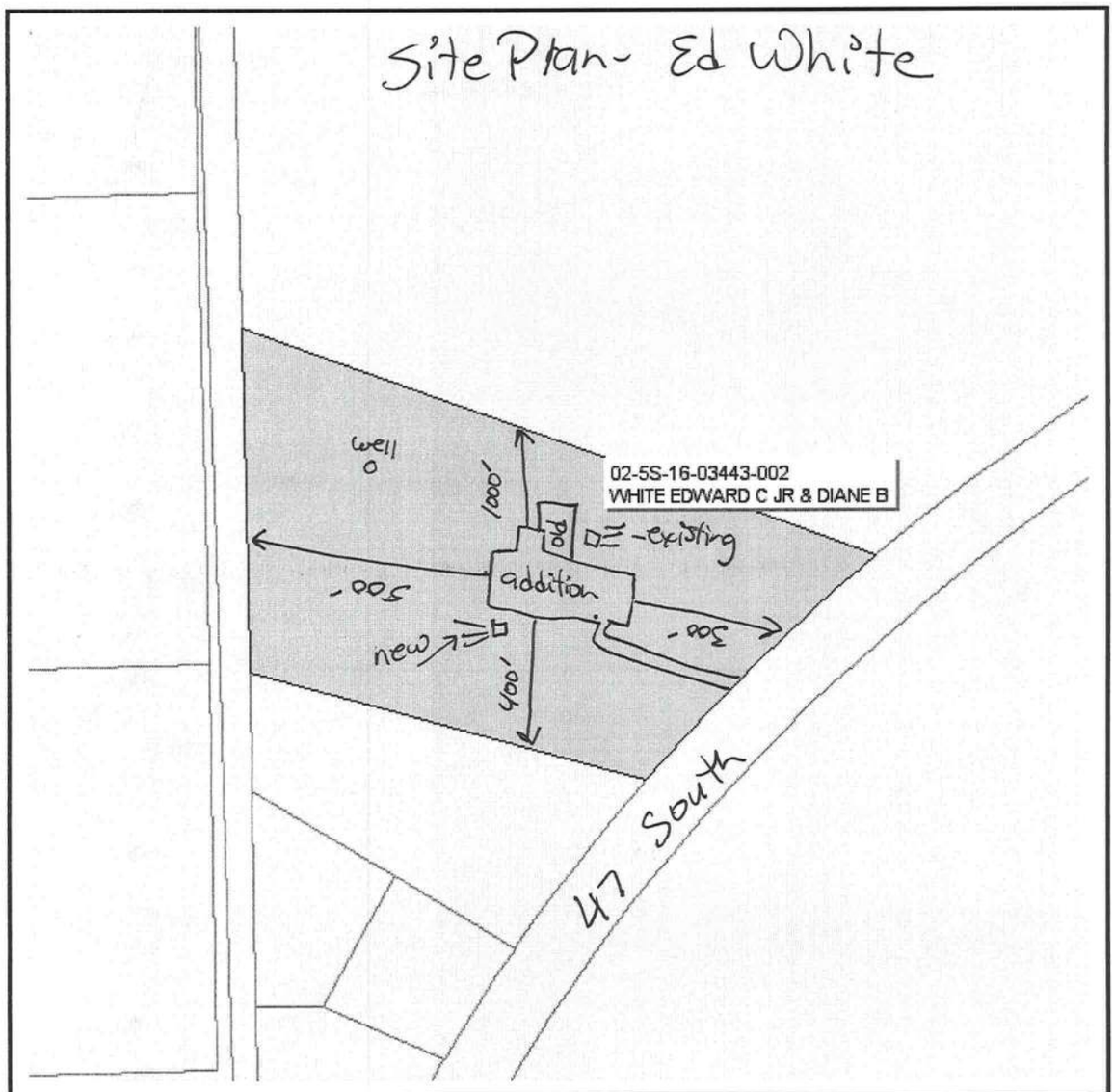
NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

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J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

**PARCEL: 02-5S-16-03443-002** - IMPROVED A (005000)

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|---|---|---|---|---|
| Name: | WHITE EDWARD C JR & DIANE B | | LandVal | $9,600.00 |
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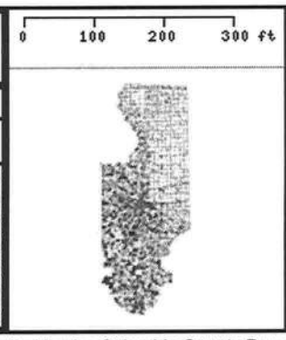
| Mail: | P O BOX 2111 | | ApprVal | $106,960.00 |
| | LAKE CITY, FL 32056 | | JustVal | $145,858.00 |
| Sales | 4/28/2003 | $100.00 I / U | Assd | $106,960.00 |
| Info | 7/26/2001 | $100.00 I / U | Exmpt | $0.00 |
| | | | Taxable | $106,960.00 |





This information, GIS Map Updated: 4/4/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

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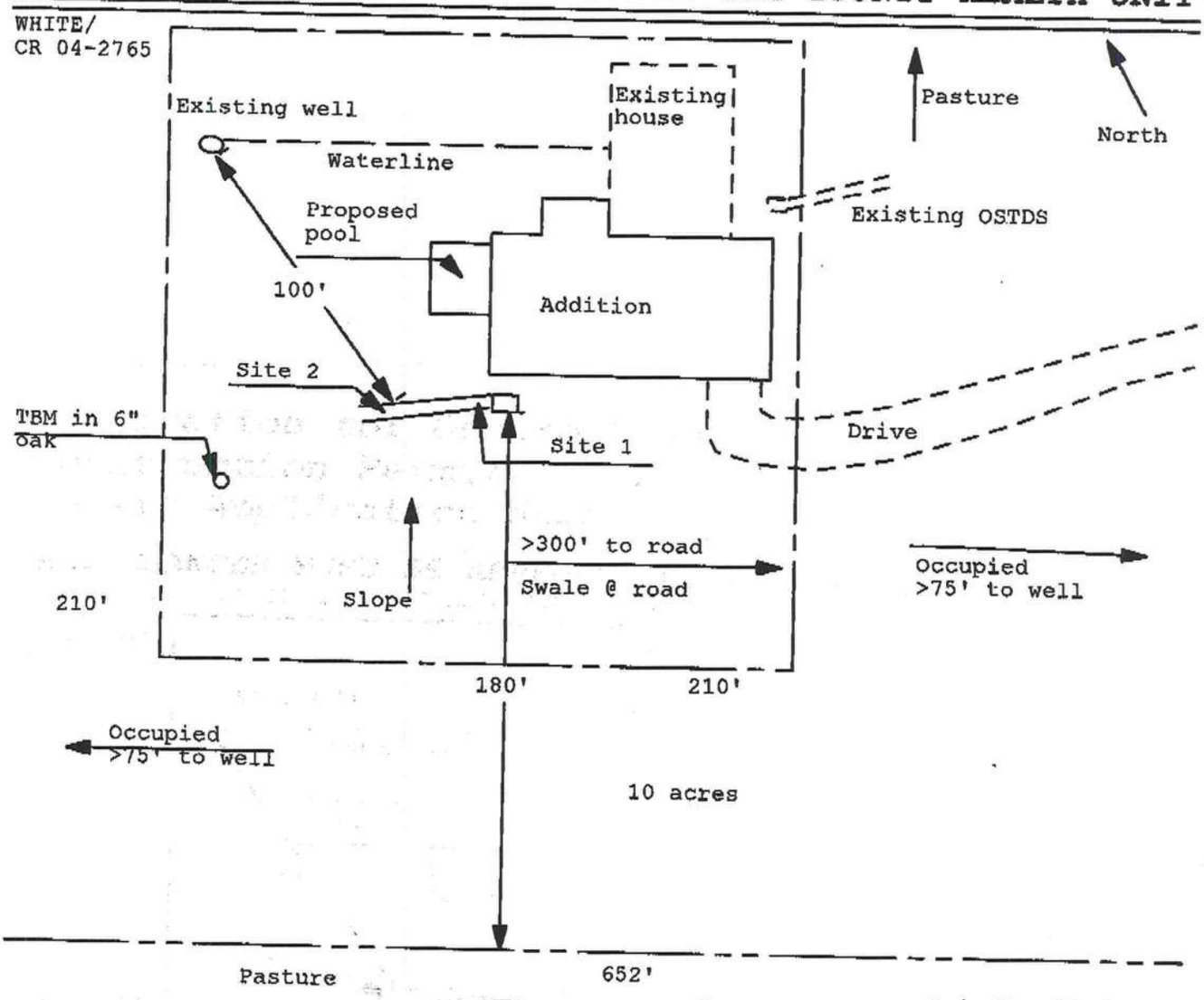
**Permit Application Number:** 05-0461N

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT**

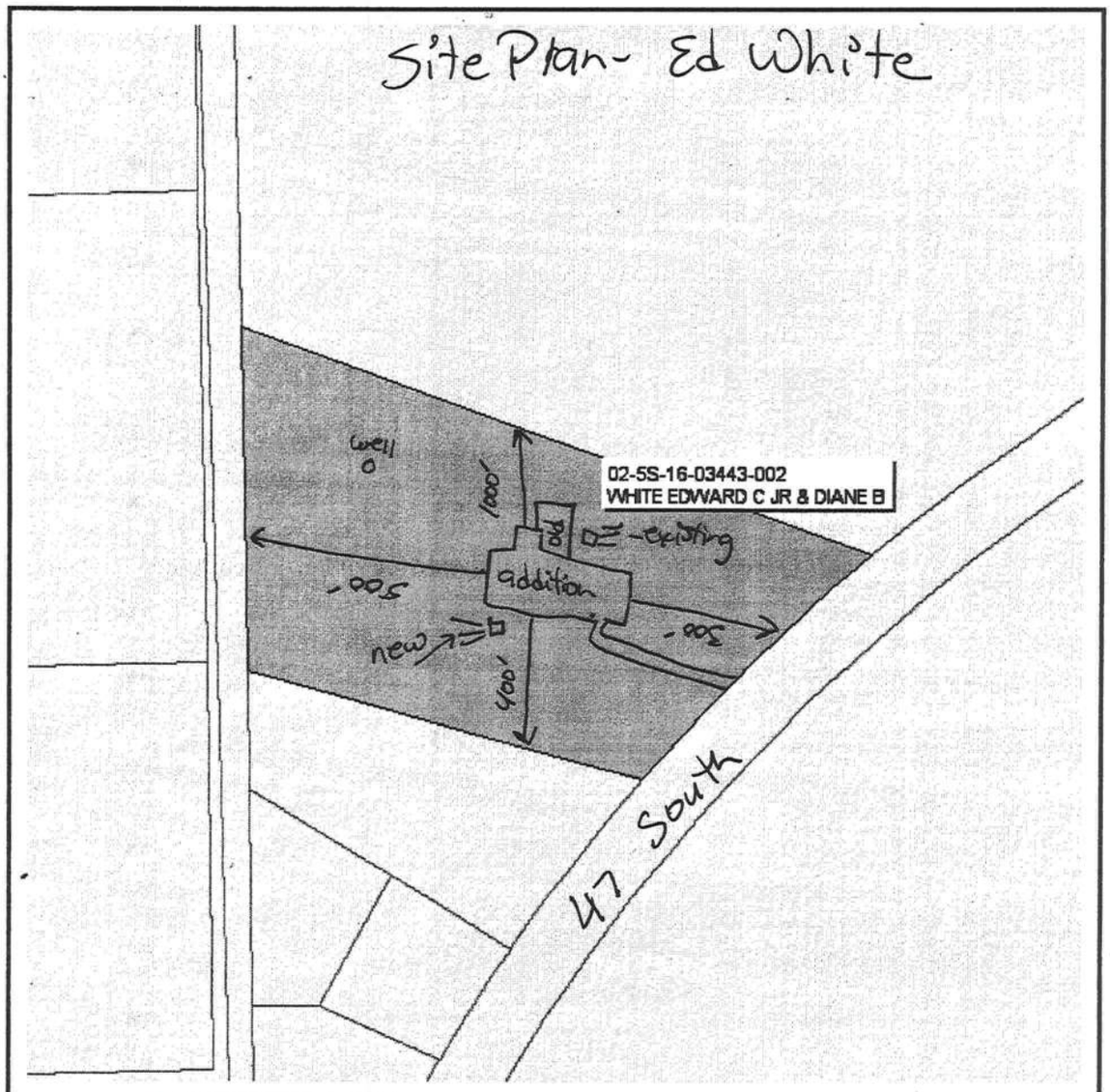



Site Plan Submitted By Paul Lloyd Date 4/21/05

Plan Approved ✓ Not Approved ___ Date 6-28-05

By [signature] Columbia CPHU

Notes: _____




## Columbia County Property Appraiser

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**PARCEL: 02-5S-16-03443-002** - IMPROVED A (005000)

COMM SW COR OF SW1/4 OF NW1/4, RUN N 658.91 FT FOR POB, CONT N 500 FT, SE 1000.05 FT TO W

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| | | | JustVal | $145,858.00 |
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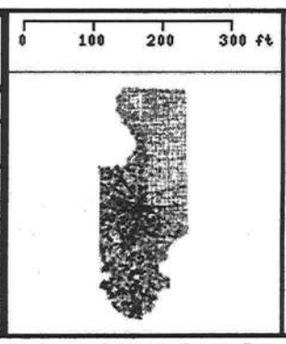



**FORM 600B-01**

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION
## Residential Component Prescriptive Method B

**NORTH 1 2 3**

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

| | | | |
|---|---|---|---|
| **PROJECT NAME AND ADDRESS:** Bishop White | | **BUILDER:** | |
| | | **PERMITTING OFFICE:** Columbia County | **CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☐ |
| **OWNER:** EJ & Diane Bishop White | | **PERMIT NO.:** | **JURISDICTION NO.:** |

**GENERAL DIRECTIONS**

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.
2. Choose one of the component packages "A" through "E" fromTable 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages", Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

| | Please Print | CK |
|---|---|---|
| 1. **Compliance package chosen (A-F)** | D | |
| 2. **New construction or addition** | Addition | |
| 3. **Single family detached or Multifamily attached** | Sgl. Fam. | |
| 4. **If Multifamily—No. of units covered by this submission** | | |
| 5. **Is this a worst case?** (yes / no) | NO | |
| 6. **Conditioned floor area** (sq. ft.) | 3044 | |
| 7. **Predominant eave overhang** (ft.) | 3 | |
| 8. **Glass type and area :** | Single Pane / Double Pane | |
| a. Clear glass | _____ sq. ft. / _____ sq. ft. | |
| b. Tint, film or solar screen | _____ sq. ft. / 636 sq. ft. | |
| 9. **Percentage of glass to floor area** | 20 % | |
| 10. **Floor type, area or perimeter, and insulation:** | | |
| a. Slab on grade (R-value) | R= 0 / _____ lin. ft. | |
| b. Wood, raised (R-value) | R= _____ / _____ sq. ft. | |
| c. Wood, common (R-value) | R= _____ / _____ sq. ft. | |
| d. Concrete, raised (R-value) | R= _____ / _____ sq. ft. | |
| e. Concrete, common (R-value) | R= _____ / _____ sq. ft. | |
| 11. **Wall type, area and insulation:** | | |
| a. Exterior: 1. Masonry (Insulation R-value) | R= _____ / _____ sq. ft. | |
| 2. Wood frame (Insulation R-value) | R= 11 / 1749 sq. ft. | |
| b. Adjacent: 1. Masonry (Insulation R-value) | R= _____ / _____ sq. ft. | |
| 2. Wood frame (Insulation R-value) | R= _____ / _____ sq. ft. | |
| 12. **Ceiling type, area and insulation:** | | |
| a. Under attic (Insulation R-value) | R= 30 / 2440 sq. ft. | |
| b. Single assembly (Insulation R-value) | R= _____ / _____ sq. ft. | |
| 13. **Air Distribution System: Duct insulation, location Test report** (attach if required) | R= 6 | |
| 14. **Cooling system** (Types: central, room unit, package terminal A.C., gas, none) | 14a. Type: Central | |
| | 14b. SEER/EER: 12.5 | |
| | 14c. Capacity: 5 Ton | |
| 15. **Heating system:** (Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none) | 15a. Type: Heat Pump | |
| | 15b. HSPF/COP/AFUE: | |
| | 15c. Capacity: 60K | |
| 16. **Hot water system:** (Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none) | 16a. Type: Elect. HRV | |
| | 16b. EF: | |

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

**PREPARED BY:** _____ **DATE:** 6-6-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

**OWNER AGENT:** _____ **DATE:** 6-7-05

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

**BUILDING OFFICIAL:** _____

**DATE:** _____

## TABLE 6B-1 MINIMUM REQUIREMENTS

Climate Zones 1 2 3

| COMPONENTS | | PACKAGES FOR NEW CONSTRUCTION | | | | | TO BE INSTALLED |
|---|---|---|---|---|---|---|---|
| | | A | B | C | D | E | |
| GLASS | Max.%of glass to Floor Area | 15% | 15% | 20% | 20% | 25% | 30 % |
| | Type | Double Clear (DC) | Double Clear (DC) | Double Clear (DC) | Double Clear (DC) | Double Tint (DT) | DC: ☑ DT: ☐ |
| | Overhang | 1'4" | 2' | 2' | 2' | 2' | 2 FEET |
| WALLS | Masonry | EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE. | | | | | EXT: R = ADJ: R = COM: R = |
| | Wood Frame | EXTERIOR, ADJACENT, AND COMMON WOOD FRAME WALLS R-11 | | | | | EXT: R = 13 ADJ: R = COM: R = |
| CEILINGS | | R-30 | R-30 | R-30 | R-30 | R-30 | UNDER ATTIC: R = 30 COMMON: R = |
| | | (NO SINGLE ASSEMBLY CEILINGS ALLOWED) | | | | | |
| FLOORS | Slab-On-Grade | R-0 | | | | | R = 0 |
| | Raised Wood | R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C) | | | | | R = |
| | Raised Concrete | R-7 | | | | | R = |
| DUCTS | | R-6 | R-6 | R-6, TESTED | R-6 | R-6, TESTED | R = 6 COND. ☐ |
| SPACE COOLING (SEER) | | 12.0 | 10.5 | 12.0 | 11.0 | 12.0 | SEER = 12.0 |
| HEAT | Elect. (HSPF) | 7.9 | 7.1 | 7.4 | 7.4 | 7.4 | COP= 7.4 |
| | Gas/Oil (AFUE) | MINIMUM OF .73 (Direct heating) or .78 (Central) | | | | | AFUE = |
| HOT WATER SYSTEM | Electric Resistance** | EF .88 | NOT ALLOWED (SEE BELOW) | EF .91 | NOT ALLOWED (SEE BELOW) | EF .91 | EF = |
| | Gas & Oil ** | MINIMUM EF OF .54 | | | | NATURAL GAS ONLY (SEE BELOW) | EF = |
| | Other | Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system. | | | | | DHP: ☑ EF= HRU: ☑ SOLAR: ☐ EF= |

* Single package units minimum SEER=9.7, HSPF = 6.6.

** Minimum efficiencies for gas and electric hot water systems apply to to 40 gallon water heaters. Refer to Table 6-12 for minimum Code efficiencies for oil water heaters and other sizes.

**DESCRIPTION OF BUILDING COMPONENTS LISTED**

**Percent of Glass to Floor Area:** This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

**Overhang:** The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions:
1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multi-story house.

**Wall, Ceiling and Floor Insulation Values:** The R-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The R-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multifamily building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

**Floor:** Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

**Ducts:** "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a State-approved tester.

**Space Cooling System:** Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTAC's equal to or greater than the prescribed value.

**Electric Space Heating Option:** Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

**Electric Resistance Hot Water Option:** For packages designated "Not Allowed", an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options". See below.

**Other Hot Water System Options:** Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having an EF of .88 or greater, or natural gas systems with EF .54 or greater may be used in conjunction with these systems.

## TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

| COMPONENTS | SECTION | REQUIREMENTS | CHECK |
|---|---|---|---|
| Exterior Joints & Cracks | 606.1 | To be caulked, gasketed, weather-stripped or otherwise sealed. | ✓ |
| Exterior Windows & Doors | 606.1 | Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area. | ✓ |
| Sole & Top Plates | 606.1 | Sole plates and penetrations through top plates of exterior walls must be sealed. | ✓ |
| Recessed Lighting | 606.1 | Type IC rated with no penetrations (two alternatives allowed). | ✓ |
| Multi-story Houses | 606.1 | Air barrier on perimeter of floor cavity between floors. | ✓ |
| Exhaust Fans | 606.1 | Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork. | ✓ |
| Water Heaters | 612.1 | Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers. | ✓ |
| Swimming Pools & Spas | 612.1 | Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%. | ✓ |
| Hot Water Pipes | 612.1 | Insulation is required for hot water circulating systems (including heat recovery units). | ✓ |
| Shower Heads | 612.1 | Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG. | ✓ |
| HVAC Duct Construction, Insulation & Installation | 610.1 | All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6. | ✓ |
| HVAC Controls | 607.1 | Separate readily accessible manual or automatic thermostat for each system. | ✓ |

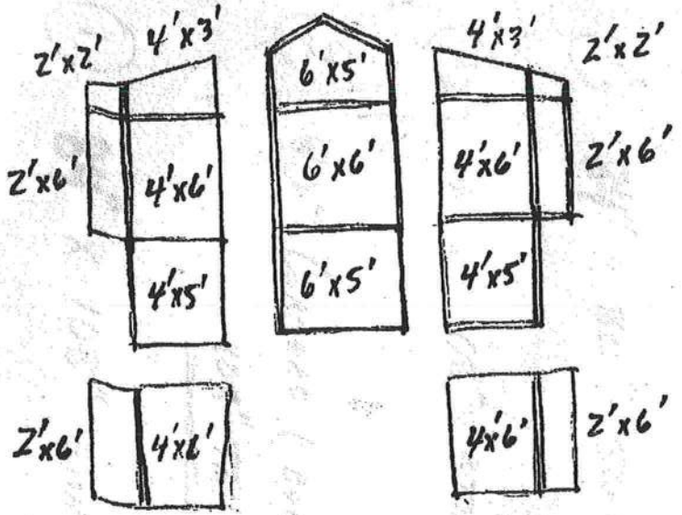
White Residence

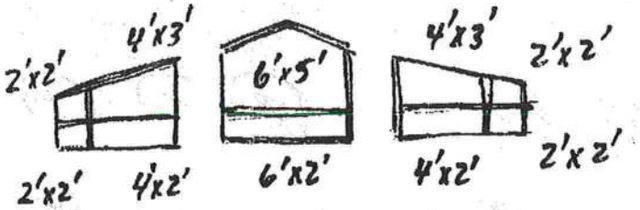
Boozer

| # | QTY | |
|---|---|---|
| 1.) | (2) | 90° CORNERS (Butt Glazed) 80" TALL   |
| 2.) | (2) | 36" x 80" STORE FRONT |
| 3.) | (1) | 84" x 72" STOREFRONT PENTAGON   |
| 4.) | (2) | 90° CORNERS (Butt glazed) 80" TALL   |
| 5.) | (1) | 84" x 80" STORE FRONT |
| 6.) | (2) | 96" x 108" STORE FRONT — THIS MUST BE Split into 2pcs. |
| 7.) | (2) | 90° CORNERS (Butt glazed) 108" TALL   |
| 8.) | (1) | 84" x 108" STORE FRONT — THIS MUST BE Split into 2pcs. |
| 9.) | (2) | 60" x 48" STORE FRONT 1-LEFT 1-RIGHT   |

May 26 05 07:24a Chris Whitfield 386-454-0335

386-454-0395
Chris Whitfield




1/8" = 1' Foot
To Scale

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

**FORM 600B-01** — **Residential Component Prescriptive Method B** — **NORTH 1 2 3**

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

| | | | |
|---|---|---|---|
| **PROJECT NAME AND ADDRESS:** Bishop White | | **BUILDER:** | |
| | | **PERMITTING OFFICE:** Columbia County | **CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☑ |
| **OWNER:** EJ & Diane Bishop White | | **PERMIT NO.:** 23411 | **JURISDICTION NO.:** 221000 |

**GENERAL DIRECTIONS**

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.
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6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

| | Please Print | CK |
|---|---|---|
| **1. Compliance package chosen (A-F)** | D | |
| **2. New construction or addition** | Addition | |
| **3. Single family detached or Multifamily attached** | Sgl. Fam. | |
| **4. If Multifamily—No. of units covered by this submission** | | |
| **5. Is this a worst case?** (yes / no) | NO | |
| **6. Conditioned floor area** (sq. ft.) | 3044 | |
| **7. Predominant eave overhang** (ft.) | 3 | |
| **8. Glass type and area :** | Single Pane / Double Pane | |
| a. Clear glass | 8a. _____ sq. ft. / _____ sq. ft. | |
| b. Tint, film or solar screen | 8b. _____ sq. ft. / 636 sq. ft. | |
| **9. Percentage of glass to floor area** | 9. 20 % | |
| **10. Floor type, area or perimeter, and insulation:** | | |
| a. Slab on grade (R-value) | 10a. R= 0 _____ lin. ft. | |
| b. Wood, raised (R-value) | 10b. R= _____ _____ sq. ft. | |
| c. Wood, common (R-value) | 10c. R= _____ _____ sq. ft. | |
| d. Concrete, raised (R-value) | 10d. R= _____ _____ sq. ft. | |
| e. Concrete, common (R-value) | 10e. R= _____ _____ sq. ft. | |
| **11. Wall type, area and insulation:** | | |
| a. Exterior: 1. Masonry (Insulation R-value) | 11a-1 R= _____ _____ sq. ft. | |
| 2. Wood frame (Insulation R-value) | 11a-2 R= 11 1749 sq. ft. | |
| b. Adjacent: 1. Masonry (Insulation R-value) | 11b-1 R= _____ _____ sq. ft. | |
| 2. Wood frame (Insulation R-value) | 11b-2 R= _____ _____ sq. ft. | |
| **12. Ceiling type, area and insulation:** | | |
| a. Under attic (Insulation R-value) | 12a. R= 30 2440 sq. ft. | |
| b. Single assembly (Insulation R-value) | 12b. R= _____ _____ sq. ft. | |
| **13. Air Distribution System: Duct insulation, location Test report** (attach if required) | 13. R= 6 _____ | |
| **14. Cooling system** | 14a. Type: Central | |
| (Types: central, room unit, package terminal A.C., gas, none) | 14b. SEER/EER: 12.2 | |
| | 14c. Capacity: 5 Ton | |
| **15. Heating system:** | 15a. Type: Heat Pump | |
| (Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none) | 15b. HSPF/COP/AFUE: | |
| | 15c. Capacity: 60K | |
| **16. Hot water system:** | 16a. Type: Elect. HRV | |
| (Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none) | 16b. EF: | |

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

**PREPARED BY:** _____ **DATE:** 6-6-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

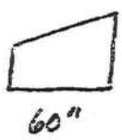
**OWNER AGENT:** _____ **DATE:** 6-7-05

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

**BUILDING OFFICIAL:** _____

**DATE:** _____

White Residence
BOOZER

| # | QTY | |
|---|---|---|
| 1.) | (2) | 90° CORNERS (Butt Glazed) 80" TALL   |
| 2.) | (2) | 36" x 80" STORE FRONT |
| 3.) | (1) | 84" x 72" STOREFRONT PENTAGON   |
| 4.) | (2) | 90° Corners (Butt glazed) 80" TALL   |
| 5.) | (1) | 84" x 80" STORE FRONT |
| 6.) | (2) | 96" x 108" STORE FRONT → THIS MUST BE Split into 2pcs. |
| 7.) | (2) | 90° Corners (Butt glazed) 108" TALL   |
| 8.) | (1) | 84" x 108" STORE FRONT → THIS MUST BE Split into 2pcs. |
| 9.) | (2) | 60" x 48" STORE FRONT 1-LEFT 1-RIGHT   |


386-454-0395 Chris Whitfield
May 26 05 07:24a

1/8" = 1' Foot
To Scale

## NOTICE OF COMMENCEMENT FORM
## COLUMBIA COUNTY, FLORIDA

HE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance ith Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

ax Parcel ID Number Part of 03-55-16-03445-001

Description of property: (legal description of the property and street address or 911 address)
7018 SW SR 47 Lake City, FL 32024

Inst:2005016124 Date:07/08/2005 Time:10:33
DC,P.DeWitt Cason,Columbia County B:1051 P:653

General description of improvement: Single Family Dwelling

Owner Name & Address Ed and Diane White P.O. Box 28639
Atlanta, GA 30358 Interest in Property

Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name Isaac Construction, Inc. Phone Number (386)-719-7143
Address 144 SW Waterford Ct. Lake City Fl. 32025

6. Surety Holders Name N/A Phone Number
Address
Amount of Bond N/A

7. Lender Name N/A Phone Number
Address

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name N/A Phone Number
Address

9. In addition to himself/herself the owner designates N/A of
to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –
(a) 7. Phone Number of the designee

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording,
(Unless a different date is specified)

**NOTICE AS PER CHAPTER 713, Florida Statutes:**
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Sworn to (or affirmed) and subscribed before
day of 1st July, 2005

Signature of Owner

NOTARY PUBLIC STATE OF FLORIDA
Barbara C. Webster
Commission # DD329279
Expires July 2, 2008
Bonded Troy Fain - Insurance, Inc. 800-385-7019

NOTARY STAMP/SEAL

Barbara C Webster
Signature of Notary

**ISAAC Construction Inc.**

**Remodel & Addition of a single family dwelling**

**Property owner Edward & Diane White**

**Application for building permit # 0506-88 date file 6/28/05**

**July 21, 2005 a meeting at the Columbia County Building Department with Robert Taylor (Associated Florida Architects Inc). Edward White (Property owner) Isaac Bratkovich (ISAAC Construction Inc.) Joe Haltiwanger (Columbia County Building Department Plan Examiner) to review structural plans as presented with application for permit number 0506-88.**

**July 22, 2005 Robert Taylor (Associated Florida Architects Inc) submits a letter of discovery, filed with application for permit number 0506-88.**

**July 22, 2005 Addendum Number One submitted from: Robert Taylor (Associated Florida Architects Inc) to application for permit number 0506-88.**

**July 25, 2005 Columbia County Building Department issues building permit to ISAAC Construction Inc. Permit number 23411**

August 09, 2005 Temporary power service inspection by Randy Jones (Assistant Building Official) Passed inspection.

August 15, 2005 Rough plumbing inspection by Randy Jones (Assistant Building Official) Passed inspection

September 06, 2005 Foundation inspection (Monolithic Slab) for addition by Randy Jones (Assistant Building Official) Passed inspection.

September 06, 2005 Zoning land use setback inspection by Randy Jones (Assistant Building Official) Passed inspection.

December 13, 2005 Inspection of dwelling requested by owner Edward White, whom was present with Personal from Columbia County Building Department, Randy Jones Assistant Building Official, Harry Dicks Building Inspector and Joe Haltiwanger Plans Examiner. Performed inspection and photographed construction site.

December 15, 2005 a letter from the Columbia County Building Department was given to Mr. White and ISAAC Construction Inc. This letter included photographs and structural deficiency found on the date of the inspection. This letter also required a structural evaluation be preformed by a professional architect or engineer before continuing work on the dwelling.

July 25, 2006 building permit number 23411expries, no request by ISAAC Construction Inc. was made for an extension of the building permit.

July 28, 2006 the Columbia County Building Department received a hand delivered letter from Edward White to John Kerce Columbia County Building Official. This letter informed the building department that a professional engineer had made an inspection of the dwelling. This letter also requested that additional inspections be preformed by the Columbia County Building Department to take action of permit violations.

August 21, 2006 Letter from Edward White to John Kerce Columbia County Building Official was forwarded to Columbia County Attorney Marlin Feagle.

August 24, 2006 a letter from Columbia County Attorney Marlin Feagle to Columbia County plans examiner Joe Haltiwanger was received.
This letter informed Mr. White the process to renew the expired permit and the method to file a compliant with the Florida Department of Business and Professional Regulations due the contractor Isaac Bratkovich CBC059323 (ISAAC Construction Inc.) is licensed and regulate by the Florida Department of Business and Professional Regulations.

August 29, 2006 the Columbia County Attorney Marlin Feagle and Columbia County Building Department personal Randy Jones Assistant Building Official, Harry Dicks Building Inspector and Joe Haltiwanger Plans Examiner met with Mr. Edward White at the Columbia County Building Department office. A letter from William Freeman P.E. (Freeman Design Group Inc.) was presented, which detailed a structural inspection preformed by Mr. Freeman. This letter presented methods to correct structural problems and was placed into Mr. White file.

Columbia County Attorney Marlin Feagle presented to Mr. White methods to resolve the financial aspect of his request to the Columbia County Building Department. Mr. Feagle explained that Columbia County Building Department did not license or regulate Mr. Robert Taylor or Mr. Isaac Bratkovich, but the building department would contact the Florida Department of Business and Professional Regulations and assist this department after a formal complaint was filed by Mr. White with the Florida Department of Business and Professional Regulations.

August 30, 2006 Harry Dicks Building Inspector with the Columbia County Building Department spoke by phone to Mr. Jim Patton field investigator with the Florida Department of Business and Professional Regulations. Mr. Dicks discussed the request from Mr. White to the Columbia County Building Department. Mr. Patton advised Mr. Dicks that he would contact Mr. White and assist him in making any formal complaints to his department.

August 30, 2006 Mr. Jim Patton field investigator with the Florida Department of Business and Professional Regulations contacted Mr. Harry Dicks Columbia County Building Inspector by phone and advised that after contacting Mr. White by phone Mr. White at this date wish not to make any formal complaint to the Florida Department of Business and Professional Regulations on either Mr. Isaac Bratkovich or Mr. Robert Taylor.

# Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1SNC487-Z0125081043

| | |
|---|---|
| Truss Fabricator: | Anderson Truss Company |
| Job Identification: | 5-214-ISAAC CONST/ED & DIANE WHITE - ROOF |
| Truss Count: | 13 |
| Model Code: | Florida Building Code 2001 |
| Truss Criteria: | ANSI/TPI-1995 |
| Engineering Software: | Alpine Software,Version 7.04. |
| Structural Engineer of Record: | |
| Address: | |
| Minimum Design Loads: | Roof - 40.0 PSF @ 1.25 Duration<br>Floor - N/A<br>Wind - 110 MPH ASCE 7-98 -Closed |

Seal Date: 05/25/2005

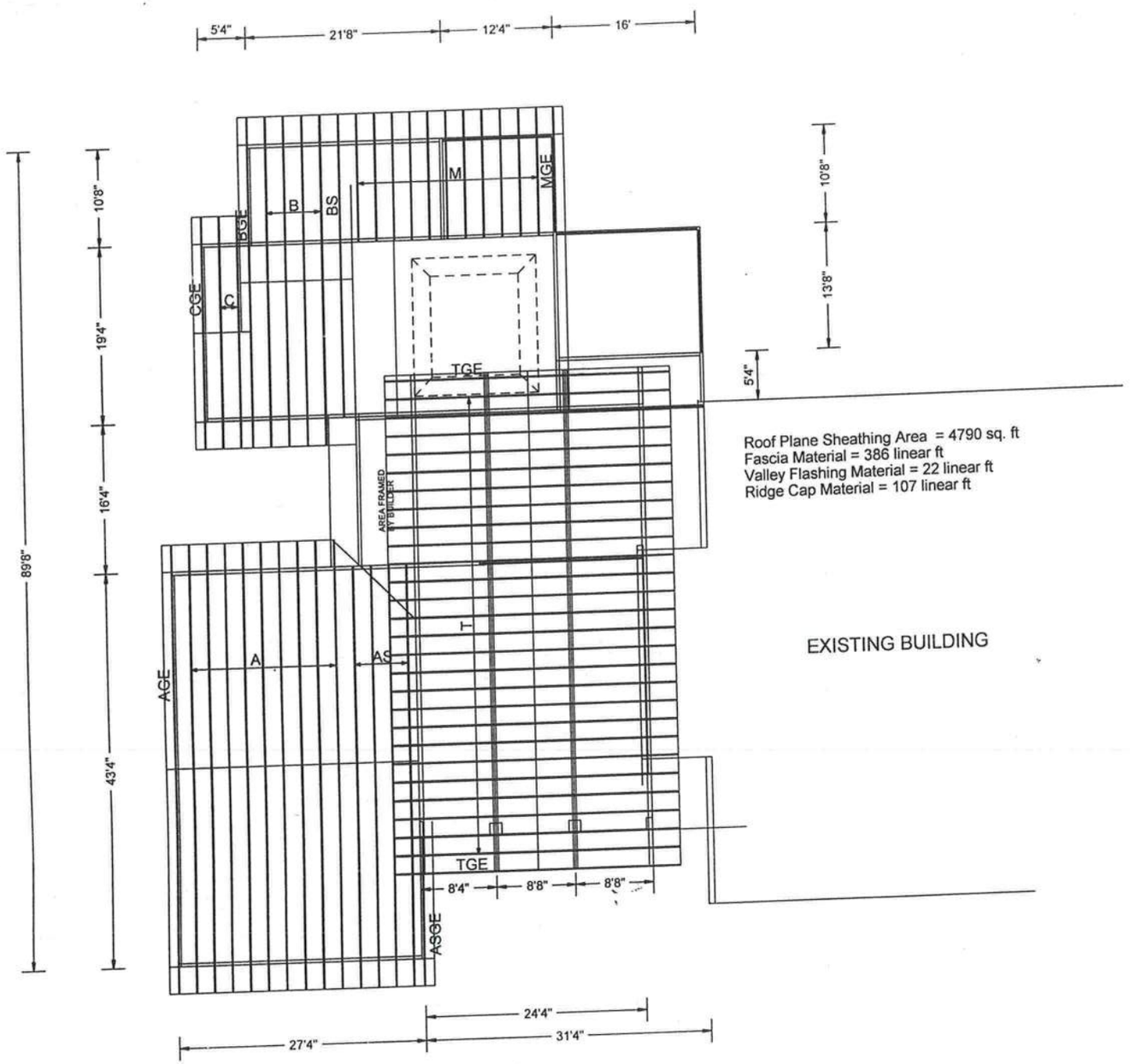
-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-1995 Section 2.2
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-A11015EC-GBLLETIN

| # | Ref Description | Drawing# | Date |
|---|---|---|---|
| 1 | 66515--A | 05144001 | 05/24/05 |
| 2 | 66516--AS | 05144002 | 05/24/05 |
| 3 | 66517--AGE | 05144003 | 05/24/05 |
| 4 | 66518--B | 05144004 | 05/24/05 |
| 5 | 66519--BS | 05144005 | 05/24/05 |
| 6 | 66520--BGE | 05144006 | 05/24/05 |
| 7 | 66521--C | 05144007 | 05/24/05 |
| 8 | 66522--CGE | 05144013 | 05/24/05 |
| 9 | 66523--M | 05144008 | 05/24/05 |
| 10 | 66524--MGE | 05144009 | 05/24/05 |
| 11 | 66525--ASGE | 05144010 | 05/24/05 |
| 12 | 66526--T | 05144011 | 05/24/05 |
| 13 | 66527--TGE | 05144012 | 05/24/05 |




ISAAC CONSTRUCTION/
ED & DIANE WHITE /ROOF

JOB LOCATION:

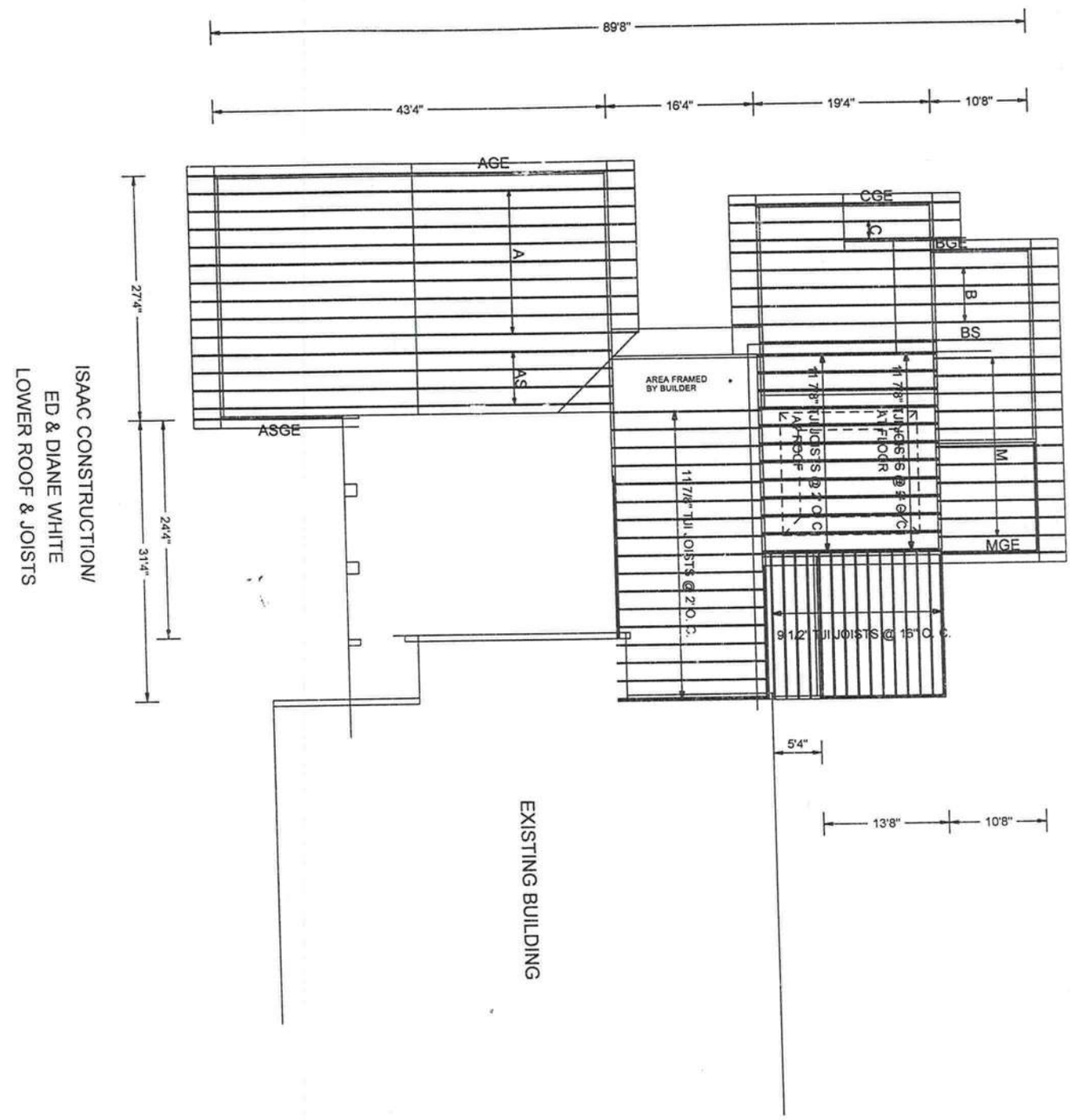
JOB DESCRIPTION: ISAAC CONST/ED & DIANE WHITE

DESIGNED BY:

JOB NO: 5-214

PAGE NO: 1 OF 1

ISAAC CONSTRUCTION/
ED & DIANE WHITE
LOWER ROOF & JOISTS




| DESIGNED BY: | JOB DESCRIPTION: | JOB LOCATION: |
|---|---|---|
| | ISAAC CONST/ED & DIANE WHITE | |

JOB NO: 5-214A

PAGE NO: 1 OF 1

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W6, W16 2x4 SP #2 Dense:

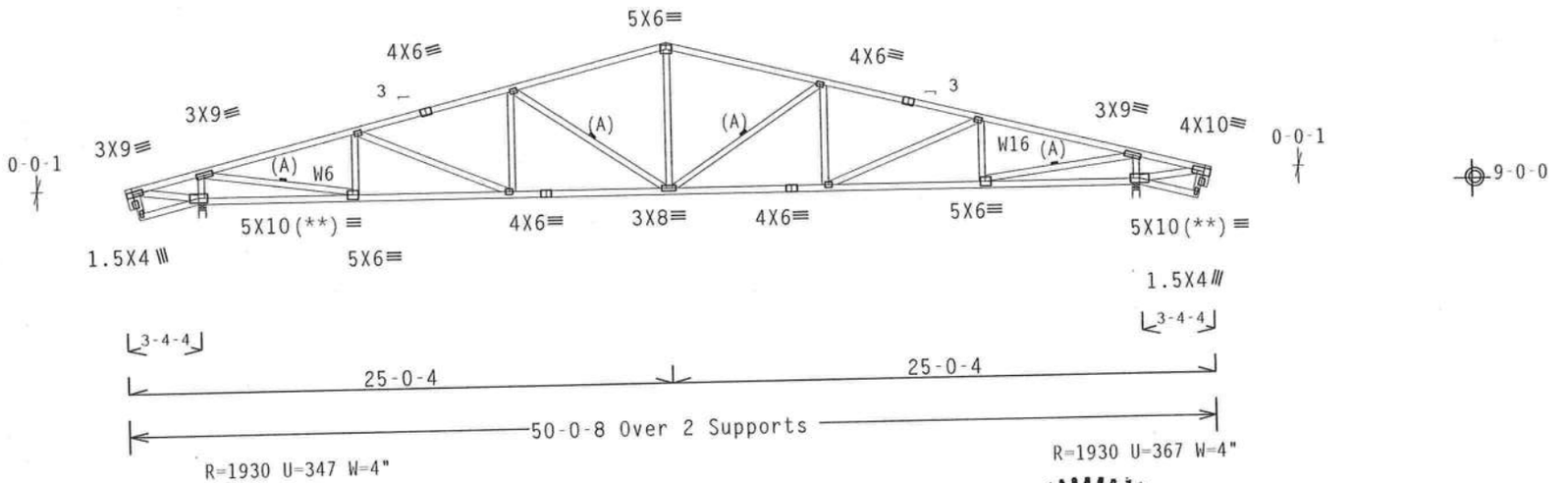
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load.

110 mph wind, 12.91 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

(**) Plate relocated as shown.




Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave TPI  Design Crit: TPI-1995(STD)/FBC  7.04.0005.15  FL/-/3/-/-/R/-  Scale =.125"/Ft.



ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

ARTHUR R. FISHER CERTIFICATE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 25 '05

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 66515 |
| TC DL | 10.0 PSF | DATE | 05/24/05 |
| BC DL | 10.0 PSF | DRW | HCUSR487 05144001 |
| BC LL | 0.0 PSF | HC-ENG | JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- | 17112 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1SNC487_Z01 |

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - AS)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W14 2x4 SP #2 Dense:

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load.

(**) Plate relocated as shown.

110 mph wind, 12.91 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

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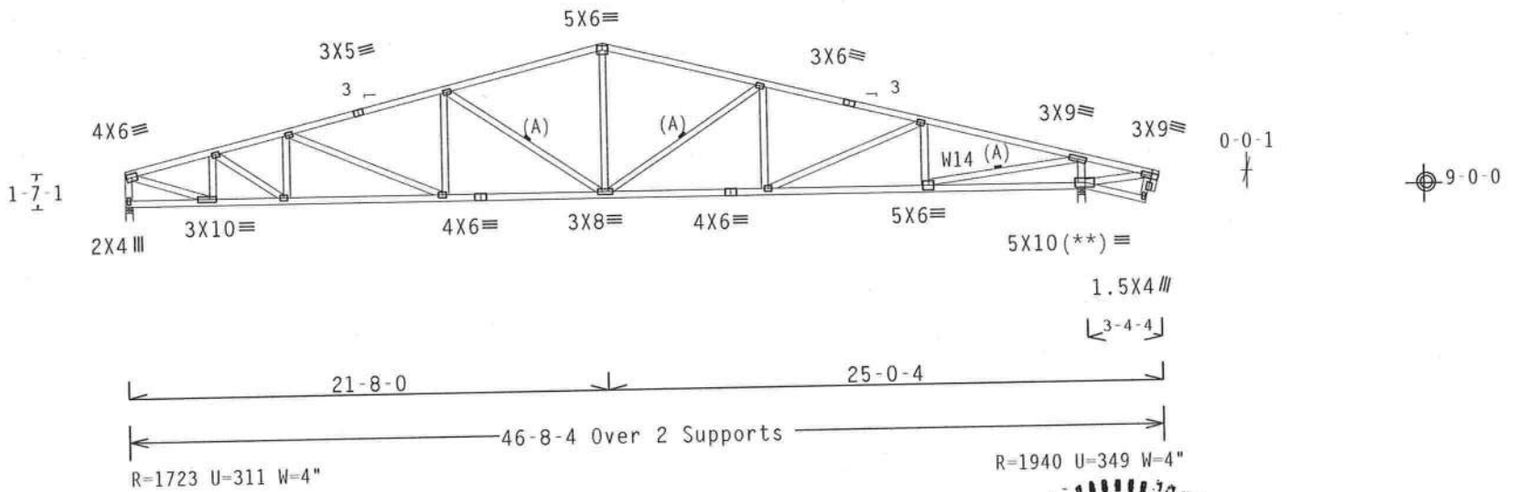



Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC 7.04.08

FL/-/3/-/-/R/-

Scale =.125"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER CERTIFICATE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 25 '05

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 66516 |
| TC DL | 10.0 PSF | DATE | 05/24/05 |
| BC DL | 10.0 PSF | DRW | HCUSR487 05144002 |
| BC LL | 0.0 PSF | HC-ENG | JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- | 17128 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1SNC487_Z01 |

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - AGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

| | | | | | |
|---|---|---|---|---|---|
| TC - From | 81 PLF | at -3.07 | to | 81 PLF | at 46.12 |
| TC - From | 60 PLF | at 46.12 | to | 60 PLF | at 46.40 |
| BC - From | 4 PLF | at -3.07 | to | 4 PLF | at 0.00 |
| BC - From | 20 PLF | at 0.00 | to | 20 PLF | at 43.33 |
| BC - From | 4 PLF | at 43.33 | to | 20 PLF | at 43.33 |
| BC - From | 4 PLF | at 43.33 | to | 4 PLF | at 46.40 |

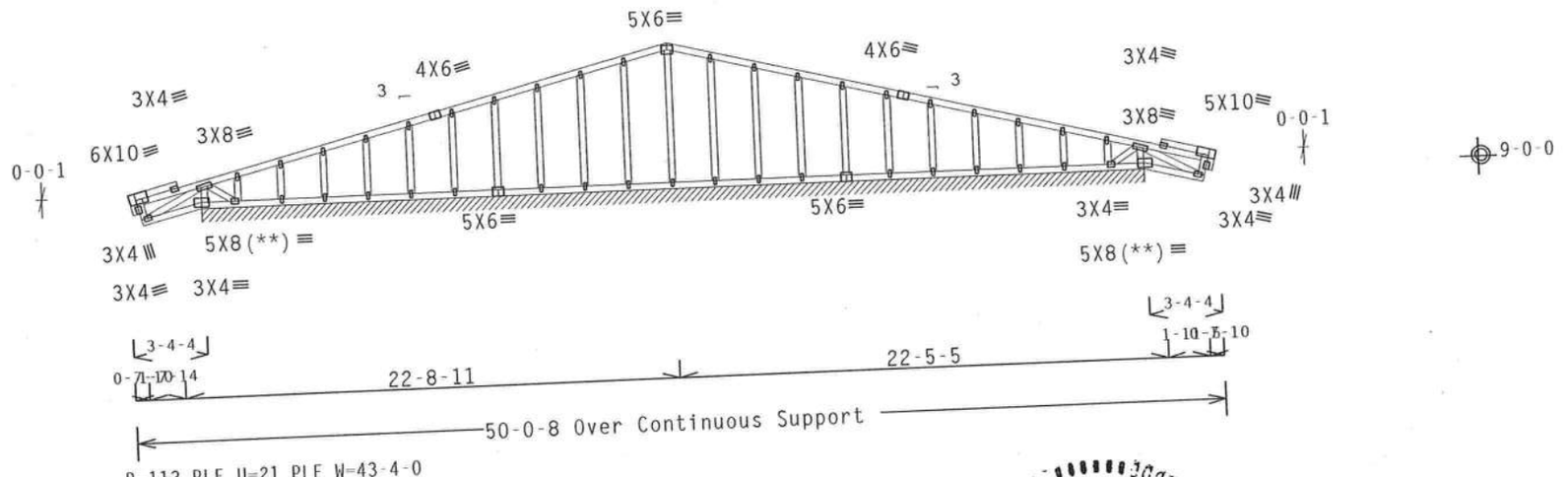
(**) Plate relocated as shown.

110 mph wind, 12.66 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

See DWGS A11015EN1103 & GBLLETIN1103 for more requirements.




Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC 7.04.0005 TYP: 1 FL/-/3/-/-/R/-

Scale =.125"/Ft.

**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (M,H/S/K) ASTM A653 GRADE 40/60 (M, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
May 25 '05

| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 10.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 40.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 66517 |
| DATE | 05/24/05 |
| DRW | HCUSR487 05144003 |
| HC-ENG | JB/AF |
| SEQN- | 17159 |
| JREF- | 1SNC487_Z01 |

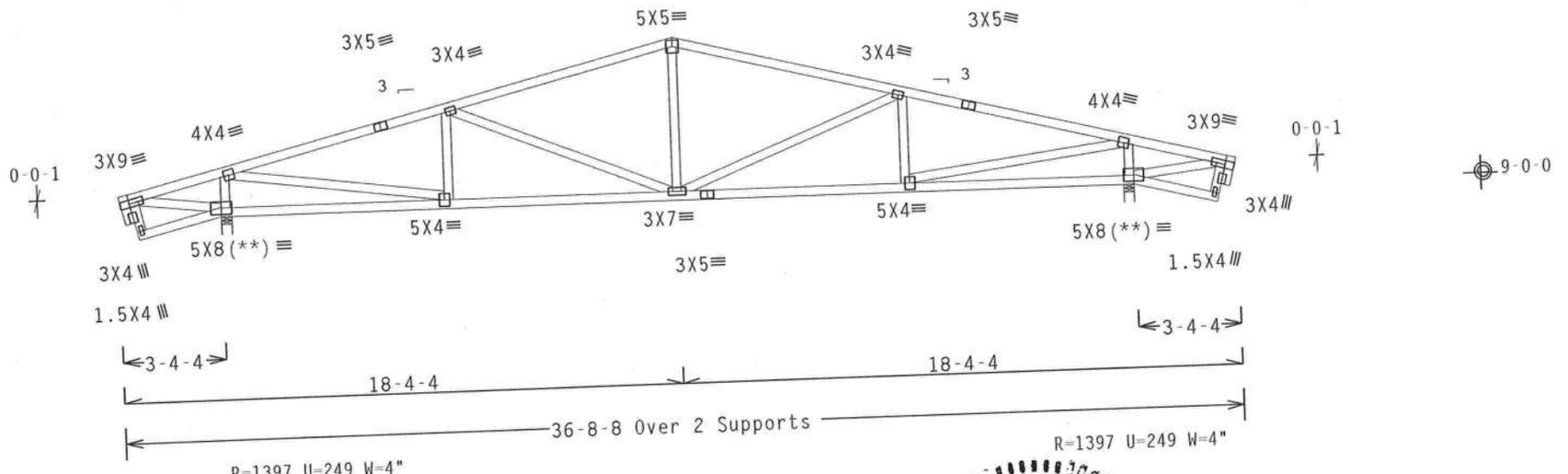
(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load.

(**) Plate relocated as shown.

110 mph wind, 12.08 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.




PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.04

FL/-/3/-/-/R/-

Scale =.1875"/Ft.

| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 10.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 40.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 66518 |
| DATE | 05/24/05 |
| DRW | HCUSR487 05144004 |
| HC-ENG | JB/AF |
| SEQN- | 17168 |
| | |
| JREF- | 1SNC487_Z01 |

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 25 '05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - BS)

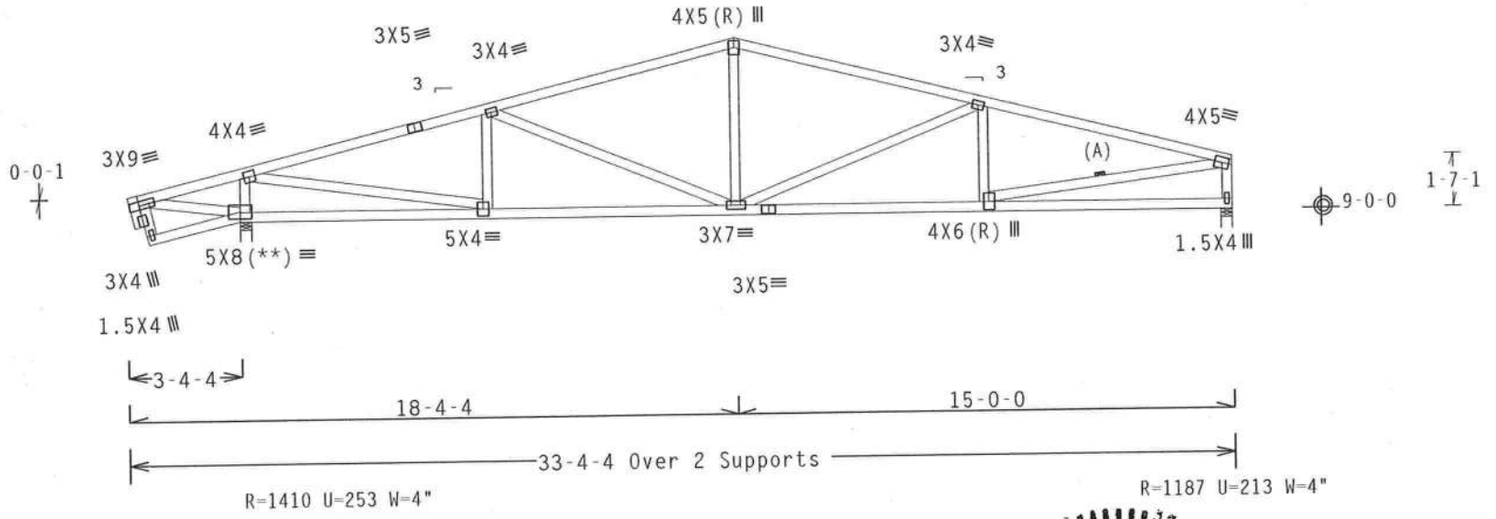
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load.

(**) Plate relocated as shown.

110 mph wind, 12.08 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.




PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.04.0808 Ver:11

FL/-/3/-/-/R/-

Scale =.1875"/Ft.

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 66519 |
| TC DL | 10.0 PSF | DATE | 05/24/05 |
| BC DL | 10.0 PSF | DRW | HCUSR487 05144005 |
| BC LL | 0.0 PSF | HC-ENG | JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- | 17180 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1SNC487_Z01 |

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

May 25 '05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - CGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

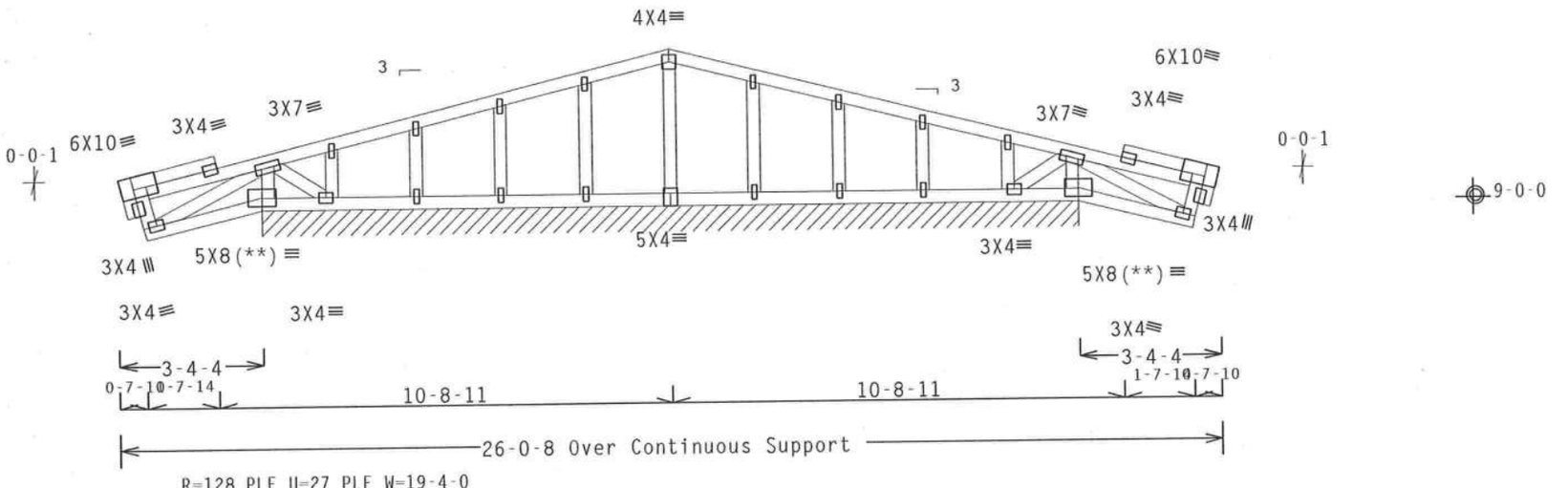
| | | | | | |
|---|---|---|---|---|---|
| TC - From | 81 PLF | at -3.07 | to | 81 PLF | at 22.40 |
| BC - From | 4 PLF | at -3.07 | to | 4 PLF | at 0.00 |
| BC - From | 20 PLF | at 0.00 | to | 20 PLF | at 19.33 |
| BC - From | 4 PLF | at 19.33 | to | 20 PLF | at 19.33 |
| BC - From | 4 PLF | at 19.33 | to | 4 PLF | at 22.40 |

See DWGS A11015EN1103 & GBLLETIN1103 for more requirements.

110 mph wind, 11.16 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

(**) Plate relocated as shown.




R=128 PLF U=27 PLF W=19-4-0

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.04.00.05 FLORIDA

FL/-/3/-/-/R/-

Scale =.25"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

May 25 '05

| | | |
|---|---|---|
| TC LL | 20.0 PSF | REF R487-- 66522 |
| TC DL | 10.0 PSF | DATE 05/24/05 |
| BC DL | 10.0 PSF | DRW HCUSR487 05144013 |
| BC LL | 0.0 PSF | HC-ENG JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- 17222 |
| DUR.FAC. | 1.25 | |
| SPACING | 24.0" | JREF- 1SNC487_Z01 |

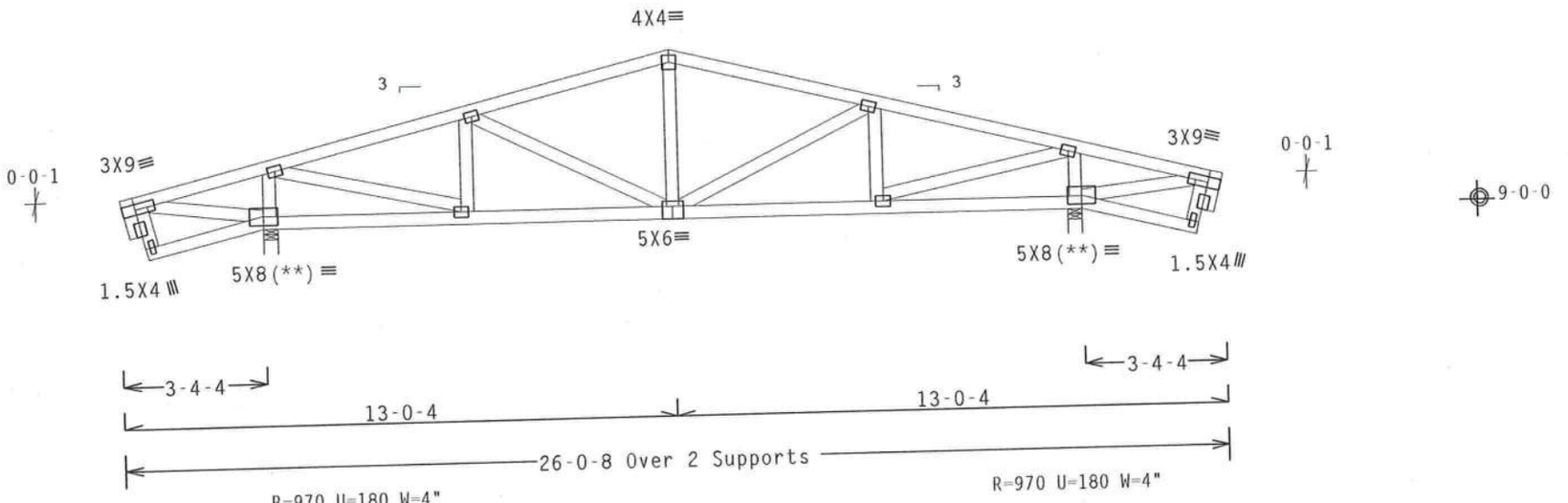
(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - C)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load.

(**) Plate relocated as shown.

110 mph wind, 11.41 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.




Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.04.0825.71 ... 2 FL/-/3/-/-/R/-

Scale =.25"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 25 '05

| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 10.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 40.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 66521 |
| DATE | 05/24/05 |
| DRW | HCUSR487 05144007 |
| HC-ENG | JB/AF |
| SEQN- | 17206 |
| JREF- | 1SNC487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - BGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W16 2x4 SP #2 Dense:

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

| | | | | | |
|---|---|---|---|---|---|
| TC - From | 60 PLF | at -3.07 | to | 60 PLF | at -3.00 |
| TC - From | 81 PLF | at -3.00 | to | 81 PLF | at 20.33 |
| TC - From | 60 PLF | at 20.33 | to | 60 PLF | at 33.07 |
| BC - From | 4 PLF | at -3.07 | to | 4 PLF | at 0.00 |
| BC - From | 20 PLF | at 0.00 | to | 20 PLF | at 30.00 |
| BC - From | 4 PLF | at 30.00 | to | 20 PLF | at 30.00 |
| BC - From | 4 PLF | at 30.00 | to | 4 PLF | at 33.07 |

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

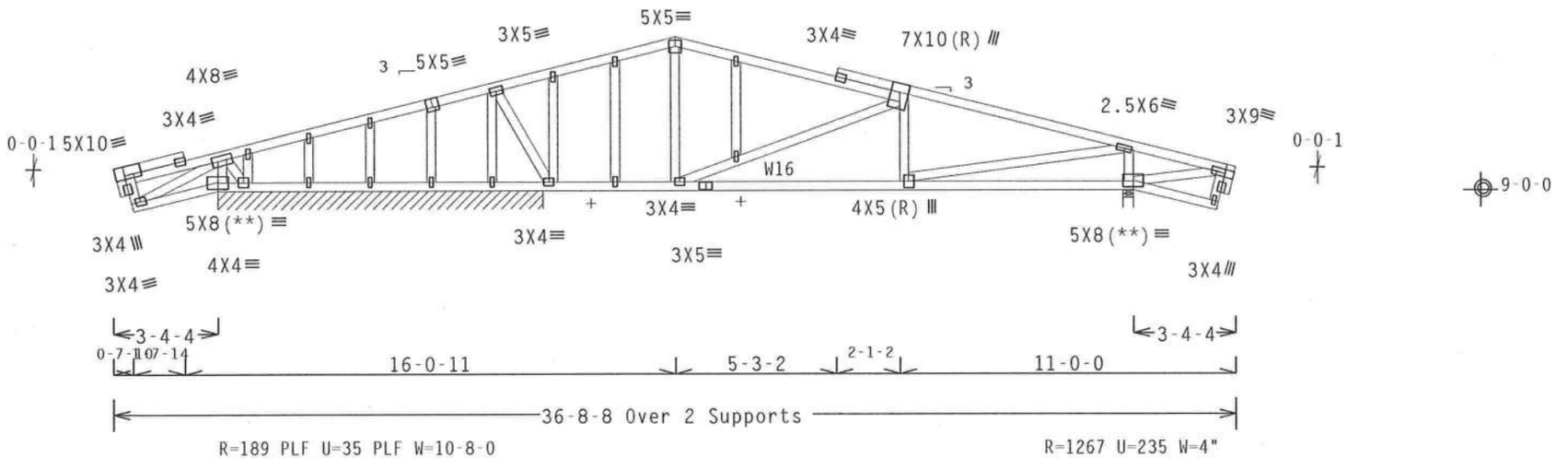
See DWGS A11015EN1103 & GBLLETIN1103 for more requirements.

110 mph wind, 11.82 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

(**) Plate relocated as shown.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.




Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave TPI | Design Crit: TPI-1995(STD)/FBC | 7.04.0005 | TY: 1 | FL/-/3/-/-/R/- | Scale =.1875"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 25 '05

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 66520 |
| TC DL | 10.0 PSF | DATE | 05/24/05 |
| BC DL | 10.0 PSF | DRW | HCUSR487 05144006 |
| BC LL | 0.0 PSF | HC-ENG | JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- | 17197 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1SNC487 701 |

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - M)

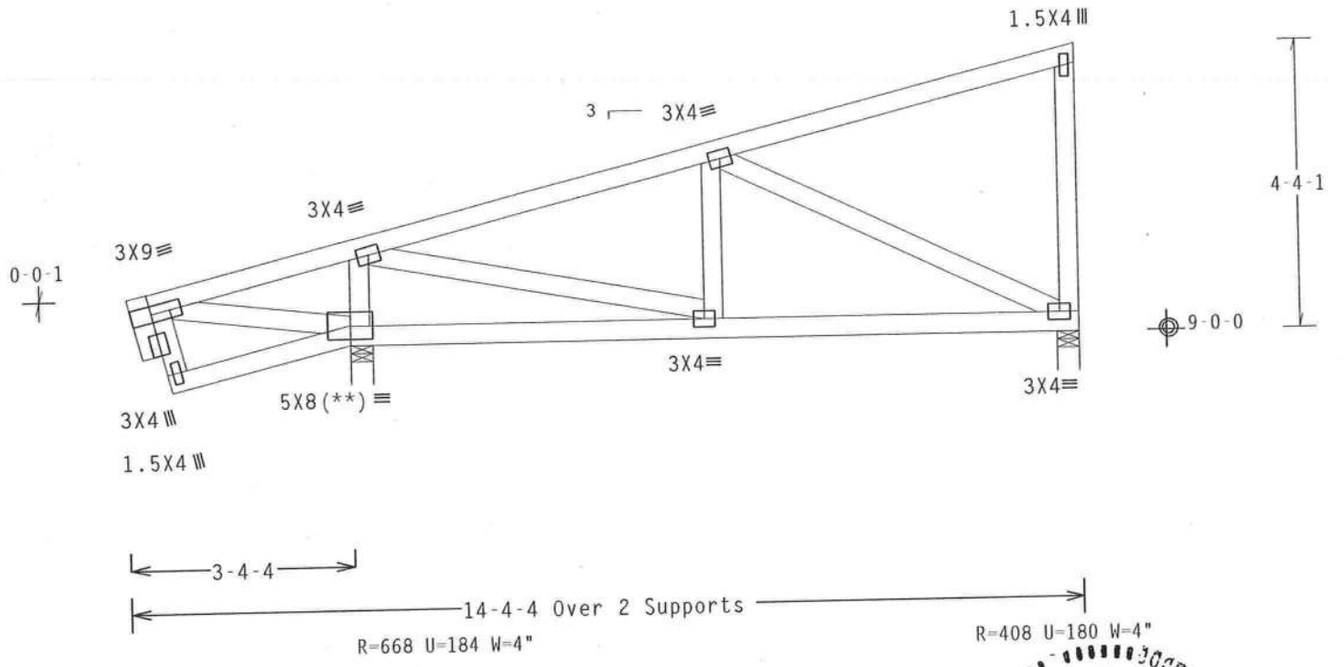
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load.

(**) Plate relocated as shown.

110 mph wind, 11.58 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.




PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.04.0505 QTY:11 FL/-/3/-/-/R/-

Scale =.375"/Ft.

ALPINE

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1950 Marley Drive
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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 25 '05

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 66523 |
| TC DL | 10.0 PSF | DATE | 05/24/05 |
| BC DL | 10.0 PSF | DRW | HCUSR487 05144008 |
| BC LL | 0.0 PSF | HC-ENG | JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- | 17228 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1SNC487_Z01 |

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - MGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 81 PLF at -3.07 to 81 PLF at 11.00
BC - From 4 PLF at -3.07 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 11.00

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

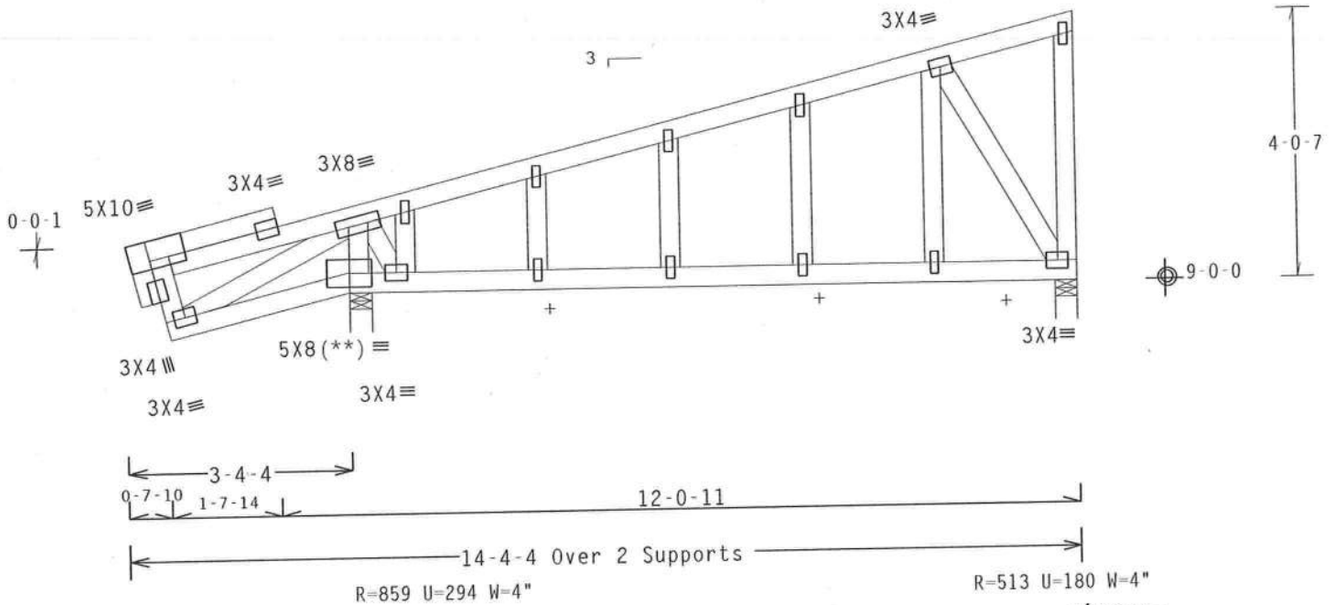
See DWGS A11015EN1103 & GBLLETIN1103 for more requirements.

110 mph wind, 11.32 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.




Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave TPI Design Crit: TPI-1995(STD)/FBC 7.04.0805 T=11 FL/-/3/-/-/R/- Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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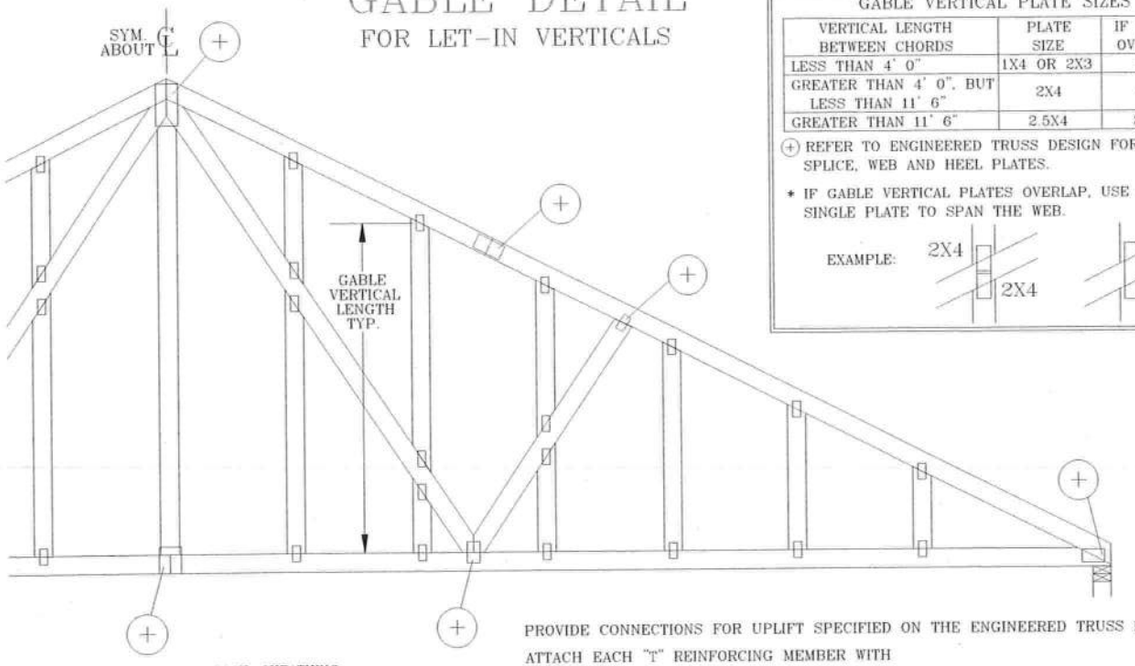
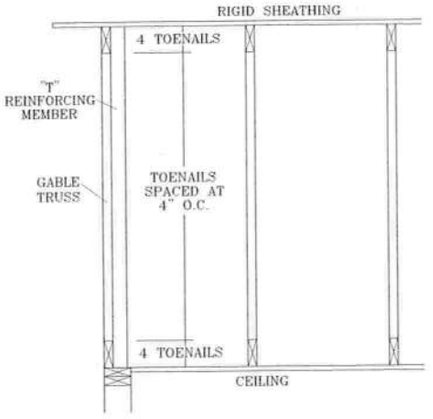
ARTHUR R. FISHER
CERTIFICATE
NO. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 25 '05

| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 10.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 40.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 66524 |
| DATE | 05/24/05 |
| DRW | HCUSR487 05144009 |
| HC-ENG | JB/AF |
| SEQN- | 17250 |
| | |
| JREF- | 1SNC487_Z01 |

# GABLE DETAIL
## FOR LET-IN VERTICALS




| GABLE VERTICAL PLATE SIZES | | |
|---|---|---|
| VERTICAL LENGTH BETWEEN CHORDS | PLATE SIZE | IF PLATES OVERLAP* |
| LESS THAN 4' 0" | 1X4 OR 2X3 | 2X8 |
| GREATER THAN 4' 0", BUT LESS THAN 11' 6" | 2X4 | 2X8 |
| GREATER THAN 11' 6" | 2.5X4 | 2.5X8 |

(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

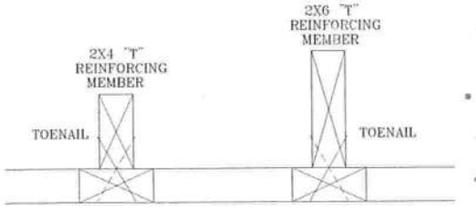
EXAMPLE: 2X4 2X4 2X8

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
- HAND DRIVEN NAILS:
  10d COMMON TOENAILS AT 4" O.C. PLUS (4) 16d COMMON TOENAILS IN TOP AND BOTTOM CHORD.
- GUN DRIVEN NAILS - 0.131" X 3":
  TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
- ASCE 7-93 GABLE DETAIL DRAWINGS
  A11015EN1103, A10015EN1103, A09015EN1103, A08015EN1103, A07015EN1103
  A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103
- ASCE 7-98 GABLE DETAIL DRAWINGS
  A13015EC1103, A12015EC1103, A11015EC1103, A10015EC1103, A08515EC1103
  A13030EC1103, A12030EC1103, A11030EC1103, A10030EC1103, A08530EC1103
- SBCCI GABLE DETAIL DRAWINGS
  S11015EN1103, S10015EN1103, S09015EN1103, S08015EN1103, S07015EN1103
  S11030EN1103, S10030EN1103, S09030EN1103, S08030EN1103, S07030EN1103

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.




TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

### WEB LENGTH INCREASE W/ "T" BRACE

| WIND SPEED AND MRH | "T" REINF. MBR. SIZE | SBCCI | ASCE |
|---|---|---|---|
| 110 MPH | 2x4 | 10 % | 10 % |
| 15 FT | 2x6 | 40 % | 50 % |
| 110 MPH | 2x4 | 10 % | 10 % |
| 30 FT | 2x6 | 50 % | 50 % |
| 100 MPH | 2x4 | 10 % | 10 % |
| 15 FT | 2x6 | 30 % | 50 % |
| 100 MPH | 2x4 | 10 % | 10 % |
| 30 FT | 2x6 | 40 % | 40 % |
| 90 MPH | 2x4 | 20 % | 10 % |
| 15 FT | 2x6 | 20 % | 40 % |
| 90 MPH | 2x4 | 10 % | 10 % |
| 30 FT | 2x6 | 30 % | 50 % |
| 80 MPH | 2x4 | 10 % | 20 % |
| 15 FT | 2x6 | 10 % | 30 % |
| 80 MPH | 2x4 | 20 % | 10 % |
| 30 FT | 2x6 | 20 % | 40 % |
| 70 MPH | 2x4 | 0 % | 20 % |
| 15 FT | 2x6 | 0 % | 20 % |
| 70 MPH | 2x4 | 10 % | 20 % |
| 30 FT | 2x6 | 10 % | 30 % |

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"

ALPINE
ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER CERTIFICATE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER 05/21/2005

REPLACES DRAWINGS GAB98117 876,719 & HC26294035

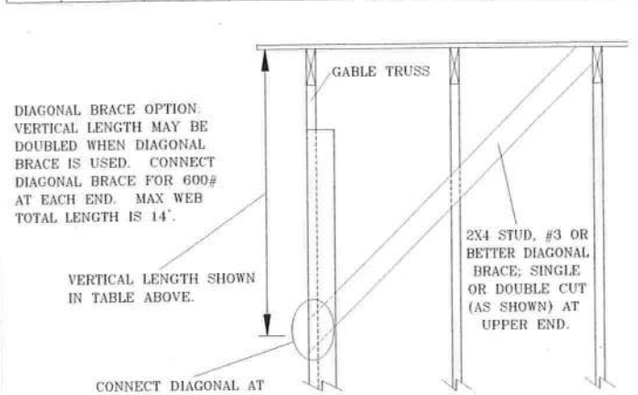
MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

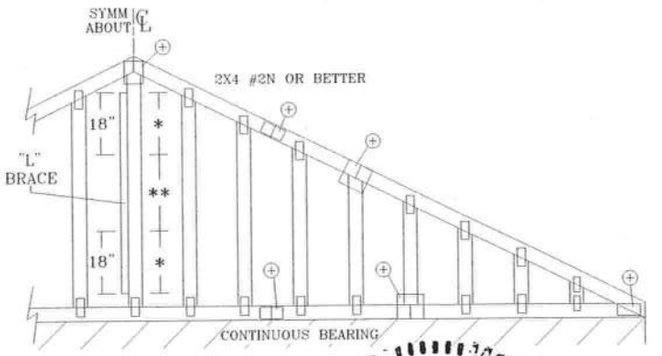
| | |
|---|---|
| REF | LET-IN VERT |
| DATE | 01/16/04 |
| DRWG | GBLLETIN1103 |
| -ENG | DLJ/KAR |

# ASCE 7-98: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

| 2X4 GABLE VERTICAL SPACING | SPECIES | BRACE GRADE | NO BRACES | (1) 1X4 "L" BRACE * | | (1) 2X4 "L" BRACE * | | (2) 2X4 "L" BRACE ** | | (1) 2X6 "L" BRACE * | | (2) 2X6 "L" BRACE ** | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | GROUP A | GROUP B | GROUP A | GROUP B | GROUP A | GROUP B | GROUP A | GROUP B | GROUP A | GROUP B |
| 24" O.C. | SPF HF | #1 / #2 | 3' 10" | 6' 8" | 6' 10" | 7' 11" | 8' 1" | 9' 5" | 9' 8" | 12' 5" | 12' 9" | 14' 0" | 14' 0" |
| | | #3 | 3' 9" | 6' 0" | 6' 0" | 7' 11" | 7' 11" | 9' 5" | 9' 5" | 12' 4" | 12' 4" | 14' 0" | 14' 0" |
| | | STUD | 3' 9" | 6' 0" | 6' 0" | 7' 11" | 7' 11" | 9' 5" | 9' 5" | 12' 3" | 12' 3" | 14' 0" | 14' 0" |
| | | STANDARD | 3' 9" | 5' 2" | 5' 2" | 6' 9" | 6' 9" | 9' 1" | 9' 1" | 10' 7" | 10' 7" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 4' 3" | 6' 8" | 7' 2" | 7' 11" | 8' 6" | 9' 5" | 10' 2" | 12' 5" | 13' 5" | 14' 0" | 14' 0" |
| | | #2 | 4' 2" | 6' 8" | 7' 2" | 7' 11" | 8' 6" | 9' 5" | 10' 2" | 12' 5" | 13' 5" | 14' 0" | 14' 0" |
| | | #3 | 4' 0" | 6' 2" | 6' 2" | 7' 11" | 8' 1" | 9' 5" | 9' 11" | 12' 5" | 12' 8" | 14' 0" | 14' 0" |
| | | STUD | 4' 0" | 6' 1" | 6' 1" | 7' 11" | 8' 0" | 9' 5" | 9' 11" | 12' 5" | 12' 6" | 14' 0" | 14' 0" |
| | | STANDARD | 3' 10" | 5' 3" | 5' 3" | 6' 11" | 6' 11" | 9' 4" | 9' 4" | 10' 10" | 10' 10" | 14' 0" | 14' 0" |
| 16" O.C. | SPF HF | #1 / #2 | 4' 5" | 7' 8" | 7' 10" | 9' 1" | 9' 4" | 10' 10" | 11' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 4" | 7' 4" | 7' 4" | 9' 1" | 9' 1" | 10' 10" | 10' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STUD | 4' 4" | 7' 4" | 7' 4" | 9' 1" | 9' 1" | 10' 10" | 10' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STANDARD | 4' 4" | 6' 4" | 6' 4" | 8' 4" | 8' 4" | 10' 10" | 10' 10" | 12' 11" | 12' 11" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 4' 10" | 7' 8" | 8' 3" | 9' 1" | 9' 9" | 10' 10" | 11' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 4' 9" | 7' 8" | 8' 3" | 9' 1" | 9' 9" | 10' 10" | 11' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 6" | 7' 7" | 7' 7" | 9' 1" | 9' 6" | 10' 10" | 11' 4" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STUD | 4' 6" | 7' 6" | 7' 6" | 9' 1" | 9' 6" | 10' 10" | 11' 4" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STANDARD | 4' 5" | 6' 5" | 6' 5" | 8' 6" | 8' 6" | 10' 10" | 11' 1" | 13' 3" | 13' 3" | 14' 0" | 14' 0" |
| 12" O.C. | SPF HF | #1 / #2 | 4' 11" | 8' 5" | 8' 8" | 10' 0" | 10' 3" | 11' 11" | 12' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 9" | 8' 5" | 8' 5" | 10' 0" | 10' 0" | 11' 11" | 11' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STUD | 4' 9" | 8' 5" | 8' 5" | 10' 0" | 10' 0" | 11' 11" | 11' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STANDARD | 4' 9" | 7' 3" | 7' 3" | 9' 7" | 9' 7" | 11' 11" | 11' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 5' 4" | 8' 5" | 9' 1" | 10' 0" | 10' 9" | 11' 11" | 12' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 5' 3" | 8' 5" | 9' 1" | 10' 0" | 10' 9" | 11' 11" | 12' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 5' 0" | 8' 5" | 8' 5" | 10' 0" | 10' 6" | 11' 11" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STUD | 5' 0" | 8' 5" | 8' 7" | 10' 0" | 10' 6" | 11' 11" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | STANDARD | 4' 11" | 7' 5" | 7' 5" | 9' 10" | 9' 10" | 11' 11" | 12' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |




REFER TO CHART ABOVE FOR [illegible] VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

| SPRUCE-PINE-FIR | |
|---|---|
| #1 / #2 | STANDARD |
| #3 | STUD |

| HEM-FIR | |
|---|---|
| #2 | STUD |
| #3 | STANDARD |

| DOUGLAS FIR-LARCH |
|---|
| #3 |
| STUD |
| STANDARD |

| SOUTHERN PINE |
|---|
| #3 |
| STUD |
| STANDARD |

GROUP B:

| HEM-FIR |
|---|
| #1 & BTR |
| #1 |

| SOUTHERN PINE |
|---|
| #1 |
| #2 |

| DOUGLAS FIR-LARCH |
|---|
| #1 |
| #2 |

## GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

| VERTICAL LENGTH | NO SPLICE |
|---|---|
| LESS THAN 4' 0" | 1X4 OR 2X3 |
| GREATER THAN 4' 0", BUT LESS THAN 11' 6" | 2X4 |
| GREATER THAN 11' 6" | 2.5X4 |

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

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POMPANO BEACH, FLORIDA

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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
05/23/2005
PROFESSIONAL ENGINEER

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-98-GAB11015
DATE 11/26/03
DRWG A11015EC1103
-ENG

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

## NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

| WEB MEMBER SIZE | SPECIFIED CLB BRACING | ALTERNATIVE BRACING T OR L-BRACE | ALTERNATIVE BRACING SCAB BRACE |
|---|---|---|---|
| 2X3 OR 2X4 | 1 ROW | 2X4 | 1-2X4 |
| 2X3 OR 2X4 | 2 ROWS | 2X6 | 2-2X4 |
| 2X6 | 1 ROW | 2X4 | 1-2X6 |
| 2X6 | 2 ROWS | 2X6 | 2-2X4(*) |
| 2X8 | 1 ROW | 2X6 | 1-2X8 |
| 2X8 | 2 ROWS | 2X6 | 2-2X6(*) |

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

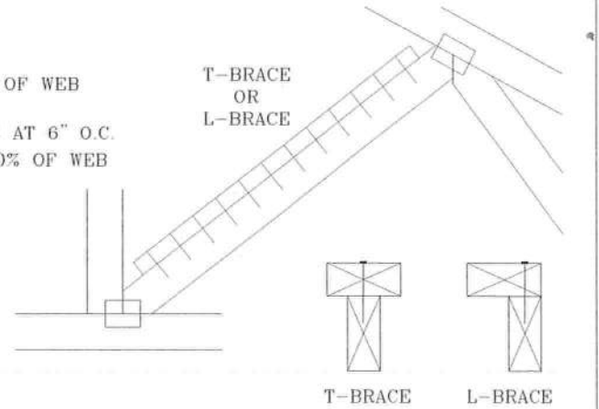
(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

## T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE

ATTACH WITH 16d NAILS AT 6" O.C.

BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH

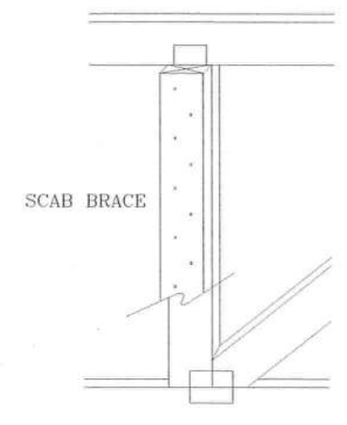



## SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.

NO MORE THAN (1) SCAB PER FACE.

ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH




THIS DRAWING REPLACES DRAWING 579,640

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
05/28/2003
PROFESSIONAL ENGINEER

| | | |
|---|---|---|
| TC LL | PSF | REF CLB SUBST. |
| TC DL | PSF | DATE 11/26/03 |
| BC DL | PSF | DRWG BRCLBSUB1103 |
| BC LL | PSF | -ENG MLH/KAR |
| TOT. LD. | PSF | |
| DUR. FAC. | | |
| SPACING | | |

(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - TGE)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

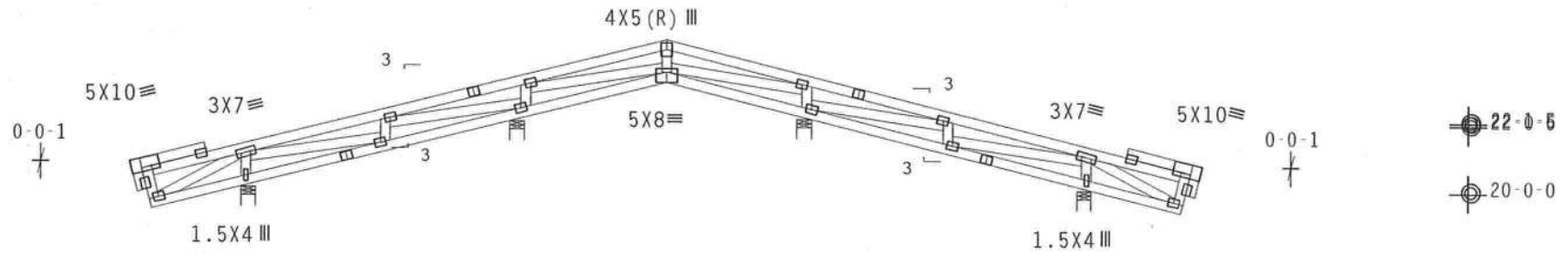
SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

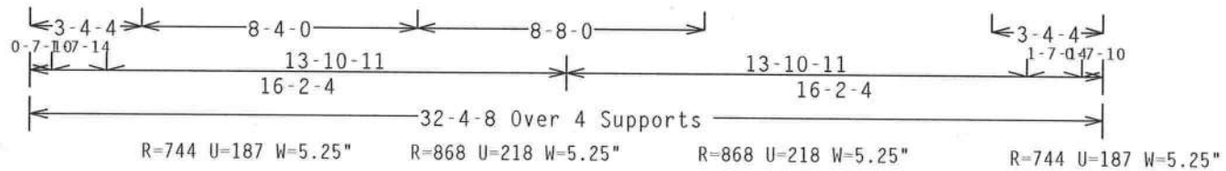
| | | | | | | |
|---|---|---|---|---|---|---|
| TC - From | 81 PLF | at | -3.07 | to | 81 PLF | at 28.74 |
| BC - From | 4 PLF | at | -3.07 | to | 4 PLF | at -2.77 |
| BC - From | 24 PLF | at | -2.77 | to | 24 PLF | at 0.00 |
| BC - From | 20 PLF | at | 0.00 | to | 20 PLF | at 25.67 |
| BC - From | 24 PLF | at | 25.67 | to | 24 PLF | at 28.43 |
| BC - From | 4 PLF | at | 28.43 | to | 4 PLF | at 28.74 |

110 mph wind, 22.55 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

Shim all supports to solid bearing.




Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave TPI  Design Crit: TPI-1995(STD)/FBC  7.04-08  TY:2  FL/-/3/-/-/R/-  Scale =.1875"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
CERTIFICATE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
May 25

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 66527 |
| TC DL | 10.0 PSF | DATE | 05/24/05 |
| BC DL | 10.0 PSF | DRW | HCUSR487 05144012 |
| BC LL | 0.0 PSF | HC-ENG | JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- | 17305 |
| DUR.FAC. | 1.25 | | |

(5-214-ISAAC CONST/ED & DIANE WHITE ROOF ASGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 81 PLF at 0.00 to 81 PLF at 16.40
BC - From 20 PLF at 0.00 to 20 PLF at 13.33
BC - From 4 PLF at 13.33 to 4 PLF at 16.40

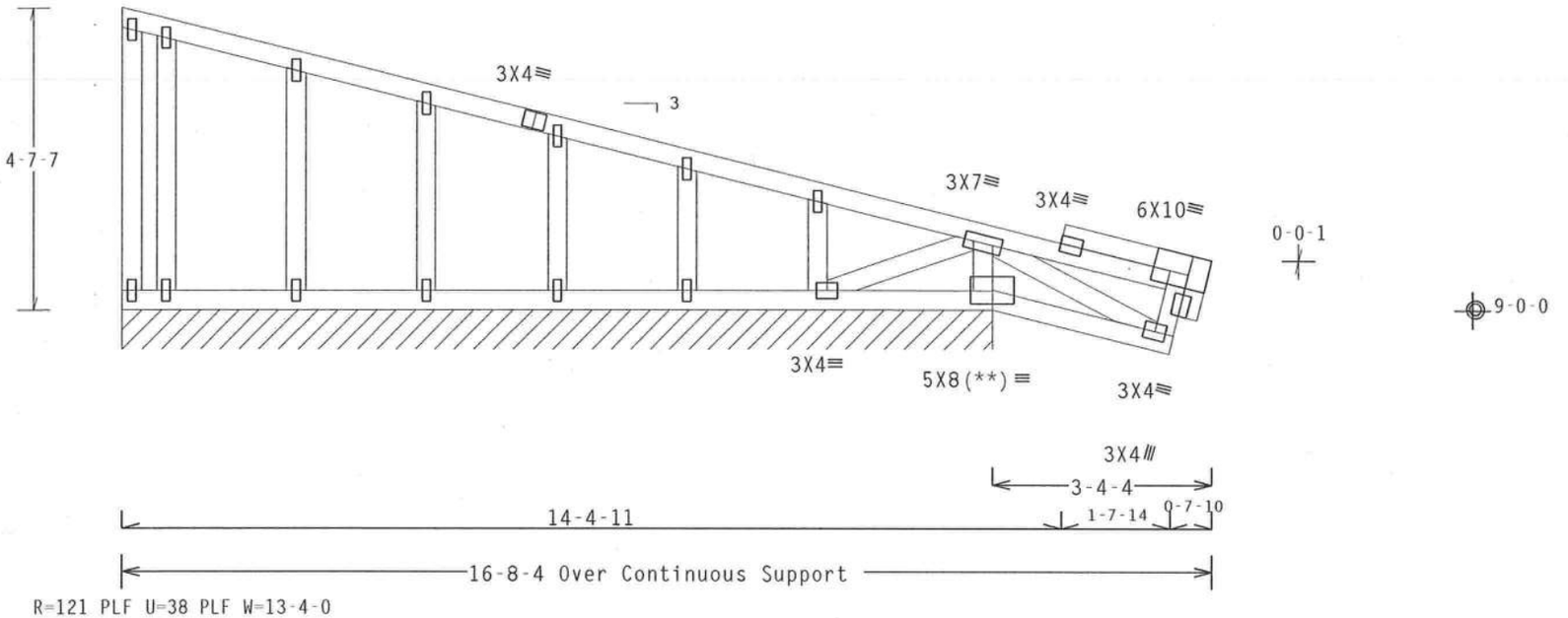
Left end vertical not exposed to wind pressure.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 11.61 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS A11015EC1103 & GBLLETIN0405 for more requirements.

Deflection meets L/360 live and L/240 total load.




R=121 PLF U=38 PLF W=13-4-0

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave TPI Design Crit: TPI-1995(STD)/FBC 7.04.08[illegible] TY:1 FL/-/3/-/-/R/- Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER CERTIFICATE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 25 '05

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 66525 |
| TC DL | 10.0 PSF | DATE | 05/24/05 |
| BC DL | 10.0 PSF | DRW | HCUSR487 05144010 |
| BC LL | 0.0 PSF | HC-ENG | JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- | 17263 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1SNC487_Z01 |

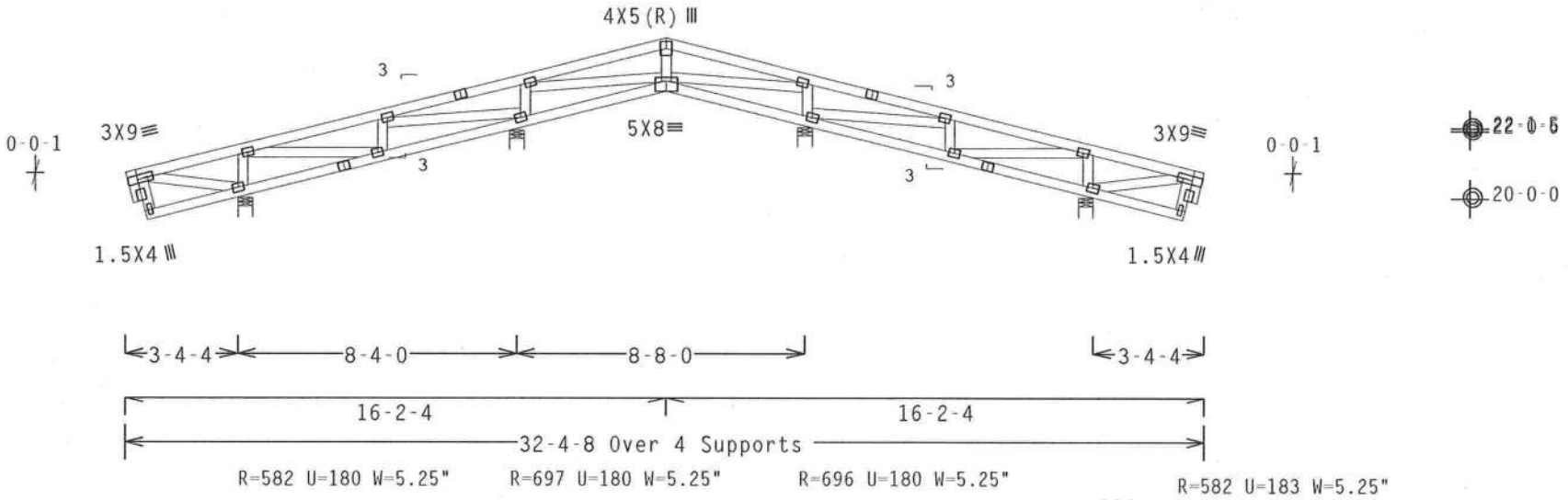
(5-214-ISAAC CONST/ED & DIANE WHITE - ROOF - T)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load.

110 mph wind, 22.81 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Shim all supports to solid bearing.




Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave TPI | Design Crit: TPI-1995(STD)/FBC | 7.04.08 | V126 FL/-/3/-/-/R/- | Scale =.1875"/Ft.

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ARTHUR R. FISHER
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STATE OF FLORIDA
PROFESSIONAL ENGINEER

May 25, '05

| | | |
|---|---|---|
| TC LL | 20.0 PSF | REF R487-- 66526 |
| TC DL | 10.0 PSF | DATE 05/24/05 |
| BC DL | 10.0 PSF | DRW HCUSR487 05144011 |
| BC LL | 0.0 PSF | HC-ENG JB/AF |
| TOT.LD. | 40.0 PSF | SEQN- 17297 |
| DUR.FAC. | 1.25 | |

Ed + Diane White

# COLUMBIA COUNTY BUILDING DEPARTMENT

## RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001
## ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

**APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL**

**GENERAL REQUIREMENTS:** Two (2) complete sets of plans containing the following:

| Applicant | Plans Examiner | |
|---|---|---|
| ☑ | ☐ | All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans. |
| ☑ | ☐ | Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed. |
| ☐ | ☐ | **Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property. |
| ☐ | ☐ | **Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional |
| ☐ | ☐ | **Elevations including:** a) All sides |
| ☐ | ☐ | b) Roof pitch |
| ☐ | ☐ | c) Overhang dimensions and detail with attic ventilation |
| ☐ | ☐ | d) Location, size and height above roof of chimneys |
| ☐ | ☐ | e) Location and size of skylights |
| ☐ | ☐ | f) Building height |
| ☑ | ☐ 2 | e) Number of stories |

**Floor Plan including:**

a) Rooms labeled and dimensioned

b) Shear walls

c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)

d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth

e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails

f) Must show and identify accessibility requirements (accesssable bathroom)

**Foundation Plan including:**

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing

b) All posts and/or column footing including size and reinforcing

c) Any special support required by soil analysis such as piling

d) Location of any vertical steel

**Roof System:**

a) Truss package including:

1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:

1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

**Wall Sections including:**

a) Masonry wall

1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

**Floor Framing System:**

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

**Plumbing Fixture layout**

**Electrical layout including:**

☐ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☐ ☐ b) Ceiling fans

☐ ☐ c) Smoke detectors

☐ ☐ d) Service panel and sub-panel size and location(s)

☐ ☐ e) Meter location with type of service entrance (overhead or underground)

☐ ☐ f) Appliances and HVAC equipment

**HVAC information**

☐ ☐ a) Manual J sizing equipment or equivalent computation

☐ ☐ b) Exhaust fans in bathroom

☐ ☐ **Energy Calculations** (dimensions shall match plans)

☐ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

**Disclosure Statement for Owner Builders**

**Notice Of Commencement**

**Private Potable Water**

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

XX
Glazed Outswing Unit

COP-WL-JH4162-02

# WOOD-EDGE STEEL DOORS

**APPROVED DOOR STYLES:**

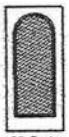
**3/4 GLASS:**

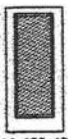

404 Series

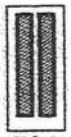

410 Series

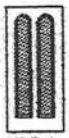

450 Series

**FULL GLASS:**


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

**CERTIFIED TEST REPORTS:**

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

**PRODUCT COMPLIANCE LABELING:**

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

**COMPANY NAME**
CITY, STATE

**To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).**

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Johnson EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

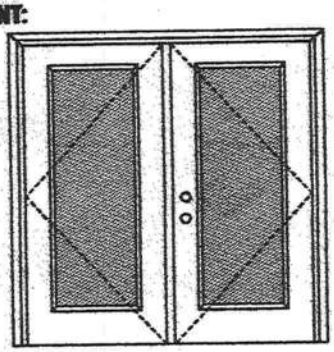
Exclusively from Masonite
Masonite International Corporation

**XX**
Glazed Outswing Unit

COP-WL-JH4162-02

# WOOD-EDGE STEEL DOORS

## APPROVED ARRANGEMENT:



**Note:**
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

**Design Pressure**
+40.5/-40.5
Limited water unless special threshold design is used.

**Large Missile Impact Resistance**
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

## APPROVED DOOR STYLES:

### 1/4 GLASS:


100 Series


133, 135 Series


136 Series


680 Series

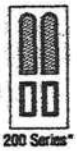

822 Series

### 1/2 GLASS:

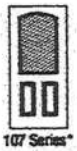

105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

**Johnson EntrySystems**

March 29, 2002
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PREMDOR Collection Premium Quality Doors

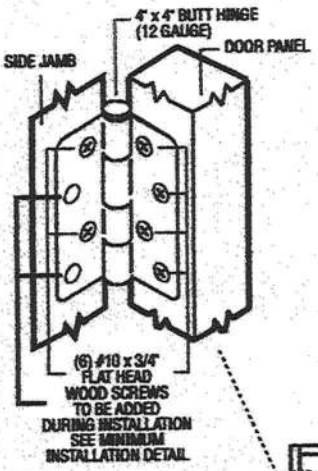
Exclusively from Masonite®
Masonite International Corporation

**XX**
Unit

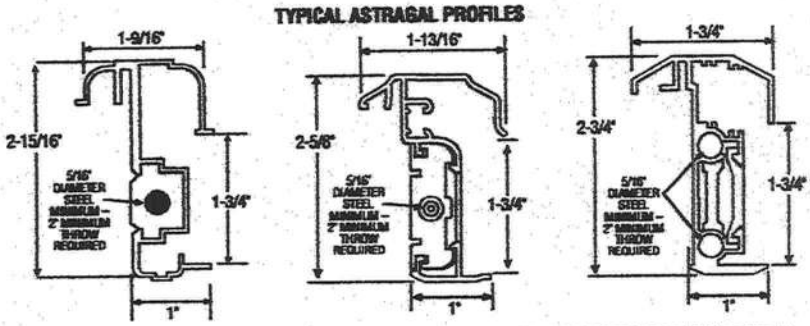
MAD-WL-MA0012-02

# OUTSWING UNITS WITH DOUBLE DOOR

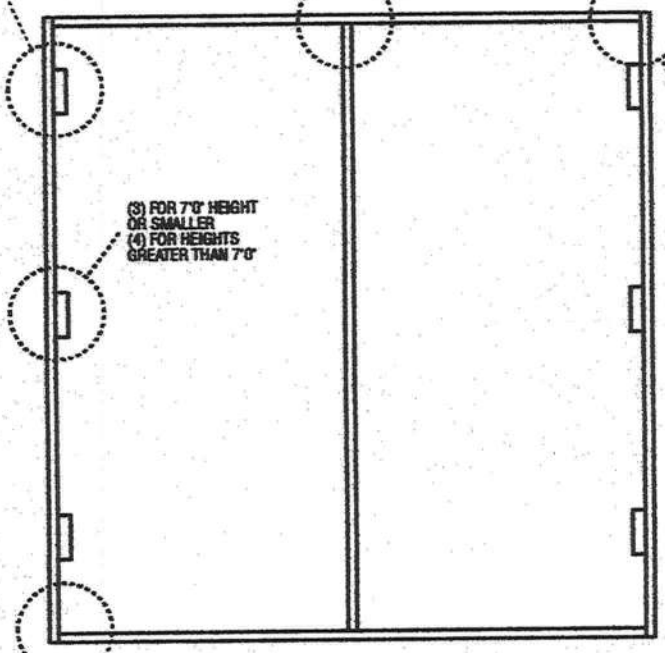
## TYPICAL HINGE ATTACHMENT



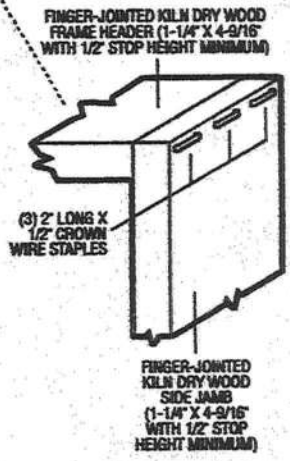

## TYPICAL ASTRAGAL PROFILES



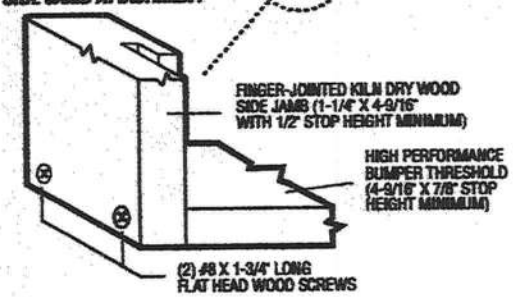

ALUMINUM EXTRUDED ASTRAGAL (0.06" MINIMUM WALL THICKNESS) WITH ADDED REINFORCEMENT INSERTS AT TOP EXTENSION BOLT, BOTTOM EXTENSION BOLT AND CYLINDERICAL/DEADBOLT LATCHING LOCATIONS. ATTACH WITH #8 X 1" PAN HEAD SCREWS – LOCATE 1" FROM EACH END MINIMUM AND 22" O.C. MAXIMUM.




## TYPICAL HEADER & SIDE JAMB ATTACHMENT




## TYPICAL THRESHOLD & SIDE JAMB ATTACHMENT




PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®

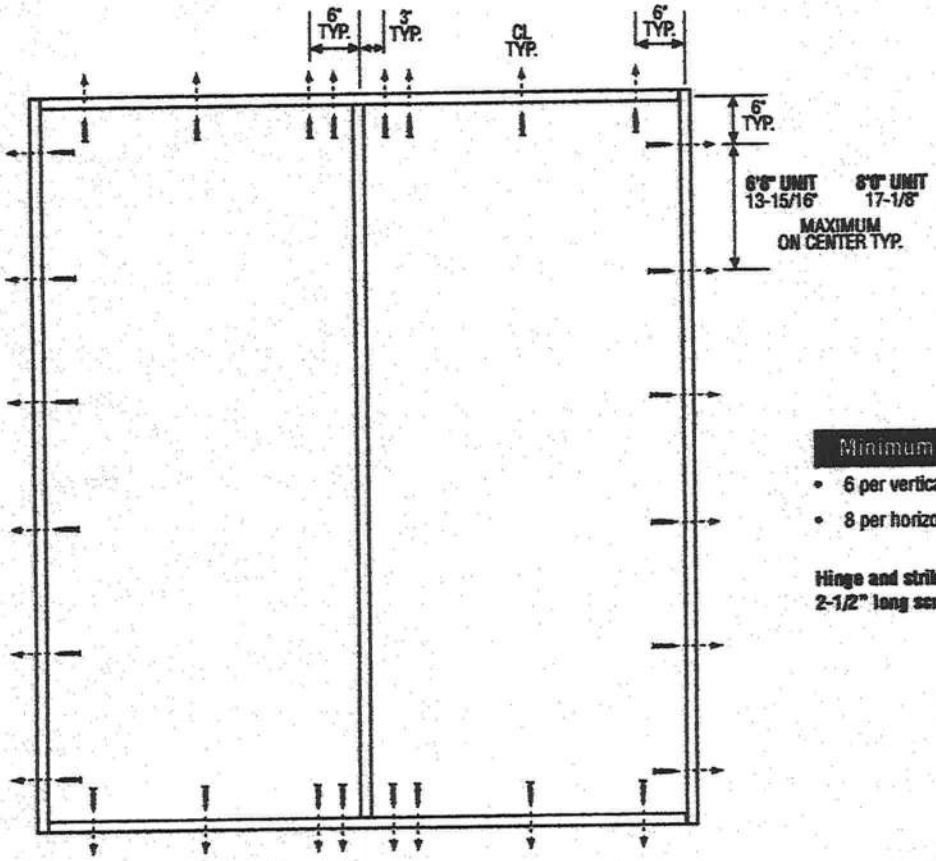
Masonite International Corporation

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

**XX**
**Unit**

MID-WL-MA0002-02

# DOUBLE DOOR




**Minimum Fastener Count**

- 6 per vertical framing member
- 8 per horizontal framing member

**Hinge and strike plates require two 2-1/2" long screws per location.**

## Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

## Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

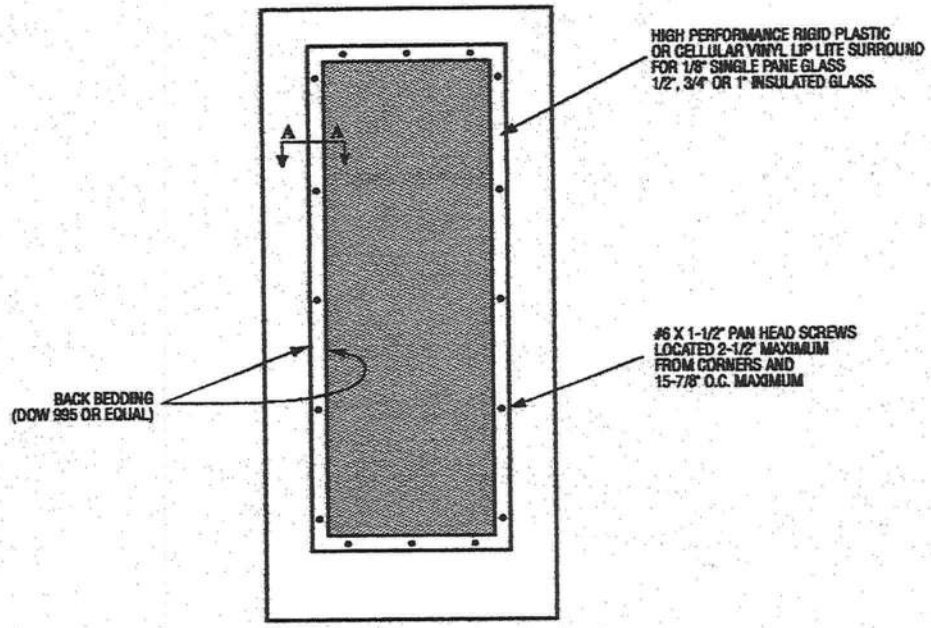
PREMDOR Collection Premium Quality Doors

Exclusively from Masonite

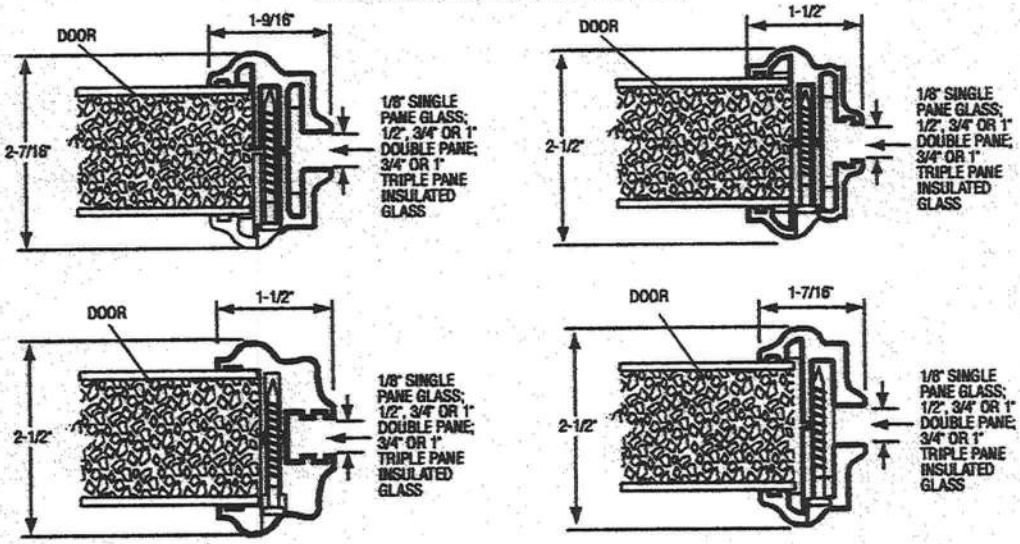
Masonite International Corporation

March 29, 2002

# GLASS INSERT IN DOOR OR SIDELITE PANEL




## SECTION A-A
### TYPICAL RIGID PLASTIC LIP LITE SURROUND



THIS DOCUMENT WAS PREPARED WITHOUT BENEFIT OF TITLE SEARCH.

FOR RECORDER

Inst:2003008855 Date:04/29/2003 Time:09:29
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:981 P:2183

THIS INSTRUMENT WAS PREPARED BY:
Krista L. Waldron, Esquire (PES)
Fisher, Tousey, Leas & Ball, P.A.
818 North A1A, Suite 104
Ponte Vedra Beach, Florida 32082

## SPECIAL WARRANTY DEED

THIS INDENTURE, made this 28 day of APRIL, 2003, between VIRGINIA H. BISHOP, as Trustee of the Virginia H. Bishop Living Trust, whose address is P. O. Box 1298, Lake City, Florida 32056, party of the first part, and EDWARD C. WHITE, JR. and DIANE B. WHITE, husband and wife, whose address is 80 Cliffcreek Trace, Atlanta, Georgia 30350, parties of the second part.

WITNESSETH:

That the said party of the first part, in consideration of love and affection, has granted, bargained, and conveyed to the said parties of the second part, their heirs, successors and assigns forever, her remaining one-half interest in the following described lands, situate, lying and being in Columbia County, Florida, to wit:

Commence at the Southwest Corner of the Southwest ¼ of Northwest ¼ of Section 2, Township 5 South, Range 16 East and run thence North 0 degrees 49 minutes West along the West line of said Section 2, 658.91 feet for Point of Beginning; thence continue North 0 degrees 49 minutes West along the West line of said Section 2, 500 feet; thence run South 65 degrees 59 minutes 30 seconds East, 1000.05 feet to the Northerly line of State Road No. 47; thence run South 40 degrees 53 minutes West along the West line of State Road No. 47 a distance of 474.23 feet; thence run North 65 degrees 59 minutes 30 seconds West, a distance of 652.5 feet more or less to the Point of Beginning, said land lying in the Southwest ¼ of the Northwest ¼ of Section 2, Township 5 South, Range 16 East, Columbia County, Florida, containing 10 acres, more or less.

Real Estate Assessment No: 02-5S-16-03443-002

Subject to covenants, restrictions, easements, mortgages and all other encumbrances of record and taxes assessed subsequent to December 31, 2002; provided, however, this reference will not serve to reimpose any such covenants, restrictions or easements.

Inst:2003008855 Date:04/29/2003 Time:09:29
Doc Stamp-Deed : 0.70
MK DC,P.DeWitt Cason,Columbia County B:981 P:2184

And the said party of the first part does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever claiming by, through or under the said party of the first part, but not otherwise.

IN WITNESS WHEREOF, the undersigned has hereunto set her hand and seal as of the day and year first above written.

Signed and Sealed in Our Presence:

Sign: Doris A. Johnson
Print Name: Doris A. Johnson

Sign: Sheryl Litteral
Print Name: Sheryl Litteral

Virginia H. Bishop
VIRGINIA H. BISHOP, as Trustee of the Virginia H. Bishop Living Trust

STATE OF FLORIDA
COUNTY OF Columbia

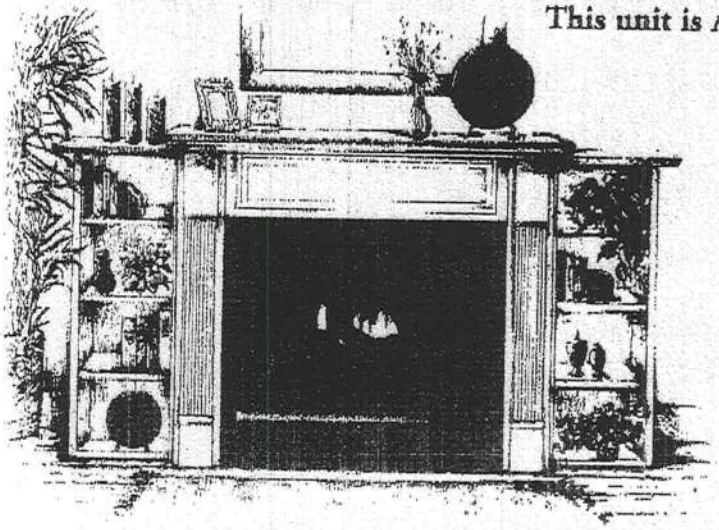
The foregoing instrument was acknowledged before me this 28 day of April, 2003, by VIRGINIA H. BISHOP, as Trustee of the Virginia H. Bishop Living Trust, (notary must check one box): (X) who is personally known to me or ( ) who has produced _____ (State) driver's license No. _____ as identification.

Elaine K. Tolar
Name: ELAINE K. TOLAR
NOTARY PUBLIC, State of FLORIDA
Commission Number: _____

ELAINE K. TOLAR
MY COMMISSION # DD 021574
EXPIRES: October 2, 2005
Bonded Thru Notary Public Underwriters

36293

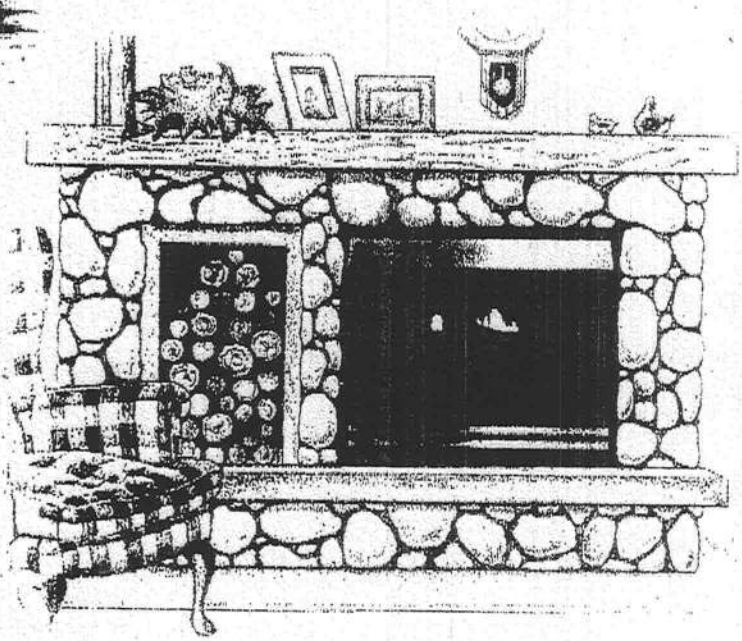
# VENT-FREE



**This unit is A.G.A. certified as a heater with 99% heat efficiency**
**No chimney or flue system required**
**Wide selection of factory installed options offered**

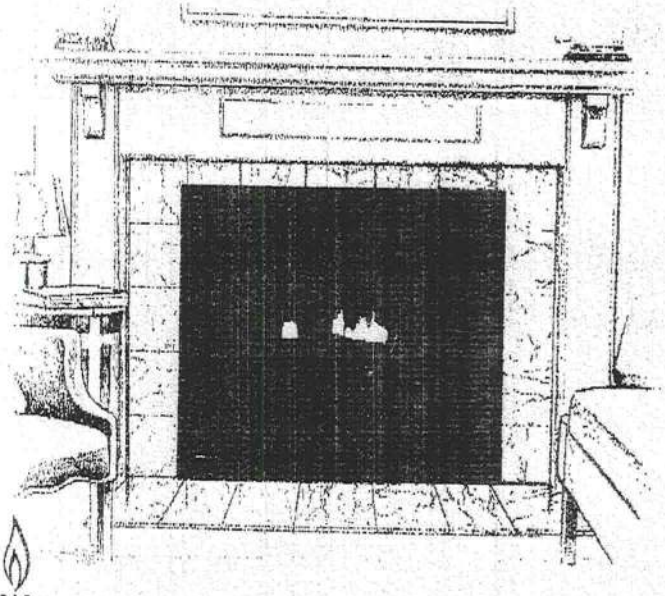
## VF-4000

- 14,000 – 25,000 Btu/hr with manual control valve
- 19,500 – 25,000 Btu/hr with millivolt control valve
- Fully assembled and ready to install
- Attractive wood surrounds available
- 15" x 30" fixed or operable screen opening



## VF-5000

- 25,000 Btu/hr millivolt variable heat output
- 15" X 30" glass or screen viewing area
- Clean burning, safe and easy to install
- Realistic charred oak logs with glowing embers



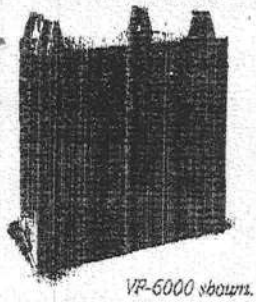
## VF-6000

- 32,000 Btu/hr millivolt variable heat output
- Beautiful 20" X 34" glass or screen viewing area
- Will operate during a power failure
- Designed for large rooms

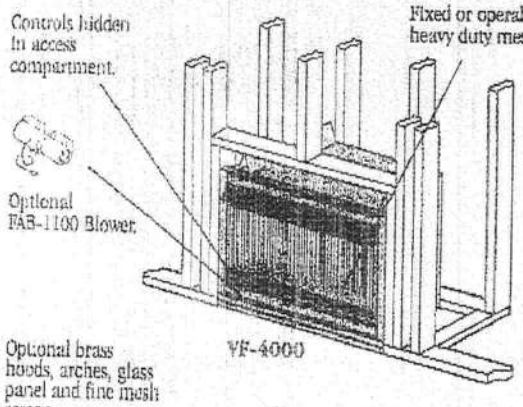
GAS

SUPERIOR.

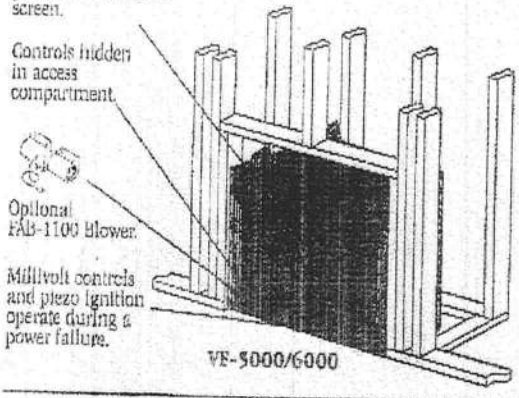
# VF-4000/5000/6000



*VF-6000 shown.*



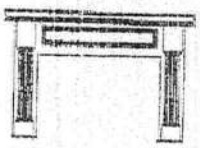

VF-4000

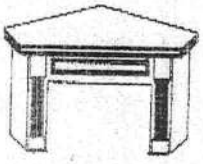



VF-5000/6000

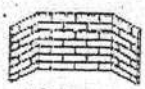
## SURROUNDS

The Charleston Poplar Surround is hand crafted using a combination of solid Poplar and Poplar veneer. Using the unique wood type of Poplar allows you the option to paint or stain this elegantly detailed surround. The surround is constructed using easy to assemble cam locks, and available in corner and wall units.



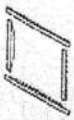


Distributed by:



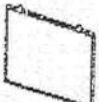
Refractory tan brick panels

Gas flex liner kit.

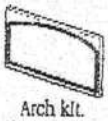


Square brass trim kit.

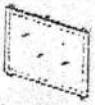
Brass Louver Kit (For VF-4 only)



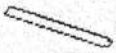
Screen panel kit. (For VF-5 & VF6 only)



Arch kit. (For VF-5 & VF6 only)



Glass door kit. (For VF-5 & VF6 only)

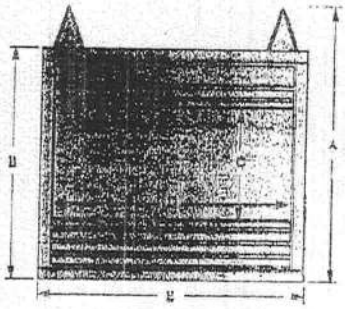


Brass hood. (For VF-5 & VF6 only)

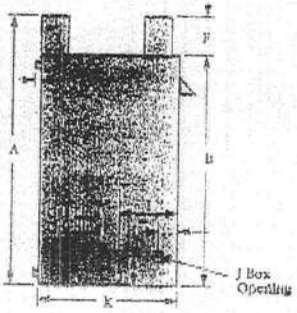
Wall switch or optional wireless remote available. (For VF-4MV, VF-5 & VF-6)

Wall thermostat. (For VF-4MV, VF-5 & VF-6)

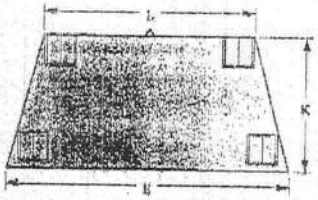
Front View



Left Side View




Top View



### Vent-Free Product Dimensions

| | VF-4000/5000C | VF-6000C |
|---|---|---|
| A | | 42-1/8" |
| B | 31-1/2" | 36-5/8" |
| C | | 20" |
| D | 30" | 34" |
| E | | 40" |
| F | 5-1/2" | 5-1/2" |
| G | | 1/2" |
| H | 3-3/4" | 3-3/4" |
| I | | 8-1/2" |
| J | 3" | 3" |
| K | 15-1/2" | 19-1/2" |
| L | 27" | 28-1/2" |

### Btu Chart

| Model | Natural | Propane |
|---|---|---|
| VF-4000 manual | 14,000 – 25,000 | 14,000 – 25,000 |
| VF-4000/5000 millivolt | 19,500 – 25,000 | 19,500 – 25,000 |
| VF-6000 | 25,000 – 32,000 | 25,000 – 32,000 |

### Framing Dimension

| Model | Width | Height | Depth |
|---|---|---|---|
| VF-4000/5000 | 37" | 37-1/4" | 15-1/2" |
| VF-6000 | 41" | 42-3/8" | 19-1/2" |

NOTE: Diagrams and illustrations are not to scale. Product designs, materials, dimensions, specifications, colors and prices subject to change or discontinuation without notice. Built to ANSI Z21.11.2 standard and approved by A.G.A. (report # L2970017).

Consult your distributor for local fireplace code information.

SUPERIOR

www.LennoxHearthProducts.com

P/N 903564 REV F 2/00

The Florida Department of Community Affairs

Building Code Information System

FLORIDA BUILDING CODE

Overview | User Registration | Organization Registration | User Authorization | Organization Search | Organization Accreditation

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Approval Status: (ALL)

Organization Name: General American Door - Product Manufacturer

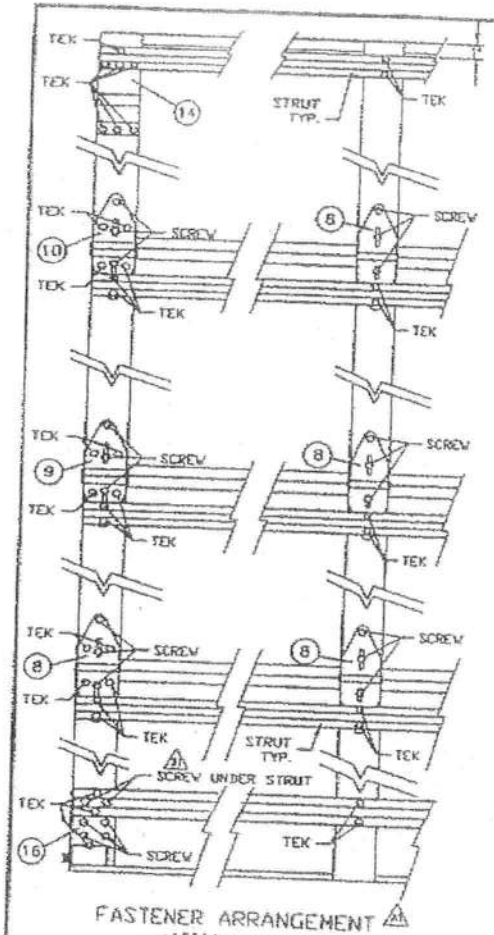
Cancel Search

## Result List for Organizations

Displaying 1-1 of 1

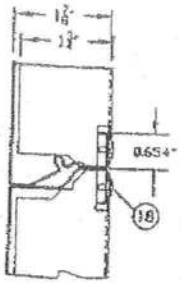
| Name | City | Contact | Phone | Type | Expire | Status |
|---|---|---|---|---|---|---|
| General American Door | Montgomery | James Campbell | 6308593000 | Product Manufacturer | 01/01/2099 | Approved |
| **Org Code:** PDM **System ID:** 3585 | | | **Site Link:** www.gadco.com | | | |

Displaying 1-1 of 1

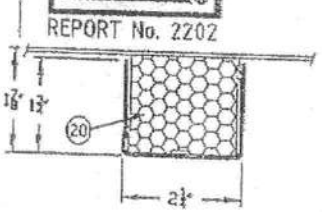



FASTENER ARRANGEMENT

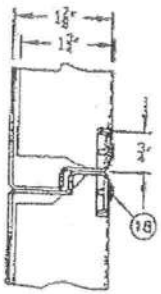
LISTED
SBCCI PST & ESI
REPORT No. 2202



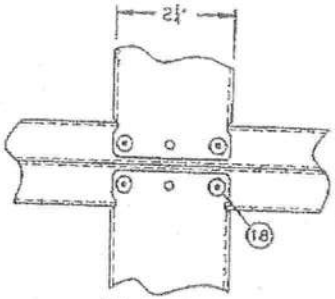
SEC D-D
PAN ATTACHMENT TO STILE (OPTIONAL)



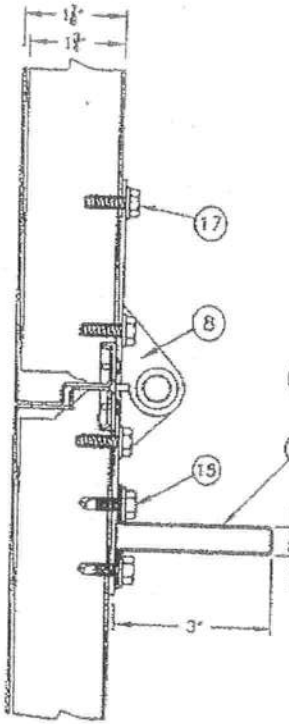
SEC G-G
CENTER STILE
20 GA GALVANIZED



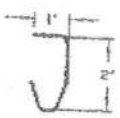
SEC D-D
PAN ATTACHMENT TO STILE (AS TESTED)



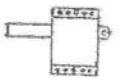
PAN ATTACHMENT TO STILE



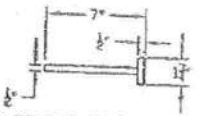
SEC A-A



TRACK
16 GA. (.055 MIN.)



(12) SIDE LOCK

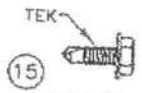


(11) ROLLER 10 BALLS

(4) #6 JAMB BRACKET

(1) 3"x20 GA. 80 KSI YIELD STRENGTH HEMMED STRUT APPLIED WITH 2 TEK SCREWS PER HINGE OR STILE LOCATION (14 PER STRUT, MINIMUM)

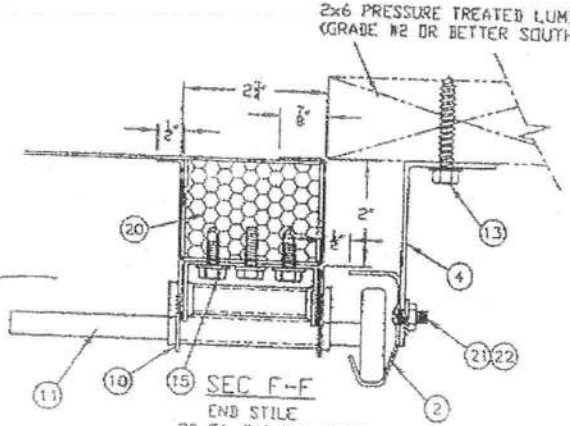
(17) ¼-20 X ¾ HEX WASHERHEAD SCREW



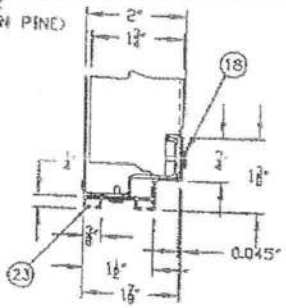
(15) ¼-20 X ¾ HWH TEK SCREW WITH #2 REDUCED POINT

| REV. | DATE | BY | DESCRIPTION |
|---|---|---|---|
| A | 11-27-00 | GW | SEE ECN 836 |
| B | 12-1-00 | GW | SEE ECN 836 |

| ITEM | PART NO. | DESCRIPTION |
|---|---|---|
| 23 | 3486 | BOTTOM RAIL 6063 T6 ALUM EXT. |
| 22 | 1391 | 1/4"-20 X 5/8" RIBBED NECK SCREW |
| 21 | 4044 | 1/4"-20 SERRATED FLANGE NUT |
| 20 | | LOW DENSITY FILLER BLOCK EPS |
| 19 | | |
| 18 | 4385 | POP RIVET, 5/32" STEEL |
| 17 | 4229 | HEX WASHERHEAD SCREW 1/4"-20 X 3/4" |
| 16 | 1399CLR | BOTTOM ROLLER BRACKET |
| 15 | 0138 | HWH TEK SCREW 1/4-20 X 3/4 W/ REDUCED PT. |
| 14 | 3875 | TOP SUPPORT BRACKET |
| 13 | 4043 | 5/16" X 1-5/8" LAG SCREW |
| 12 | 4710 | SIDE LOCK |
| 11 | 4305 | LONG STEM ROLLER 2" |
| 10 | 4203 | ROLLER CARRIER HINGE #3 (14 GA) |
| 9 | 4202 | ROLLER CARRIER HINGE #2 (14 GA) |
| 8 | 4201 | ROLLER CARRIER HINGE #1 (14 GA) |
| 7 | | COUNTER BALANCE (EXTENSION BAR & SPRINGS) |
| 6 | | HORIZONTAL TRACK & ANGLE (16 GA) |
| 5 | | VERTICAL FLAG ANGLE |
| 4 | 4585-6 | TRACK BRACKET #6 (12 GA) |
| 3 | | |
| 2 | | VERTICAL TRACK (16 GA.) |
| 1 | | 3" HEMMED STRUT (20 GA.) MIN YIELD STR. 80KSI |




SEC F-F
END STILE
20 GA. PAINTED STEEL



SEC E-E

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NABIL A. KEYVAN

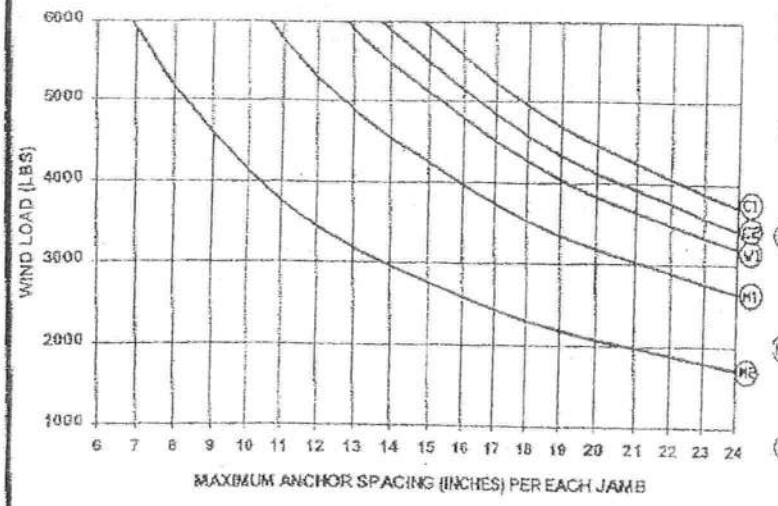
The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

GADCO
GENERAL AMERICAN DOOR COMPANY
9050 BASELINE ROAD
MONTGOMERY, IL 60538

SEAL: NONE | APPROVED BY: | DRAWN BY: D. WIEDMAN
DATE: 11-7-00 | REVISED: (B) 12-1-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR-WINDLOAD ±20 PSF

PAGE 2 OF 2 | DRAWING NUMBER: W13220-2

WIND LOAD vs ANCHOR SPACING




(C1) CONCRETE BACKUP HILTI KWIK BOLT II EXPANSION ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(C2) CONCRETE BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(M1) MASONRY(CMU) BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(M2) MASONRY(CMU) BACKUP ITW BUILDEX TAPCON MASONRY ANCHOR 1/4" DIA. 1-3/4" EMBEDMENT

(V1) WOOD STUD BACKUP LAG SCREWS 5/16" DIA. 1-1/2" EMBEDMENT

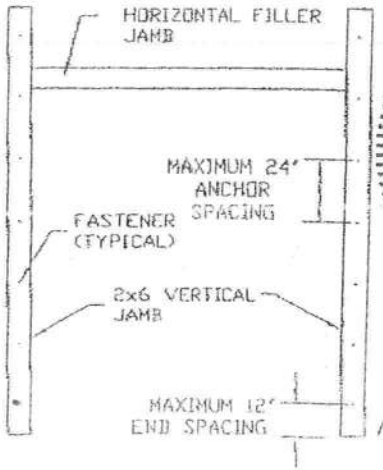
DESIGN LOAD (LBS)/FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X (16 FT WIDE X 8 FT HIGH) = 3840 LBS

- (C1) USE 22" SPACING
- (C2) USE 21" SPACING
- (V1) USE 19" SPACING
- (M1) USE 16" SPACING
- (M2) USE 10" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS




NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASER R KEYVAN

3/8/2002

NASER R KEYVAN 53774 STATE OF FLORIDA PROFESSIONAL ENGINEER

## 2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER 'HURRICANE' POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI 'STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION' SSTD 10,' CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

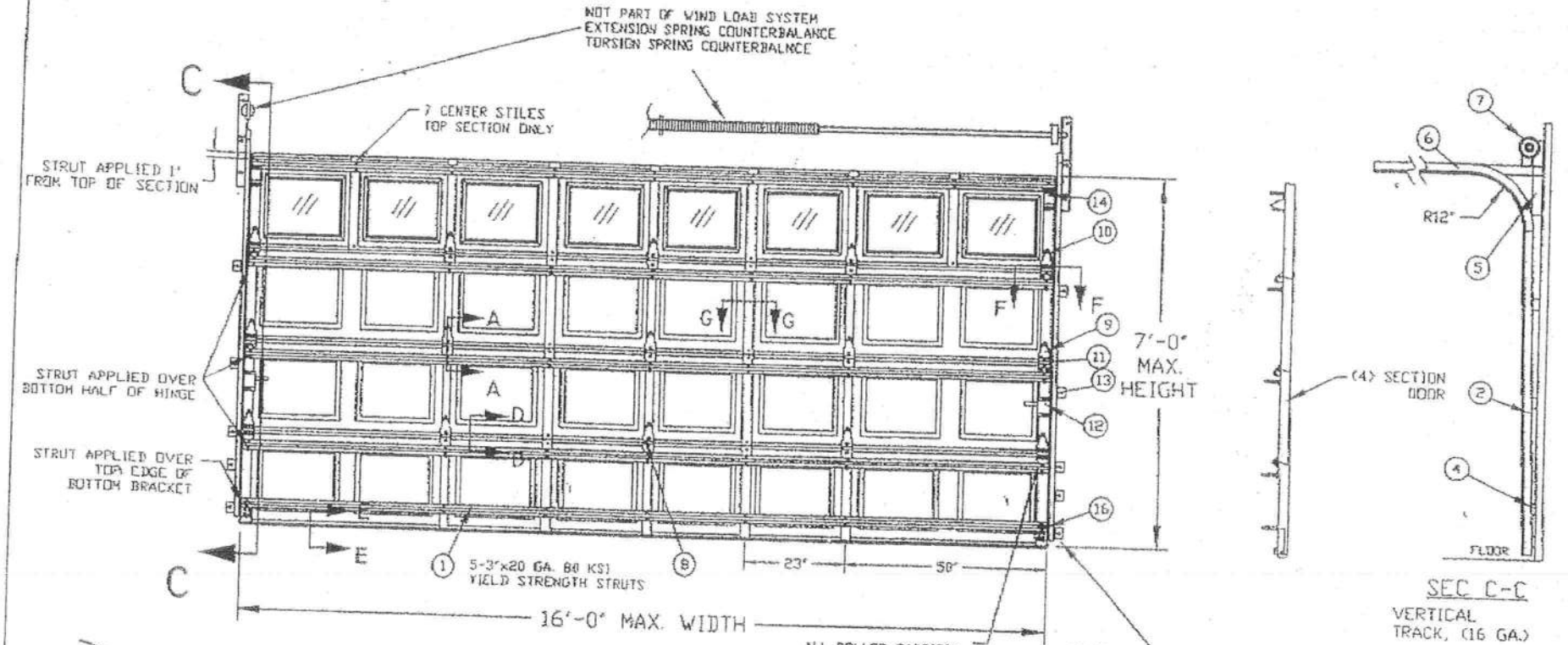
GADCO® GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

| SCALE: NONE | APPROVED BY: | DRAWN BY: DW |
|---|---|---|
| DATE: 8-30-99 | | REVISED |
| DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS | | |
| | MASTER DRAWING NUMBER: A10560 | DRAWING NUMBER |

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION, (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 12 AS TESTED).
9. ▲ DROP IN TYPE OF INSULATION IS OPTIONAL

| REV. | DATE | BY | DESCRIPTION |
|---|---|---|---|
| A ▲ | 11-10-00 | DW | SEE E.C.N. 831 |




INSIDE ELEVATION




SEC C-C
VERTICAL TRACK, (16 GA.)

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 024280 WALTER R. TEEVAN

LISTED SBCCI PST & ESI
REPORT No. 2202

TEST REPORTS ON FILE: VIDEO 10/19/00 (00293)

GADCO DOORS
SERIES 7480, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN ▲
SERIES 7524, EXTERIOR STEEL =.024" MIN ▲
(TESTED) WITH WINDOWS

| MAXIMUM DOOR WIDTH | MAXIMUM DOOR HEIGHT | TYPICAL CTR. STILE SPACING | STRUTS 80 KSI SIZE | STRUTS 80 KSI QTY. | VERTICAL TRACK |
|---|---|---|---|---|---|
| 16' | 7' | 23" | 3" | 5 | 2 IN. |

DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: 1/25 | APPROVED BY: | DRAWN BY: D. WIEDMAN
DATE: 10-20-00 | REVISED: (A) 11-10-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF
PART NUMBER: PAGE 1 OF 2 | DRAWING NUMBER: W13220-1

DOOR DATA

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

# PRODUCT CONTROL NOTICE OF ACCEPTANCE

Premdor Entry Systems
911 E. Jeferson, P.O. Box 76
Pittsburgh ,KS 66762

Your application for Notice of Acceptance (NOA) of:
**Entergy 6-8 S-W/E Inswing Opaque Double w/sidelites Residential Insulated Steel Door**
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 01-0314.24
EXPIRES: 04/02/2006

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 06/05/2001

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

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PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

# PRODUCT CONTROL NOTICE OF ACCEPTANCE

**Premdor Entry Systems**
**911 E. Jeferson, P.O. Box 76**
**Pittsburgh ,KS 66762**

Your application for Notice of Acceptance (NOA) of:
**Entergy 6-8 S-W/E Inswing Opaque Single w/sidelites Residential Insulated Steel Door**
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 01-0314.18
EXPIRES: 04/02/2006

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 06/05/2001

**AAMA/NWWDA 101/I.S.2-97**
**TEST REPORT SUMMARY**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

**SERIES/MODEL: 650**
**TYPE: Aluminum Picture Window**

| Title of Test | Results |
|---|---|
| Rating | F-R45 60 x 80 |
| Overall Design Pressure | +45.0 psf<br>-47.2 psf |
| Air Infiltration | 0.04 cfm/ft$^2$ |
| Water Resistance | 8.25 psf |
| Structural Test Pressure | +67.5 psf<br>-70.8 psf |
| Forced Entry Resistance | Grade 10 |

Reference should be made to Report No. 01-41135.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:mlb

ALLEN N. REEVES
CERTIFICATE NO. [illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Architectural Testing

# AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to.

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

**Test Specimen Description**

**Series/Model:** 650

**Type:** Aluminum Picture Window

**Overall Size:** 5' 0" wide by 6' 8" high

**Daylight Opening Size:** 4' 9-1/4" wide by 6' 5-1/4" high

**Finish** All aluminum was white.

**Glazing Details:** The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

ALLEN M. REEVES
CERTIFICATE NO. 19559
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves
1 APR 12 2002

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

**Test Specimen Description**: (Continued)

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

**Reinforcement:** No reinforcement was utilized.

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

**Test Results:**

The results are tabulated as follows:

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| | Air Infiltration (ASTM E 283-91) | | |
| | @ 1.57 psf (25 mph) | 0.04 cfm/ft$^2$ | 0.3 cfm/ft$^2$ max. |
| | *Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.* | | |
| | Water Resistance (ASTM E 547-00) | | |
| | WTP = 2.86 psf | No leakage | No leakage |
| 2.1.4.1 | Uniform Load Deflection (ASTM E 330-97) | | |
| | (Measurements reported were taken on the jamb) | | |
| | (Loads were held for 33 seconds) | | |
| | @ 25.9 psf (positive) | 0.01" | 0.41" max. |
| | @ 34.7 psf (negative) | 0.01" | 0.41" max. |
| 2.1.4.2 | Uniform Load Structural (ASTM E 330-97) | | |
| | (Measurements reported were taken on the jamb) | | |
| | (Loads were held for 10 seconds) | | |
| | @ 38.9 psf (positive) | 0.0" | 0.29" max. |
| | @ 52.1 psf (negative) | 0.01" | 0.29" max. |

ALLEN M. REEVES
No. 12354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves
1 APRIL 2002

**Test Results:** (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| | Forced Entry Resistance (ASTM F 588-97) | | |
| | Type: D<br>Grade: 10 | | |
| | Hand and Tool Manipulation Test | No entry | No entry |
| Optional Performance | | | |
| 4.3 | Water Resistance (ASTM E 547-00)<br>WTP = 8.25 psf | No leakage | No leakage |
| | Uniform Load Deflection (ASTM E 330-97)<br>(Measurements reported were taken on the jamb)<br>(Loads were held for 33 seconds) | | |
| | @ 45.0 psf (positive) | 0.02" | 0.41" max. |
| | @ 47.2 psf (negative) | 0.02" | 0.41" max. |
| | Uniform Load Structural (ASTM E 330-97)<br>(Measurements reported were taken on the jamb)<br>(Loads were held for 10 seconds) | | |
| | @ 67.5 psf (positive) | 0.01" | 0.29" max. |
| | @ 70.8 psf (negative) | 0.02" | 0.29" max. |

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:nlb
01-41135.01

Allen N. Reeves
Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

I

**AAMA/NWWDA 101/I.S.2-97**
**TEST REPORT SUMMARY**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

**SERIES/MODEL: 650 Fin**
**TYPE: Aluminum Single Hung Window**

| Title of Test | Results |
|---|---|
| Rating | H-R40 52 x 72 |
| Overall Design Pressure | +45.0 psf<br>-47.2 psf |
| Operating Force | 11 lb max. |
| Air Infiltration | 0.13 cfm/ft$^2$ |
| Water Resistance | 6.00 psf |
| Structural Test Pressure | +67.5 psf<br>-70.8 psf |
| Deglazing | Passed |
| Forced Entry Resistance | Grade 10 |

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:mlb

ALLEN N. REEVES
LICENSE
NO. 12254
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

II

Architectural Testing

# AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

**Test Specimen Description:**

**Series/Model:** 650 Fin

**Type:** Aluminum Single Hung Window

**Overall Size:** 4' 4-1/4" wide by 6' 0-3/8" high

**Active Sash Size:** 4' 1-3/4" wide by 3' 0-5/8" high

**Daylight Opening Size:** 3' 11-3/8" wide by 2' 9-1/2" high

**Screen Size:** 4' 0-1/4" wide by 2' 11-1/8" high

**Finish:** All aluminum was white.

**Glazing Details:** The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
No. 12354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

III



**Test Specimen Description:** (Continued)

**Weatherstripping:**

| Description | Quantity | Location |
|---|---|---|
| 0.230" high by 0.270" backed polypile with center fin | 1 Row | Fixed meeting rail |
| 0.250" high by 0.187" backed polypile with center fin | 2 Rows | Active sash stiles |
| 1/2" x 1/2" dust plug | 4 Pieces | Active sash, top and bottom of stiles |
| 1/4" foam-filled vinyl bulb seal | 1 Row | Active sash, bottom rail |

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

**Hardware:**

| Description | Quantity | Location |
|---|---|---|
| Metal cam lock with keeper | | Midspan, active meeting rail with keeper adjacent on fixed meeting rail |
| Plastic tilt latch | 2 | Active sash, meeting rail ends |
| Metal tilt pin | 2 | Active sash, bottom rail ends |
| Balance assembly | 2 | One in each jamb |
| Screen plunger | 2 | 4" from rail ends on top rail |

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES ★ CERTIFICATE NO. 19854 ★ STATE OF FLORIDA ★ PROFESSIONAL ENGINEER

IV



**Test Specimen Description:** (Continued)

**Drainage:** Sloped sill

**Reinforcement:** No reinforcement was utilized.

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

**Test Results:**

The results are tabulated as follows:

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.2.1.6.1 | Operating Force | 11 lbs | 30 lbs max |
| | Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph) | 0.13 cfm/ft$^2$ | 0.3 cfm/ft$^2$ max |

*Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

| | | | |
|---|---|---|---|
| | Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf | No leakage | No leakage |
| 2.1.4.1 | Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds) | | |
| | @ 25.9 psf (positive) | 0.42"* | 0.26" max. |
| | @ 34.7 psf (negative) | 0.43"* | 0.26" max. |

**Exceeds L/175 for deflection, but passes all other test requirements.*

| | | | |
|---|---|---|---|
| 2.1.4.2 | Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) | | |
| | @ 38.9 psf (positive) | 0.02" | 0.18" max. |
| | @ 52.1 psf (negative) | 0.02" | 0.18" max. |

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
NO. 1837
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

**Test Specimen Description:** (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.2.1.6.2 | Deglazing Test (ASTM E 987) | | |
| | In operating direction at 70 lbs | | |
| | Meeting rail | 0.12"/25% | 0.50"/100% |
| | Bottom rail | 0.12"/25% | 0.50"/100% |
| | In remaining direction at 50 lbs | | |
| | Left stile | 0.06"/12% | 0.50"/100% |
| | Right stile | 0.06"/12% | 0.50"/100% |
| | Forced Entry Resistance (ASTM F 588-97) | | |
| | Type: A<br>Grade: 10 | | |
| | Lock Manipulation Test | No entry | No entry |
| | Tests A1 through A5 | No entry | No entry |
| | Test A7 | No entry | No entry |
| | Lock Manipulation Test | No entry | No entry |
| Optional Performance | | | |
| 4.3 | Water Resistance (ASTM E 547-00)<br>(with and without screen)<br>WTP = 6.00 psf | No leakage | No leakage |
| | Uniform Load Deflection (ASTM E 330-97)<br>(Measurements reported were taken on the meeting rail)<br>(Loads were held for 33 seconds) | | |
| | @ 45.0 psf (positive) | 0.47"* | 0.26" max. |
| | @ 47.2 psf (negative) | 0.46"* | 0.26" max. |
| | Uniform Load Structural (ASTM E 330-97)<br>(Measurements reported were taken on the meeting rail)<br>(Loads were held for 10 seconds) | | |
| | @ 67.5 psf (positive) | 0.05" | 0.18" max. |
| | @ 70.8 psf (negative) | 0.05" | 0.18" max. |

*Exceeds L/175 for deflection, but passes all other test requirements.*

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE NO. 18334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

VI



Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:nlb
01-41134.01

A

**AAMA/NWWDA 101/I.S.2-97**
**TEST REPORT**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

**SERIES/MODEL: 650**
**TYPE: Aluminum Triple Single Hung Window**

| Title of Test | Summary of Results |
|---|---|
| AAMA Rating | H-R35 112 x 72 |
| Uniform Load Deflection Test Pressure | +35.3 psf -47.2 psf |
| Operating Force | 25 lb max. |
| Air Infiltration | 0.16 cfm/ft$^2$ |
| Water Resistance Test Pressure | 5.25 psf |
| Uniform Load Structural Test Pressure | +53.0 psf -52.5 psf |
| Deglazing | Passed |
| Forced Entry Resistance | Grade 10 |

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
No. 18254
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

# AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

**Test Specimen Description:**

**Series/Model:** 650

**Type:** Aluminum Triple Single Hung Window

**Overall Size:** 9' 3-1/2" wide by 5' 11-11/16" high

**Active Sash Size (3):** 3' 0-1/4" wide by 2' 10-3/4" high

**Fixed Daylight Opening Size (3):** 2' 8-1/4" wide by 2' 9-1/8" high

**Screen Size (3):** 2' 9-1/8" wide by 2' 11" high

**Finish:** All aluminum was painted white.

ALLEN T. REEVES JR. CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Allen T. Reeves
7 JUNE 2002

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

**Test Specimen Description:** (Continued)

**Glazing Details:** The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

**Weatherstripping:**

| Description | Quantity | Location |
|---|---|---|
| 0.230" high by 0.270" backed polypile with center fin | Row | Fixed meeting rail |
| 0.250" high by 0.187" backed polypile with center fin | 2 Rows | Active sash stiles |
| 1/2" by 1/2" dust plug | 4 Pieces | Active sash, top and bottom of stiles |
| 1/4" foam filled vinyl bulb seal | 1 Row | Active sash, bottom rail |

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
7 JUNE 2002

**Test Specimen Description:** (Continued)

**Hardware:**

| Description | Quantity | Location |
|---|---|---|
| Metal cam lock with keeper | 1 | Midspan of each active meeting rail with adjacent keepers |
| Plastic tilt latch | 2 | Each active sash meeting rail ends |
| Metal tilt pin | 2 | Each active sash bottom rail ends |
| Balance assembly | 2 | Each active sash contained one in each jamb |
| Screen plunger | 2 | Each screen contained two 4" from rail ends on top rail |

**Drainage:** Sloped sill

**Reinforcement:** No reinforcement was utilized.

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

**Test Results:**

The results are tabulated as follows.

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.2.1.6.1 | Operating Force | 25 lbs | 30 lbs max. |
| | Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph) | 0.16 cfm/ft$^2$ | 0.3 cfm/ft$^2$ max. |

*Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

| | | |
|---|---|---|
| Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf | No leakage | No leakage |

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 13254 STATE OF FLORIDA PROFESSIONAL ENGINEER

**Test Results:** (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.1.4.1 | Uniform Load Deflection (ASTM E 330-97)<br>(Measurements reported were taken on the mullion)<br>(Loads were held for 52 seconds) | | |
| | @ 15.0 psf (positive) | 0.15" | 0.41" max. |
| | @ 15.0 psf (negative) | 0.29" | 0.41" max. |
| 2.1.4.2 | Uniform Load Structural (ASTM E 330-97)<br>(Measurements reported were taken on the mullion)<br>(Loads were held for 10 seconds) | | |
| | @ 22.5 psf (positive) | 0.01" | 0.29" max. |
| | @ 22.5 psf (negative) | 0.01" | 0.29" max. |
| 2.2. .6.2 | Deglazing Test (ASTM E 987-88)<br>In operating direction at 70 lbs | | |
| | Right sash, meeting rail | 0.12"/25% | 0.50"/100% |
| | Right sash, bottom rail | 0.12"/25% | 0.50"/100% |
| | Middle sash, meeting rail | 0.12"/25% | 0.50"/100% |
| | Middle sash, bottom rail | 0.12"/25% | 0.50"/100% |
| | Left sash, meeting rail | 0.12"/25% | 0.50"/100% |
| | Left sash, bottom rail | 0.12"/25% | 0.50"/100% |
| | In remaining direction at 50 lbs | | |
| | Right sash, right stile | 0.06"/12% | 0.50"/100% |
| | Right sash, left stile | 0.06"/12% | 0.50"/100% |
| | Middle sash, right stile | 0.06"/12% | 0.50"/100% |
| | Middle sash, left stile | 0.06"/12% | 0.50"/100% |
| | Left sash, right stile | 0.06"/12% | 0.50"/100% |
| | Left sash, left stile | 0.06"/12% | 0.50"/100% |
| 2 .8 | Forced Entry Resistance (ASTM F 588-97) | | |
| | Type: A<br>Grade: 10 | | |
| | Lock Manipulation Test | No entry | No entry |
| | Test A1 through A5 | No entry | No entry |
| | Test A7 | No entry | No entry |
| | Lock Manipulation Test | No entry | No entry |

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
CERTIFICATE NO. 13854
STATE OF FLORIDA
PROFESSIONAL ENGINEER

**Test Results:** (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| **Optional Performance** | | | |
| 4.3 | Water Resistance (ASTM E 547-00)<br>(with and without screen)<br>WTP = 5.25 psf | No leakage | No leakage |
| | Uniform Load Deflection (ASTM E 330-97)<br>(Measurements reported were taken on the mullion)<br>(Loads were held for 52 seconds) | | |
| | @ 35.3 psf (positive) | 0.46"* | 0.41" max |
| | @ 47.2 psf (negative) | 0.67"* | 0.41" max |
| | Uniform Load Structural (ASTM E 330-97)<br>(Measurements reported were taken on the mullion)<br>(Loads were held for 10 seconds) | | |
| | @ 53.0 psf (positive) | 0.03" | 0.29" max |
| | @ 52.5 psf (negative) | 0.02" | 0.29" max |

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess
Technician

MAH:nlb
01-41641.01

Allen N. Reeves, P.E.
Director - Engineering Services
7 June 2002

ALLEN N. REEVES
CERTIFICATE NO. 19364
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Premdor Entry Systems

ACCEPTANCE No.: 01-0314.18

APPROVED : JUN 0 5 2001

EXPIRES : April 02, 2006

## NOTICE OF ACCEPTANCE: STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.

2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.

3. Renewals of Acceptance will not be considered if:
   a. There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes.
   b. The product is no longer the same product (identical) as the one originally approved.
   c. If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product.
   d. The engineer who originally prepared, signed and sealed the required documentation initially submitted, is no longer practicing the engineering profession.

4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fee) and granted by this office.

5. Any of the following shall also be grounds for removal of this Acceptance:
   a. Unsatisfactory performance of this product or process.
   b. Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purposes.

6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.

7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer needs not reseal the copies.

8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.

9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

END OF THIS ACCEPTANCE

Manuel Perez

Manuel Perez, P.E., Product Control Examiner
Product Control Division

Premdor Entry Systems

ACCEPTANCE No.: 01-0314.18

APPROVED : JUN 05 2001

EXPIRES : April 02, 2006

## NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. SCOPE

1.1 This renews the Notice of Acceptance No. 00-0321.20 which was issued on April 28, 2000. It approves a residential insulated door, as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.

2. PRODUCT DESCRIPTION

2.1 The Series Entergy 6-8 S-W/E Inswing Opaque Single Residential Insulated Steel Door with Sidelites- Impact Resistant Door Slab Only and its components shall be constructed in strict compliance with the following documents: Drawing No 31-1020-EW-I, Sheets 1 through 6 of 6, titled "Premdor (Entergy Brand) Wood Edge Single Door in Wood Frames with a Bumper Threshold (Inswing)," prepared by manufacturer, dated 7/29/97 with revision C dated 01/15/01, bearing the Miami-Dade County Product Control approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

3.1 This approval applies to single unit applications of single door only, as shown in approved drawings.

3.2 Unit shall be installed only at locations protected by a canopy or overhang such that the angle between the edge of canopy or overhang to sill is less than 45 degrees. Unless unit is installed in non-habitable areas where the unit and the area are designed to accept water infiltration.

4. INSTALLATION

4.1 The residential insulated steel door and its components shall be installed in strict compliance with the approved drawings.

4.2 Hurricane protection system (shutters):

4.2.1 Door: the installation of this unit will not require a hurricane protection system.

4.2.2 Sidelite: the installation of this unit will require a hurricane protection system.

5. LABELING

5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT REQUIREMENTS

6.1 Application for building permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings, as identified in Section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.

6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.

Manuel Perez

Manuel Perez, P.E. Product Control Examiner
Product Control Division

TAMKO®
ROOFING PRODUCTS

FEB - 4 RECD

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

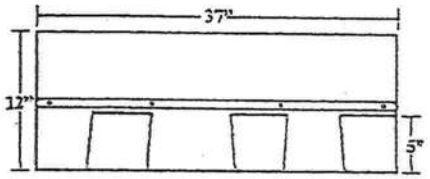
TAMKO Roofing Products, Inc.

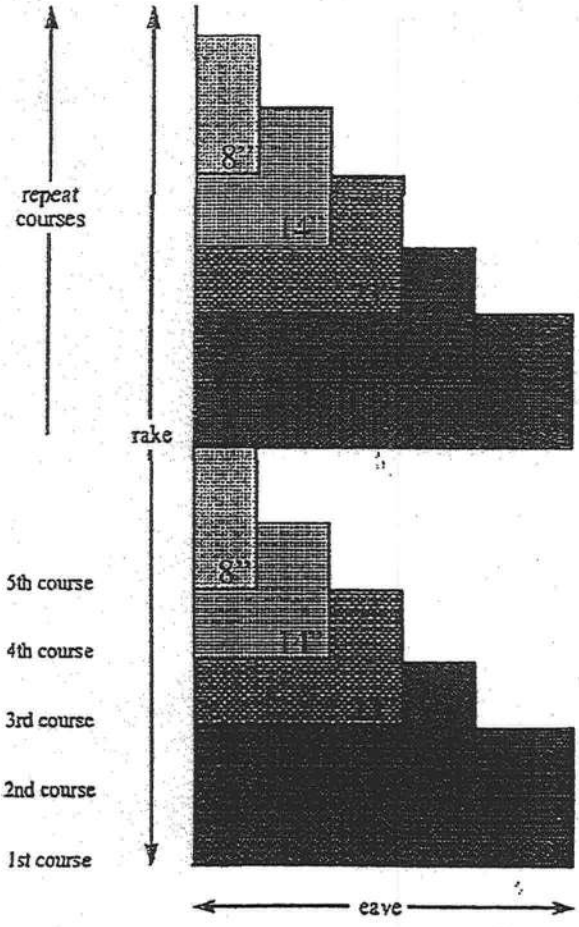
CORPORATE HEADQUARTERS
220 W. FOURTH STREET P.O. BOX 1404 JOPLIN, MO 64802-1404 800-641-4691 FAX 800-641-1925

TAMKO
ROOFING PRODUCTS

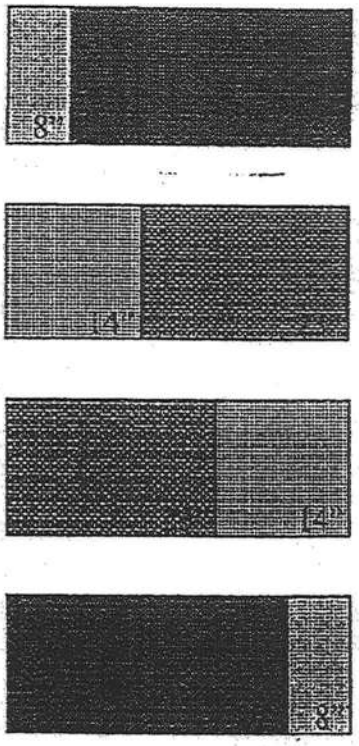
# Application Instructions For Heritage® 25 Series Shingles

| SPECIFICATIONS (APPROX.) | |
|---|---|
| Length | 37" |
| Width | 12" |
| Bundles per Sq. | 3 |
| Shingles per Sq. | 78 |
| Shingles per Bundle | 26 |
| Coverage per Sq. (Sq. Ft.) | 100 |
| Exposure | 5" |




The 4 cuts in the first 10 courses:



In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

**NOTE:** These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.

TAMKO®
ROOFING PRODUCTS

# Application Instructions for

- Glass-Seal
- Glass-Seal AR
- Elite Glass-Seal®
- Elite Glass-Seal® AR

## THREE-TAB ASPHALT SHINGLES

**THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.**
**THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.**
**IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.**

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

### 1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

### 2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.
FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

**IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.**

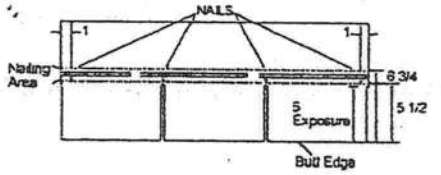
### 3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

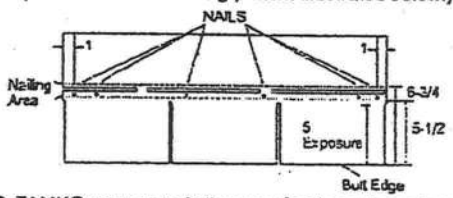
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below).




2) Mansard or High Wind Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fastener per shingle. (See Mansard fastening pattern illustrated below.)




NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

**TAMKO ROOFING PRODUCTS**

- Glass-Seal
- Glass-Seal AR
- Elite Glass-Seal®
- Elite Glass-Seal® AR

## THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a $.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Mansard Fastening Pattern.

### 3. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

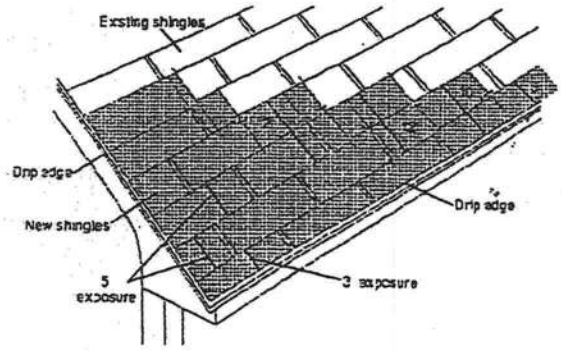
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nesting procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Succeeding Courses: According to the off-set application method you choose to use, remove the appropriate length from the rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.




### 3. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 50 lb. roll roofing in the valley. Nail the felt only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

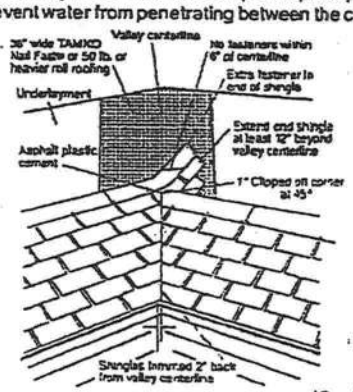
*Note: For a neater installation, snap a chalkline over the shingles for guidance.*

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

- ***CAUTION:*** *Adhesive must be applied in smooth, thin, even layers.*

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.




(Continued)

(CONTINUED from Pg. 3)

TAMKO® ROOFING PRODUCTS

# • Glass-Seal • Elite Glass-Seal®
# • Glass-Seal AR • Elite Glass-Seal® AR

## THREE-TAB ASPHALT SHINGLES

FOR ALTERNATE VALLEY APPLICATION METHODS, PLEASE CONTACT TAMKO'S TECHNICAL SERVICES DEPARTMENT.

### 10. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener 5-1/2 in. back from the exposed end and 1 in. up from the edge. Do not nail directly into the sealant.

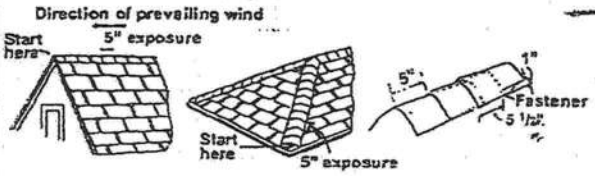
TAMKO recommends the use of TAMKO Hip & Ridge shingle products. Where matching colors are available, it is acceptable to use TAMKO's Glass-Seal or Elite Glass-Seal shingles cut down to 12 in. pieces.

*NOTE: AR type shingle products should be used as Hip & Ridge on Glass-Seal AR and Elite Glass-Seal AR shingles.*

Fasteners should be 1/4 in. longer than the one used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES IN COOL WEATHER.

**THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.**




**THIS PRODUCT IS COVERED BY A LIMITED WARRANTY. THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.**

**IMPORTANT - READ CAREFULLY BEFORE OPENING BUNDLE**

In this paragraph "You" and "Your" refer to the installer of the shingles and the owner of the building on which these shingles will be installed. This is a legally binding agreement between You and TAMKO Roofing Products, Inc. ("TAMKO"). By opening this bundle You agree: (a) to install the shingles strictly in accordance with the instructions printed on this wrapper; or (b) that shingles which are not installed strictly in accordance with the instructions printed on this wrapper are sold "AS IS" and are not covered by the limited warranty that is also printed on this wrapper, or any other warranty, including, but not limited to (except where prohibited by law) implied warranties of MERCHANTABILITY and FITNESS FOR USE.

# Project Review and Design Remediation Recommendations

for the

Remodeling and Addition to Existing Residence for
Mr. Ed and Diane Bishop White of Lake City, Florida

prepared for:

Mr. Jefferson Braswell of the law firm
Scruggs and Carmichael, PA of Gainesville, Florida.

Prepared by:

Martin Gold, AIA
AR 93691
m_gold design and Consulting
2710 NW 27th Place
Gainesville, Florida

20 March 2007

## Executive Summary

The execution of the residential addition design, as envisioned by Mr. and Mrs. White has been severely compromised through the architectural development and the execution of the phases of construction that have occurred up to 13 December, 2005. Errors in the drawings and unconventional building practices (that do not meet the minimum standard set by the Florida Building Code) have produced a partial structure that cannot be effectively completed as illustrated in the architectural drawings.

The fundamental massing, balance and proportion of an architectural composition compatible with the existing residence cannot be physically achieved with the construction that is presently in-place. This conclusion is based on the fact that the existing residence, of noted architectural integrity, has a roof pitch of 2 in 12 (2 foot rise in 12 feet of horizontal run), yet the design of the addition is based on a 3 in 12 roof pitch. The roof pitch is a critical global architectural parameter that establishes the building proportion, aesthetic symmetry, potential for room height and detailing required for the integration of second floor and loft space. The base bearing elevation of the roof was incorrectly identified on the drawings with the existing roof shown approximately 1' - 0" higher than it actually is. Furthermore, the relationship between the building and the ground is not correctly represented in the drawings — a critical error given the multiple ground levels around the building. Unfortunately, errors of incorrectly representing the existing roof pitch; incorrectly establishing the building ground relationship, using multiple incorrect dimensions; and incorrectly establishing the existing building height are present throughout the drawings. These errors do not seem to have been detected until late in the construction phase.

Many unconventional structural framing practices seem to have been developed by the contractor including the cutting and modification of trusses and engineered wood structural members in a manner inconsistent with the architectural and structural drawings and the Florida Building Code. The initial compromised structural integrity of the phase of construction completed to date has been exacerbated by exposure to weather rendering it unusable. Please also refer to the detailed structural assessment provided by TLC Engineering for Architecture (dated August 28, 2006) and the Freeman Design Group, Inc, (dated July, 28, 2006). Furthermore, these deficiencies were also documented in a facsimile follow-up letter and stop work order based on site inspections conducted December 13, 2005 by Columbia County Building Department officials.

We recommend a complete redesign of the addition to be architecturally compatible with the existing home. In order to use conventional structural systems and that would meet or exceed the standard of the Florida Building Code, the existing addition should be demolished. The incorrect 3 in 12 roof pitch and excessive building height results in massing and structural incongruities in the present construction that poses such a complex puzzle of questions regarding the usability of the slab as a 'correct footprint'; a suitable foundation for structural bearing; and the probable relocation of in-slab elements such as plumbing and electrical systems renders the slab likely unusable. Detailed structural analysis would be required to verify the integrity of the slab which would add expense and may prove it to be useless. Therefore, we suggest a complete demolition of the current phase of construction as the most efficacious solution to achieve an architecturally compatible and complete residential addition.

## Introduction

M_gold design and consulting has been engaged to evaluate the architectural design drawings of the *Remodeling and Addition to Existing Residence for Mr. Ed and Diane Bishop White* dated 9 May, 2005 with revisions on 21, July 2005; 23 July, 2005; 13 September, 2005; and 13 November 2005 as provided by Associated Florida Architects, Inc. The drawing set was signed and sealed by Mr. Robert S. Taylor, Sr., Architect, AR-0007526. We inspected the remaining existing structure and unfinished construction of the proposed design and met with Mr. White on February 6, 2007. Our analysis, recommendations and this report is based on the review of architectural drawings noted above, site visits, meetings with Mr. White and reviews of structural engineering reports and notes from the Columbia County Building Department. Detailed evaluations of the architectural drawings are included under the heading Drawing Review.

## Architectural Integrity

The existing residence of Mr. and Mrs. White is architecturally notable as a Prairie style home. The prairie style, developed by Frank Lloyd Wright in the first decades of the 20th Century, is characterized by a low-pitched roof, overhanging eaves, horizontal lines, central hearth (fireplace) corner windows, open floor plan and clerestory windows. These characteristics are important as the roof pitch is one of the fundamental defining features of the prairie style. Furthermore, the existing home represented a very high quality design interpretation that can be clearly characterized as prairie style architecture. Mr. and Mrs. White were quite sensitive to the architectural character of their home and engaged Associated Florida Architects, Inc. to translate their expansion ideas and spatial needs into an addition that would compliment the existing prairie style home.

## Drawing Review

This section includes our review of the individual drawing sheets that were provided by Associated Florida Architects, Inc. This review is not an extensive review and does not propose to detail all of the inconsistencies between the individual drawings, drawings and the existing structure nor conflicts between the drawings and the construction phase completed as of 13 December, 2005. Our comments are intended to reveal important inconsistencies that would cause serious difficulties in completing the project as shown in the drawings.

Field measurements were conducted to verify that the existing roof pitch has a slope of 2 feet of rise in 12 feet of run (2 in 12). The drawings prepared by Associated Florida Architects, Inc. represents the existing roof as having a 3 in 12 roof pitch. Field measurements were also used to verify the height of bearing at the lower eave walls to establish a reference for projecting the roof slope. We found the height of the existing wall at the lowest roof bearing to be approximately eight feet (8' - ½" actual) from the top of slab to the roof bearing point. The architectural drawings provided by Associated Florida Architects, Inc. , incorrectly, show this existing height of bearing to be nine feet (9') — an error of 1'-0".

| **Sheet** | **Description/Comments (in italics)** | |
|---|---|---|
| Cover | Cover, Index, Site Plan @ 1"=30' | (09 May 2005) |
| | • *no site plan actually shown on the cover sheet.* | |
| A - 1.01 | Existing Floor Plan @ 1/8" = 1'-0"<br>Demolition Plan @ 1/8" = 1'-0" | (09 May 2005) |
| | • *no accordion door and transom shown at bedroom entry.* | |
| A - 2.01 | Floor Plan @ 1/8" = 1'-0"<br>Second Floor Plan 1/8" = 1'-0" | (Rev 13 Nov. 2005) |
| | • *The stairs to the second level show 15 risers on the First Floor Plan and 16 risers on the Second Floor Plan. The landing location is not consistent between the plans resulting in ambiguity as to the circulation to and from the second level.*<br>• *Gate/entry fence columns not centered on courtyard doors nor are they aligned with the centers of the structural columns of the high ceiling family room. This imbalance will appear as odd.* | |

- *Roof overhang is not centered on Family Room facade. There is a 2'-0" overhang at the Garage side and a 1'-6" overhang at the existing residence side — they are both shown as 1'-6" on the elevation drawings (A - 4.01, revised 21, July, 2005).*

- *no door shown at bedroom entry in existing residence.*

A - 4.01 Elevations @ 1/4" = 1 foot (Rev, 13 Sep. 2005)
(field directive number one)

- *The elevation drawings show the existing walls at the lowest roof bearing to be approximately 9' tall. Field measurements revealed this dimension to be 8' - ½". This error can cause substantial problems in construction especially with a two-story, low roof slope design.*

- *The elevation drawings incorrectly show the building ground relationship of the existing structure.*

- *The elevation drawings incorrectly show the existing eave (overhang) to be 3'-0" when in fact it is actually 5' - 0".*

- *Multiple dimensions on the "Right Side Elevation" as shown on the drawings do not match field measurements with differences of 1'-4" in a dimension of less than 7' - 0". This is far beyond normal tolerances for architectural design at a the drawing scale of 1/8" = 1 foot used in the elevation drawings.*

- *no chimney or fire place venting system is shown for the existing fire place(s). The chimney element is usually a notable design feature of prairie style architecture.*

- *The spiral stair connecting the deck at the second level to the ground should be shown on the elevation as it makes a significant visual impact on the design.*

- *Brick veneer is shown graphically two different sizes of brick.*

A - 6.01 Building Section @ 1/4" = 1' - 0" (9 May 2005)

- *The building section is not keyed to the plan as is standard practice. Without the reference to the plan it is somewhat ambiguous as to where the section is taken.*

- *The circulation connection between the Entry Foyer (area inside of the entry doors) and the Family Room is not shown*

*on this drawing. There should be an opening of some kind to show how these spaces will be connected.*

- *The high roof structure is not shown. The structural system and depth of the structure should be shown for this element as is shown for other framing systems.*

- *16 stair risers are shown on the Building Section which is inconsistent with the First Floor Plan. Furthermore, all risers are not shown with the same height — The Florida Building Code requires all stair risers to be the same height.*

- *The Partial Stair Section detail calls for 7-3/4" risers, which with 16 risers would come to 10' - 4" above the first floor — the second floor is shown at 10' - 0". With 15 stair risers, as is shown on some drawings, the height of the stair would be 9" - 8-1/4 ". The stairs do not connect with the second floor level as shown in any of the drawings.*

A - 7.01 Typical Wall Sections @ 3/4" = 1' - 0" (09 May 2005)

- *We count at least six different wall/roof or wall/deck bearing conditions. As only one "typical" detail is provided, there is great ambiguity as to how the roof and walls will be joined the variety of configurations that exist in the design.*

- *The "Optional Typical Detail @ Existing Roof" does not have dimensions noted. When the detail is scaled as per the 3/4" = 1' - 0" scale, the wall height is shown at nine feet (9' - 0"). Field measurements indicate the existing wall is approximately eight feet tall (8' - ½" actual).*

M - 1.01 Mechanical Plan - First Floor @ 1/8" = 1' - 0" (09 May 2005)
Mechanical Plan - Second Floor @ 1/8" = 1' - 0"

- *No drop ceiling or duct soffit shown for supply and return air outlets in the Pub space.*

- *The supply duct leading to the playroom looks like it will be exposed on the elevation of the building if it is run up to typical supply air height. This plan also shows a window at the top of the stairs that is not in the final version of the plan and elevation drawings.*

- *A curved landing is also shown here that does not appear in other drawings.*

E - 1.01 Electrical Plan - First Floor @ 1/8" = 1' - 0" (09 May 2005)
Electrical Plan - Second Floor @ 1/8" = 1' - 0"

- *The existing panel box is shown to be located on a wall that is shown as removed on the floor plan (A - 2.01, all revision dates) and on the building section (A - 6.01, 9 May, 2005).*
- *A light is shown at a balcony on the second floor. This balcony is not shown in the most recent First Floor Plan or Second Floor Plan.*

S - 1.01 Details, Foundation Plan @ 1/8" = 1' - 0" (09 May 2005)

- *No comments*

S - 2.01 Roof Framing Plan: New Addition (13 Sep. 2005)
@ 1/8" = 1' - 0", Notes, Details

- *Bearing details for framing over the existing structurally modified wall are not shown. This is a complicated structural connection that should be detailed and reviewed by a licensed structural engineer.*
- *The new second level deck is in conflict with the existing roof edge based on the drawings — they are shown to overlap.*
- *18 stair risers shown on the First Floor Framing Plan (15 and 16 risers shown on other drawings as noted above).*
- *A Balcony is shown at the Second Level that is not shown in the most recent Second Floor Plan (13 Sep. 2005).*

The drawings do not meet, in our professional opinion, the minimum standard of care expected for architectural design. There are substantial conflicts between and within the drawings of various dates including the most recent revision. There are, in our professional judgement, significant omissions of important details and drawings such as building sections or wall sections that show the integration of details with major building components.

## Structural Integrity

There are serious concerns with the structural integrity of the construction phase that is in place at the time of this report. Deficiencies and inconsistencies regarding the structural veracity of the Construction Documents drawings and the existing partial structure are well documented by the structural evaluation conducted by TLC Engineering for Architects dated August 28, 2006. The review and recommendations by the Freeman Design Group Engineers and Planners points out many difficulties that seemed to arise from trying to correct design conflicts during construction. Furthermore, the building officials from Columbia County in their follow-up letter from a December 13, 2005 site inspection clearly notes the deficiency of the first floor walls ability to support the second floor. These are very serious deficiencies that could lead to structural failure.

## Course of Action

A complete redesign of the addition based on the original sketches that Mr. White developed is required to achieve the renovated home originally commissioned. The correct eave wall height and roof pitch of the existing prairie style home must be used in the earliest phases of design to achieve an integrated and aesthetically balanced union between the new and existing structures. The design should not only be studied in orthographic drawings (2D) but also in 3D computer models and/or physical scale models. This is required to work out the complexities of multiple levels interpenetrating a low-slope (2 in 12) roof system.

Demolish, in it's entirety, the partially complete addition that is in place.

A budget cost estimate must be developed based on new construction costs in the locality and projected out to the estimated time of actual construction. This construction cost estimate should be based on the square foot area with specific adjustments for the quality of materials that are clearly indicated in the design proposal by Associated Architects. Furthermore, adjustments will be required for integrating the structural connection between the new addition and the existing residence — this may need to go beyond the original design as structural analysis does not seem to have been integrated into the construction drawings. A qualified cost consultant should be engaged to develop a project budget based on a best estimate given the quality and scope (area and massing) of the design suggested in drawings by Associated Florida Architects, Inc. The cost estimate must include any specific materials, appliances and finishes called out in the construction documents including the Architectural Specifications for the project.

A demolition cost estimate will also need to be developed.

Architects fees and engineering fees will need to be estimated. These fees can be estimated as a percentage of the cost of construction. Architectural services for design phases, construction documents phases and site supervision phase for a home of this scale and architectural significance would be approximately 10% of the actual cost of construction. Engineering fees could range from 1% to 3% of the cost of construction — an engineering firm should be consulted to hone this estimate.

## Conclusions

1. The drawings provided by Associated Florida Architects, Inc. do not meet the minimum professional standard and are rife with significant errors and omissions.

2. The Associated Florida Architects, Inc. drawings incorrectly establish the roof pitch of the existing structure subsequently compromising the design of the addition — as drawn, it will not match the existing when constructed.

3. The Associated Florida Architects, Inc. drawings incorrectly establish the height of the low eave bearing point with an error of approximately one foot — the addition, if constructed to the drawings, will not align architecturally with the existing structure when constructed.

4. The Associated Florida Architects, Inc. drawings incorrectly establish the various ground/foundation heights of the existing structure.

5. Modifications to the design were made in the field to correct design errors that contributed to additional conflicts and the noted lack of structural integrity.

6. The exposed frame structure has been compromised by the weather, moisture and molds and is unusable.

7. The structural integrity of the slab does not meet, as per the TLC engineering report, the bearing capacity required for the roof and wall loads.

8. The partially complete addition should be entirely demolished and removed.

9. A complete redesign of the addition is required to integrate the original plan provided by Mr. and Mrs White with the prairie style existing

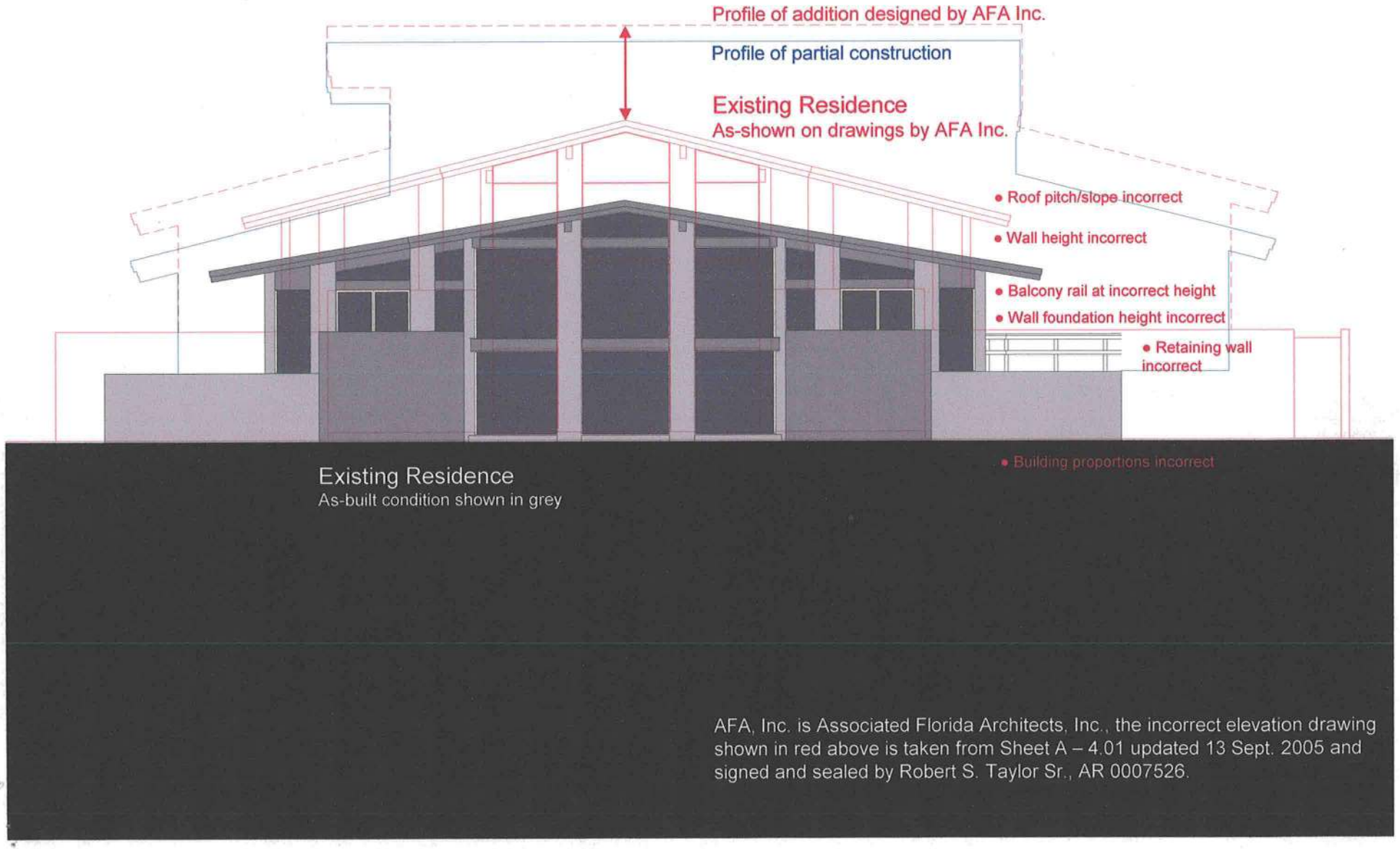
home. This will require new and complete architectural and engineering services.

10. A project construction budget must be established based on the built floor area and adjustments for structural requirements and the quality of finishes (as already defined) to establish the professional fees.

11. A complete redesign with integrated structural analysis could take 6 to 12 months.

End of Report

Martin A. Gold, AIA
Principal Architect
AR 93691




AFA, Inc. is Associated Florida Architects, Inc., the incorrect elevation drawing shown in red above is taken from Sheet A – 4.01 updated 13 Sept. 2005 and signed and sealed by Robert S. Taylor Sr., AR 0007526.

ENGINEERING
FOR ARCHITECTURE

August 28, 2006

Ed and Diane White
P.O. Box 2111
Lake City, FL 32024

Re: Ed and Diane White Residence
Structural Condition Survey Report
TLC Project No. 106628

Dear Mr. & Mrs. White:

In accordance with your request, on July 27, 2006 a Registered Engineer from this office of TLC Engineering for Architecture, Inc. (TLC) conducted limited visual observations of your residence located approximately 7 miles southwest of Lake City, Florida on State Route 47.

The purpose of this inspection was to observe existing structural conditions and gather information that would enable TLC to render an opinion concerning the condition of the construction work of the building addition to your residence. In addition to evaluating the condition of the work in-place, TLC was asked to review the construction documents prepared by Robert S. Taylor, Sr., Associated Florida Architects, Gainesville, Florida and Tamara G. Baker, P.E., Baker Engineering & Consulting, P.L., Jacksonville, Florida and render an opinion regarding the content of the construction documents. TLC's field visit and this report are not intended to cover hidden defects, hazardous materials, mechanical, electrical or architectural features.

GENERAL INFORMATION

Ed and Diane White are the owners of the residence constructed in 1966 by the father of Diane White. It was the desire of the White's to construct a large, 2-story, 3000 square foot building addition to the existing structure and hired Associated Florida Architects (AFA) to prepare the construction documents. The Whites gave AFA floor plan design sketches and building elevations as a general basis of their concept for the design. AFA prepared construction documents and submitted them to the Columbia County Building Authority for construction permit. A building permit was issued July 25, 2005. Isaac Construction Company, Lake City, Florida was hired by the Whites to construct the building addition. On December 13, 2005, Columbia County Building Authority issued a Stop Work Order and directed the structure be evaluated by a Florida Professional Engineer for compliance with Florida Building Code 2001. To date, no further construction work has been performed by Isaac Construction.

TLC ENGINEERING FOR ARCHITECTURE, INC.
1717 South Orange Avenue . Suite 300 . Orlando, FL 32806-2953
Phone 407.841.9050 www.tlc-engineers.com Fax 407.540.1039

TLC was given copies of many of the construction documents prepared by AFA and Baker Engineering and Consulting, L.P., Jacksonville, Florida. The AFA permit drawings consisted of the following sheets dated May 9, 2005:

| | |
|---|---|
| Cover | Index of drawings and Wind Load Design Parameters |
| A1.01 | Existing Floor Plan, Demolition Plan |
| A2.01 | First & Second Floor Plans |
| A4.01 | Exterior Building Elevations |
| A6.01 | Building Sections |
| A7.01 | Typical Wall Sections |
| M1.01 | Mechanical Plan First Floor, Mechanical Plan Second Floor |
| E1.01 | Electrical Plan First Floor, Electrical Plan Second Floor |
| S1.01 | Foundation Plan and Details |
| S2.01 | Roof Framing Plan: New Addition, Notes, Details |

Revised drawings dated July 21, 2005, identified as Addendum Number One:

| | |
|---|---|
| A2.01 | First & Second Floor Plans, Finish Schedules, Partial Stair Section, Wall Legend |
| A4.01 | Exterior Building Elevations (4 Each) |
| S2.01 | First & Second Floor Roof Framing Plans |

Structural drawings prepared by Baker Engineering & Consulting Signed/Sealed May 23, 2005 for inclusion in Addendum Number One.

| | |
|---|---|
| S-1 | General Structural Notes |
| S-2 | Framing Details |

OBSERVATIONS

The following is a list of comments concerning the content of the construction documents:

1. Construction drawings and details do not specify an adequate building lateral bracing system to resist Florida Building Code 2001, with 2003 Revisions Building Code (FBC) minimum specified Main Wind Force Pressures. The construction documents do not meet, in our professional opinion, the minimum standard of care for the building structural systems.

2. Construction drawings and details do not specify adequate building foundations at building columns to resist FBC minimum specified Main Wind Force uplift pressures and gravity loads. The construction documents do not meet, in our professional opinion, the minimum standard of care for the building structural systems.

3. Construction drawings and details do not, in our professional opinion, provide adequate fastening and anchoring of new wall sections and roof members to existing structure.

4. Construction drawings and details do not correctly portray the roof pitch of the existing building structure. The design of the building addition was based on faulty information and resulted in building elevations not consistent with as built conditions. The building elevations depicted in the construction drawings and the expectations of the Owners could not be achieved as represented by the drawings. The architect did not verify the as built conditions and developed a design not constructible as shown on the drawings. In our professional opinion, the construction drawings do not meet the minimum standard of care for professionally prepared construction documents.

The following is a list of observations made during the site investigation July 27, 2006:

5. The high roof area is constructed without any evidence of a lateral bracing system to resist FBC main wind forces.

6. Anchoring of the new wall on the west end of the existing building structure is structurally inadequate. No continuous load path to foundation for wind lateral and uplift loads is provided.

7. Columns supporting the high roof structure, second floor loft area and the rear second floor deck structure are not as specified on the construction drawings. No column foundations were observed for these columns.

8. Extension of the side walls supporting the high roof structure is unconventional and does not provide continuous load path for wind uplift forces.

9. Construction of cantilevered engineered wood beams supporting high roof structure is unconventionally framed. The framing does not provide continuous load path to the foundation for lateral and uplift wind forces.

10. Construction of the east wall supporting high roof structure was founded on the existing roof structure not competent to support the loads. Fastening of the new wall to the existing structure is structurally inadequate and provides no continuous load path to foundation for wind lateral and uplift forces.

11. Modification of high roof structure pre-engineered wood trusses is unconventional and structurally inadequate. The truss modifications appear to be absent of engineering input from the truss engineer.

12. The foundation slab on grade was observed to have minimum random cracks. No settlement of the foundation slab was observed. Verification of the foundation size and depth was not made.

13. Mono-pitched roof framing at the second floor loft area is unconventional framing and structurally inadequate. The beam splice/connection is on the verge of structural failure.

14. Construction of second floor deck framing is improperly framed to direct rain water away from the building interior.

15. Interior wood columns supporting the high roof structure are not the members specified on the construction documents and are inadequate to support the roof loads. The columns as constructed bear on the second floor sheathing and are not continuous members to the foundation level. The framing system is unconventional and not compliant with building code standards.

16. High roof trusses are not anchored by metal straps as specified in contract documents and are not in compliant with building code standards.

17. The exposed to weather wood framing materials are severely deteriorated and warped. The majority of the exposed framing must be replaced because of deterioration and sub-standard framing assemblies.

CONCLUSIONS

Based on our observations, we have the following comments:

1. The construction documents do not, in our professional opinion, meet the standard of care for professionally prepared documents. The documents are incomplete, incorrect depiction of as built conditions and structurally inadequate to specify the contraction of the work in accordance with building code standards and accepted engineering standards.

2. The contractor proceeded with construction of the work prior to verification of as built conditions and specifically the roof slope. In our professional opinion, the work proceeded long after the roof slope discrepancy was discovered and framing alterations were initiated by the contractor to compensate for the roof slope discrepancy. Much of the framing alterations were made, in our professional opinion, in the absence of structural engineering consultations as evidenced by the large extent of unconventional framing observed. At the time of the Stop Work Order by Colombia County Building Authority, the as built configuration of the building addition did not resemble the building elevations shown in the construction documents.

3. Construction of the work did not comply with the specified 6"x6" square wood structural columns and isolated concrete foundations shown on the construction documents for the high roof and rear deck structure. The columns constructed are not adequate to support the loads of the structure. The concrete foundations were not constructed.

4. Deterioration of the exposed to weather wood framing is severe and in our professional opinion, should be removed. The extent of the deterioration of the under roof structure framing is, in our professional opinion, minor to severe. However, the extent of unconventional framing and

absences of consistent load path through the structure to the foundation indicates the entire framing be removed from the site and reconstructed to building code standards.

5. Further investigation of the foundation slab on grade is recommended to verify if the foundation system as built is in accordance with building code standards. The existing slab was observed to be generally free of random cracking and a portion of the slab could be used if investigation proves the construction is acceptable. Although several isolated column foundations were not constructed, the foundations could be added to the existing slab on grade if deemed cost effective.

Our opinions are based upon engineering judgment to an extent normal for a field review of this type. We reserve the right to revise or amend our opinions if additional information becomes available in the future. Our review was walk-through in nature and we did not use any special tools or instruments were used for our observations. No material testing, intrusive investigations or structural analysis were made for preparation of this report.

This report is prepared for the sole benefit of Ed and Diane White. Unauthorized use of the information contained in this report without our permission shall result in no liability or legal exposure to TLC.

We appreciate this opportunity to be of service to you. If we can be of further assistance, please call me.

Very truly yours,

TLC ENGINEERING FOR ARCHITECTURE, INC.

D.B. Griffith

David B. Griffith, PE
Principal/Senior Project Engineer

Roger L. Jeffery, P.E.
Principal/Structural Division Director

L:\2006-Jobs\106628 White Residence\Report letter.doc

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Dear Mr. Ed White

On December 13, 2005 per your request Mr. Randy Jones Assistant Building Official, Mr. Harry Dicks Building Inspector and I as the Plans Examiner accompanied you with a structural inspection of your resident at 7018 SW SR 47. The inspection pertains to Building permit number 23411 issued July 25, 2005 to ISAAC. This build permit was to construct an addition on to an existing single family dwelling.

You requested that we inspect the structural framing which has been completed thus far for building code compliance. The Columbia County Building Department has several concerns which are:

1. Mr. Robert Taylor Architect, of Associated Florida Architects, Inc. 802 NW 23rd Avenue Gainesville Florida, State of Florida Registration Number AR-0007526 filed a letter with the Columbia County Building Department on July 22, 2005 which stated that Associated Florida Architects, Inc. were the architects of record for the construction of a addition on to your single family dwelling. Mr. Robert Taylor in this letter also stated that certain structural issues which I as the plan examiner had would be addressed once the construction of this addition was started. Mr. Robert Taylor stated that within forty-five (45) days of commencing with construction a written structural report addressing the needed issues would be filed with the Columbia County Building Department. **No reports have been**

**filed with the Columbia County Building Department by Mr. Taylor or Associated Florida Architects.**

2. An example of one of the issues which was not addressed was the use of an exterior load bearing foundations and shear walls which was used to construct a second story load bearing wall without the contractor and the architect confirming that the foundation and the structural walls could safely support the additional structural load-bearing weight which has been applied to the foundation and walls for of a second story and a roof truss system.
3. The addition onto the structure of 3,044 square feet incorporated a first and second floor, using wood frame structural walls. Within the first floor several exterior and interior load bearing walls have been created to provide support for the second floor, flooring and roof truss system. Several first floor walls lack sufficient structural strength to properly support the second floor.
2. The roof trusses have been modified by the contractor, the truss designer will be required to assess the trusses system to certify that the trusses system meet the original truss system design.

In order to continue work on this structure, a structural evaluation by a professional architect or engineer is needed to comply with the Florida Building Codes 2001. If you should have any additional question please contact me.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

Freeman Design Group inc
Engineers • Planners

161 N.W. Madison St., Suite 102
Lake City, Florida 32055
Tel: 386-758-4209
Fax: 386-758-4290

7/28/2006

Ed White
P. O. Box 1376
Lake City, FL. 32056

Re: Structural Inspection

Dear Mr. White,

I have completed the structural inspection and have reviewed the original plans and the following evidence is evident:

The plans call for an exterior bearing wall height of 9'0". The exterior bearing walls were constructed at an 8'0" height. The ceiling height of the master bedroom is shown as a 9'0" ceiling height but was constructed as a 10'0" ceiling height. During construction the roof of the first floor interfered with the floor level of the second floor since the wall was constructed 1'0" too short and the floor level 1'0" too high. This caused a problem with floor joist layout as indicated on the plans. Since the floor joists interfered with the incorrectly placed roof system, the floor joists were altered so that they were no longer bearing on the walls as indicated on the plans. The plans did indicate structural load bearing walls to support the upper floor joist system, however, by shifting the bearing locations to new non-structural walls there was no provision for a structural foundation or structural headers. This could cause failure in the slab and wall components.

The two-story portion of the house was not constructed to the proper wall height as indicated on the plans. The original design called for a set of four (4) beams to run continuous from front to rear of the structure. The exterior walls were constructed in this manor to perform as a post and beam system. The beams are intended to span a certain distance and then bear on a post which has a structural footing directly underneath. The exterior walls on the two story portion were altered to bring the height up according to the original design. When this was done, the structural beam was removed in order to build a new knee wall above the original wall that was built too low.
This created two main problems. When the beam was removed, the post-beam structure became a continuous load-bearing wall. There is no interior structural footing to carry the load along the entire length of wall. When the knee wall was constructed to extend the

wall height per design, a hinge point was created in the wall. This could likely fail when wind pressure is applied to the wall.

A new wall was constructed on top of the original house. At this point, there is no evidence that the exterior wall is resting on anything structural. The weight of the wall and the roof trusses are only bearing on the existing roof decking and is showing signs of excessive deflection. The load-bearing wall needs to have a continuous load path from the foundation, up to the roof.

In order to solve the issues discussed above the structure would need to be dismantled. A redesign of the project should be accomplished taking into account the correct pitch of the existing roof. It should contain detail for the joining of the new and the old structure and foundation requirements to support changes in the correct plans. Because the structure could not be completed the trusses have now been exposed to the weather for nearly a year and have shown signs of twisting and buckling.

If you have any questions, you may contact me at (386) 758-4209.

Sincerely,

William Freeman, P.E.

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Dear Mr. Ed White

On December 13, 2005 per your request Mr. Randy Jones Assistant Building Official, Mr. Harry Dicks Building Inspector and I as the Plans Examiner accompanied you with a structural inspection of your resident at 7018 SW Kemp Court. The inspection pertains to Building permit number 23411 issued July 25, 2005 to ISAAC. A Build Permit to construction an addition on to an existing single family dwelling.

You requested that we inspect the structural framing which has been completed thus far for building code compliance. The Columbia County Building Department has several questions and concerns which are describer below.

1. Mr. Robert Taylor Architect, of Associated Florida Architects, Inc. 802 NW 23rd Avenue Gainesville Florida, State of Florida Registration Number AR-0007526 filed a letter (attached) with the Columbia County Building Department on July 22, 2005 which stated that Associated Florida Architects, Inc. were the architects of record for the construction of a addition on to your single family dwelling. Mr. Robert Taylor in this letter also stated that certain structural issues which I as the plan examiner had would be address once the construction of this addition was started. Also with this correspondence Mr. Robert Taylor confirmed that within forty-five (45) day/of commencing with construction a written report would be filed with the Columbia County Building Department to comply with the Florida Building Code 2001 Chapter thirty four (34) sections 3401.8.2.3.1, 3401.8.2.3.2

and 3401.8.2.3.3 which are attached. As of this date these code requirements have not be fulfilled by Mr. Taylor

2. The north existing wall prior to construction was a shear wall for the single family dwelling, additional structural load-bearing weight has been applied to the foundation and wall by the contractor without confirming that the foundation and the structural wall could accommodate these structural loads for of a second story and a roof truss system. The building department questions the foundation integrity to have sufficient load bearing capacity to support the additional weight which has been applied to this foundation and wall. An engineering report will be required to be submitted to building department to state that the existing foundation and wall as modified complies with the Florida building code, as this code relates to foundation and wind-load design requirements for a structural wall of the height to which the wall has been constructed and all components of the wall will receive the required anchoring and strapping devices to comply with the wind-load design requirements.
3. The addition onto the structure of 3,044 square feet incorporated a first and second floor, with wood frame structural walls. Within the first floor several exterior and interior load bearing wall were created to provide support for the second floor, flooring system and roof trusses. Several of these first floor walls lack sufficient structural strength to properly support the second floor. An engineering review of these load bearing walls will be required to bring these walls into compliance with the Florida building code requirements for load bearing walls.

4. Several of the second story roof trusses have been modified by the builder, the engineered truss designer will be required to assess the trusses as modified to certify that these trusses still meet the original truss design.
5. In order to continue work on this structure, a structural evaluation by a professional architect or engineer will be required to be preformed to meet the requirement of chapter 34 of the Florida Building Code sections 3401.8.2.3.1, 3401.8.2.3.2 and 3401.8.2.3.3.

If you should have any question please contact me.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

**associated florida architects, inc.**
florida registration AA-C001907
802 nw 23rd avenue • gainesville, florida 32609-3534
352 375-3005 • 352 378-2523 • 352 375-5397 fax

July 22, 2005

Columbia County Building Department
135 NE Hernando Ave.
Lake City, FL 32055
Re: Ed & Diane Bishop-white Residence

To Whom It May Concern:

We are the architects for the referenced project. We have a full service contract with the owners including contract administration.

Due to the fact that this is an addition we will be able to do the discovery work on the existing residence when construction commences.

We will write a report with findings and recommendations within forty-five (45) days of start of construction. As per 3401.8.2.3.1, 3401.8.2.3.2 and 3401.8.2.3.3 of 2001 Florida Building Code.

Robert S. Taylor, Sr. - Architect
Florida Registration AR-0007526

**3401.8.1.2** The requirements of this section shall not supersede specific requirements of the code for construction in Fire Zones.

**3401.8.2 Additions**

**3401.8.2.1** Any addition or alterations increasing the floor area of the building, shall meet the requirements of this section. For purposes of this section, whether an addition falls within the stated percentages shall be calculated based on the cumulative increase of the building during the course of one calendar year.

**3401.8.2.2** All except Group R3 occupancies shall comply with the following:

**3401.8.2.2.1** When additions, or alterations increasing floor area, are made to an existing building, and the addition and existing building are separated by a fire rated wall, as defined in Section 704, the addition shall conform to all the requirements of the code applicable to a building of the area of the addition.

**3401.8.2.2.2** Where the existing building and the addition are not separated by a fire rated wall and the area of the addition is 25 percent or more of the area of the existing building, the existing building and the addition shall be made to comply with all requirements of the code for a building of area equal to the combined area for the addition and existing building.

**3401.8.2.2.3** Where the existing building and the addition are not separated by a fire rated wall or where the addition is vertically superimposed on an existing building, and the area of the addition is less than 25 percent of the area of the existing building, the following requirements shall apply:

1. The addition shall conform to all requirements of the code applicable to a building having the combined area and height of the existing building and the addition.
2. The existing building shall conform to all requirements of the means of egress for a building of the combined area and height of the addition and the existing building.
3. An approved detection, alarm and communications system, detecting products of combustion, shall be required for all public areas and means of egress within the existing building.

**3401.8.2.3** Group R3 Occupancies shall comply with the following:

**3401.8.2.3.1** When additions, or alterations increasing floor area, are made to an existing building and the addition constitutes 25 percent or more of the area of the existing building, the addition shall be made to comply with all the requirements of the code and the existing building shall comply with the following:

1. Impact resistance devices having a valid NOA shall be installed at openings to provide protection against storms.
2. Corners of buildings of masonry construction shall be checked for tie downs. If tie downs are not found in corners, testing shall be performed to locate tie downs in all walls. Proper installation of tie downs shall be done at 20 foot intervals and at each corner except that interior tie downs may be provided in each side not less than 2 feet on each side of each corner.
   2.1 Tie down refers to the anchorage from the foundation to the tie beam and shall provide the equivalent strength of a vertical #5 reinforcing bar properly attached to the foundation and tie beam encased in concrete or mortar and lapped a minimum of 30 inches or otherwise spliced in a manner which will develop the full strength of the bar.
   2.2 Alternate methods of providing anchorage of equivalent strength to that described in 2.1 may be used where design calculations which admit rational analysis are submitted by a Registered Engineer or Architect proficient in structural design.
3. Roof anchorage shall be checked at all walls where the addition connects to the existing building. If major deficiencies are found and the anchorage is not in compliance with the minimum requirements of the code, the roof anchorage shall be checked for all the existing roof. Minimum anchorage shall be provided to each member bearing on the exterior walls.
4. Permanent roof bracing shall be provided at all gable ends.
5. G.F.C.I. outlets shall be installed where required by the code.
6. Smoke detectors shall be installed where required by the code.

**3401.8.2.3.2** The design professional shall conduct a site visit to ascertain the necessary work to be performed to comply with the requirements of this Section.

**3401.8.2.3.3** The design professional shall provide an inspection report and indicate on the drawings all remedial actions to be performed on the building submittal as a part of the permit plans.

**3401.8.3 Repairs and alterations**

**3401.8.3.1** Repairs and alterations not increasing the area of the building, made within any 12 month period, shall meet the requirements of this section.