DATE 10/04/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000023688

APPLICANT BRENDA HAYGOOD PHONE 397-6348

ADDRESS 12592 S US HIGHWAY 441 LAKE CITY FL 32025

OWNER JERRY & MARTHA LEVERETT PHONE

ADDRESS 140 SE SEARS COURT LAKE CITY FL 32025

CONTRACTOR HAYGOOD HOMES PHONE 752-3496

LOCATION OF PROPERTY 41S, TL ON CR 238, TR ON OCTOBER, TL ON PLEASURE, AROUND CURVE, TO SEARS COURT ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 100000.00

HEATED FLOOR AREA 2000.00 TOTAL AREA 3093.00 HEIGHT .00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 19

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 02-6S-17-09553-033 SUBDIVISION ROLLING HILLS UNREC

LOT 21 BLOCK PHASE UNIT TOTAL ACRES

000000835 CRC1326715 Brenda Haygood

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

CULVERT 05-0906-N BK Y

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD,

Check # or Cash 2206

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 500.00 CERTIFICATION FEE $ 15.47 SURCHARGE FEE $ 15.47

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 630.94

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

23688
Leverett Job

Leverett Residence, Columbia County FL

Windload/Structural Requirements Addendum/Modification

(In Compliance with the 2004 Florida Building Code)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

At the request of the contractor I have reevaluated the structural requirements for the residence for closing in the rear porch area as a sunroom. The rear porch may be closed in as a sunroom. Also, the windows may be placed adjacent to the porch columns. Existing shearwall lengths in the structure is sufficient for this change.

Marty J. Humphries

12-16-05

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 02-6S-17-09553-033 **Building permit No.** 000023688

Use Classification SFD,UTILITY **Fire:** 41.44

Permit Holder HAYGOOD HOMES **Waste:** 85.75

Owner of Building JERRY & MARTHA LEVERETT **Total:** 127.19

Location: 140 SE SEARS COURT, LAKE CITY, FL 32025

Date: 03/31/2006

John D. Kerce

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

23688

THIS INSTRUMENT PREPARED BY
AND RETURN TO:
TITLE OFFICES, LLC
1089 SW MAIN BLVD.
LAKE CITY, FLORIDA 32025

Parcel I.D. #: ***09553-033***

Inst:2005024375 Date:10/03/2005 Time:14:38
DC,P.DeWitt Cason,Columbia County B:1050 P:

SPACE ABOVE THIS LINE FOR PROCESSING DATA *SPACE ABOVE THIS LINE FOR RECORDING DATA*

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713.13, Florida Statutes, the following information is provided in this Notice of Commencement. This Notice shall be void and of no force and effect if construction is not commenced within ninety (90) days after recordation.

1. Description of property: (Legal description of property, and street address if available)

SOUTHEAST PLEASURE DRIVE, LAKE CITY, FL 32025
COMMENCE AT THE SOUTHEAST CORNER OF SECTION 2, TOWNSHIP 6 SOUTH, RANGE 17 EAST AND RUN S 88°11'41" W ALONG THE SOUTH LINE OF SAID SECTION 2 A DISTANCE OF 235.36 FEET TO THE WESTERLY RIGHT OF WAY LINE OF A 50 FOOT ROAD DEDICATION AS RECORDED IN O.R. BOOK 330, PAGE 285 FOR A POINT OF BEGINNING: THENCE N 11°18'19" W ALONG SAID WESTERLY RIGHT OF WAY 366.32 FEET; THENCE S 85°51'56" W ALONG THE SOUTHERLY LINE OF SAID RIGHT OF WAY 315.61 FEET; THENCE S 01°38'45" E, 348.43 FEET TO THE SOUTH LINE OF SAID SECTION 2; THENCE N 88°11'41" E, 376.78 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.

2. **General description of improvement: construction of single family dwelling**

3. Owner information:
   a. Name and address:
   JERRY L. LEVERETT and MARTHA ANN LEVERETT
   1078 SE GILES MARTIN AVE, LAKE CITY, FL 32024
   b. Interest in property: **Fee Simple**
   c. Name and Address of Fee Simple Titleholder (if other than owner):

4. Contractor: (Name and Address)
HAYGOOD HOMES, INC.
12592 S. US HWY 441, LAKE CITY, FL 32025
Telephone Number: **386-752-3496**

5. Surety (if any):
   a. Name and Address:
   Telephone Number: _____
   b. Amount of Bond $_____

6. Lender: (Name and Address)
CAMPUS USA CREDIT UNION
2511 NW 41ST STREET, GAINESVILLE, FLORIDA 32606
Telephone Number: **754-9088**

7. Persons within the State of Florida designated by Owner upon whom notice or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: (Name and Address)
N/A

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: (Name and Address)
CAMPUS USA CREDIT UNION
2511 NW 41ST STREET, GAINESVILLE, FLORIDA 32606
Telephone Number: **754-9088**

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

_____ {SEAL}
JERRY L. LEVERETT
BY MARTHA A. LEVERETT, ATTORNEY-IN-FACT

_____ {SEAL}
MARTHA ANN LEVERETT

Sworn to and subscribed before me this **30th** day of **September, 2005,** by **MARTHA A. LEVERETT as ATTORNEY-IN-FACT for JERRY L. LEVERETT and MARTHA ANN LEVERETT as INDIVIDUAL,** who are personally known to me or who have produced _____ as identification.

_____
Notary Public
My Commission Expires: _____

Bonita Hadwin
MY COMMISSION # DD236004 EXPIRES
August 10, 2007
BONDED THRU TROY FAIN INSURANCE, INC.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0509-33 Date Received 9/9/05 By JA Permit # 835 / 23688

Application Approved by - Zoning Official BLK Date 20.09.05 Plans Examiner OK JTH Date 9-15-05

Flood Zone X Development Permit N/A Zoning A3 Land Use Plan Map Category A-3

Comments - NOC -

Lic # 752 3496

Applicants Name Brenda Haygood Phone 386-397-6348

Address 12592 S. US Hwy 441 Lc 32025

Owners Name Jerry L. and Martha A. Leverett Phone

911 Address 140 SE Sears ct, L. C. 32025

Contractors Name Haygood Homes, Inc Phone 386-752-3496 home
386-303-1981 - cell

Address 12592 S. US Hwy 441 LC 32025

Fee Simple Owner Name & Address Campus USA -

Bonding Co. Name & Address

Architect/Engineer Name & Address Pat Haygood - Monty Humphries & Engineering

Mortgage Lenders Name & Address Campus - USA

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 02-65-17-09553-033 Estimated Cost of Construction 208,000.

Subdivision Name Soldiers Hills Unrec Lot 31 Block Unit Phase

Driving Directions Highway 41 South, Turn Left on CR238, Turn Right on October, Turn Left on Pleasure, around curve (Sears Court), Lot on Right

(map attached)

Type of Construction new home Number of Existing Dwellings on Property 0

Total Acreage 2.820 acres Lot Size 340 X 376 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 140' Side 100' Side 166' Rear 154'

Total Building Height 19 ft Number of Stories 1 Heated Floor Area 2000 Roof Pitch 6/12

Porches 549 GARAGE 544 TOTAL 3093

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CRC 1326715

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

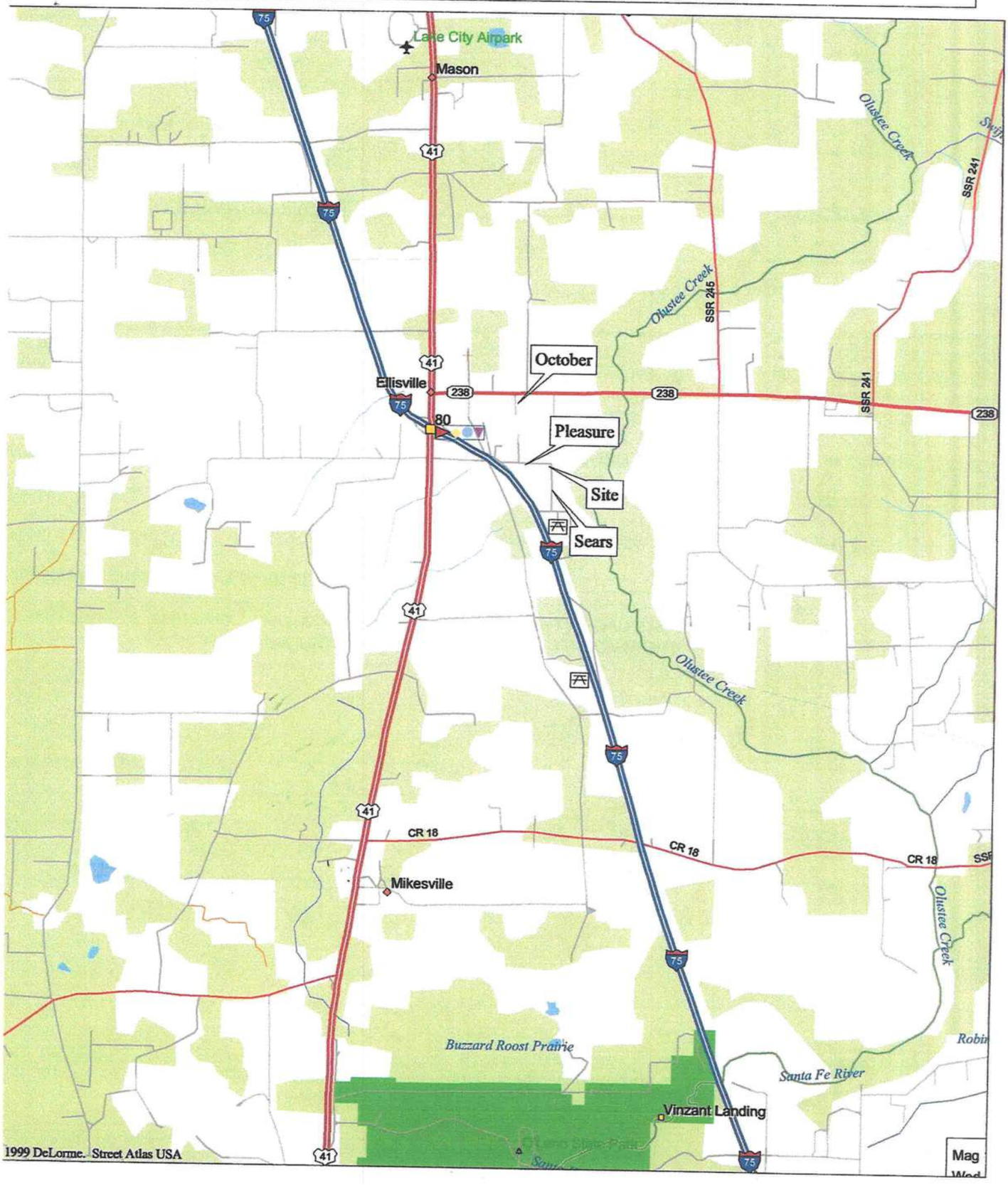
Sworn to (or affirmed) and subscribed before me this 31 day of August 2005.

Personally known ✓ or Produced Identification

Notary Signature

Donna S. Higgs
MY COMMISSION EXPIRES March 31, 2007
#DD184369
Bonded thru Troy Fain-Insurance
NOTARY PUBLIC, STATE OF FLORIDA

Jerry & Martha Leverett

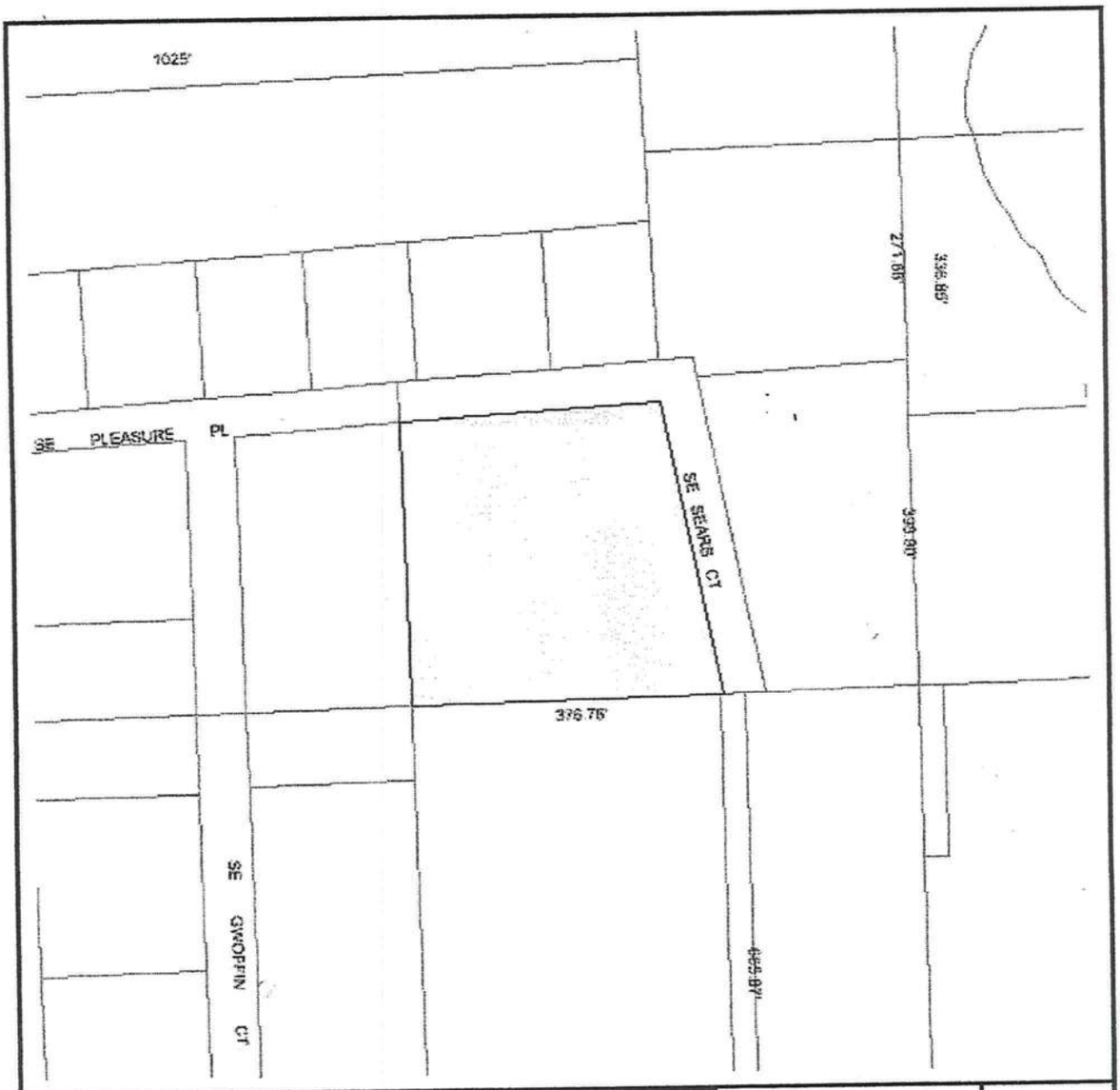


@ CAM112M01 S CamaUSA Appraisal System Columbia County
9/09/2005 13:13 **Legal Description Maintenance** **15228 Land 001**
Year T Property Sel **AG 000**
2005 R 02-6S-17-09553-033 ... **Bldg 000**
Xfea 000
LEVERETT JERRY L & MARTHA ANN **15228 TOTAL B**

COMM SE COR OF SEC, RUN W 235.36 FT FOR POB, RUN N
366.32 FT, W 315.61 FT, S 348.43 FT TO S LINE OF SEC, E
376.78 FT TO POB. ORB 375-362, (AKA LOT 21 ROLLING HILLS UNR)
WD 1026-1556, WD 1037-1214.

Mnt **2/17/2005 KYLIE**

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More



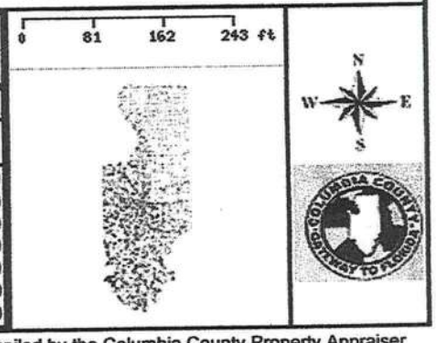
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 02-6S-17-09553-033 - NO AG ACRE (009900)

COMM SE COR OF SEC, RUN W 235.36 FT FOR POB, RUN N 366.32 FT, W 315.61 FT, S

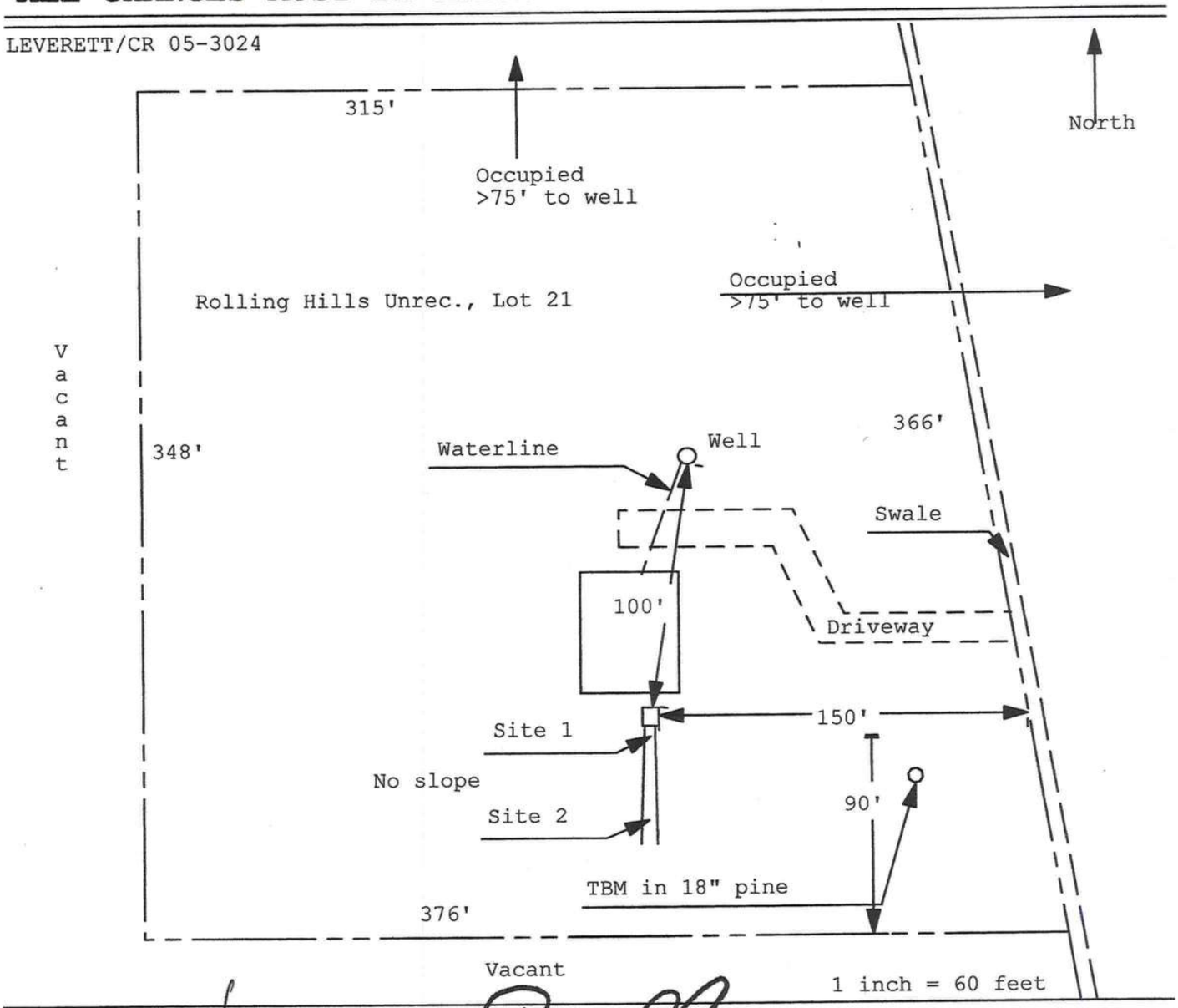
Name:	LEVERETT JERRY L & MARTHA ANN	LandVal	$15,228.00
Site:		BldgVal	$0.00
Mail:	P O BOX 1421 APO, AP 96555	ApprVal	$15,228.00
		JustVal	$15,228.00
Sales	2/4/2005 $22,500.00 V / Q	Assd	$15,228.00
Info	9/22/2004 $20,000.00 V / Q	Exmpt	$0.00
		Taxable	$15,228.00



Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 05-0906N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul Lloyd **Date** 8/24/05

Plan Approved ✓ **Not Approved** ____ **Date** 9/01/05

By R. Reames REII Columbia **CPHU**

Notes: ____

COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: August 16, 2005

ENHANCED 9-1-1 ADDRESS:

140 SE SEARS CT (LAKE CITY, FL 32025)

Addressed Location 911 Phone Number: NOT AVAIL.

OCCUPANT NAME: NOT AVAIL.

OCCUPANT CURRENT MAILING ADDRESS:

PROPERTY APPRAISER PARCEL NUMBER: 02-6S-17-09553-033

Other Contact Phone Number (If any):

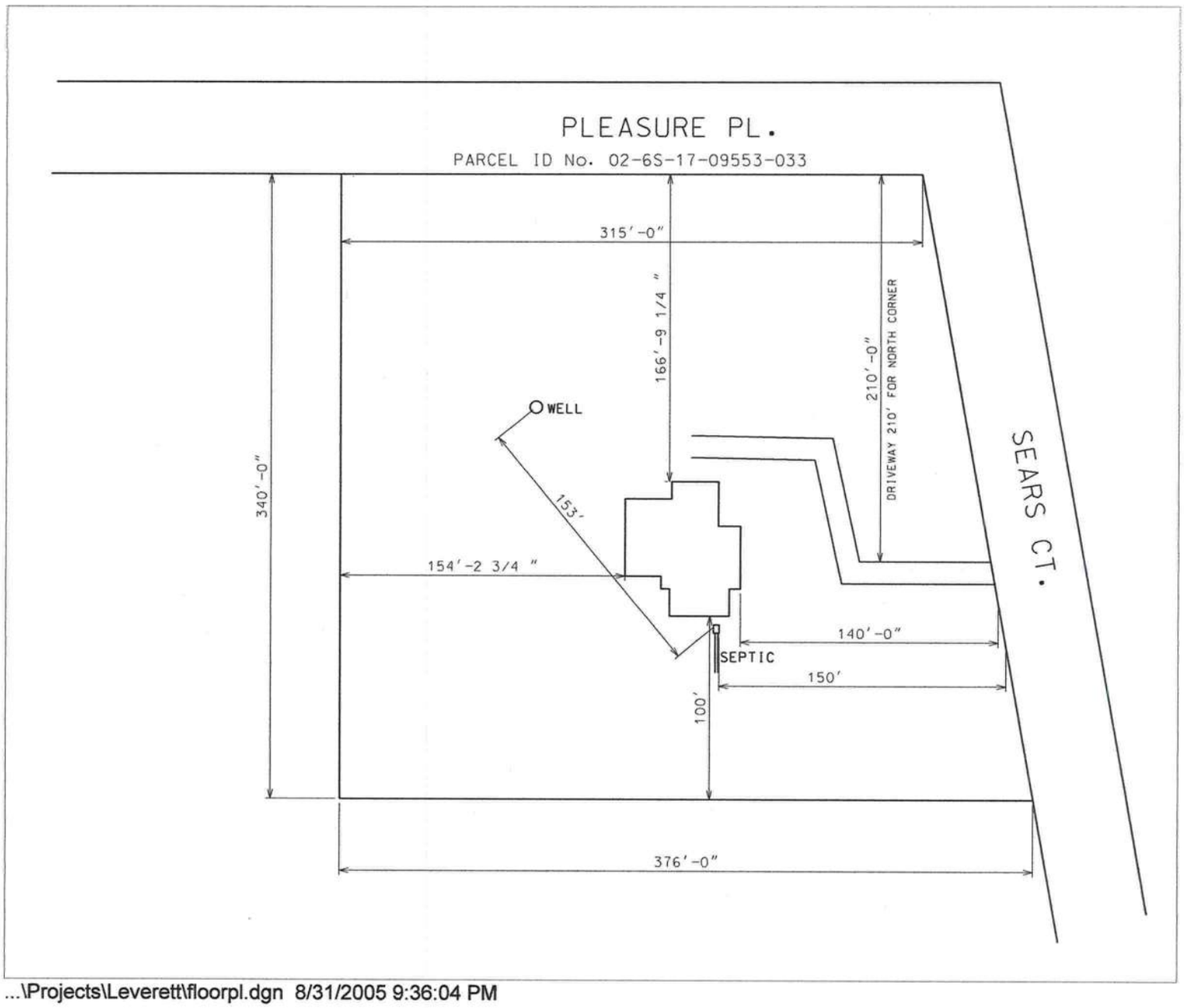
Building Permit Number (If known):

Remarks:

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED



...\Projects\Leverett\floorpl.dgn 8/31/2005 9:36:04 PM

This Instrument Prepared by & return to:
Name: ***tapeadmin, an employee of TITLE OFFICES, LLC***
Address: ***1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025***
File No. 05Y-01132NM

Parcel I.D. #: ***09553-033***

SPACE ABOVE THIS LINE FOR PROCESSING DATA

Inst:2005002847 Date:02/07/2005 Time:15:47
Doc Stamp-Deed : 157.50
DC,P.DeWitt Cason,Columbia County B:1037 P:1214

SPACE ABOVE THIS LINE FOR RECORDING DATA

THIS WARRANTY DEED

Made the ***4th*** *day of* ***February****, A.D.* ***2005****, by* ***BILL BYRD****, A MARRIED MAN, hereinafter called the grantor, to* ***JERRY L. LEVERETT*** *and* ***MARTHA ANN LEVERETT****, HIS WIFE, whose post office address is* ***P.O. BOX 1421, APO AP 96555****, hereinafter called the grantees:*

(Wherever used herein the terms "grantor" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantor, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in* ***Columbia County, State of FLORIDA****, viz:*

COMMENCE AT THE SOUTHEAST CORNER OF SECTION 2, TOWNSHIP 6 SOUTH, RANGE 17 EAST AND RUN S 88°11'41" W ALONG THE SOUTH LINE OF SAID SECTION 2 A DISTANCE OF 235.36 FEET TO THE WESTERLY RIGHT OF WAY LINE OF A 50 FOOT ROAD DEDICATION AS RECORDED IN O.R. BOOK 330, PAGE 285 FOR A POINT OF BEGINNING: THENCE N 11°18'19" W ALONG SAID WESTERLY RIGHT OF WAY 366.32 FEET; THENCE S 85°51'56" W ALONG THE SOUTHERLY LINE OF SAID RIGHT OF WAY 315.61 FEET; THENCE S 01°38'45" E, 348.43 FEET TO THE SOUTH LINE OF SAID SECTION 2; THENCE N 88°11'41" E, 376.78 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.

THE ABOVE DESCRIBED PROPERTY IS NOT THE HOMESTEAD PROPERTY OF THE GRANTOR.

Together *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.*

To Have and to Hold *the same in fee simple forever.*

And *the grantor hereby covenants with said grantees that he is lawfully seized of said land in fee simple; that he has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.*

In Witness Whereof, *the said grantor has signed and sealed these presents, the day and year first above written.*

Signed, sealed and delivered in the presence of:

Martha Bryan
Witness Signature
MARTHA BRYAN

X Bill Byrd L.S.
BILL BYRD
Address:

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
~~XXX NORTH FIRST STREET~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

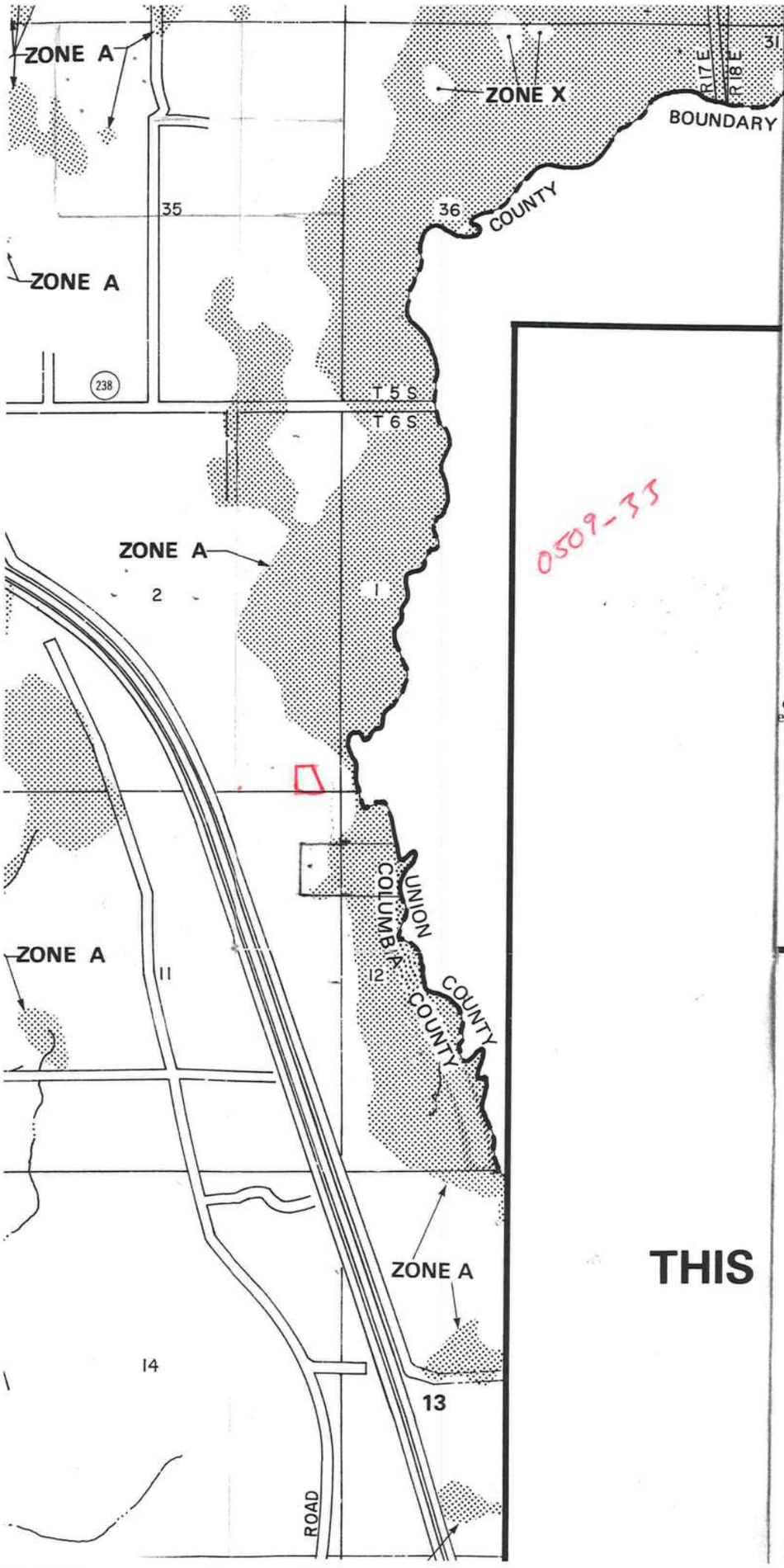
NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk



THIS

rporate limits shown are current as of the date of this map.
er should contact appropriate community officials to deter
corporate limits have changed subsequent to the issuance of
ap.

adjoining panels, see separately printed Map Index.

MAP REPOSITORY

unty Coordinator's Office, County Courthouse, Lake City, Flo
aps available for reference only, not for distribution.)

INITIAL IDENTIFICATION:
JANUARY 20, 1978

FLOOD HAZARD BOUNDARY MAP REVISIONS:

FLOOD INSURANCE RATE MAP EFFECTIVE:

JANUARY 6, 1988

FLOOD INSURANCE RATE MAP REVISIONS:

determine if flood insurance is available, contact an insura
ent or call the National Flood Insurance Program at (800) 638-6

APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PRO

FIRM

FLOOD INSURANCE RATE M

COLUMBIA
COUNTY,
FLORIDA
(UNINCORPORATED AREA

PANEL 250 OF 290

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600B-04 — **Residential Component Prescriptive Method B** — **NORTH 1 2 3**

Compliance with Method B of Subchapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single-and multiple-family residences of three stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptives listed in this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the code.

PROJECT NAME AND ADDRESS:	Leverett Residence 140 SE Sears Ct Lake City 32025	**BUILDER:** Haygood Homes, Inc	
		PERMITTING OFFICE:	**CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☑
OWNER: Jerry L. & Martha A. Leverett		**PERMIT NO.:** 23688	**JURISDICTION NO.:** 221000

1. New construction including additions which incorporate any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other nonvertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the code. Circle the column of the package you have choosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on "Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages," Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. **Compliance package chosen (A-E)**	1. B	
2. **New construction or addition**	2. new	
3. **Single-family detached or multiple-family attached**	3. Single	
4. **If multiple-family–No. of units covered by this submission**	4.	
5. **Is this a worst case? (yes/no)**	5. yes	
6. **Conditioned floor area (sq. ft.)**	6. 2000	
7. **Predominant eave overhang (ft.)**	7. 1'6"	
8. **Glass type and area:**	Single Pane / Double Pane	
a. Clear glass	8a. ____ sq. ft. 230 sq. ft.	
b. Tint, film or solar screen	8b. ____ sq. ft. 0 sq. ft.	
9. **Percentage of glass to floor area**	9. .11 %	
10. **Floor type, area or perimeter, and insulation:**		
a. Slab-on-grade (*R*-value)	10a R=____ 272 lin. ft.	
b. Wood, raised (*R*-value)	10b. R=____ ____ sq. ft.	
c. Wood, common (*R*-value)	10c. R=____ ____ sq. ft.	
d. Concrete, raised (*R*-value)	10d. R=____ ____ sq. ft.	
e. Concrete, common (*R*-value)	10e. R=____ ____ sq. Ft.	
11. **Wall type, area and insulation:**		
a. **Exterior:** 1. Masonry (Insulation R-value)	11a-1 R=____ ____ sq. ft.	
2. Wood frame (Insulation *R*-value)	11a-2 R=13 2176 sq. ft.	
b. **Adjacent:** 1. Masonry (Insulation *R*-value)	11b-1 R=____ ____ sq. ft.	
2. Wood frame (Insulation *R*-value)	11b-2 R=____ ____ sq. ft.	
12. **Ceiling type, area and insulation:**		
a. Under attic (Insulation *R*-value)	12a. R=30 sq. ft. 2000	
b. Single assembly (Insulation *R*-value)	12b. R=____ ____ sq. ft.	
13. **Air distribution system: Duct insulation, location**	13. R=6 ____	
Test report (attach if required)		
14. **Cooling system:**	14a. Type: Central	
(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 12	
	14c. Capacity: 3 TON	
15. **Heating system:**	15a. Type: Heat Pump	
(Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE: ____	
	15c. Capacity: 3 ~~TON~~ 50gal	
16. **Hot water system:**	16a. Type: Elec	
(Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)	16b. EF: .92	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: [signature] **DATE:** 8/31/05

I hereby certify that this building is in compliance with the Florida Energy Code:

OWNER AGENT: [signature] **DATE:** 8/31/05

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

TABLE 6B-1 — MINIMUM REQUIREMENTS — Climate Zones 1 2 3

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION					TO BE INSTALLED
		A	B	C	D	E	
GLASS	Max. % of Glass to Floor Area	15%	15%	20%	20%	25%	15 %
	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)	DC: ☑ DT: ☐
	Overhang	1'4"	2'	2'	2'	2'	1.6 FEET
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.					EXT: R = ADJ: R = COM: R =
	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD-FRAME WALLS R-11					EXT: R = 13 ADJ: R = COM: R =
CEILINGS		R-30	R-30	R-30	R-30	R-30	UNDER ATTIC: R= 30 COMMON: R:
		(NO SINGLE ASSEMBLY CEILINGS ALLOWED)					
FLOORS	Slab-On-Grade	R-0					R = 0
	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)					R =
	Raised Concrete	R-7					R =
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED	R = 6 COND. ☐
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0	SEER = 12
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4	HSPF = 7.9
	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)					AFUE =
HOT WATER SYSTEM	Electric Resistance**	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	EF = .92
	Gas & Oil**	MINIMUM EF OF .59				NATURAL GAS ONLY (SEE BELOW)	EF =
	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.					DHP: ☐ EF = HRU: ☐ SOLAR: ☐ EF =

* Single package units minimum SEER=9.7, HSPF = 6.6.
** Minimum efficiencies for gas and electric hot water systems apply to 40 gallon water heaters. Refer to Table 612.1 ABC.3.2 for minimum code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horzontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions: 1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multistory house.

Wall, Ceiling and Floor Insulation Values: The *R*-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The *R*-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multiple-family building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a state-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTACs equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of Section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed," an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options." See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having and EF of .92 or greater, or natural gas systems with EF .59 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multistory Houses	606.1	Air barrier on perimeter of floor cavity between floors.	N/A
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker electric or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Noncommercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	✓
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	N/A
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 psig.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000835

DATE 10/04/2005 PARCEL ID # 02-6S-17-09553-033

APPLICANT BRENDA HAYGOOD PHONE 397-6348

ADDRESS 12592 S. US HIGHWAY 441 LAKE CITY FL 32025

OWNER JERRY & MARTHA LEVERETT PHONE

ADDRESS 140 SE SEARS COURT LAKE CITY FL 32025

CONTRACTOR HAYGOOD HOMES PHONE 752-3496

LOCATION OF PROPERTY 41S, TL ON CR238, TR ON OCTOBER, TL ON PLEASURE, AROUND CURVE TO SEARS COURT, LOT ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT ROLLINGHILLS UNREC 21

SIGNATURE Brenda Haygood

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY · GATEWAY TO FLORIDA ·

Leverett Residence
Columbia County FL
Wind Load Analysis Requirements
(In Compliance with the 2004 Florida Building Code)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Description of New Residence:
Footprint: 70'8" x 60' overall with a 6' x 31'6" covered front porch and a 20'2" x 19'8" rear porch (see Haygood Homes plan 0506)
Walls: 2x4-16" O.C. with 7/16" OSB sheathing minimum with hardiboard lap siding and ½"gypsum wall board interior.
Roof Structure: Pre-engineered roof trusses and 1/2" OSB sheathing minimum
Roof Type: Gable construction (analyzed for 2' eave overhang and porch areas)
Foundation: footer with stemwall, with slab construction

Windload Data and Exposure:
Basic Wind Speed = 110 mph
Importance Factor = 1.0
Exposure category = B
Height and Exposure Adjustment Coefficient = 1.0
Residential Occupancy = Group R3
Analysis Method = FBC 1609.6 - Simplified Provisions for Low Rise Buildings
(see tables 1609.6A, 1609.6B, 1609.6C and 1609.6E for wind pressure values)
Mean roof height = 15'
Roof Cross Slope = 3.5:12, 6:12 & 5:12
Eave Overhang= (Analyzed for 2' overhang and porches)
Wall Height = 8'
Shear Wall locations = exterior walls only(all walls 3' in length or greater)
Bracing method for gable locations = framing from wall to roof diaphragm(see attached detail)

Nailing Pattern Requirements:

Wall sheathing:	Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center around edges(including around doors and windows) and 6" on center interior. Full depth blocking shall be installed At horizontal joints in sheathing.
Roof sheathing:	Shall be 1/2" OSB minimum nailed with 8d common nails 3" on center at panel ends and overhangs and 6" on center elsewhere.
Top wall plate:	Nail with 1-16d common nail 12" O.C.(average)

Marty J. Humphries
8-22-05

Strapping and Anchor Requirements:

truss to exterior wall plate and porch beam locations: install one Simpson model H10 hurricane anchor at each location

wall strap tie requirements: at top and bottom of wall install one Simpson model SP4 at each side of each door and window 4' or less in width. At top and bottom of wall for windows and doors larger than 4' in width install two Simpson model SP4's each side of each opening. At each side of garage door opening install three Simpson model SPH4's top and bottom of the wall. All other wall locations install SP4's top and bottom of wall 4' on center.

Porch Columns: Install Simpson model ABU44and d Simpson model AC4Max or (ACE4Max may be used for end columns)

Lookouts: Install one Simpson model H5 where lookouts connect to end gable truss.

Gable end: Install one LSTA18 - 4' on center connecting gable end truss to wall framing.

Gable End Bracing Requirements:

At each gable end install one 2x4 SPF 8' stud spaced 6' on center horizontal along top of bottom chord of trusses, nail with 2-12d nails at each truss including end truss. In addition, install a 2x4 brace extending from this stud at the gable end truss approx. 45 degrees to truss at roof sheathing, nail with 2 -12d nails where it crosses truss members and at ends. Gable end trusses shall be built to receive sheathing with vertical members 2' on center. Vertical members of gable end truss greater than 5' in height shall be stiffened with one 2x4 SPF nailed with 12d nails 8" on center to back of vertical member. (See attached detail)

Foundation Requirements:

Stemwall: Minimum size of footer shall be 10" x 20" wide with 2-#5 rebar continuous and 1-#5 vertical rebar 48" on center. All cells shall be filled with concrete. ½" anchor bolts with 2" washers shall be installed 3' on center and 9" from corners each way and at each side of door openings. (3000 psi concrete min.)(Note: foundation designed using an allowable bearing pressure of 1000 psf)

Header Requirements:

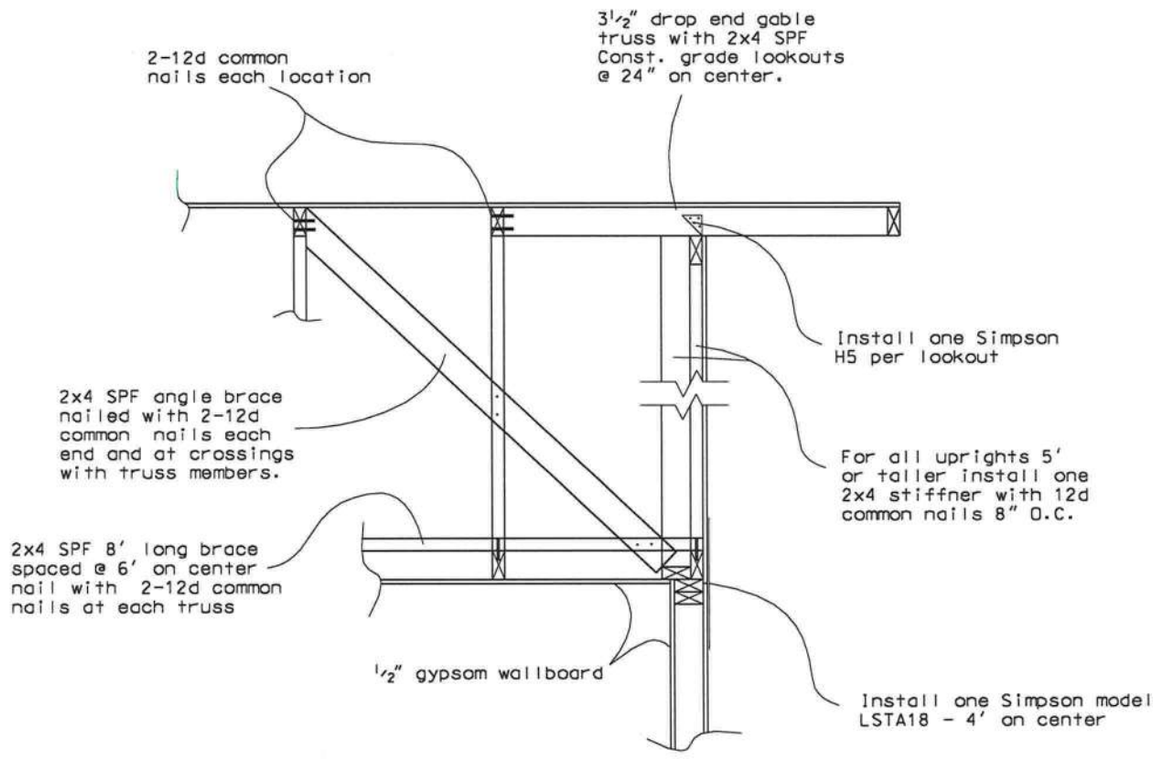
Windows & Doors: Minimum header shall be 2 - #2 SYP 2x12's with ½" plywood/OSB between.

Garage Door: Minimum header shall be 2 - #2 SYP 2x12's with ½" plywood/OSB between. .

Porch Beams: Minimum header shall be 2-#2 SYP 2x10's with ½" plywood/OSB between

Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

Marty D. Hoy
8-22-05



GABLE END BRACING DETAIL (N.T.S.)

Marty J. Humphries
8-22-05

Leverett Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071

SIMPSON Strong-Tie CONNECTORS

NEW! The H2.5A is symetrically designed for easy installation, with higher uplift loads to meet new code requirements. A placement mark allows easy installation on double top plates.

NEW! The H5A has an installed cost benefit, as it only requires 6 nails, to meet lower uplift requirements.

The H connector series provides wind and seismic ties for trusses and rafters.

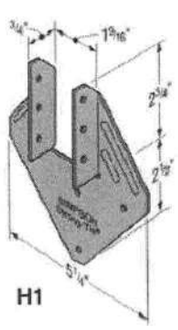
Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
Design Shear/Allowable Shear + Design Tension/Allowable Tension < 1.0.

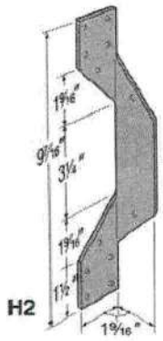
MATERIAL: See table

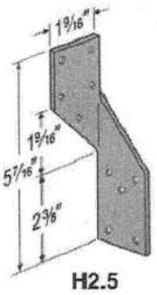
FINISH: Galvanized; H10-2, H11Z–Z-MAX. Other models available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

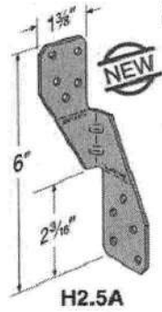
INSTALLATION:
- Use all specified fasteners. See General Notes.
- H1 can be installed with flanges facing outwards (reverse of drawing number 1). When installed inside a wall, a birdsmouth cut is required.
- H2.5, H3, H4, H5 and H6 ties are shipped in equal quantities of rights and lefts.
- Bend the H7 over the top of the truss. Install a minimum of four 8d nails into the truss, including two into the truss side.
- Hurricane Ties do not replace solid blocking.

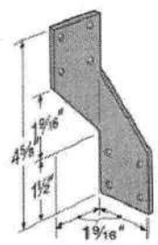
CODES: BOCA, ICBO, SBCCI NER-422, NER-393, NER-432; NER-499; City of L.A. RR 24818; Dade Co. FL 00-0512.11 (H10); 00-0926.01 (H1, H2.5, H3, H4, H5).

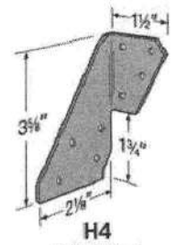

H1

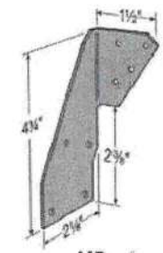

H2

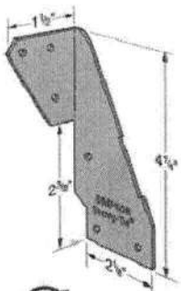

H2.5

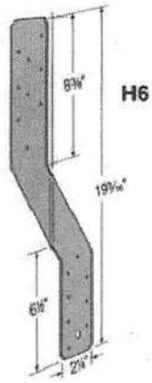

H2.5A

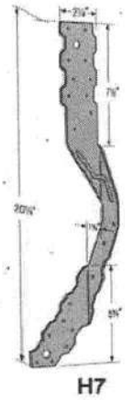

H3

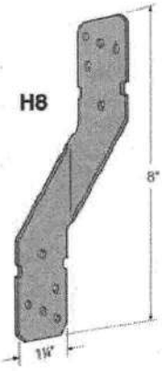

H4
U.S. Patent 4,714,372

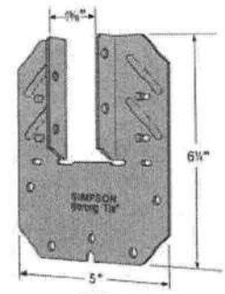

H5
U.S. Patent 4,714,372

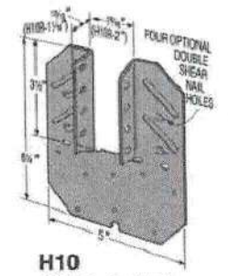

H5A
U.S. Patent 4,714,372


H6

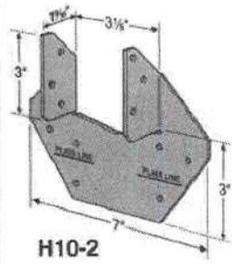

H7

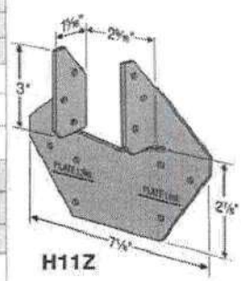

H8


H9


H10
(H10R similar)
U.S. Patents 4,480,941; Canada Patent 1,193,418

Model No.	Ga	Fasteners			Uplift Avg Ult	Doug-Fir Larch/So. Pine Allowable Loads[1,2]				Uplift Load with 8dx1½ Nails (133 & 160)	Spruce-Pine-Fir Allowable Loads[1,2]				Uplift Load with 8dx1½ Nails (133 & 160)
						Uplift		Lateral (133/160)			Uplift		Lateral (133/160)		
		To Rafters/ Truss	To Plates	To Studs		(133)	(160)	F_1	F_2		(133)	(160)	F_1	F_2	
H1	18	6-8dx1½	4-8d	—	1958	490	585	485	165	455	400	400	415	140	370
H2	18	5-8d	—	5-8d	1040	335	335	—	—	335	230	230	—	—	230
H2.5	18	5-8d	5-8d	—	1300	415	415	150	150	415	365	365	130	130	365
NEW H2.5A	18	5-8d	5-8d	—	1793	600	600	110	110	480	520	535	110	110	480
H3	18	4-8d	4-8d	—	1433	455	455	125	160	415	320	320	105	140	290
H4	20	4-8d	4-8d	—	1144	360	360	165	160	360	235	235	140	135	235
H5	18	4-8d	4-8d	—	1485	455	465	115	200	455	265	265	100	170	265
NEW H5A	18	3-8d	3-8d	—	1500	350	420	115	180	290	245	245	100	120	170
H6	16	—	8-8d	8-8d	3983	915	950	650	—	—	785	820	560	—	—
H7	16	4-8d	2-8d	8-8d	2991	930	985	400	—	—	800	845	345	—	—
H8	18	5-10dx1½	5-10dx1½	—	2422	620	745	—	—	—	530	565	—	—	—
H9KT	18	4-SDS¼x1½	5-SDS¼x1½	—	2812	875	875	680	125	—	755	755	680	125	—
H10	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10R	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10-2	18	6-10d	6-10d	—	2447	760	760	455	395	—	655	655	390	340	—
H11Z	18	6-16dx2½	6-16dx2½	—	5097	830	830	525	760	—	715	715	450	655	—

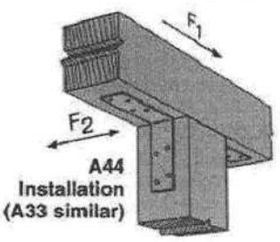

H10-2


H11Z

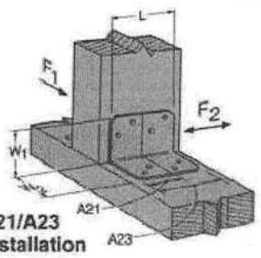
1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).
4. The H9KT is sold in 20 piece packs with screws.
5. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
6. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

Z2 clips secure 2x4 flat blocking between joists or trusses to support sheathing.
MATERIAL: Z clips–see table. A21 and A23–18 ga.; all other A angles–12 ga.
FINISH: Galvanized
INSTALLATION: • Use all specified fasteners. See General Notes.
• Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.
CODES: BOCA, ICBO, SBCCI NER-421 (except A33, A44); City of L.A. RR 25076 (except A33, A44); Dade Co. FL 99-0623.04 (A21 and A23).

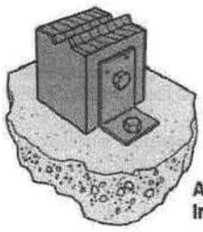
Model No.	Dimensions			Fasteners				Avg Ult F_2	Allowable Loads[2] DF/SP			
				Base		Post			(133)		(160)	
	W_1	W_2	L	Bolts	Nails	Bolts	Nails		F_1	F_2	F_1	F_2
A21	2	1½	1⅜	—	2-10dx1½	—	2-10dx1½	540	245	175	290	175
A23	2	1½	2¾	—	4-10dx1½	—	4-10dx1½	1767	485	485	585	565
A33	3	3	1½	—	4-10d	—	4-10d	2635	625	330	750	330
A44	4⅜	4⅜	1½	—	4-10d	—	4-10d	2490	625	295	750	295
A66	5⅛	5⅛	1½	2-⅜	—	2-⅜	—	N/A	N/A	N/A	N/A	N/A
A88	8	8	2	3-⅜	—	3-⅜	—	N/A	N/A	N/A	N/A	N/A
A24	3⅛	2	2½	1-½	—	1-½	2-10d	N/A	N/A	N/A	N/A	N/A
A311	11	3⅝	2	1-½	—	1-½	4-10d	N/A	N/A	N/A	N/A	N/A

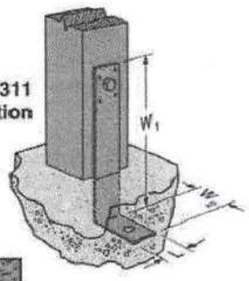


A44 Installation (A33 similar)



A21/A23 Installation

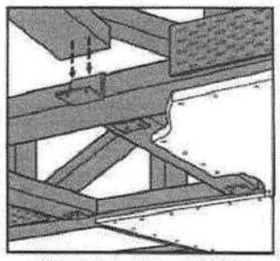

A24 Installation

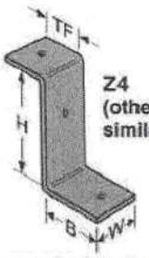


A311 Installation

Model No.	Ga	Dimensions				Fasteners[1] (Total)	Avg Ult	Allowable[2] Download (125)
		W	H	B	TF			
Z2	20	2⅝	1½	1⅜	1⅜	4-10dx1½	1507	465
Z4	12	1½	3½	2⅛	1¼	2-16d	1450	465
Z6	12	1½	5⅜	2	1⅜	2-16d	1517	485
Z28	28	2⅝	1½	1⅜	1⅜	10dx1½[3]	—	—
Z38	28	2⅝	2½	1⅜	1⅜	10dx1½[3]	—	—
Z44	12	2½	3½	2	1⅜	4-16d	2800	865

1. Z28 and Z38 do not have nail holes. Fastener quantities are as required.
2. Allowable loads have been increased 25% for roof loading (Z clips), 33% and 60% for earthquake or wind loading (A angles); no further increase allowed; reduce for other load durations according to the code.
3. Z4 and Z6 loads apply with a nail into the top and a nail into the seat.


Typical Z2 Installation



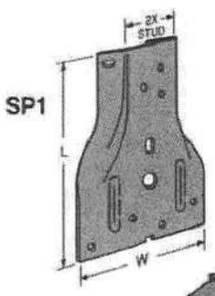
Z4 (others similar)

SP/SPH/RSP4 STUD PLATE TIES

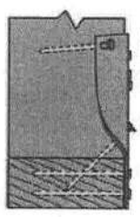
The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.
MATERIAL: SPH–18 gauge, all others–20 gauge **FINISH:** Galvanized
INSTALLATION: • Use all specified fasteners; see General Notes.
• SP–one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.
CODES: BOCA, ICBO, SBCCI NER-432, NER-443, NER-499; SBCCI 9603A; City of LA RR 25318 (RSP4); Dade Co. FL 99-0623.04 (SP1, SP2, SP4, SP6, SP8).

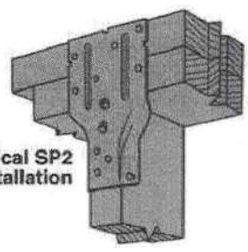
Model No.	Dimensions		Fasteners		Avg Ult	Allowable Uplift Loads DF/SP	
	W	L	Stud[1]	Plate		(133)[2]	(160)[2]
SP1	3½	5⅛	6-10d	4-10d	1950	585	585
SP2	3½	6⅝	6-10d	6-10d	3300	890	1065
SP3	4½	6⅝	6-10d	6-10d	3467	890	1065
SP4	3⅝	7¼	6-10dx1½	—	2917	735	885
SP5	4½	5⅛	6-10d	4-10d	1950	585	585
SP6	5⅛	7¾	6-10dx1½	—	2917	735	885
SP8	7⅛	8⅝	6-10dx1½	—	2917	735	885
SPH4	3⅝	8¾	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH6	5⅝	9¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH8	7⅜	8¾	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
RSP4 (1)	2⅛	4½	4-8dx1½	4-8dx1½	1032	315	315
RSP4 (2)	2⅛	4½	4-8dx1½	4-8dx1½	1445	450	450

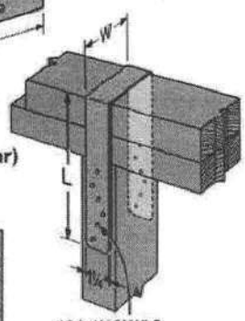
1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).
2. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
3. RSP4–see Installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement



SP1

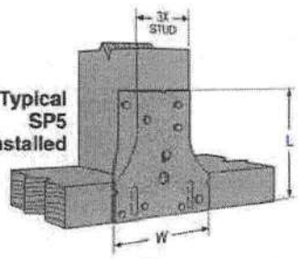

SP1 Nailing Profile


Typical SP2 Installation

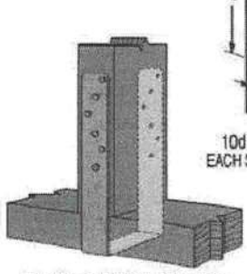


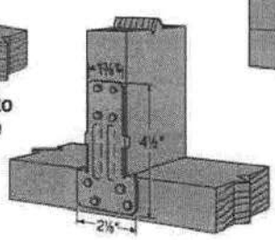
Typical SPH Installation (SP4, 6, 8 similar)

10d×1½" NAILS EACH SIDE OF STUD

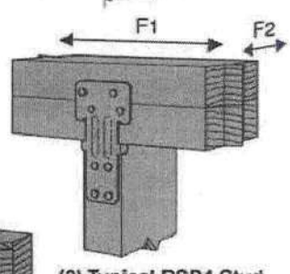


Typical SP5 Installed


Typical SPH4 Stud to Single Bottom Plate



(1) Typical RSP4 Stud to Single Bottom Plate



(2) Typical RSP4 Stud to Double Top Plate (see footnote 4)

The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling. The RPS meets UBC and City of Los Angeles code requirements for notching plates where plumbing, heating or other pipes are placed in partitions.

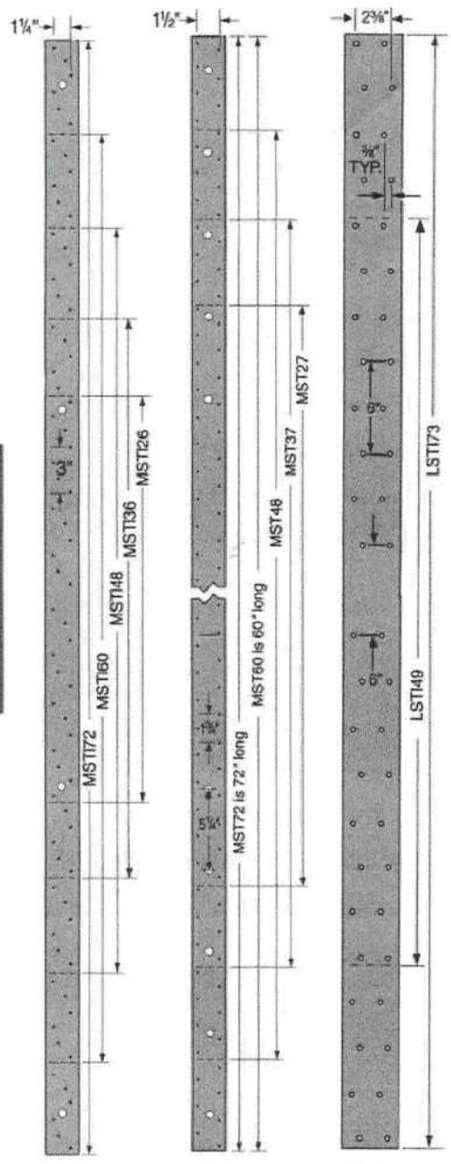
Install Strap Ties where plates or soles are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" center-to-center nail spacing reduces the possibility of splitting. For the MST, this may be a problem on lumber narrower than 3½"; either fill every nail hole with 10dx1½" nails or fill every other nail hole with 16d commons. Reduce the allowable load based on the size and quantity of fasteners used. The LSTI light strap ties are suitable where gun-nailing is necessary through diaphragm decking and wood chord open web trusses.

FINISH: HST–Simpson gray paint; PS–HDG; all others–galvanized. Some products are available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION: Use all specified fasteners. See General Notes.

OPTIONS: Special sizes can be made to order. See also HCST.

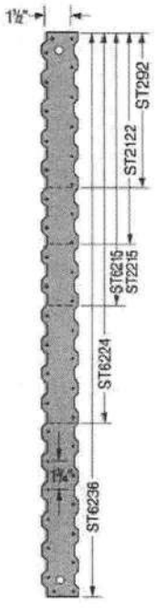
CODES: BOCA, ICBO, SBCCI NER-413, NER-443; ICBO 4935, 5357; Dade County, FL. 00-1023.05 (MSTA30, MSTA36, ST12, ST18, ST22); City of L.A. RR 25119, RR 25149, RR 25281.



MSTI

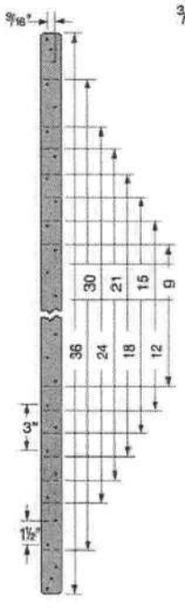
MST

LSTI

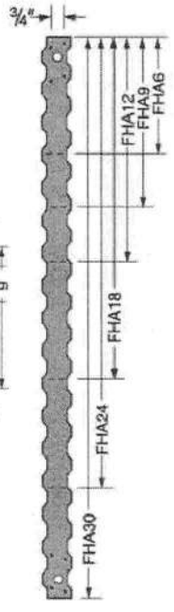


ST
U.S. Patent 4,367,973

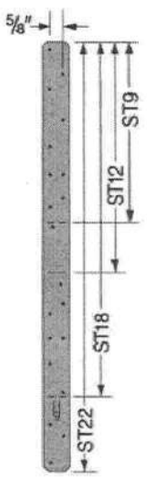
MSTAM36



LSTA and MSTA
(pilot holes not shown)

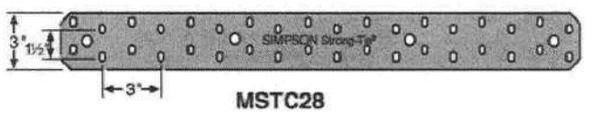


FHA

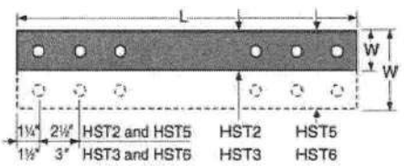


ST9, ST12, ST18, ST22

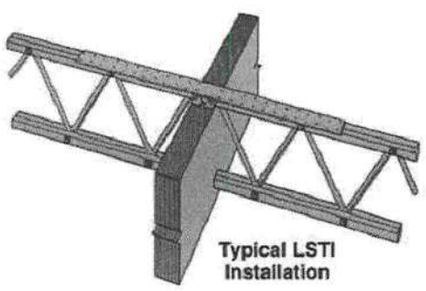
ST2115



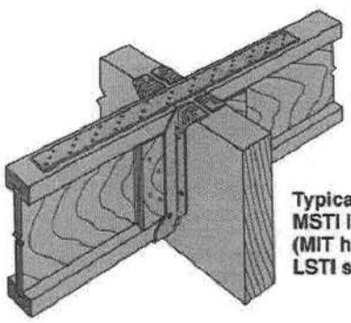
MSTC28



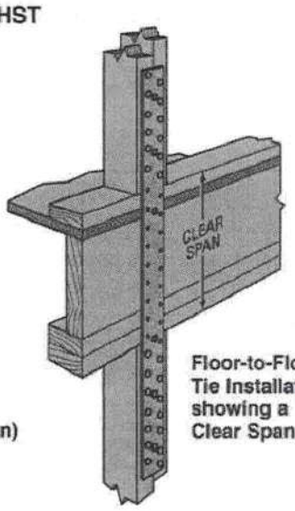
HST



Typical LSTI Installation



Typical MSTI installation (MIT hanger shown) LSTI similar

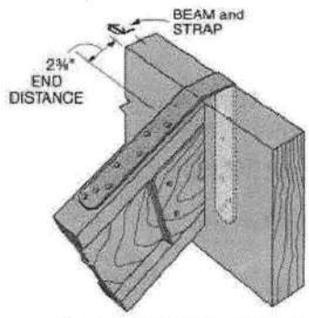


Floor-to-Floor Tie Installation showing a Clear Span

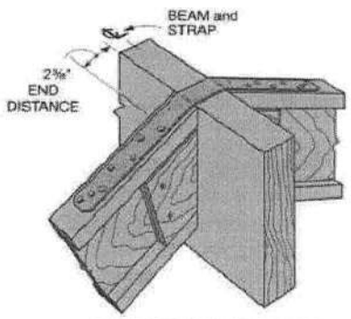
RPS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI

STRAP TIES

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Allowable Tension Loads Floor (100)	(133)	(160)
RPS18	16	1½	18⁵⁄₁₆	12-16d	810	1080	1295
RPS22		1½	22⁵⁄₁₆	16-10d	905	1205	1445
RPS28		1½	28⁵⁄₁₆	12-16d	810	1080	1295
LSTA9	20	1¼	9	8-10d	450	605	725
LSTA12		1¼	12	10-10d	565	755	905
LSTA15		1¼	15	12-10d	680	905	1085
LSTA18		1¼	18	14-10d	790	1055	1265
LSTA21		1¼	21	16-10d	905	1205	1295
LSTA24		1¼	24	18-10d	1015	1295	1295
ST292		2⁹⁄₁₆	9⅝	12-16d	790	1055	1130
ST2122		2⁹⁄₁₆	12¹³⁄₁₆	16-16d	1070	1425	1505
ST2115		¾	16⅜	10-16d	450	600	600
ST2215		2⁹⁄₁₆	16⅜	20-16d	1270	1695	1695
LSTA30	18	1¼	30	22-10d	1255	1670	1715
LSTA36		1¼	36	26-10d	1480	1715	1715
LSTI49		3¾	49	32-10dx1½	1455	1940	2330
LSTI73		3¾	73	48-10dx1½	2185	2910	3495
MSTA9		1¼	9	8-10d	455	610	730
MSTA12		1¼	12	10-10d	570	760	910
MSTA15		1¼	15	12-10d	685	910	1095
MSTA18		1¼	18	14-10d	800	1065	1275
MSTA21		1¼	21	16-10d	910	1215	1460
MSTA24		1¼	24	18-10d	1025	1370	1640
MSTA30	16	1¼	30	22-10d	1265	1685	2025
MSTA36		1¼	36	26-10d	1495	1995	2135
ST6215		2⁹⁄₁₆	16⅜	20-16d	1330	1775	2130
ST6224		2⁹⁄₁₆	23⅛	28-16d	1890	2520	2630
ST9		1¼	9	8-16d	530	705	850
ST12		1¼	11⅝	10-16d	665	885	1065
ST18		1¼	17¾	14-16d	900	1200	1200
ST22		1¼	21⅝	18-16d	1025	1370	1370
MSTC28		3	28¼	36-16d sinkers	2070	2760	3310
MSTC40		3	40¼	52-16d sinkers	2990	3985	4740
MSTC52		3	52¼	62-16d sinkers	3555	4740	4740
MSTC66	14	3	65¾	76-16d sinkers	4390	5855	5855
MSTC78		3	77¼	76-16d sinkers	4390	5855	5855
ST6236		2⁹⁄₁₆	33¹³⁄₁₆	40-16d	2575	3430	3430
FHA6	12	1⁹⁄₁₆	6⅜	8-16d	550	735	885
FHA9		1⁹⁄₁₆	9	8-16d	550	735	885
FHA12		1⁹⁄₁₆	11⅝	8-16d	550	735	885
FHA18		1⁹⁄₁₆	17¾	8-16d	550	735	885
FHA24		1⁹⁄₁₆	23⅞	8-16d	550	735	885
FHA30		1⁹⁄₁₆	30	8-16d	550	735	885
MSTI26		2⁹⁄₁₆	26	26-10dx1½	1130	1510	1810
MSTI36		2⁹⁄₁₆	36	36-10dx1½	1565	2090	2505
MSTI48		2⁹⁄₁₆	48	48-10dx1½	2135	2850	3420
MSTI60		2⁹⁄₁₆	60	60-10dx1½	2760	3680	4415
MSTI72		2⁹⁄₁₆	72	72-10dx1½	3310	4415	4725

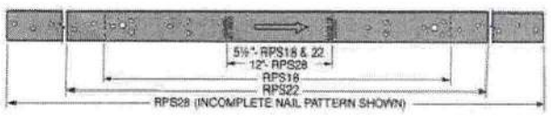


Typical LSTA Installation (hanger not shown)

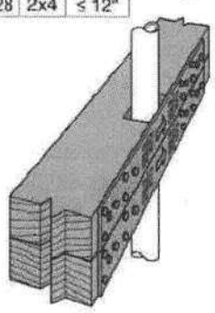


Typical LSTA Installation (hanger not shown)

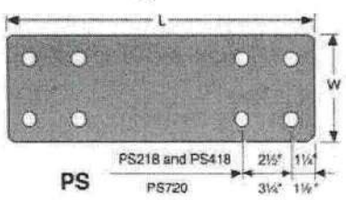
Model No.	Plate	Notch Width
RPS18	2x4	≤ 5½"
RPS22	2x6	≤ 5½"
RPS28	2x4	≤ 12"



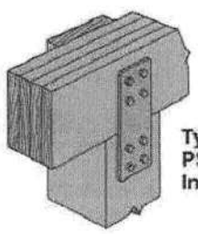
RPS



Typical RPS Installation



PS



Typical PS720 Installation

Model No.	Ga	Dimensions W	Dimensions L	Bolts Qty	Bolts Dia
PS218[8]	7	2	18	4	⅝
PS418[8]		4	18	4	⅝
PS720[9]		6¾	20	8	½

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Load (133)	(160)
MSTC28	18	12-16d sinker	920	1105
	16	16-16d sinker	1225	1470
MSTC40	18	28-16d sinker	2145	2575
	16	36-16d sinker	2455	2945
MSTC52	18	44-16d sinker	3375	4050
	16	48-16d sinker	3680	4415
MSTC66	18	64-16d sinker	5035	5855
	16	68-16d sinker	5350	5855
MSTC78	18	80-16d sinker	5855	5855
	16	80-16d sinker	5855	5855
MST37	18	20-16d	1905	2285
	16	22-16d	2100	2515
MST48	18	32-16d	3135	3765
	16	34-16d	3330	4000
MST60	18	46-16d	4785	5740
	16	48-16d	4990	5800
MST72	18	56-16d	5800	5800
	16	56-16d	5800	5800
MSTI36	18	14-10dx1½	810	975
	16	16-10dx1½	930	1115
MSTI48	18	26-10dx1½	1545	1855
	16	28-10dx1½	1660	1990
MSTI60	18	38-10dx1½	2330	2800
	16	40-10dx1½	2455	2945
MSTI72	18	50-10dx1½	3065	3680
	16	52-10dx1½	3190	3830

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Bolts Qty	Bolts Dia	Allowable Tension Loads Nails Floor (100)	Nails (133)	Nails (160)	Bolts[5] Floor (100)	Bolts[5] (133)	Bolts[5] (160)
MST27	12	2⁹⁄₁₆	27	30-16d	4	½	2070	2760	2790	1295	1725	2070
MST37		2⁹⁄₁₆	37½	42-16d	6	½	2860	3815	3815	1825	2435	2920
MST48		2⁹⁄₁₆	48	46-16d	8	½	3345	4460	4460	2225	2970	3560
MST60	10	2⁹⁄₁₆	60	56-16d	10	½	4350	5800	5800	2670	3565	4275
MST72		2⁹⁄₁₆	72	56-16d	10	½	4350	5800	5800	2670	3565	4275
HST2	7	2½	21¼	—	6	⅝	—	—	—	3130	4175	5005
HST5		5	21¼	—	12	⅝	—	—	—	6385	8510	10210
HST3	3	3	25½	—	6	¾	—	—	—	4645	6195	7435
HST6		6	25½	—	12	¾	—	—	—	9350	12465	14955

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed. Floor loads may not be increased for other load durations.
2. 10dx1½" nails may be substituted where 16d sinkers are specified at 0.80 of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers (9 gauge x 3¼") or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
5. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: MST–2½"; HST2 and HST5–4"; HST3 and HST6–4½".
6. PS strap design loads must be determined by the building designer for each installation. Bolts are installed both perpendicular and parallel-to-grain.
7. Use half of the nails at each member being connected to achieve the listed loads.

Locking prongs inserts into concrete. The one-piece design assures maximum strength.

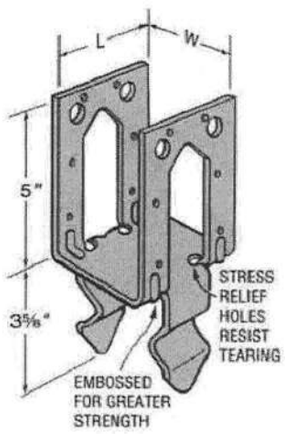
MATERIAL: 12 gauge. **FINISH:** Galvanized

INSTALLATION:
- Use all specified fasteners. See General Notes.
- Holes are provided for installation with either 16d commons or ½" bolts for PB66 and PB66R; all other models use 16d commons only.
- A 2" minimum sidecover is required to obtain the full load.
- Not recommended for non-top-supported installations such as fences.

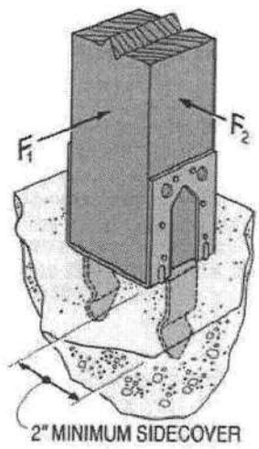
CODES: BOCA, ICBO, SBCCI NER-443; City of LA RR 25149; **Dade Co. 00-0512.11 (PB44).**

Model No.	Dimensions		Uplift Avg Ult	Allowable Loads			
	W	L		12-16d Nails (133 & 160)			2-½MB
				Uplift	F_1	F_2	Uplift (133 & 160)
PB44	3⅜	3¼	4267	1365	765	1325	—
PB44R	4	3¼	4267	1365	765	1325	—
PB46	5½	3¼	4267	1365	765	1325	—
PB46R	6	3¼	4267	1365	765	1325	—
PB66	5½	5¼	5143	1640	765	1325	1640
PB66R	6	5¼	5143	1640	765	1325	1640

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading, with no further increase allowed.



PB



Typical PB Installation

AC/LPC/LCE POST CAPS

The LCE4's universal design provides high capacity while eliminating the need for rights and lefts.

The AC MAX design allows for higher load capacity to match comparable post bases.

LPC—Adjustable design allows greater connection versatility.

MATERIAL: LCE4—20 ga; AC, ACE, LPC4—18 ga; LPC6—16 ga

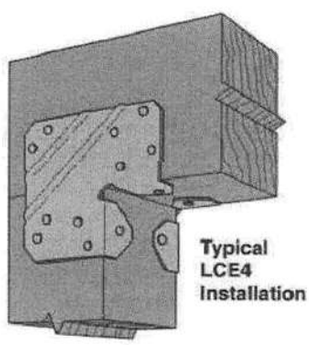
FINISH: Galvanized. Some products available with Z-MAX; see Corrosion-Resistance, page 5.

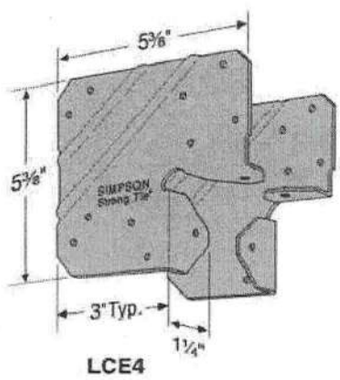
INSTALLATION:
- Use all specified fasteners. See General Notes.
- Install all models in pairs. LPC—2½" beams may be used if 10dx1½" nails are substituted for 10d commons.

CODES: BOCA, ICBO, SBCCI NER-421, NER-443, NER-469; City of L.A. RR 25076; Dade County, FL 99-0623.04 (LPC) and Dade County, FL 99-0713.05 (AC, ACE).

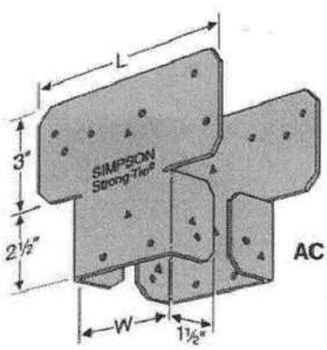
Model No.	Dimensions		Total No. Fasteners		Uplift Avg Ult	Allowable Loads (133 & 160)[1]	
	W	L	Beam	Post		Uplift	Lateral
AC4 MIN	3⅜	6½	12-16d	8-16d	4467	1430	715
AC4 MAX	3⅜	6½	14-16d	14-16d	10000	2500	1070
AC4R MIN	4	7	12-16d	8-16d	4467	1430	715
AC4R MAX	4	7	14-16d	14-16d	10000	2500	1070
ACE4 MIN	—	4½	8-16d	6-16d	4215	1070	715
ACE4 MAX	—	4½	10-16d	10-16d	6238	1785	1070
AC6 MIN	5½	8½	12-16d	8-16d	4467	1430	715
AC6 MAX	5½	8½	14-16d	14-16d	10000	2500	1070
AC6R MIN	6	9	12-16d	8-16d	4467	1430	715
AC6R MAX	6	9	14-16d	14-16d	10000	2500	1070
ACE6 MIN	—	6½	8-16d	6-16d	4537	1070	715
ACE6 MAX	—	6½	10-16d	10-16d	6432	1785	1070
LPC4	3⅜	3½	8-10d	8-10d	2333	760	325
LPC6	5⅜	5½	8-10d	8-10d	2817	915	490
LCE4	—	5⅜	14-16d	10-16d	5518	1800	1425

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce for other load durations according to the code.
2. Loads apply only when used in pairs.
3. LPC lateral load is in the direction of the beam's axis.
4. MIN nailing quantity and load values – fill all round holes; MAX nailing quantities and load values – fill round and triangle holes.

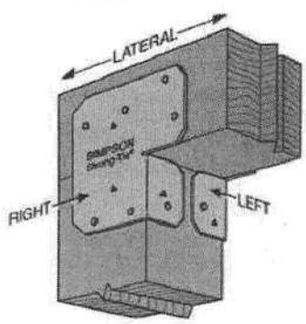

Typical LCE4 Installation



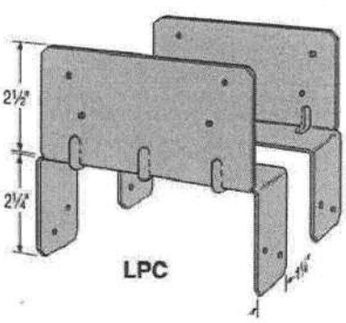
LCE4



AC



Typical ACE Installation



LPC

AB/ABA/ABE/ABU/PBS ADJUSTABLE AND STANDOFF POST BASES

SIMPSON StrongTie

The AB is a fully-adjustable post base which offers moisture protection and finished hardware appearance.

Post Bases provide tested capacity. They feature 1" standoff height above concrete floors, code-required when supporting permanent structures that are exposed to the weather or water splash, or in basements. They reduce the potential for decay at post and column ends.

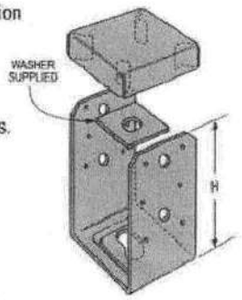
MATERIAL: AB—12 ga plates; 16 ga base cover; all others—see table.

FINISH: Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 5.

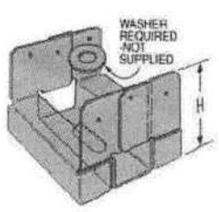
INSTALLATION: • Use all specified fasteners.See General Notes.

- Not recommended for non-top-supported installations such as fences.
- PBS embed into wet concrete up to the bottom of the 1" standoff base plate. A 2" minimum side cover is required to obtain the full load for PBS. Holes in the bottom of the PBS straps allow for free concrete flow.
- AB—Post nail holes are sized for 10d commons. Rectangular adjustment plate assumes ½" dia anchorage. Supplied as shown; position the post, secure the easy-access nut, then bend up the fourth side.
- AB, ABA, ABE and ABU—for pre-pour installed anchors. For epoxy or wedge anchors, select and install according to anchor manufacturer's recommendations; anchor diameter shown in table. Install required washer, which is not included for ABAs.
- See Simpson Anchor Systems for tested, load-rated anchors.

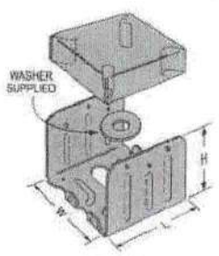
CODES: BOCA, ICBO, SBCCI NER-393, NER-422, NER-432, NER-469, NER-499; **ICBO 5670**; City of L.A. RR 24818, RR 25064, 25074, 25158; Dade Co FL. 99-0713.05 (ABA, ABE), **00-0512.11 (ABU)**.



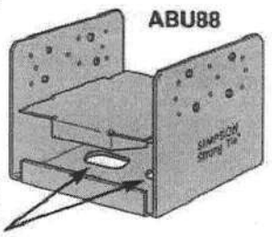
ABU44 (other sizes similar)



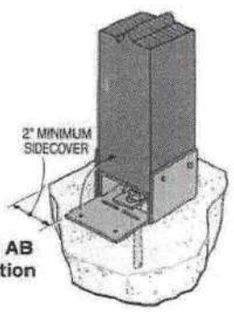
ABA44 (other sizes similar)
U.S. Patent 5,333,435



ABE44
ABE46,46R,66 and 66R supplied with rectangular washer.



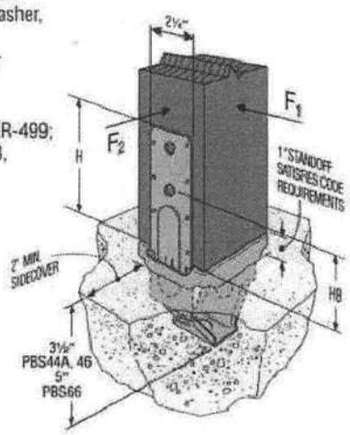
ABU88
2 load transfer plates supplied



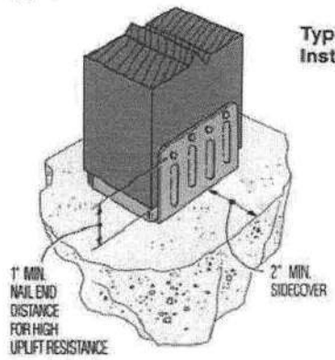
Typical AB Installation

Model No.	Dimensions W	Dimensions L	Allowable Downloads (100)
AB44	3⅜	3⅜	4065
AB44R	4	4⅛	4065
AB46	3⅜	5⅜	4165
AB46R	4	6	4165
AB66	5½	5⅜	5335
AB66R	6	6	5335

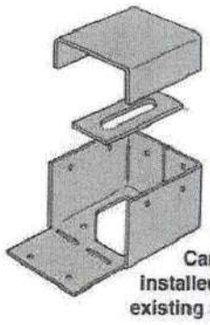
1. Loads may not be increased for short-term loading.



Typical PBS44A Installation



Typical ABE46R Installation for rough lumber (ABE similar)



AB Can be installed on existing slab

Model No.	Nominal Post Size	Material Base (Ga)	Material Strap (Ga)	Dimensions W	Dimensions L	Dimensions H	Dimensions HB	Fasteners Anch. Dia	Fasteners Post Nails	Fasteners Post Bolts Qty	Fasteners Post Bolts Dia	Uplift Avg Ult	Allowable Loads Uplift (133) Nails	Uplift (133) Bolts	Uplift (160) Nails	Uplift (160) Bolts	F_1 (133 & 160) Nails	F_1 (133 & 160) Bolts	F_2 (133 & 160) Nails	F_2 (133 & 160) Bolts	Down (100)
ABA44	4x4	16	16	3⅜	3⅛	3¹⁵⁄₁₆	—	½	6-10d	—	—	2120	555	—	555	—	—	—	—	—	6000
ABE44	4x4	16	16	3⅜	3½	2¾	—	½	6-10d	—	—	1893	520	—	520	—	—	—	—	—	6665
ABU44	4x4	16	12	3⅜	3	5½	1¼	⅝	12-16d	2	½	7833	2200	1800	2200	2160	—	—	—	—	6665
PBS44A	4x4	12	14	3⅜	2¼	6¼	3¹⁵⁄₁₆	—	14-16d	2	½	7733	2400	2400	2400	2400	1165	230	885	885	6665
ABA44R	RGH 4x4	16	16	4⅛	3⅜	2¹³⁄₁₆	—	½	6-10d	—	—	2120	555	—	555	—	—	—	—	—	8000
ABE44R	RGH 4x4	16	16	4	3½	2⅝	—	½	6-10d	—	—	1893	400	—	400	—	—	—	—	—	6665
ABE46	4x6	12	16	3⅜	5⅛	4⅛	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
PBS46	4x6	12	14	3⅜	2¼	6⅝	3⅜	—	14-16d	2	½	7733	2400	2400	2400	2400	1165	360	885	885	9335
ABA46	4x6	14	14	3⅜	5⅜	3⅛	—	⅝	8-16d	—	—	2967	700	—	700	—	—	—	—	—	9435
ABU46	4x6	12	12	3⅜	5	7	2⅜	⅝	12-16d	2	½	8633	2255	2300	2300	2300	—	—	—	—	10335
ABE46R	RGH 4x6	12	16	4⅛	5⅛	3¹⁵⁄₁₆	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
ABA46R	RGH 4x6	14	14	4⅛	5⅜	2⅞	—	⅝	8-16d	—	—	2967	935	—	935	—	—	—	—	—	12000
PBS66	6x6	12	12	5½	2¼	6½	3¹⁵⁄₁₆	—	14-16d	2	½	13100	2630	3560	3160	4000	1865	570	1700	1700	9335
ABA66	6x6	14	14	5½	5¼	3⅛	—	⅝	8-16d	—	—	3050	720	—	720	—	—	—	—	—	10665
ABE66	6x6	12	14	5½	5⅛	3⅛	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU66	6x6	12	10	5½	5	6¹⁵⁄₁₆	1¼	⅝	12-16d	2	½	8900	2300	2300	2300	2300	—	—	—	—	12000
ABA66R	RGH 6x6	14	14	6	5⅛	2⅛	—	⅝	8-16d	—	—	3050	985	—	985	—	—	—	—	—	12665
ABE66R	RGH 6x6	12	14	6⅛	5⅛	2⅞	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU88[4]	8x8	12	14	7½	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335
ABU88R[4]	RGH 8x8	12	14	8	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335

1. Uplift and lateral loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
2. Downloads may not be increased for short-term loading.
3. Specifier to design concrete for shear capacity.
4. ABU88 and ABU88R may be installed with 8-SDS¼X3 wood screws for the same table load.

Caps & Bases

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☐	☑	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☐	☑	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☐	☑	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☐	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☐	☑	**Elevations including:**
☐	☑	a) All sides
☐	☑	b) Roof pitch
☐	☑	c) Overhang dimensions and detail with attic ventilation
☐	☑	d) Location, size and height above roof of chimneys
☐	☑	e) Location and size of skylights
☐	☑	f) Building height
☐	☑	e) Number of stories

Floor Plan including:

- ☐ ☑ a) Rooms labeled and dimensioned
- ☐ ☑ b) Shear walls
- ☐ ☑ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☐ ☑ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☐ ☑ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☐ ☑ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- ☐ ☑ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☐ ☑ b) All posts and/or column footing including size and reinforcing
- ☐ ☐ c) Any special support required by soil analysis such as piling
- ☐ ☑ d) Location of any vertical steel

Roof System:

- ☐ ☑ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- ☐ ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☐ ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☐ ☑ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☐ ☑ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☐ ☑ b) Ceiling fans

☐ ☑ c) Smoke detectors

☐ ☑ d) Service panel and sub-panel size and location(s)

☐ ☑ e) Meter location with type of service entrance (overhead or underground)

☐ ☑ f) Appliances and HVAC equipment

☐ ☑ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☐ ☑ a) Manual J sizing equipment or equivalent computation

☐ ☑ b) Exhaust fans in bathroom

☐ ☑ **Energy Calculations** (dimensions shall match plans)

☐ ☑ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

☐ ☑ **Disclosure Statement for Owner Builders**

☐ ☐ Need ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☑ **Private Potable Water**

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

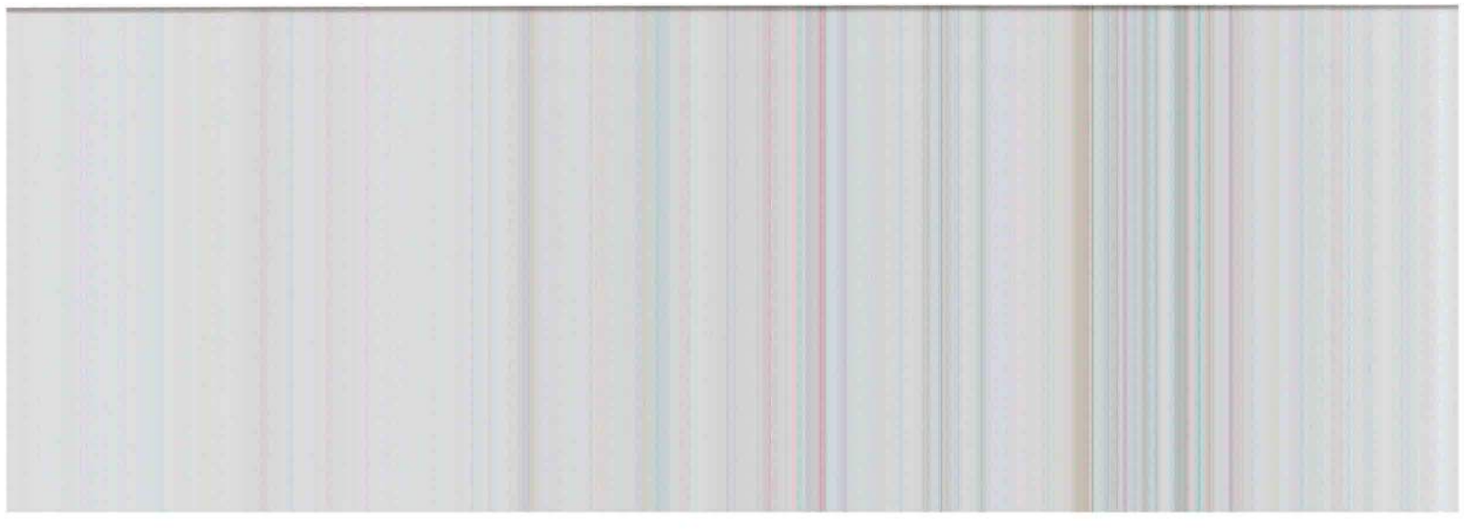
4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK



NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

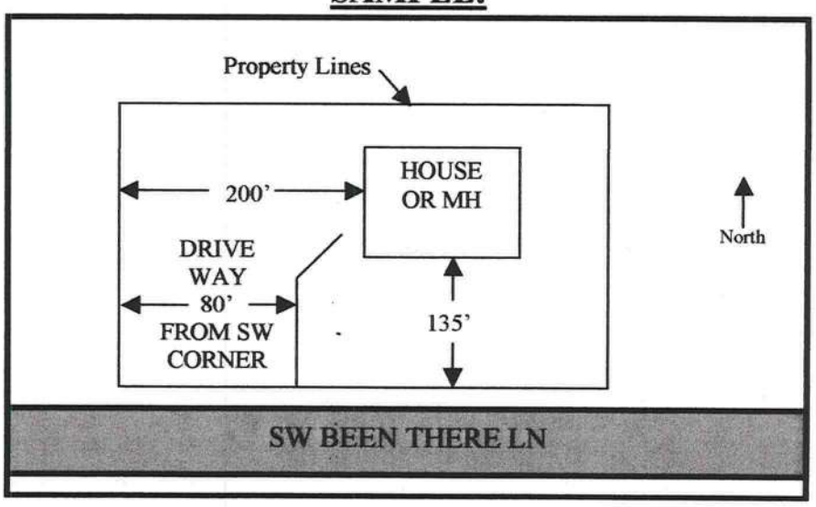
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

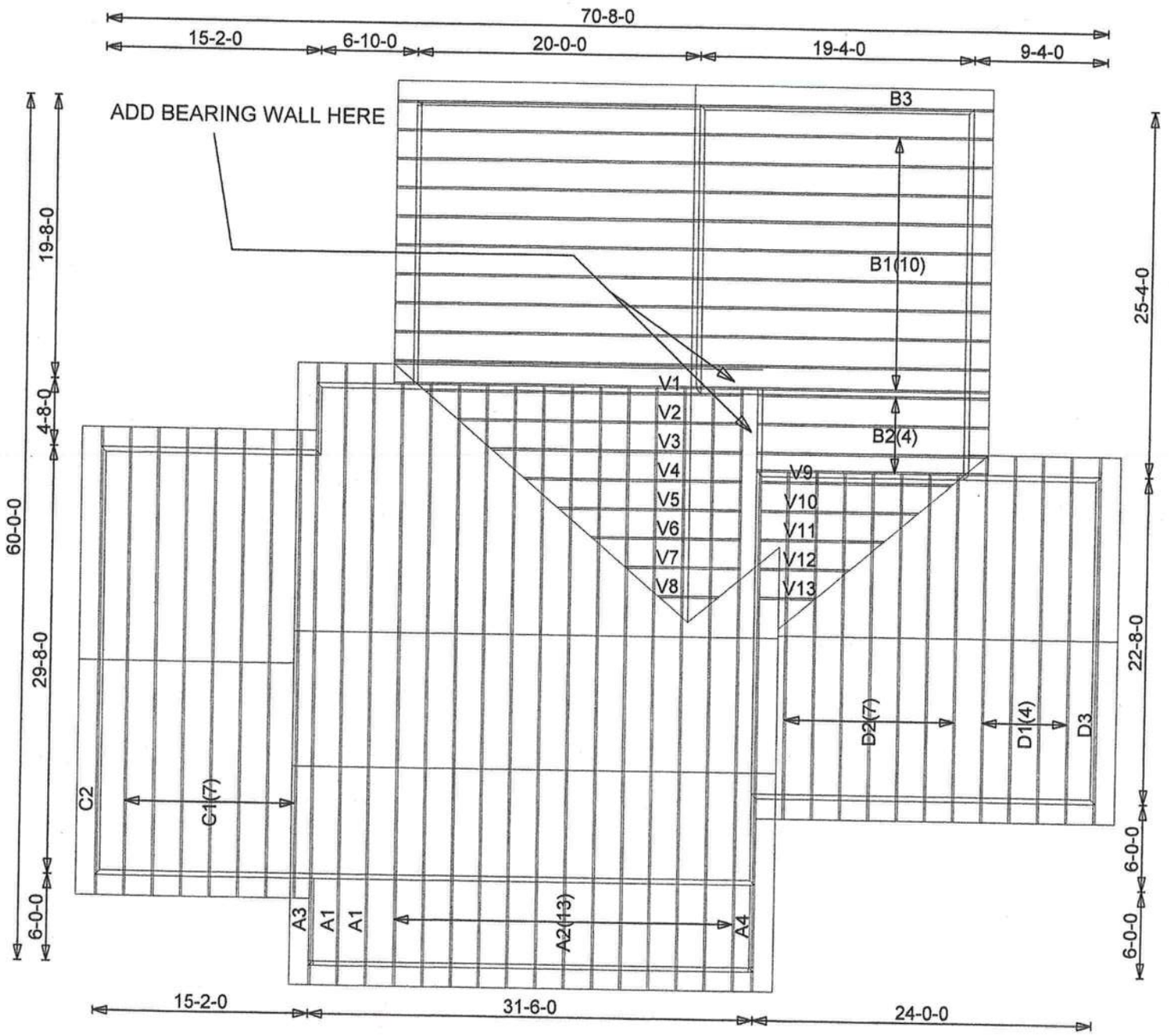
THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.



Mayo Truss Co. Inc.
362 NE CLYDE AVE.
MAYO, FL 32066
(386)294-3988
(877)-558-6262

HAYGOOD-LEVERETT

110 MPH ASCE WIND LOAD

Roof Loading
TC Live: 20.00 psf
TC Dead: 10.00 psf
BC Live: 0.00 psf
BC Dead: 10.00 psf
TC Stress Inc: 25.00
BC Stress Inc: 25.00
Spacing: 2-0-0 o.c.

Account: CONTRACTORS
Job: HAYGOOD-LEVERET
Designer: A. HIGHSMITH
Checker: M.MURRAY
Date: 08-30-05

Permit Number:_____ Lot Number:_____

Miscellaneous:_____ Address:_____

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator:Mayo Truss Company, Inc

Job Reference:HAYGOOD-LEVERET - LEVERETT

Standard Loading:

T.C. Live	20 psf
T.C Dead	10 psf
B.C Live	0 psf
B.C. Dead	10 psf
Total	40 psf

ANSI/ASCE 7-98
Wind Speed - 110 mph
Mean Roof Ht. - 15 ft.
Exposure Catergory -B
Occupancy Factor - 1.00
MWFRS
Enclosed

ROBBINS ENGINEERING, INC.

P.O. Box 280055
Tampa, FL 33682-0055
Phone: (813) 972-1135

Engineering Index Sheet

Index Page 1 of 1

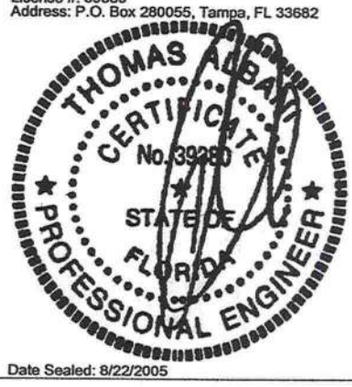
Job Number	Date		Specification Quantity
T05082269	8/22/2005	FBC - 2001 Chapter 16 and 23	25

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibilty of the Building Designer, as defined in ANSI/TPI 1-1995, Section 2.2. Permanent files of the original Truss Specificstion Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Sofware - Online Plus)

Notes: Refer to individual truss design drawings for special loading conditions.

	Date	Mark		Date	Mark		Date	Mark		Date	Mark
1	11/17/04	A1	2	11/17/04	A2	3	11/17/04	A3	4	11/17/04	A4
5	11/17/04	B1	6	11/17/04	B2	7	11/17/04	B3	8	11/17/04	C1
9	11/17/04	C2	10	11/17/04	D1	11	11/17/04	D2	12	11/17/04	D3
13	11/17/04	V1	14	11/17/04	V2	15	11/17/04	V3	16	11/17/04	V4
17	11/17/04	V5	18	11/17/04	V6	19	11/17/04	V7	20	11/17/04	V8
21	11/17/04	V9	22	11/17/04	V10	23	11/17/04	V11	24	11/17/04	V12
25	11/17/04	V13									

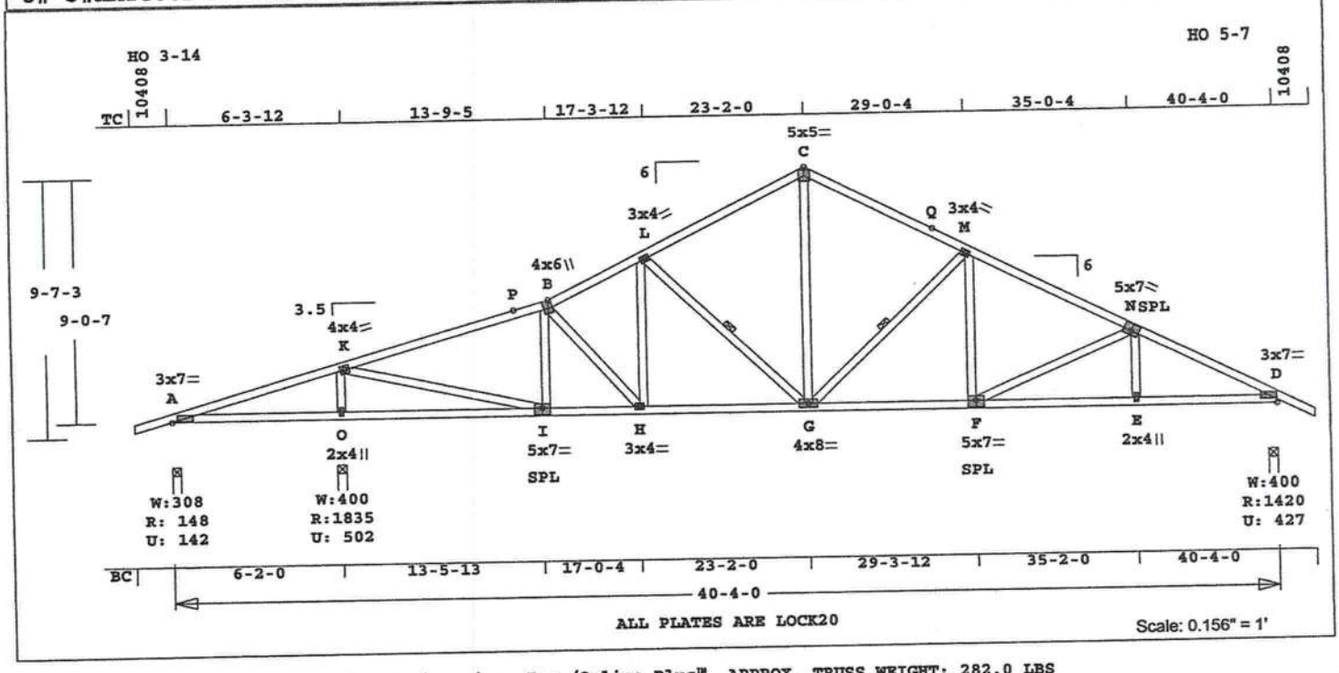
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/22/2005

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	A1	2	PCH3	400400	6	1- 4- 8	1- 4- 8	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 282.0 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.69	2X 4 SP-#2	1720
BTM	0.75	2X 4 SP-#2	1720
WBS	0.46	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - L-G G-M
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	1	148	3- 8
O	HORZ RLR	0	1835	4- 0
D	HORZ RLR	0	1420	4- 0

	LEFT	RIGHT
HEEL	0IN - 4SX	1IN - 8SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-K	0.69	433	T	-392	-3965
K-P	0.65	1844	C	3965	850
P-B	0.24	1782	C	-850	-1387
B-L	0.28	1812	C	1004	-1529
L-C	0.31	1416	C	1529	-1627
C-Q	0.27	1404	C	1639	-31
Q-M	0.39	1483	C	31	-2403
M-N	0.40	1930	C	2403	-1060
N-D	0.52	2332	C	1060	-2870
BOTTOM CHORDS					
A-O	0.28	400	C	-1095	-1826
O-I	0.28	400	C	1826	-370
I-H	0.38	1751	T	370	-362
H-G	0.41	1624	T	362	-750
G-F	0.45	1732	T	750	-843
F-E	0.51	2072	T	843	277
E-D	0.75	2072	T	-277	-2390

WEBS

O-K	=	1646	C	K-I	=	2217	T
I-B	=	446	C	B-H	=	193	C
H-L	=	233	T	L-G	=	496	C
G-C	=	912	T	G-M	=	645	C
F-M	=	292	T	F-N	=	374	C
E-N	=	66	T				

DL+LL DEFL = 0.28" IN L-C
LL DEFL = 0.10" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2001	3.00	X 7.00	9.1	3.1
B	5094R	4.00	X 6.00	3.5	2.0
C	3001	5.00	X 5.00	CTR	CTR
D	2001	3.00	X 7.00	3.7	1.6
E	1001	2.00	X 4.00	CTR	CTR
F	1151	5.00	X 7.00	CTR	3.0
G	1070	4.00	X 8.00	CTR	CTR
H	1030	3.00	X 4.00	CTR	CTR
I	1131	5.00	X 7.00	CTR	3.0
K	1050	4.00	X 4.00	CTR	CTR
L	1050	3.00	X 4.00	CTR	CTR
M	1050	3.00	X 4.00	CTR	CTR
N	1151	5.00	X 7.00	CTR	3.0
O	1001	2.00	X 4.00	CTR	CTR
P					
Q					

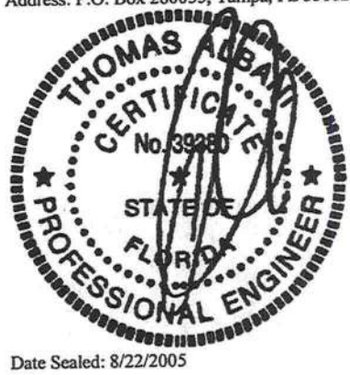
R = PLATE IS ROTATED BY 90 DEG

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 188 LBS.
5. FASTEN TRUSS TO BRG A FOR 142 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG O FOR 502 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG D FOR 427 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

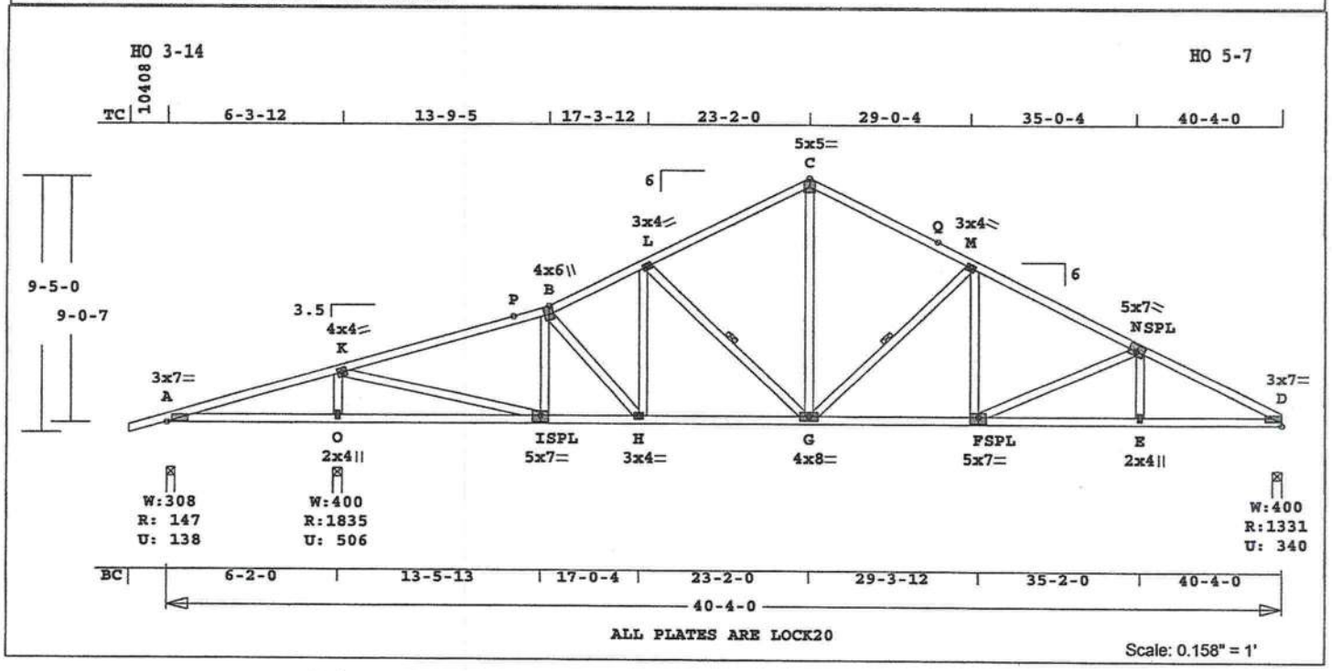
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/22/2005

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	A2	13	PCH3	400400	6	1- 4- 8	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 279.0 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE	LUMBER	1.15FB
TOP	0.69	2X 4	SP-#2	1720
BTM	0.75	2X 4	SP-#2	1720
WBS	0.46	2X 4	SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - L-G G-M
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	147	3- 8	O	1835	4- 0
D	1331	4- 0			

	LEFT	RIGHT
HEEL	0IN - 4SX	1IN - 8SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-K	0.69	434	T	-393	-3965
K-P	0.65	1844	C	3965	850
P-B	0.24	1782	C	-850	-1387
B-L	0.28	1812	C	1004	-1528
L-C	0.31	1416	C	1528	-1627
C-Q	0.27	1404	C	1639	-31
Q-M	0.39	1483	C	31	-2403
M-N	0.40	1930	C	2403	-1060
N-D	0.52	2332	C	1060	-2869
BOTTOM CHORDS					
A-O	0.28	399	C	-1097	-1826
O-I	0.28	399	C	1826	-370
I-H	0.38	1753	T	370	-362
H-G	0.41	1626	T	362	-750
G-F	0.45	1734	T	750	-843
F-E	0.51	2073	T	843	276
E-D	0.75	2073	T	-276	-2389

WEBS					
O-K	= 1646	C	K-I	= 2218	T
I-B	= 446	C	B-H	= 193	C
H-L	= 233	T	L-G	= 496	C
G-C	= 912	T	G-M	= 645	C
F-M	= 292	T	F-N	= 374	C
E-N	= 66	T			

DL+LL DEFL = 0.28" IN L-C
LL DEFL = 0.10" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2001	3.00 X	7.00	9.1	3.1
B	5094R	4.00 X	6.00	3.5	2.0
C	3001	5.00 X	5.00	CTR	CTR
D	2001	3.00 X	7.00	3.7	1.6
E	1001	2.00 X	4.00	CTR	CTR
F	1151	5.00 X	7.00	CTR	3.0
G	1070	4.00 X	8.00	CTR	CTR
H	1030	3.00 X	4.00	CTR	CTR
I	1131	5.00 X	7.00	CTR	3.0
K	1050	4.00 X	4.00	CTR	CTR
L	1050	3.00 X	4.00	CTR	CTR
M	1050	3.00 X	4.00	CTR	CTR
N	1151	5.00 X	7.00	CTR	3.0
O	1001	2.00 X	4.00	CTR	CTR
P					
Q					

R = PLATE IS ROTATED BY 90 DEG

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

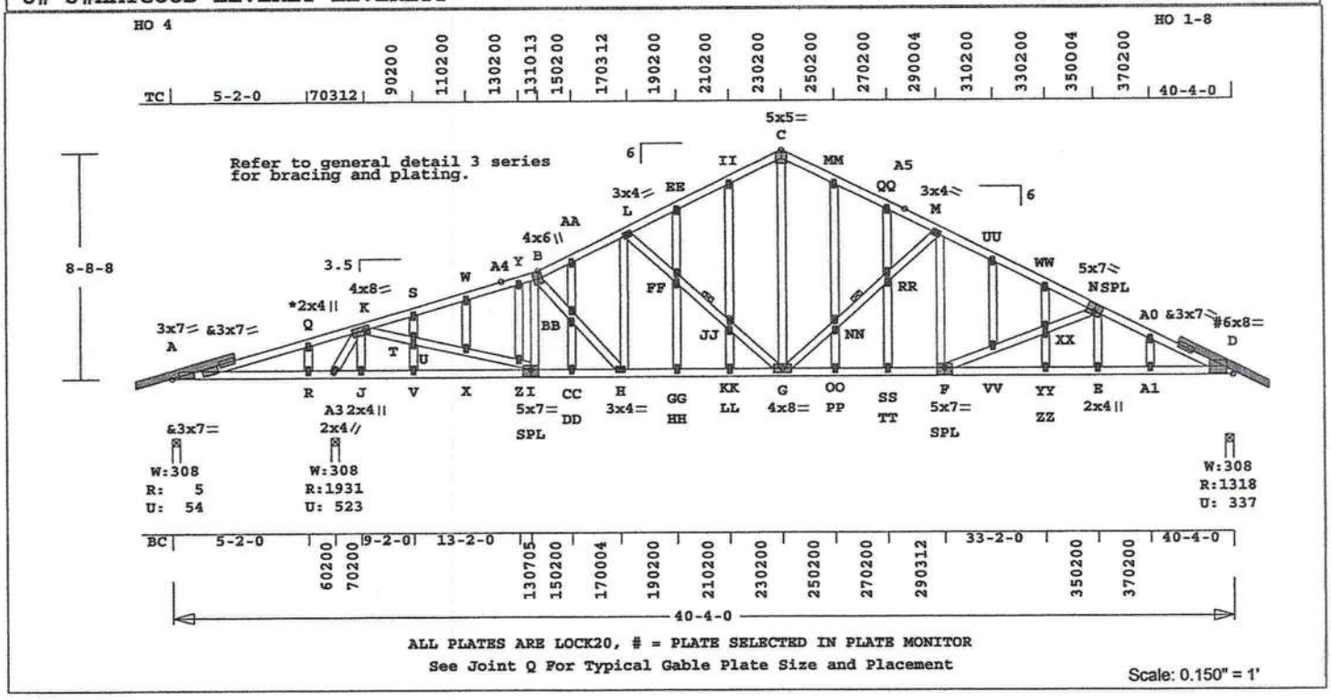
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98 TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM FOR EXTERIOR ZONE LOCATION WIND SPEED - 110 MPH MEAN ROOF HEIGHT - 15' EXPOSURE CATEGORY - B OCCUPANCY FACTOR - 1.00 ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 193 LBS.
5. FASTEN TRUSS TO BRG A FOR 138 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG O FOR 506 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG D FOR 340 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/22/2005

HAYGOOD-LEVERET A3	1	SP	400400	3.5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See Joint Q For Typical Gable Plate Size and Placement

Scale: 0.150" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 371.1 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE	LUMBER	1.15FB
TOP	0.78	2X 4	SP-#2	1720
BTM	0.59	2X 4	SP-#1	2130
WBS	0.47	2X 4	SP-#2	1720

EXCEPTIONS:
F-E 2X 4 SP-#2 1720
E-D SAME AS F-E
REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - L-G G-M
GABLE STUDS ARE CONSIDERED NON-STRUCTURAL. ALL BRACING FOR HORIZONTAL WIND LOADS ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	-22	3- 8	A3	1931	3- 8
D	1318	3- 8			

	LEFT	RIGHT
GIRD	0IN - 4SX	1IN - 8SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-K	0.78	917	T	1474	-3941
K-A4	0.65	1958	C	3941	985
A4-B	0.24	1902	C	-985	-951
B-L	0.30	1898	C	498	-1608
L-C	0.30	1472	C	1608	-1647
C-A5	0.27	1458	C	1656	97
A5-M	0.37	1537	C	-97	-2232
M-N	0.38	2062	C	2232	-1713
N-D	0.33	2712	C	1713	-498
BOTTOM CHORDS					
A-A3	0.26	882	C	-1474	-2050
A3-J	0.26	113	T	88	-395
J-I	0.13	113	T	395	-367
I-H	0.32	1859	T	422	-325
H-G	0.35	1710	T	325	-804
G-F	0.37	1847	T	804	-648
F-E	0.55	2468	T	648	-420
E-D	0.59	2468	T	420	498

WEBS

A3-K	= 1966	C	J-K	= 99	T
K-I	= 1856	T	I-B	= 387	C
B-H	= 223	C	H-L	= 249	T
L-G	= 529	C	G-C	= 960	T
G-M	= 714	C	F-M	= 370	T
F-N	= 669	C	E-N	= 124	T

DL+LL DEFL = 0.29" IN L-C
LL DEFL = 0.11" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	7.00	9.5	3.2
B	5094R	4.00 X	6.00	3.5	2.0
C	3001	5.00 X	5.00	CTR	CTR
!D	2003	6.00 X	8.00	8.4	5.2
E	1001	2.00 X	4.00	CTR	CTR
F	1151	5.00 X	7.00	CTR	3.0
G	1070	4.00 X	8.00	CTR	CTR
H	1030	3.00 X	4.00	CTR	CTR
I	1191	5.00 X	7.00	3.5	3.0
J	1001	2.00 X	4.00	CTR	CTR
K	1070	4.00 X	8.00	CTR	CTR
L	1050	3.00 X	4.00	CTR	CTR
M	1050	3.00 X	4.00	CTR	CTR
N	1151	5.00 X	7.00	CTR	3.0
A3	1001	2.00 X	4.00	CTR	CTR
A4					
A5					

R = PLATE IS ROTATED BY 90 DEG
! SELECTED VIA PLATE MONITOR

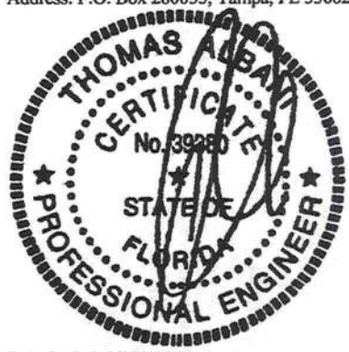
19-GABLE STUDS TO BE ATTACHED WITH 2.0X4.0 PLATES AT EACH END.

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 167 LBS.
5. FASTEN TRUSS TO BRG A FOR 54 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/22/2005

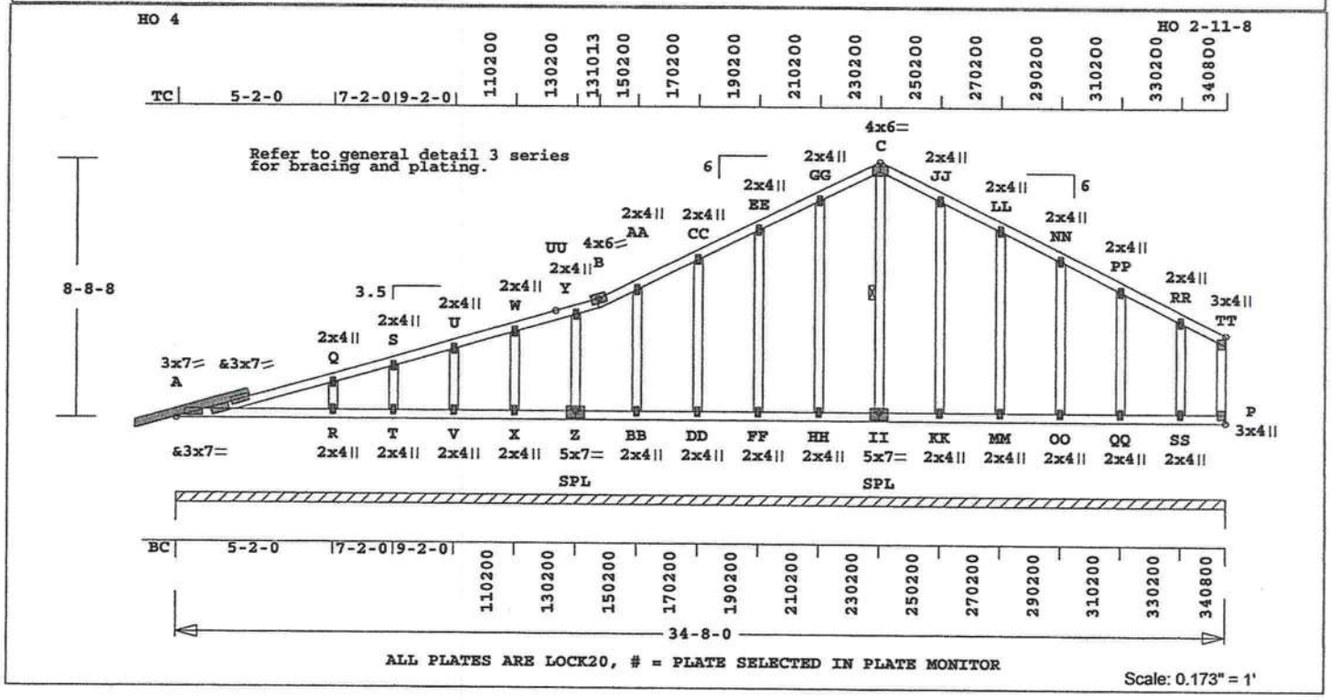
Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	A3	1	SP	400400	3.5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT

6. FASTEN TRUSS TO BRG A3
   FOR 523 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG D
   FOR 337 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	A4	1	PCH3	340800	6	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.173" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 274.6 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.23	2X 4 SP-#2	1720
BTM	0.14	2X 4 SP-#2	1720
WBS	0.11	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - II-C
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & P

	LEFT	RIGHT
GIRD	0IN - 4SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-Q	0.23	201 C	-345	558
Q-S	0.23	167 C	-558	-25
S-U	0.05	157 C	25	125
U-W	0.05	155 T	-86	107
W-UU	0.04	172 T	-107	-111
UU-Y	0.04	175 T	111	-72
Y-B	0.06	195 T	72	232
B-AA	0.06	200 T	-232	-20
AA-CC	0.05	229 T	20	114
CC-EE	0.05	261 T	-114	104
EE-GG	0.05	292 T	-104	-3
GG-C	0.07	315 T	3	184
C-JJ	0.06	307 T	-166	106
JJ-LL	0.06	259 T	-106	208
LL-NN	0.05	205 T	-208	185
NN-PP	0.04	154 T	-185	197
PP-RR	0.04	101 T	-197	157
RR-TT	0.03	55 T	-157	6
BOTTOM CHORDS				
A-R	0.14	46 C	345	129
R-T	0.05	46 C	-129	-34
T-V	0.01	46 C	34	9
V-X	0.01	46 C	-9	-2
X-Z	0.01	46 C	2	0
Z-BB	0.01	46 C	0	0
BB-DD	0.01	46 C	0	0
DD-FF	0.01	46 C	0	0
FF-HH	0.01	46 C	0	0
HH-II	0.01	46 C	0	0
II-KK	0.01	46 C	0	0
KK-MM	0.01	46 C	0	0
MM-OO	0.01	46 C	0	0
OO-QQ	0.01	46 C	0	0
QQ-SS	0.01	46 C	0	0
SS-P	0.01	46 C	0	0
WEBS				
P-TT	0.05	47 T	0	0
R-Q =	290 C	T-S =	39 C	
V-U =	141 C	X-W =	116 C	
Z-Y =	121 C	BB-AA=	121 C	
DD-CC=	121 C	FF-EE=	119 C	
HH-GG=	127 C	II-C =	216 C	
KK-JJ=	127 C	MM-LL=	119 C	
OO-NN=	120 C	QQ-PP=	123 C	
SS-RR=	111 C			

DL+LL DEFL = 0.05" IN A-Q
LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	7.00	9.5	3.2
B	5093	4.00 X	6.00	3.0	2.0
C	3001	4.00 X	6.00	3.0	2.2
P	4000	3.00 X	4.00	CTR	CTR
Q	1001	2.00 X	4.00	CTR	CTR
R	1001	2.00 X	4.00	CTR	CTR
S	1001	2.00 X	4.00	CTR	CTR
T	1001	2.00 X	4.00	CTR	CTR
U	1001	2.00 X	4.00	CTR	CTR
V	1001	2.00 X	4.00	CTR	CTR
W	1001	2.00 X	4.00	CTR	CTR
X	1001	2.00 X	4.00	CTR	CTR
Y	1001	2.00 X	4.00	CTR	CTR
Z	1102	5.00 X	7.00	CTR	3.0
AA	1001	2.00 X	4.00	CTR	CTR
BB	1001	2.00 X	4.00	CTR	CTR
CC	1001	2.00 X	4.00	CTR	CTR
DD	1001	2.00 X	4.00	CTR	CTR
EE	1001	2.00 X	4.00	CTR	CTR
FF	1001	2.00 X	4.00	CTR	CTR
GG	1001	2.00 X	4.00	CTR	CTR
HH	1001	2.00 X	4.00	CTR	CTR
II	1102	5.00 X	7.00	CTR	3.0
JJ	1001	2.00 X	4.00	CTR	CTR
KK	1001	2.00 X	4.00	CTR	CTR
LL	1001	2.00 X	4.00	CTR	CTR
MM	1001	2.00 X	4.00	CTR	CTR
NN	1001	2.00 X	4.00	CTR	CTR
OO	1001	2.00 X	4.00	CTR	CTR
PP	1001	2.00 X	4.00	CTR	CTR
QQ	1001	2.00 X	4.00	CTR	CTR
RR	1001	2.00 X	4.00	CTR	CTR
SS	1001	2.00 X	4.00	CTR	CTR
TT	4100	3.00 X	4.00	CTR	CTR
UU					

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/22/2005

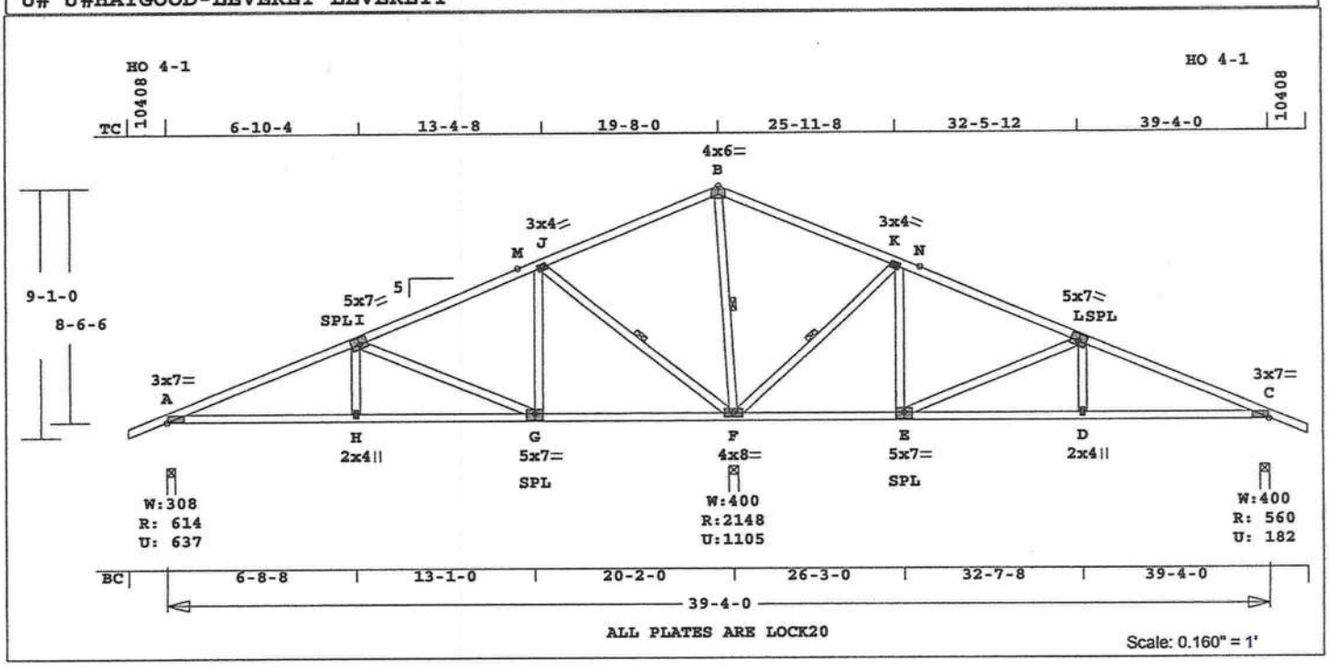
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	A4	1	PCH3	340800	6	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT

3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A
   MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL
   HORIZONTAL LOAD OF 258 LBS.
5. FASTEN TRUSS TO BRG A
   FOR 71 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG R
   FOR 214 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG V
   FOR 97 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG X
   FOR 84 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
9. FASTEN TRUSS TO BRG Z
   FOR 81 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
10. FASTEN TRUSS TO BRG BB
    FOR 92 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
11. FASTEN TRUSS TO BRG DD
    FOR 94 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
12. FASTEN TRUSS TO BRG FF
    FOR 98 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
13. FASTEN TRUSS TO BRG HH
    FOR 92 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
14. FASTEN TRUSS TO BRG KK
    FOR 86 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
15. FASTEN TRUSS TO BRG MM
    FOR 100 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
16. FASTEN TRUSS TO BRG OO
    FOR 94 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
17. FASTEN TRUSS TO BRG QQ
    FOR 97 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
18. FASTEN TRUSS TO BRG SS
    FOR 87 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	B1	10	TR	390400	5	1- 4- 8	1- 4- 8	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 261.9 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE	LUMBER	1.15FB
TOP	0.53	2X 4	SP-#2	1720
BTM	0.31	2X 4	SP-#2	1720
WBS	0.62	2X 4	SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - J-F B-F F-K
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)
TOP CHD	20.0	10.0
BTM CHD	0.0	10.0
TOTAL	20.0	20.0 40.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	614	3- 8	F	2148	4- 0
C	560	4- 0			

	LEFT	RIGHT
HEEL	0IN - 4SX	0IN - 4SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-I	0.42	897 T	0	2050
I-M	0.41	320 T	-2050	885
M-J	0.41	333 T	-885	2000
J-B	0.51	688 T	2642	-1677
B-K	0.53	749 T	1557	-2686
K-N	0.44	287 C	-838	330
N-L	0.42	300 C	-330	996
L-C	0.42	607 C	2721	-592
BOTTOM CHORDS				
A-H	0.31	918 C	347	1700
H-G	0.30	918 C	-1700	1780
G-F	0.22	255 C	-1780	906
F-E	0.17	398 T	-906	-737
E-D	0.29	570 T	597	-1205
D-C	0.29	570 T	1205	886

WEBS

H-I	=	256 C	I-G	=	727 C
G-J	=	543 C	J-F	=	886 T
B-F	=	922 C	F-K	=	814 C
E-K	=	403 T	E-L	=	733 C
D-L	=	163 T			

DL+LL DEFL = 0.13" IN A-I
LL DEFL = 0.15" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

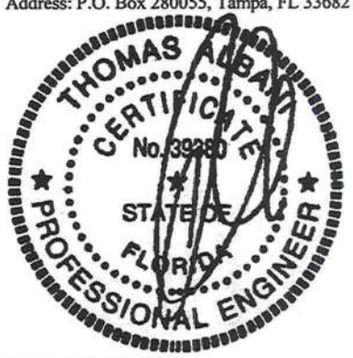
JT	TYPE	PLATE	SIZE	X	Y
A	2001	3.00 X	7.00	7.5	3.2
B	3001	4.00 X	6.00	3.0	2.2
C	2001	3.00 X	7.00	7.5	3.2
D	1001	2.00 X	4.00	CTR	CTR
E	1151	5.00 X	7.00	CTR	3.0
F	1070	4.00 X	8.00	CTR	CTR
G	1131	5.00 X	7.00	CTR	3.0
H	1001	2.00 X	4.00	CTR	CTR
I	1151	5.00 X	7.00	CTR	3.0
J	1050	3.00 X	4.00	CTR	CTR
K	1050	3.00 X	4.00	CTR	CTR
L	1151	5.00 X	7.00	CTR	3.0
M					
N					

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98 TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM FOR EXTERIOR ZONE LOCATION WIND SPEED - 110 MPH MEAN ROOF HEIGHT - 15' EXPOSURE CATEGORY - B OCCUPANCY FACTOR - 1.00 ENCLOSED BUILDING.
TC DEAD LOAD = 5.0 PSF
BC DEAD LOAD = 5.0 PSF
4. PREVENT TRUSS ROTATION AT ALL BEARING LOCATIONS.
5. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 176 LBS.
6. FASTEN TRUSS TO BRG A FOR 637 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG F FOR 1105 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG C FOR 182 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

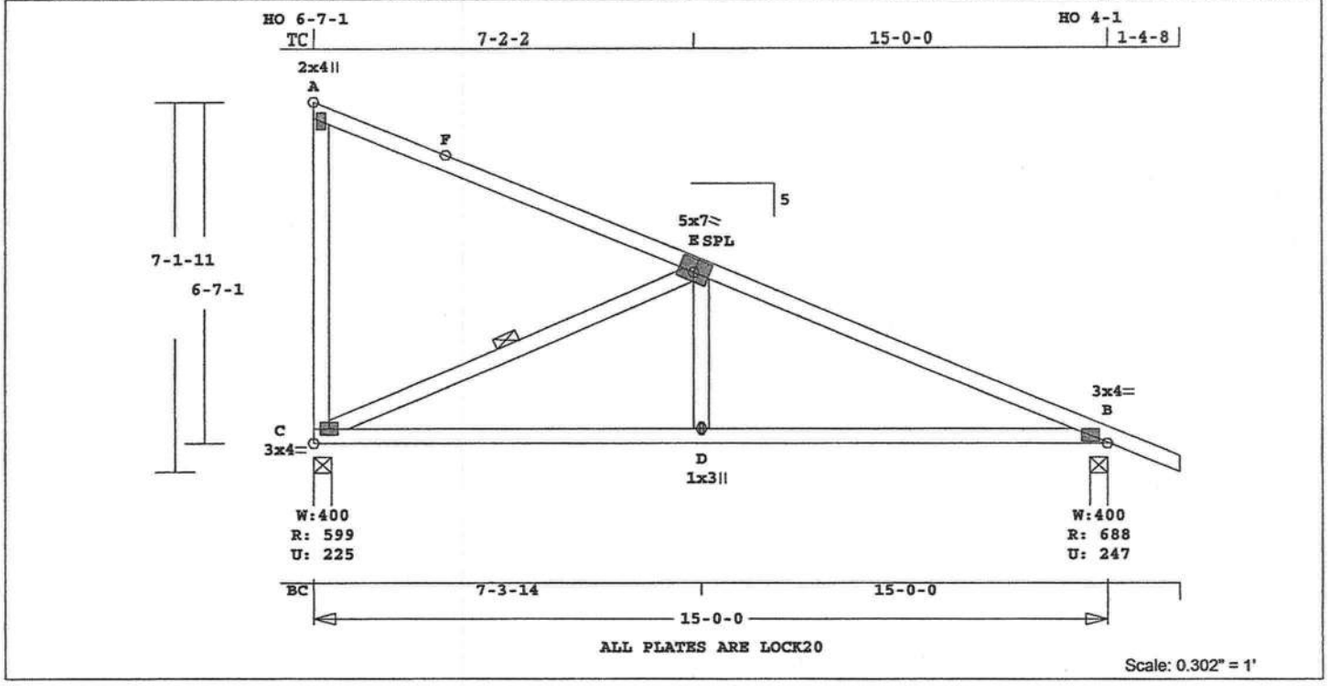
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/22/2005

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	B2	4	SP	150000	5	0	1- 4- 8	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 93.2 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.67	2X 4 SP-#2	1720
BTM	0.40	2X 4 SP-#2	1720
WBS	0.39	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - C-E
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
C	599	4- 0	B	688	4- 0

	LEFT	RIGHT
HEEL		0IN - 4SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-F	0.41	152	C	-3	-1151
F-E	0.66	166	C	1151	1864
E-B	0.67	874	C	4318	-792
BOTTOM CHORDS					
C-D	0.40	821	T	2	-1666
D-B	0.40	821	T	1666	897
WEBS					
C-A	0.26	175	C	0	0

C-E = 901 C D-E = 193 T

DL+LL DEFL = 0.20" AT F
LL DEFL = 0.02" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 878

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	4200	2.00 X 4.00	CTR	CTR
B	2001	3.00 X 4.00	5.9	3.1
C	4010	3.00 X 4.00	CTR	CTR
D	1001	1.00 X 3.00	CTR	CTR
E	1151	5.00 X 7.00	CTR	3.0
F				

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

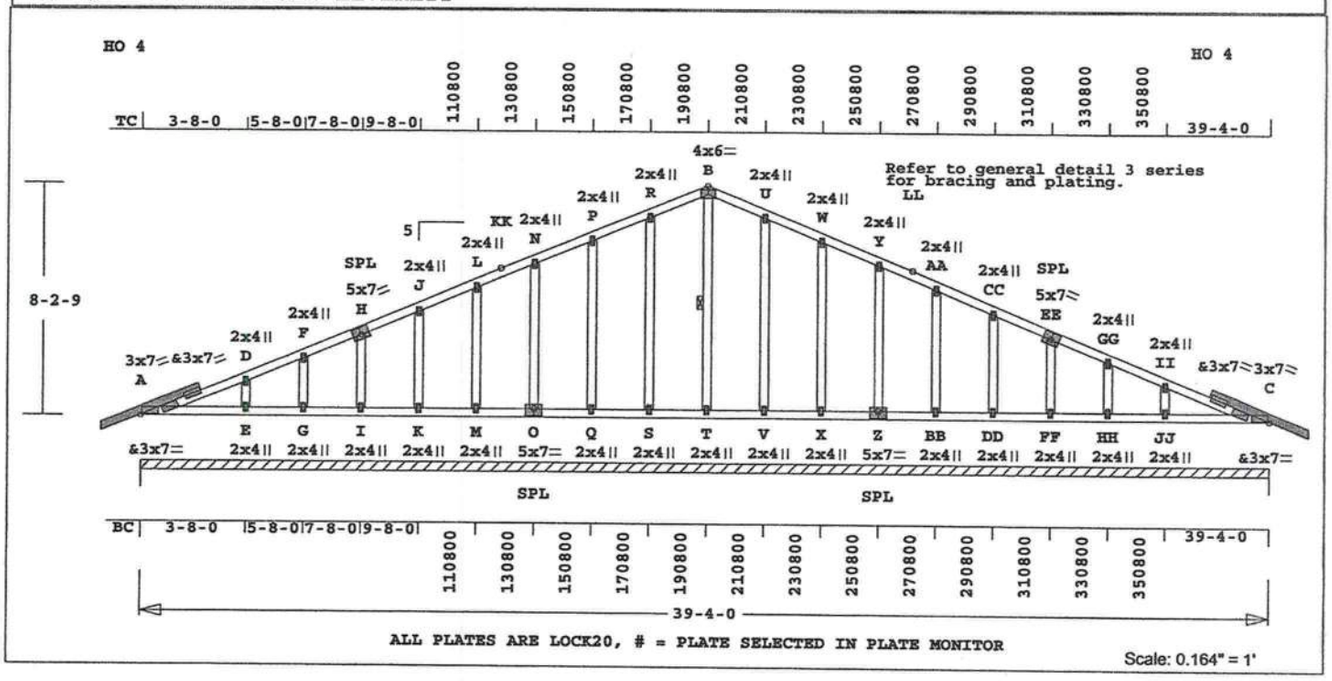
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 423 LBS.
5. FASTEN TRUSS TO BRG C FOR 225 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG B FOR 247 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/22/2005

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	B3	1	SP	390400	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.164" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 299.1 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.11	2X 4 SP-#2	1720
BTM	0.07	2X 4 SP-#2	1720
WBS	0.11	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - T-B
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 4SX	0IN - 4SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-D	0.11	180 C	-330	506
D-F	0.11	125 C	-506	93
F-H	0.04	98 T	-45	91
H-J	0.04	123 T	-91	80
J-L	0.04	146 T	-80	83
L-KK	0.04	168 T	-83	-38
KK-N	0.04	172 T	38	80
N-P	0.04	193 T	-80	94
P-R	0.04	218 T	-94	23
R-B	0.05	239 T	-23	136
B-U	0.05	232 T	-118	127
U-W	0.05	192 T	-127	186
W-Y	0.04	149 T	-186	174
Y-LL	0.04	109 T	-174	-81
LL-AA	0.04	105 T	81	177
AA-CC	0.04	75 T	-83	80
CC-EE	0.04	52 T	-80	91
EE-GG	0.04	27 T	-91	44
GG-II	0.11	54 C	-93	506
II-C	0.11	111 C	-506	330
BOTTOM CHORDS				
A-E	0.07	132 T	151	51
E-G	0.03	132 T	-51	-13
G-I	0.02	132 T	13	3
I-K	0.02	132 T	-3	0
K-M	0.02	132 T	0	0
M-O	0.02	132 T	0	0
O-Q	0.02	132 T	0	0
Q-S	0.02	132 T	0	0
S-T	0.02	132 T	0	0
T-V	0.02	132 T	0	0
V-X	0.02	132 T	0	0
X-Z	0.02	132 T	0	0
Z-BB	0.02	132 T	0	0
BB-DD	0.02	132 T	0	-2
DD-FF	0.02	132 T	2	8
FF-HH	0.02	132 T	-8	-30
HH-JJ	0.03	132 T	30	112
JJ-C	0.07	132 T	-112	-330

WEBS

E-D	=	202 C	G-F	=	89 C
I-H	=	128 C	K-J	=	119 C
M-L	=	121 C	O-N	=	121 C
Q-P	=	119 C	S-R	=	126 C
T-B	=	124 C	V-U	=	126 C
X-W	=	119 C	Z-Y	=	121 C
BB-AA	=	121 C	DD-CC	=	119 C
FF-EE	=	128 C	HH-GG	=	89 C
JJ-II	=	202 C			

DL+LL DEFL = 0.01" IN II-C
LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 7.00	7.7	3.2
B	3001	4.00 X 6.00	3.0	2.2
C	2003	3.00 X 7.00	7.7	3.2
D	1001	2.00 X 4.00	CTR	CTR
E	1001	2.00 X 4.00	CTR	CTR
F	1001	2.00 X 4.00	CTR	CTR
G	1001	2.00 X 4.00	CTR	CTR
H	1102	5.00 X 7.00	CTR	3.0
I	1001	2.00 X 4.00	CTR	CTR
J	1001	2.00 X 4.00	CTR	CTR
K	1001	2.00 X 4.00	CTR	CTR
L	1001	2.00 X 4.00	CTR	CTR
M	1001	2.00 X 4.00	CTR	CTR
N	1001	2.00 X 4.00	CTR	CTR
O	1102	5.00 X 7.00	CTR	3.0
P	1001	2.00 X 4.00	CTR	CTR
Q	1001	2.00 X 4.00	CTR	CTR
R	1001	2.00 X 4.00	CTR	CTR
S	1001	2.00 X 4.00	CTR	CTR
T	1001	2.00 X 4.00	CTR	CTR
U	1001	2.00 X 4.00	CTR	CTR
V	1001	2.00 X 4.00	CTR	CTR
W	1001	2.00 X 4.00	CTR	CTR
X	1001	2.00 X 4.00	CTR	CTR
Y	1001	2.00 X 4.00	CTR	CTR
Z	1102	5.00 X 7.00	CTR	3.0
AA	1001	2.00 X 4.00	CTR	CTR
BB	1001	2.00 X 4.00	CTR	CTR
CC	1001	2.00 X 4.00	CTR	CTR
DD	1001	2.00 X 4.00	CTR	CTR
EE	1102	5.00 X 7.00	CTR	3.0
FF	1001	2.00 X 4.00	CTR	CTR
GG	1001	2.00 X 4.00	CTR	CTR
HH	1001	2.00 X 4.00	CTR	CTR
II	1001	2.00 X 4.00	CTR	CTR
JJ	1001	2.00 X 4.00	CTR	CTR
KK				
LL				

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/22/2005

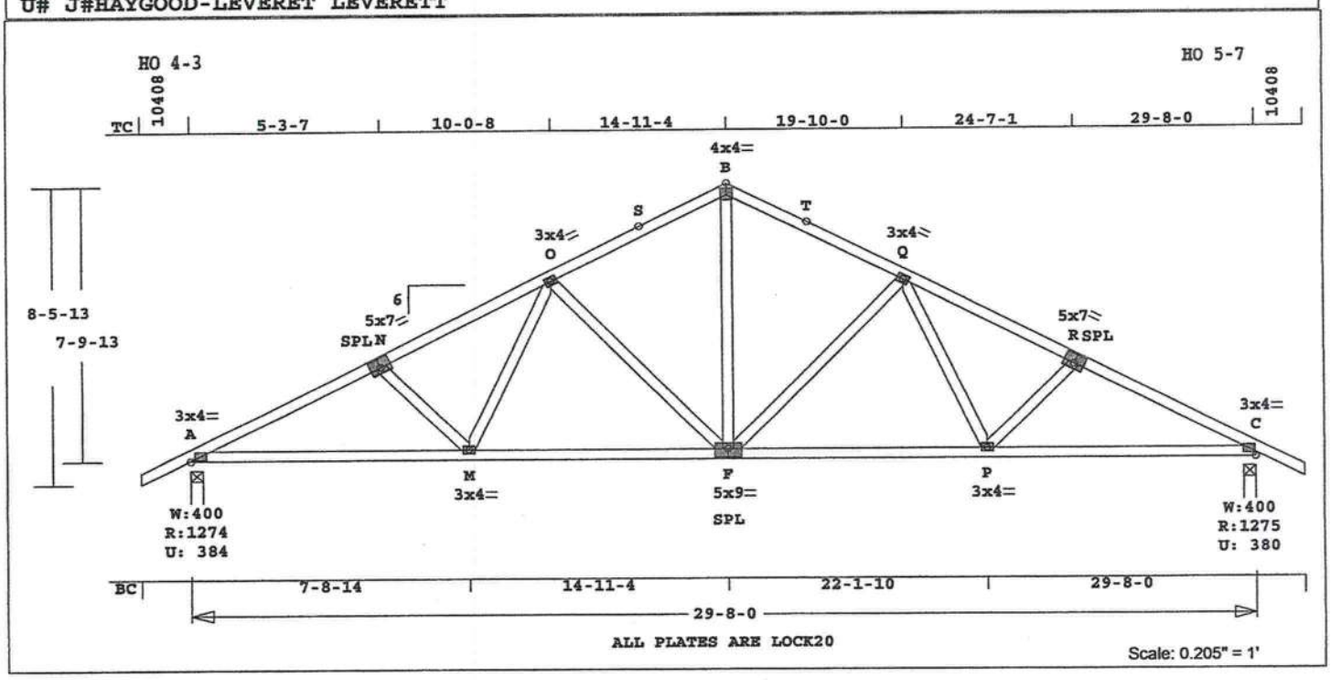
Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	B3	1	SP	390400	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT

3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A
   MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL
   HORIZONTAL LOAD OF 157 LBS.
5. FASTEN TRUSS TO BRG E
   FOR 146 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG G
   FOR 59 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG I
   FOR 95 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG K
   FOR 86 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
9. FASTEN TRUSS TO BRG M
   FOR 88 LBS OF UPLIFT,
   WHILE PERMITTING NO UPWARD
   MOVEMENT OF WALL OR BRG.
10. FASTEN TRUSS TO BRG O
    FOR 87 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
11. FASTEN TRUSS TO BRG Q
    FOR 90 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
12. FASTEN TRUSS TO BRG S
    FOR 87 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
13. FASTEN TRUSS TO BRG V
    FOR 84 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
14. FASTEN TRUSS TO BRG X
    FOR 91 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
15. FASTEN TRUSS TO BRG Z
    FOR 87 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
16. FASTEN TRUSS TO BRG BB
    FOR 88 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
17. FASTEN TRUSS TO BRG DD
    FOR 86 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
18. FASTEN TRUSS TO BRG FF
    FOR 95 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
19. FASTEN TRUSS TO BRG HH
    FOR 59 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
20. FASTEN TRUSS TO BRG JJ
    FOR 151 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	C1	7	SP	290800	6	1- 4- 8	1- 4- 8	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 197.1 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.45	2X 4 SP-#2	1720
BTM	0.62	2X 4 SP-#2	1720
WBS	0.43	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	1	1274	4- 0
C	HORZ RLR	0	1275	4- 0

	LEFT	RIGHT
HEEL	0IN - 4SX	1IN - 8SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-N	0.22	2086	C	1120	-1153
N-O	0.24	1886	C	1153	-1391
O-S	0.23	1344	C	1391	885
S-B	0.17	1280	C	-885	-1014
B-T	0.17	1278	C	1016	825
T-Q	0.25	1342	C	-825	-1509
Q-R	0.26	1836	C	1509	-676
R-C	0.45	2024	C	676	-2624
BOTTOM CHORDS					
A-M	0.48	1864	T	813	-870
M-F	0.46	1533	T	870	-1140
F-P	0.45	1516	T	1140	-568
P-C	0.62	1795	T	568	-1891
WEBS					
N-M	=	264	C	M-O	= 370 T
O-F	=	524	C	F-B	= 888 T
F-Q	=	498	C	Q-P	= 314 T
P-R	=	221	C		

DL+LL DEFL = 0.21" IN F-P
LL DEFL = 0.09" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	2001	3.00 X 4.00	5.2	3.1
B	3001	4.00 X 4.00	2.0	2.2
C	2001	3.00 X 4.00	3.7	2.4
F	1170	5.00 X 9.00	CTR	3.0
M	1010	3.00 X 4.00	CTR	CTR
N	1102	5.00 X 7.00	CTR	3.0
O	1010	3.00 X 4.00	CTR	CTR
P	1010	3.00 X 4.00	CTR	CTR
Q	1010	3.00 X 4.00	CTR	CTR
R	1102	5.00 X 7.00	CTR	3.0
S				
T				

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

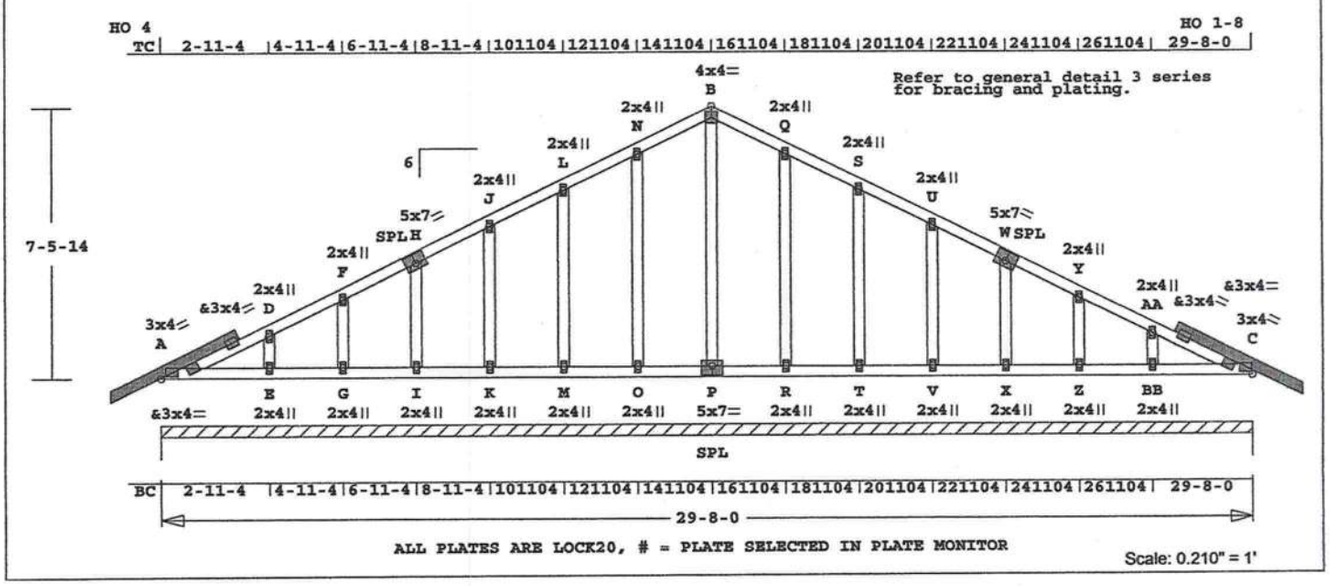
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 162 LBS.
5. FASTEN TRUSS TO BRG A FOR 384 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG C FOR 380 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/22/2005

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	C2	1	SP	290800	6	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 222.6 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.07	2X 4 SP-#2	1720
BTM	0.05	2X 4 SP-#2	1720
WBS	0.12	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 4SX	1IN - 8SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-D	0.07	170 C	-223	352
D-F	0.07	115 C	-352	149
F-H	0.04	94 T	-72	95
H-J	0.04	124 T	-95	87
J-L	0.04	152 T	-87	102
L-N	0.04	183 T	-102	35
N-B	0.05	209 T	-35	136
B-Q	0.05	201 T	-117	147
Q-S	0.04	150 T	-147	199
S-U	0.04	97 T	-199	186
U-W	0.04	63 T	-88	94
W-Y	0.04	33 T	-94	75
Y-AA	0.06	55 C	-158	316
AA-C	0.06	113 C	-316	197
BOTTOM CHORDS				
A-E	0.05	130 T	102	32
E-G	0.02	130 T	-32	-8
G-I	0.02	130 T	8	2
I-K	0.02	130 T	-2	0
K-M	0.02	130 T	0	0
M-O	0.02	130 T	0	0
O-P	0.02	130 T	0	0
P-R	0.02	130 T	0	0
R-T	0.02	130 T	0	0
T-V	0.02	130 T	0	-1
V-X	0.02	130 T	1	4
X-Z	0.02	130 T	-4	-16
Z-BB	0.03	130 T	16	60
BB-C	0.04	130 T	-60	-197
WEBS				
E-D	=	167 C	G-F	=	105 C
I-H	=	124 C	K-J	=	120 C
M-L	=	120 C	O-N	=	126 C
P-B	=	128 C	R-Q	=	126 C
T-S	=	120 C	V-U	=	120 C
X-W	=	123 C	Z-Y	=	109 C
BB-AA=		157 C			

DL+LL DEFL = 0.01" IN A-D
LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	5.5	3.2
B	3001	4.00 X	4.00	2.0	2.2
C	2003	3.00 X	4.00	5.5	3.2
D	1001	2.00 X	4.00	CTR	CTR
E	1001	2.00 X	4.00	CTR	CTR
F	1001	2.00 X	4.00	CTR	CTR
G	1001	2.00 X	4.00	CTR	CTR
H	1102	5.00 X	7.00	CTR	3.0
I	1001	2.00 X	4.00	CTR	CTR
J	1001	2.00 X	4.00	CTR	CTR
K	1001	2.00 X	4.00	CTR	CTR
L	1001	2.00 X	4.00	CTR	CTR
M	1001	2.00 X	4.00	CTR	CTR
N	1001	2.00 X	4.00	CTR	CTR
O	1001	2.00 X	4.00	CTR	CTR
P	1102	5.00 X	7.00	CTR	3.0
Q	1001	2.00 X	4.00	CTR	CTR
R	1001	2.00 X	4.00	CTR	CTR
S	1001	2.00 X	4.00	CTR	CTR
T	1001	2.00 X	4.00	CTR	CTR
U	1001	2.00 X	4.00	CTR	CTR
V	1001	2.00 X	4.00	CTR	CTR
W	1102	5.00 X	7.00	CTR	3.0
X	1001	2.00 X	4.00	CTR	CTR
Y	1001	2.00 X	4.00	CTR	CTR
Z	1001	2.00 X	4.00	CTR	CTR
AA	1001	2.00 X	4.00	CTR	CTR
BB	1001	2.00 X	4.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 142 LBS.
5. FASTEN TRUSS TO BRG E FOR 123 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG G FOR 80 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG I FOR 99 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG K FOR 94 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
9. FASTEN TRUSS TO BRG M FOR 97 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/22/2005

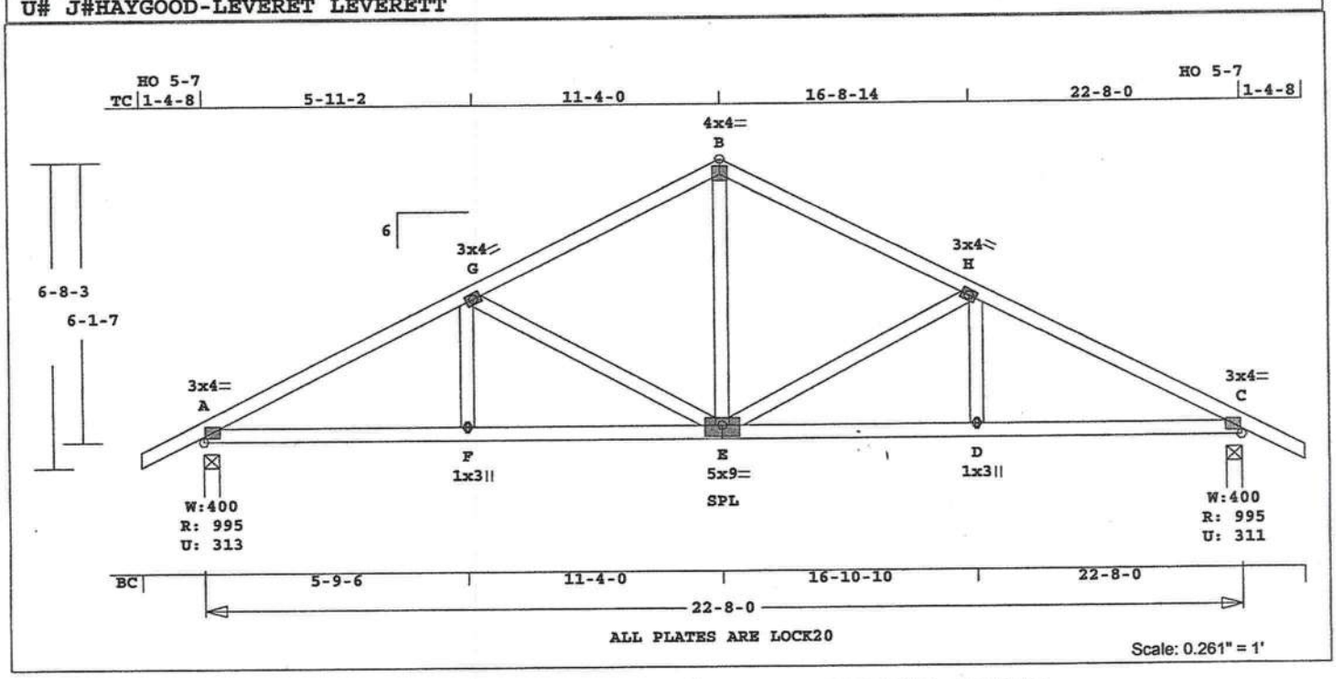
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	C2	1	SP	290800	6	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT

10. FASTEN TRUSS TO BRG O
    FOR 95 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
11. FASTEN TRUSS TO BRG R
    FOR 92 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
12. FASTEN TRUSS TO BRG T
    FOR 98 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
13. FASTEN TRUSS TO BRG V
    FOR 94 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
14. FASTEN TRUSS TO BRG X
    FOR 98 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
15. FASTEN TRUSS TO BRG Z
    FOR 83 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.
16. FASTEN TRUSS TO BRG BB
    FOR 125 LBS OF UPLIFT,
    WHILE PERMITTING NO UPWARD
    MOVEMENT OF WALL OR BRG.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	D1	4	SP	220800	6	1- 4- 8	1- 4- 8	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 140.5 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.34	2X 4 SP-#2	1720
BTM	0.40	2X 4 SP-#2	1720
WBS	0.30	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	995	4- 0	C	995	4- 0

	LEFT	RIGHT
HEEL	1IN - 8SX	1IN - 8SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-G	0.34	1443	C	2065	-1636
G-B	0.26	999	C	1636	-1348
B-H	0.26	999	C	1348	-1636
H-C	0.34	1443	C	1636	-2065
BOTTOM CHORDS					
A-F	0.40	1287	T	1098	-216
F-E	0.36	1287	T	216	-777
E-D	0.36	1287	T	777	-216
D-C	0.40	1287	T	216	-1099

WEBS

F-G	=	89	T		
G-E	=	453	C		
E-B	=	562	T		
E-H	=	453	C		
D-H	=	89	T		

DL+LL DEFL = 0.14" IN B-H
LL DEFL = 0.05" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	2001	3.00 X 4.00	3.7	2.4
B	3001	4.00 X 4.00	2.0	2.2
C	2001	3.00 X 4.00	3.7	2.4
D	1001	1.00 X 3.00	CTR	CTR
E	1170	5.00 X 9.00	CTR	3.0
F	1001	1.00 X 3.00	CTR	CTR
G	1050	3.00 X 4.00	CTR	CTR
H	1050	3.00 X 4.00	CTR	CTR

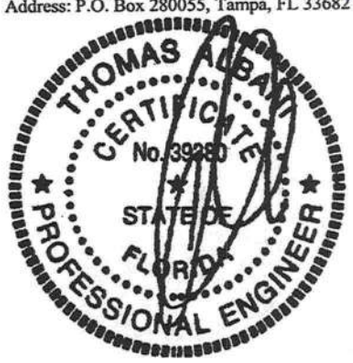
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

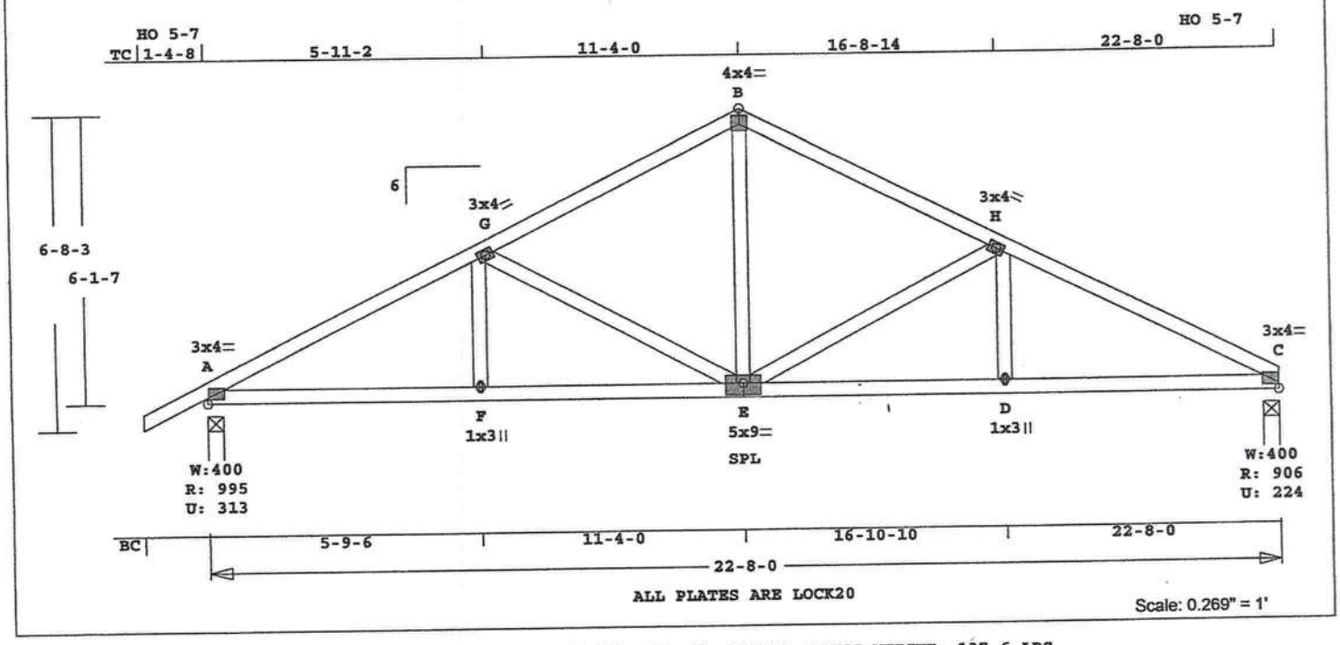
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 125 LBS.
5. FASTEN TRUSS TO BRG A FOR 313 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG C FOR 311 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	D2	7	SP	220800	6	1- 4- 8	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 137.6 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.34	2X 4 SP-#2	1720
BTM	0.40	2X 4 SP-#2	1720
WBS	0.30	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	-1	995	4- 0
C	HORZ RLR	0	906	4- 0

	LEFT	RIGHT
HEEL	1IN - 8SX	1IN - 8SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-G	0.34	1443 C	2065	-1636
G-B	0.26	999 C	1636	-1348
B-H	0.26	999 C	1348	-1636
H-C	0.34	1443 C	1636	-2065
BOTTOM CHORDS				
A-F	0.40	1288 T	1098	-216
F-E	0.36	1288 T	216	-777
E-D	0.36	1288 T	777	-216
D-C	0.40	1288 T	216	-1098
WEBS				
F-G	=	89 T	G-E	= 453 C
E-B	=	562 T	E-H	= 453 C
D-H	=	89 T		

DL+LL DEFL = 0.14" IN B-H
LL DEFL = 0.05" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2001	3.00	X 4.00	3.7	2.4
B	3001	4.00	X 4.00	2.0	2.2
C	2001	3.00	X 4.00	3.7	2.4
D	1001	1.00	X 3.00	CTR	CTR
E	1170	5.00	X 9.00	CTR	3.0
F	1001	1.00	X 3.00	CTR	CTR
G	1050	3.00	X 4.00	CTR	CTR
H	1050	3.00	X 4.00	CTR	CTR

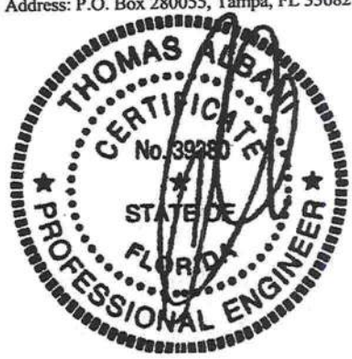
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

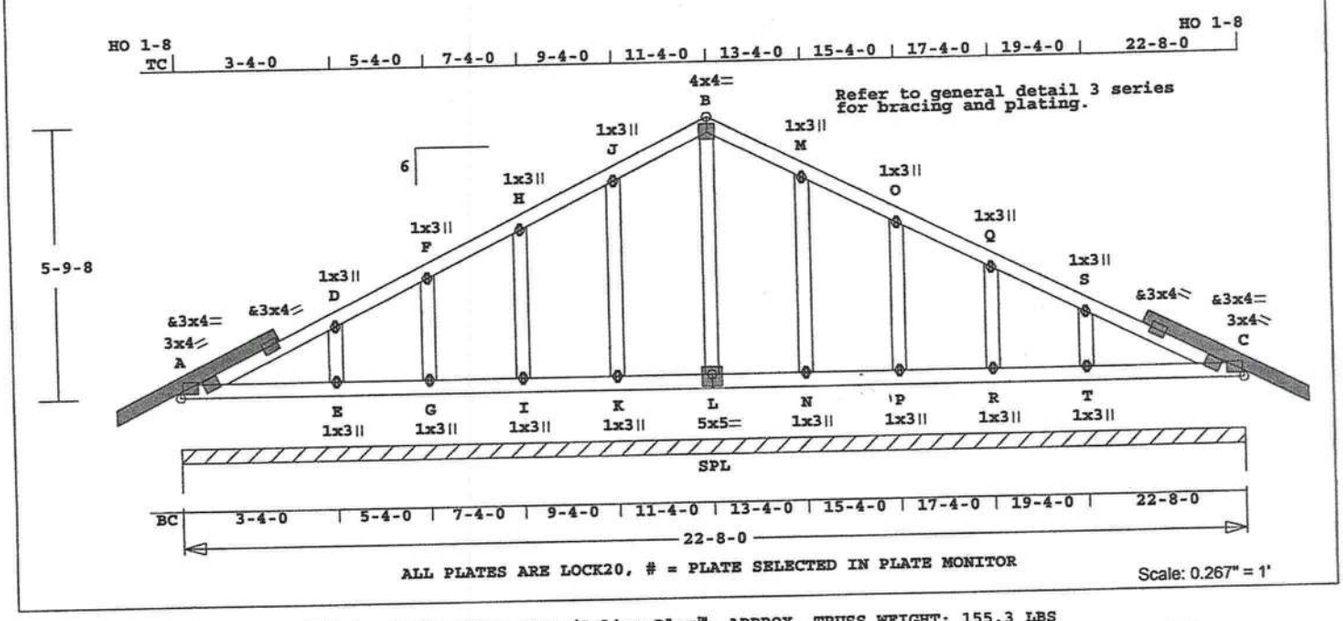
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 143 LBS.
5. FASTEN TRUSS TO BRG A FOR 313 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG C FOR 224 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	D3	1	SP	220800	6	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 155.3 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.09	2X 4 SP-#2	1720
BTM	0.06	2X 4 SP-#2	1720
WBS	0.05	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	1IN - 8SX	1IN - 8SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-D	0.09	117 C	-303	455
D-F	0.09	74 T	-215	58
F-H	0.04	99 T	-58	105
H-J	0.04	131 T	-105	55
J-B	0.04	157 T	-55	109
B-M	0.04	149 T	-91	169
M-O	0.04	98 T	-169	209
O-Q	0.04	53 T	-102	58
Q-S	0.09	28 T	-58	215
S-C	0.09	71 C	-455	303
BOTTOM CHORDS				
A-E	0.06	98 T	141	46
E-G	0.02	98 T	-46	-12
G-I	0.02	98 T	12	3
I-K	0.01	98 T	-3	-1
K-L	0.01	98 T	1	0
L-N	0.01	98 T	0	-1
N-P	0.02	98 T	1	7
P-R	0.02	98 T	-7	-26
R-T	0.03	98 T	26	99
T-C	0.06	98 T	-99	-303
WEBS				
E-D	=	185 C	G-F	= 98 C
I-H	=	125 C	K-J	= 125 C
L-B	=	101 C	N-M	= 125 C
P-O	=	125 C	R-Q	= 98 C
T-S	=	185 C		

DL+LL DEFL = 0.01" IN S-C

LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 4.00	5.5	3.2
B	3001	4.00 X 4.00	2.0	2.2
C	2003	3.00 X 4.00	5.5	3.2
D	1001	1.00 X 3.00	1.5	0.5
E	1001	1.00 X 3.00	CTR	CTR
F	1001	1.00 X 3.00	1.5	0.5
G	1001	1.00 X 3.00	CTR	CTR
H	1001	1.00 X 3.00	1.5	0.5
I	1001	1.00 X 3.00	CTR	CTR
J	1001	1.00 X 3.00	1.5	0.5
K	1001	1.00 X 3.00	CTR	CTR
L	1102	5.00 X 5.00	CTR	3.0
M	1001	1.00 X 3.00	CTR	CTR
N	1001	1.00 X 3.00	CTR	CTR
O	1001	1.00 X 3.00	CTR	CTR
P	1001	1.00 X 3.00	CTR	CTR
Q	1001	1.00 X 3.00	CTR	CTR
R	1001	1.00 X 3.00	CTR	CTR
S	1001	1.00 X 3.00	CTR	CTR
T	1001	1.00 X 3.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

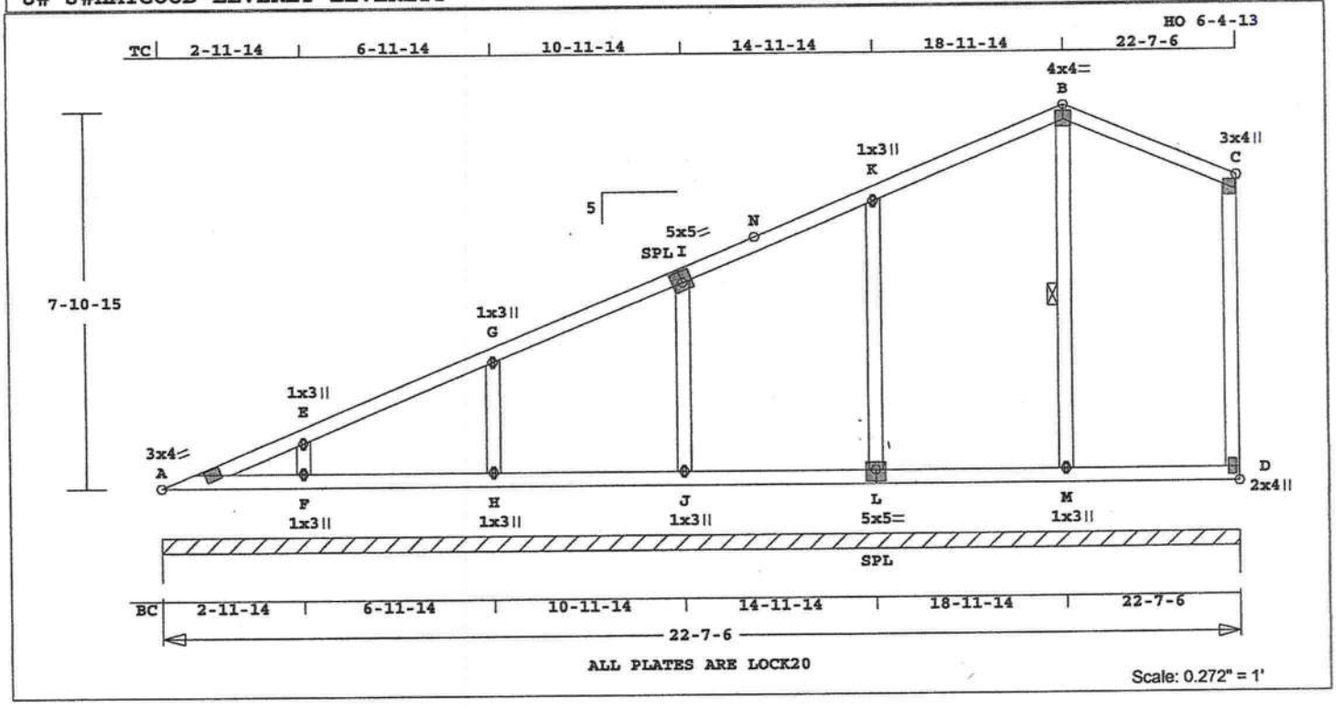
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 108 LBS.
5. FASTEN TRUSS TO BRG E FOR 152 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG G FOR 71 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG I FOR 101 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG K FOR 96 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
9. FASTEN TRUSS TO BRG N FOR 94 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
10. FASTEN TRUSS TO BRG P FOR 102 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
11. FASTEN TRUSS TO BRG R FOR 71 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
12. FASTEN TRUSS TO BRG T FOR 154 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V1	1	VLS.SB	220706	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 136.9 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.17	2X 4 SP-#2	1720
BTM	0.03	2X 4 SP-#2	1720
WBS	0.24	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
ONE BRACE - M-B
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
		TOP CHORDS		
A-E	0.13	292 C	-81	353
E-G	0.16	257 C	-353	432
G-I	0.16	201 C	-432	400
I-N	0.14	151 C	-400	-164
N-K	0.17	143 C	164	450
K-B	0.16	132 T	-311	344
B-C	0.12	120 T	-255	3
		BOTTOM CHORDS		
A-F	0.03	108 C	81	18
F-H	0.02	108 C	-18	-5
H-J	0.01	108 C	5	1
J-L	0.01	108 C	-1	0
L-M	0.01	108 C	0	0
M-D	0.01	108 C	0	0
		WEBS		
D-C	0.24	106 T	0	0

F-E = 224 C H-G = 245 C
J-I = 237 C L-K = 251 C
M-B = 232 C

DL+LL DEFL = 0.02" IN B-C

LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 4.00	6.2	3.2
B	3001	4.00 X 4.00	2.0	2.2
C	4100	3.00 X 4.00	CTR	CTR
D	4000	2.00 X 4.00	CTR	CTR
E	1001	1.00 X 3.00	CTR	CTR
F	1001	1.00 X 3.00	CTR	CTR
G	1001	1.00 X 3.00	CTR	CTR
H	1001	1.00 X 3.00	CTR	CTR
I	1102	5.00 X 5.00	CTR	3.0
J	1001	1.00 X 3.00	CTR	CTR
K	1001	1.00 X 3.00	CTR	CTR
L	1102	5.00 X 5.00	CTR	3.0
M	1001	1.00 X 3.00	CTR	CTR
N				

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

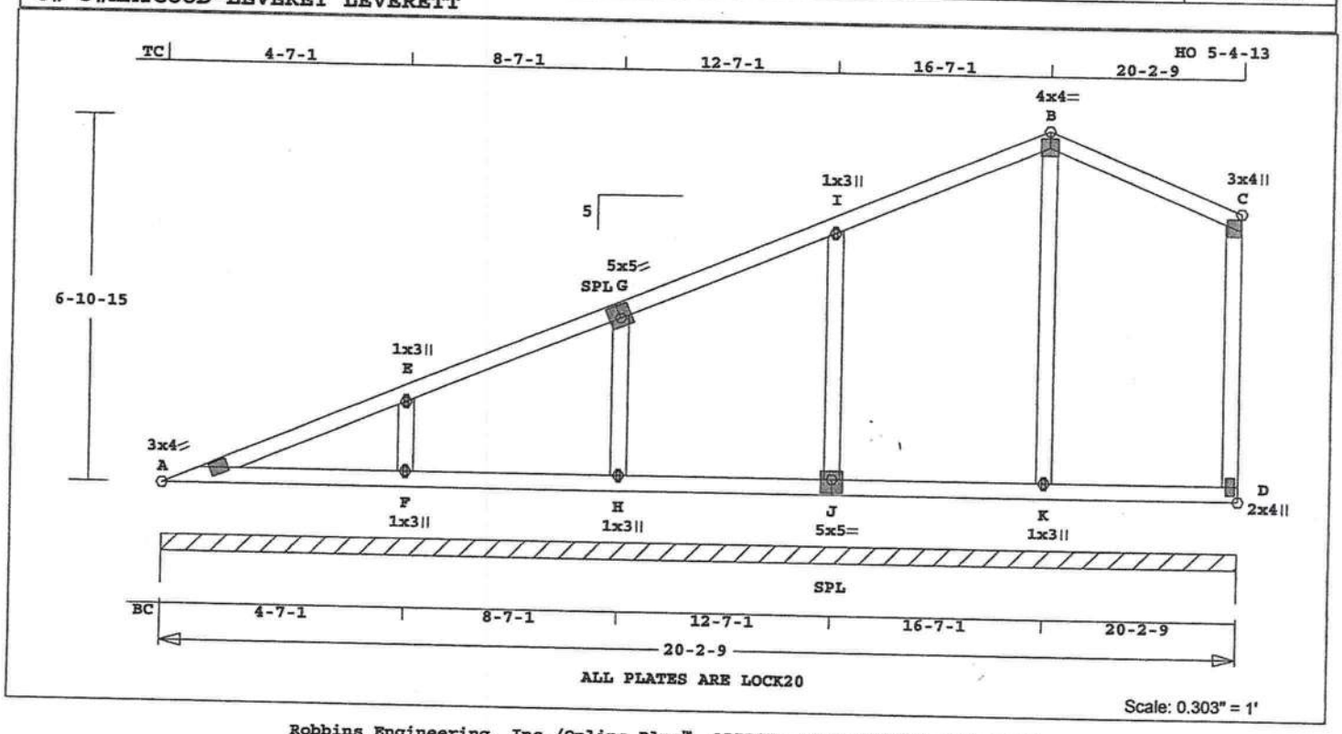
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 398 LBS.
5. FASTEN TRUSS TO BRG D FOR 105 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG F FOR 145 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG H FOR 179 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG J FOR 173 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
9. FASTEN TRUSS TO BRG L FOR 187 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
10. FASTEN TRUSS TO BRG M FOR 153 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V2	1	VLS.SB	200209	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 116.7 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.21	2X 4 SP-#2	1720
BTM	0.11	2X 4 SP-#2	1720
WBS	0.18	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-E	0.21	233 C	-279	537
E-G	0.20	181 C	-537	373
G-I	0.17	128 C	-373	455
I-B	0.16	119 T	-319	340
B-C	0.12	104 T	-247	3
	BOTTOM CHORDS			
A-F	0.11	91 C	279	79
F-H	0.03	91 C	-79	-21
H-J	0.01	91 C	21	5
J-K	0.01	91 C	-5	-1
K-D	0.01	91 C	1	0
	WEBS			
D-C	0.17	101 T	0	0
F-E	=	278 C	H-G =	227 C
J-I	=	253 C	K-B =	232 C

DL+LL DEFL = 0.03" IN A-E
LL DEFL < BRG-SPAN/240

SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	6.2	3.2
B	3001	4.00 X	4.00	2.0	2.2
C	4100	3.00 X	4.00	CTR	CTR
D	4000	2.00 X	4.00	CTR	CTR
E	1001	1.00 X	3.00	CTR	CTR
F	1001	1.00 X	3.00	CTR	CTR
G	1102	5.00 X	5.00	CTR	3.0
H	1001	1.00 X	3.00	CTR	CTR
I	1001	1.00 X	3.00	CTR	CTR
J	1102	5.00 X	5.00	CTR	3.0
K	1001	1.00 X	3.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

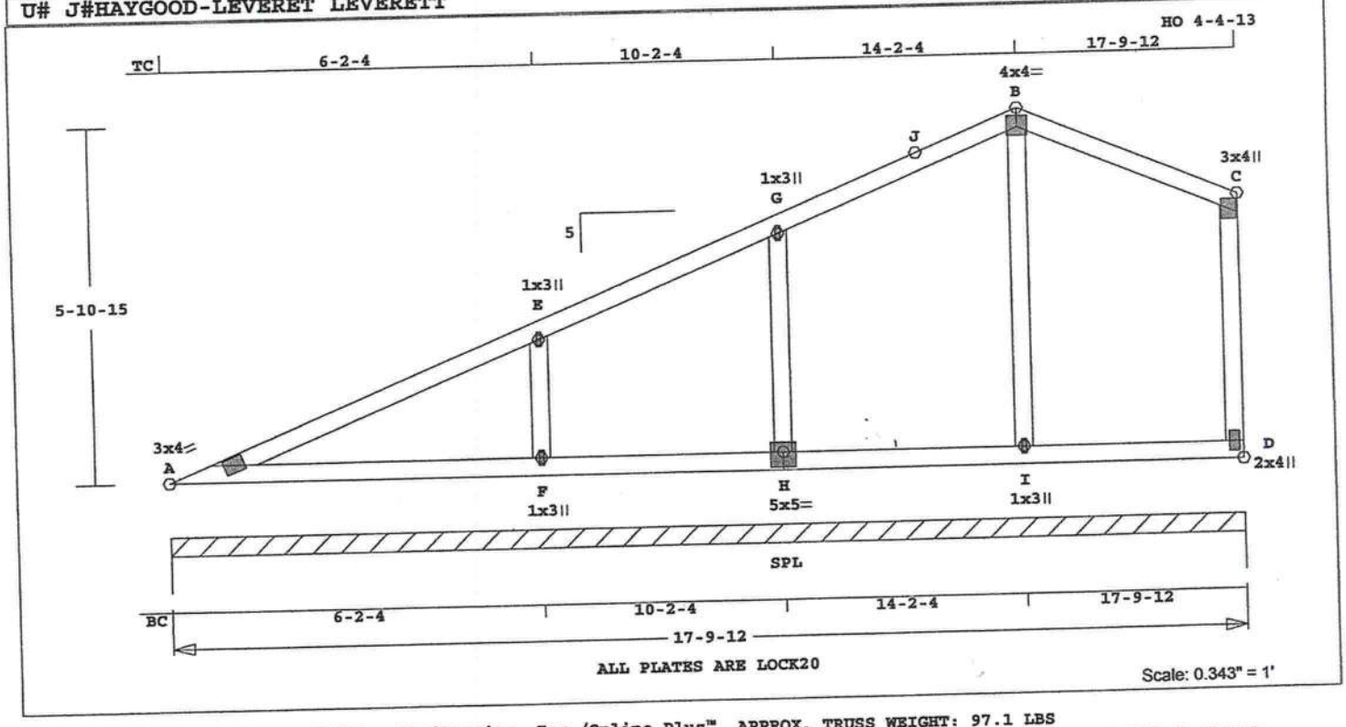
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 332 LBS.
5. FASTEN TRUSS TO BRG D FOR 100 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG F FOR 205 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG H FOR 162 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG J FOR 190 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
9. FASTEN TRUSS TO BRG K FOR 138 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V3	1	VLS.SB	170912	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 97.1 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.33	2X 4 SP-#2	1720
BTM	0.20	2X 4 SP-#2	1720
WBS	0.14	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-E	0.33	172 C	-553	870
E-G	0.32	104 C	-870	325
G-J	0.11	102 T	-219	-164
J-B	0.11	110 T	164	351
B-C	0.12	91 T	-312	5
	BOTTOM CHORDS			
A-F	0.20	73 C	553	177
F-H	0.07	73 C	-177	-47
H-I	0.02	73 C	47	12
I-D	0.01	73 C	-12	0
	WEBS			
D-C	0.11	97 C	0	0
F-E =	344 C	H-G =	213 C	
I-B =	241 C			

DL+LL DEFL = 0.10" IN A-E
LL DEFL < BRG-SPAN/240

SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	6.2	3.2
B	3001	4.00 X	4.00	2.0	2.2
C	4100	3.00 X	4.00	CTR	CTR
D	4000	2.00 X	4.00	CTR	CTR
E	1001	1.00 X	3.00	CTR	CTR
F	1001	1.00 X	3.00	CTR	CTR
G	1001	1.00 X	3.00	CTR	CTR
H	1102	5.00 X	5.00	CTR	3.0
I	1001	1.00 X	3.00	CTR	CTR
J					

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

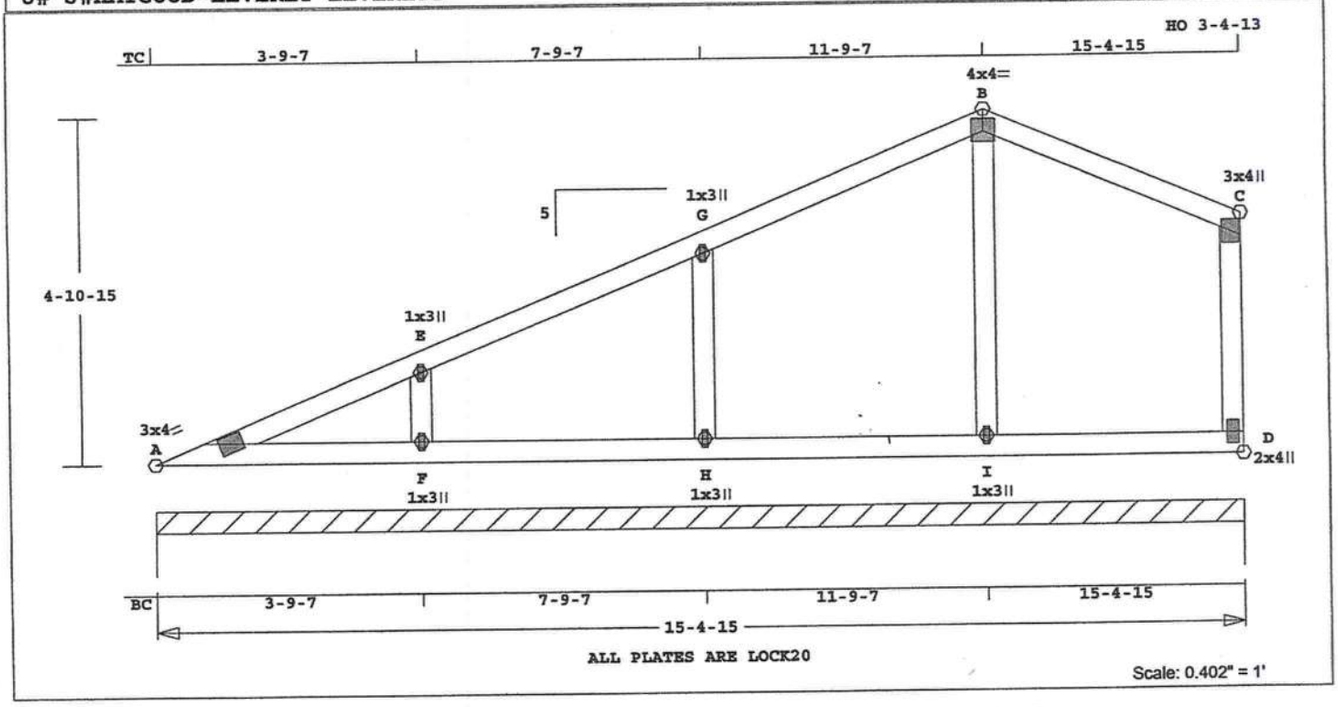
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 267 LBS.
5. FASTEN TRUSS TO BRG D FOR 94 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG F FOR 267 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG H FOR 151 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG I FOR 129 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V4	1	VLS.SB	150415	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 80.1 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.16	2X 4 SP-#2	1720
BTM	0.07	2X 4 SP-#2	1720
WBS	0.10	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-E	0.16	136 C	-172	415
E-G	0.16	89 C	-415	441
G-B	0.16	93 T	-314	335
B-C	0.12	78 T	-294	5
	BOTTOM CHORDS			
A-F	0.07	56 C	172	45
F-H	0.02	56 C	-45	-12
H-I	0.01	56 C	12	3
I-D	0.01	56 C	-3	0
	WEBS			
D-C	0.07	98 C	0	0
F-E	=	246 C	H-G =	249 C
I-B	=	233 C		

DL+LL DEFL = 0.02" IN B-C
LL DEFL < BRG-SPAN/240

SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

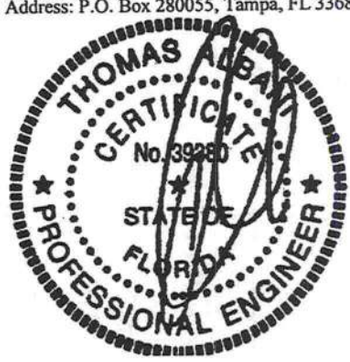
JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	6.2	3.2
B	3001	4.00 X	4.00	2.0	2.2
C	4100	3.00 X	4.00	CTR	CTR
D	4000	2.00 X	4.00	CTR	CTR
E	1001	1.00 X	3.00	CTR	CTR
F	1001	1.00 X	3.00	CTR	CTR
G	1001	1.00 X	3.00	CTR	CTR
H	1001	1.00 X	3.00	CTR	CTR
I	1001	1.00 X	3.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

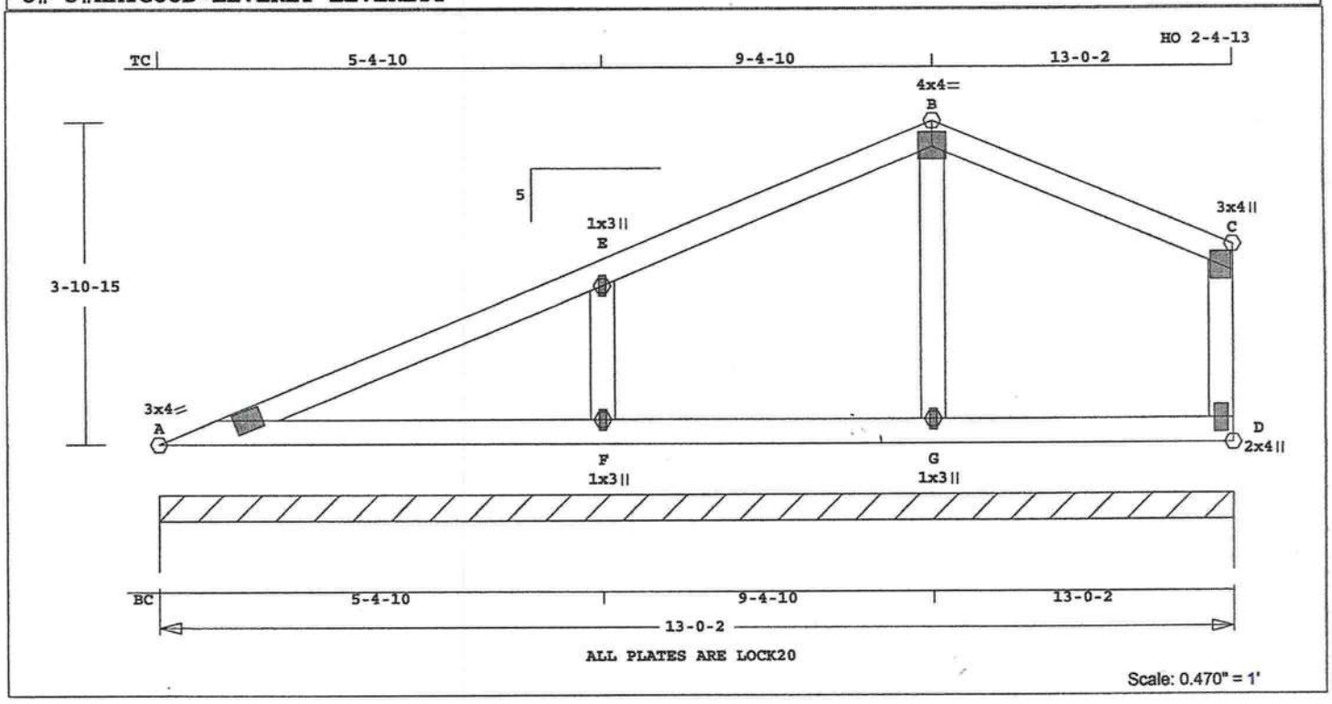
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 201 LBS.
5. FASTEN TRUSS TO BRG D FOR 90 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG F FOR 178 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG H FOR 184 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG I FOR 110 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V5	1	VLS.SB	130002	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 63.1 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.27	2X 4 SP-#2	1720
BTM	0.15	2X 4 SP-#2	1720
WBS	0.06	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-E	0.27	75 C	-400	704
E-B	0.26	82 T	-530	302
B-C	0.13	65 T	-256	5
	BOTTOM CHORDS			
A-F	0.15	38 C	400	121
F-G	0.05	38 C	-121	-32
G-D	0.01	38 C	32	0
	WEBS			
D-C	0.03	101 C	0	0
F-E	=	318 C	G-B =	213 C

DL+LL DEFL = 0.05" IN A-E
LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

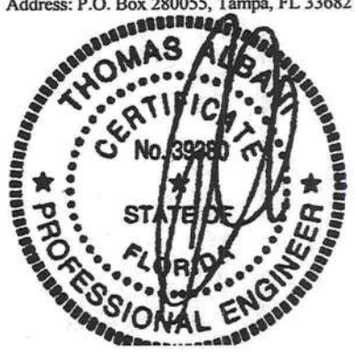
JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	6.2	3.2
B	3001	4.00 X	4.00	2.0	2.2
C	4100	3.00 X	4.00	CTR	CTR
D	4000	2.00 X	4.00	CTR	CTR
E	1001	1.00 X	3.00	CTR	CTR
F	1001	1.00 X	3.00	CTR	CTR
G	1001	1.00 X	3.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

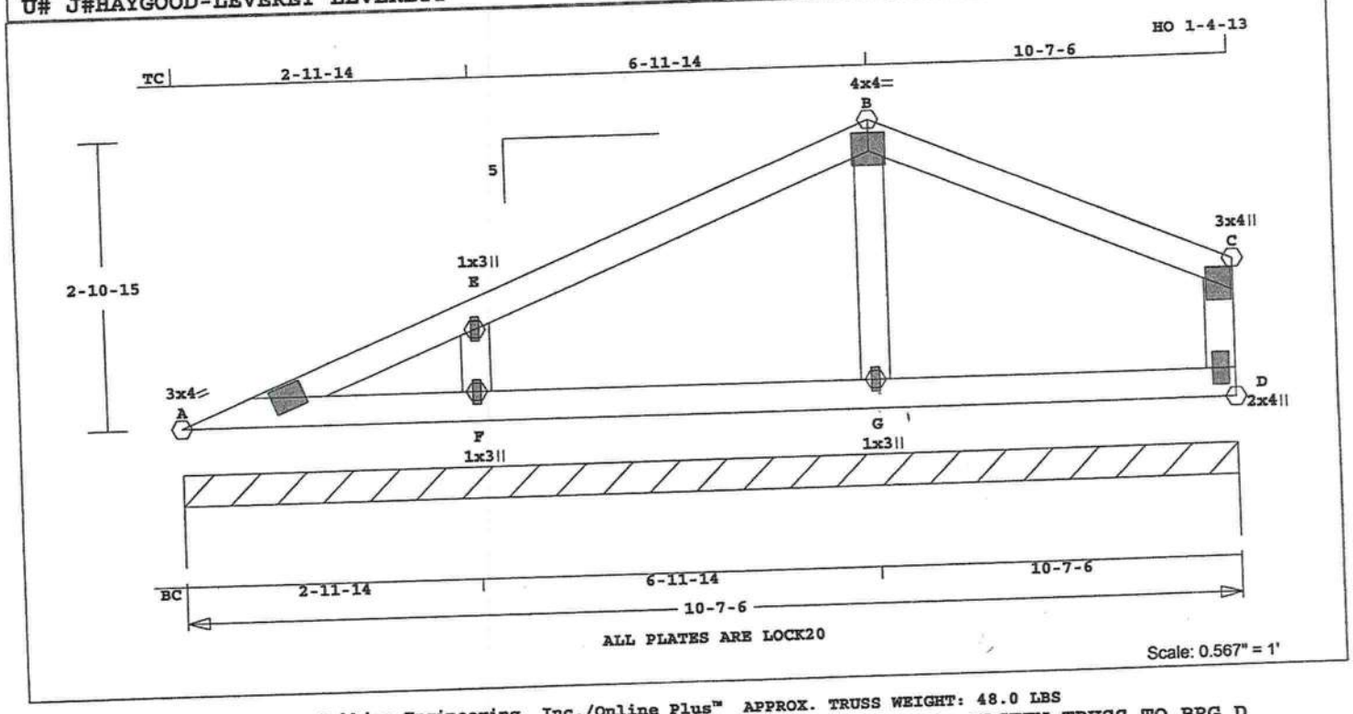
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 135 LBS.
5. FASTEN TRUSS TO BRG A FOR 55 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG D FOR 86 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG F FOR 249 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
8. FASTEN TRUSS TO BRG G FOR 84 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V6	1	VLS.SB	100706	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.0 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.15	2X 4 SP-#2	1720
BTM	0.03	2X 4 SP-#2	1720
WBS	0.04	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-E	0.15	52 C	-106	712
E-B	0.14	66 T	-267	338
B-C	0.12	51 T	-298	5
BOTTOM CHORDS				
A-F	0.03	21 C	68	15
F-G	0.01	21 C	-15	-4
G-D	0.00	21 C	4	0
WEBS				
F-E =		237 C	G-B =	235 C
D-C =		98 C		

DL+LL DEFL = 0.02" IN B-C
LL DEFL < BRG-SPAN/240

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

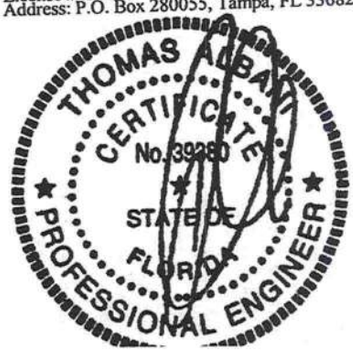
JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	6.2	3.2
B	3001	4.00 X	4.00	2.0	2.2
C	4100	3.00 X	4.00	CTR	CTR
D	4000	2.00 X	4.00	CTR	CTR
E	1001	1.00 X	3.00	CTR	CTR
F	1001	1.00 X	3.00	CTR	CTR
G	1001	1.00 X	3.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

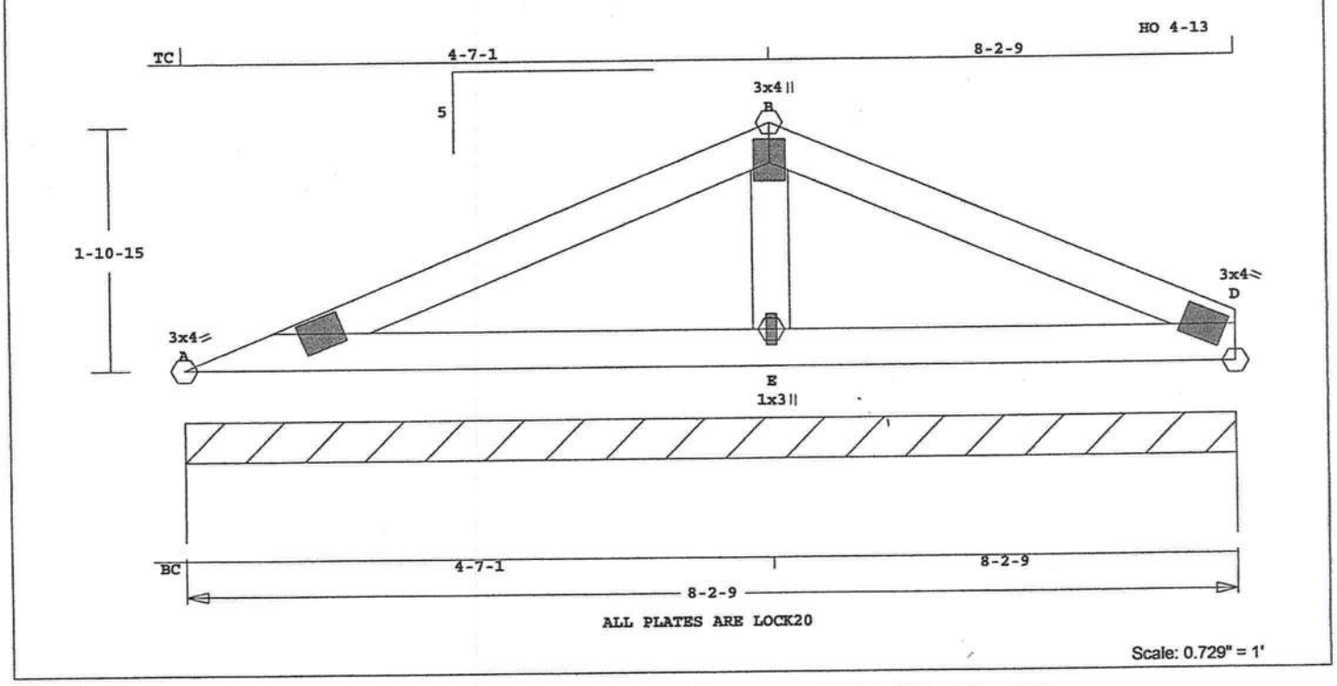
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. FASTEN TRUSS TO BRG D FOR 80 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
5. FASTEN TRUSS TO BRG F FOR 175 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG G FOR 82 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V7	1	VLS.SB	80209	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.4 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.16	2X 4 SP-#2	1720
BTM	0.14	2X 4 SP-#2	1720
WBS	0.02	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

	LEFT	RIGHT
GIRD	0IN - 0SX	4IN - 13SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
		TOP CHORDS		
A-B	0.16	66 T	-259	315
B-D	0.10	56 T	-293	178
		BOTTOM CHORDS		
A-E	0.14	53 T	-850	-336
E-D	0.07	53 T	336	377
		WEBS		
E-B	=	207 C		

DL+LL DEFL = 0.04" IN A-B
LL DEFL = 0.00" < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691

ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 4.00	6.2	3.2
B	3001R	3.00 X 4.00	2.2	1.5
D	2005	3.00 X 4.00	4.7	2.6
E	1001	1.00 X 3.00	CTR	CTR

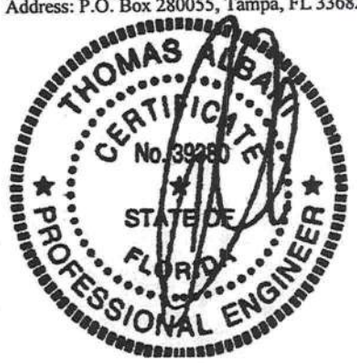
R = PLATE IS ROTATED BY 90 DEG

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

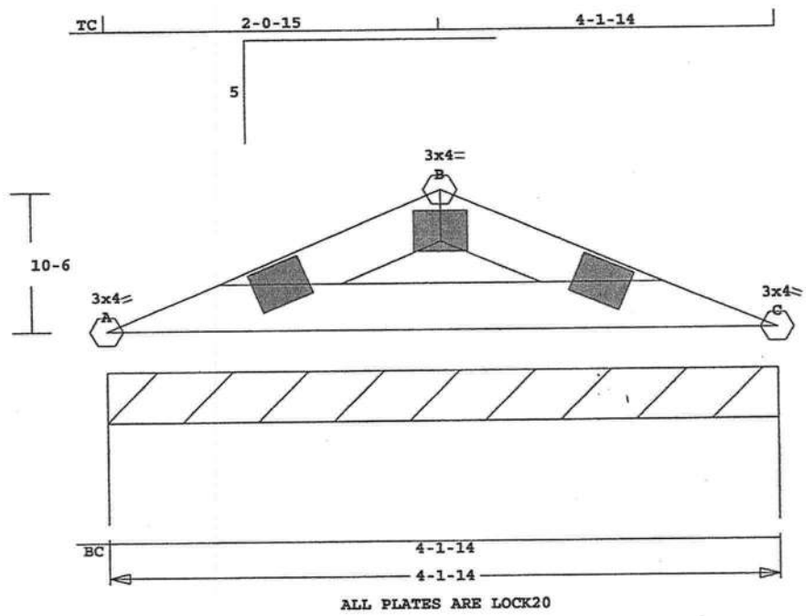
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. FASTEN TRUSS TO BRG A FOR 74 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
5. FASTEN TRUSS TO BRG D FOR 77 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG E FOR 125 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V8	1	VL.SB	40114	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



ALL PLATES ARE LOCK20

Scale: 0.918" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 13.5 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.04	2X 4 SP-#2	1720
BTM	0.04	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 0SX	0IN - 0SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-B	0.04	176 C	94	-247
B-C	0.04	176 C	247	-94
	BOTTOM CHORDS			
A-C	0.04	165 T	-94	94
	WEBS			

DL+LL DEFL = 0.00" IN B-C
LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

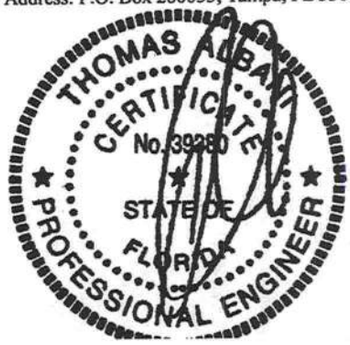
JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 4.00	6.2	3.2
B	3000	3.00 X 4.00	2.0	2.2
C	2003	3.00 X 4.00	6.2	3.2

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

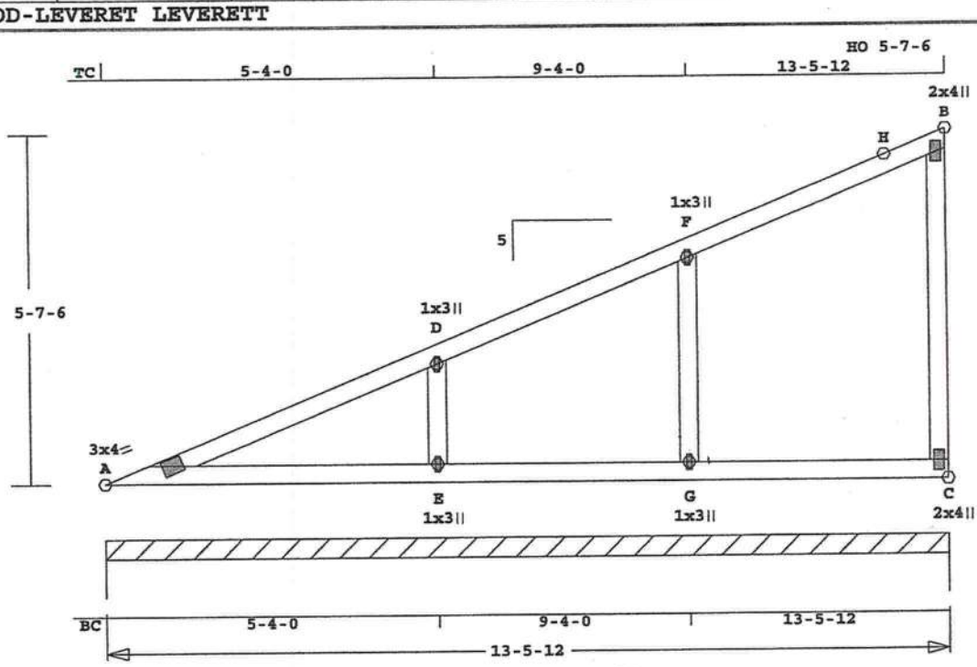
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. FASTEN TRUSS TO BRG A FOR 62 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
5. FASTEN TRUSS TO BRG C FOR 62 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V9	1	VLM.SB	130512	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 70.5 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.25	2X 4 SP-#2	1720
BTM	0.15	2X 4 SP-#2	1720
WBS	0.06	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
		TOP CHORDS			
A-D	0.25	216	C	-693	1112
D-F	0.24	126	C	-1112	776
F-H	0.16	41	C	-776	-533
H-B	0.11	33	C	533	5
		BOTTOM CHORDS			
A-E	0.15	0	T	-989	-298
E-G	0.05	0	T	298	74
G-C	0.01	0	T	-74	0
		WEBS			

E-D = 300 C G-F = 251 C
C-B = 108 C

DL+LL DEFL = 0.05" IN A-D
LL DEFL < BRG-SPAN/240

SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

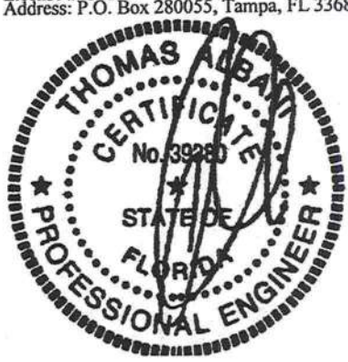
JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	6.2	3.2
B	4200	2.00 X	4.00	CTR	CTR
C	4000	2.00 X	4.00	CTR	CTR
D	1001	1.00 X	3.00	CTR	CTR
E	1001	1.00 X	3.00	CTR	CTR
F	1001	1.00 X	3.00	CTR	CTR
G	1001	1.00 X	3.00	CTR	CTR
H					

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

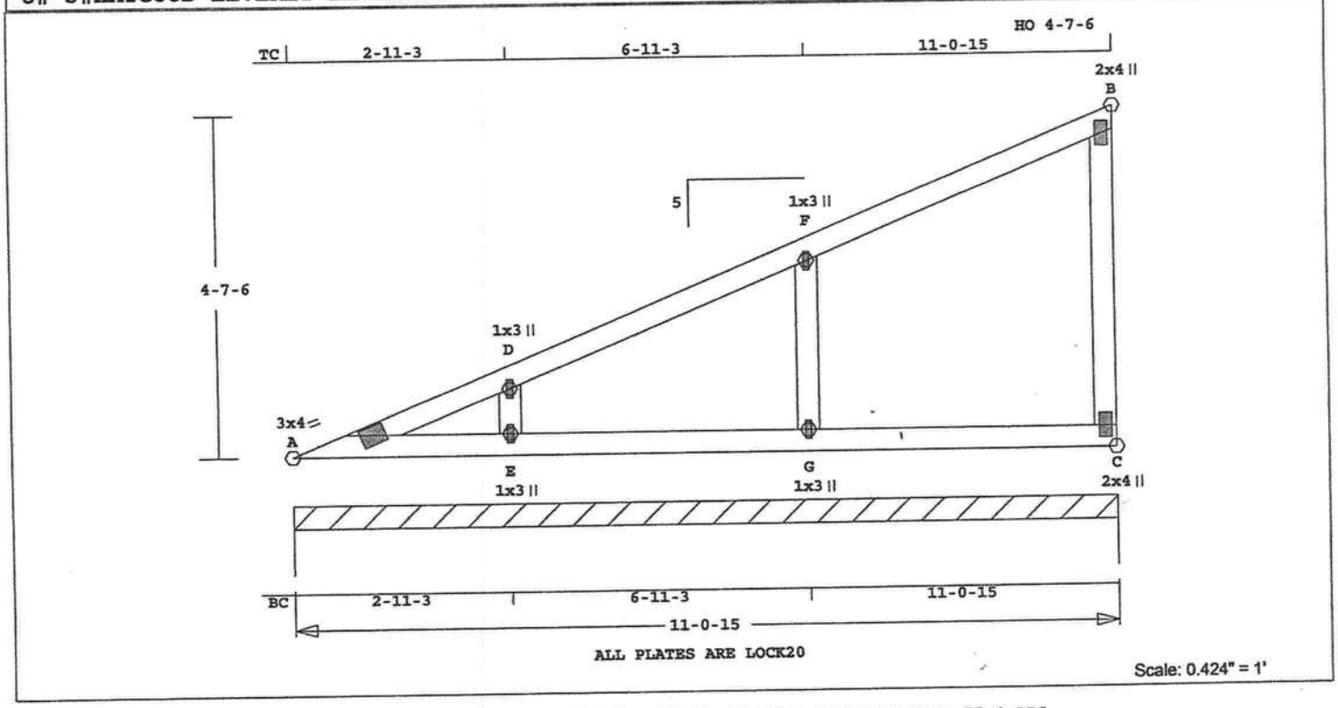
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM FOR EXTERIOR ZONE LOCATION
WIND SPEED - 110 MPH
MEAN ROOF HEIGHT - 15'
EXPOSURE CATEGORY - B
OCCUPANCY FACTOR - 1.00
ENCLOSED BUILDING.
TC DEAD LOAD = 5.0 PSF
BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 245 LBS.
5. FASTEN TRUSS TO BRG C FOR 79 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG E FOR 225 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG G FOR 179 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V10	1	VLM.SB	110015	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 55.4 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.20	2X 4 SP-#2	1720
BTM	0.03	2X 4 SP-#2	1720
WBS	0.04	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-D	0.12	190	C	-141	542
D-F	0.20	132	C	-542	918
F-B	0.19	38	C	-918	5
BOTTOM CHORDS					
A-E	0.03	0	T	-216	-49
E-G	0.01	0	T	49	12
G-C	0.00	0	T	-12	0

WEBS
E-D = 214 C G-F = 276 C
C-B = 104 C

DL+LL DEFL = 0.02" IN F-B
LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2003	3.00 X	4.00	6.2	3.2
B	4200	2.00 X	4.00	CTR	CTR
C	4000	2.00 X	4.00	CTR	CTR
D	1001	1.00 X	3.00	CTR	CTR
E	1001	1.00 X	3.00	CTR	CTR
F	1001	1.00 X	3.00	CTR	CTR
G	1001	1.00 X	3.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

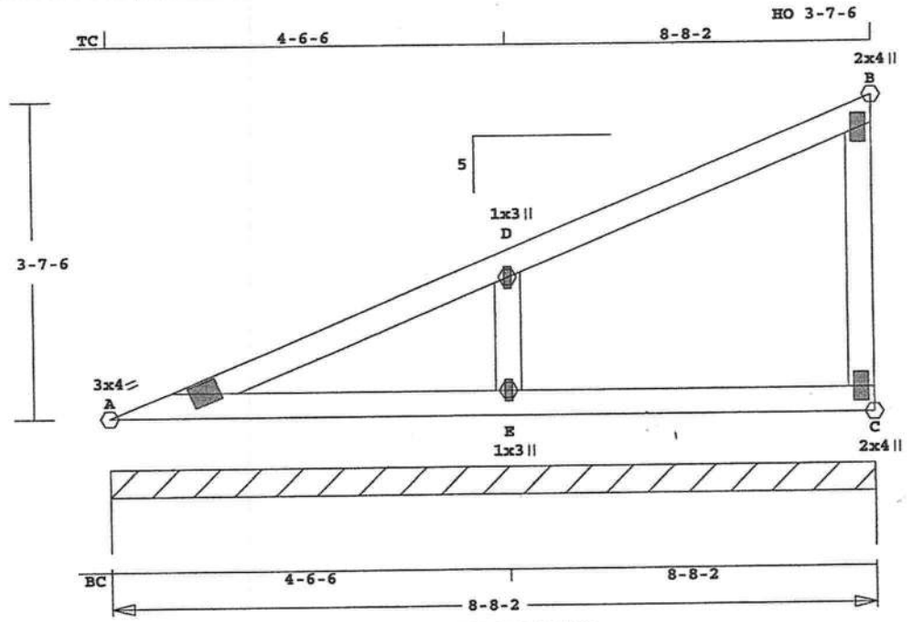
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 199 LBS.
5. FASTEN TRUSS TO BRG C FOR 76 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG E FOR 141 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
7. FASTEN TRUSS TO BRG G FOR 202 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V11	1	VLM.SB	80802	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



ALL PLATES ARE LOCK20

Scale: 0.502" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.9 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.24	2X 4 SP-#2	1720
BTM	0.09	2X 4 SP-#2	1720
WBS	0.03	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-D	0.24	128 C	-418	1065
D-B	0.23	37 C	-1065	5
BOTTOM CHORDS				
A-E	0.09	0 T	-609	-161
E-C	0.02	0 T	161	0
WEBS				
E-D =	303 C	C-B =	98 C	

DL+LL DEFL = 0.02" IN A-D
LL DEFL < BRG-SPAN/240
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

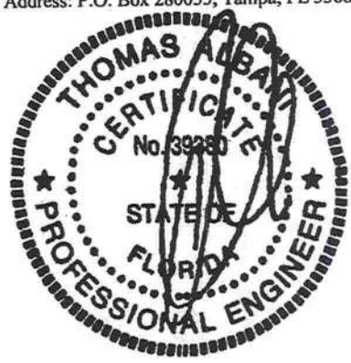
JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 4.00	6.2	3.2
B	4200	2.00 X 4.00	CTR	CTR
C	4000	2.00 X 4.00	CTR	CTR
D	1001	1.00 X 3.00	CTR	CTR
E	1001	1.00 X 3.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

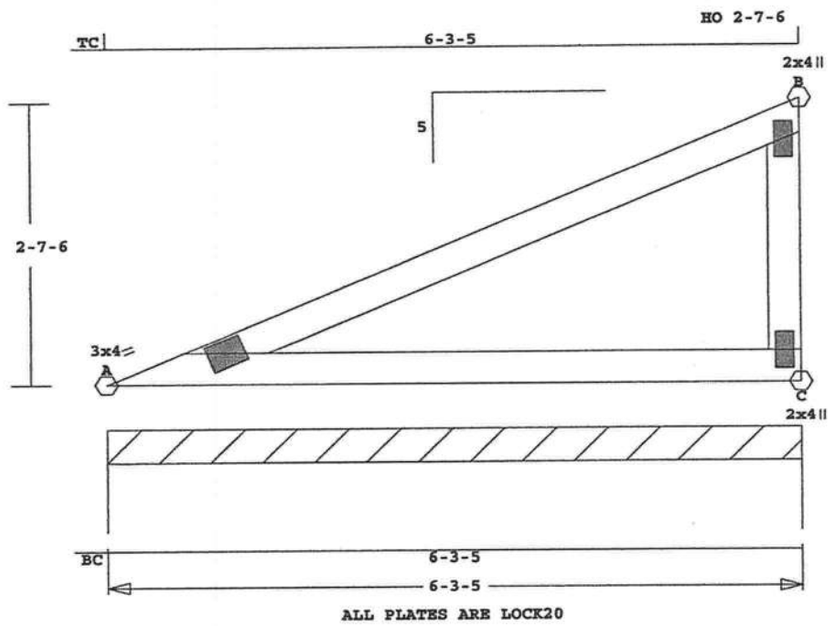
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 152 LBS.
5. FASTEN TRUSS TO BRG C FOR 70 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG E FOR 223 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V12	1	VLM.SB	60305	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



Scale: 0.631" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 27.0 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.39	2X 4 SP-#2	1720
BTM	0.26	2X 4 SP-#2	1720
WBS	0.02	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-B	0.39	53 C	-1232	5
	BOTTOM CHORDS			
A-C	0.26	0 T	-1742	0
	WEBS			
C-B	=	169 C		

DL+LL DEFL = 0.16" IN A-B
LL DEFL < BRG-SPAN/240

SPAN/DEFL (DL+LL) = 459

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 4.00	6.2	3.2
B	4200	2.00 X 4.00	CTR	CTR
C	4000	2.00 X 4.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

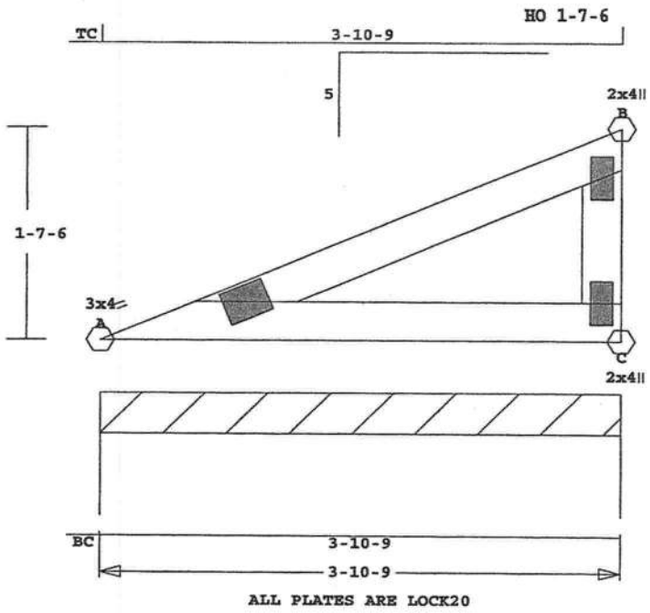
NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   FOR EXTERIOR ZONE LOCATION
   WIND SPEED - 110 MPH
   MEAN ROOF HEIGHT - 15'
   EXPOSURE CATEGORY - B
   OCCUPANCY FACTOR - 1.00
   ENCLOSED BUILDING.
   TC DEAD LOAD = 5.0 PSF
   BC DEAD LOAD = 5.0 PSF
4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 106 LBS.
5. FASTEN TRUSS TO BRG A FOR 90 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
6. FASTEN TRUSS TO BRG C FOR 136 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/22/2005

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-LEVERET	V13	1	VLM.SB	31009	5	0	0	T05082269

U# J#HAYGOOD-LEVERET LEVERETT



ALL PLATES ARE LOCK20

Scale: 0.765" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.7 LBS

Online Plus -- Version 17.8.021
RUN DATE: 8-22-05

	CSI	SIZE LUMBER	1.15FB
TOP	0.14	2X 4 SP-#2	1720
BTM	0.10	2X 4 SP-#2	1720
WBS	0.01	2X 4 SP-#2	1720

REPETITIVE MEMBER INCREASES:
FB 15.0% FT 0.0% FC 0.0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	20.0	10.0	
BTM CHD	0.0	10.0	
TOTAL	20.0	20.0	40.0

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & C

	LEFT	RIGHT
GIRD	0IN - 0SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-B	0.14	31 C	-443	5
	BOTTOM CHORDS			
A-C	0.10	0 T	-642	0
	WEBS			
C-B	=	107 C		

DL+LL DEFL = 0.02" IN A-B
LL DEFL < BRG-SPAN/240

SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
PLATES - 20 GAUGE LOCK
GRIPPING 632-312 PSI PER PAIR
INCLUDES 25.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

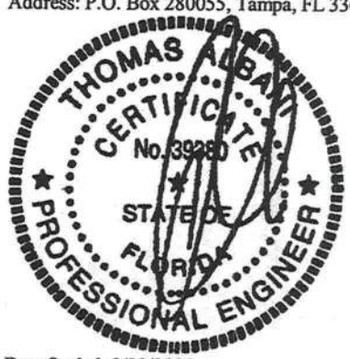
JT	TYPE	PLATE SIZE	X	Y
A	2003	3.00 X 4.00	6.2	3.2
B	4200	2.00 X 4.00	CTR	CTR
C	4000	2.00 X 4.00	CTR	CTR

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
1. TRUSSES MANUFACTURED BY - Mayo Truss Co. Inc.
2. EMPIRICAL ANALOG IS USED.
3. WIND LOADS - ANSI/ASCE 7-98
TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM FOR EXTERIOR ZONE LOCATION
WIND SPEED - 110 MPH
MEAN ROOF HEIGHT - 15'
EXPOSURE CATEGORY - B
OCCUPANCY FACTOR - 1.00
ENCLOSED BUILDING.
TC DEAD LOAD = 5.0 PSF
BC DEAD LOAD = 5.0 PSF
4. FASTEN TRUSS TO BRG A FOR 56 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.
5. FASTEN TRUSS TO BRG C FOR 83 LBS OF UPLIFT, WHILE PERMITTING NO UPWARD MOVEMENT OF WALL OR BRG.

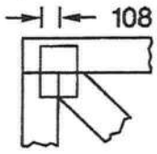
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/22/2005

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



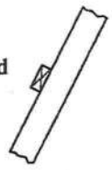
Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE

6.3 x 8.8

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

LATERAL BRACING

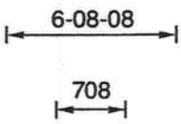


1x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION

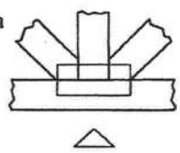
When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters. Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 or BCSI 1-03 as published by the Truss Plate Institute, 583 D'Onofrio Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Robbins Engineering, Inc.

CORPORATE HEADQUARTERS

P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

Short Form
Entire House
SHATTO HEATING & AIR, INC.

Job: JERRY & MARTHA LEV...
Date: Sep 01, 2005
By: KIM SHATTO

222 WEST MAIN STREET, LAKE BUTLER, FL 32054 Phone: 386-496-8224 Fax: 386-496-9065 Email: KIMSHATTO@SHATTOAIR.COM Web: WWW.SHATTOAIR.COM

Project Information

For: HAYGOOD HOMES
12592 S US HWY 441, LAKE CITY, FL 32025
Phone: 386-752-3496 Fax: 386-752-3496

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

HEATING EQUIPMENT

Make n/a
Trade n/a
Model n/a

Efficiency	n/a	
Heating input	0	Btuh
Heating output	0	Btuh
Temperature rise	0	°F
Actual air flow	1733	cfm
Air flow factor	0.046	cfm/Btuh
Static pressure	0.00	in H2O
Space thermostat		

COOLING EQUIPMENT

Make
Trade
Cond
Coil

Efficiency	14 EER	
Sensible cooling	0	Btuh
Latent cooling	0	Btuh
Total cooling	0	Btuh
Actual air flow	1733	cfm
Air flow factor	0.048	cfm/Btuh
Static pressure	0.00	in H2O
Load sensible heat ratio	0.85	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
MDBDR/BATH	520	3134	3001	145	144
UTILITY	72	940	5204	43	249
KITCH	297	8516	9772	394	468
DBRM2	176	3134	3282	145	157
BATH	128	0	660	0	32
DBRM3	176	3134	3001	145	144
LVRM	598	18596	11295	860	540
Entire House d	1967	37455	36215	1733	1733
Other equip loads		0	0		
Equip. @ 0.97 RSM			35129		
Latent cooling			6438		
TOTALS	1967	37455	41566	1733	1733

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

Series/Model: 8550
Type: PVC Horizontally Sliding Window (XO)

Title of Test	Results
AAMA Rating	HS-R40 72 x 48
Operating Force	Pass
Uniform Load Deflection Test Pressure	40.0 psf
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Pass
Corner Weld Test	Pass
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-46303.03
Report Date: 10/23/03
Expiration Date: 07/29/07

Architectural Testing

AAMA/WDMA 101/I.S. 2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No: 02-46303.03
Test Date: 07/29/03
Through: 07/31/03
Report Date: 10/23/02
Expiration Date: 07/30/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform testing on a 8550 horizontal slider (XO). The sample tested successfully met the performance requirements for a HS-R40* 72 x 48. Test specimen description and results are reported herein.

General Note: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the minimum test size for the product type and class. Refer to ATI Report No. 02-46303.01 for Gateway testing results.

Test Procedure: The test specimens were evaluated in accordance AAMA/WDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8550

Type: PVC horizontally sliding window (XO)

Overall Size: 5' 11-5/8" wide by 3' 11-5/8" high

Sash Sizes: 2' 11" wide by 3' 9" high

Screen Size: 2' 9-1/2" wide by 3' 8-1/4" high

Overall Size: 23.7 ft^2

Finish: All PVC was white

Glazing Type: The sashes were glazed with nominal 3/4" insulating glass comprised of two single strength annealed sheets separated by a desiccant-filled metal spacer system. The glass was set from the exterior a bed of silicone with PVC stops used on the interior.

849 Western Avenue North
Saint Paul, MN 55117-5245
phone: 651.636.3835
fax: 651.636.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.260" high by 0.187" backed pile with center fin	2 rows	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	1 row	All sash stiles

Frame Construction: Frame corners were miter cut and welded. Stationary meeting stiles utilized aluminum reinforcement and were secured to the head and sill with two 2-1/2" screws per end.

Sash Construction: Sash corners were miter cut and welded. Sash meeting stiles utilized aluminum reinforcement.

Screen Construction: The screen was comprised of roll-formed aluminum with plastic corner keys. Fiberglass screen cloth was attached with a vinyl spline.

Hardware:

Metal sweep lock with keepers	2	4-1/2" from ends on sash locking stile with keepers on fixed meeting stile
Plastic housed metal rollers	2	2" from ends on sash bottom rail

Drainage:

1/8" by 1/2" weep slot	4	Exterior face of sill

Installation: The window was installed within a 2" by 10" SPF #2 wood test frame and secured through the nail fin 1-5/8" screws located 6" from each corner and 6" to 8" on center. Silicone sealant was used to seal the frame to the buck.

Test Results:

Paragraph	Title of Test – Test Method	Results	Allowed
2.2.1.6.1	Operating Force*		
	Bottom Sash		
	Open	5 lbs	20 lbs max.
	Close	6 lbs	20 lbs max.

Test Results: (Continued)

Paragraph	Title of Test – Test Method	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-91* (See Note #1)		
	@ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	0.50 cfm/ft^2	--

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/WDMA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547-00 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher then the minimum required. Those results are listed under "Optional Performance".

2.1.7	Corner Weld Test*	Meets as Stated	Meets as Stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1-A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
2.2.1.6.2	Deglazing Test per ASTM E 987-88		
	In operating direction @ 70 lbs		
	Left Stile	0.10"/20%	0.500"/100%
	Right Stile	0.09"/19%	0.500"/100%
	In remaining direction at 50 lbs		
	Top Rail	0.08"/16%	0.500"/100%
	Bottom Rail	0.07"/14%	0.500"/100%

Optional Performance

4.3	Water Resistance per ASTM E 547-00 (With and without screen) WTP = 6.00 psf	No leakage	No leakage

Test Results: (Continued)

Paragraph	Title of Test – Test Method	Results	Allowed
4.4.1	Uniform Load at Design Pressure per ASTM E 330-97 (See note #3) (Measurements reported were taken on the meeting stile) (60 seconds)		
	@ 40.0 psf (positive)	0.48"	No damage
	@ 40.0 psf (negative)	0.41"	No damage

Note #3: The Uniform Load at Design test is not an AAMA/WDMA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting stile) (10 seconds)		
	@ 60.0 psf (positive)	0.02"	0.19" max.
	@ 60.0 psf (negative)	0.03"	0.19" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Eric J. Schoenthaler
Technician

Daniel A. Johnson
Regional Manager

EJS/jb
02-46303.03

DOCUMENT CONTROL ADDENDUM 02-46303.00

Current Issue Date: 10/23/03

Report No. 02-46303.01

Requested by: Darrel Booth, Jordan Companies
Purpose: AAMA/WDMA 101/I.S. 2-97 structural testing on an 8550 PVC Horizontal Slider 108 x 72 (XOX)
Issue Date: 10/23/03
Comments: Reports and drawings will be forwarded to ALI for AAMA certification.

Report No. 02-46303.02

Requested by: Darrel Booth, Jordan Companies
Purpose: AAMA/WDMA 101/I.S. 2-97 structural testing on an 8550 PVC Horizontal Slider 108 x 60 (XOX)
Issue Date: 10/23/03
Comments: Reports and drawings will be forwarded to ALI for AAMA certification.

Report No. 02-46303.03

Requested by: Darrel Booth, Jordan Companies
Purpose: AAMA/WDMA 101/I.S. 2-97 structural testing on an 8550 PVC Horizontal Slider 72 x 48 (XO)
Issue Date: 10/23/03
Comments: Reports and drawings will be forwarded to ALI for AAMA certification.

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: Series 8900
TYPE: PVC Fixed Window

Title of Test	Results
AAMA Rating	F-C50 60 x 78
Uniform Load Deflection Test Pressure	±50.0 psf
Air Infiltration	<0.01 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	±75.0 psf
Corner Weld Test	Pass
Forced Entry Resistance	Grade 40

Reference should be made to full report for test specimen description and data.

Report No: 02-46046.01
Report Date: 07/23/03
Expiration Date: 07/17/07

Architectural Testing

AAMA/WDMA 101/I.S. 2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
4661 Burbank Road, P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-46046.01
Test Date:	07/17/03
Report Date:	07/23/03
Expiration Date:	07/17/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies, to perform testing on Series 8900 PVC Fixed window. The sample tested successfully met the performance requirements for a F-C50 60 x 78 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimens were evaluated in accordance with AAMA/WDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: Series 8900

Type: PVC Fixed Window

Overall Size: 4' 11-3/4" wide by 6' 5-3/4" high

Area: 32.3 ft²

Finish: All vinyl was white.

Glazing Details: The window utilized a nominal 3/4" thick insulating glass unit fabricated from two nominal double strength sheets of annealed glass separated by a desiccant filled metal spacer system. The glass was set from the interior against a silicone sealant backbedding. PVC glazing stops were utilized on the interior.

Frame Construction: The frame corners were miter cut and welded.

Installation: The window was installed within a nominal 2" by 8" SPF wood test buck. The window was anchored to the buck with #8 by 1-5/8" wood screws spaced 6" from each corner and 8" to 10" on center. Silicone sealant was used to seal the window to the test buck.

849 Western Avenue North
Saint Paul, MN 55117-5245
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Test Results: The results are tabulated as follows:

Paragraph	Title of Test – Test Method	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-91 (See Note #1)		
	@ 1.57 psf (25 mph)	<0.01 cfm/ft^2	0.30 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	<0.01 cfm/ft^2	--

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/WDMA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547-00 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.1.7	Welded Corner Test	Pass	<100% break on weld
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type D		
	Grade 40		
	Lock Manipulation Test	No entry	No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00 and 331-00		
	WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3)		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 60 seconds)		
	@ 50.0 psf (positive)	0.04"	No Damage
	@ 50.0 psf (negative)	0.03"	No Damage
4.4.2	Uniform Load Structural per ASTM E 330-97		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 10 seconds)		
	@ 75.0 psf (positive)	<0.01"	0.16" max.
	@ 75.0 psf (negative)	<0.01"	0.16" max.

Note #3: The Uniform Load Deflection test is not an AAMA/WDMA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Eric J. Schoenthaler
Technician

Daniel A. Johnson
Regional Manager

EJS/mb
02-46046.01

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8540
TYPE: PVC Casement Window

Title of Test	Results
AAMA/WDMA Rating	C-R40 (36 x 72)
Uniform Load Deflection Test Pressure	± 40.0 psf
Air Infiltration	0.08 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Forced Entry Resistance	Pass Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48974.01
Report Date: 02/06/04
Expiration Date: 02/06/08

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48974.01
Test Dates:	01/13/04
Thru:	02/06/04
Report Date:	02/12/04
Expiration Date:	02/06/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8540 Casement Window. The sample tested successfully met the performance requirements for a C-R40 36 x 72 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8540

Type: PVC Casement Window

Overall Size: 3' 0" wide by 6' 0" high

Sash Size: 2' 10-1/4" wide by 5' 10-1/4" high

Finish: All PVC was white.

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two double-strength annealed sheets and a desiccant-filled metal spacer system. The glass was set from the exterior against a bed of silicone with PVC stops used on the exterior.

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.460" high pile with center fin	1 Row	Perimeter of sash exterior
Foam-filled vinyl bulb gasket	1 Row	Perimeter of sash interior
1/4" EPDM rubber bulb	1 Row	Perimeter of frame

Frame Construction: Frame corners were miter-cut and welded.

Sash Construction: Sash corners were miter-cut and welded.

Hardware:

Dual arm roto-operator	1	Sill
4-point lock with keepers on the sash	1	Locking jamb
Casement hinges	2	Top and bottom corner of sash on hinge side
Metal snubbers	2	24" from top and bottom on hinge side

Installation: The unit was installed into a grade 2 SPF 2" by 8" wood test buck and secured with 1-5/8" screws through the nail fin spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results:

The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-01 (See Note #1)		
	@ 1.57 psf (25 mph)	0.08 cfm/ft^2	0.3 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	0.13 cfm/ft^2	--

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA101/I.S.2-97 for air infiltration.

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

***Note #2**: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."*

2.2.5.6.1	Vertical Deflection Test @ 45lbs	0.09"	0.71"
2.2.5.6.2	Hardware Load Test @ 5lbs/ft²	No damage	No damage
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type B		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests B1 through B3	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00		
	WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3)		
	(Measurements reported were taken on the top rail)		
	(Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.10"	(See Note #3)
	@ 40.0 psf (negative)	0.30"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97		
	(Measurements reported were taken on the top rail)		
	(Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.01"	0.136" max.
	@ 60.0 psf (negative)	0.01"	0.136" max.

***Note #3**: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

PLS/jb
02-48974.01

DOCUMENT CONTROL ADDENDUM 02-48974.00

Current Issue Date: 02/12/04

Report No. 02-48974.01

Requested by: Darrel Booth, Jordan Companies
Purpose: AAMA/WDMA 101/I.S. 2-97 testing on a Jordan 8540 Casement
Issue Date: 02/12/04
Comments: Reports and drawings forwarded to ALI for AAMA certification.

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8500
TYPE: PVC Single Hung Window

Title of Test	Results
AAMA/WDMA Rating	H-R40 (44 x 84)
Uniform Load Deflection Test Pressure	± 40.0 psf
Operating Force	10 lbs max.
Air Infiltration	0.21 cfm/ft^2
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48976.02
Report Date: 02-26-04
Expiration Date: 02-25-08

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.archtest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48976.02
Test Date:	02/25/04
Report Date:	02/26/04
Expiration Date:	02/25/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8500 Single Hung Window. The sample tested successfully met the performance requirements for a H-R40 44 x 84 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8500

Type: PVC Single Hung Window

Overall Size: 3' 8" wide by 7' 0" high

Sash Size: 3' 4-3/8" wide by 2' 5" high

Fixed D.L.O. Size: 3' 4-3/4" wide by 4' 5" high

Screen Size: 3' 4-3/4" wide by 2' 4-1/4" high

Finish: All PVC was white

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.archtest.com

Test Specimen Description: (Continued)

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two single-strength annealed sheets in the operating sash and two double-strength sheets in the fixed lite and a desiccant-filled metal spacer system. The glass for the fixed area was set from the interior into a bed of silicone sealant with PVC stops used on the interior. The sash was glazed from the exterior into a bed of silicone sealant with PVC stops used on the exterior.

Weatherstripping:

Description	Quantity	Location
0.260" high by 0.187" backed pile with center fin	1 Row	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	2 Rows	Sash stiles

Frame Construction: Frame corners were miter-cut and welded. Aluminum reinforcement was utilized in the fixed meeting rail (Jordan part number H-2447).

Sash Construction: Sash corners were miter-cut and welded. Aluminum reinforcement was utilized in the top rail (Jordan part number H-2448).

Hardware:

Metal cam locks with keepers	2	6" from ends and meeting rail
Plastic tilt latches	2	Sash top rail corners
Metal tilt pins	2	Sash bottom rail corners
Block-and-tackle balances	2	One per jamb

Drainage:

3/16" by 5/8" slots	2	1-3/4" from ends in sill pocket to hollow below
1/8" by 1/2" slots	4	1-3/4" and 2" from each end through sill exterior face

Installation: The unit was installed into a Grade 2 SPF 2" by 8" wood test buck secured through the flange with 1-5/8" screws spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results: The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.2.1.6.1	Operating Force		
	Force to initiate motion	10 lbs	30 lbs max.
	Force to keep in motion	8 lbs	30 lbs max.
2.1.2	Air Infiltration per ASTM E 283-97 (See Note #1)		
	@ 1.57 psf (25 mph)	0.21 cfm/ft^2	0.30 cfm/ft^2

Note #1: The tested specimen meets the performance levels specified in AAMA/WDMA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction @ 70 lbs		
	Top rail	0.04"/ 8%	0.500"/100%
	Bottom rail	0.06"/12%	0.500"/100%
	In remaining direction @ 50 lbs		
	Left stile	0.04"/8%	0.500"/100%
	Right stile	0.03"/6%	0.500"/100%
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type A		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
Optional Performance:			
4.3	Water Resistance per ASTM E 547-97 WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the meeting rail) (Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.45"	(See Note #3)
	@ 40.0 psf (negative)	0.52"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.03"	0.16" max.
	@ 60.0 psf (negative)	0.03"	0.16" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

DAJ/jb
02-48976.02

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

Allen M. Reeves
1 APRIL 2002

ALLEN M. REEVES
CERTIFICATE
NO. 18354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen M. Reuss
1 APRIL 2002

REGISTERED
No. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES
CERTIFICATE
NO. 19256
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max

***Note #1**: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Exceeds L/175 for deflection, but passes all other test requirements.

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
NO. 1937
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.18" max.
	@ 70.8 psf (negative)	0.05"	0.18" max.

Allen N. Reeves
1 APRIL 2002

LEN N. REEVES
CERTIFICATE NO. 19356
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:nlb
01-41134.01

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
NO. 19334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

ALLEN N. REEVES
CERTIFICATE
NO. 19254
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
7 JUNE 2002

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
7 JUNE 2002

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 13354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.15"	0.41" max.
	@ 15.0 psf (negative)	0.29"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.29" max.
	@ 22.5 psf (negative)	0.01"	0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs		
	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC

Mark A. Hess
Technician

MAH:nlb
01-41641.01

Allen N. Reeves, P.E.
Director - Engineering Services
7 JUNE 2002

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Picture Window

Title of Test	Results
Rating	F-R45 60 x 80
Overall Design Pressure	+45.0 psf -47.2 psf
Air Infiltration	0.04 cfm/ft^2
Water Resistance	8.25 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41135.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
No. [illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650

Type: Aluminum Picture Window

Overall Size: 5' 0" wide by 6' 8" high

Daylight Opening Size: 4' 9-1/4" wide by 6' 5-1/4" high

Finish All aluminum was white.

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen M. Reeves
1 APR 12 2002

ALLEN M. REEVES
CERTIFICATE
NO. 11854
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
	Air Infiltration (ASTM E 283-91)		
	@ 1.57 psf (25 mph)	0.04 cfm/ft^2	0.3 cfm/ft^2 max.
	Water Resistance (ASTM E 547-00)		
	WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.01"	0.41" max.
	@ 34.7 psf (negative)	0.01"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.0"	0.29" max.
	@ 52.1 psf (negative)	0.01"	0.29" max.

Note #1: *The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

ALLEN N. REEVES
CERTIFICATE NO. 13554
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
	Forced Entry Resistance (ASTM F 588-97)		
	Type: D Grade: 10		
	Hand and Tool Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) WTP = 8.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.02"	0.41" max.
	@ 47.2 psf (negative)	0.02"	0.41" max.
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.01"	0.29" max.
	@ 70.8 psf (negative)	0.02"	0.29" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:nlb
01-41135.01

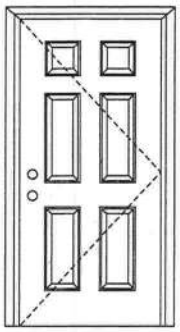
Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Single Door
Maximum unit size = 3'0" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure

+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

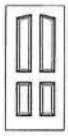

Flush


Arch Top 3-panel


3-panel

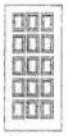

6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

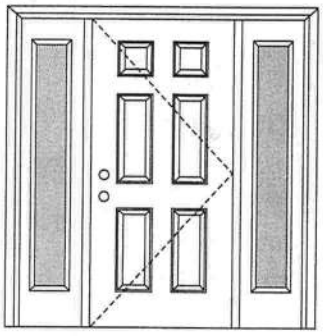
Exclusively from Masonite®
Masonite International Corporation

OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:


129 Series


200 Series


12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series


109 Series


120, 122 Series


300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft™ Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

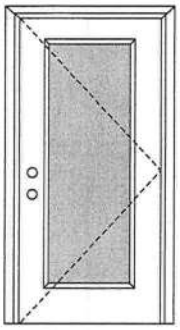
Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-MA0141-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+52.0/-52.0
Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

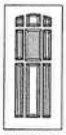
Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:

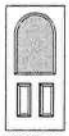

100 Series


133, 135 Series


136 Series

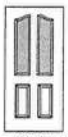

822 Series

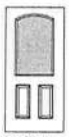
1/2 GLASS:


105 Series


106, 160 Series*


129 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door style: Eyebrow 5-panel with scroll.

Oakcraft™ Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

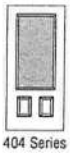
Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-MA0141-02

FIBERGLASS DOORS

APPROVED DOOR STYLES:
3/4 GLASS:

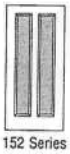

404 Series

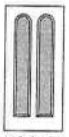

410 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

CTLA-805W-2

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

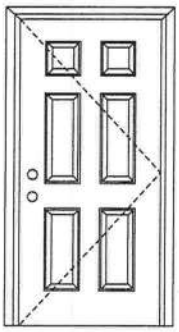
Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft
Wood-grain Textured
FIBERGLASS ENTRY DOORS

ARTEK™
Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOOR Collection
Premium Quality Doors

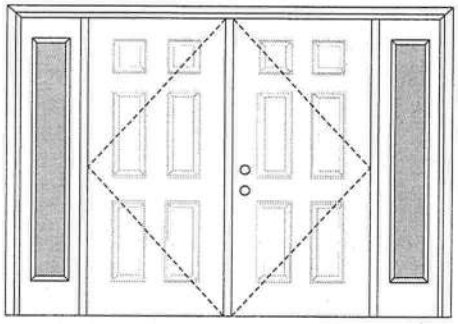
Exclusively from Masonite®
Masonite International Corporation

OXXO
Opaque Inswing Unit

COP-WL-MA0105-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door with 2 Sidelites
Maximum unit size = 8'4" x 6'8"

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0005-02 or MAD-WL-MA0008-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0005-02.

APPROVED DOOR STYLES:

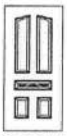

Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

OXXO
Opaque Inswing Unit

COP-WL-MA0105-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:


129 Series


200 Series


12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft™ Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

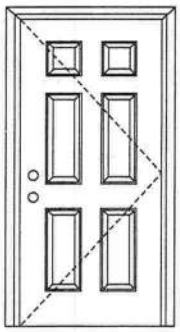
Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Single Door
Maximum unit size = 3'0" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure

+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

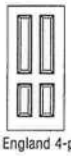
APPROVED DOOR STYLES:

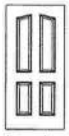

Flush


Arch Top 3-panel


3-panel

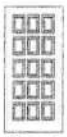

6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

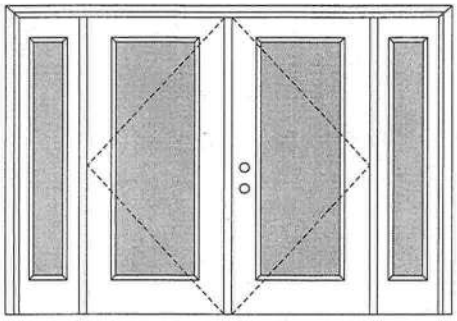
Exclusively from Masonite®
Masonite International Corporation

OXXO
Glazed Inswing Unit

COP-WL-MA0145-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door with 2 Sidelites
Maximum unit size = 12'0" x 6'8"

Design Pressure

+52.0/-52.0
Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0005-02 or MAD-WL-MA0008-02 and MAD-WL-MA0041-02.

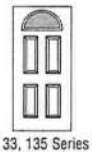
MINIMUM INSTALLATION DETAIL:

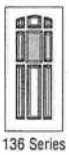
Compliance requires that minimum installation details have been followed – see MID-WL-MA0005-02.

APPROVED DOOR STYLES:

1/4 GLASS:

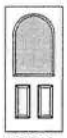

100 Series


133, 135 Series


136 Series

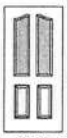

822 Series

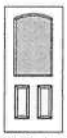
1/2 GLASS:


105 Series


106, 160 Series*


129 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door style: Eyebrow 5-panel with scroll.

Oakcraft Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

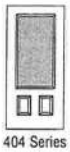
OXXO
Glazed Inswing Unit

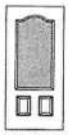
COP-WL-MA0145-02

FIBERGLASS DOORS

APPROVED DOOR STYLES:

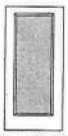
3/4 GLASS:


404 Series

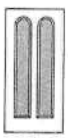

410 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

APPROVED SIDELITE STYLES:


129 Series


200 Series


12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series


109 Series


120, 122 Series


300 Series

CERTIFIED TEST REPORTS:

CTLA-805W-2

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft™ Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

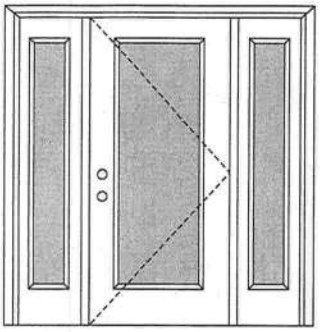
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

OXO
Glazed Inswing Unit

COP-WL-MA0144-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Design Pressure

+52.0/-52.0
Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

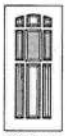
Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

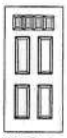
APPROVED DOOR STYLES:

1/4 GLASS:

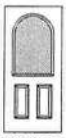

100 Series

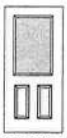

133, 135 Series

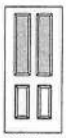

136 Series


822 Series

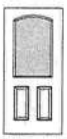
1/2 GLASS:


105 Series


106, 160 Series*


129 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door style: Eyebrow 5-panel with scroll.

Oakcraft Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

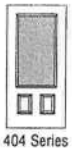
OXO
Glazed Inswing Unit

COP-WL-MA0144-02

FIBERGLASS DOORS

APPROVED DOOR STYLES:

3/4 GLASS:

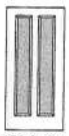

404 Series

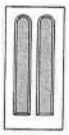

410 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

APPROVED SIDELITE STYLES:


129 Series


200 Series


12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series


109 Series


120, 122 Series


300 Series

CERTIFIED TEST REPORTS:

CTLA-805W-2

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft™ Wood-grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

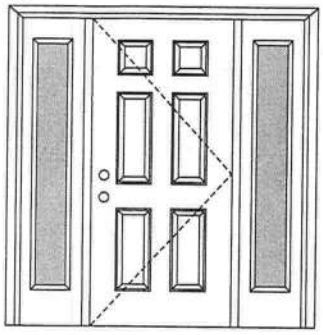
Exclusively from Masonite®
Masonite International Corporation

OXO
Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure

+57.0/-57.0 with maximum sidelite panel width of 1'2"
+45.0/-45.0 with maximum sidelite panel width of 3'0"
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panels, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


Arch Top 3-panel


3-panel


6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

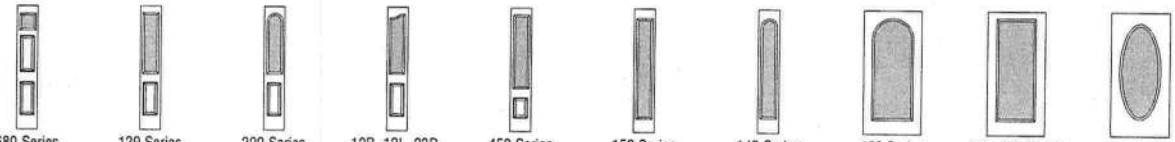
Exclusively from Masonite®
Masonite International Corporation

OXO
Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS

APPROVED SIDELITE STYLES:



680 Series | 129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1905-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL-210-1880-7, 9, 10, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

NOTICE OF INSPECTION AND/OR TREATMENT

#23688

Date of Inspection

11/05

Date of Treatment

Bora-care

Pesticide Used

subterranean Termites

Wood-Destroying Organisms Treated

Notice

It is a violation of Florida State Law (Chap. 482.226) for anyone other than the property owner to remove this notice.

Address:

Pestmaster Services of Lake City

879 S.W. Arlington Blvd., Suite 106 • Lake City, FL 32025

NOTICE OF INSPECTION AND/OR TREATMENT

23688

Date of Inspection
11/21/05

Date of Treatment
Bora - care

Pesticide Used
subterranean Termites

Wood-Destroying Organisms Treated

Notice
It is a violation of Florida State Law (Chap. 482.226) for anyone other than the property owner to remove this notice.

Address:
Pestmaster Services of Lake City
879 S.W. Arlington Blvd., Suite 106 • Lake City, FL 32025