07/11/2008

INSPECTORS OFFICE

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMI
000027160

APPLICANT JACOB KIRSCH	PHONE	344-4817	
ADDRESS 484 NW TURNER AVENUE	LAKE CITY	FL	32055
OWNER SOUTHEAST DEVELOPERS	PHONE	344-4817	
ADDRESS 298 SW MORNING GLORY DRIVE	LAKE CITY	<u>FL</u>	32024
CONTRACTOR JACOB KIRSCH	PHONE	344-4817	
LOCATION OF PROPERTY 90W, TL 247S, TL CALLAHAN	, TR HOPE HENRY, TR M	IORNING	
GLORY, 8TH ON RIGHT			
TYPE DEVELOPMENT SFD,UTILITY ES	STIMATED COST OF CO	NSTRUCTION 12	25050.00
HEATED FLOOR AREA 1787.00 TOTAL AR	EA2501.00	HEIGHT	STORIES 1
FOUNDATION CONC WALLS FRAMED	ROOF PITCH 8/12	FLOOR	SLAB
LAND USE & ZONING RSF-2	MAX.	HEIGHT 21	
Minimum Set Back Requirments: STREET-FRONT 25.00	REAR	15.00 SIDE	10.00
NO. EX.D.U. 0 FLOOD ZONE X PP	DEVELOPMENT PERM	IIT NO.	
PARCEL ID 15-4S-16-03023-552 SUBDIVISION S	ON ROLLING MEADO)WS	
LOT 52 BLOCK PHASE UNIT		L ACRES 0.50	
EOI 32 BEOCK PHASE CIVIT	TOTA	L'ACRES _0.50	
000001633 CBC1253775			
Culvert Permit No. Culvert Waiver Contractor's License Nu		Applicant/Owner/Contr	
CULVERT 08-0054 BK Driveway Connection Septic Tank Number LU & Zon	ing checked by Appr	roved for Issuance	Y New Resident
COMMENTS: PLAT REQUIRES MFE AT 107 FT., ELEVATION C		loved for issuance	New Resident
	OMEIDMATION I ETTED		
	ONFIRMATION LETTER		
REQUIRED AT SLAB		Check # or Cash	12110
REQUIRED AT SLAB			
REQUIRED AT SLAB FOR BUILDING & ZONI		ONLY	12110 (footer/Slab)
FOR BUILDING & ZONI Temporary Power Foundation			(footer/Slab)
FOR BUILDING & ZONI Temporary Power Foundation date/app. by	NG DEPARTMENT	ONLY Monolithic	(footer/Slab) date/app. by
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NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

CLERKS OFFICE

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.



COLUMBIA COUNTY, FLORIDA

artment of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 15-4S-16-03023-552

Fire: Building permit No. 000027160 64.20

Use Classification SFD, UTILITY

Waste: 167.50

Owner of Building SOUTHEAST DEVELOPERS

Permit Holder JACOB KIRSCH

Total: 231.70

Location: 298 SW MORNING GLOR DR, LAKE CITY, FL 32024

Date: 12/18/2008

POST IN A CONSPICUOUS PLACE (Business Places Only)

Building Inspector

WILLIAM N. KITCHEN

PROFESSIONAL SURVEYOR AND MAPPER 152 N. MARION AVENUE LAKE CITY, FLORIDA 32055 PHONE (386) 755-7786 FAX (386) 755-5506 E-MAIL BSSK@BELLSOUTH.NET



DATE: 7/17/2008

To Whom It May Concern:

RE: COMPASS BUILDER LOT 52 ROLLING MEADOWS

SUBJECT Parcel: 15-4S-16-03023-552

IS NOT IN A FLOOD ZONE ACCORDING TO FEMA FLOOD INSURANCE RATE MAP NO. 120070 0175 B DATED JANUARY 6, 1988.

AND THE TOP OF FINISH FLOOR = ELEVATION 107.0 FEET.

LOT 52 PER PLAT SHOWS A MINIMUM FLOOR ELEVATION OF 107.0 FEET.

Thank you,

WILLIAM N. KITCHEN PSM # 5490

7-17-2008

Project Name:

Address:

Owner:

City, State:

Compass Builders - Rae

Spec House

Lot: Sub: Rolling Meadows, Plat:

Lake City, FL 32025- (ot 52

Compass Builders

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

Permitting Office: Cour mass.
Permit Number: 27098

Jurisdiction Number: 22/000

Climate Zone: North			
New construction or existing	New _	12. Cooling systems	
2. Single family or multi-family		a. Central Unit	Cap: 37.0 kBtu/hr
3. Number of units, if multi-fam	ily 1 _		SEER: 13.00
4. Number of Bedrooms	3 _	b. N/A	_
5. Is this a worst case?	No _		
6. Conditioned floor area (fl²)	1787 ft²	c. N/A	_
	reqd. by 13-104.4.5 if not default)		_
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAU	JLT) 7a. (Dble Default) 253.7 ft ²	a. Electric Heat Pump	Cap: 37.0 kBtu/hr
b. SHGC:			HSPF: 7.70
(or Clear or Tint DEFAULT) 7b. (Clear) 253.7 ft ²	b. N/A	
8. Floor types			
 a. Slab-On-Grade Edge Insulation 	on R=5.0, 198.0(p) ft	c. N/A	
b. N/A	77 - 78 - 78 - 78 - 78 - 78 - 78 - 78 -		_
c. N/A		14. Hot water systems	_
Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Frame, Wood, Exterior	R=13.0, 1230.3 ft ²		EF: 0.90
 b. Frame, Wood, Adjacent 	R=13.0, 164.0 ft ²	b. N/A	
c. N/A	Silver Accessional Control of Silver		
d. N/A	-	c. Conservation credits	
e. N/A	-	(HR-Heat recovery, Solar	_
Ceiling types		DHP-Dedicated heat pump)	
a. Under Attic	R=30.0, 1950.0 ft ²	15. HVAC credits	PT,
b. N/A		(CF-Ceiling fan, CV-Cross ventilation,	3.33 <u></u>
c. N/A	-	HF-Whole house fan,	
11. Ducts	-	PT-Programmable Thermostat,	
a. Sup: Unc. Ret: Unc. AH: Into	erior Sup. R=6.0, 50.0 ft	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
	·	mas it manusone nearing)	
Glass/Floor A	Total as-built	points: 22115	
Glass/Floor A	Total base	points: 24303 PASS	
I hereby certify that the plans a	and specifications covered by	Review of the plans and	
this calculation are in compljar	ge with the Florida Energy	specifications covered by this	OF THE STATE
Code.	// //_	calculation indicates compliance	
PREPARED BY:	SIN	with the Florida Energy Code.	8 8
DATE: //-	9-08	Before construction is completed	
		this building will be inspected for	3 3
I hereby certify that this buildin	g, as designed, is in compliance	compliance with Section 553.908	
with the Florida Energy Code.		Florida Statutes.	GOD WE TRUST
OWNER/AGENT:	1-1/2	DIM DING OFFICIAL	WE I
	1/46	BUILDING OFFICIAL:	
DATE:	1110108	DATE:	

EnergyGauge® (Version: FLRCPB v4.5.2)

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

BASE		AS-BUILT	
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area	O Type/SC Orr	verhang nt Len Hgt Area X SPM X Se	OF = Points
.18 1787.0 18.59 5980.0	1.Double, Clear	W 1.5 10.0 15.0 38.52 0	.98 565.0
	2.Double, Clear	W 7.5 10.0 40.0 38.52 0	.60 928.0
	3.Double, Clear		.60 1253.0
	4.Double, Clear		.98 603.0
	5.Double, Clear		.98 226.0
	6.Double, Clear 7.Double, Clear		.97 371.0 .96 1208.0
	8.Double, Clear		.56 155.0
	9.Double, Clear		98 1234.0
	10.Double, Clear		92 993.0
	11.Double, Clear	S 1.5 8.0 6.0 35.87 0	92 198.0
	As-Built Total:	253.7	7734.0
WALL TYPES Area X BSPM = Points	Туре	R-Value Area X SPM	= Points
Adjacent 164.0 0.70 114.8	1. Frame, Wood, Exterior	13.0 1230.3 1.50	1845.5
Exterior 1230.3 1.70 2091.5	2. Frame, Wood, Adjacent	13.0 164.0 0.60	98.4
Base Total: 1394.3 2206.3	As-Built Total:	1394.3	1943.9
DOOR TYPES Area X BSPM = Points	Туре	Area X SPM	= Points
Adjacent 20.0 2.40 48.0	1.Exterior Insulated	20.0 4.10	82.0
Exterior 20.0 6.10 122.0	2.Adjacent Insulated	20.0 1.60	32.0
Base Total: 40.0 170.0	As-Built Total:	40.0	114.0
CEILING TYPES Area X BSPM = Points	Туре	R-Value Area X SPM X SCM	= Points
Under Attic 1787.0 1.73 3091.5	1. Under Attic	30.0 1950.0 1.73 X 1.00	3373.5
Base Total: 1787.0 3091.5	As-Built Total:	1950.0	3373.5
FLOOR TYPES Area X BSPM = Points	Туре	R-Value Area X SPM	= Points
Slab 198.0(p) -37.0 -7326.0 Raised 0.0 0.00 0.00	Slab-On-Grade Edge Insulation	5.0 198.0(p -36.20	-7167.6
Base Total: -7326.0	As-Built Total:	198.0	-7167.6
INFILTRATION Area X BSPM = Points		Area X SPM	= Points
1787.0 10.21 18245.3		1787.0 10.21	18245.3

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT							
Summer Ba	se Points: 2	22367.1	Summer As-Built Points: 24243.0							
Total Summer Points	X System : Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)							
22367.1	0.3250	7269.3	(sys 1: Central Unit 37000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 24243 1.00 (1.09 x 1.147 x 0.91) 0.260 0.950 6812.6 24243.0 1.00 1.138 0.260 0.950 6812.6							

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

BASE			AS-	BUI	LT			-		
GLASS TYPES .18 X Conditioned X BWPM = Po	oints	Type/SC C		hang Len	Hgt	Area X	WF	эм х	woi	= Points
.18 1787.0 20.17	6488.0	1.Double, Clear	w	1.5	10.0	15.0	2	0.73	1.01	312.0
		2.Double, Clear	W	7.5	10.0	40.0	2	0.73	1.13	940.0
		3.Double, Clear	W	7.5	10.0	54.0		0.73	1.13	1270.0
		4.Double, Clear	W	1.5	10.0	16.0		0.73	1.01	333.0
		5.Double, Clear	W	1.5	10.0	6.0		0.73	1.01	125.0 491.0
		6.Double, Clear 7.Double, Clear	N E	1.5 1.5	8.0	20.0 30.0		4.58 8.79	1.00	574.0
		8.Double, Clear	E	8.5	10.0	6.7		8.79	1.24	156.0
		9.Double, Clear	E	1.5	10.0	30.0		8.79	1.01	570.0
		10.Double, Clear	S	1.5	8.0	30.0		3.30	1.04	415.0
		11.Double, Clear	s	1.5	8.0	6.0	1	3.30	1.04	83.0
		As-Built Total:				253.7				5269.0
WALL TYPES Area X BWPM =	Points	Туре		R-	Value	Area	х	WPN	/ =	Points
Adjacent 164.0 3.60	590.4	1. Frame, Wood, Exterior			13.0	1230.3		3.40	(4183.0
Exterior 1230.3 3.70	4552.1	2. Frame, Wood, Adjacent			13.0	164.0		3.30		541.2
Base Total: 1394.3	5142.5	As-Built Total:				1394.3				4724.2
DOOR TYPES Area X BWPM =	Points	Туре				Area	Х	WPN	/ =	Points
Adjacent 20.0 11.50	230.0	1.Exterior Insulated				20.0		8.40		168.0
Exterior 20.0 12.30	246.0	2.Adjacent Insulated				20.0		8.00		160.0
Base Total: 40.0	476.0	As-Built Total:				40.0				328.0
CEILING TYPES Area X BWPM =	Points	Туре	R-	Value	Ar	ea X W	PM	x wo	CM =	Points
Under Attic 1787.0 2.05	3663.3	1. Under Attic			30.0	1950.0	2.05	X 1.00		3997.5
Base Total: 1787.0	3663.3	As-Built Total:				1950.0			-	3997.5
FLOOR TYPES Area X BWPM =	Points	Туре		R-	Value	Area	Х	WPM	1 =	Points
Slab 198.0(p) 8.9 Raised 0.0 0.00	1762.2 0.0	Slab-On-Grade Edge Insulat	ion		5.0	198.0(p		7.60		1504.8
Base Total:	1762.2	As-Built Total:				198.0				1504.8
INFILTRATION Area X BWPM =	Points					Area	х	WPM	1 =	Points
1787.0 -0.59	-1054.3					1787.0)	-0.59		-1054.3

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

	BASE		AS-BUILT							
Winter Base	Points:	16477.7	Winter As-Built Points:	14769.2						
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)							
16477.7	0.5540	9128.7	(sys 1: Electric Heat Pump 37000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),In 14769.2 1.000 (1.069 x 1.169 x 0.93) 0.443 0.950 14769.2 1.00 1.162 0.443 0.950	7221.4						

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

BASE						AS-BUILT							
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multiplie		
3		2635.00		7905.0	50.0	0.90	3		1.00	2693.56	1.00	8080.7	
					As-Built To	otal:						8080.7	

	CODE COMPLIANCE STATUS												
	BASE							AS-BUILT					
Cooling Points						Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	
7269		9129	27	7905		24303	6813		7221		8081		22115

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.2

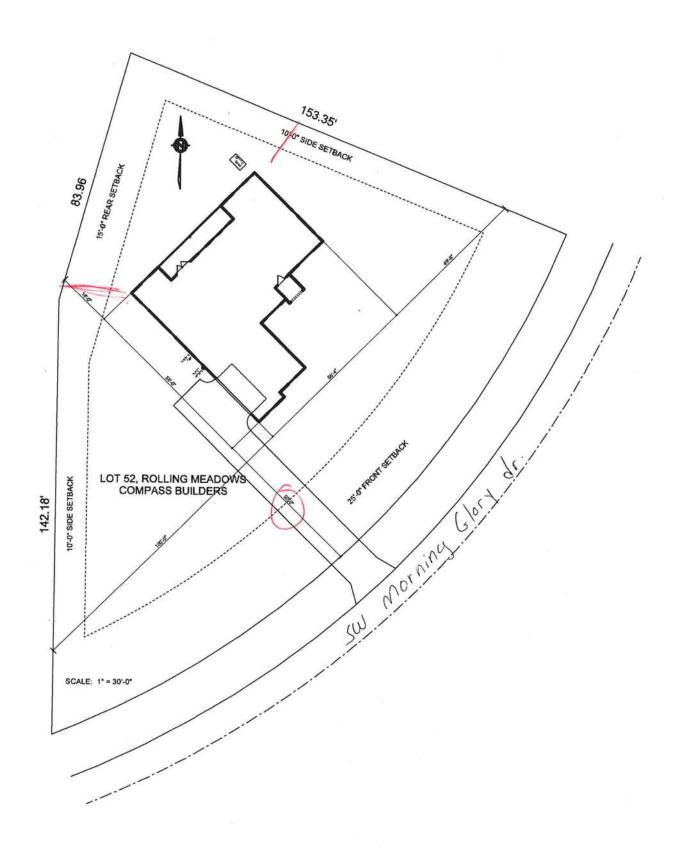
The higher the score, the more efficient the home.

Spec House, Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

1.	New construction or existing	New	12.	. Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 37.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	3		b. N/A	Sea part of the Conference of
5.	Is this a worst case?	No	_		-
6.	Conditioned floor area (ft²)	1787 ft²	_	c. N/A	
7.	Glass type 1 and area: (Label reqd.	by 13-104.4.5 if not default)	_		
a.	U-factor:	Description Area	13.	. Heating systems	_
	(or Single or Double DEFAULT)	7a. (Dble Default) 253 7 ft²		a. Electric Heat Pump	Cap: 37.0 kBtu/hr
b	. SHGC:	Bote Belluity 255.7 it	_		HSPF: 7.70
	(or Clear or Tint DEFAULT)	7b. (Clear) 253.7 ft ²	1	b. N/A	
8.	Floor types	(Clear) 255.7 It	_		
	Slab-On-Grade Edge Insulation	R=5.0, 198.0(p) ft		c. N/A	-
	. N/A	10 5.0, 150.0(p) 10		. 11/11	_
	N/A		- 14	. Hot water systems	_
9.	Wall types			a. Electric Resistance	Cap: 50.0 gallons
	Frame, Wood, Exterior	R=13.0, 1230.3 ft ²		a. Licette Resistance	EF: 0.90
	Frame, Wood, Adjacent	R=13.0, 164.0 ft ²	-	b. N/A	L1 . 0.70
	N/A	K-13.0, 104.0 K	-	D. N/A	
	N/A		1000 N	c. Conservation credits	100
	N/A		-	(HR-Heat recovery, Solar	
	Ceiling types		_	DHP-Dedicated heat pump)	
	Under Attic	R=30.0, 1950.0 ft ²	15	HVAC credits	РТ,
	. N/A	K-30.0, 1930.0 It	13.		
	N/A			(CF-Ceiling fan, CV-Cross ventilation	on,
	Ducts		_	HF-Whole house fan,	
	Sup: Unc. Ret: Unc. AH: Interior	Su- B-60 500 B		PT-Programmable Thermostat,	
	. N/A	Sup. R=6.0, 50.0 ft	_	MZ-C-Multizone cooling,	
υ.	. N/A		_	MZ-H-Multizone heating)	
I ce	rtify that this home has compli	ed with the Florida Energy	y Efficienc	ey Code For Building	
	struction through the above en				OF THE STATE
in th	his home before final inspection	n Otherwise a new FPI I	Display Co	ard will be completed	
has	ed on installed Code compliant	features	Display Co	ard will be completed	6
			ъ.		E SERVICE E
Bui	lder Signature:		Date:		[
Add	dress of New Home:		City/FL 2	Zip:	COD WE TRUST

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)



15-4S-16-03023-552 LOT 52 ROLLING MEADOWS S/D. WD 1063-1963. WD 1067-2450. CWD 1104-327

Corporate Warranty Deed

This Indenture, made this December 4, 2006 A.D.

Between

Burbach Investment Group, LLC, a Florida Limited Liability Company, whose post office address is: 507 W. Duval Street, Lake City, FL 32055; Grantor and Southeast Developers Group, Inc., a Florida Corporation whose post office address is: 197 SW Waterford Court, Lake City, Florida 32025

Witnesseth, that the said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00), to it in hand paid by the said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee forever, the following described land, situate, lying and being in the County of Columbia, State of Florida, to wit:

Lots 7, 48, 50 (52) and 54, ROLLING MEADOWS, according to the Plat thereof, recorded in Plat Book 8, Pages 45 and 46, of the Public Records of Columbia County, Florida.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number: R03023-507, R03023-548, R03023-550, R03023-552, and R03023-554

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2006.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

> **Burbach Investment Group, LLC** a Florida Limited Liability Company

Signed and Sealed in Our Presence:

MELINDA WEAVER

County of

Florida Columbia

The foregoing instrument was acknowledged before me this 4th day of December, 2006, by Thomas P Cady, Manager of Burbach Investment Group, LLC, a Florida Limited Liability Company, on behalf of the company.

He is personally known to me or has produced

as identification.

Notary Public Notary Printed Name:

My Commission Expires::

#06-0389 Prepared by & Return to: Matt Rocco Sierra Title, LLC, 619 SW Baya Drive, Suite 102 Lake City, Florida 32025

Inst:2006028778 Date:12/06/2006 Time:12:42

Doc Stamp-Deed : 1400.00

_DC,P.DeWitt Cason,Columbia County B:1104 P:327

Notary Public State of Florida

My Commission DD578349

Matthew Rocco

Expires 09/17/2010

COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787
Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

ADDRESS ASSIGNMENT DATA

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

Residential or Other Structure on Parcel Number: 15-4S-16-03023-552

Address Assignment: 298 SW MORNING GLORY DR, LAKE CITY, FL, 32024

Note: LOT 52 ROLLING MEADOWS S/D

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

10.00

Return To: Eddie Anderson 27160

THIS INSTRUMENT PREPARED BY & RETURN TO: Columbia Bank 173 NW Hillsboro Street Lake City, FL 32055

t;200812012851 Date:7/9/2008 Time:10:21 AM DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1154 P:438

NOTICE OF COMMENCEMENT

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property:

Lot 52 Rolling Meadows S/D, according to the Tax Parcel #15-4S-16-

03023-552 of the Public Records of Columbia County, Florida.

2. General Description of Improvements:

Construction of a single family dwelling.

3. Owner Information:

Southeast Developers Group Inc

484 NW Turner Avenue, Suite 101

Lake City, Fl 32055 Phone: 386-755-2082

Owner's Interest in Property:

Fee Simple

4. Contractor:

Compass Builders and Associates Corp 484 NW Turner Avenue, Suite 101

Lake City, Fl 32055 Phone: 386-755-2082

5. Lender:

Columbia Bank

173 NW Hillsboro Street Lake City, FL 32055

- 6. Additional persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:
- 7. Expiration date of Notice of Commencement is one (1) year from the date of recording.

Southeast Revelopers G

Joshua A. Nickelson.

Jacob C. Kirsch, Vice President

STATE OF FLORIDA **COUNTY OF Columbia**

> The foregoing instrument was acknowledged before me this 30th day of June, 2008 by Joshua A. Nickelson and Jacob C. Kirsch

> > NOTARY PUBL

Name:

State of Florida at Large

(SEAL)

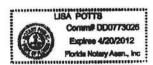
Personally Known:

Produced Identification:

Type:

My Commission Expires:

(NOC)



Columbia County Building Department Culvert Permit

Culvert Permit No.

000001633

DATE <u>07/1</u>	1/2008 PARCEL ID # 15-4	S-16-03023-552			
APPLICANT	JACOB KIRSCH	PHONE	344-4817		
ADDRESS _	484 NW TURNER AVENUE	LAKE CITY		FL_	32055
OWNER SO	UTHEAST DEVELOPERS	PHONE	344-4817		
ADDRESS 29	98 SW MORNING GLORY DRIVE	LAKE CITY		FL	32024
CONTRACTO	R JACOB KIRSCH	PHONE	344-4817		
LOCATION OF	F PROPERTY 90W, TL 247S, TL CALLAHAN,	TR HOPE HENRY, TR	R MORNING		
GLORY, 8TH ON	RIGHT	,		HINOT C	
SUBDIVISION	/LOT/BLOCK/PHASE/UNIT ROLLING MEA	ADOWS	52	<u> </u>	
Х	INSTALLATION REQUIREMENTS Culvert size will be 18 inches in diameter wit driving surface. Both ends will be mitered 4 thick reinforced concrete slab.	foot with a 4 : 1 slop	2 feet, leavir ee and poure	ng 24 ed witl	feet of n a 4 inch
	INSTALLATION NOTE: Turnouts will be reca) a majority of the current and existing drib) the driveway to be served will be paved Turnouts shall be concrete or paved a maconcrete or paved driveway, whichever is current and existing paved or concreted	veway turnouts are or formed with con- inimum of 12 feet v s greater. The width	crete. vide or the w	vidth o	of the the
	Culvert installation shall conform to the app	proved site plan star	ndards.		
	Department of Transportation Permit install	ation approved star	ndards.		
	Other				
			7		

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 080|-54

AUL	CHANGES	MUSI BE	APPROVE	D BY THE	COUNTY	HEALTH	UNIT
COMPA	SS BLDRS/CR 0	7-4240			g.		1
F	Rolling Meadov	ws, Lot 52	V	acant			North
		TBM	is top of st	ake			
Occi No v	NT-AMERICA		153'		 		
Site				/// ₁₅	Waterline	sement	
	ПП 37 ПП ЦП ЦП 10 ПП 10	'	ive				
	OSTDS I 142'	75'		_	Vacant		
				a. 00	Î	inch = 50 f	eet
Site D	Plan Submit	ted By Not App	Jan	Days	Date/	111/08	
By	MA C	Sand			1-15-08 (dumbia	СРНИ	
lotes	:						

Columbia County Building Permit Application

For Office Use Only Application # 0801 - 5# Date Re	popular Indiana di Asia
Application Approved by - Zoning Official BLK Date	Sectived 1-11-08 By 114 Permit # 1633/27160
- XAYL	RSF-2 Land Use Plan Map Category RS Low DE
0	Confirmation Lette Regular
	Fox 386 - 759 - 5047
Name Authorized Person Signing Permit Jacob Kil	Sch Phone 386-344-4817
Address To	ake City, FL 3205-5-
Owners Name Southeast Developers	Phone 386 - 755 - 2082.
911 Address 298 SW MORNING GILONG De	Loke City Fl 37/24
Contractors Name Jacob Kirsch - Compass	Builders Phone 386-344-4817
Address 484 NW turner ave, #101, 1	ake lity Fl 32055
Fee Simple Owner Name & Address Southeast D	ruelopeis Group, Lake (in F)
Bonding Co. Name & Address N/H	The lake (114- PZ
Architect/Engineer Name & Address Nicholas Paul	Geisler 1758 NW Brown H. L.C. Fe
Mortgage Lenders Name & Address Olumbia Bank	173 Nul hillshorp st Lake City
Property ID Number 15-45-16-03023-5-5	Flac Summer V. W.
Subdivision Name Rolling Meadows	stimated Cost of Construction 98,500.
Driving Directions from Usgo/441 - go usgo West	rnase
Sw Callanan ave, I, tollow to	
TALLAULA MOCALIO II	olling Meadows Sch. (TR. Hope Henry
Type of Construction S. F. D.	
Total Acreage Lot Size Do you need a Culve	omber of Existing Dwellings on Property
Actual Distance of Structure from Property Lines - Front	Side 19 Side 190
Total Building Height	Near - Rear
	TID AVI A PART
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuance of a permit and all laws regulating construction in this jurisdiction.	tallations as indicated. I certify that no work or
all laws regulating construction in this jurisdiction.	that all work be performed to meet the standards of
DWNERS AFFIDAVIT: I hereby certify that all the foregoing inform compliance with all applicable laws and regulating construction a	nation is accurate and all work will be done in
VARNING TO OWNER: YOUR FAILURE TO RECORD A MOTION	and zoning.
VARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTELLED ON ATTORNEY BEFORE RECORDING YOUR NOTICE OF	ND TO OBTAIN FINANCING, CONSULT IN YOU PAYING
ENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF	COMMENCEMENT.
	Jul 1. Hind
Owner Builder or Authorized Person by Notarized Letter Michael Michael Michael	Contractor Signature
	Contractors License Number (R(1753775
CONT TOP COLUMBIA	Competency Card NumberNOTARY STAMP/SEAL
DONDING CO., INC.	
day of	Mychelle Foxhor
ersonally known or Produced Identification	Notary Signature (Revised Sept. 2006)
Ju Litt Meson 1, 18.08	(полоси Зерг. 2006)
1 18.08	→
A 1. 1 A	

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

<u>CONTRACTORS AFFIDAVIT:</u> By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's License Number CSC 125 3775

Contractor's Signature (Permitee)

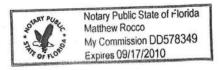
Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 31 day of 320 and 20

Personally known or Produced Identification

SEAL:

State of Florida Notary Signature (For the Contractor)



1-4

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

	Plans Ex	EMENTS: Two (2) complete sets of plans containing the following:
G .	0	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be about the clear.
б	a	Designers name and signature on document (PDC 100 and
A	-	or original scal shall be attived
		Site Plan including:
		a) Dimensions of lot
		b) Dimensions of building set backs c) Location of all other buildings
,		applicable, and all utility easements. d) Provide a full legal description of property.
ν		Wind-load Engineering Summers and all at
		Wind-load Engineering Summary, calculations and any details required Plans or specifications must state compliance with FBC Section 1609. The following information must be also be a section 1609.
		and manufacture in the shown as ther continue 1002 to the state of the
		THE SPACE CONTRACTION OF THE PARTY OF THE PA
		vital mipol lance lactor by and building 1 te
		The same of the sa
		wind exposure and applicable wind direction shall be indicated. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.
		e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design wind pressures in terms of
		manufall in Significant by the maintain it
į.	0	Elevations including:
	Ö	a) All sides
	Ö	b) Roof pitch
	_	c) Overhang dimensions and detail with attic ventilation

	0 0	d) Location, size and height above roof of chimneys.e) Location and size of skylightsf) Building height
W.	0	e) Number of stories Floor Plan including:
5	0	a) Rooms labeled and dimensioned.
H	0	b) Shear walls identified.
4		c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
P.	0	d) Show safety glazing of glass, where required by code.e) Identify egress windows in bedrooms, and size.
6	Ö	f) Fireplace (gas vented), (gas non-vented) or wood burning with
1		hearth, (Please circle applicable type).
ф	0	g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
r q		h) Must show and identify accessibility requirements (accessible bathroom)
[A	0	Foundation Plan including:
P	u	 a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
Çi .	0	b) All posts and/or column footing including size and reinforcing
	0	c) Any special support required by soil analysis such as piling
9		d) Location of any vertical steel.
A	0	Roof System: a) Truss package including:
7		Truss layout and truss details signed and sealed by Fl. Pro. Eng. Roof assembly (FBC 106.1.1.2)Roofing system, materials,
		manufacturer, fastening requirements and product evaluation with
İ	o	wind resistance rating) b) Conventional Framing Layout including:
4	u	Rafter size, species and spacing
		2. Attachment to wall and uplift
		 Ridge beam sized and valley framing and support details
		4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials,
		manufacturer, fastening requirements and product evaluation with wind resistance rating)
		Wall Sections including:
		a) Masonry wall
		All materials making up wall
		 Block size and mortar type with size and spacing of reinforcement Lintel, tie-beam sizes and reinforcement
		4. Gable ends with rake beams showing reinforcement or gable truss
		and wall bracing details
		5. All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss
		plans.
		6. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with resistance rating) 7. Fire resistant construction (if required)
		8. Fireproofing requirements
		Shoe type of termite treatment (termiticide or alternative method)
		10. Slab on grade
(2)		 Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
		b. Must show control joints, synthetic fiber reinforcement or
		Welded fire fabric reinforcement and supports
		 Indicate where pressure treated wood will be placed
		12. Provide insulation R value for the following:

		b) Wood frame wall
		 All materials making up wall
		Size and species of studs
		Sheathing size, type and nailing schedule
		4. Headers sized
		5. Gable end showing balloon framing detail or gable truss and wall
		ninge bracing detail
		 All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		100.1.1.2) Rooting system, materials, manufacturer fastening
		requirements and product evaluation with wind resistance rating)
		o. File resistant construction (if applicable)
		9. Fireproofing requirements
		 Show type of termite treatment (termiticide or alternative method) Slab on grade
		 Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
		b. Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports
		 Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity
6 1	0	c. Crawl space (if applicable)
4	u	c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
300		Engineer or Architect) Floor Framing System:
ø		a) Floor trues package including largest and the state of
	, 	 a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
ф		b) Floor joist size and spacing
		c) Girder size and spacing
Þ		d) Attachment of joist to girder
D		e) Wind load requirements where applicable
à		Plumbing Fixture layout
1		Electrical layout including:
þ		a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
þ		b) Ceiling fans
þ		c) Smoke detectors
		d) Service panel and sub-panel size and location(s)
ф		e) Meter location with type of service entrance (overhead or underground)
4		f) Appliances and HVAC equipment
Ø		g) Arc Fault Circuits (AFCI) in bedrooms
ф		h) Exhaust fans in bathroom
		HVAC information
		a) Energy Calculations (dimensions shall match plans)
		b) Manual J sizing equipment or equivalent computation
		c)Gas System Type (LP or Natural) Location and BTU demand of equipment
		Disclosure Statement for Owner Builders
D		*** Notice Of Commencement Required Before Any Inspections Will Be Done
þ		Private Potable Water
)		

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Jeld-Wen	Exterior Swinging door	FL- 49X-R1
B. SLIDING		1	1 10 11
C. SECTIONAL/ROLL UP	ROYNOF	Rayhor	FL-4867
D. OTHER			12 1161
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI- Products	Single Hung Window	FL-5108
B. HORIZONTAL SLIDER			123108
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	James Hurdie	Hardi Plant Siding	FL-889-R1
B. SOFFITS	Kaycan	Aluminum Soffit	FL- 4957
C. STOREFRONTS			1-1132
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS	+		
A. ASPHALT SHINGLES	Elk Roofing	Asphalt shingles	FL-586-RZ
B. NON-STRUCT METAL	7	Topical Shrington	12-28C-KZ
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS		 	
A. WOOD CONNECTORS	SIMPSON Strong to	Truss Strups	FL- 474-RI
B. WOOD ANCHORS		Triale	FL- 7/7-N
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
Α			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE DATE

Residential System Sizing Calculation

Summary Project Title:

Lake City, FL 32025-

Spec House

Project Title: Compass Builders - Rae Code Only Professional Version Climate: North

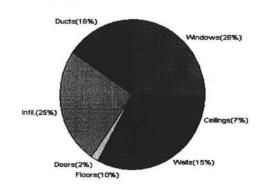
1/10/2008

				1/10/200	0
Location for weather data: Gaine	sville - Def	aults: Latiti	ude(29) Altitude(152 ft.) Temp Ran	ige(M)	
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)	3-()	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	31426	Btuh	Total cooling load calculation	46421	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	117.7	37000	Sensible (SHR = 0.75)	73.2	27750
Heat Pump + Auxiliary(0.0kW)	117.7	37000	Latent	108.4	
			Total (Electric Heat Pump)		37000

WINTER CALCULATIONS

Winter Heating Load (for 1787 sqft)

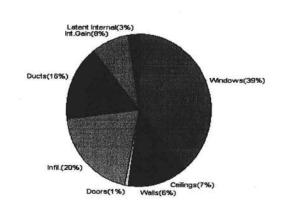
Load component			Load	
Window total	254	sqft	8166	Btuh
Wall total	1394	sqft	4579	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1950	sqft	2298	Btuh
Floor total	198	saft	3238	Btuh
Infiltration	191	cfm	7721	Btuh
Duct loss		***************************************	4907	Btuh
Subtotal			31426	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS		504034545	31426	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1787 sqft)

Load component			Load	
Window total	254	sqft	18233	Btuh
Wall total	1394	sqft	2814	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1950	sqft	3229	Btuh
Floor total			0	Btuh
Infiltration	167	cfm	3104	Btuh
Internal gain			3780	Btuh
Duct gain			6332	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain		1	37884	Btuh
Latent gain(ducts)			1242	Btuh
Latent gain(infiltration)			6095	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain		´	8537	Btuh
TOTAL HEAT GAIN			46421	Btuh



Version 8
For Florida residences only

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House

Project Title:

Lake City, FL 32025-

Compass Builders - Rae

Code Only Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	15.0	32.2	483 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	W	54.0	32.2	1738 Btuh
4	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btuh
5	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btuh
6 7	2, Clear, Metal, 0.87	N	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	E	6.7	32.2	215 Btuh
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
11	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		254(sqft)	Salad vice value	8166 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1230	3.3	4040 Btuh
2	Frame - Wood - Adj(0.09)	13.0	164	3.3	539 Btuh
	Wall Total		1394		4579 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1950	1.2	2298 Btuh
	Ceiling Total		1950		2298Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	198.0 ft(p)	16.4	3238 Btuh
	Floor Total		198		3238 Btuh
			Envelope Sul	ototal:	18798 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqft)	CFM=	
	Natural	0.80	14296 1394	190.6	7721 Btuh
Ductload			(DL	.M of 0.185)	4907 Btuh
All Zones		Sens	sible Subtotal All	Zones	31426 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Rae Code Only Professional Version

Climate: North

1/10/2008

WHOLE HOUSE TOTALS		
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	31426 Btuh 0 Btuh 31426 Btuh

EQUIPMENT

1. Electric Heat Pump # 37000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House

Project Title:

Professional Version

Lake City, FL 32025-

Compass Builders - Rae

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	15.0	32.2	483 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	W	54.0	32.2	1738 Btuh
4	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btuh
5	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btuh
6	2, Clear, Metal, 0.87	N	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	E	6.7	32.2	215 Btuh
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
11	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		254(sqft)		8166 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1230	3.3	4040 Btuh
2	Frame - Wood - Adj(0.09)	13.0	164	3.3	539 Btuh
	Wall Total		1394		4579 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40	1000000	518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1950	1.2	2298 Btuh
	Ceiling Total		1950	******	2298Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	198.0 ft(p)	16.4	3238 Btuh
	Floor Total		198		3238 Btuh
		Z	one Envelope Su	btotal:	18798 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqff	t) CFM=	
	Natural	0.80	14296 1394	190.6	7721 Btuh
Ductload	Average sealed, Supply(R6.	0-Attic), Retur	n(R6.0-Attic) (DI	LM of 0.185)	4907 Btuh
Zone #1	-	Sens	sible Zone Subto	otal	31426 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Rae Code Only Professional Version Climate: North

1/10/2008

Subtotal Sensible	31426 Btuh
Ventilation Sensible Total Btuh Loss	0 Btuh 31426 Btuh

85	-	proq.	oppose.	de sed	2799	(COLUMN)	1500	UHC:
351			2 61	0	88		2.1	
			U	:	1992		8.00	16 B
	MORN	12.5	COSC 1	830	edantari		MA.	20.00

1. Electric Heat Pump # 37000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Rae

Code Only Professional Version

Climate: North

Lake City, FL 32025-

9. 3 11969

1/10/2008

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

Component Loads for Whole House

	Type*		Over	hang	Wine	dow Area	(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		16 511(50)	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	15.0	0.0	15.0	29	80	1193	Btuh
2	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	40.0	17.3	22.7	29	80	2304	
3	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	54.0	20.0	34.0	29	80	3282	
4	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	16.0	0.0	16.0	29	80		Btuh
5	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	6.0	0.0	6.0	29	80	477	
6	2, Clear, 0.87, None, N, N	N	1.5ft	8ft.	20.0	0.0	20.0	29	29	579	
7	2, Clear, 0.87, None, N, N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	
8	2, Clear, 0.87, None, N, N	E	8.5ft	10ft.	6.7	3.7	2.9	29	80	342	
9	2, Clear, 0.87, None, N, N	E	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385	
10	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	30.0	30.0	0.0	29	34	869	
11	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	6.0	6.0	0.0	29	34	370 (70)	Btuh
	Excursion									100.00	Btuh
	Window Total				254 (saft)				18233	
Walls	Туре		R-Va	alue/U	-Value	Area(saft)		нтм	Load	Dian
1	Frame - Wood - Ext			13.0/	100 mm	123			2.1		D
2	Frame - Wood - Adj			13.0/	7,475,77	164	200		1.5		Btuh
177	Wall Total			13.0/	0.09	9.50			1.5	77.000	Btuh
Doors	Type						4 (sqft)			2814	Btun
						Area (sqft)		HTM	Load	
1	Insulated - Exterior					20.	0		9.8	196	Btuh
2	Insulated - Adjacent					20.	0		9.8	196	Btuh
	Door Total					40	(sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area(sqft)		HTM	Load	
1	Vented Attic/DarkShingle			30.0		1950	T. 54		1.7		Btuh
	Ceiling Total						(sqft)			3229	
Floors	Type		R-Va	lue		Siz			нтм	Load	Dian
1	Slab On Grade		11 00								
	Floor Total			5.0			3 (ft(p))		0.0		Btuh
	Floor Total					198.0	(sqft)			0	Btuh
						En	velope §	Subtotal	:	24668	Btuh
nfiltration	Туре		^	СН	Volum	o(ou#\	all cros	(naft)	CFM=	Land	
	SensibleNatural			0.70	Volum	e(cuft) w		sqit)	**************************************	Load	D. .
Internal			20011-			14296	1394		190.6	3104	Btuh
			Occup			Btuh/occ		P	ppliance	Load	
gain				6	,	(230	+		2400	3780	Btuh
					-	Se	nsible E	nvelope	Load:	31552	Btuh
ouct load							(DGN	f of 0.20	01)	6332	Btuh
						Sens	sible Lo	ad All 2	Zones	37884 E	3tuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Rae Code Only **Professional Version** Climate: North

1/10/2008

WHOLE HOUSE TOTALS

		r	
7	Sensible Envelope Load All Zones	31552	Btuh
	Sensible Duct Load	6332	Btuh
	Total Sensible Zone Loads	37884	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	37884	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6095	Btuh
	Latent ventilation gain	0	Btuh
ii.	Latent duct gain	1242	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	8537	Btuh
	TOTAL GAIN	46421	Btuh

EQUIPMENT	

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House

Project Title: Compass Builders - Rae

Code Only
Professional Version
Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

Component Loads for Zone #1: Main

	Type*		Over	hang	Win	dow Area	a(saft)	H	ITM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded		Unshaded		
1	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	15.0	0.0	15.0	29	80	1193	Btuh
2	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	40.0	17.3	22.7	29	80	2304	
3	2, Clear, 0.87, None, N, N	W	7.5ft	10ft.	54.0	20.0	34.0	29	80	3282	
4	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	16.0	0.0	16.0	29	80	1272	
5	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	6.0	0.0	6.0	29	80	477	
6	2, Clear, 0.87, None, N, N	N	1.5ft	8ft.	20.0	0.0	20.0	29	29	579	
7	2, Clear, 0.87, None, N, N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	100000
8	2, Clear, 0.87, None, N, N	E	8.5ft	10ft.	6.7	3.7	2.9	29	80	342	
9	2, Clear, 0.87, None, N, N	Ē	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385	
10	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	30.0	30.0	0.0	29	34	869	
11	2, Clear, 0.87, None, N, N	S	1.5ft	8ft.	6.0	6.0	0.0	29	34	, , , , , , , , , , , , , , , , , , , ,	Btul
1700	Window Total		1.51	OIL.			0.0	29	34	5.507	
Walls			5		254 (THE RESERVE THE PARTY OF THE PA				15262	Btur
20 A 10 A 20 A 20 A 20 A 20 A 20 A 20 A	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	123	0.3		2.1	2566	Btuh
2	Frame - Wood - Adj			13.0/	0.09	16	4.0		1.5	247	Btul
	Wall Total					139	4 (sqft)			2814	Btuł
Doors	Туре					Area			нтм	Load	Dia
1	Insulated - Exterior								N.C. DOLLA GREAT		
2	Insulated - Adjacent					-57747	.0		9.8	196	
2							.0		9.8	0.40-0.7	Btuh
A 111	Door Total						0 (sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area	(sqft)		HTM	Load	
1	Vented Attic/DarkShingle			30.0		195	0.0		1.7	3229	Blub
	Ceiling Total					0.70	0 (sqft)			3229	
Floors	Type		R-Va	مبراه		Si			нтм	Load	Diui
1	Slab On Grade		11 00			297					80 8
				5.0			8 (ft(p))		0.0		Btuh
	Floor Total					198.	0 (sqft)			0	Btuh
						Z	ne Enve	lope Su	ibtotal:	21697	Btuh
nfiltration	Туре		Α	СН	Volum	e(cuft) v	vall area	(saft)	CFM=	Load	-
	SensibleNatural			0.70		14296	1394	. 1.7	166.8	3104	Btuh
Internal		(Occup			Btuh/oc		Δ	ppliance	Load	
gain			Jooup	6		X 23					D.
9				0		A 23	0 +		2400	3780	Btul
						Se	ensible E	nvelope	Load:	28581	Btuh
Ouct load	Average sealed, Supply	(R6.0-A	Attic),	Retur	n(R6.0-	Attic)		(DGM o	f 0.201)	5736	Btul
							Sensib	le Zone	Load	34317 E	3tuh

			ystem loads.

Windows	July excursion for System 1	2971 Btuh
	Excursion Sub	total: 2971 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Compass Builders - Rae

Code Only Professional Version

Climate: North

1/10/2008

Sensible Excursion L	oad 3567 Btuh
Duct load	596 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Rae

Code Only Professional Version Climate: North

1/10/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	31552	Btuh
	Sensible Duct Load	6332	Btuh
	Total Sensible Zone Loads	37884	Btuh
,	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	37884	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6095	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1242	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	8537	Btuh
	TOTAL GAIN	46421	Btuh

EQUIPMENT		
1. Central Unit	#	37000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

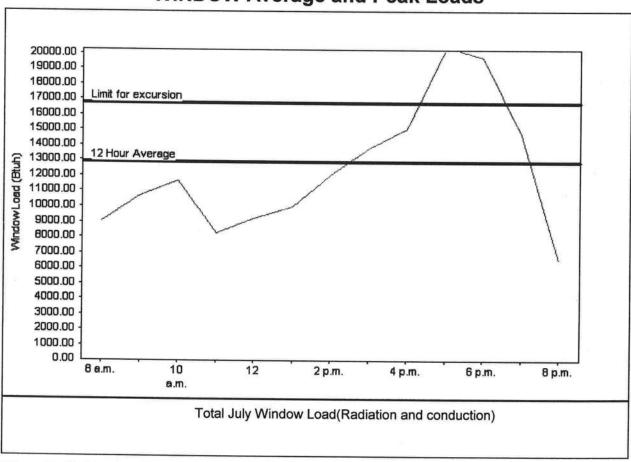
Spec House Lake City, FL 32025Project Title: Compass Builders - Rae

Code Only Professional Version Climate: North

1/10/2008

Weather data for: Gainesville - Defaults									
Summer design temperature	92	F	Average window load for July	12894 Btt					
Summer setpoint	75	F	Peak window load for July	20440 Bt					
Summer temperature difference	17	F	Excusion limit(130% of Ave.)	16763 Btu					
Latitude	29	North	Window excursion (July)	3677 Btut					

WINDOW Average and Peak Loads



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only PREPARED BY:

DATE:

EnergyGauge® FLRCPB v4.5.2





Project Information for:

L265559

Builder:

Compass Builders

Lot:

Subdivision:

Rolling Meadows

County: Truss Count: Baker 45

Design Program: MiTek 20/20 6.3 **Building Code:**

FBC2004/TPI2002

Truss Design Load Information: Gravity:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 120

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for the Structural Engineering:

Jacob C. Kirsch Florida License No. CBC1253775

Address: 196 Southwest Huntsview Way Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

28

J1925021

T16G

1/10/08

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T17

T18

T19

T20

T21

T22

T23

T24

T25

T26

T27

T28

T29

T30

T31

T32

T17G

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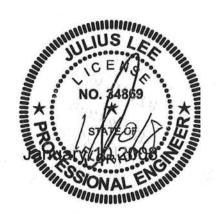
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No.	Drwg. #	Truss ID	Date	No.	Drwg. #
1	J1924994	CJ01	1/10/08	29	J1925022
2	J1924995	CJ02	1/10/08	30	J1925023
3	J1924996	CJ03	1/10/08	31	J1925024
4	J1924997	CJ04	1/10/08	32	J1925025
5	J1924998	CJ05	1/10/08	33	J1925026
6	J1924999	CJ06	1/10/08	34	J1925027
7	J1925000	CJ07	1/10/08	35	J1925028
8	J1925001	EJ07	1/10/08	36	J1925029
9	J1925002	EJ07A	1/10/08	37	J1925030
10	J1925003	HJ07	1/10/08	38	J1925031
11	J1925004	HJ07A	1/10/08	39	J1925032
12	J1925005	T01	1/10/08	40	J1925033
13	J1925006	T01G	1/10/08	41	J1925034
14	J1925007	T02	1/10/08	42	J1925035
15	J1925008	T03	1/10/08	43	J1925036
16	J1925009	T04	1/10/08	44	J1925037
17	J1925010	T05	1/10/08	45	J1925038
18	J1925011	T06	1/10/08		
19	J1925012	T07	1/10/08		
20	J1925013	T08	1/10/08		
21	J1925014	T09	1/10/08		
22	J1925015	T10	1/10/08		
23	J1925016	T11	1/10/08		
24	J1925017	T12	1/10/08		
25	J1925018	T13	1/10/08		
26	J1925019	T14	1/10/08		
27	J1925020	T15	1/10/08		
		A CONTRACTOR OF THE PARTY OF TH			



Job	Truss	Truss Type	Qty	Ply	00	
,	CJ01	JACK	4	1		J1924994
a ʻ L	1555	1-3-3-3	100		Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:41 2008 Page 1

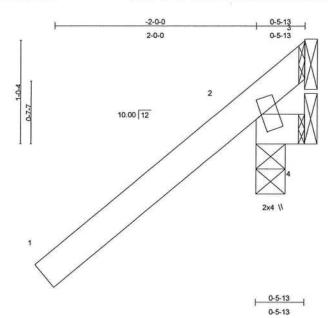


Plate Of	fsets (X, Y	'): [2:0-2-0,0-0-5]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.00	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Matrix)		,					Weight: 6 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

0-5-13 oc purlins.

BOT CHORD Rigin

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=404/0-3-8, 4=2/Mechanical, 3=-263/Mechanical

Max Horz 2=114(load case 6)

Max Uplift 2=-503(load case 6), 3=-263(load case 1)

Max Grav 2=404(load case 1), 4=7(load case 2), 3=382(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-240/479

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.75

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 503 Colkinging and 263 lb uplift at joint 3.

Trues Design Engineer Florida PE No. 34869 1109 Ceastal Bay Blvri Boynton Beach, FL 9343:

January 10,2008

Scale = 1:10.8

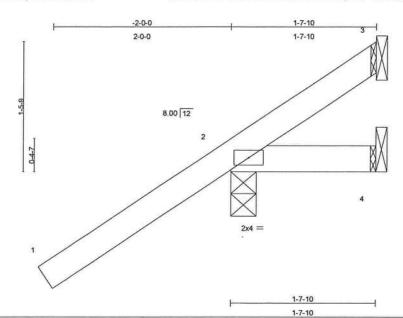
Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and baded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	CJ02	JACK	4	1		J1924995
	0002	0.757			Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mati	rix)	' '					Weight: 9 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-7-10 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=240/0-3-8, 4=8/Mechanical, 3=-32/Mechanical

Max Horz 2=124(load case 6)

Max Uplift 2=-219(load case 6), 3=-32(load case 1)

Max Grav 2=240(load case 1), 4=24(load case 2), 3=67(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-69/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.32

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 Ib uplift at joint 2 and 32 lb uplift at joint 3.

January 10,2008

Scale = 1:12.2

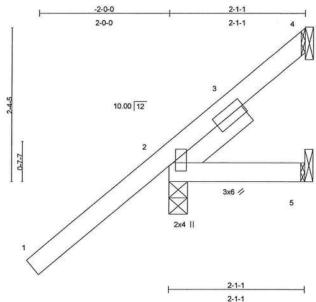
LOAD CASE(S) Standard

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
*	CJ03	JACK	4	1	J1924996
*					Job Reference (optional)

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-	-2-0-0	2-1-1	
	2-0-0	2-1-1	1
2.4-5	10.00 12	3/	
0-7-7	2/		
		3x6 %	5
1/		(Tare) (18)	
,**.		2-1-1	
			376

Plate Of	fsets (X,)	(): [2:0-1-8,0-0-8]		,								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-5	>999	240	CONT. IN GENERAL	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	, , ,					Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

Left 2 X 4 SYP No.2 1-4-6 SLIDER

BRACING

TOP CHORD

Structural wood sheathing directly applied or

2-1-1 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=-21/Mechanical, 2=253/0-3-8, 5=9/Mechanical

Max Horz 2=163(load case 6)

Max Uplift 4=-21(load case 1), 2=-202(load case 6)

Max Grav 4=46(load case 4), 2=253(load case 1), 5=28(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-97/41, 3-4=-48/28

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.72, 2 = 0.13 and 3 = 0.00

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb Committed in the 202 lb uplift at joint 2.

January 10,2008

Scale = 1:16.7

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job		Truss	Truss Type	Qty	Ply	0 0	
	*	CJ03	JACK	4	1	J192499	96
						Job Reference (optional)	

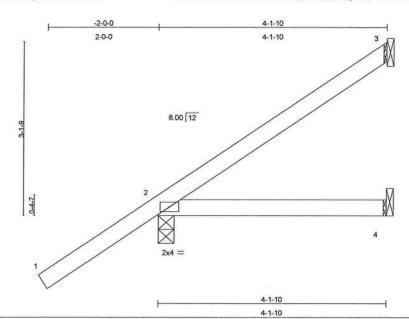
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:42 2008 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	П
 *	CJ04	JACK	4	1	J1924997	7
			- 007	*	Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)	, ,					Weight: 17 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-1-10 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=72/Mechanical, 2=280/0-3-8, 4=20/Mechanical

Max Horz 2=186(load case 6)

Max Uplift 3=-51(load case 5), 2=-171(load case 6)

Max Grav 3=72(load case 1), 2=280(load case 1), 4=59(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-81/29

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.34

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 3 and 171 lb uplift at joint 2.

January 10,2008

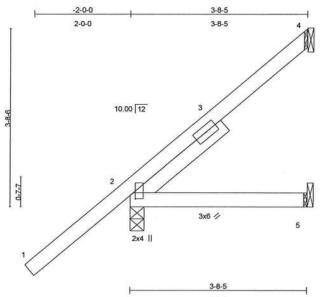
Scale = 1:19.6

LOAD CASE(S) Standard



ĺ	Job	Truss	Truss Type	Qty	Ply	00
	*	CJ05	JACK	4	1	J1924998
*		0000	on on			Job Reference (optional)

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1	-2-0-0	3-8-5	
Ī.	2-0-0	3-8-5	→ M
		7	//0
		//	
	10	0.00 12 3	
		///	
*			
0-7-7	2		
		3x6 //	5
1 /		2x4	
		3-8-5	
		3-8-5	

fsets (X, Y	′): [2:0-1-8,0-0-8]										
G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.01	2-5	>999	360	MT20	244/190
7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-5	>999	240	120% 197004 884	
10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 20 lb	
	G (psf) 20.0 7.0 10.0	20.0 Plates Increase 10.0 Lumber Increase 10.0 Rep Stress Incr	G (psf) SPACING 2-0-0 20.0 Plates Increase 1.25 7.0 Lumber Increase 1.25 10.0 * Rep Stress Incr YES	G (psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 7.0 Lumber Increase 1.25 BC 10.0 * Rep Stress Incr YES WB	G (psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 0.42 7.0 Lumber Increase 1.25 BC 0.09 10.0 * Rep Stress Incr YES WB 0.00	G (psf) SPACING 2-0-0 CSI DEFL 20.0 Plates Increase 1.25 TC 0.42 Vert(LL) 7.0 Lumber Increase 1.25 BC 0.09 Vert(TL) 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL)	G (psf) SPACING 2-0-0 CSI DEFL in 20.0 Plates Increase 1.25 TC 0.42 Vert(LL) -0.01 7.0 Lumber Increase 1.25 BC 0.09 Vert(TL) -0.01 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00	G (psf) SPACING 2-0-0 CSI DEFL in (loc) 20.0 Plates Increase 1.25 TC 0.42 Vert(LL) -0.01 2-5 7.0 Lumber Increase 1.25 BC 0.09 Vert(TL) -0.01 2-5 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 4	G (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl 20.0 Plates Increase 1.25 TC 0.42 Vert(LL) -0.01 2-5 >999 7.0 Lumber Increase 1.25 BC 0.09 Vert(TL) -0.01 2-5 >999 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 4 n/a	G (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d 20.0 Plates Increase 1.25 TC 0.42 Vert(LL) -0.01 2-5 >999 360 7.0 Lumber Increase 1.25 BC 0.09 Vert(TL) -0.01 2-5 >999 240 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 4 n/a n/a	G (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d PLATES 20.0 Plates Increase 1.25 TC 0.42 Vert(LL) -0.01 2-5 >999 360 MT20 20.0 Lumber Increase 1.25 BC 0.09 Vert(TL) -0.01 2-5 >999 240 No.00 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 4 n/a n/a

	٠
LUMBER	٠.

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

Left 2 X 4 SYP No.2 2-4-6 SLIDER

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-8-5 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=55/Mechanical, 2=271/0-3-8, 5=17/Mechanical

Max Horz 2=214(load case 6)

Max Uplift 4=-61(load case 5), 2=-153(load case 6)

Max Grav 4=55(load case 1), 2=271(load case 1), 5=52(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-98/8, 3-4=-71/23

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.76, 2 = 0.13 and 3 = 0.00

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb Committed in the and 153 lb uplift at joint 2.

January 10,2008

Scale = 1:22.6

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



	Job	Truss	Truss Type	Qty	Ply	00
	,		1020			J1924998
		CJ05	JACK	4	1	Weeks the second of the second
- [Job Reference (optional)

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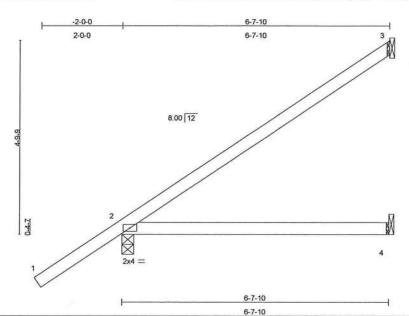
LOAD CASE(S) Standard

Les Design Engineer & PE No. 34868 Coestel Bay Blvd on Beach, Ft. 33435



Job	Truss	Truss Type	Qty	Ply	00	
,	CJ06	JACK	4	1		J1924999
		1		-	Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.09	2-4	>861	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.16	2-4	>492	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	, ,					Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=153/Mechanical, 2=347/0-3-8, 4=32/Mechanical

Max Horz 2=250(load case 6)

Max Uplift 3=-126(load case 6), 2=-162(load case 6)

Max Grav 3=153(load case 1), 2=347(load case 1), 4=96(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-151/67

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.39

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 3 and 162 lb uplift at joint 2.

Julius Les Trues Design Engineer Florida PE No. 34869 1196 Ceasial Bay Blvd Boynton Beach, FL 32421

January 10,2008

Scale = 1:26.9

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
,	CJ07	JACK	2			J1925000
•	C307	JACK	2	1 1	Job Reference (optional)	

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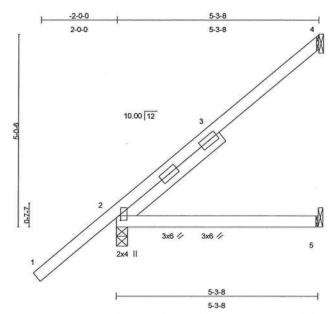


Plate Of	fsets (X,Y	'): [2:0-1-8,0-0-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.04	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.06	2-5	>995	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 27 lb	

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	I IN	пъ	<.	- 14	

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

SLIDER Left 2 X 4 SYP No.2 3-4-15

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-3-8 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=111/Mechanical, 2=310/0-3-8, 5=25/Mechanical

Max Horz 2=265(load case 6)

Max Uplift 4=-119(load case 6), 2=-134(load case 6)

Max Grav 4=111(load case 1), 2=310(load case 1), 5=76(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/66, 2-3=-155/7, 3-4=-141/52

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.85, 2 = 0.07, 2 = 0.07 and 3 = 0.00

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 Colhi Holes and 134 lb uplift at joint 2.

Truss Design Engineer Florida PE No. 24869 1109 Caastal Bay Blvd Boynton Besch, FL 3343

January 10,2008

Scale = 1:28.4

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	CJ07	JACK	2	1		J1925000
*				5.57	Job Reference (optional)	

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LOAD CASE(S) Standard

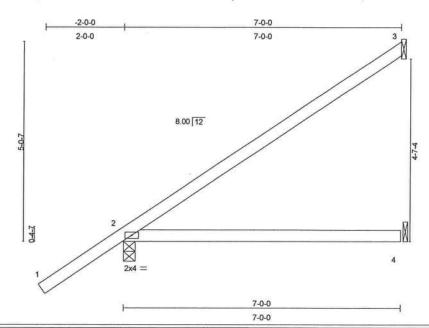
Julius Lee Trues Cesign Engineer Flonda PE No. 24868 1109 Coestel Bay Blvd



Job Truss Truss Type Qty Ply 00 J1925001 **EJ07** MONO TRUSS 7 Job Reference (optional)

Builders First Source, Jacksonville, Florida 32244

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LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.48 Vert(LL) >730 360 MT20 -0.112-4 244/190 TCDL BC 7.0 Lumber Increase 1.25 0.33 -0.20Vert(TL) 2-4 >417 240 10.0 0.00 BCLL Rep Stress Incr YES WB Horz(TL) -0.003 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 27 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=164/Mechanical, 2=357/0-3-8, 4=34/Mechanical

Max Horz 2=259(load case 6)

Max Uplift 3=-136(load case 6), 2=-161(load case 6)

Max Grav 3=164(load case 1), 2=357(load case 1), 4=102(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-160/72

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.41

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 3 and 161 lb uplift at joint

LOAD CASE(S) Standard

January 10,2008

Scale = 1:27.4

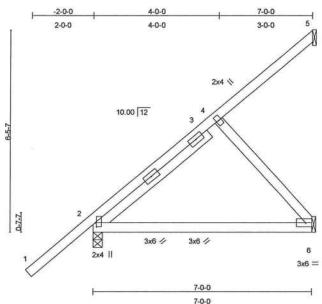
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
,	EJ07A	MONO TRUSS	g.	1	J1925002
	LJOTA	WONO TROSS	0	,	Job Reference (optional)

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1	2-0-0	4-0-0	3-0-0
			//
			2x4 🔻
			//
		10.00 12 3	*
6-5-7		18	*/
		///	
		///	
		////	
		///	
D-7-Z	2/2	{/	
Ιď		3x6 // 3x6 //	
	// 2x] 3x6 ⁄/ 3x6 ∕/ 4 II	6
<			3x6 =
		7-0-0	1
		7-0-0	

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.12	2-6	>710	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.20	2-6	>406	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	10 10					Weight: 42 lb	

LUMBER		BRACING
TOP CHORD	2 X 4 SYP No.2	TOP CHO
BOT CHORD	2 X 4 SYP No.2	
WEBS	2 X 4 SYP No.3	BOT CHO
SLIDER	Left 2 X 4 SYP No.2 4-7-11	

ORD Structural wood sheathing directly applied or 6-0-0 oc purlins. ORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=67/Mechanical, 2=359/0-3-8, 6=130/Mechanical

Max Horz 2=320(load case 6)

Max Uplift 5=-70(load case 6), 2=-123(load case 6), 6=-84(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/66, 2-3=-201/0, 3-4=-83/0, 4-5=-89/33

BOT CHORD 2-6=-152/84 **WEBS** 4-6=-127/230

Plate Offsets (X,Y): [2:0-1-8.0-0-8]

JOINT STRESS INDEX

2 = 0.73, 2 = 0.06, 2 = 0.06, 3 = 0.00, 4 = 0.13 and 6 = 0.09

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Scale = 1:34.7

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation availed from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
*	EJ07A	MONO TRUSS	8	1		J1925002
				531	Job Reference (optional)	

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 5, 123 lb uplift at joint 2 and 84 lb uplift at joint 6.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
	HJ07	MONO TRUSS	2	1		J1925003
,	100000000		0.723		Job Reference (optional)	

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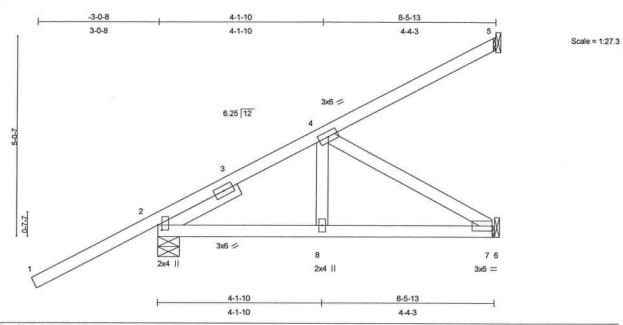


Plate Of	fsets (X,Y)): [2:0-	1-8,0-1-4]										
LOADIN	G (psf)	SP	ACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Pla	tes Increase	1.25	TC	0.56	Vert(LL)	-0.01	7-8	>999	360	MT20	244/190
TCDL	7.0	Lun	nber Increase	1.25	BC	0.16	Vert(TL)	-0.03	7-8	>999	240		
BCLL	10.0	* Rep	Stress Incr	NO	WB	0.12	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0	Cod	de FBC2004/TI	PI2002	(Mat	rix)						Weight: 46 lb	ì

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 WEBS

SLIDER Left 2 X 4 SYP No.2 2-3-0

REACTIONS (lb/size) 5=188/Mechanical, 2=389/0-6-9, 7=164/Mechanical

Max Horz 2=259(load case 5)

Max Uplift 5=-161(load case 5), 2=-182(load case 5), 7=-33(load case 5) Max Grav 5=188(load case 1), 2=389(load case 1), 7=174(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-304/0, 3-4=-284/0, 4-5=-101/65

BOT CHORD 2-8=-93/220, 7-8=-93/220, 6-7=0/0

WEBS 4-7=-254/107, 4-8=0/157

JOINT STRESS INDEX

2 = 0.89, 2 = 0.14, 3 = 0.00, 4 = 0.12, 7 = 0.07 and 8 = 0.12

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 5, 182 lb uplift at joint 2 and 33 lb uplift at joint 7.

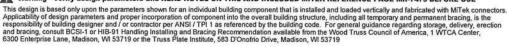
E) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Structural wood sheathing directly applied or 6-0-0

Rigid ceiling directly applied or 10-0-0 oc bracing.

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00	
,	HJ07	MONO TRUSS	2	1		J1925003
ř.					Job Reference (optional)	

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-0(F=27, B=27)-to-5=-114(F=-30, B=-30), 2=0(F=5, B=5)-to-6=-21(F=-6, B=-6)



Job	Truss	Truss Type	Qty	Ply	00	
	НЈ07А	MONO TRUSS	2	1		J1925004
			(CA)		Job Reference (optional)	

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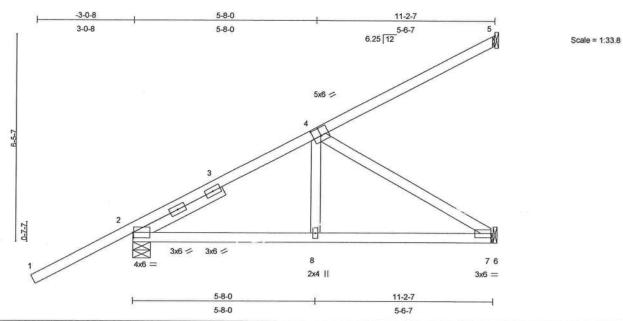


Plate Of	fsets (X,Y):	[2:0-0-4,0-2-0], [4:0	-3-0,0-3-0]			apr.						
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.03	2-8	>999	360	MT20	244/190
TCDL	7.0	1	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.08	7-8	>999	240	3/7.5192779:T9X	
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.39	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 59 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.2 3-1-5

BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=302/Mechanical, 2=521/0-6-9, 7=339/Mechanical

Max Horz 2=411(load case 5)

Max Uplift 5=-267(load case 5), 2=-177(load case 5), 7=-128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-562/0, 3-4=-477/0, 4-5=-168/102

BOT CHORD 2-8=-264/459, 7-8=-264/459, 6-7=0/0

WEBS 4-8=0/214, 4-7=-535/307

JOINT STRESS INDEX

2 = 0.77, 2 = 0.11, 2 = 0.11, 3 = 0.00, 4 = 0.44, 7 = 0.15 and 8 = 0.16

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 5, 177 lb uplift at joint 2 and 128 lb uplift at joint 7.

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

January 10,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	НЈ07А	MONO TRUSS	2	1		J1925004
	200000000		JESA:		Job Reference (optional)	

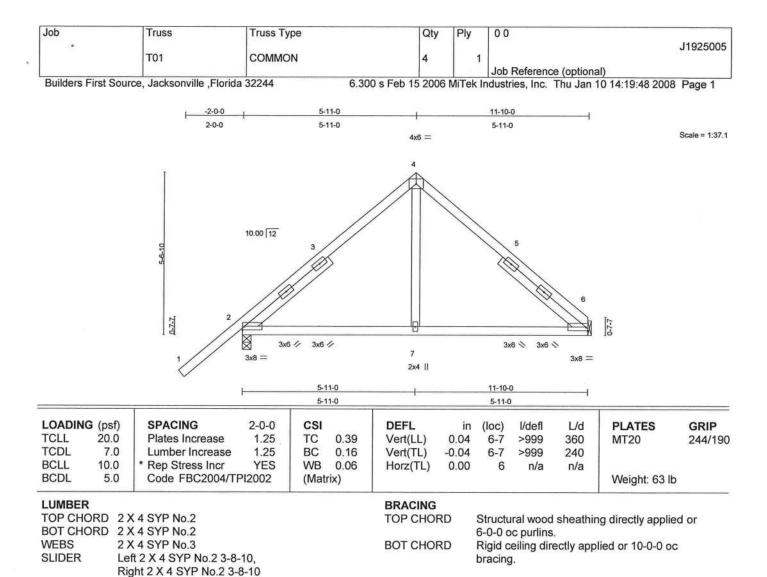
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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-0(F=27, B=27)-to-5=-150(F=-48, B=-48), 2=0(F=5, B=5)-to-6=-28(F=-9, B=-9)





REACTIONS (lb/size) 6=360/Toenail or 0-3-8 bearing, 2=505/0-3-8

Max Horz 2=205(load case 5)

Max Uplift 6=-99(load case 7), 2=-234(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-418/208, 3-4=-229/236, 4-5=-300/213, 5-6=-405/195

BOT CHORD 2-7=-50/230, 6-7=-50/230

WEBS 4-7=0/187

JOINT STRESS INDEX

2 = 0.68, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.66, 5 = 0.00, 6 = 0.68, 6 = 0.09, 6 = 0.09 and 7 = 0.13

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00	
•	T01	COMMON	4	1		J1925005
	23.23	3 - 1000-10			Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 6 and 234 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
	T01G	GABLE	1	1		J1925006
					Job Reference (optional)	

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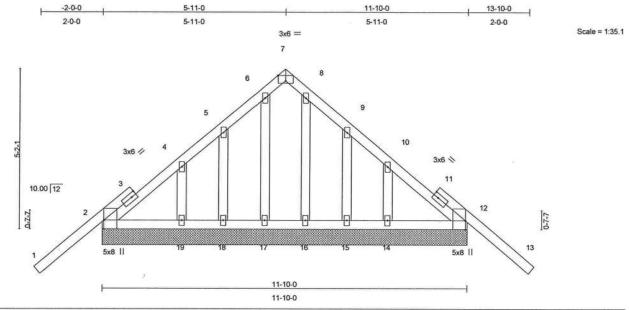


Plate Of	fsets (X,Y	'): [2:0-3-8,Edge], [7:	0-3-0,Edg	e], [12:0)-3-8,Edg	ge]						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.03	13	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.04	13	n/r	90	0.0000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)					5,41,390	Weight: 79 lb	

LUMBER	L	U	M	В	E	R
--------	---	---	---	---	---	---

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size)

2=249/11-10-0, 12=249/11-10-0, 17=72/11-10-0, 18=93/11-10-0,

19=77/11-10-0, 16=72/11-10-0, 15=93/11-10-0, 14=77/11-10-0

Max Horz 2=-157(load case 4)

Max Uplift 2=-155(load case 6), 12=-177(load case 7), 17=-19(load case 5),

18=-109(load case 6), 19=-51(load case 5), 15=-112(load case 7),

Max Grav 2=249(load case 1), 12=249(load case 1), 17=72(load case 4), 18=94(load case 10), 19=90(load case 2), 16=72(load case 1), 15=94(load case 11),

14=90(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-120/89, 3-4=-109/102, 4-5=-62/75, 5-6=-40/136, 6-7=-33/118,

7-8=-33/118, 8-9=-40/136, 9-10=-38/34, 10-11=-68/60, 11-12=-79/47, 12-13=0/65

BOT CHORD

2-19=0/231, 18-19=0/231, 17-18=0/231, 16-17=0/231, 15-16=0/231, 14-15=0/231,

6-17=-62/28, 5-18=-77/149, 4-19=-70/68, 8-16=-60/0, 9-15=-77/149, 10-14=-70/69

JOINT STRESS INDEX

WEBS

2 = 0.62, 3 = 0.00, 3 = 0.23, 4 = 0.04, 5 = 0.07, 6 = 0.03, 7 = 0.19, 8 = 0.03, 9 = 0.07, 10 = 0.04, 11 = 0.00, 11 = 0.23, 12 = 0.040.62, 14 = 0.04, 15 = 0.08, 16 = 0.02, 17 = 0.02, 18 = 0.08 and 19 = 0.04

Continued on page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
3.	T01G	GABLE	1	1		J1925006
	1.55.5				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:49 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2, 177 lb uplift at joint 12, 19 lb uplift at joint 17, 109 lb uplift at joint 18, 51 lb uplift at joint 19, 112 lb uplift at joint 15 and 53 lb uplift at joint 14.

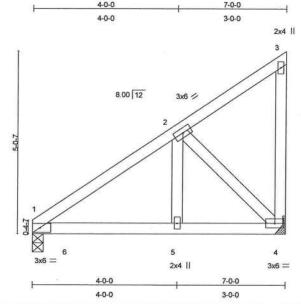
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34899 1109 Coestel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
¥	T02	MONO TRUSS	1	1		J1925007
		C-44-150-210-200-50-4-4-4-4-4-4-4-4-4-4-4-4-4-4-4-4-4-			Job Reference (optional)	

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Simpson HTU26

Plate Of	fsets (X, Y	'): [1:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.04	1-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.07	1-5	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 39 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Ric

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=674/0-3-8, 4=809/Mechanical

Max Horz 1=176(load case 5)

Max Uplift 1=-190(load case 5), 4=-361(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-772/179, 2-3=-56/21

BOT CHORD 1-6=-272/600, 5-6=-272/600, 4-5=-272/600 WEBS 3-4=-53/43, 2-5=-245/771, 2-4=-829/375

JOINT STRESS INDEX

1 = 0.37, 2 = 0.57, 3 = 0.02, 4 = 0.25 and 5 = 0.55

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 1 and 361 lb uplift at joint 4.

Continued on page 2

Julius Leer Truse Design Engineer Flonda PE No. 3-1869 1109 Caastal Bay Blvd Boynton Besch, FL 33435

January 10,2008

Scale = 1:30.0

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T02	MONO TRUSS	1	1		J1925007
	1000000	1 000 000 000 000 000 000 000 000 000 0			Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

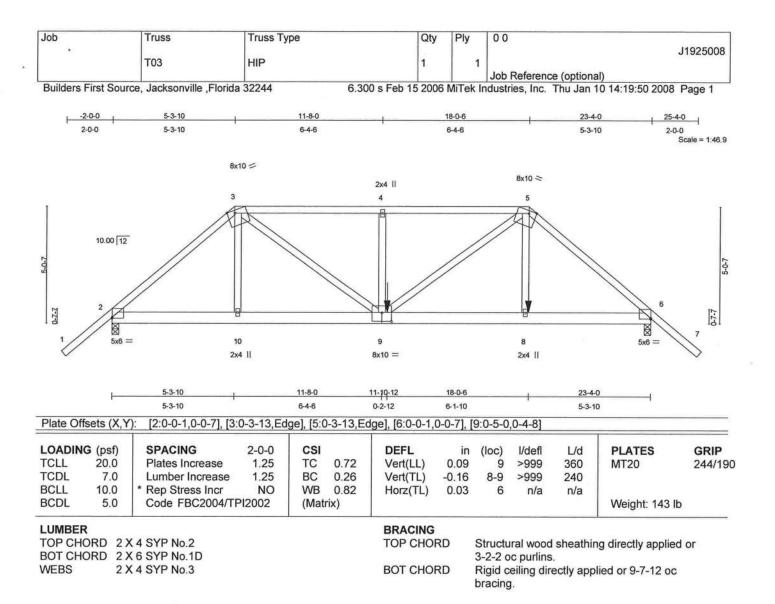
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 1-6=-10, 4-6=-190(B=-180)

Julius Lee Truse Design Engineer Florida PE No. 34869 1 100 Coastal Bay Blyd





REACTIONS (lb/size) 2=1507/0-3-8, 6=1807/0-3-8

Max Horz 2=-151(load case 3)

Max Uplift 2=-600(load case 4), 6=-757(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1921/810, 3-4=-2547/1198, 4-5=-2547/1197, 5-6=-2383/1045,

6-7=0/69

BOT CHORD 2-10=-631/1359, 9-10=-630/1361, 8-9=-711/1739, 6-8=-709/1726

WEBS 3-10=0/190, 3-9=-806/1513, 4-9=-572/407, 5-9=-563/1045, 5-8=-101/398

JOINT STRESS INDEX

2 = 0.72, 3 = 0.87, 4 = 0.33, 5 = 0.87, 6 = 0.72, 8 = 0.33, 9 = 0.35 and 10 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

EbAll pearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Law Truse Cesian Endineer Florida PE No. 24869 1109 Cessial Bay Blvd Boynton Beach, FL 23435

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	00
*	T03	HIP	1	1	J1925008
	100				Job Reference (optional)

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 600 lb uplift at joint 2 and 757 lb uplift at joint 6.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

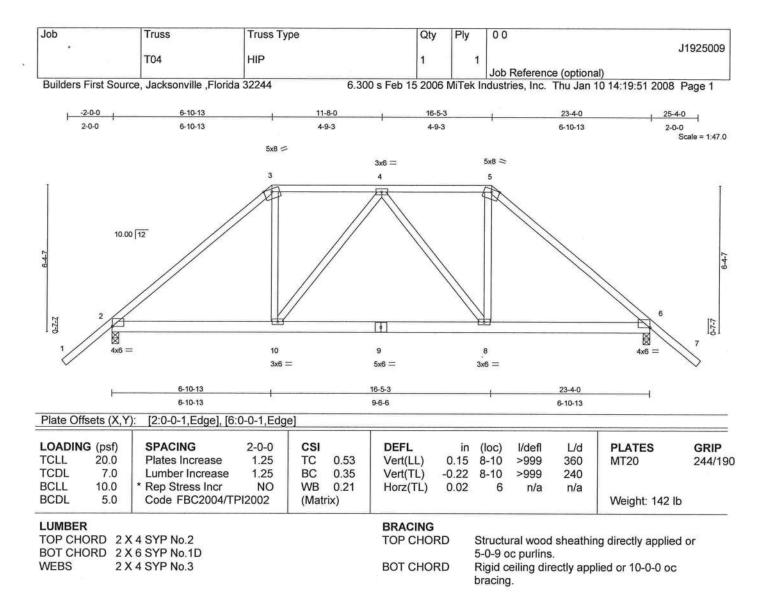
Vert: 1-3=-54, 3-4=-54, 4-5=-118(F=-64), 5-7=-54, 2-9=-10, 8-9=-22(F=-12), 6-8=-10

Concentrated Loads (lb)

Vert: 9=-809(F) 8=-311(F)

Julius Lee Truse Cesign Engineer Florida PE No. 34865 1199 Cestal Bay Blvd Boyoton Besch, Ft. 23436





REACTIONS (lb/size) 2=1135/0-3-8, 6=1135/0-3-8

Max Horz 2=194(load case 5)

Max Uplift 2=-408(load case 6), 6=-408(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1413/821, 3-4=-977/753, 4-5=-977/753, 5-6=-1413/821, 6-7=0/69

BOT CHORD 2-10=-412/963, 9-10=-419/1055, 8-9=-419/1055, 6-8=-315/963

WEBS 3-10=-238/573, 4-10=-209/211, 4-8=-209/211, 5-8=-238/573

JOINT STRESS INDEX

2 = 0.70, 3 = 0.73, 4 = 0.39, 5 = 0.73, 6 = 0.70, 8 = 0.39, 9 = 0.65 and 10 = 0.39

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

EbAll hearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Design Engineer Florida PE No. 34899 1 100 Caastal Bay Blyn Boynton Besch, FL 33435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	00	
	Т04	HIP	1	1		J1925009
	1355.7		100		Job Reference (optional)	

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NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 2 and 408 lb uplift at joint 6.

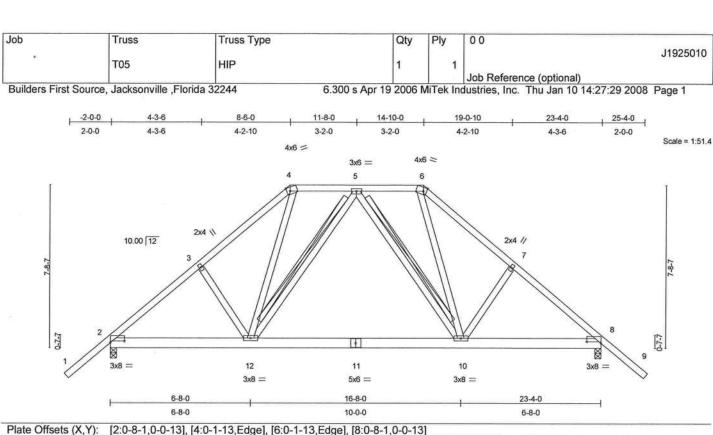
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-10=-10, 8-10=-70, 6-8=-10

Julius Lee Truse Design Engineer Flonda PE No. 24869 1109 Coasial Bay Blvd





LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.21	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.30	10-12	>910	240	The second secon	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)	And the second section to a					Weight: 164 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.3

BRACING

WEBS

TOP CHORD BOT CHORD Structural wood sheathing directly applied or

4-11-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace:

2 X 4 SYP No.3 - 5-12,

5-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1162/0-3-8, 8=1162/0-3-8

Max Horz 2=-238(load case 4)

Max Uplift 2=-428(load case 6), 8=-428(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1485/864, 3-4=-1351/917, 4-5=-833/694, 5-6=-833/694, 6-7=-1351/917,

7-8=-1485/864, 8-9=0/69

BOT CHORD 2-12=-458/1033, 11-12=-311/868, 10-11=-311/868, 8-10=-402/1033

WEBS 3-12=-97/209, 5-12=-152/182, 5-10=-152/182, 7-10=-97/209, 4-12=-382/599,

6-10=-382/599

JOINT STRESS INDEX

2 = 0.84, 3 = 0.34, 4 = 0.57, 5 = 0.42, 6 = 0.57, 7 = 0.34, 8 = 0.84, 10 = 0.65, 11 = 0.81 and 12 = 0.65 there begins to be a second control of the control

NOTES

1) Unbalanced roof live loads have been considered for this design.

January 10,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job	Truss	Truss Type	Qty	Ply	00	
	T05	HIP	1	1		J1925010
					Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 428 lb uplift at joint 2 and 428 lb uplift at joint 8.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 6-9=-54, 2-12=-10, 10-12=-70, 8-10=-10

Julius Les Trues Design Engineer Florida PE No. 34869 1100 Coastal Bay Blvd.





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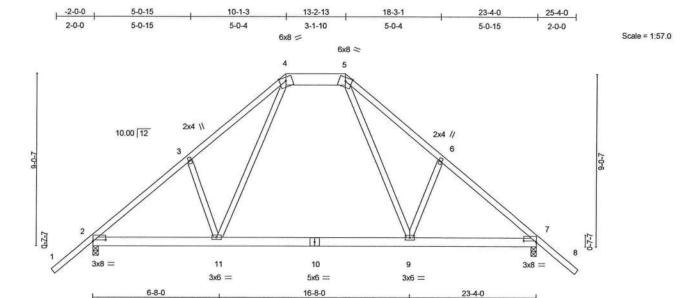


Plate Offsets (X,Y): [2:0-8-1,0-0-13], [7:0-8-1,0-0-13] LOADING (psf) SPACING 2-0-0 CSI DEFL **PLATES** in (loc) I/defl L/d GRIP TCLL 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) 0.21 9-11 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.319-11 >904 240 BCLL 10.0 Rep Stress Incr NO WB 0.68 Horz(TL) 0.02 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 151 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

4-5 2 X 8 SYP No.1D

BOT CHORD 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 5-0-6

oc purlins

6-8-0

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1162/0-3-8, 7=1160/0-3-8

Max Horz 2=-280(load case 4)

Max Uplift 2=-436(load case 6), 7=-435(load case 7)

6-8-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1488/850, 3-4=-1362/976, 4-5=-749/680, 5-6=-1353/967, 6-7=-1482/847,

7-8=0/69

BOT CHORD 2-11=-416/1034, 10-11=-227/749, 9-10=-227/749, 7-9=-377/1030

WEBS 3-11=-184/272, 4-11=-457/631, 5-9=-450/626, 6-9=-184/271

JOINT STRESS INDEX

2 = 0.78, 3 = 0.34, 4 = 0.37, 5 = 0.37, 6 = 0.34, 7 = 0.78, 9 = 0.56, 10 = 0.80 and 11 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

E) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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ì	Job	Truss	Truss Type	Qty	Ply	00	
	*	T06	HIP	1	4	J	1925011
		100	rue		I.	Job Reference (optional)	

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NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 436 lb uplift at joint 2 and 435 lb uplift at joint 7.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-54, 5-8=-54, 2-11=-10, 9-11=-70, 7-9=-10





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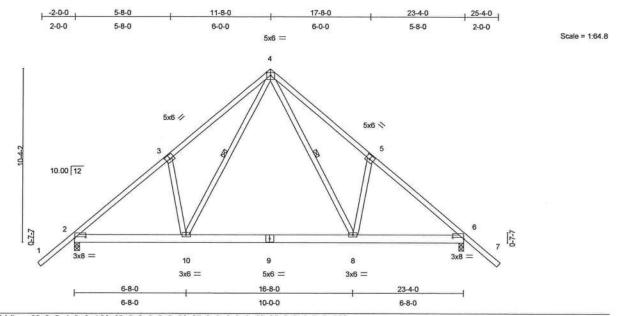


Plate Offsets (X,Y): [2:0-8-1,0-0-13], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-8-1,0-0-13]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.21	8-10	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.32	8-10	>859	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Matrix)							Weight: 154 lb	

LUMBER				
	 IR	•	_	
	 IIV	ıĸ	-	ĸ

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

1 Row at midpt

4-10, 4-8

REACTIONS (lb/size) 2=1162/0-3-8, 6=1162/0-3-8

Max Horz 2=-325(load case 4)

Max Uplift 2=-440(load case 6), 6=-440(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1488/801, 3-4=-1390/1008, 4-5=-1390/1008, 5-6=-1488/801,

6-7=0/69

BOT CHORD 2-10=-335/1034, 9-10=-121/660, 8-9=-121/660, 6-8=-335/1034

WEBS 3-10=-225/360, 4-10=-558/753, 4-8=-558/753, 5-8=-225/360

JOINT STRESS INDEX

2 = 0.76, 3 = 0.67, 4 = 0.71, 5 = 0.67, 6 = 0.76, 8 = 0.61, 9 = 0.79 and 10 = 0.61

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

AV Truse Design Engineer Florida PE No. 34889 1100 Crastal Bay Blvd Boynton Beach, FL 33435

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive leads a page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
•	Т07	COMMON	3	1		J1925012
	The state of the s				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:19:54 2008 Page 2

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 2 and 440 lb uplift at joint 6.

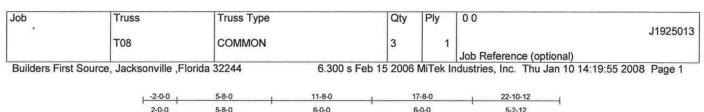
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70, 6-8=-10

Julius Lee Trues Design Engineer Florida PE No. 34869 1189 Goastal Bay Blvd





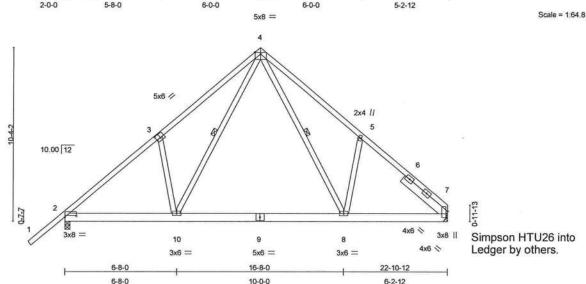


Plate Offsets (X,Y):	[2:0-8-1,0-0-13]], [3:0-3-0,0-3-0], [7:0-4-8,0-1-4]
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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.20	8-10	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.32	8-10	>852	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	2002 (Matrix)							Weight: 156 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D **WEBS** 2 X 4 SYP No.3

SLIDER Right 2 X 6 SYP No.1D 3-4-15

BRACING

TOP CHORD Structural wood sheathing directly applied or

4-11-8 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS 1 Row at midpt

4-10, 4-8

REACTIONS (lb/size) 7=1030/Mechanical, 2=1155/0-3-8

Max Horz 2=362(load case 5)

Max Uplift 7=-312(load case 7), 2=-438(load case 6)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/69, 2-3=-1477/804, 3-4=-1377/1010, 4-5=-1341/1021, 5-6=-1359/827,

6-7=-1451/805

BOT CHORD 2-10=-460/1025, 9-10=-161/653, 8-9=-161/653, 7-8=-463/994

WEBS 3-10=-222/358, 4-10=-549/751, 4-8=-572/711, 5-8=-189/339

JOINT STRESS INDEX

2 = 0.76, 3 = 0.66, 4 = 0.57, 5 = 0.33, 6 = 0.00, 7 = 0.87, 7 = 0.26, 7 = 0.26, 8 = 0.57, 9 = 0.79 and 10 = 0.61

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 🕏 B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Coffy Rue of page 2

January 10,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conf Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	Т08	COMMON	3	1		J1925013
					Job Reference (optional)	

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NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 7 and 438 lb uplift at joint 2.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70, 7-8=-10

Julius Lee Truss Design Engineer Florida PE No. 24869 1 109 Gestal Bay Blyd



Job	Truss	Truss Type	Qty	Ply	00	
	Т09	COMMON	5	1		J1925014
					Job Reference (optional)	

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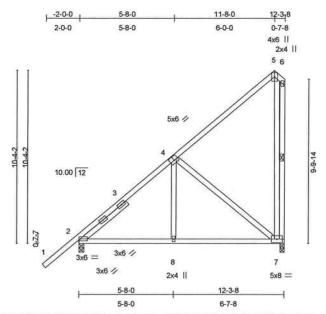


Plate Of	fsets (X, Y	(): [4:0-3-0,0-3-0]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.06	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.31	Horz(TL)	-0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		, ,					Weight: 99 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3 *Except*

6-7 2 X 4 SYP No.2

SLIDER

Left 2 X 4 SYP No.2 3-6-10

BRACING

BOT CHORD

WEBS

TOP CHORD

Structural wood sheathing directly applied or

5-7

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing. 1 Row at midpt

REACTIONS (lb/size) 2=507/0-3-8, 7=379/0-3-0

Max Horz 2=460(load case 6)

Max Uplift 2=-116(load case 6), 7=-280(load case 6) Max Grav 2=507(load case 10), 7=379(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-423/0, 3-4=-245/0, 4-5=-131/80, 5-6=-182/125, 6-7=-301/193

BOT CHORD 2-8=-306/241, 7-8=-305/241

WEBS 4-8=0/193, 5-7=-356/532, 4-7=-284/358

JOINT STRESS INDEX

2 = 0.53, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.35, 5 = 0.66, 6 = 0.51, 7 = 0.35 and 8 = 0.14

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

January 10,2008

Scale = 1:64.8

Continued on page 2

🗥 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
,	T09	COMMON	5	1		J1925014
					Job Reference (optional)	

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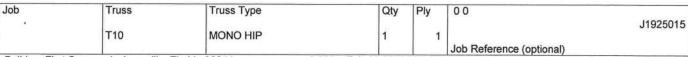
NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 7.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2 and 280 lb uplift at joint 7.

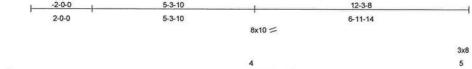
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34889 1100 Coestal Bay Blvd





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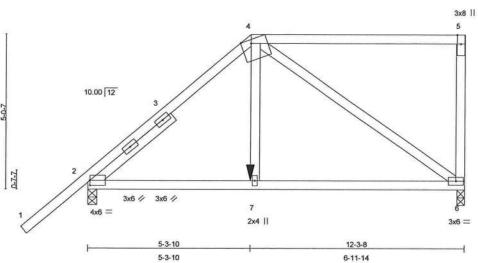


Plate Offsets ()	X Y).	12:0-0-0 0-2-0], [4:0-3-13,Edge]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.11	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 73 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3 *Except*

5-6 2 X 4 SYP No.2

SLIDER

Left 2 X 4 SYP No.2 3-5-0

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=878/0-3-0, 2=836/0-3-8

Max Horz 2=270(load case 5)

Max Uplift 6=-416(load case 4), 2=-379(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-921/307, 3-4=-756/332, 4-5=-106/58, 5-6=-411/295

BOT CHORD 2-7=-317/635, 6-7=-319/646 WEBS 4-7=-72/369, 4-6=-658/329

JOINT STRESS INDEX

2 = 0.87, 2 = 0.17, 2 = 0.17, 3 = 0.00, 4 = 0.87, 5 = 0.82, 6 = 0.74 and 7 = 0.26

NOTES

 Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee Truss Design Engineer Florida PE No. 24866 1100 Caastal Bay Blvd Boynton Beach, FL 33436

January 10,2008

Scale = 1:35.3

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, eraction and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T10	MONO HIP	1	1		J1925015
	(Name)				Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 416 lb uplift at joint 6 and 379 lb uplift at joint 2.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-118(B=-64), 2-7=-10, 6-7=-22(B=-12)

Concentrated Loads (lb)

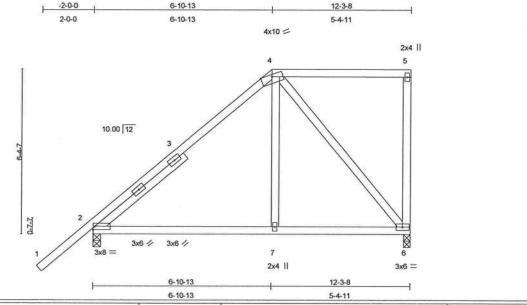
Vert: 7=-311(B)

Julius Les Truse Design Engineer Flonda PE No. 34899 1109 Ceastal Bay Blvd





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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.04	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.07	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.34	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002 (Matrix)		8.0					Weight: 79 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.2 4-5-8

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=373/0-3-0, 2=516/0-3-8

Max Horz 2=320(load case 6)

Max Uplift 6=-159(load case 5), 2=-200(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=

1-2=0/66, 2-3=-408/98, 3-4=-278/116, 4-5=-12/12, 5-6=-117/108

BOT CHORD

2-7=-225/213, 6-7=-225/215

WEBS

4-7=0/201, 4-6=-311/335

JOINT STRESS INDEX

2 = 0.64, 2 = 0.08, 2 = 0.08, 3 = 0.00, 4 = 0.77, 5 = 0.57, 6 = 0.18 and 7 = 0.14

NOTES

- Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
 Continued on page 2

Truse Design Engineer Florida PE No. 34899 1100 Coastal Bay Blyd Boynton Beach, Ft. 93495

January 10,2008

Scale = 1:42.0

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
,	T11	MONO HIP	1	1		J1925016
	1 2500				Job Reference (optional)	

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NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 6 and 200 lb uplift at joint 2.

LOAD CASE(S) Standard

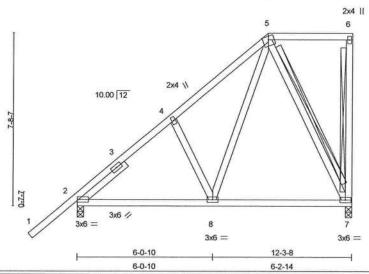


Job	Truss	Truss Type	Qty	Ply	00	
	T12	MONO HIP	1	1	N	J1925017
					Job Reference (optional)	

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Scale: 1/4"=1"



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	7-8	>999	240	2557F/18/75/	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.19	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 88 lb	

LUMBER					
		IN	ID	E	D

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS**

SLIDER

Left 2 X 4 SYP No.2 2-8-1

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 6-7, 5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=373/0-3-8, 2=516/0-3-8

Max Horz 2=371(load case 6)

Max Uplift 7=-170(load case 6), 2=-180(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-442/58, 3-4=-306/76, 4-5=-326/159, 5-6=-9/1, 6-7=-80/74

BOT CHORD 2-8=-323/260, 7-8=-157/136

4-8=-144/248, 5-8=-184/245, 5-7=-293/352 **WEBS**

JOINT STRESS INDEX

2 = 0.72, 2 = 0.18, 3 = 0.00, 4 = 0.14, 5 = 0.58, 6 = 0.23, 7 = 0.23 and 8 = 0.23

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
*	T12	MONO HIP	1	1		J1925017
		100000000000000000000000000000000000000		2	Job Reference (optional)	

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 7 and 180 lb uplift at joint 2.

LOAD CASE(S) Standard

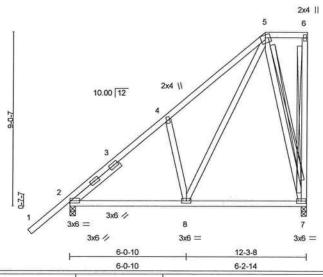


Job	Truss	Truss Type	Qty	Ply	00	
*	T13	MONO HIP	1	1		J1925018
		1			Job Reference (optional)	

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Scale = 1:56.3



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.39	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	7-8	>999	240	333703670703	
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.54	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0		Code FBC2004/TI	PI2002	(Mat	rix)	10.00000-10.000-20					Weight: 96 lb	

L			

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

SLIDER

Left 2 X 4 SYP No.2 3-2-5

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.3 - 6-7, 5-7 T-Brace:

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=373/0-3-8, 2=516/0-3-8

Max Horz 2=422(load case 6)

Max Uplift 7=-222(load case 6), 2=-151(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-436/0, 3-4=-275/0, 4-5=-355/181, 5-6=-7/0, 6-7=-34/42

BOT CHORD 2-8=-321/252, 7-8=-103/81

WEBS 5-8=-334/295, 4-8=-212/355, 5-7=-344/454

JOINT STRESS INDEX

2 = 0.67, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.19, 5 = 0.46, 6 = 0.17, 7 = 0.36 and 8 = 0.28

NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
S#1	T13	MONO HIP	1	1		J1925018
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 14:41:24 2008 Page 2

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 7 and 151 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
	T14	MONO HIP	1	1		J1925019
					Job Reference (optional)	

11-8-6

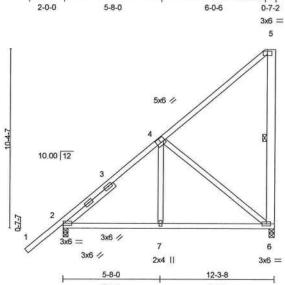
5-8-0

-2-0-0

Builders First Source, Jacksonville , Florida 32244

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12-3-8



					3x6 =
10-4-7			5x6 1/		
10-	10.00 12	3/			
177	2 3x6 =	3x6 //	7		6
	3x	6 // 5-8-0	2x4	12-3-8	3x6 =
	-	5-8-0		6-7-8	

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.03	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	6-7	>999	240		
BCLL	10.0	 Rep Stress Incr 	YES	WB	0.32	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)						Weight: 93 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

5-6 2 X 6 SYP No.1D

Plate Offsets (X V): [4:0-3-0 0-3-0] [5:0-3-0 Edge]

SLIDER

Left 2 X 4 SYP No.2 3-8-5

BRACING

WEBS

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

REACTIONS

(lb/size) 6=370/0-3-8, 2=514/0-3-8

Max Horz 2=478(load case 6)

Max Uplift 6=-293(load case 6), 2=-106(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-431/0, 3-4=-254/0, 4-5=-167/84, 5-6=-159/242

BOT CHORD 2-7=-318/248, 6-7=-317/249 **WEBS** 4-7=0/187, 4-6=-286/367

JOINT STRESS INDEX

2 = 0.57, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.37, 5 = 0.86, 6 = 0.41 and 7 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 293 lb uplift at Colflint & and page buplift at joint 2.

January 10,2008

Scale = 1:62.9

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T14	MONO HIP	1	1		J1925019
		MISS. 155-1, 111			Job Reference (optional)	

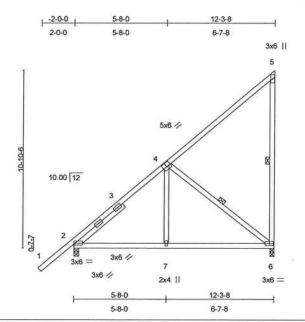
6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 14:41:30 2008 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
	T15	MONO TRUSS	3	1		J1925020
					Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.07	6-7	>999	240)275325272746	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	7750-208-2 X 0.74 X					Weight: 86 lb	

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.2 3-9-8

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

1 Row at midpt

5-6, 4-6

REACTIONS (lb/size) 6=373/0-3-0, 2=516/0-3-8

Max Horz 2=485(load case 6)

Max Uplift 6=-299(load case 6), 2=-102(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/66, 2-3=-438/0, 3-4=-261/0, 4-5=-173/87, 5-6=-153/235

BOT CHORD 2-7=-329/256, 6-7=-328/257

WEBS 4-7=0/199, 4-6=-305/389

JOINT STRESS INDEX

2 = 0.57, 2 = 0.09, 2 = 0.09, 3 = 0.00, 4 = 0.42, 5 = 0.46, 6 = 0.37 and 7 = 0.14

NOTES

 Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Configuration (by others) of truss to bearing plate at joint(s) 6.

January 10,2008

Scale = 1:66.5

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	W100-0-0-0000
,	T15	MONO TRUSS	3	1	i i	J1925020
	115	More mose	3		Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 6 and 102 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	200
	T16G	GABLE	1	1		J1925021
	1100	OABEL			Job Reference (optional)	

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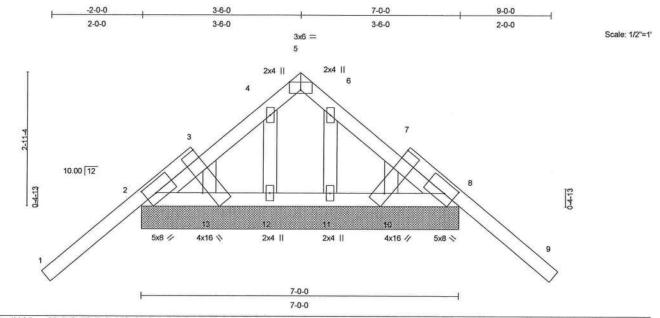


Plate Of	fsets (X,Y):	[2:0-2-12,0-2-12], [5	:0-3-0,Edg	je], [8:0-2	2-12,0-2-1	2], [10:Edge,0)-2-8], [1	3:Edge	,0-5-3]			
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.06	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.10	9	n/r	90	141 511 (000000)	Service and Alberta
BCLL	10.0	* Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)	An distriction ()				(2000 BB)	Weight: 43 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=527/7-0-0, 8=527/7-0-0, 12=174/7-0-0, 13=-5/7-0-0, 11=174/7-0-0, 10=-5/7-0-0

Max Horz 2=110(load case 6)

Max Uplift 2=-323(load case 6), 8=-328(load case 7), 12=-85(load case 6), 13=-11(load case

7), 11=-75(load case 7), 10=-10(load case 4)

Max Grav 2=527(load case 1), 8=527(load case 1), 12=174(load case 1), 13=84(load case 6), 11=174(load case 1), 10=83(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-47/155, 2-3=-176/80, 3-4=-49/37, 4-5=-62/51, 5-6=-62/51, 6-7=-49/37, 7-8=-176/80,

8-9=-47/155

2-13=0/254, 12-13=0/250, 11-12=0/250, 10-11=0/250, 8-10=0/254

WEBS 4-12=-161/143, 3-13=-91/22, 6-11=-161/143, 7-10=-91/22

JOINT STRESS INDEX

BOT CHORD

2 = 0.65, 3 = 0.00, 4 = 0.09, 5 = 0.02, 6 = 0.09, 7 = 0.00, 8 = 0.65, 10 = 0.05, 11 = 0.08, 12 = 0.08 and 13 = 0.05

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

January 10,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0.0	A. C.
,	T16G	GABLE	1	1		J1925021
					Job Reference (optional)	

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NOTES

- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 2, 328 lb uplift at joint 8, 85 lb uplift at joint 12, 11 lb uplift at joint 13, 75 lb uplift at joint 11 and 10 lb uplift at joint 10.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-118, 5-9=-118, 2-8=-10

Julius Lee Truss Design Engineer Flonda PE No. 24869 1 109 Coesial Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
	T17	COMMON	2	1		J1925022
		COMMON			Job Reference (optional)	

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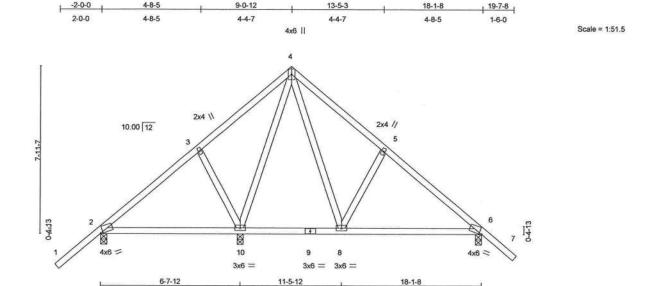


Plate Offsets (X,Y): [2:0-1-6,0-0-6], [6:0-1-6,0-0-6]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.12	2-10	>650	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.10	2-10	>744	240	NAME OF THE PARTY	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)	DANGER OF THE					Weight: 105 lb	

4-10-0

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	нил	-	×

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural woo

Structural wood sheathing directly applied or

6-0-0 oc purlins.

6-7-12

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=339/0-3-8, 10=559/0-3-8, 6=458/0-3-8

Max Horz 2=258(load case 5)

Max Uplift 2=-257(load case 6), 10=-292(load case 6), 6=-204(load case 7)

Max Grav 2=342(load case 10), 10=559(load case 1), 6=458(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/73, 2-3=-179/109, 3-4=-64/160, 4-5=-290/260, 5-6=-407/173, 6-7=0/55

6-7-12

BOT CHORD 2-10=-104/203, 9-10=0/258, 8-9=0/258, 6-8=0/249

WEBS 3-10=-196/297, 4-10=-355/97, 4-8=-164/242, 5-8=-197/272

JOINT STRESS INDEX

2 = 0.82, 3 = 0.33, 4 = 0.47, 5 = 0.33, 6 = 0.82, 8 = 0.49, 9 = 0.26 and 10 = 0.49

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Les Trues Design Engineer Flonds FE No. 24868 1100 Coastal Bay Blvd Boynton Besch, FL 30425

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive load nonconc

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job	Truss	Truss Type	Qty	Ply	00	
	T17	COMMON	2	1		J1925022
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:01 2008 Page 2

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 lb uplift at joint 2, 292 lb uplift at joint 10 and 204 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34869 1109 Crestal Bay Blvd Boynton Besch, FL 33436



Job	Truss	Truss Type	Qty	Ply	00	
	T17G	GABLE	1	1		J1925023
*	10	O, IDEE			Job Reference (optional)	

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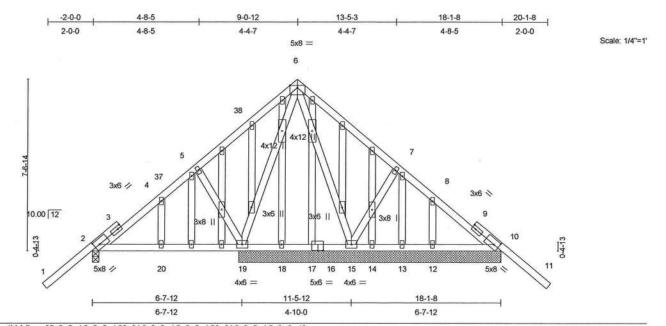


Plate Offsets (X,Y):	[2:0-2-12,0-2-12],	[10:0-2-12,0-2-12], [[16:0-2-12,0-0-4]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	0.04	19-20	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	0.03	19-20	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2004/T	PI2002	(Mat	rix)						Weight: 166 lb	

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L		ΝИ	n	_	п
	u	IVI	ь	_	ĸ

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3 **OTHERS**

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=399/0-3-8, 19=727/11-7-8, 15=570/11-7-8, 10=590/11-7-8, 18=-6/11-7-8, 17=22/11-7-8, 14=10/11-7-8, 13=13/11-7-8, 12=155/11-7-8

Max Horz 2=236(load case 5)

Max Uplift 2=-317(load case 6), 19=-578(load case 6), 15=-325(load case 6), 10=-339(load case 7), 18=-6(load case 1), 17=-3(load case 4), 13=-10(load case 7), 12=-84(load case 6)

Max Grav 2=403(load case 10), 19=727(load case 1), 15=570(load case 1), 10=596(load case 11), 18=48(load case 4), 17=62(load case 2), 14=35(load case 2), 13=26(load case 2), 12=155(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/73, 2-3=-290/279, 3-4=-225/304, 4-37=-186/206, 5-37=-84/138, 5-38=-23/233,

6-38=-95/231, 6-7=-116/215, 7-8=-174/210, 8-9=-167/172, 9-10=-284/268, 10-11=-120/1495-13-2-13-20-144/473, 49-40-20/251, 47-40-20

BOT CHORD 2-20=-114/173, 19-20=-114/173, 18-19=-26/251, 17-18=-26/251, 16-17=-26/251,

15-16=-26/251, 14-15=-9/129, 13-14=-9/129, 12-13=-9/129, 10-12=-9/129

5-19=-423/715, 6-19=-337/370, 6-15=-309/258, 7-15=-321/502, 4-20=-155/78,

8-12=-138/144

JOINT STRESS INDEX

WEBS

2 = 0.75, 3 = 0.00, 3 = 0.53, 4 = 0.34, 5 = 0.40, 6 = 0.60, 7 = 0.40, 8 = 0.34, 9 = 0.00, 9 = 0.53, 10 = 0.75, 12 = 0.34, 13 = 0.34, 14 = 0.340.34, 15 = 0.35, 16 = 0.46, 17 = 0.00, 18 = 0.34, 19 = 0.35, 20 = 0.34, 21 = 0.33, 22 = 0.34, 23 = 0.16, 24 = 0.34, 25 = 0.79, 26 a TOU a A Y 10,2008Continued on page 20.34, 29 = 0.34, 30 = 0.34, 31 = 0.33, 32 = 0.16, 33 = 0.34, 34 = 0.34, 35 = 0.79 and 36 = 0.34

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T17G	GABLE	1	1		J1925023
	1		l'		Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2, 578 lb uplift at joint 19, 325 lb uplift at joint 15, 339 lb uplift at joint 10, 6 lb uplift at joint 18, 3 lb uplift at joint 17, 10 lb uplift at joint 13 and 84 lb uplift at joint 12.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-37=-54, 37-38=-141(F=-87), 6-38=-114(F=-60), 6-11=-114(F=-60), 2-10=-10

Julius Les Truss Design Engineer Florida PE No. 34866 1160 Ceastal Bay Blvd Bovnton Besch, FL 33436



Job	Truss	Truss Type	Qty	Ply	00	
١.	T18	COMMON	1	1		J1925024
		Commerc			Job Reference (optional)	

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Scale: 1/4"=1"

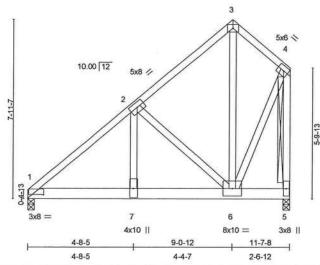


Plate Of	ffsets (X,Y)	[1:0-8-6,0-1-14]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.06	1-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.11	1-7	>999	240	142 c c c c c c c c c c c c c c c c c c c	
BCLL	10.0	* Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)						Weight: 91 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3 *Except*

4-5 2 X 4 SYP No.2

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 3-7-6 oc purlins, except end verticals.

Rigid ceiling directly applied or 8-5-15 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 4-5 Fasten T and I braces to narrow edge of web with

10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=2486/0-3-8, 5=2486/0-3-8

Max Horz 1=240(load case 4)

Max Uplift 1=-899(load case 5), 5=-1008(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2606/916, 2-3=-969/378, 3-4=-919/388, 4-5=-2142/878

BOT CHORD 1-7=-857/1946, 6-7=-857/1946, 5-6=-7/15

WEBS 2-7=-733/1943, 2-6=-1695/773, 3-6=-402/975, 4-6=-686/1665

JOINT STRESS INDEX

1 = 0.84, 2 = 0.69, 3 = 0.58, 4 = 0.77, 5 = 0.35, 6 = 0.47 and 7 = 0.47

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

A) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

January 10,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
(A)	T18	COMMON	1	,		J1925024
	110	COMMON	l'		Job Reference (optional)	

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 899 lb uplift at joint 1 and 1008 lb uplift at joint 5.
- 6) Girder carries tie-in span(s): 25-5-0 from 0-0-0 to 11-7-8
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 1-5=-385(F=-375)





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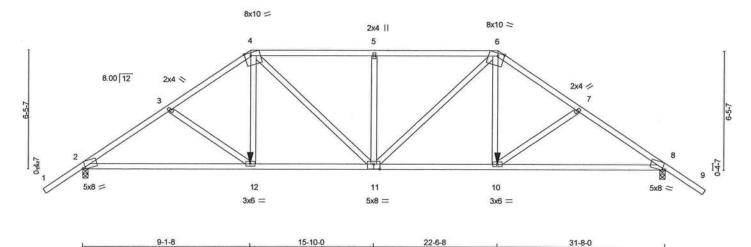


Plate Of	ffsets (X,Y):	[2:0-1-13,Edge], [4:	0-4-0,Edge], [6:0-4-	0,Edge],	[8:0-1-13,Edge	∋], [11:0-	4-0,0-3	-4]			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.30	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.42	10-11	>895	240	90000000000000	
BCLL	10.0	Rep Stress Incr	NO	WB	0.85	Horz(TL)	0.14	8	n/a	n/a		

TCDL 7.0 Lumber Increase 1.25 BC 0.92 Vert(TL) -0.42 10-11 >895 240 BCLL 10.0 * Rep Stress Incr NO WB 0.85 Horz(TL) 0.14 8 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 175 lb	LUMBER	3						BRACING					
	BCLL	10.0	*	Rep Stress Incr	NO	WB	0.85		200	0.00		Weight: 175 lb	

6-8-8

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 TOP CHORD

6-8-8

Structural wood sheathing directly applied or 3-2-2

9-1-8

oc purlins.

BOT CHORD Rigid ceiling directly applied or 4-8-12 oc bracing.

REACTIONS (lb/size) 2=2170/0-3-8, 8=2170/0-3-8

Max Horz 2=-200(load case 3)

9-1-8

Max Uplift 2=-1202(load case 5), 8=-1202(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-3352/1915, 3-4=-3179/1916, 4-5=-3169/1961, 5-6=-3169/1961,

6-7=-3179/1916, 7-8=-3352/1916, 8-9=0/62

BOT CHORD 2-12=-1673/2681, 11-12=-1634/2622, 10-11=-1491/2622, 8-10=-1473/2681 WEBS

3-12=-158/98, 4-12=-544/850, 4-11=-605/820, 5-11=-507/396, 6-11=-605/820,

6-10=-544/850, 7-10=-158/98

JOINT STRESS INDEX

2 = 0.86, 3 = 0.34, 4 = 0.83, 5 = 0.34, 6 = 0.83, 7 = 0.34, 8 = 0.86, 10 = 0.56, 11 = 0.82 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

January 10,2008

Continued on page 2

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
•	T19	HIP	1	1	J192	25025
	11.0			'	Job Reference (optional)	

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1202 lb uplift at joint 2 and 1202 lb uplift at joint 8.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

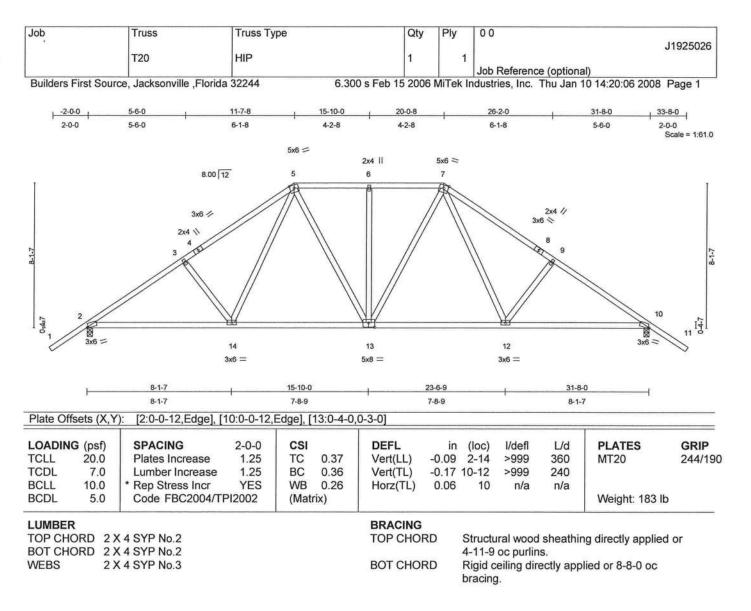
Vert: 1-4=-54, 4-6=-72(F=-18), 6-9=-54, 2-12=-10, 10-12=-69(F=-59), 8-10=-10

Concentrated Loads (lb)

Vert: 12=-535(F) 10=-535(F)

Julius Les Truss Design Engineer Flonda PE No. 24868 1109 Coestal Bay Blvd Boynton Besch, FL 23436





REACTIONS (lb/size) 2=1124/0-3-8, 10=1124/0-3-8

Max Horz 2=-256(load case 4)

Max Uplift 2=-391(load case 6), 10=-391(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1536/888, 3-4=-1364/878, 4-5=-1272/905, 5-6=-1000/790,

6-7=-1000/790, 7-8=-1272/905, 8-9=-1364/878, 9-10=-1536/888, 10-11=0/62

BOT CHORD 2-14=-515/1200, 13-14=-294/935, 12-13=-259/935, 10-12=-515/1200

WEBS 3-14=-245/297, 5-14=-193/333, 5-13=-190/259, 6-13=-197/151, 7-13=-190/259,

7-12=-193/333, 9-12=-245/297

JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.33, 5 = 0.72, 6 = 0.33, 7 = 0.72, 8 = 0.33, 9 = 0.33, 10 = 0.78, 12 = 0.46, 13 = 0.38 and 14 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp Hills Exp H

Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T20	HIP	1	1		J1925026
	1000000				Job Reference (optional)	

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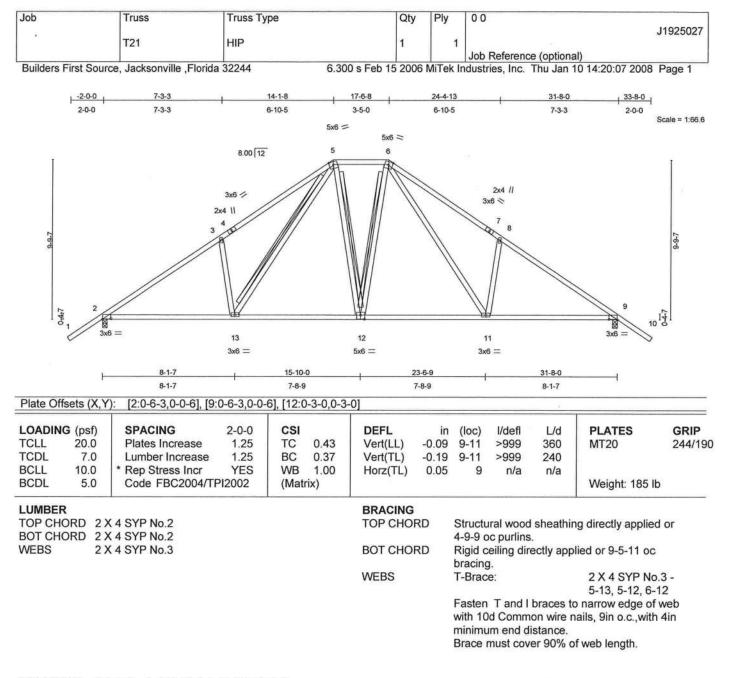
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 2 and 391 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda ME No. 24866 1100 Coastal Bay Blvd.





REACTIONS (lb/size) 2=1124/0-3-8, 9=1124/0-3-8

Max Horz 2=-311(load case 4)

Max Uplift 2=-405(load case 6), 9=-429(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1506/836, 3-4=-1435/1017, 4-5=-1415/1049, 5-6=-829/726,

6-7=-1413/1047, 7-8=-1433/1015, 8-9=-1505/836, 9-10=0/62

BOT CHORD 2-13=-444/1157, 12-13=-169/820, 11-12=-133/823, 9-11=-444/1157

3-13=-356/434, 5-13=-448/524, 5-12=-173/216, 6-12=-171/218, 6-11=-444/514,

8-11=-348/430

JOINT STRESS INDEX

WEBS

2 = 0.70, 3 = 0.33, 4 = 0.60, 5 = 0.67, 6 = 0.67, 7 = 0.60, 8 = 0.33, 9 = 0.70, 11 = 0.42, 12 = 0.57 and 13 = 0.42

Continued on page 2

January 10,2008

Engineer Bay Blvd B. FL 33405



Job	Truss	Truss Type	Qty	Ply	0.0	
1 "	T21	HIP	1	1		J1925027
	1.21		1		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:07 2008 Page 2

NOTES

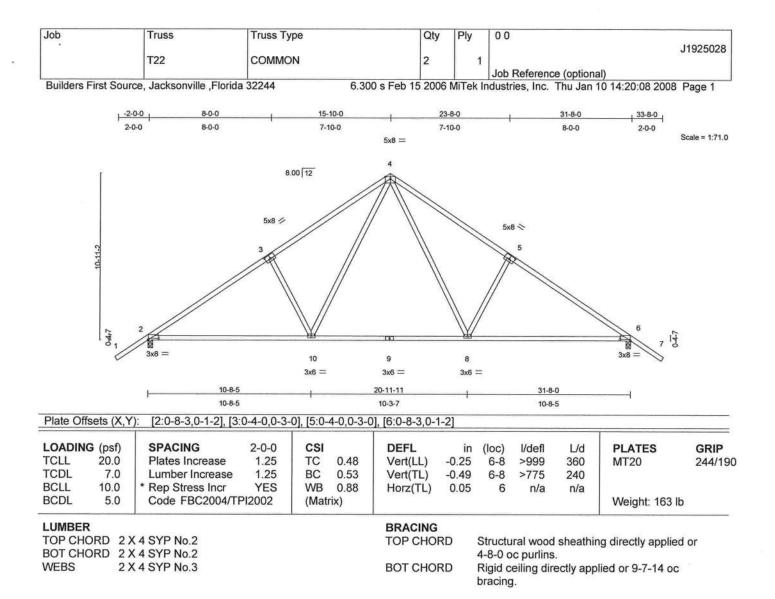
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 2 and 429 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34898 1169 Geestal Bay Blvd Boyoton Beach Et 20425





REACTIONS (lb/size) 2=1124/0-3-8, 6=1124/0-3-8

Max Horz 2=348(load case 5)

Max Uplift 2=-413(load case 6), 6=-413(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1474/821, 3-4=-1277/906, 4-5=-1277/906, 5-6=-1474/821, 6-7=0/62

BOT CHORD 2-10=-420/1127, 9-10=-93/753, 8-9=-93/753, 6-8=-420/1127

WEBS 3-10=-377/429, 4-10=-355/500, 4-8=-355/500, 5-8=-377/429

JOINT STRESS INDEX

2 = 0.73, 3 = 0.69, 4 = 0.72, 5 = 0.69, 6 = 0.73, 8 = 0.45, 9 = 0.38 and 10 = 0.45

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truse Design Engineer Flonds PE No. 34869 1109 Coestal Bay Blvd Boynton Besch, FL 33435

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
	T22	COMMON	2	1		J1925028
	122	COMMON	2	'	Job Reference (optional)	

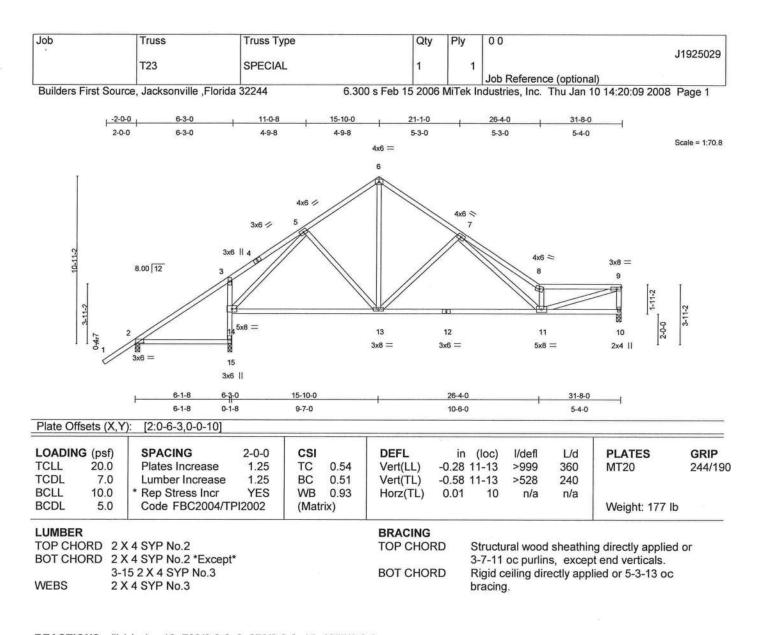
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:08 2008 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 413 lb uplift at joint 2 and 413 lb uplift at joint 6.

LOAD CASE(S) Standard





REACTIONS (lb/size) 10=799/0-3-8, 2=272/0-3-8, 15=1057/0-3-0

Max Horz 2=375(load case 5)

Max Uplift 10=-262(load case 7), 2=-273(load case 4), 15=-472(load case 6) Max Grav 10=799(load case 1), 2=289(load case 10), 15=1057(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-274/319, 3-4=-144/260, 4-5=-123/280, 5-6=-752/568, 6-7=-766/566,

7-8=-2614/1637, 8-9=-1990/1175, 9-10=-762/478

BOT CHORD 2-15=-74/0, 14-15=-1029/653, 3-14=-298/348, 13-14=-289/551, 12-13=-554/975,

11-12=-554/975, 10-11=-97/136

5-13=-65/146, 5-14=-926/450, 6-13=-392/480, 7-13=-558/499, 7-11=-997/1606,

8-11=-1659/1119, 9-11=-1133/1949

Trues Design Engineer Florida FE No. 34869 1100 Caastal Bay Blvd. Boynton Beach, FL 33436

JOINT STRESS INDEX

2 = 0.60, 3 = 0.26, 4 = 0.15, 5 = 0.27, 6 = 0.52, 7 = 0.62, 8 = 0.80, 9 = 0.66, 10 = 0.73, 11 = 0.88, 12 = 0.59, 13 = 0.56, 14 = 0.63 and 15 = 0.44

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T23	SPECIAL	1	1		J1925029
	123	OI LOIAL			Job Reference (optional)	

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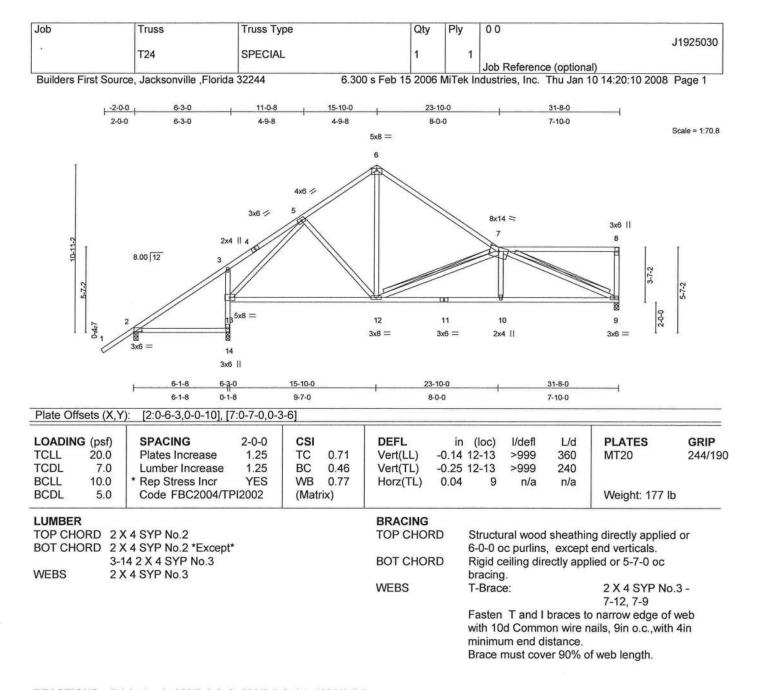
NOTES

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 262 lb uplift at joint 10, 273 lb uplift at joint 2 and 472 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 34869 1109 Coastal Bay Blvd Boynton Beach, Ft. 20405





REACTIONS (lb/size) 9=803/0-3-8, 2=288/0-3-8, 14=1038/0-3-0

Max Horz 2=373(load case 5)

Max Uplift 9=-266(load case 7), 2=-243(load case 4), 14=-491(load case 6) Max Grav 9=803(load case 1), 2=296(load case 10), 14=1038(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-270/269, 3-4=-150/225, 4-5=-130/245, 5-6=-755/554, 6-7=-824/523,

7-8=-83/55, 8-9=-194/158

BOT CHORD 2-14=-74/0, 13-14=-1005/691, 3-13=-323/375, 12-13=-350/558, 11-12=-887/1396,

10-11=-887/1396, 9-10=-884/1398

5-12=-66/134, 5-13=-853/466, 6-12=-274/436, 7-12=-878/655, 7-10=0/229,

7-9=-1431/912

Julius Les Trues Cesign Engineer Flonds PE No. 24869 1109 Coastal Bay Blvd Boynton Beach, Ft. 33435

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	00	
,	T24	SPECIAL	1	4		J1925030
	124	OFLOIAL		'	Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.57, 3 = 0.57, 4 = 0.15, 5 = 0.27, 6 = 0.76, 7 = 0.40, 8 = 0.54, 9 = 0.53, 10 = 0.33, 11 = 0.53, 12 = 0.56, 13 = 0.74 and 14 = 0.48

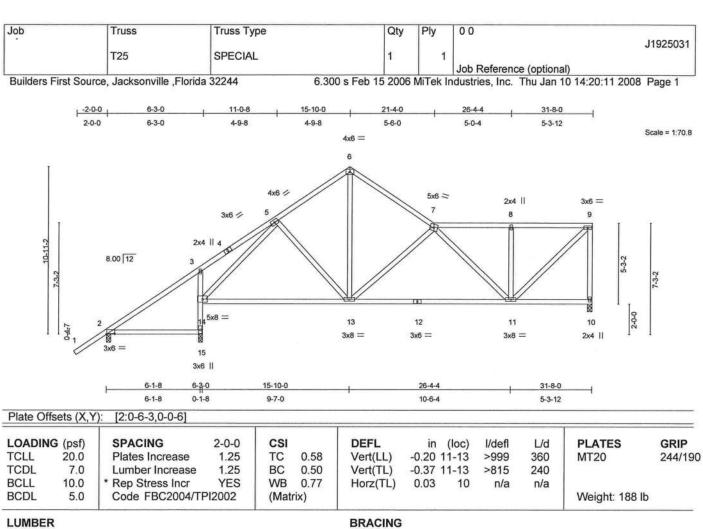
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 9, 243 lb uplift at joint 2 and 491 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda FE No. 34868 1 100 Ceastal Bay Blvd Bovnton Besch, FL 33435





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-15 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 5-9-0 oc

bracing.

REACTIONS (lb/size) 10=805/0-3-8, 2=298/0-3-8, 15=1025/0-3-0

Max Horz 2=398(load case 6)

Max Uplift 10=-272(load case 7), 2=-210(load case 4), 15=-512(load case 6)

Max Grav 10=805(load case 1), 2=302(load case 10), 15=1025(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-270/210, 3-4=-141/167, 4-5=-120/188, 5-6=-763/526, 6-7=-775/518,

7-8=-703/438, 8-9=-702/438, 9-10=-786/521

BOT CHORD 2-15=-82/0, 14-15=-990/735, 3-14=-316/373, 13-14=-412/573, 12-13=-662/996,

11-12=-662/996, 10-11=-17/19

WEBS 5-13=-72/121, 6-13=-326/474, 7-13=-561/447, 5-14=-847/526, 7-11=-412/315,

8-11=-285/245, 9-11=-585/949

Julius Les Truss Design Engineer Florida PE No. 24869 1 109 Casstal Bay Blvd Boynton Beach, FL 33436

JOINT STRESS INDEX

2 = 0.66, 3 = 0.56, 4 = 0.15, 5 = 0.27, 6 = 0.59, 7 = 0.62, 8 = 0.33, 9 = 0.61, 10 = 0.37, 11 = 0.91, 12 = 0.54, 13 = 0.56, 14 = 0.63 and 15 = 0.51

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 10,2008

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Job	Truss	Truss Type	Qty	Ply	00	
1	T25	SPECIAL	1	1	J	1925031
					Job Reference (optional)	

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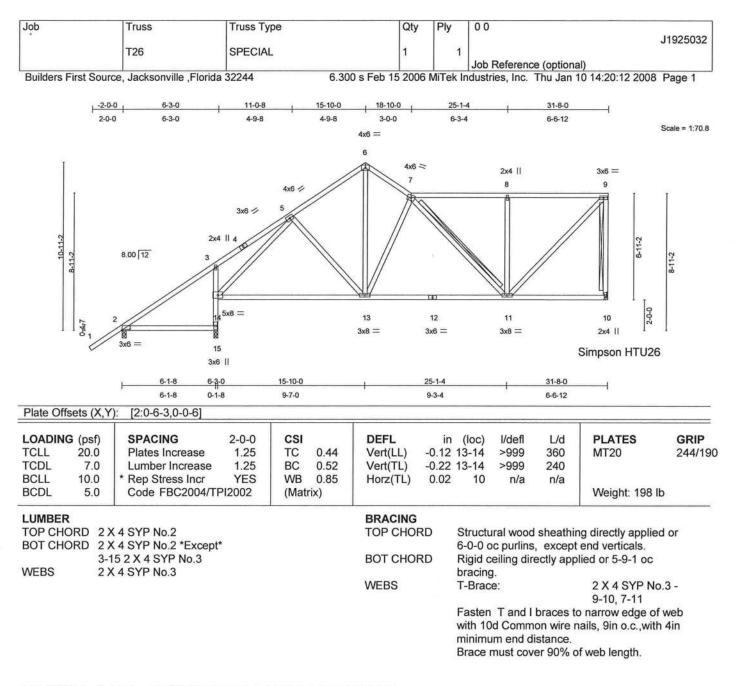
NOTES

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 10, 210 lb uplift at joint 2 and 512 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Lee Truse Cesign Engineer Florida PE No. 34899 1109 Coastel Bay Blyd Boydoo Basel Bay Byd





REACTIONS (lb/size) 10=805/Mechanical, 2=299/0-3-8, 15=1023/0-3-0

Max Horz 2=438(load case 6)

Max Uplift 10=-331(load case 5), 2=-177(load case 4), 15=-530(load case 6) Max Grav 10=805(load case 1), 2=302(load case 10), 15=1023(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-323/153, 3-4=-134/109, 4-5=-114/129, 5-6=-762/495, 6-7=-733/524,

7-8=-621/414, 8-9=-620/415, 9-10=-774/549

BOT CHORD 2-15=-91/0, 14-15=-989/778, 3-14=-312/371, 13-14=-466/576, 12-13=-540/776,

11-12=-540/776, 10-11=-14/20

WEBS 5-14=-848/588, 5-13=-74/123, 6-13=-382/553, 7-13=-498/382, 7-11=-226/183,

8-11=-381/338, 9-11=-576/863

Julius Les Trues Design Engineer Florida PE No. 24869 1100 Coestal Bay Blvd

January 10,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	0.0	
			1	9525		J1925032
	T26	SPECIAL	1	1		
					Job Reference (optional)	,

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:12 2008 Page 2

JOINT STRESS INDEX

2 = 0.64, 3 = 0.55, 4 = 0.15, 5 = 0.27, 6 = 0.37, 7 = 0.63, 8 = 0.33, 9 = 0.73, 10 = 0.47, 11 = 0.83, 12 = 0.42, 13 = 0.61, 14 = 0.69 and 15 = 0.54

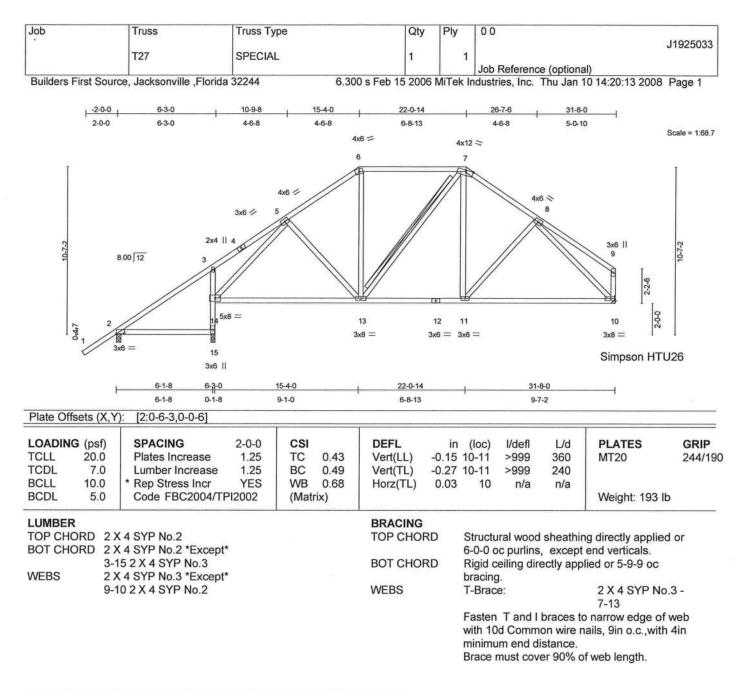
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 10, 177 lb uplift at joint 2 and 530 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24869 1 100 Coastel Bay Blyd Boynton Beach, FL 23436





REACTIONS (lb/size) 2=304/0-3-8, 15=1018/0-3-0, 10=806/Mechanical

Max Horz 2=364(load case 5)

Max Uplift 2=-266(load case 4), 15=-495(load case 5), 10=-222(load case 7) Max Grav 2=304(load case 10), 15=1018(load case 1), 10=806(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-255/310, 3-4=-116/264, 4-5=-104/283, 5-6=-766/593, 6-7=-591/572,

7-8=-781/598, 8-9=-197/153, 9-10=-204/181

BOT CHORD 2-15=-81/4, 14-15=-982/643, 3-14=-319/354, 13-14=-329/565, 12-13=-246/602,

11-12=-246/602, 10-11=-349/611

5-14=-819/427, 5-13=-43/125, 6-13=-51/184, 7-13=-118/108, 7-11=-63/205,

8-11=-103/154, 8-10=-751/460

Julius Les Truse Design Engineer Florida PE No. 34869 1109 Coastal Bay Blvd

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	00	decoultering stawn are
71	T27	SPECIAL	1	1		J1925033
		0. 2512		1	Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:13 2008 Page 2

JOINT STRESS INDEX

2 = 0.66, 3 = 0.68, 4 = 0.15, 5 = 0.28, 6 = 0.72, 7 = 0.90, 8 = 0.27, 9 = 0.34, 10 = 0.70, 11 = 0.37, 12 = 0.23, 13 = 0.56, 14 = 0.73 and 15 = 0.48

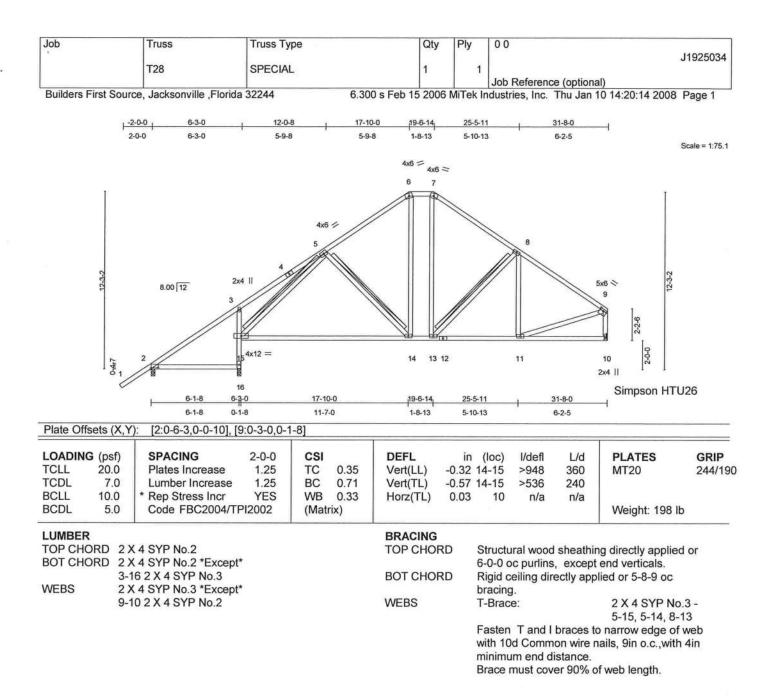
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 2, 495 lb uplift at joint 15 and 222 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1109 Chastal Bay Blvd





REACTIONS (lb/size) 2=298/0-3-8, 16=1026/0-3-0, 10=805/Mechanical

Max Horz 2=419(load case 5)

Max Uplift 2=-319(load case 4), 16=-488(load case 6), 10=-238(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-329/404, 3-4=-183/332, 4-5=-158/356, 5-6=-721/573, 6-7=-524/566,

7-8=-721/579, 8-9=-865/533, 9-10=-769/492

2-16=-93/3, 15-16=-990/624, 3-15=-356/392, 14-15=-333/595, 13-14=-153/524,

12-13=-333/647, 11-12=-333/647, 10-11=-83/95

5-15=-797/377, 5-14=-146/256, 6-14=-125/268, 7-13=-178/135, 8-13=-232/257,

8-11=-129/125, 9-11=-263/581

JOINT STRESS INDEX

BOT CHORD

WEBS

NT STRESS INDEX
2 = 0.57, 3 = 0.38, 4 = 0.22, 5 = 0.26, 6 = 0.67, 7 = 0.52, 8 = 0.41, 9 = 0.82, 10 = 0.83, 11 = 0.34, 12 = 0.30, 13 = 0.36, 14 =

January 10,2008 Continued 55 page 2 and 16 = 0.48

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T28	SPECIAL	1	1		J1925034
					Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:14 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 3x6 MT20 unless otherwise indicated.

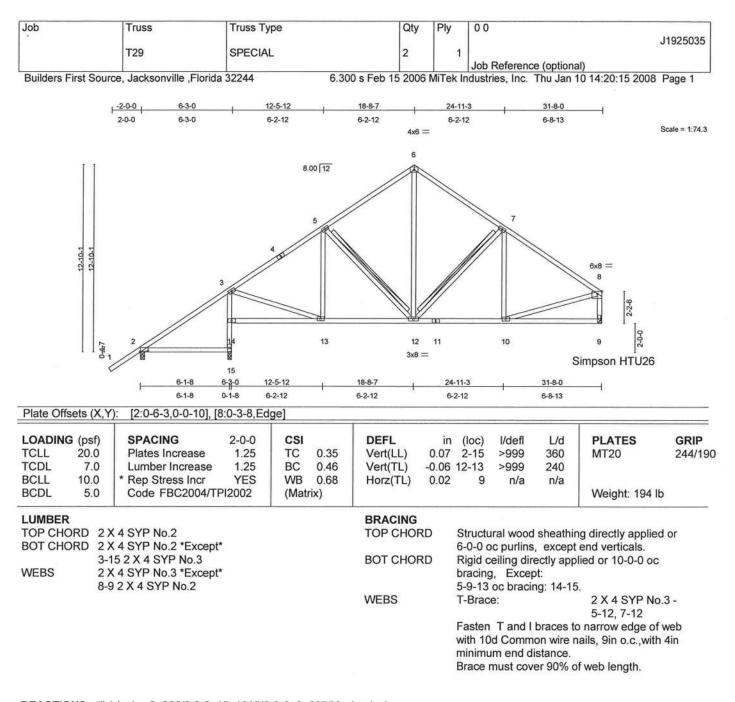
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 2, 488 lb uplift at joint 16 and 238 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34889 1100 Coestel Bay Blvd





REACTIONS (lb/size) 2=306/0-3-8, 15=1015/0-3-0, 9=807/Mechanical

Max Horz 2=437(load case 5)

Max Uplift 2=-314(load case 4), 15=-507(load case 6), 9=-239(load case 7) Max Grav 2=308(load case 10), 15=1015(load case 1), 9=807(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-353/391, 3-4=-845/507, 4-5=-741/522, 5-6=-698/567, 6-7=-699/568,

7-8=-878/529, 8-9=-769/487

BOT CHORD 2-15=-75/21, 14-15=-980/638, 3-14=-947/656, 13-14=-80/48, 12-13=-303/622,

11-12=-316/650, 10-11=-316/650, 9-10=-105/115

WEBS 3-13=-235/621, 5-13=-136/114, 5-12=-220/256, 6-12=-346/375, 7-12=-255/275,

7-10=-103/111, 8-10=-220/559

Truse Design Engineer Flonda PE No. 34898 1109 Crastal Bay Blvd Boynton Beach, FL 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T29	SPECIAL	2	1		J1925035
			VOLUME TO THE REAL PROPERTY OF THE PARTY OF		Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:15 2008 Page 2

JOINT STRESS INDEX

2 = 0.59, 3 = 0.41, 4 = 0.38, 5 = 0.41, 6 = 0.73, 7 = 0.41, 8 = 0.49, 9 = 0.29, 10 = 0.34, 11 = 0.22, 12 = 0.56, 13 = 0.34, 14 = 0.32 and 15 = 0.47

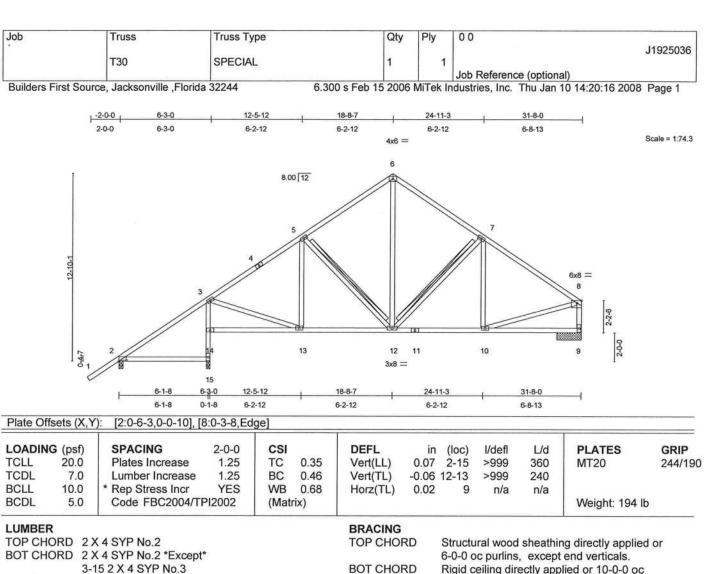
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2, 507 lb uplift at joint 15 and 239 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda PE No. 34869 1189 Coestel Bay Blvd





2 X 4 SYP No.3 *Except*

8-9 2 X 4 SYP No.2

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except:

5-9-13 oc bracing: 14-15.

WEBS T-Brace: 2 X 4 SYP No.3 -5-12, 7-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=306/0-3-8, 15=1015/0-3-0, 9=807/1-8-0

Max Horz 2=437(load case 5)

Max Uplift 2=-314(load case 4), 15=-507(load case 6), 9=-239(load case 7) Max Grav 2=308(load case 10), 15=1015(load case 1), 9=807(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-353/391, 3-4=-845/507, 4-5=-741/522, 5-6=-698/566, 6-7=-699/568,

7-8=-878/529, 8-9=-769/487

BOT CHORD 2-15=-75/21, 14-15=-980/638, 3-14=-947/656, 13-14=-80/48, 12-13=-303/622,

11-12=-316/650, 10-11=-316/650, 9-10=-105/115

WEBS 3-13=-235/621, 5-13=-136/114, 5-12=-220/256, 6-12=-346/375, 7-12=-255/275,

7-10=-103/111, 8-10=-220/559

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	00	
2.	T30	SPECIAL	1	1		J1925036
		5. 25# iZ			Job Reference (optional)	

Builders First Source, Jacksonville , Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:16 2008 Page 2

JOINT STRESS INDEX

2 = 0.58, 3 = 0.41, 4 = 0.38, 5 = 0.41, 6 = 0.73, 7 = 0.41, 8 = 0.49, 9 = 0.29, 10 = 0.34, 11 = 0.22, 12 = 0.56, 13 = 0.34, 14 = 0.32 and 15 = 0.47

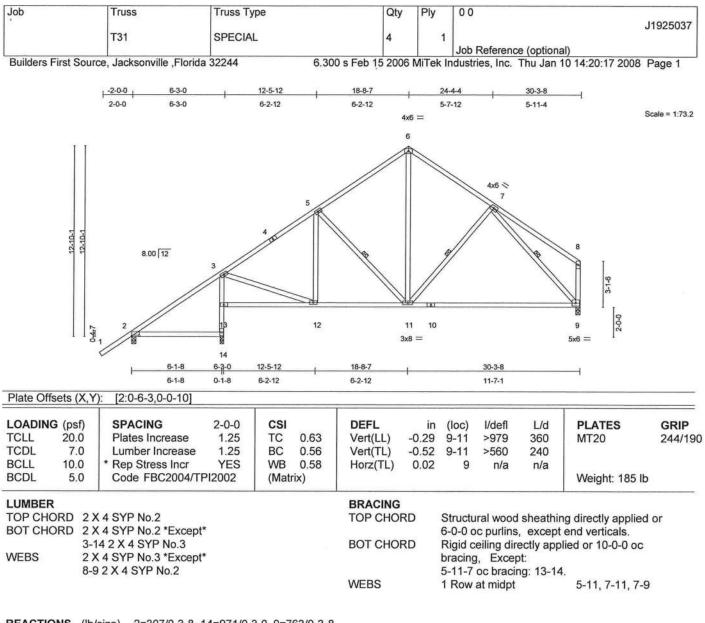
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2, 507 lb uplift at joint 15 and 239 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34865 1 109 Goastal Bay Blvd





REACTIONS (lb/size) 2=307/0-3-8, 14=971/0-3-0, 9=763/0-3-8

Max Horz 2=436(load case 5)

Max Uplift 2=-295(load case 4), 14=-509(load case 6), 9=-220(load case 7) Max Grav 2=308(load case 10), 14=971(load case 1), 9=763(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-350/359, 3-4=-791/462, 4-5=-687/477, 5-6=-641/514, 6-7=-630/520,

7-8=-184/158, 8-9=-207/196

BOT CHORD 2-14=-82/18, 13-14=-936/638, 3-13=-900/660, 12-13=-88/52, 11-12=-314/577,

10-11=-271/506, 9-10=-271/506

WEBS 3-12=-238/571, 5-12=-132/103, 5-11=-216/268, 6-11=-299/348, 7-11=-139/223,

7-9=-651/376

Julius Lee Trues Design Engineer Florida FE No. 24889 1109 Ceastal Ray Blyd Boynton Besch, FL 23435

JOINT STRESS INDEX

2 = 0.59, 3 = 0.41, 4 = 0.37, 5 = 0.41, 6 = 0.68, 7 = 0.29, 8 = 0.41, 9 = 0.66, 10 = 0.19, 11 = 0.56, 12 = 0.34, 13 = 0.39 and 14 = 0.48

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
	T31	SPECIAL	4	1		J1925037
	1 2 2 2		201	1 55	Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:17 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are 3x6 MT20 unless otherwise indicated.

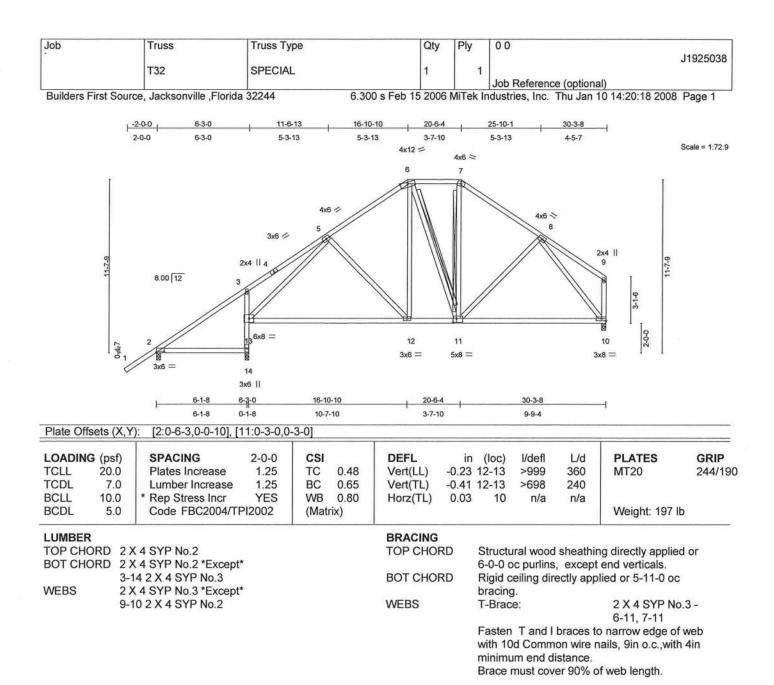
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2, 509 lb uplift at joint 14 and 220 lb uplift at joint 9.

LOAD CASE(S) Standard

dullus Les Truss Design Engineer Flonda PE No. 24869 1109 Coestel Bay Blvd Boveron Bassel Bay Blvd





REACTIONS (lb/size) 2=302/0-3-8, 14=977/0-3-0, 10=762/0-3-8

Max Horz 2=396(load case 5)

Max Uplift 2=-272(load case 4), 14=-480(load case 6), 10=-212(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-286/320, 3-4=-143/254, 4-5=-121/276, 5-6=-681/534, 6-7=-487/516,

> 7-8=-665/522, 8-9=-138/85, 9-10=-135/114 2-14=-92/0, 13-14=-941/633, 3-13=-337/374, 12-13=-342/548, 11-12=-191/498,

10-11=-279/475

WEBS 5-13=-749/405, 5-12=-96/219, 6-12=-116/193, 6-11=-136/147, 7-11=-82/158,

8-11=-84/143, 8-10=-714/466

JOINT STRESS INDEX

BOT CHORD

NT STRESS INDEX
2 = 0.56, 3 = 0.54, 4 = 0.15, 5 = 0.27, 6 = 0.67, 7 = 0.51, 8 = 0.31, 9 = 0.61, 10 = 0.70, 11 = 0.70, 12 = 0.36, 13 = 0.75 and 14
January 10,2008 Continuedon page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
	T32	SPECIAL	1	1		J1925038
	15.57		1		Job Reference (optional)	

Builders First Source, Jacksonville , Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 14:20:18 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 2, 480 lb uplift at joint 14 and 212 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34869 1100 Coestal Bay Blvd Devices Bassel Bay Blvd

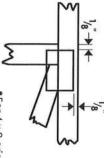


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless plates to both sides of truss and securely seat. Dimensions are in inches. Apply dimensions indicate otherwise.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates

PLATE SIZE

4 × 4

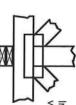
perpendicular to slots. Second to slots dimension is the length parallel The first dimension is the width

LATERAL BRACING



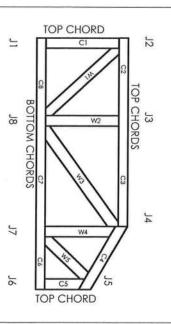
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA ICBO 3907, 4922 96-31, 96-67

9667, 94324

SBCCI

NER WISC/DILHR 960022-W, 970036-N

561





MiTek Engineering Reference Sheet: MII-7473

Failure to Follow Could Cause Property General Safety Notes

Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

Damage or Personal Injury

- 12 Cut members to bear tightly against each
- joint and embed fully. Avoid knots and wane at joint locations. Place plates on each face of truss at each
- 4. Unless otherwise noted, locate chord splices at $\frac{1}{4}$ panel length (\pm 6" from adjacent joint.)
- S Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and practice is to camber for dead load deflection is the responsibility of truss fabricator. General
- Plate type, size and location dimensions shown indicate minimum plating requirements.
- Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 12. Anchorage and / or load transferring others unless shown connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling. erection and installation of trusses
- © 1993 MiTek® Holdings, Inc.

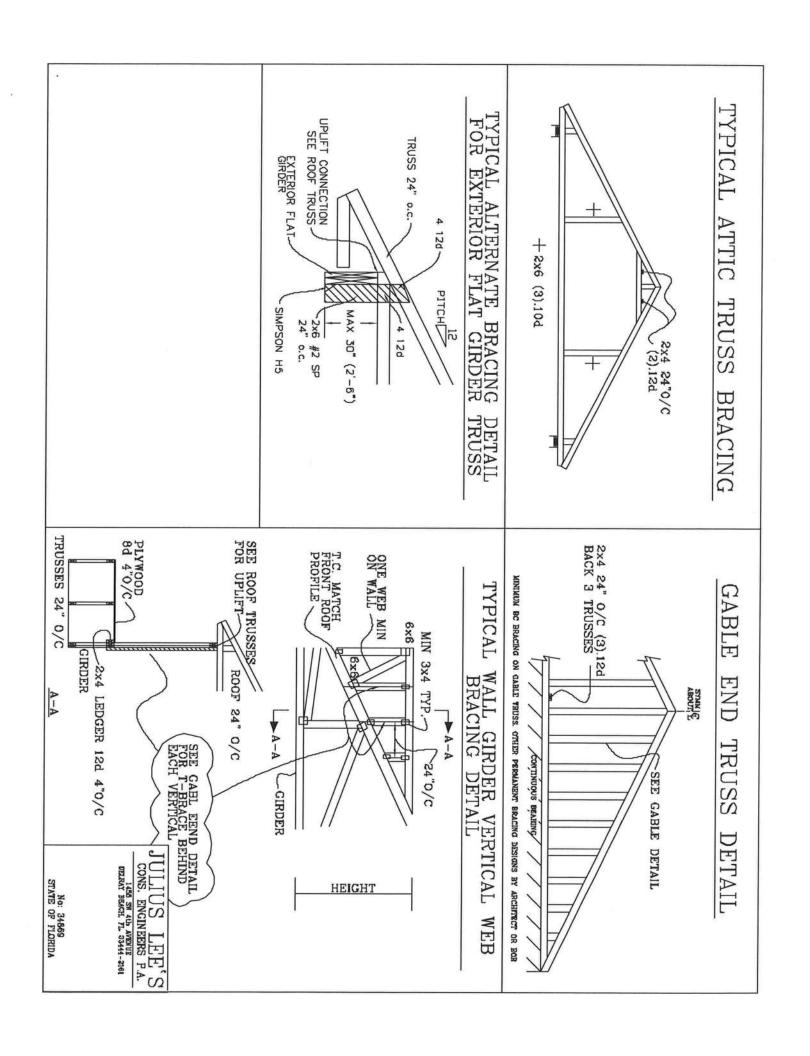
ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

** WARRONG. RETURN 1985. REQUIRE EXTREME CARE IN FABRICATING, HANDLING, Suppende, Installing and Brachne. Return 10 1619. For 1980. Billing copenient safety indomition, published by the crisis part is existived, say deviced by, suffer so, waddson, vol. 53799 and vita. Vidor insus course. Our marting, sad entering, sade entering indicated, the core that the course indicated. The core same to prefer insus the course shall now pare to and botton chose sault have a proper a attached right chose structure.	DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IN USED. CONNECT INACONAL BRACE FOR SAG! AT EACH END. MAX WEB TOTAL LENGTH IS 14'. VERTICAL LENGTH SHOWN IN TABLE ABOVE. CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.	THE SPACING SPECIES GRADE AND A GROUP B GROUP B GROUP A GROUP B GROUP B GROUP A GROUP B GROUP A GROUP B GROUP A GROUP	GABLE VERTICAL BRACE NO (1) 1X4 "L" BRACE *
ATING, MANDLING, SECRETING, INSTALLING AND CONS. ENGINEERS P.A., VI. 327/39 AND VIVA (V.COD TRUSS CONS.) REPORT PRACTICES PROBERT ATTACED STATE PROBERT ATTACED STORES OF SHALL HAVE PROBERT ATTACED STORES OF STATE OF FLORIDA NO. 34869 STATE OF FLORIDA	SYMMIC ABOUT FOR MAX GABLE VERTICAL	A CROUP B CROUP A CROUP B CROUP A CROUP B CROUP B CROUP B CROUP B CROUP A CROUP B CROU	"L" BRACE • (2) 2X4 "L" BRACE •• (1) 2X6 "L"
REF ASCEY-02-GABI3045	<u>√</u>	GROUP B 13: 3: 11: 11: 11: 11: 11: 11: 11: 11: 1	- 4

SS DESIGN FOR	REFER TO COMMON TRUSS PEAK, SPLICE, AND HEEL I
2.5X4	GREATER THAN 11' 6"
2%4	GREATER THAN 4 0 BUT
1X4 OR 2X3	LESS THAN 4' 0"
NO SPLICE	VERTICAL LENGTH
SIZES	GABLE VERTICAL PLAT

TOR	PLATES.	PEAK, SPLICE, AND HEEL
	2.5X4	GREATER THAN 11' 6"
	200	LESS THAN 11 ST
ex3	1X4 OR	LESS THAN 4' O'
a	NO SPL	VERTICAL LENGTH
u,	TE SIZE	GABLE VERTICAL PLA

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WIRN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR SEG AT EACH END. MAX WEB TOTAL LENGTH IS 14*. LENGTH MAX **GABLE** VERTICAL VERTICAL LENGTH IN TABLE ABOVE. SPACING SPECIES 24" O.C. O.C. O.C. CONNECT DIAGONAL AT GABLE VERTICAL SPF SPF DFI DFL SPF DFLSP H SP SP H H ASCE NAOHS STANDARD STANDARD STANDARD STANDARD GRADE STANDARD STANDARD \$1 / #2 STUD #2 #2 STUD STUD STUD BEEM 3 #3 BRACE 7-02: ENVARRIGER TRANSES REBURE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BACKNG. REPER TO BEST 1-43 SAULLING COMPERENT SAFETY HETDRANDON, PUBLISHED BY TPE CIRRASS PLATE INSTITUTE, 383 THORRED BY, SUITE 200, MINISON, HE SAFETY PARK, CAUDID TRUSS CLOUCH, OF ANERICA, 6300 ENTERPRISE IM, MINISON, HE SAFETY PARK-TIPES PRIBE TO PERFORMING THESE FINITIONS. UNLESS OF IMPRINE MINISON, HE SAFETY PARK. PRIPER TO PERFORMING THESE FINITIONS. UNLESS OF IMPRINED HOR CHEMOSTAND CORD SHALL HAVE A PROPERTY ATTACHED REGID CELLING. GABLE TRUSS NO BRACES **4** 0 0 130 GROUP A ZX4 SP OR DF-L #2 OR BETTER DIAGONAL BRACE; SINGLE OR DOUBLE CUT (AS SHOWN) (1) 1X4 °L" BRACE • AT UPPER END MPH GROUP B WIND (1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• (1) 2X6 "L" BRACE • (2) 2X8 "L" BRACE GROUP A SPEED, GROUP B REPER TO 30 GROUP A MEAN CHART ABOVE FOR MAX GABLE e, 10, 10, CONTINUOUS BEARING EX4 MEN OR BETTER GROUP B GROUP A GROUP B HEIGHT, 0 CONS. 18 2 2 8 1 1 1 1 DELRAY BEACH, FL. 33444-2161 No: 34869 STATE OF FLORIDA IUS LEI ENCLOSED, 12' B" VERTICAL LENGTH PAC GROUP A 14 0 0 S MAX. MAX. GROUP B II 14' 0" 14' 0" 1D' 10" 10 10 10 10 6 6 8 8 13. 3 14 0 12 11 14' O" 10. 10, 12 12 14 0 TOT. SPACING 1.00, 8 E ATTACH EACH 'L' BRACE WITH 104 NAILS. # FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C. # FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. ## FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4: 0" PROVIDE UPLATT CONNECTIONS FOR 180 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). TO BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. LIVE LOAD DEPLECTION CRITERIA IS L/240. DOUGLAS FIR-LARCH #3 STUD STANDARD SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD PLYWOOD OVERHANG. BRACING VERTICAL LENGTH LESS THAN 4' 0" GREATER THAN 4' 0", BUT LESS THAN 11' B' CABLE TRUSS EXPOSURE 60 SDUTHERN PINI GREATER THAN 11' 6" 24.0 PEAK, SPLICE, AND CABLE VERTICAL PLATE SIZES PSF 12 5 GROUP SPECIES DWG DATE REF HEM-FIR GROUP GROUP DETAIL NOTES: MITEK STD GABLE 30' E HT HEEL PLATES. TRUBS DESIGN FOR 0 DOUGLAS FIR-LARCH 11/26/03 ASCE7-02-CAB13030 Ä A: SOUTHERN PINE 13 STANDARD NO SPLICE AND 2.5X4 HEM-PIR X GRADES:



BOT CHORD 2X4 2X4 2X4 数数数 经保保 BETTER BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS 2

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H >

4XB 2X4 30

5X8

5X6 3X5 52

2.5X4

2.6X4

88 2

.5X3 5X4

.5X4 5X5

1.5X4 5X6

SPACE PIGGYBACK VERTICALS AT 4' OC MAX. REFER TO SEALED DESIGN FOR DASHED PLATES

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL-5 PSF
110 MPH WIND, 30' MEAN HGT, FBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL-5 PSF, WIND BC DL-5 PSF

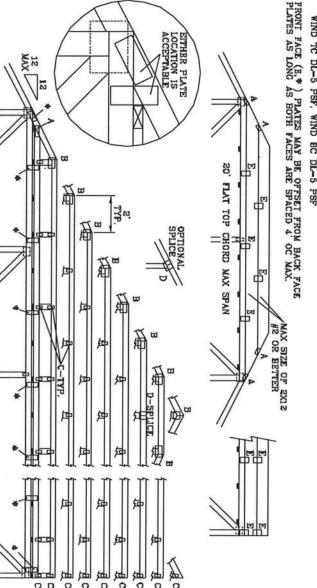
130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

Ħ U C

4XB

5X6 .5X4 5X6

OR 3X6 TRULOX AT 4' OC,



WEB LENGTH
EB LENGTH
o' To 7'9"
7'9" TO 10'
10' TO 14'

* PIGGYBACK SPECIAL PLATE

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX

INFORMATION.

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THIS DRAWING REPLACES DRAWINGS 634 018 634 017 & B47.045

*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE

		BACING. REFER TO BEST 1-TO QUILIDING COMPONENT SAFETY INFORMATION, PAIR CHAID BY THE CAUSE PART INSTITUTE, MAD DOWNER OF A PART INSTITUTE, MAD DOWNER OF A PARTICLE, SATO OFFICERS COUNCIL OF A PARTICLE, SATO OFFICERS COUNCIL THE SATE OF PARTICLES PROPERLY A FIFTCHED THE STRUCKLY PROPERLY A FIFTCHED STRUCKLY PARTICLES PROPERLY A FIFTCHED STRUCKLY PARTICLES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT CELLING.	WAY TO KEEL BUTTER LABE IN LABOLATING THAT INC. CHISSIAN I
STATE OF FLORIDA		CONS. ENGINEERS P.A. 1400 SW 4th AVENUE DELRAY BEACH, FL. 33444-2161	JULIUS LEF'S
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.	FF, S MAX LOADING REF PIGG
		DATE 09/12/07 DRWGMITEK STD PIGGY -ENG JL	REF PIGGYBACK

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.

BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.

WEBS 2X4 SP #3 OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 15' MEAN HEICHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

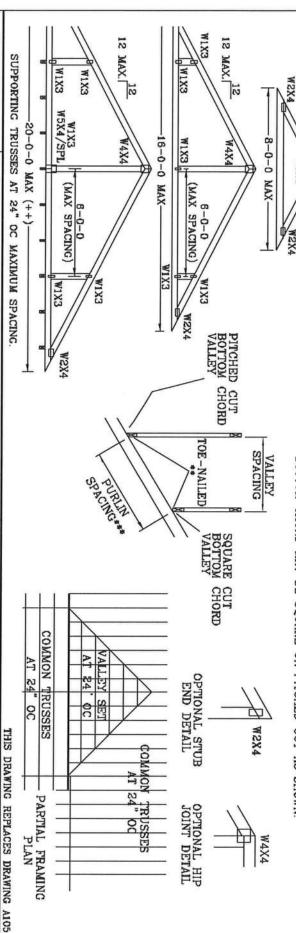
4-0-0 MAX

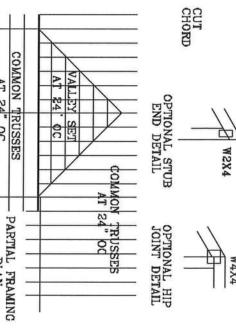
12 NAX.

W2X4

++ LARGER SPANS NAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN





STRUCTURAL P	PLATE INS	
AL PANEL	REFER TO	
DE BNY S	MILENAIS	
PANELS AND BOTTOM CHORD SHALL	IRE EXTE	
INDICATE	THE CONFO	
L HAVE A PROPERLY ATTACHED RIGID CEILING	WE IN FABRI	
BOPERLY /	ETY INFORM	
T HAVE F	PRACTIC	
SJOD CEI	PUBLISHED TEA CYCO	
LING.	NSTALLIN BY TPI D TRUSS C	
	TRUSS TRUSS	

			E) ACHCB	ERI DANDAC	ALLING AND		
STATE OF FLORIDA	No: 34869			DELRAY BEACH, I'L 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SPACING	DUR.FAC. 1.25 SPACING	DUR	TOT	BC	BC	TC	TC
		TOT. LD.	F	DI	DL	T	
-		32	0	U	~2	20	
24"	1.25	40	0	Ç	15	20	
		PSF	PSF	PSF	PSF	PSF	
			-ENG JL	DRWG	DATE	PSF REF	
			JL	VALTRUSS1103	11/26/03	VALLEY DETAIL	

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE. END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

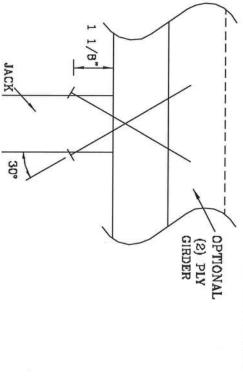
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

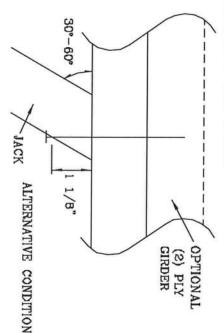
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL WALLING WAY OF THE PROPERTY OF THE PROPERT	5 48	4 39		2 18	1-4	NUMBER OF TOE-NAILS 1	
	493#	394#	298#	187#	PLY	SOUTHERN PINE	
	639#	511#	383#	256#	2 PLIES	N PINE	
	452#	361#	271#	181#	1 PLY	DOUGLAS	
	585#	468#	351#	234#	2 PLIES	FIR-LARCH	DOUGLAS FIR-LARCH
	390#	312#	234#	156#	1 PLY		
	507#	406#	304#	203#	2 PLIES	HEM-FIR	
	384#	307#	230#	154#	1 PLY	SPRUCE	
"	496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR	

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





THIS DRAWING REPLACES DRAWING 784040

	HAVARONG: HERESES REDURE EXTREME CARE IN FARRICATING, HANDLING, SUPPING, INSTALLING AND BRACING. RETER TO BOSI 1-03 COULDING COMPONENT SAFETY INFRANTIDOS, PILLISHED BY TP! CRUSSS PLATE INSTITUEL, 983 PROCREID SA, SUTIE 200, ANXISSIN, UI. 50719) AND YOLK AVOID THOUSE COLACING THOUSE COLACTIONS. UNICES OTHERWISE LIN, MARISSIN, UT 50719) FOR SAFETY PRACTICES PROPERTY ATTACHED REPERRY ATTACHED REGID CEILING. STRUCTURAL PAMELS AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED REGID CEILING.					
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG	DATE	REF
			JL	CNTONAIL1103	09/12/07	TOE-NAIL

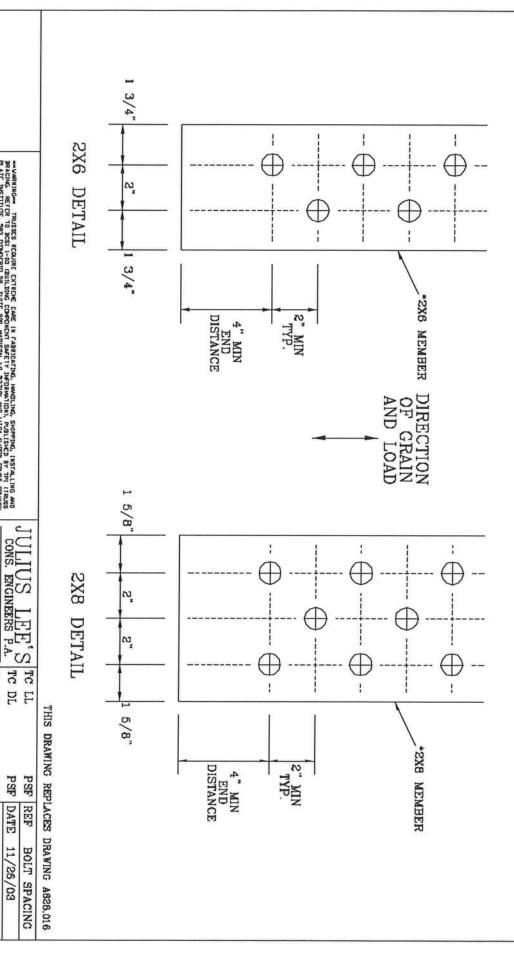
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW. APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPP BRACING. REFER TO BOSI 1-00 GIUILDING COPPORIEM SAFETY INFORMATION, PUBL PLATE INSTITUTE, 500 D'OND-FRED DE, SUITE 200, ANDISON, V. 1,33719 AND LYCO FEARCICA, 6300 ENTERPAISE LM, MADISON, VEI 33719 FOR SAFETY PRACTICES PINTERS FINCTIONS. UNLESS DITERVISE NOIGNEED, TOP D-ORD SHALL HAVE PROPISE LM, MADISON, VEI 33719 FOR SAFETY ATTACAS AND STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PREPEREY ATTACAS AND STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PREPEREY ATTACAS AND STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PREPEREY ATTACAS AND STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PREPEREY ATTACAS AND STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PREPEREY ATTACAS AND SAFETY AND STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PREPEREY ATTACAS AND SAFETY
CONS.

DELRAY BEACH, FL 33444-2161

BC DL TC DL

DRWG DATE

CNBOLTSP1103 11/26/03

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No: 34869 STATE OF FLORIDA

SPACING

DUR. FAC

TRULOX CONNECTION

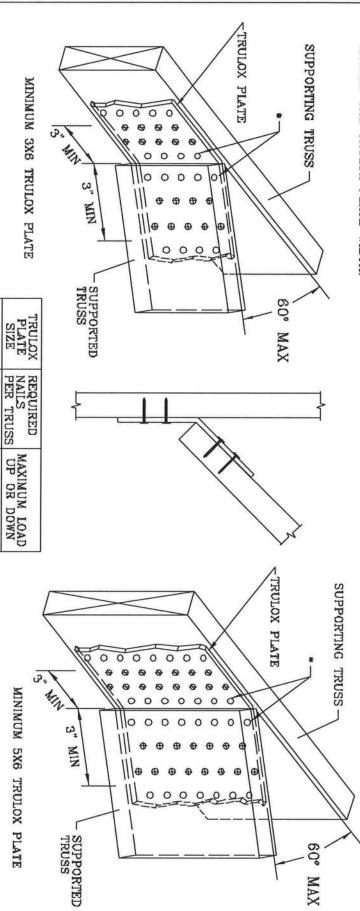
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

NAILS MAY BE OMITTED FROM THESE ROWS THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN. THIS DETAIL FOR LUMBER, PLATES, AND OTHER

MAX



THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

MINIMUM 5X6 TRULOX PLATE

WARRING* TRUSSES REQUIRE EXTROME CAME IN FABRICATING, HANDLING, SHIPPING, BRACING REFER TO BES) 1-03 (BUILING COMPONENT SAFETY INFORMATION, PUBLISSED PLATE INSTITUT, 383 D'OURTEID B., SUITE 200, ANDISON, VI 33799 AND VICA VOD) OF AMERICA, 6300 CHICRAPRISE LM, MAISON, VI 33799 FOR SAFETY PAGAPTICES, SHOW DIFFERNISE LM, MAISON, VI 33799 FOR SAFETY PAGAPTICES, SHOW DIFFERNISE LM, MAISON, VI 33799 FOR SAFETY PAGAPTICES, SHOW DIFFERNISE LM, MAISON, VI 33799 FOR SAFETY PAGAPTICES, SHOW DIFFERNISE LM, MAISON, VI 33799 FOR SAFETY PAGAPTICES, SHOW DIFFERNISE LM, MAISON, VI 33799 FOR SAFETY PAGAPTICES, SHOW DIFFERNISE LM, MAISON, VI 33799 FOR SAFETY PAGAPTICES, PORTONION OF THE SAFETY PAGAPTICES AND SAFETY PAGAPTICES.

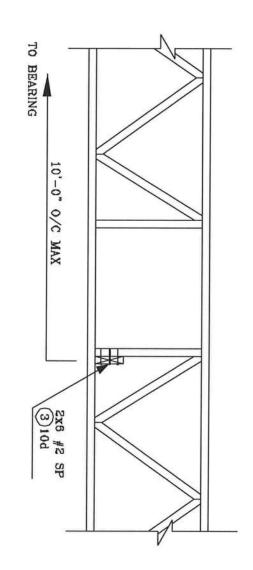
5X6 3X6

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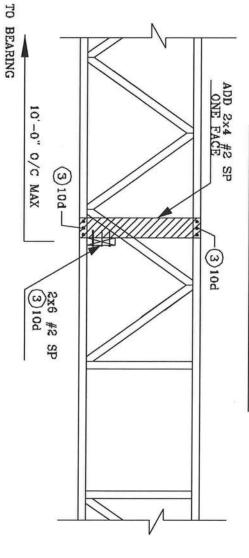
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	NSTALLING AND DEPT FIT (TRUSS DOD FRUSS COUNCIL TO SERTIFICATION OF ATTACHED SELLING.				
No: 34869 STATE OF FLORIDA		DELRAY BEACH, IL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
	-ENG JL	DRWG	DATE	REF	
	JL.	CNTRULOX1103	11/26/03	TRULOX	

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S P.A.

CONS. ENGINEERS P.A.

1455 571 4th AVENUE

DEJRAY BEACH, JL. 33441-2161

No: 34869 STATE OF FLORIDA

