

BOT CHORD

WEBS

BOT CHORD

4-18: 2x6 SP No.2, 19-28,21-22,24-25: 2x4 SP No.3

2x4 SP No.3 *Except*

WEBS

11-28: 2x6 SP No.2

SLIDER Left 2x8 SP 2400F 2.0E 1-11-8

REACTIONS. (lb/size) 18=1406/0-3-0, 28=1004/Mechanical

Max Horz 18=-241(LC 10)

Max Uplift 18=-262(LC 12), 28=-191(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

2-4=-250/323, 4-6=-352/552, 6-7=-916/520, 7-8=-840/543, 8-9=-1081/609, TOP CHORD

9-10=2117/1049, 10-11=1633/709, 12-28=-1004/462, 11-12=-976/455

BOT CHORD 2-18=-211/303, 17-18=-1368/771, 4-17=-341/232, 16-17=-239/687, 14-16=-146/705,

13-14=-407/1049

7-14=142/250, 8-14=-148/434, 9-14=-511/358, 9-13=-468/937, 10-13=-1041/598,

11-13=-603/1503, 6-17=-1440/794

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 18=262, 28=191
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

5-10-0 oc bracing: 17-18

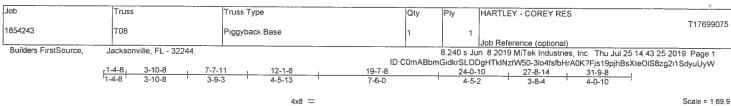
1 Row at midpt

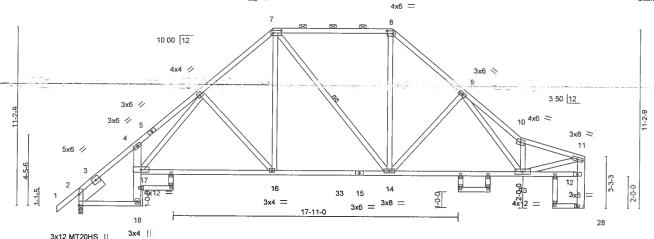
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date

July 25,2019

♠ WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters.shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Compositions available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314







Di-1-07-1-0/10	3-10-8 5-11-0 3-10-8 2-0-8	12-1-8 6-2-8	19-7-8 7-6-0		4 29-9-8 31-9-8 4 2-0-10 2-0-0
Plate Offsets (X,Y)— [LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	7:0-6-4,0-2-0]. [8:0-4-4,0-2-0] SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.92 BC 0.76 WB 0.68 Matrix-MS		in (loc) I/defl L/d 33 16-17 >999 240 52 16-17 >735 180 54 28 n/a n/a	PLATES GRIP MT20 244/190 MT20HS 187/143 Weight: 230 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

4-18; 2x6 SP M 26, 19-20,22-23,26-28; 2x4 SP No.3

WEBS 2x4 SP No.3 *Except*

11-28: 2x6 SP No.2

SLIDER Left 2x8 SP 2400F 2.0E 1-11-8

REACTIONS. (lb/size) 2=1244/0-3-0, 28=1166/Mechanical

Max Horz 2=-241(LC 10)

Max Uplift 2=-232(LC 12), 28=-212(LC 13)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1306/641, 4-6=-2735/1408, 6-7=-1378/757, 7-8=-1017/656, 8-9=-1364/754,

9-10=-2490/1241, 10-11=-1924/859, 12-28=-1166/545, 11-12=-1136/536

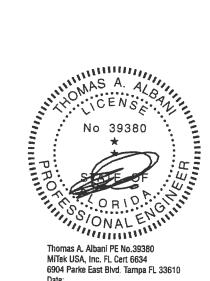
BOT CHORD 2-18=-452/989, 4-17=-876/517, 16-17=-563/1390, 14-16=-290/1026, 13-14=-527/1283 WEBS

6-16=-601/401, 7-16=-245/667, 8-14=-241/580, 9-14=-532/372, 9-13=-515/1029,

10-13=-1194/677, 11-13=-747/1782, 6-17=-667/1339

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat, II; Exp C; Encl. GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (ft=lb) 2=232, 28=212,
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 3-5-3 oc purlins,

except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 7-8.

Rigid ceiling directly applied or 7-9-13 oc bracing. Except

8-11-0 oc bracing: 17-18

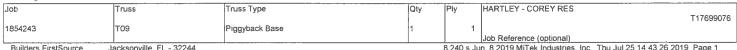
1 Row at midpt

6904 Parke East Blvd. Tampa FL 33610

July 25,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE





Structural wood sheathing directly applied or 3-6-12 oc purlins,

7-16, 8-16, 8-14

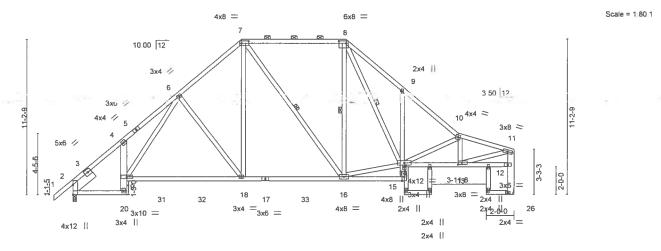
except end verticals, and 2-0-0 oc purlins (3-1-0 max.): 7-8.

Rigid ceiling directly applied or 8-3-12 oc bracing. Except:

10-0-0 oc bracing: 14-15

1 Row at midpt





		12-1-8 8-3-0	19-7-8 7-6-0	23-10-0 4-2-8	25-10-0 27-8-14 2-0-0 1-10-14	31-9-8 4-0-10
Plate Offsets (X,Y)	[2:Edge,0-0-0], [7:0-6-4,0-2-0], [8:0-6-4,0)-2-0], [13:0-3-8,0-1-8], [1	4:0-1-8,0-1-8]			
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.88 BC 0.97 WB 0.67 Matrix-MS		in (loc) -0.19 18-19 -0.41 18-19 0.17 26	l/defl L/d >999 240 >919 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 252 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* BOT CHORD

4-20; 2x6 SP No.2, 9-15,21-22,24-26; 2x4 SP No.3 2x4 SP No.3 *Except*

WEBS 11-26: 2x6 SP No.2

SLIDER Left 2x8 SP 2400F 2.0E 1-11-8

REACTIONS. (lb/size) 2=1244/0-3-0, 26=1166/Mechanical

Max Horz 2=-241(LC 10)

Max Uplift 2=-232(LC 12), 26=-212(LC 13)

(lb) - Max, Comp./Max. Ten. - All forces 250 (lb) or less except when shown FORCES.

TOP CHORD 2-4=-1376/636, 4-6=-1908/986, 6-7=-1258/721, 7-8=-938/628, 8-9=-1698/1030,

9-10=-1705/824, 10-11=-1906/878, 12-26=-1166/545, 11-12=-1117/545

BOT CHORD 2-20=-463/1074, 4-19=-394/296, 18-19=-461/1217, 16-18=-260/977, 9-14=-362/300,

13-14=-825/1827

6-18=-472/339, 7-18=-223/667, 14-16=-232/849, 8-14=-556/936, 10-14=-632/366,

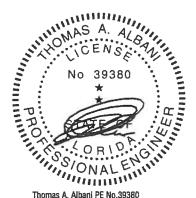
10-13=-412/250, 11-13=-779/1760, 6-19=-321/612

NOTES-

WEBS

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10,0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Refer to girder(s) for truss to truss connections
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=232, 26=212,
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord



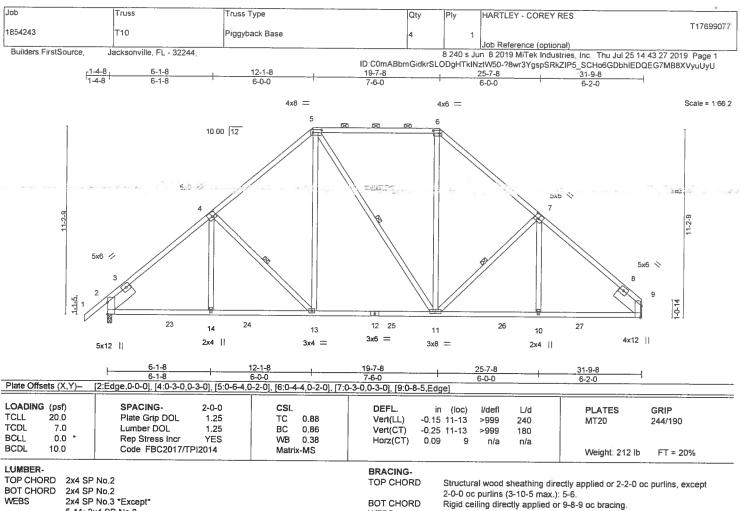
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

July 25,2019

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ANSI/THY Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314





WEBS

1 Row at midpt

4-13, 5-11, 7-11

2x4 SP No.3 *Except*

5-11: 2x4 SP No.2

SUDER Left 2x8 SP 2400F 2.0E 1-11-8, Right 2x8 SP 2400F 2.0E 1-11-8

REACTIONS. (lb/size) 2=1252/0-3-0, 9=1175/Mechanical

Max Horz 2=273(LC 11) Max Uplift 2=-236(LC 12), 9=-211(LC 13) Max Grav 2=1305(LC 19), 9=1223(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4-1486/654, 4-5-1225/677, 5-6-918/619, 6-7-1210/679, 7-9-1484/659

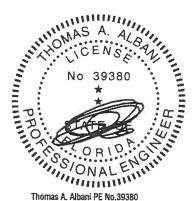
BOT CHORD 2-14=-364/1213, 13-14=-364/1214, 11-13=-173/947, 10-11=-370/1068, 9-10=-370/1066

WEBS 4-13=-422/296, 5-13=-163/525, 6-11=-166/460, 7-11=-429/304

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=236, 9=211
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd Tampa FL 33610 Date:

July 25,2019

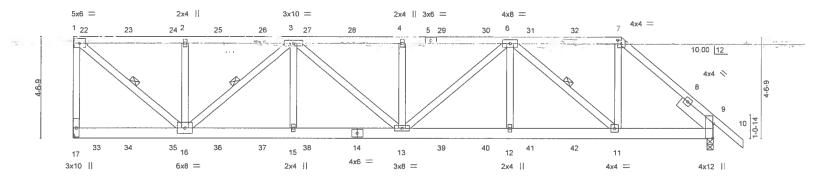
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	Job		Truss		Truss Type			Qty	Ply	HARTLEY - COREY RES		
Ì									-			T17699078
	1854243		T11		Roof Special Girder		l	1	1			
	L									Job Reference (optional)		
	Builders	FirstSource, Ja	acksonville, FL -	- 32244,					8,240 s Ju	in 8 2019 MiTek Industries, Ind	Thu Jul 25 14 43 29	2019 Page 1
							ID C0mAB	bmGidkr!	SLODgHTI	kINztW50-yX2bUEi6L4hRpcZL	J5tEltXMaPU?YhCgZa	ggFbOyuUyS
	1	5-0-0		9-10-5	1	14-8-9		19-6-14		24-6-14	28-8-14	30-0-14
	,	5-0-0	,	4-10-4	1	4-10-4	1	4-10-4		5-0-0	4-2-0	1-4-0

Scale = 1 49 8



	0-0	9-10-5 4-10-4	14-8-9 4-10-4	19-6-14 4-10-4	24-6-14 5-0-0	28-8-14 4-2-0
Plate Offsets (X,Y)	[7:0-2-0,0-1-13], [9:0-8	-1,0-0-6]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLŁ 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017	1.25 NO	CSI. TC 0.80 BC 0.81 WB 0.90 Matrix-MS		c) I/defl L/d 13 >999 240 13 >999 180 9 n/a n/a	PLATES GRIP MT20 244/190 Weight: 197 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No,2

BOT CHORD 2x6 SP No.2

WEBS 2x4 SP No.3

SLIDER Right 2x6 SP No.2 1-11-8

REACTIONS. (lb/size) 17=1885/Mechanical, 9=1722/0-3-8

Max Horz 17=-149(LC 24)

Max Uplift 17=1328(LC 4), 9=981(LC 4) Max Grav 17=1891(LC 31), 9=1722(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-17=-1768/1300, 1-2=-1817/1267, 2-3=-1817/1267, 3-4=-3143/2182, 4-6=-3143/2182,

6-7=-1498/1003, 7-9=-1998/1267

BOT CHORD 15-16=-1939/2905, 13-15=-1939/2905, 12-13=-1823/2789, 11-12=-1823/2789,

9-11=-877/1473 WEBS 1-16=-1643/236

1-16=-1643/2361, 2-16=-468/458, 3-16=-1360/948, 3-15=-32/332, 3-13=-258/388,

4-13=-419/410, 6-13=-388/528, 6-12=-66/393, 6-11=-1685/1224, 7-11=-683/1080

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp.C; Encl., GCpi=0.18, MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 6) Refer to girder(s) for truss to truss connections
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 17=1328, 9=981.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 120 lb down and 153 lb up at 0-6-2, 126 lb down and 156 lb up at 2-6-2, 126 lb down and 156 lb up at 4-6-2, 126 lb down and 156 lb up at 156-2, 126 lb down and 156 lb up at 156-2, 126 lb down and 156 lb up at 156
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



Structural wood sheathing directly applied or 3-0-1 oc purlins,

1-16, 3-16, 6-11

Rigid ceiling directly applied or 5-4-0 oc bracing

except end verticals.

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

July 25,2019

COMPLEASE (SheStandard



Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES	
1			1	1		T17699078
1854243	T11	Roof Special Girder	4	1		11/0550/0
		Troof opecial Gilder	'	,	Job Reference (optional)	
			1		Job Reference (optional)	

Builders FirstSource,

Jacksonville, FL - 32244

8 240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 29 2019 Page 2 ID:C0mABbmGidkrSLODgHTklNztW50-yX2bUEi6L4hRpcZU5tEltXMaPU?YhCgZaggFbOyuUyS

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-7=-54, 7-10=-54, 17-18=-20

Concentrated Loads (lb)

Vert: 14=-39(B) 4=-71(B) 13=-39(B) 11=-76(B) 22=-88(B) 23=-71(B) 24=-71(B) 25=-71(B) 26=-71(B) 27=-71(B) 28=-71(B) 29=-71(B) 30=-71(B) 31=-71(B)

32=-71(B) 33=-45(B) 34=-39(B) 35=-39(B) 36=-39(B) 37=-39(B) 38=-39(B) 39=-39(B) 40=-39(B) 41=-39(B) 42=-39(B)

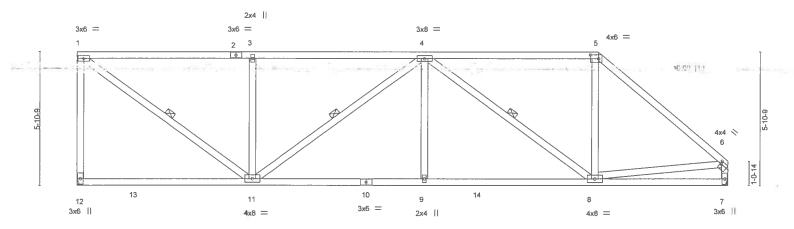
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES	4
					1	T17699079
1854243	T12	Roof Special]1	1		
					Job Reference (optional)	
Builders First	Source, Jacksonville, FL - 32244	i,		8 240 s Ju	in 8 2019 MiTek Industries	s, Inc. Thu Jul 25 14 43 29 2019 Page 1
			ID_C0mABbmGi	dkrSLODgH	TkINztW50-yX2bUEi6L4hI	RpcZU5tEltXMZaU15hleZaggFbOyuUyS
L	7-8-8	15-3-3	1	22-1	1-11	28-8-14
	7-8-8	7-6-12	ı	7-8	3-8	5-9-3

Scale = 1 48 9



		7-8-8	l l	15-3-3		22-11	1-11		, 28-8-14	
		7-8-8	,	7-6-12	1	7-8	-8		5-9-3	
Plate Off	sets (X,Y)-	[5:0-4-4,0-2-0], [6:0-0-12	,0-1-8]							
LOADING	G (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.85	Vert(LL)	-0.09 11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.65	Vert(CT)	-0.20 11-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	YEŞ	WB 0.52	Horz(CT)	0.04 7	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix-MS					Weight: 169 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 12=1053/Mechanical, 7=1053/Mechanical

Max Horz 12=-160(LC 13)

Max Uplift 12=-284(LC 8), 7=-213(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten, - All forces 250 (lb) or less except when shown,

TOP CHORD 1-12=-985/543, 1-3=-1115/564, 3-4=-1115/564, 4-5=-907/521, 5-6=-1254/548,

6-7=-1001/472

BOT CHORD 9-11=-516/1401, 8-9=-516/1401

WEBS 1-11=-687/1358, 3-11=-435/336, 4-11=-403/210, 4-9=0/325, 4-8=-657/264, 5-8=-89/461,

6-8=-186/757

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=284, 7=213.



Structural wood sheathing directly applied or 4-3-10 oc purlins,

1-11, 4-11, 4-8

Rigid ceiling directly applied or 8-2-6 oc bracing.

except end verticals.

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

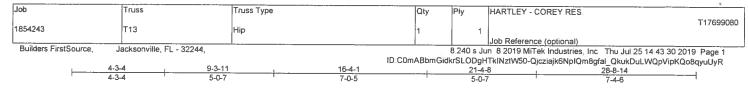
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and his for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property interiparate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

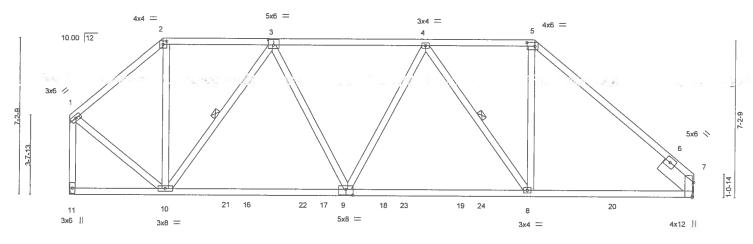
AMSUTPH Quality Criteria, DSB-89 and BCSI Building Componer Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314





Scale = 1:51.2



		107	12-3			21-4-0				26-6-14	
	<u> </u>	4-3-4	8-6-	-10		8-6-10				7-4-6	
Plate Of	fsets (X,Y)-	[2:0-2-0,0-1-13], [3:0-3	-0,0-3-0], [5:0-4-4	1,0-2-0], [7:0-8-5,Edge], [9:	:0-4-0,0-3-0]						***************************************
LOADIN TCLL TCDL BCLL	20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	1.25 YES	CSI. TC 0.86 BC 0.76 WB 0.29	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.16 -0.28 0.08	(loc) 8-9 8-9 7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code FBC2017	/TPI2014	Matrix-MS						Weight 176 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

24 4 9

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Right 2x8 SP 2400F 2.0E 1-11-8

4-3-4

REACTIONS. (lb/size) 7=1058/Mechanical, 11=1058/Mechanical

Max Horz 11=-151(LC 8)

Max Uplift 7=-183(LC 8), 11=-190(LC 9) Max Grav 7=1101(LC 2), 11=1058(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=822/409, 2-3=592/388, 3-4=1152/619, 4-5=932/558, 5-7=1309/584,

1-11=-1046/502 BOT CHORD 9-10=-320/1009, 8-9=-383/1144, 7-8=-271/920

WEBS 2-10=91/345, 3-10=731/372, 3-9=34/337, 4-8=462/211, 5-8=144/600,

1-10=-271/743

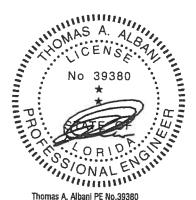
NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60

12-9-14

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=183, 11=190.



Structural wood sheathing directly applied, except end verticals.

3-10:4-8

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midnt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

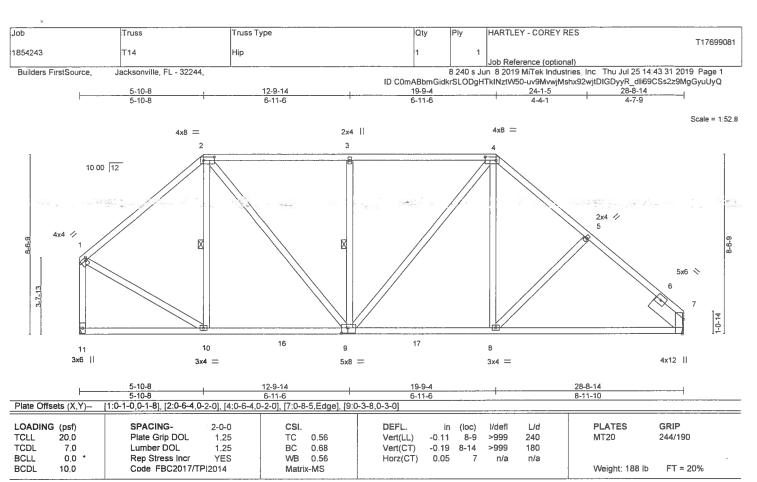
July 25,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord membe, so my. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314.



6904 Parke East Blvd Tampa, FL 36610



BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Right 2x8 SP 2400F 2.0E 1-11-8

REACTIONS. (lb/size) 7=1058/Mechanical, 11:

(lb/size) 7=1058/Mechanical, 11=1058/Mechanical

Max Horz 11=-184(LC 8)

Max Uplift 7=-175(LC 13), 11=-156(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=897/452, 2-3=-943/610, 3-4=-947/613, 4-5=-1104/610, 5-7=-1248/606,

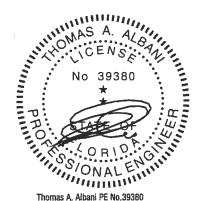
1-11=-1007/502

BOT CHORD 9-10=-180/648, 8-9=-199/812, 7-8=-348/885

WEBS 2-9=-259/559, 3-9=-433/316, 4-9=-161/287, 4-8=-94/341, 5-8=-250/211, 1-10=-229/694

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=175, 11=156.



Structural wood sheathing directly applied or 4-2-2 oc purlins,

2-10, 3-9

Rigid ceiling directly applied or 9-10-12 oc bracing.

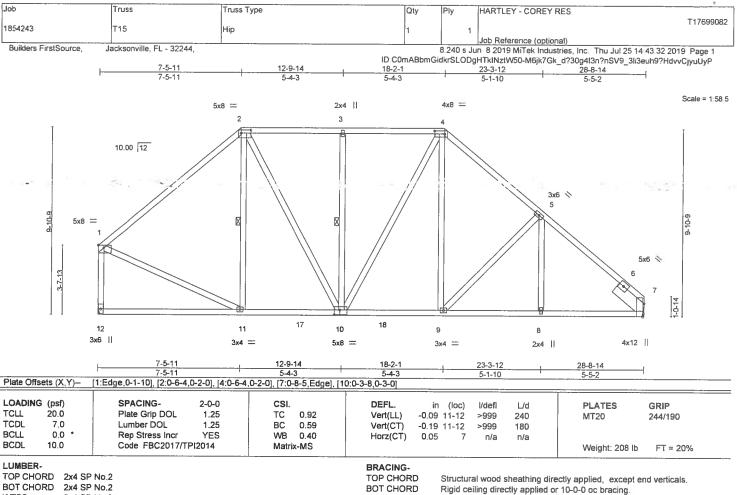
except end verticals.

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019





WEBS

1 Row at midot

2-11 3-10

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 SLIDER Right 2x8 SP 2400F 2.0E 1-11-8

REACTIONS. (lb/size) 7=1058/Mechanical, 12=1058/Mechanical

Max Horz 12=-217(LC 8)

Max Uplift 7=-183(LC 13), 12=-167(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

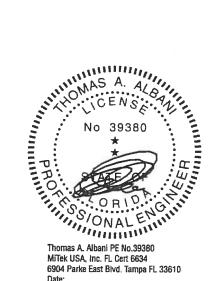
TOP CHORD 1-2=-946/472, 2-3=-793/567, 3-4=-796/569, 4-5=-1063/609, 5-7=-1256/583,

1-12=-988/500

BOT CHORD 10-11=-172/662, 9-10=-142/751, 8-9=-324/891, 7-8=-324/891

WEBS 2-10=-179/412, 3-10=-322/208, 4-9=-152/378, 5-9=-364/260, 1-11=-173/638

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=183, 12=167.



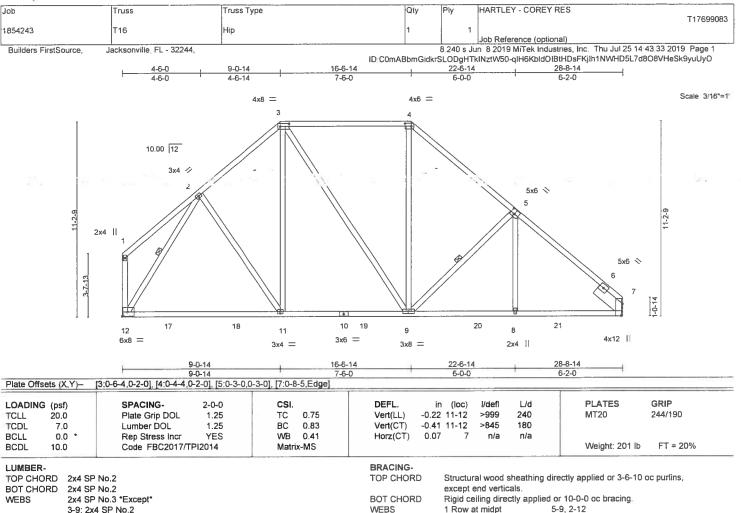
6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 10/03/2015 BEFORE USE. Design valid for use only with MiTel® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design from the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Adultional temporary and pennanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Composition available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





3-9: 2x4 SP No.2

SLIDER Right 2x8 SP 2400F 2.0E 1-11-8

REACTIONS.

(lb/size) 12=1058/Mechanical, 7=1058/Mechanical

Max Horz 12=251(LC 8)

Max Uplift 12=-175(LC 12), 7=-189(LC 13) Max Grav 12=1126(LC 2), 7=1110(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3-942/559, 3-4-789/551, 4-5-1032/592, 5-7-1322/576 11-12=-208/697, 9-11=-149/737, 8-9=-309/948, 7-8=-309/946

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

WEBS

NOTES-

3-11=-71/304, 4-9=-107/341, 5-9=-448/311, 2-12=-1019/435

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber

DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=|b|) 12=175, 7=189.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd, Tampa FL 33610

July 25,2019

🛦 WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property amage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information
available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES	
1854243	T17	Half Hip Girder	1	1		T17699084
					Job Reference (optional)	
Builders FirstSource	e, Jacksonville, FL - 3	2244,		B 240 s Ju	n 8 2019 MiTek Industries, Inc. Thu Ju	Il 25 14 43 34 2019 Page 1
			ID C0mABbmGidkrS	LODgHTk	INztW50-IUrUYxmF9cJkvNRSuQqwaa3	3YKVrvMcelkxO0HbyuUvN
⊢	-1-4-0	2-8-2	6-2-5		9-9-8	,
'	1-4-0 '	2-8-2	3-6-3		3-7-3	

Scale = 1.20.7

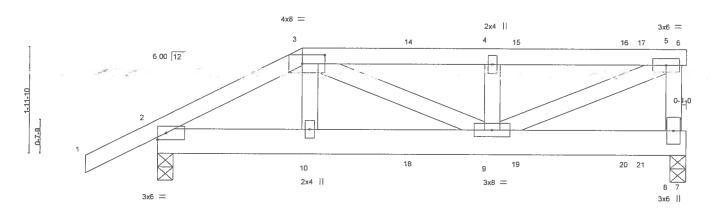


Plate Offse	ets (X,Y)— [3:0-5-0,0-2-0]	2-8-2 2-8-2		6-2-5 3-6-3		-		9-9-8 3-7-3	
LOADING TCLL TCDL BCLL	(psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 NO	CSI. TC 0.34 BC 0.22 WB 0.34	DEFL. Vert(LL) Vert(CT) Horz(CT)	0.03 9	loc) l/defl l-10 >999 l-10 >999 8 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code FBC2017/TI	PI2014	Matrix-MS					Weight: 56 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 8=763/0-3-8, 2=597/0-3-8

Max Horz 2=64(LC 27) Max Uplift 8=-608(LC 5), 2=-357(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-806/575, 3-4=-895/689, 4-5=-895/689, 5-8=-613/484

BOT CHORD 2-10=-522/689, 9-10=-532/698 WEBS 4-9=-347/252, 5-9=-685/896

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat, II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=608, 2=357.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 72 lb down and 112 lb up at 4-8-14, 72 lb down and 112 lb up at 6-8-14, and 76 lb down and 111 lb up at 8-8-14, and 84 lb down and 110 lb up at 9-0-12 on top chord, and 97 lb down and 115 lb up at 2-8-2, 56 lb down and 75 lb up at 4-8-14, 56 lb down and 75 lb up at 6-8-14, and 56 lb down and 75 lb up at 8-8-14, and 62 lb down and 75 lb up at 9-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-3=54, 3-5=54, 5-6=-14, 7-11=-20

Concentrated Loads (lb)

Vert: 10=-74(F) 14=-72(F) 15=-72(F) 16=-76(F) 17=-84(F) 18=-48(F) 19=-48(F) 20=-48(F) 21=-50(F)



Structural wood sheathing directly applied or 5-11-15 oc purlins,

Rigid ceiling directly applied or 9-11-14 oc bracing.

except end verticals.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

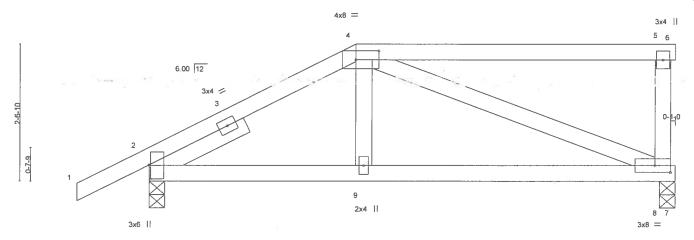
July 25,2019

warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent oracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty Ply HARTLEY - COREY RES
			T17699085
1854243	T18	Half Hip	1 1
		_	Job Reference (optional)
Builders FirstSource	Jacksonville, FL - 32244,		8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 35 2019 Page 1
			ID C0mABbmGidkrSL0DgHTklNztW50-mhPslHntwwRbXX0eS8L97ochnvAa54YRzb7Zp1yuUyM
	-1-4-0	3-10-2	9-9-8
	1-4-0	3-10-2	5-11-6

Scale = 1:20.7



			3-10- 3-10-			+			9-9 5-11			
Plate Offse	ets (X,Y)-	[2:0-3-2,0-0-5], [4:0-5-0,0-	-2-0], [8:0-3-8,	0-1-8]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.49	Vert(LL)	0.06	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.26	Vert(CT)	-0.05	8-9	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.23	Horz(CT)	-0.01	8	n/a	n/a		
BCDL	10.0	Code FBC2017/TF	PI2014	Matrix	c-MS						Weight: 49 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 **WEBS** 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=431/0-3-8, 8=357/0-3-8

Max Horz 2=84(LC 12)

Max Uplift 2=-181(LC 9), 8=-205(LC 9)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-4=-402/684

BOT CHORD 2-9=-656/386, 8-9=-675/392 **WEBS** 4-9=-298/202, 4-8=-341/611

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=181, 8=205.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-9-10 oc bracing.

except end verticals.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

July 25,2019

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Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES	
1854243	T19	Half Hip	1	1		T17699086
					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s JL	in 8 2019 MiTek Industries, Inc.	Thu Jul 25 14 43 35 2019 Page 1
			ID:C0mABbmGidl	rSLODgHTk	dNztW50-mhPslHntwwRbXX0eS	8L97ockcvB455FRzb7Zp1yuUyM
	-1-4-0	5-0-2			9-9-8	
	1-4-0	5-0-2			4-9-6	

Scale = 1.22.8

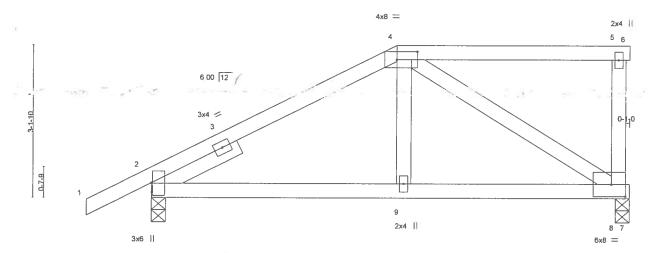


Plate Offsets (X,Y)-	[2:0-3-2,0-0-5], [4:0-5-0,0-2-0], [8:0-3-8,	5-0-2 D-3-0]	4-9-6	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCOL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.30 BC 0.23 WB 0.19 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) 0.05 9-12 >999 240 Vert(CT) 0.04 9-12 >999 180 Horz(CT) -0.01 2 n/a n/a	PLATES GRIP MT20 244/190 Weight: 50 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

9-9-8

except end verticals

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 7-6-4 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 1-11-8

Sales Con Extrem 170.50 Feb.

REACTIONS. (lb/size) 2=431/0-3-8, 8=357/0-3-8

Max Horz 2=103(LC 12)

Max Uplift 2=-172(LC 9), 8=-204(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-363/551

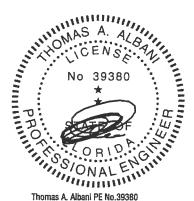
BOT CHORD 2-9=-559/324, 8-9=-576/330 WEBS 4-9=-323/205, 4-8=-357/637

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

5-0-2

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=172, 8=204.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

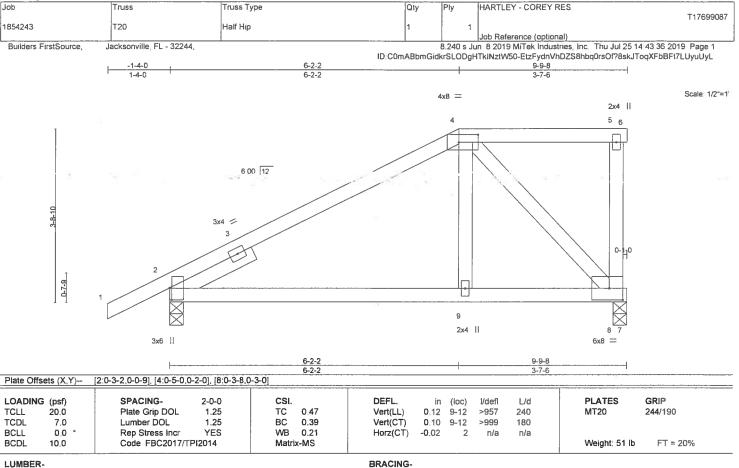
July 25,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a triss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated its to prevent bucking of individual triss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see.

ANSITP! Quality Criteria, DSB-89 and BCSI Building Component Safety Information.





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

Left 2x4 SP No.3 1-11-8 SLIDER

REACTIONS. (lb/size) 2=431/0-3-8, 8=357/0-3-8

Max Horz 2=123(LC 12)

Max Uplift 2=-162(LC 9), 8=-202(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown, TOP CHORD 2-4=-312/429

BOT CHORD 2-9=-471/268, 8-9=-487/274

4-9=-384/231, 4-8=-384/692 WEBS

NOTES-

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=162, 8=202,



Structural wood sheathing directly applied or 6-0-0 oc purlins,

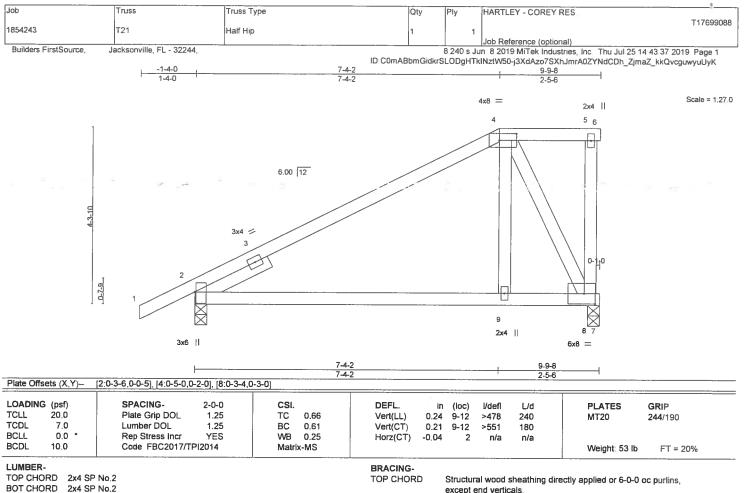
Rigid ceiling directly applied or 7-6-15 oc bracing

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

July 25,2019

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BOT CHORD

Rigid ceiling directly applied or 7-4-7 oc bracing.

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=431/0-3-8, 8=357/0-3-8

Max Horz 2=143(LC 12)

Max Uplift 2=-154(LC 9), 8=-200(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-360/351

BOT CHORD 2-9=-385/214, 8-9=-404/221

4-9=518/297, 4-8=-467/856 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl. GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone, porch left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=154, 8=200



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314



Qty Ply HARTLEY - COREY RES Job Truss Truss Type T17699089 1854243 T22 Half Hip Job Reference (optional) 8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 37 2019 Page 1
ID C0mABbmGidkrSLODgHTkINztW50-j3XdAzo7SXhJmrA0ZYNdCDh4gjsVZuHkQvcguwyuUyK Builders FirstSource Jacksonville, FL - 32244 8-6-2 9-9-8 4-4-13 1-4-0 4-4-13 Scale = 1.30.5 5x6 = 2x4 || 6 7 5 6.00 12 2x4 1 -10-1 0-1-0 10 3x4 = \parallel 6x8 3x6 5-4-11 [2:0-3-2,0-0-5], [5:0-3-0,0-2-0], [9:0-3-8,0-3-0] Plate Offsets (X,Y)-LOADING (psf) GRIP SPACING-2-0-0 CSI. DEFL I/defi L/d **PLATES** (loc) TCLL 20.0 Plate Grip DOL 1.25 TC 0.27 Vert(LL) 0.05 9-10 >999 240 MT20 244/190 7.0 Lumber DOL 1.25 вС 0.23 Vert(CT) -0.04 >999 180 TCDL 9-10 WB 0.60 Rep Stress Incr -0.00 **BCLL** 0.0 YES Horz(CT) 9 n/a n/a Code FBC2017/TPI2014 Weight: 61 lb FT = 20% Matrix-MS BCDL 10.0 BRACING-LUMBER-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, **BOT CHORD** 2x4 SP No.2 except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-11-4 oc bracing

2x4 SP No.3 WEBS

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 9=357/0-3-8, 2=431/0-3-8

Max Horz 2=163(LC 12)

Max Uplift 9=-197(LC 9), 2=-145(LC 9)

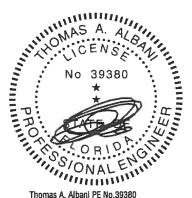
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-4=-373/550, 4-5=-465/706

BOT CHORD 2-10=-683/367

4-10=-225/314, 5-10=-840/439, 5-9=-307/480 WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone, porch left and right exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=197, 2=145,



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

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ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type HARTLEY - COREY RES Qty Ply T17699090 1854243 T23 Monopitch 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 38 2019 Page 1 ID: C0mABbmGidkrSLODgHTklNztW50-BF4?NJplDrpAO?ID7GuskQD6S64nlRttfZMDQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyUyJBPAO?ID7GuskQD6S64nlRttfQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyUyJBPAO?ID7GuskQMyuUyUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyJBPAO?ID7GuskQMyuUyUyUyJBPAO?ID7GuskQ4-10-12 Scale = 1 32 B 2x4 || 5 6 6.00 12 224 3 3x4 / × 3x8 9-9-8 Plate Offsets (X,Y)- [2:0-3-10 Edge]

LOADING (psf) TCIL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.84 BC 0.74 WB 0.20 Matrix-MS	DEFL. in Vert(LL) 0,41 Vert(CT) 0.36 Horz(CT) -0.02	(loc) 8-11 8-11 2	l/defl >281 >322 n/a	L/d 240 180 n/a	PLATES GRIP MT20 244/190 Weight 51 lb FT = 20%
LUMBER-			BRACING-				

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=431/0-3-8, 8=357/0-3-8

Max Horz 2=183(LC 12)

Max Uplift 2=-137(LC 9), 8=-193(LC 9)

FORCES. (lb) - Max, Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-4=-884/1375

BOT CHORD 2-8=-545/321

WEBS 4-8=-340/559

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3,0psf; h=18ft; Cat, II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=137, 8=193,



Structural wood sheathing directly applied or 5-2-0 oc purlins,

Rigid ceiling directly applied or 4-11-14 oc bracing

except end verticals.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

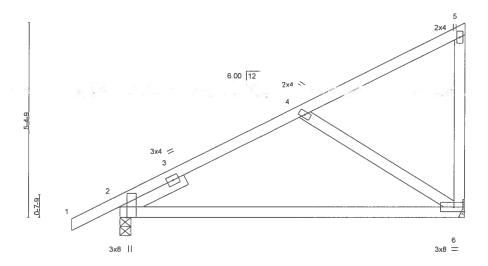
July 25,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314



Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES			
						T17699091		
1854243	T24	MONO TRUSS	3	1				
					Job Reference (optional)			
Builders FirstSource,	Jacksonville, FL - 32244,	8 240 s Jun 8 2019 MiTek Industries, Inc Thu Jul 25 14 43 39 2019 Page 1						
		ID C0mABbmGidkrSLODgHTkINztW50-fSeNbfqO_8x1?9KPhzP5HemNWWSi1vW1uD5nypyuUyI						
	1-4-0	5-1-0			9-6-0			
	1-4-0	5-1-0			4-5-0			

Scale = 1,30,7



TCLL 20.0 Plate Grip DOL 1.25 TC 0.50 -0.18 240 MT20 244/190 Vert(LL) 6-9 >637 TCDL 7.0 Lumber DOL 1.25 0.63 -0.36 Vert(CT) >315 180 **BCLL** 0.0 Rep Stress Incr YES WB 0.17 Horz(CT) 0.02 n/a n/a

 BCDL
 10.0
 Code FBC2017/TPI2014
 Matrix-MS
 Weight: 50 lb
 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-11-8

REACTIONS. (lb/size) 2=423/0-3-8, 6=341/Mechanical

Max Horz 2=176(LC 12)

Max Uplift 2=-72(LC 12), 6=-134(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

TOP CHORD 2-4=-883/127 BOT CHORD 2-6=-325/294 WEBS 4-6=-329/379

NOTES-

- Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II, Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 6=134.



Structural wood sheathing directly applied or 5-1-3 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019



Job Truss Truss Type HARTLEY - COREY RES Qty Ply T17699092 1854243 T25 Half Hip Girder 1 Job Reference (optional) Builders FirstSource Jacksonville FL - 32244 8,240 s Jun 8 2019 MiTek Industries, Inc Thu Jul 25 14,43 40 2019 Page 1 ID. C0mABbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSLODgHTklNztW50-7eClo?q0lS3udlvbFhwKqrJWTwskmLUA6trKUFyuUyHabbmGidkrSloDghtklDfhwAbbmGidkrSloDghtklDfh9-6-0 4x8 = Scale = 1.28 6 2x4 || 6.00 12 0-7-9 6 3x8 || 4x4 = 3x8 7-10-0 9-6-0 7-10-0 Plate Offsets (X,Y)-[2:0-8-0,0-0-3], [3:0-5-0,0-2-0] LOADING (psf) SPACING-2-0-0 CSL DEFL. in (loc) **I/defl** L/d **PLATES** GRIP **TCLL** 20.0 Plate Grip DOL 0.62 1.25 TC 0.04 Vert(LL) 6-9 >999 240 MT20 244/190 TCDL 7.0 1.25 вс Lumber DOL 0.32 Vert(CT) -0.076-9 >999 180 BCLL 0.0 Rep Stress Incr 0.25 NO WB Horz(CT) 0.01 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 58 lb FT = 20% BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2

WEB\$ 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3

(lb/size) 2=446/0-3-8, 5=474/Mechanical REACTIONS.

Max Horz 2=151(LC 8)

Max Uplift 2=-111(LC 8), 5=-257(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-344/72

WERS 3-6=-200/535, 3-5=-682/342

NOTES-

1) Unbalanced roof live loads have been considered for this design,

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 168 lb down and 205 lb up at 7-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=54, 3-4=54, 5-7=-20 Concentrated Loads (ib)

Vert: 6=-156(B)

No 39380

No 39380

Thomas A Albani PE No.39380

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL 7473 rev. 10/03/2015 REFORE USE Design valid for use only with MT els6 connectors. This design is based only upon parameters and included building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Braiding indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component

Safety Information available from Truss Plate Institute, 218 N. Lee Street, Sune 312, Alexandria, VA 22314



Job Truss Truss Type Qty HARTLEY - COREY RES T17699093 Half Hip Girder 1854243 T26 Job Reference (optional) Jacksonville, FL - 32244 8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 41 2019 Page 1 Builders FirstSource ID.C0mABbmGidkrSLODgHTkINztW50-bqm80KreWmBkFSUooOSZM3rotKF1VoFKLXau1hyuUyG-1-4-8 8-3-8 11-5-8 Scale = 1 32 4 4x8 = 2x4 11 3x4 = 5 6 10 00 12 4x4 / 2x4 -11 3x8 = 3x10 || 2x4 | 5-1-8 8-3-8 11-5-8 5-1-8 3-2-0 3-2-0 Plate Offsets (X,Y)-[2:0-7-8,0-0-2], [4:0-6-4,0-2-0] LOADING (psf) SPACING-2-0-0 CSI DEFL I/defl L/d**PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.13 Vert(LL) -0.01 9 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 ВC 0.19 Vert(CT) -0.02 9-12 >999 180 **BCLL** 0.0 Rep Stress Incr NO WB 0.22 Horz(CT) -0.01 n/a n/a Code FBC2017/TPI2014 FT = 20% BCDL 10.0 Matrix-MS Weight: 181 lb LUMBER-BRACING. TOP CHORD 2x4 SP No 2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x6 SP No.2 except end verticals 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS SLIDER Left 2x6 SP No.2 2-0-10 (lb/size) 7=1282/0-3-8, 2=1047/0-3-0 REACTIONS.

Max Horz 2=177(LC 23)

Max Uplift 7=527(LC 5), 2=360(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1041/401, 4-5=-607/253, 5-6=-607/253, 6-7=-1064/448 **BOT CHORD** 2-9=-349/737, 8-9=-359/757

4-9=-355/687, 4-8=-288/222, 6-8=-482/1157 WEBS

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 - Top chords connected as follows: 2x4 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope), Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=lb| 7=527, 2=360,
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 454 lb down and 277 lb up at 4-2-4, 321 lb down and 154 lb up at 6-2-4, and 321 lb down and 154 lb up at 8-2-4, and 321 lb down and 154 lb up at 10-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 7-10=-20



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

July 25,2019

Continued on page 2

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord methers only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord methers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPH Quality Criteria, DSB-89 and BCSI Building Compositely Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314



Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES	
1854243	T26	Half Hip Girder	1			T17699093
B. Tata - Flance	120	The Trip Green		2	Job Reference (optional)	

Builders FirstSource,

Jacksonville, FL - 32244,

8 240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 41 2019 Page 2 ID:C0mABbmGidkrSLODgHTkINztW50-bqm80KreWmBkFSUooOSZM3rotKF1VoFKLXau1hyuUyG

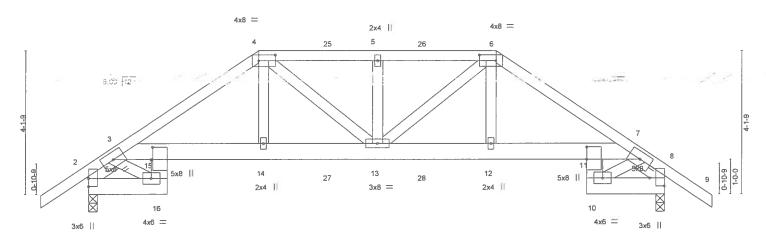
LOAD CASE(S) Standard Concentrated Loads (lb)

Vert 8=-321(F) 14=-454(F) 15=-321(F) 16=-321(F)



Truss Type HARTLEY - COREY RES Job Truss Qty T17699094 1854243 T27 Hip Girder | Job Reference (optional) 8 240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 42 2019 Page 1 Jacksonville, FL - 32244, Builders FirstSource ID C0mABbmGidkrSLODgHTkINztW50-31KWDgsGH3Jbtc2_M6zovGOvhkOqEEKTaBKRZ7yuUyF 4-10-8 2-7-8 11-8-8 14-4-0 16-7-0 17-11-8 8-3-8 3-5-0 2-7-8 1-4-8 2-3-0

Scale 3/8"=1"



	L	2-3-0 , 4-	10-8	8	-3-8	11	1-8-8		ł	14-4-0	16-7-0	_
		2-3-0 2-	-7-8		-5-0	' 3	-5-0		1	2-7-8	2-3-0	<u>'</u>
Plate Offse	ets (X,Y)-	[4:0-5-12,0-2-0], [6:0-5-12	2,0-2-0], [8:Edg	je,0-5-1], [11	:0-4-4,0-0-0],	[15:0-4-4,0-0-8]						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.38	Vert(LL)	0.09	13	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.99	Vert(CT)	-0.14	13	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.30	Horz(CT)	0.11	8	n/a	n/a		
BCDL	10.0	Code FBC2017/TF	PI2014	Matri	x-MS						Weight: 110 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 0-10-11, Right 2x4 SP No.3 0-10-11

REACTIONS. (lb/size) 2=1357/0-3-0, 8=1357/0-3-0

Max Horz 2=105(LC 7)

Max Uplift 2=-537(LC 8), 8=-537(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten, - All forces 250 (lb) or less except when shown.

TOP CHORD 3-18=-557/233, 3-4=-2462/1041, 4-5=-2346/1003, 5-6=-2346/1003, 6-7=-2462/1015,

7-8=-557/232

2-16=-422/937, 15-16=-282/634, 3-15=-689/1610, 14-15=-866/2012, 13-14=-884/2053, BOT CHORD

12-13=803/2053, 11-12=788/2012, 7-11=675/1610, 10-11=236/634, 8-10=361/937 4-14-343/774, 4-13-231/438, 5-13-439/249, 6-13-216/438, 6-12-325/774,

WEBS 3-16=-739/341, 7-10=-739/287

NOTES.

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope), Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (ft=lb) 2=537. 8=537.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 120 lb down and 98 lb up at 4-10-8, 101 lb down and 94 lb up at 6-11-4, 101 lb down and 94 lb up at 8-3-8, and 101 lb down and 94 lb up at 9-7-12, and 120 lb down and 98 lb up at 11-8-8 on top chord, and 302 lb down and 229 lb up at 4-10-8, 82 lb down and 32 lb up at 6-11-4, 82 lb down and 32 lb up at 8-3-8, and 82 lb down and 32 lb up at 9-7-12, and 302 lb down and 229 lb up at 11-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 6-9=-54, 16-17=-20, 11-15=-20, 10-21=-20



Structural wood sheathing directly applied or 3-5-5 oc purlins.

Rigid ceiling directly applied or 7-6-1 oc bracing. Except:

8-0-0 oc bracing: 11-12

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

Continued on page 2

♠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev: 10/03/2015 BEFORE USE. Design valid for use only with MT eMD connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated it is to revert buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

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Job	Truss	Truss Type	Qty Pty HARTLEY - COREY RES.
1854243	T27	Hip Girder	T17699094
	- First Course Included the File	22244	Job Reference (optional)

Builders FirstSource,

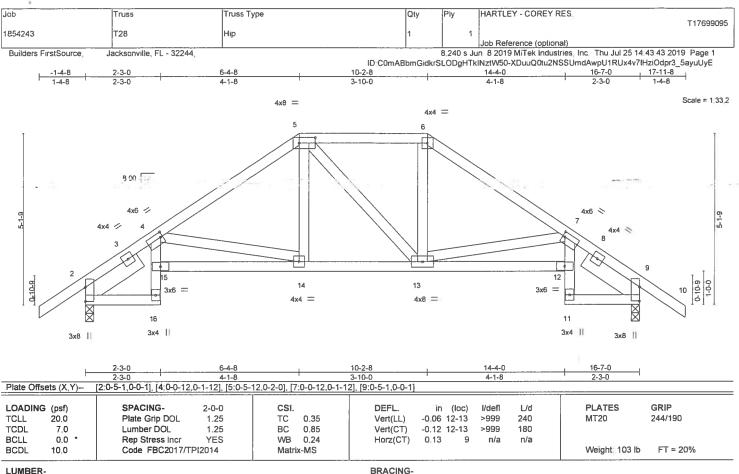
Jacksonville, FL - 32244,

8 240 s Jun 8 2019 MiTek Industries, Inc Thu Jul 25 14 43 42 2019 Page 2 ID:C0mABbmGidkrSLODgHTklNztW50-31KWDgsGH3Jbtc2_M6zovGOvhkOqEEKTaBKRZ7yuUyF

LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 4=-101(F) 6=-101(F) 14=-302(F) 13=-77(F) 5=-101(F) 12=-302(F) 25=-101(F) 26=-101(F) 27=-77(F) 28=-77(F)





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS

2x4 SP No.3

SLIDER Left 2x6 SP No.2 1-11-8, Right 2x6 SP No.2 1-11-8

REACTIONS. (lb/size) 2=688/0-3-0, 9=688/0-3-0

Max Horz 2=-129(LC 10)

Max Uplift 2=-140(LC 12), 9=-140(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-4=-743/359, 4-5=-823/368, 5-6=-643/363, 6-7=-819/368, 7-9=-743/362 TOP CHORD

2-16=-175/535, 14-15=-405/1143, 13-14=-128/639, 12-13=-424/1115, 9-11=-187/518 **BOT CHORD**

4-14-586/285, 7-13-624/304 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=140, 9=140,



Structural wood sheathing directly applied or 6-0-0 oc purlins.

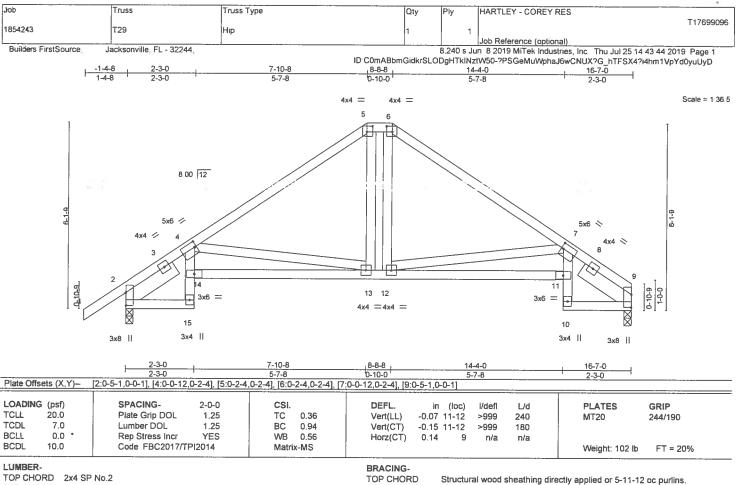
Rigid ceiling directly applied or 9-0-11 oc bracing.

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July 25,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer in the overall building designer and indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Comport Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314





BOT CHORD

Rigid ceiling directly applied or 6-11-2 oc bracing.

BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

SLIDER Left 2x6 SP No.2 1-11-8, Right 2x6 SP No.2 1-11-8

REACTIONS.

(lb/size) 9=611/0-3-0, 2=691/0-3-0

Max Horz 2=147(LC 9)

Max Uplift 9=-120(LC 13), 2=-147(LC 12)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

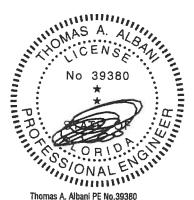
TOP CHORD 2-4=-790/368, 4-5=-751/335, 5-6=-630/353, 6-7=-751/335, 7-9=-784/379

2-15=-234/579, 13-14=-564/1312, 12-13=-108/555, 11-12=-583/1253, 9-10=-245/558 **BOT CHORD**

WEBS 4-13=-825/463, 5-13=-61/252, 6-12=-71/251, 7-12=-815/482

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp.C; Encl.. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=|b|) 9=120, 2=147.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

July 25,2019

A WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANS/ITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information. available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Truss Type HARTLEY - COREY RES Job Truss Qty T17699097 1854243 T30 Roof Special Job Reference (optional)
8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 45 2019 Page 1 Jacksonville, FL - 32244 Builders FirstSource ID: C0mABbmGidkrSLODgHTklNztW50-Uc0eriu8Z_iAk4nZ1EWVXv0PXxQ7RVfvG9Y5ASyuUyC 14-4-0 16-7-0 8-3-8 6-0-8 6-0-8 2-3-0 Scale = 1 40 1 4x6 == 5 8.00 |12 5x6 / > 6 4x4 < 10 11 3x6 4x8 = 13 9 3x8 || 3x4 II 3x4 || 3xB -11 16-7-0 2-3-0 6-0-8 6-0-8 2-3-0 [2:0-5-1,0-0-1], [4:0-0-12,0-2-0], [6:0-0-12,0-2-0], [8:0-5-1,0-0-1] Plate Offsets (X,Y)-GRIP SPACING-DEFL. in (loc) I/defl L/d **PLATES** LOADING (psf) 2-0-0 CSI. -0.07 10-11 240 MT20 244/190 TCLL 20.0 Plate Grip DOL 1.25 TC 0.40 Vert(LL) >999 180 TCDL 1.25 BC 0.95 Vert(CT) -0.16 10-11 >999 7.0 Lumber DOL Rep Stress Incr YES WB 0.64 Horz(CT) 0.15 n/a BCLL 0.0 Weight: 96 lb FT = 20% BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS LUMBER-BRACING-TOP CHORD Structural wood sheathing directly applied or 5-10-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 5-1-0 oc bracing.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

2x4 SP No.3 WEBS

Left 2x6 SP No.2 1-11-8, Right 2x6 SP No.2 1-11-8 SLIDER

REACTIONS. (lb/size) 8=611/0-3-0, 2=691/0-3-0

Max Horz 2=153(LC 9)

Max Uplift 8=-122(LC 13), 2=-148(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-4=-806/366, 4-5=-741/325, 5-6=-741/325, 6-8=-801/377

BOT CHORD 2-13=-236/595, 11-12=-581/1356, 10-11=-599/1273, 8-9=-247/562

WEBS 5-11=-125/446, 6-11=-856/512, 4-11=-866/493

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 8=122, 2=148.

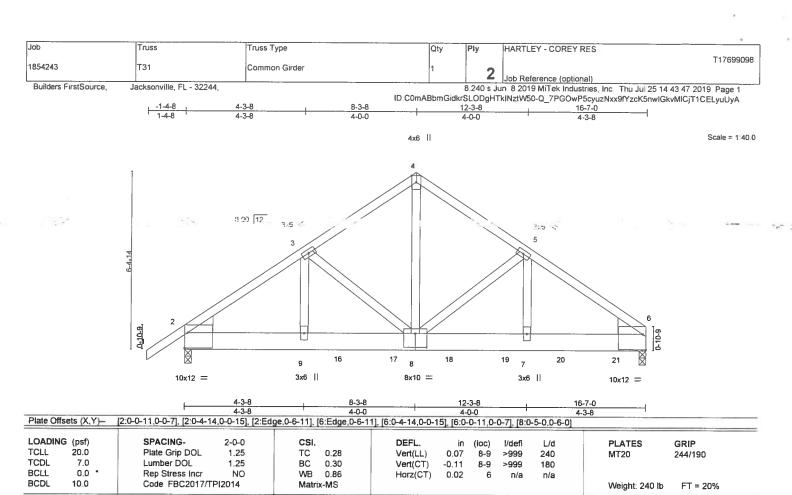


Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

MET WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing individual temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No 2 2x8 SP 2400F 2.0E **BOT CHORD**

WERS 2x4 SP No 3

WEDGE

Left; 2x8 SP 2400F 2.0E, Right: 2x8 SP 2400F 2.0E

REACTIONS. (lb/size) 2=3499/0-3-0, 6=4852/0-3-0

Max Horz 2=153(LC 5)

Max Uplift 2=-1368(LC 8), 6=-1239(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-5082/2075, 3-4=-4338/1530, 4-5=-4341/1532, 5-6=-5702/1570 **BOT CHORD**

2-9=-1724/4155, 8-9=-1724/4155, 7-8=-1248/4684, 6-7=-1248/4684

4-8-1595/4514, 5-8-1446/167, 5-7-54/1512, 3-8-774/701, 3-9-689/755

WEBS

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-6-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

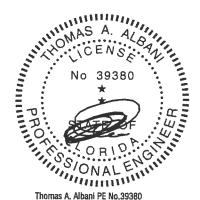
* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb)
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1865 lb down and 1348 lb up at 5-6-12, 1033 lb down and 304 lb up at 7-6-12, 1038 lb down and 210 lb up at 9-6-12, 1038 lb down and 176 lb up at 11-6-12, and 1038 ib down and 187 lb up at 13-6-12, and 1096 lb down and 195 lb up at 15-6-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25



Structural wood sheathing directly applied or 5-2-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

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Continued on page 2

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.



6904 Parke East Blvd Tampa, FL 36610

Job HARTLEY - COREY RES Truss Truss Type Qty Ply T17699098 1854243 T31 Common Girder 2 Job Reference (optional) 8 240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 47 2019 Page 2

Builders FirstSource,

Jacksonville, FL - 32244,

ID COmABbmGidkrSLODgHTkINztW50-Q_7PGOwP5cyuzNxx9fYzcK5nwlGkvMlCjT1CELyuUyA

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 10-13=-20

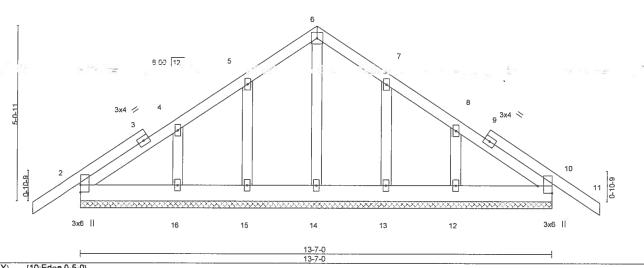
Concentrated Loads (lb)

Vert: 16=-1865(B) 17=-1033(B) 18=-1038(B) 19=-1038(B) 20=-1038(B) 21=-1038(B)



Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES	
						T17699099
1854243	T32G	COMMON SUPPORTED GAB	1	1		
					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8,240 s Ju	n 8 2019 MiTek Industries, Inc.	Thu Jul 25 14 43 48 2019 Page 1
			ID C0mABbmGidkr	SLODgHTI	kINztW50-uBhnUkx1sv4lbXW8jM	I4C8Xe?T8gHe0kMy7nlmnyuUy9
	1-4-8	6-9-8			13-7-0	14-11-8
	1-4-8	6-9-8	1		6-9-8	1-4-8

Scale: 3/8"=1"



4x4 =

Plate Oil	sets (A, T)-	[10:Eage,0-5-0]										
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.10	Vert(LL)	-0.00	11	n/r	120	MT20	244/190
TCDL	7,0	Lumber DOL	1.25	ВС	0.02	Vert(CT)	-0.00	11	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	10	n/a	n/a		
BCDL	10.0	Code FBC2017/TP	12014	Matri	x-S	''					Weight: 88 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.3 OTHERS

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-7-0.

(lb) - Max Horz 2=-159(LC 10)

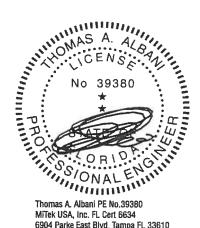
Max Uplift All uplift 100 lb or less at joint(s) 2, 10 except 15=-113(LC 12), 16=-139(LC 12), 13=-112(LC 13),

12=-138(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 16, 13, 12

FORCES. (lb) - Max, Comp./Max, Ten, - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10 except (jt=lb) 15=113, 16=139, 13=112, 12=138.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

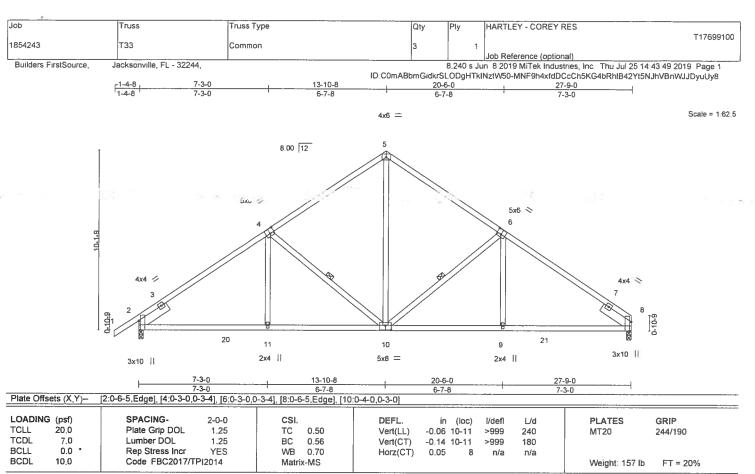


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BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x6 SP No.2 1-11-8, Right 2x6 SP No.2 1-11-8

REACTIONS. (lb/s

(lb/size) 2=1103/0-3-0, 8=1025/0-3-0

Max Horz 2=307(LC 9)

Max Uplift 2=-413(LC 12), 8=-366(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-1354/605, 4-5=-1038/556, 5-6=-1038/556, 6-8=-1360/608 BOT CHORD 2-11=-460/1218, 10-11=-460/1216, 9-10=-377/1058, 8-9=-376/1059 WEBS 5-10=-381/807, 6-10=-544/384, 6-9=0/253, 4-10=-535/378, 4-11=0/252

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=413, 8=366



Structural wood sheathing directly applied or 4-3-6 oc purlins.

6-10, 4-10

Rigid ceiling directly applied or 8-5-2 oc bracing.

1 Row at midpt

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Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty Ply HARTLEY - COREY RES T17699101 1854243 T33G GABLE Job Reference (optional) Jacksonville, FL - 32244. Builders FirstSource 8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 50 2019 Page 1 ID C0mABbmGidkrSLODgHTklNztW50-qZpXuPyHOXKTqrgWqn6gEyjF2yFl6tifQRGsrgyuUy7 7-3-0 13-10-8 20-6-0 27-9-0 7-3-0

4x6 =

Scale = 1:62.9

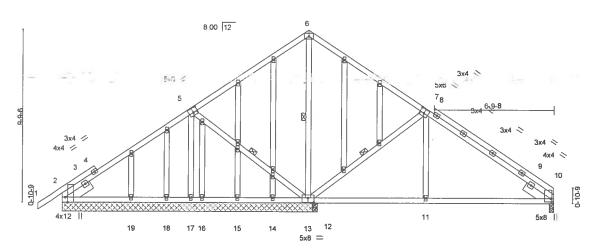


Plate Offsets (X,Y)-	7-3-0 [2:0-3-8,Edge], [5:0-3-0,0-3-0], [7:0-3-0,	6-7-8 0-3-0], [10:0-2-4,0-0-12], [0 ¹ 6-8 6-1- [13:0-4-0,0-3-0]	0 '	7-3-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.48 BC 0.44 WB 0.21 Matrix-MS	DEFL. in Vert(LL) -0.03 10 Vert(CT) -0.07 10 Horz(CT) 0.01		PLATES GRIP MT20 244/190 Weight: 221 lb FT = 20%

14-5-0

BRACING-

WERS

TOP CHORD

BOT CHORD

20-6-0

1 Row at midpt

27-9-0

Structural wood sheathing directly applied or 6-0-0 oc purlins.

6-13, 7-13, 5-13

Rigid ceiling directly applied or 10-0-0 oc bracing.

13-10-8

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

SLIDER Left 2x6 SP No.2 1-7-8, Right 2x6 SP No.2 1-7-8

REACTIONS. All bearings 14-5-0 except (jt=length) 10=0-3-0, 12=0-3-8.

(lb) - Max Horz 2=304(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 18, 19 except 2=-142(LC 12), 13=-346(LC 13), 17=-225(LC 12), 10=-222(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 14, 15, 16, 18, 19 except 2=347(LC 23), 13=535(LC 1), 17=325(LC 19), 10=488(LC 1), 12=423(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-5=288/206, 7-10=564/263

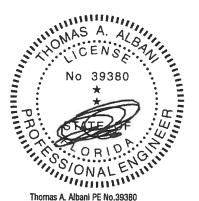
BOT CHORD 12-13=-86/376, 11-12=-86/376, 10-11=-86/377

WEBS 6-13=-302/35, 7-13=-573/385, 7-11=0/265, 5-13=-271/229, 5-17=-280/198

7-3-0

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4:2psf; BCDL=3:0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 19 except (it=lb) 2=142, 13=346, 17=225, 10=222.



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July 25,2019



Job Truss Truss Type Qty HARTLEY - COREY RES T17699102 T34 1854243 Common Girder Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244 8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 53 2019 Page 1 ID_C0mABbmGidkrSLODgHTklNztW50-F8UgXR_AhSi1hlO5VwfNrbLns9HkJ2956PUWS?yuUy4 13-10-8 18-4-13 22-11-3 27-9-0 4-9-13 4-6-5 4-6-5 Scale = 1:61.9 5 8.00 12 5x6 < 5x6 > 4x5 5 4x6 < 3x6 / 3x6 < 2 0-10-9 15 ²⁵ 28₂₉ 11 33 16 23 27 30 31 32 10 13 12 14 8x10 = 3x8 || 7x8 = 10x12 = 7x8 = 3x8 || 8x10 = 4-9-13 13-10-8 18-4-13 22-11-3 4-9-13 4-6-5 4-6-5 4-6-5 Plate Offsets (X,Y)-[12:0-4-0,0-5-4], [13:0-6-0,0-6-0], [14:0-4-0,0-5-4] LOADING (psf) SPACING-DEFL **PLATES GRIP** 2-0-0 CSL I/defl L/d TCLL 20.0 Plate Grip DOL 1.25 TC 0.45 Vert(LL) -0.15 12-13 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 ВĊ 0.39 Vert(CT) -0.27 12-13 >999 180 Rep Stress Incr BCLL 0.0 NO WB 0.93 Horz(CT) 0.06 n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS Weight, 513 lb FT = 20% **BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x8 SP 2400F 2.0E

2x4 SP No.3 *Except* WEBS 5-13: 2x4 SP No.2

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 1=8194/(0-3-0 + bearing block) (req. 0-4-13), 9=7926/(0-3-0 + bearing block) (req. 0-4-11)

Max Horz 1=-230(LC 25)

Max Uplift 1=-1560(LC 8), 9=-1524(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-11657/2199, 2-4=-9944/1917, 4-5=-7786/1576, 5-6=-7786/1576, 6-8=-10044/1950,

8-9=-11684/2239

BOT CHORD 1-15=1868/9576, 14-15=1868/9576, 13-14=1545/8244, 12-13=1508/8330, 11-12=-1786/9590, 9-11=-1786/9590

WEBS

5-13=-1626/8237, 6-13=-3285/753, 6-12=-720/3726, 8-12=-1614/416, 8-11=-341/1803,

4-13=-3136/705, 4-14=-663/3552, 2-14=-1704/409, 2-15=-325/1882

NOTES-

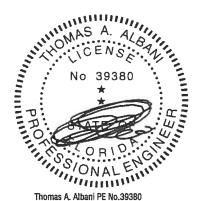
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc

Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
- 3) 2x8 SP 2400F 2.0E bearing block 12" long at jt. 1 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SP No.2.
- 4) 2x8 SP 2400F 2.0E bearing block 12" long at jt. 9 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SP No.2.
- 5) Unbalanced roof live loads have been considered for this design.
- 6) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=1560, 9=1524



Structural wood sheathing directly applied or 4-2-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	HARTLEY - COREY RES.	
1854243	T34	Common Girder	1	2	T17	699102
					Job Reference (optional)	
Builders FiretSource Is	ackeonville EL . 32244			0.040 - 1.	- 0.0040 MT-1 I-1 -1 -1 -1 - 1 - 1 - 1 - 1 - 1 - 1	

8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 53 2019 Page 2 ID C0mABbmGidkrSLODgHTklNztW50-F8UgXR_AhSi1hlO5VwfNrbLns9HkJ2956PUWS?yuUy4

NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1033 lb down and 233 lb up at 1-6-12, 1081 lb down and 203 lb up at Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1033 ib down and 233 ib up at 1-6-12, 1001 ib down and 231 ib up at 1-6-12, 1038 ib down and 195 ib up at 5-6-12, 1038 ib down and 203 ib up at 7-6-12, 1078 ib down and 209 ib up at 9-6-12, 1198 ib down and 231 ib up at 13-6-12, 1198 ib down and 231 ib up at 13-6-12, 1198 ib down and 231 ib up at 13-6-12, 1198 ib down and 231 ib up at 13-6-12, 1198 ib down and 232 ib up at 13-6-12, and 984 ib down and 231 ib up at 23-6-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-54, 5-9=-54, 17-20=-20

Concentrated Lunda (b)

Vert: 14=-1038(F) 23=-1033(F) 24=-1038(F) 25=-1038(F) 26=-1038(F) 27=-1155(F) 28=-1155(F) 29=-1155(F) 30=-1155(F) 31=-1146(F) 32=-1146(F) 33=-984(F)



Job Truss Truss Type HARTLEY - COREY RES T17699103 GABLE 1854243 V01 Job Reference (optional) 8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 54 2019 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID: C0mABbmGidkrSLODgHTklNztW50-jK22kn?oSlquJSzl3dBcOou1TZj72hsEK2E4_RyuUy3 10-11-2 21-10-4 10-11-2 Scale = 1.54.8 4x4 = 6 10 00 12 3x6 💉 23 12 18 15 22 21 3x6 = 3x6 / 21-10-4 21-10-4 SPACING-DEFL. **PLATES** GRIP LOADING (psf) 2-0-0 CSI. l/defl L/d Plate Grip DOL 1.25 TC 0.07 Vert(LL) n/a 999 MT20 244/190 TCLL 20.0 n/a 1.25 вс 0.06 Vert(CT) п/а 999 TCDL 7.0 Lumber DOL n/a YE\$ WB 0.13 Horz(CT) 0.01 n/a n/a **BCLL** 0.0 Rep Stress Incr 11 Code FBC2017/TPI2014 Matrix-S Weight: 131 lb FT = 20% BCDL 10.0 LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x4 SP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2x4 SP No.3 WEBS

1 Row at midpt 6-17

REACTIONS. All bearings 21-10-4

Max Horz 23=-261(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 11, 23, 12 except 19=135(LC 12), 20=-140(LC 12),

21=-132(LC 12), 22=-150(LC 12), 16=-133(LC 13), 15=-141(LC 13), 14=-132(LC 13), 13=-150(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 11, 23, 17, 19, 20, 21, 22, 16, 15, 14, 13, 12

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown

TOP CHORD

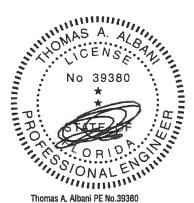
1-2=-290/205

BOT CHORD 1-23=261/261

NOTES.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11, 23, 12 except (jt=lb) 19=135, 20=140, 21=132, 22=150, 16=133, 15=141, 14=132, 13=150.



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July 25,2019

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Job Truss Truss Type HARTLEY - COREY RES Ply Qty T17699104 1854243 V02 Valley 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 55 2019 Page 1 16-9-11 4x4 = Scale = 1:46.4 8.00 12 3x6 🗸 12 10 9 8 7 3x6 = 16-9-11 16-9-11 LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) I/defi L/d **PLATES GRIP TCLL** 20.0 Plate Grip DOL 1.25 TC 0.18 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 вс 0.19 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.20 Horz(CT) -0.00 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 84 lb FT = 20%

LUMBER-

OTHERS

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 16-9-5.

2x4 SP No.3

(lb) - Max Horz 1=174(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 9 except 10=-152(LC 12), 12=-125(LC 12), 8=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 9=422(LC 19), 10=404(LC 19), 12=286(LC 19),

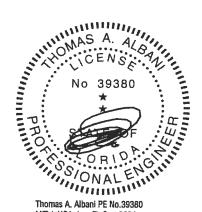
8=372(LC 20)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown,

WEBS 3-10=-298/250, 5-8=-256/215

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 9 except (it=lb) 10=152, 12=125, 8=135.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

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HARTLEY - COREY RES Job Truss Truss Type Qty T17699105 lvo3 Valley 1854243 Job Reference (optional) Jacksonville, FL - 32244, 8 240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 56 2019 Page 1 Builders FirstSource, ID COmABbmGidkrSLODgHTklNztW50-fjAo9T12_N4cYm7gB2D4TDzLkNM8WcfXoMjA2KyuUy1 8-8-3 8-8-3 14-3-11 5-7-8 4x4 = Scale = 1.36.4 3 9 M 17 2x4 | 2x4 || 2×4 2-0-7 62x4 | 9 10 2x4 || 2x4 | 2x4 || 14-3-11 LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) I/defl 1/dPLATES GRIP TC 244/190 TCLL 20,0 Plate Grip DOL 1.25 0.21 Vert(LL) n/a n/a 999 MT20 BC 999 TCDL 70 Lumber DOL 1.25 0.15 Vert(CT) n/a n/a **BCLL** 0.0 Rep Stress Incr YES WB 0.11 Horz(CT) 0.00 6 n/a n/a FT = 20% **BCDL** 10.0 Code FBC2017/TPI2014 Matrix-S Weight 64 lb **BRACING-**LUMBER-2x4 SP No.2 Structural wood sheathing directly applied or 6-0-0 oc purlins, TOP CHORD TOP CHORD BOT CHORD 2x4 SP No.2 except end verticals. 2x4 SP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing WERS

2x4 SP No.3 OTHERS

REACTIONS.

ONS. All bearings 14-3-5 (lb) - Max Horz 1=134(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 6, 8 except 9=-172(LC 12), 7=-141(LC 13)

All reactions 250 lb or less at joint(s) 1, 6 except 8=390(LC 19), 9=409(LC 19), 7=331(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS

2-9=-332/280, 4-7=-265/227

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf; BCDL=3.0psf, h=18ft, Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 8 except (jt=lb) 9=172, 7=141.



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July 25,2019

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ANSITYP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314



Job Truss Truss Type Qty Ply HARTLEY - COREY RES. T17699106 1854243 V04 Valley Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244, 8 240 s Jun 8 2019 MiTek Industries, Inc. Thu Jul 25 14 43 57 2019 Page 1 ID:C0mABbmGidkrSLODgHTklNztW50-7vkBMp2glgCTAwisklkJ0RWWXnjsF3cg10SkbmyuUy0 Scale = 1.26.3 4x4 = 8.00 12 2x4 || ₄2x4 || 8 6 3x6 / 3x6 < 2x4 || 2x4 || 2x4 || 12-4-0 12-4-0 LOADING (psf) SPACING-2-0-0 CSI. DEFL. **PLATES** GRIP in I/defl L/d (loc) TCLL 20,0 Plate Grip DOL 1.25 TC 0.15 Vert(LL) MT20 244/190 n/a n/a 999 TCDL 7.0 Lumber DOL 1.25 вс 0.12 Vert(CT) n/a 999 n/a BCLL 0,0 Rep Stress Inci YES WB 0.07 Horz(CT) 0.00 5 n/a n/a BCDL 100 Code FBC2017/TPI2014 Matrix-S Weight: 47 lb FT = 20%LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 12-3-10,

Max Horz 1=-95(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=-133(LC 12), 6=-133(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=294(LC 19), 6=294(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-263/228, 4-6=-263/228

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads,
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (jt=lb) 8=133, 6=133.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Compo



Ply Job Truss Truss Type Qty HARTLEY - COREY RES T17699107 1854243 V05 Valley Job Reference (optional) Builders FirstSource Jacksonville Ft - 32244 8.240 s Jun 8.2019 MiTek Industries, Inc. Thu Jul 25 14 43 57 2019 Page 1 ID C0mABbmGidkrSLODgHTkINztW50-7vkBMp2glgCTAwisklkJ0RWX0njBF4Cg10SkbmyuUy0 7-4-6 3-8-3 4x4 = 2 8.00 12 9-0-4 0-0-4 2x4 || 2x4 / 2x4 > 0-0-6 LOADING (psf) SPACING-2-0-0 CSI. DEFL. **PLATES** GRIP in (loc) l/defl L/d TCLL 20.0 Plate Grip DOL 1.25 TC 0.11 Vert(LL) MT20 244/190 999 n/a n/a TCDL 7.0 Lumber DOL 1.25 BC 0.10 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.03 0.00 3 Horz(CT) n/a n/a **BCDL** Code FBC2017/TPI2014 Weight: 25 lb FT = 20% 10.0 Matrix-S

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 1=116/7-3-10, 3=116/7-3-10, 4=242/7-3-10

Max Horz 1=53(LC 11)

Max Uplift 1=-31(LC 12), 3=-36(LC 13), 4=-34(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

July 25,2019

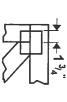
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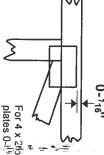


Symbols

PLATE LOCATION AND ORIENTATION



Center pate on joint unless x, y Dimensity's are in ft-in-sixteenths and fully ambed teeth Apply plates to both sides of truss offsets a p indicated



For 4 x 25 prientation, locate plates 0-15 from outside edge of truss.

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G

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required direction of slots in connector plates This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

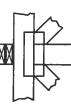
width measured perpendicular to slots. Second dimension is the length parallel to slots. The first cimension is the plate

LATERAL BRACING LOCATION



output. Use T or I bracing Indicated by symbol shown and/or by text in the bracing section of the if indicated

BEARING



Min size shown is for crushing only. number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates ocation where bearings

ANSI/TPI1: Industry Standards:

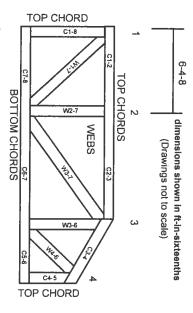
National Design Specification for Metal Plate Connected Wood Truss Construction

DSB-89 **Building Component Safety Information** Design Standard for Bracing.

BCSI:

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves bracing should be considered. may require bracing, or alternative Tor I

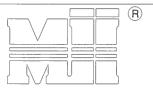
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- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building all other interested parties. designer, erection supervisor, property owner and
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each locations are regulated by ANSI/TPI 1. joint and embed fully. Knots and wane at joint
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

A MITCE ATTIMATE	•						
Nailing Pattern							
T-Brace size	Nail Size	Nail Spacing					
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.					

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

<u>A</u>

Nails	
Web	I-Brace
Nails	

	Brace for One-F	Size Ply Truss
	Specified C Rows of Late	ontinuous eral Bracing
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

		ce Size -Ply Truss
		Continuous iteral Bracing
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



Thomas A. Albani PE No.39380 MITER USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1

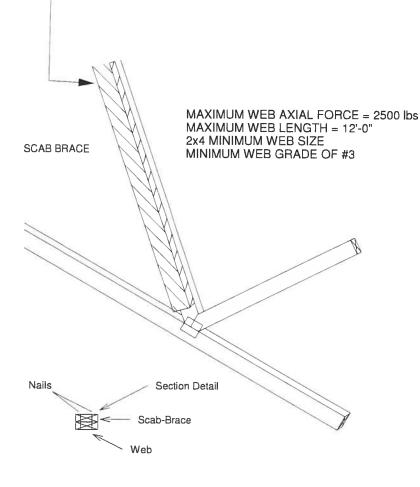


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

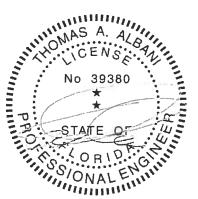
Scab must cover full length of web +/- 6".

THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

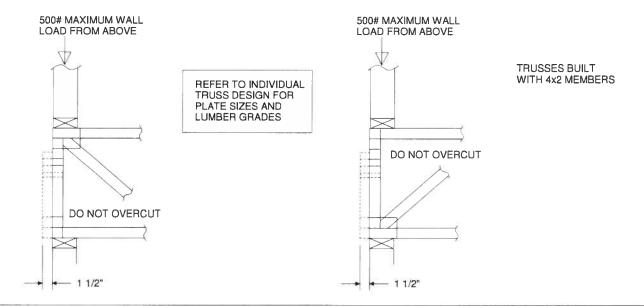
 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

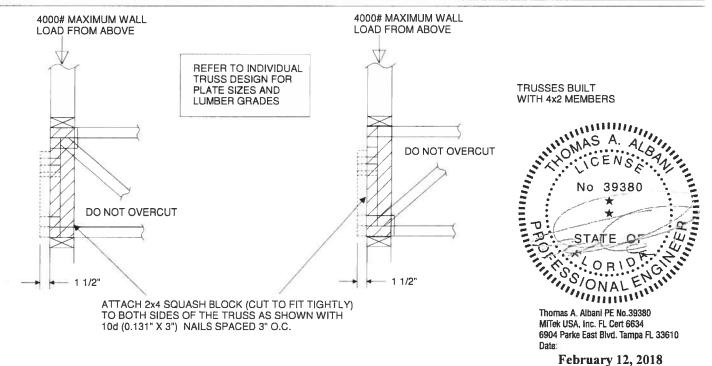
 3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.

 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.

 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





Standard Gable End Detail

MII-GE130-D-SP

Page 1 of 2

DIAGONAL

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d

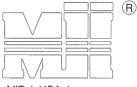
HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED

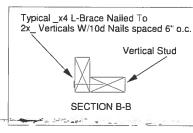
MiTek USA, Inc.



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DIAGONAL BRACE

4'-0" O.C. MAX



TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =

16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

Vertical Stud

(4) - 16d Nails

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD

(2) - 10d NAILS

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-3"

Max.

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG.
- ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A

- 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
 GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
 THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE
- 06-01-13 BY SPIB/ALSC. 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

No 39380

STATE OF THE STATE OF

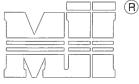
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

Standard Gable End Detail

MII-GE130-SP

Page 1 of 2

MiTek USA, Inc.



MiTek USA, Inc.

ENGINEERED BY

Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss * ** 3x4 =

L-Bracing Refer

to Section B-B

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Doof Chaothina

Refer to Section A-A NOTE

* - Diagonal Bracing

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A

2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.

(REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

06-01-13 BY SPIB/ALSC. 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Roof Sheathing /
24" Max. 1'-3" (2) - 10d NAILS (2) - 10d NAILS
Diag. Brace at 1/3 points if needed 2x6 DIAGONAL BRACE SPACED 48* O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall HORIZONTAL BRACE (SEE SECTION A-A)

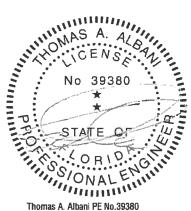
Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

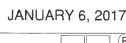
MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date



Standard Gable End Detail

В

L-Bracing Refer

to Section B-B

MII-GE140-001

Page 1 of 2

MiTek USA, Inc.



MiTek USA, Inc. ENGINEERED BY 11/19

DIAGONAL BRACE

4'-0" O.C. MAX

Typical _x4 L-Brace Nailed To Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-E

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA **

3x4 =

24" Max

Vertical Stud (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4\Stuc

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK

Refer to Section A-A

- Diagonal Bracing

NOTE: 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB

OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

5. DIAGONAL BRACE TO BE APPHOXIMATELY 45 DEGREES TO HOOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTIÓN MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

I	1'-3" Max.	(2) - 10d NAILS (2) - 10d NAILS
		Trusses @ 24" o.c.
	Diag. Brace / at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
	End Wall	HORIZONTAL BRACE (SEE SECTION A-A)
		

Roof Sheathing

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade		Maximum Stud Length				
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1,60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No 39380 MiTek USA, Inc. FL Gert 6634 6904 Parke East Blvd. Tampa FL 33610

Standard Gable End Detail

MII-GE170-D-SP

MiTek USA, Inc.

Page 1 of 2

2X6 SP OR SPF No. : DIAGONAL BRACE

2X6 SP OR SPF No. 2

16d Nails

Spaced 6" o.c.

Typical Horizontal Brace

Nailed To 2x4 Verticals w/(4)-10d Nails



MiTek USA, Inc. ENGINEERED BY K

DIAGONAL BRACE

4'-0" O.C. MAX

Typical 2x4 L-Brace Nailed To 2x4 Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 = В

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

2X4 SP OR SPF No. 2

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD. SHEATHING TO 2x4 STD SPF BLOCK

Vertical Stud

(4) - 16d Nails

CONTRACTOR (400°)

SECTION A-A

Roof Sheathing

24" Max

(2) - 10d Nails into 2x6

Diagonal Bracing Refer to Section A-A - L-Bracing Refer to Section B-B

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
- 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

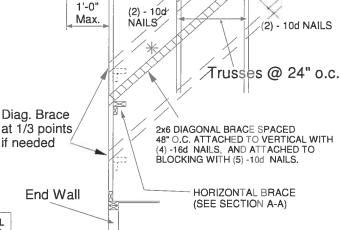
 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT. BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
 9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

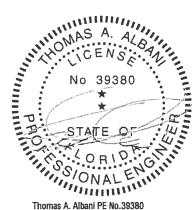
Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade			Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4		
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3		
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13		
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7		
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5		
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

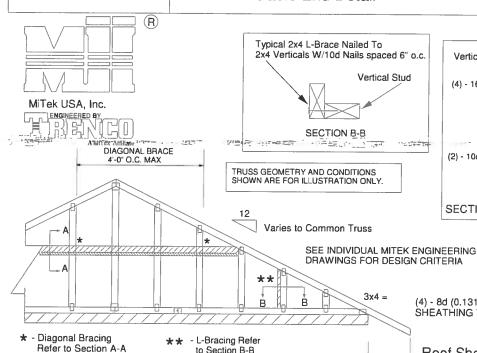
Standard Gable End Detail

MII-GE180-D-SP

Page 1 of 2

(2) - 10d NAILS

MiTek USA, Inc.



to Section B-B

Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A 2X4 SP OR SPF No. 2

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d

NAILS

Roof Sheathing

1'-0"

Max.

24" Max

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3
OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES. 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

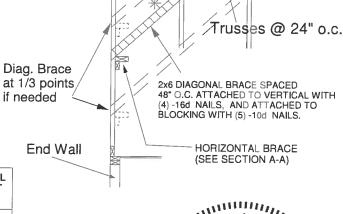
06-01-13 BY SPIB/ALSC. . NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade		Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3		
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7		
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4		
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1		
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7		
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE: 1,60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



No 39380

No 39380

No 39380

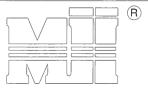
Thomas A. Albani PE No.39380

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-7-10

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY 別尉

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN
- SHALL BE CONNECTED TO EACH PUPLIN
 WITH (2) (0.131" X 3.5") TOE-NAILED.

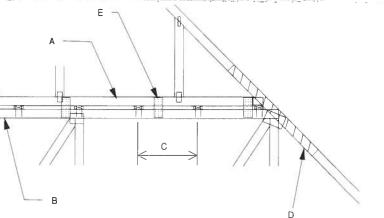
 B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

 C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.

 D 2 X _ X 4+0" SCAB, SIZE TO MATCH TOP CHORD OF
 PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
 ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
 SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
 IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
 DIRECTIONS AND: DIRECTIONS AND:

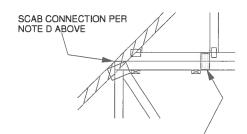
 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

 - 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 it.
- E FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 388 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/(4) (0.131* X 1.5") NAILS PER MEMBER, STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

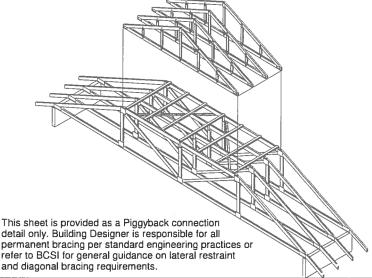


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

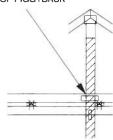
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.

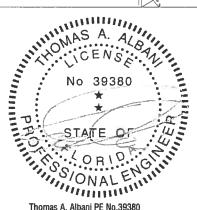


VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
- (MINIMUM 2X4)
 THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
 CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
 BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH
- THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

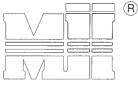
MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C

ASCE 7-10 DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED



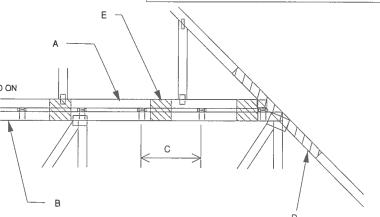
MiTek USA, Inc. ENGINEERED BY

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X _ X 4-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131" X 3.5") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND: 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

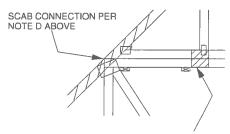
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E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT AB" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

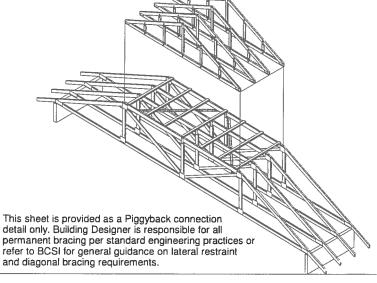


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

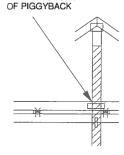
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



 $7^{\rm w}$ x $7^{\rm w}$ x $1/2^{\rm w}$ PLYWOOD (or $7/16^{\rm w}$ OSB) GUSSET EACH SIDE AT 24* O.C. ATTACH WITH 3 - 6d (0.113* X 2*) NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD**



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP

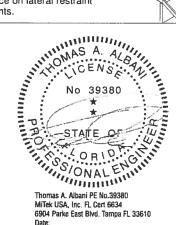
AS SHOWN IN DETAIL.

ATTACH 2 x __ x 4-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)

(MINIMINIO) 2-04 THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc.

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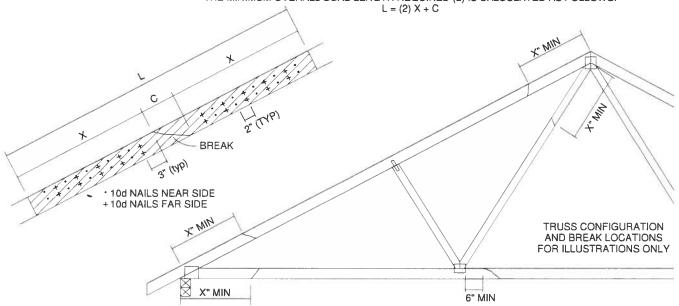
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MiTek USA, Inc.							

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *			MAXIMUM FORCE (lbs) 15% LOAD DURATION							
		X INCHES	SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39"***	The state of the s	2194	3291	2007	3017	1697	2546	1738	~ 2 608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN.
STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

- NOTES:

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES
 NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS
 SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED
 REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
 AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID
 UNUSUAL SPLITTING OF THE WOOD.

 4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID
 LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.

 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

NOTES:

- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
 ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

NEAR SIDE NEAR SIDE

SIDE VIEW (2x3) 2 NAILS

-	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)						
	DIAM.	SP	DF	HF	SPF	SPF-S	
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7	
	.135	93.5	85.6	74.2	72.6	63.4	
	.162	108.8	99.6	86.4	84.5	73.8	
LONG	.128	74.2	67.9	58.9	57.6	50.3	
	.131	75.9	69.5	60.3	59.0	51.1	
25"	.148	81.4	74.5	64.6	63.2	52.5	
က်							

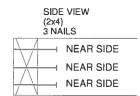
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

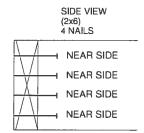
EXAMPLE:

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

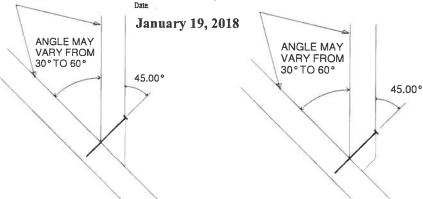
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

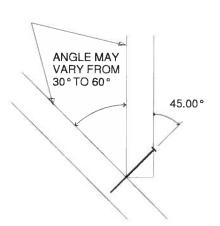






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TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

MiTek USA, Inc.

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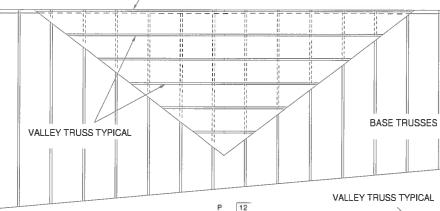


MiTek USA, Inc. ENGINEERED BY

A MITek Affiliate

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS:
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SEE DETAIL A BELOW (TYP.)

GABLE END. COMMON TRUSS OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.

ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS. **DETAIL A** (NO SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C** WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



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TRUSSED VALLEY SET DETAIL AUGUST 1, 2016 MII-VALLEY HIGH WIND2 (R)MiTek USA, Inc. Page 1 of 1 **GENERAL SPECIFICATIONS** 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES. MiTek USA, Inc. 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A GABLE END, COMMON TRUSS ENGINEERED BY OR GIRDER TRUSS HO 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS A MiTok Affiliata 6. NAILING DONE PER NDS-01 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. BASE TRUSSES VALLEY TRUSS TYPICAL GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.) SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C. WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C ATTACH 2x4 CONTINUOUS NO.2 SP WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF TO THE ROOF W/ TWO USP WS45 (1/4" X 4.5") WOOD SCREWS INTO EACH BASE TRUSS. MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

NO 39380

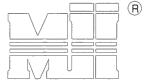
STATE OF ON AL ENGLISH
Thomas A Albani PE No.39380

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

GABLE END, COMMON TRUSS

MiTek USA, Inc.

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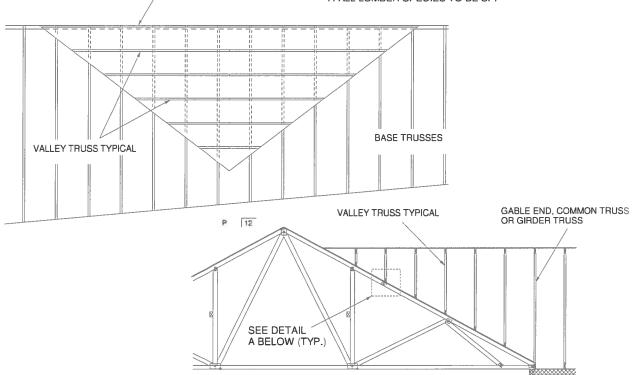
MiTek USA, Inc.

ENGINEERED BY

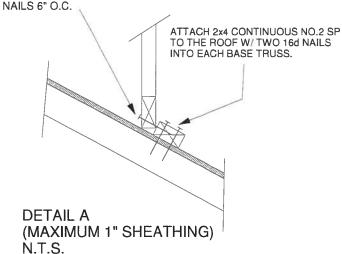
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5")
- 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING BASE TRUSS SHALL BE DESIGNED WITH A FOREING A ROLLEY TRUSS SPACING. OR CIRDER TRUSS 5. NAILING DONE PER NDS - 01

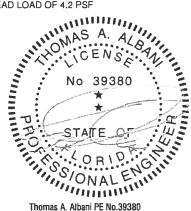
 - 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
 - 7. ALL LUMBER SPECIES TO BE SP.



SECURE VALLEY TRUSS W/ ONE ROW OF 16d



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT \pm 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

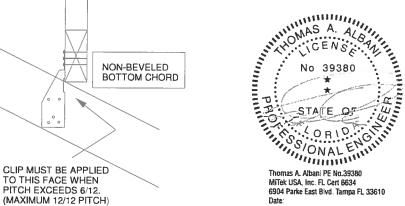


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TRUSSED VALLEY SET DETAIL MII-VALLEY **AUGUST 1, 2016** (HIGH WIND VELOCITY) NOTE: VALLEY STUD SPACING NOT R MiTek USA, Inc. Page 1 of 1 TO EXCEED 48" O.C. SPACING MiTek USA, Inc. ENGINEERED BY A MiTek Attillate FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING NON-BEVELED EXPOSURE B or C BOTTOM CHORD WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF NON-BEVELED BOTTOM CHORD 2'-10" ON AFFECTED TOP CHORDS.

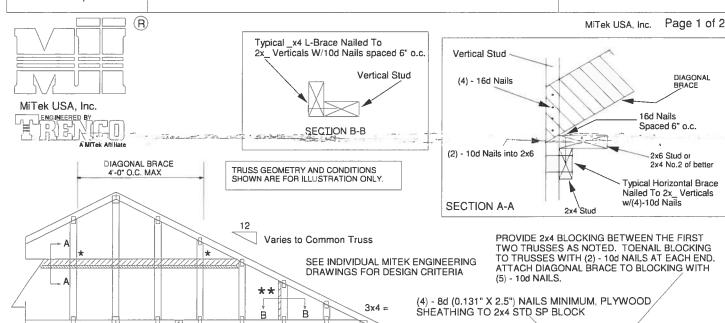
NOTES:

- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.



Standard Gable End Detail

MII-GE146-001



Roof Sheathing

1'-3"

Max.

(2) - 10d

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED

NAILS

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

L-Bracing Refer

to Section B-B

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

* - Diagonal Bracing

Refer to Section A-A

- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGITLES TO THOSE DIAGNAM AT 4'-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
 9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
- TYPE TRUSSES
- 10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
and Grade		Maximum Stud Length					
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10		
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11		
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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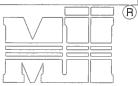
OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

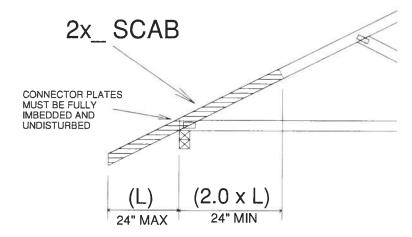
HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

NOTES:

1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVAILABLE SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

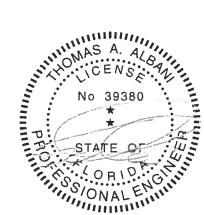


IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date: